

Site-Level Low Impact Development (LID) Design Effective Impervious Calculator

		User input			
		Calculated cells			
***Design Storm: 1-Hour Rain Depth	WQCV Event	0.53	inches		
***Minor Storm: 1-Hour Rain Depth	10-Year Event	1.75	inches		
***Major Storm: 1-Hour Rain Depth	100-Year Event	2.52	inches		
Optional User Defined Storm	CUHP				
(CUHP) NOAA 1 Hour Rainfall Depth and Frequency for User Defined Storm	100-Year Event				
Max intensity for Optional User Defined Storm	0				

Designer: Matt Larson

Company: Classic Consulting Engineers & Surveyors, LLC

Date: January 17, 2020

Project: FOREST LAKES - FILING 5

Location: POND 'D'

Sheet: 1 of 1

Scale: 1" = 100'

North Arrow: N

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CALCULATED RESULTS [OUTPUT]									
Total Calculated Area (ac, check against input)	1.350								
Directly Connected Impervious Area (DCIA, %)	0.0%								
Unconnected Impervious Area (UIA, %)	74.8%								
Receiving Pervious Area (RPA, %)	25.2%								
Separate Pervious Area (SPA, %)	0.0%								
A _R (RPA / UIA)	0.337								
I _p Check	0.750								
f / I for WQCV Event:	2.0								
f / I for 10-Year Event:	0.5								
f / I for 100-Year Event:	0.3								
f / I for Optional User Defined Storm CUHP:									
IRF for WQCV Event:	0.75								
IRF for 10-Year Event:	0.94								
IRF for 100-Year Event:	0.97								
IRF for Optional User Defined Storm CUHP:									
Total Site Imperviousness: I _{TOTAL}	74.8%								
Effective Imperviousness for WQCV Event:	56.2%								
Effective Imperviousness for 10-Year Event:	70.1%								
Effective Imperviousness for 100-Year Event:	72.3%								
Effective Imperviousness for Optional User Defined Storm CUHP:									

LID / EFFECTIVE IMPERVIOUSNESS CREDITS																
WQCV Event CREDIT: Reduce Detention By:	24.9%	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
10-Year Event CREDIT**:	6.5%	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
100-Year Event CREDIT**:	3.1%	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
User Defined CUIP CREDIT: Reduce Detention By:																
Total Site Imperviousness:	74.8%															
Total Site Effective Imperviousness for WQCV Event:	56.2%															
Total Site Effective Imperviousness for 10-Year Event:	70.1%															
Total Site Effective Imperviousness for 100-Year Event:	72.3%															
Total Site Effective Imperviousness for Optional User Defined Storm CUIP:																

Notes:

* Use Green-Ampt average infiltration rate values from Table 3-3.

** Flood control detention volume credits based on empirical equations from Storage Chapter of USDCM.

*** Method assumes that 1-hour rainfall depth is equivalent to 1-hour intensity for calculation purposed

Replace with the SDI worksheet
File can be downloaded from
<https://mapature.digitaldataservices.com/gvh/?viewer=cswdif>

Unresolved. The pdf file upload shall contain both an SDI worksheet for both ponds.

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Basin ID: SAND FILTER - POND D



Required Volume Calculation

Selected BMP Type =	SF	
Watershed Area =	1.35	acres
Watershed Length =	0.430	ft
Watershed Slope =	0.000	ft/ft
Watershed Imperviousness =	74.80%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQV Drain Time =	12.0	hours
Location for 1-hr Rainfall Depth =	Use Input	
Water Quality Capture Volume (WQCV) =	0.027	acre-feet
Excess Urban Runoff Volume (EURV) =	0.111	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.094	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.124	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.155	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	0.192	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	0.220	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	0.257	acre-feet
500-yr Runoff Volume (P1 = 3.1 in.) =	0.330	acre-feet
Approximate 2-yr Detention Volume =	0.088	acre-feet
Approximate 5-yr Detention Volume =	0.116	acre-feet
Approximate 10-yr Detention Volume =	0.145	acre-feet
Approximate 25-yr Detention Volume =	0.156	acre-feet
Approximate 50-yr Detention Volume =	0.162	acre-feet
Approximate 100-yr Detention Volume =	0.172	acre-feet

Optional User Override 1-hr Precipitation	
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.10	inches

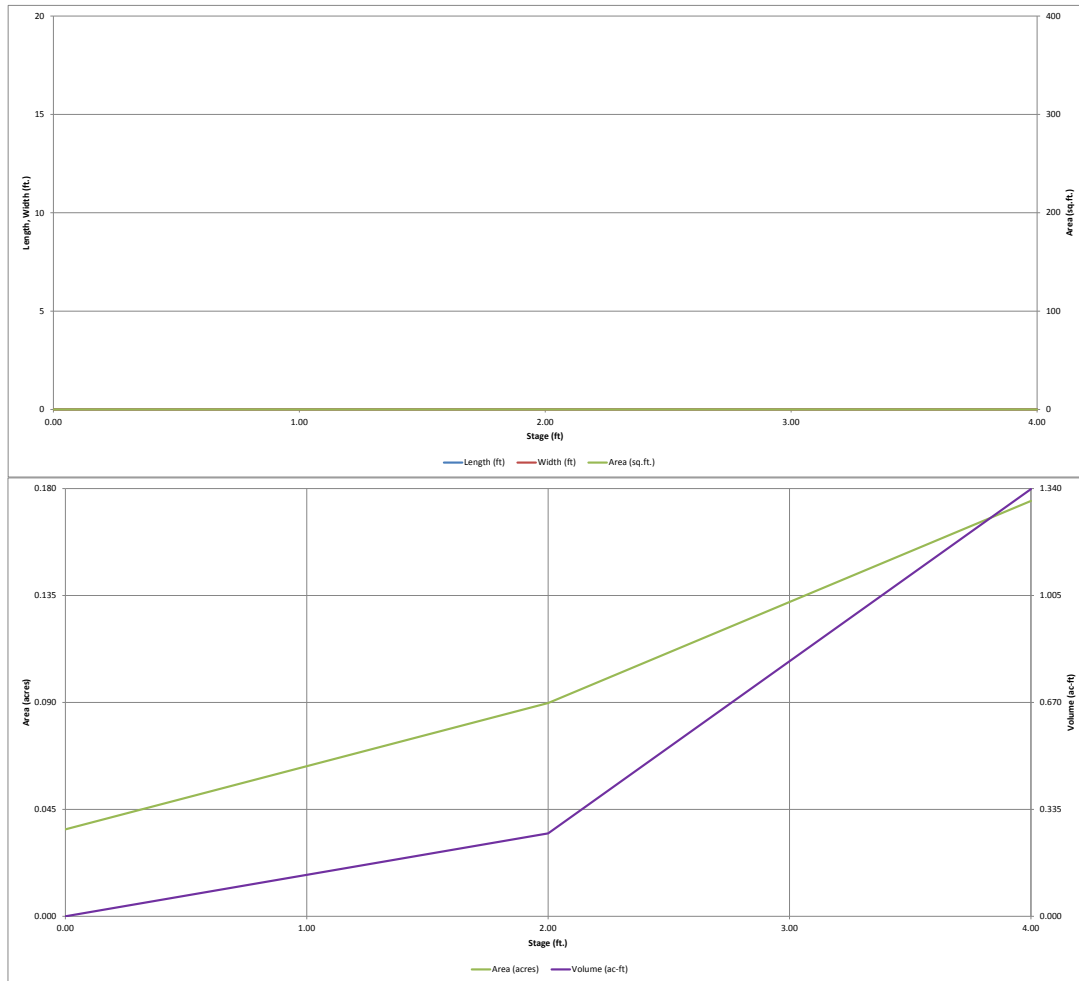
Stage-Storage Calculation

Zone 1 Volume (V_{WC1}) =	0.027	acre-feet
Zone 2 Volume (E_{URV} - Zone 1) =	0.085	acre-feet
Zone 3 Volume (100-ton - Zones 1 & 2) =	0.061	acre-feet
Total Detention Basin Volume =	0.172	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft ³
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H_{DAV}) =	N/A	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	N/A	ft/ft
Slopes of Main Basin Sides (S_{MBW}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{IS}) =	user	ft ²
Surcharge Volume Length (L_{SV}) =	user	ft
Surcharge Volume Width (W_{SV}) =	user	ft
Depth of Basin Floor (H_{f100A}) =	user	ft
Length of Basin Floor (W_{f100A}) =	user	ft
Width of Basin Floor (W_{f100A}) =	user	ft
Area of Basin Floor (A_{f100A}) =	user	ft ²
Volume of Basin Floor (V_{f100A}) =	user	ft ³
Depth of Main Basin (H_{MBW}) =	user	ft
Length of Main Basin (L_{MBW}) =	user	ft
Width of Main Basin (W_{MBW}) =	user	ft
Area of Main Basin (A_{MBW}) =	user	ft ²
Volume of Main Basin (V_{MBW}) =	user	ft ³
Calculated Total Basin Volume (V_{MBW}) =	user	acre-feet

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DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

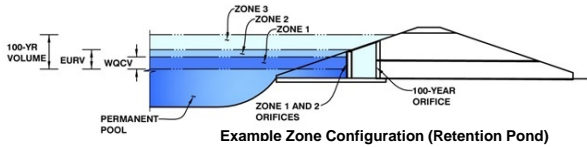


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Forest Lakes Filing 5

Basin ID: SAND FILTER - POND D



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.94	0.027	Filtration Media
Zone 2 (EURV)	1.47	0.085	Weir&Pipe (Restrict)
Zone 3 (100-year)	1.71	0.061	Weir&Pipe (Restrict)
		0.172	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	2.30	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	0.42	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	0.0	ft ²
Underdrain Orifice Centroid =	0.02	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft ²
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 2 Weir	Zone 3 Weir	
Overflow Weir Front Edge Height, H _o =	1.10	1.50	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	1.00	4.00	feet
Overflow Weir Slope =	0.00	4.00	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	0.50	4.00	feet
Overflow Grate Open Area % =	100%	60%	% grate open area/total area
Debris Clogging % =	50%	50%	%

Calculated Parameters for Overflow Weir

	Zone 2 Weir	Zone 3 Weir	
Height of Grate Upper Edge, H _u =	1.10	2.50	feet
Over Flow Weir Slope Length =	0.50	4.12	feet
Grate Open Area / 100-yr Orifice Area =	5.37	106.18	should be ≥ 4
Overflow Grate Open Area w/o Debris =	0.50	9.90	ft ²
Overflow Grate Open Area w/ Debris =	0.25	4.95	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Zone 3 Restrictor	
Depth to Invert of Outlet Pipe =	2.30	2.30	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	8.00	8.00	inches
Restrictor Plate Height Above Pipe Invert =	2.50	2.50	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 2 Restrictor	Zone 3 Restrictor	
Outlet Orifice Area =	0.09	0.09	ft ²
Outlet Orifice Centroid =	0.12	0.12	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.19	1.19	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	2.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	5.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

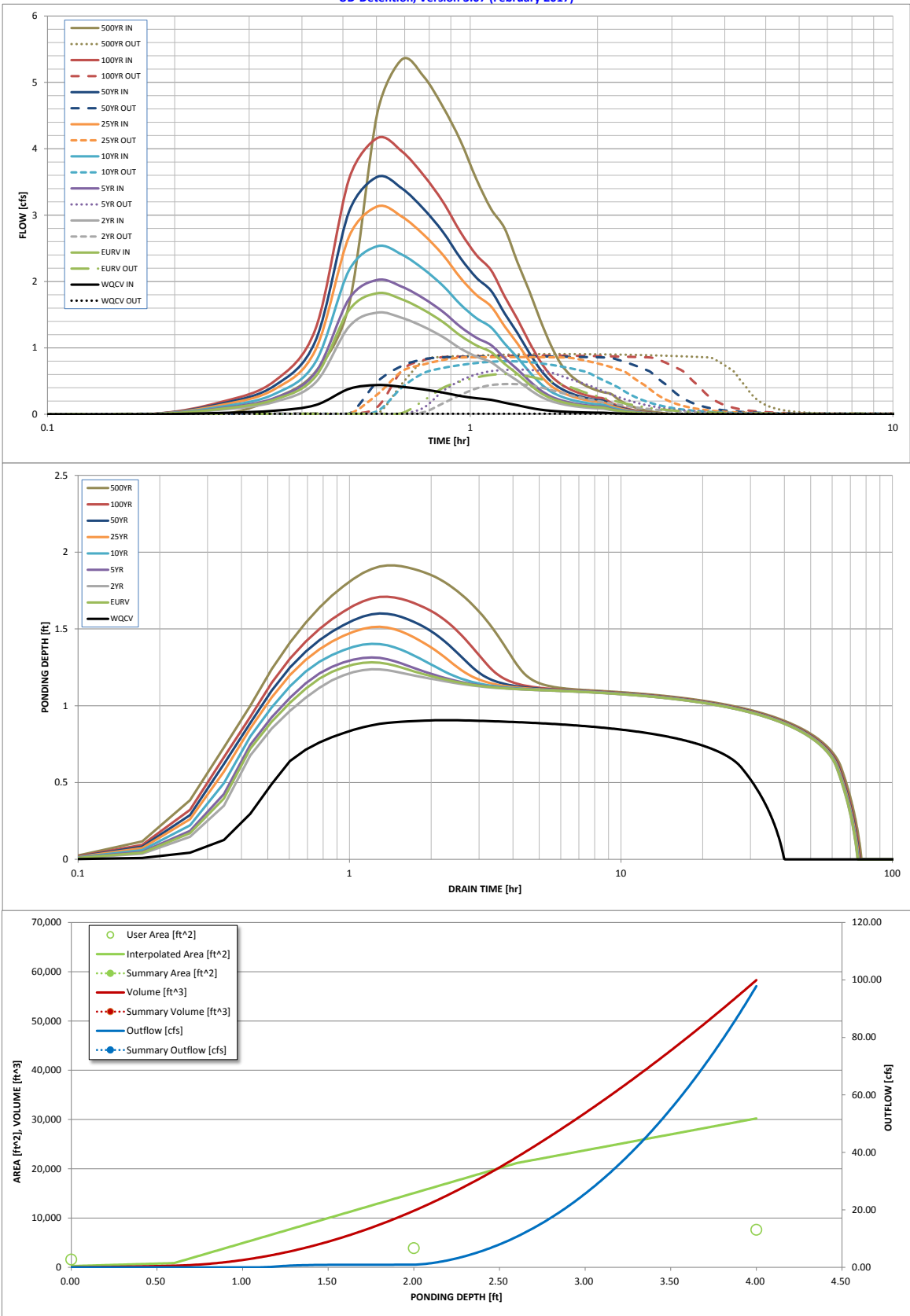
Spillway Design Flow Depth=	0.35	feet
Stage at Top of Freeboard =	3.35	feet
Basin Area at Top of Freeboard =	0.60	acres

Routed Hydrograph Results

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.10
Calculated Runoff Volume (acre-ft) =	0.027	0.111	0.094	0.124	0.155	0.192	0.220	0.257	0.330
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.026	0.111	0.093	0.123	0.155	0.192	0.220	0.256	0.330
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.19	0.64	0.88	1.19	1.73
Predevelopment Peak 1 (cfs) =	0.0	0.0	0.0	0.027	0.3	0.9	1.2	1.6	2.3
Peak Inflow Q (cfs) =	0.4	1.8	1.5	2.0	2.5	3.1	3.6	4.2	5.3
Peak Outflow Q (cfs) =	0.0	0.6	0.5	0.673	0.8	0.9	0.9	0.9	0.9
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	25.1	3.1	1.0	0.7	0.6	0.4
Structure Controlling Flow =	Filtration Media	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Grate 1 (fps) =	N/A	1.10	0.85	1.3	1.5	1.7	1.7	1.7	1.8
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	0.0	0.0	0.0	0.0
Time to Drain 97% of Inflow Volume (hours) =	39	69	70	69	67	66	65	64	61
Time to Drain 99% of Inflow Volume (hours) =	40	73	73	73	72	72	72	72	72
Maximum Ponding Depth (ft) =	0.91	1.28	1.24	1.31	1.40	1.51	1.60	1.71	1.91
Area at Maximum Ponding Depth (acres) =	0.09	0.18	0.17	0.19	0.21	0.23	0.25	0.28	0.32
Maximum Volume Stored (acre-ft) =	0.024	0.075	0.066	0.080	0.098	0.122	0.144	0.173	0.233

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

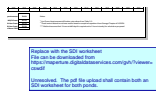
The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

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SDI V_2 redlines.pdf Markup Summary

dsdlaforce (1)



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