# FINAL DRAINAGE REPORT

#### for LDS CHURCH – PARKING ADDITION

950 Hwy 150 Monument, Colorado

#### July 2024

#### PCD File No.:PPR2419

Prepared for:

#### LDS Church – Real Estate Division

50 E North Temple #509-8866 Salt Lake City, UT 84150 Contact: Scott Hollister

Prepared by:

#### Drexel, Barrell & Co.

101 S. Sahwatch St. #100 Colorado Springs, CO 80903 Contact: Tim McConnell, P.E. (719) 260-0887

21841-00CSCV

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#### **1.0 CERTIFICATION STATEMENTS**

#### **ENGINEER'S STATEMENT**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports, and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omission on my part in preparing this report.

- O Malle Tim D. McConnell, P.E. Date 33797 Colorado P.E. License No. 33797 For and on Behalf of Drexel, Barrell & C amo

#### **DEVELOPER'S STATEMENT**

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

Business Name:

LDS Church - Real Estate Division

By:

Scott J. Hollister 8/2/2024 Scott Hollister Date

Title: Address:

50 E. North Temple #509-8866 Salt Lake City, UT 84150

#### EL PASO COUNTY

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Criteria Manual, as amended.

Joshua Palmer, P.E. County Engineer / ECM Administrator Date

Conditions:

### 2.0 PURPOSE

This report is prepared by Drexel, Barrel & Co in support of the LDS Church – Parking Addition project. The purpose of this report is to identify onsite and offsite drainage patterns, storm sewer, inlet locations, and areas tributary to the site, and to safely route developed storm water runoff to adequate outfall facilities.

### 3.0 GENERAL SITE DESCRIPTION

#### <u>Location</u>

The LDS Church – Parking Addition project is located in Monument, El Paso County, Colorado, within the SW 1/4 of the NW 1/4 of Section 13, Township 11 S, Range 67 W of the 6th P.M. The site is bounded on the west by the existing LDS church site, to the north and east by undeveloped land owned by El Paso County and to the south by Hwy 105.

#### Site Conditions

The site is approximately 1 acre in size, with 0.58 acres being disturbed. It is currently undeveloped and is covered with native grass and vegetation. The site generally slopes from east to west. It is proposed to be developed as an additional parking lot for the LDS church to the west of this project site.

#### <u>Soils</u>

According to the Soil Survey of El Paso County Area, Colorado, prepared by the U.S. Department of Agriculture Soil Conservation Service, the site is partially underlain by the Alamosa loam (Soil No. 1), and by the Tomah-Crowfoot loamy sands (Soil No. 92). The soils are type 'D' and type 'B' hydrologic soil groups, respectively. See appendix for map.

#### <u>Climate</u>

This area of El Paso County can be described as the foothills, with total precipitation amounts typical of a semi-arid region. Winters are generally cold and dry, and summers relatively warm and dry. Precipitation ranges from 12 to 14 inches per year, with the majority of this moisture occurring in the spring and summer in the form of rainfall. Thunderstorms are common during the summer months.

#### Floodplain Statement

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panels 08041C0276G, 08041C0278G and 08041C0279G (December 7, 2018) no portion of the site lies within any flood zones.

### 4.0 DRAINAGE CRITERIA

The drainage analysis has been prepared in accordance with the current El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities during the 5 year and 100 year frequency storms for historic and developed conditions using the Rational Method as required for basins containing less than 100 acres.

### 5.0 EXISTING CONDITION

The existing site is undeveloped and covered with native vegetation that consists mostly of grasses as well as some shrubs. The site generally follows a 1-42% grade from east to west. The flows that leave the site to the north and are carried to the Dirty Woman Creek. The flows that leave the site to the south are carried along Hwy 105 to the west approximately half a mile to existing Pond 115. This pond outfalls into Dirty Woman Creek.

The Rational Method was used to determine runoff quantities for the 5- and 100-year storm events. See below for a summary runoff table. See Appendix for Historic Condition Drainage Map.

BASIN	DP	AREA (AC)	% IMPERV	Q5 (cfs)	Q100 (cfs)
А	1	0.62	0%	0.6	3.4
В	2	0.52	4%	0.2	1.2

#### **Rational Method Runoff Summary**

### 6.0 DEVELOPED CONDITION

The proposed site consists of an asphalt parking lot with 2 access points off of the existing parking lot. Flows from the parking lot (Basin A) are captured by a proposed private sump 5' Type R inlet in the southwest corner of the parking lot, where they are then carried to the at-grade 5' Type R inlet along Hwy 105 via a proposed private 18" RCP pipe. The 5' inlet in the parking lot and the 18" RCP pipe will be privately owned and maintained. The portion of the site north of the proposed parking lot (Basin C) remains mostly undisturbed and will continue to flow north to Dirty Woman Creek in historical patterns. The portion of the site south of the proposed parking lot (Basin B) remains mostly undisturbed and will continue to flow south to Hwy 105 in historical patterns. Flows from Basins B & C continue west along Hwy 105 via the storm system approximately half a mile to existing Pond 115. This pond outfalls into Dirty Woman Creek. The El Paso County project manager for the Hwy 105 project and the contracted engineer have confirmed that the existing pond can handle the additional 0.4 acres of impervious area that this proposed parking lot project will produce. See email included in Appendix.

The Rational Method was used to determine runoff quantities for the 5- and 100-year storm

recurrence intervals. UD-Inlet and HydraFlow were also used to identify storm system sizing (see appendix for calculations), and below for a summary runoff table.

BASIN	DP	AREA (AC)	% IMPERV	Q5 (cfs)	Q100 (cfs)
А	1	0.52	85%	2.1	3.9
В	2	0.17	0%	0.1	0.5
С	3	0.45	0%	0.3	1.7

#### **Rational Method Runoff Summary**

#### 7.0 PROPOSED DETENTION/WATER QUALITY FACILITIES

There is no proposed on-site detention for this project. The flows from the parking lot will be captured by the Hwy 105 storm system and carried to the existing Pond 115. The El Paso County project manager for the Hwy 105 project and the contracted engineer have confirmed that the existing pond can handle the additional 0.4 acres of impervious area that this proposed parking lot project will produce. See email included in Appendix.

### 8.0 FOUR-STEP PROCESS

This project conforms to the El Paso County Four Step Process. The process for this site focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls.

- 1. **Employ Runoff Reduction Practices:** Proposed impervious areas on this site (roofs, asphalt/sidewalk) will sheet flow across landscaped area, gravel and natural grasses in an effort to slow runoff and increase time of concentration prior to entering Dirty Woman Creek. This will minimize directly connected impervious areas within the project site.
- 2. Implement BMP's that provide a Water Quality Capture Volume with slow release: The total disturbed area of the site is 0.58 acres. There is no proposed on-site detention for this project. The flows from the parking lot will be captured by the Hwy 105 storm system and carried to the existing Pond 115. The El Paso County project manager for the Hwy 105 project and the contracted engineer have confirmed that the existing pond can handle the additional 0.4 acres of impervious area that this proposed parking lot project will produce. See email included in Appendix.
- 3. **Stabilize Drainage Ways:** Dirty Woman Creek will not require any stabilization to occur due to the runoff from this site. The area of the project site that was previously running into the creek in the existing condition, but is now proposed to

be developed, is being captured in the proposed storm system and connecting to the storm system in Hwy 105 and carried west to existing Pond 115. The El Paso County project manager for the Hwy 105 project and the contracted engineer have confirmed that the existing pond can handle the additional 0.4 acres of impervious area that this proposed parking lot project will produce. See email included in Appendix. The proposed flows into Dirty Woman Creek are now less than in the existing condition. The creek is in acceptable condition and is able to convey the flows without impact to downstream facilities.

4. **Implement Site Specific and Other Source Control BMP's:** Standard commercial source control will be utilized in order to minimize potential pollutants entering the creek. Example source control measures consist of: indoor storage of chemicals; and trash receptacles in common areas.

#### 9.0 DRAINAGE/BRIDGE FEES

The project lies within the Dirty Woman Drainage Basin, and has been previously platted, therefore no drainage and bridge fees are due.

#### **10.0 CONSTRUCTION COST ESTIMATE**

Description	Quantity	Unit Cost	Cost_
5' Type R Inlet	1 EA	\$7,212/EA	\$7,212
18" RCP storm	38 LF	\$82/LF	\$3,116
		Subtotal	\$10,328
	Engineering	& Contingency (10%)	<u>\$1,033</u>
		TOTAL	\$11,361

### **11.0 CONCLUSIONS**

The LDS Church – Parking Addition project has been designed in accordance with El Paso County criteria. The flows leaving this site to the north and into Dirty Woman Creek decrease by 0.3 cfs in the 5-yr storm event and by 1.7 cfs in the 100-yr storm event. The flows leaving the site to the south towards Hwy 105 increase by 2.0 cfs in the 5-yr storm event and by 3.2 cfs in the 100-yr storm event. This increase in flows is due to increased impervious area from the existing condition to the proposed condition. The flows from the parking lot will be captured by the Hwy 105 storm system and carried to the existing Pond 115. The El Paso County project manager for the Hwy 105 project and the contracted engineer have confirmed that the existing pond can handle the additional 0.4 acres of impervious area that this proposed parking lot project will produce. See email included in Appendix. This development will not negatively impact the downstream facilities.

### **12.0 REFERENCES**

The sources of information used in the development of this study are listed below:

- 1. El Paso County Drainage Criteria Manual, October 2018.
- 2. El Paso County Engineering Criteria Manual, October 2020.
- 3. Urban Storm Drainage Criteria Manuals, Urban Drainage and Flood Control District. June 2001, Revised April 2008.
- 4. Natural Resources Conservation Service (NRCS) Web Soil Survey
- 5. Federal Emergency Management Agency, Flood Insurance Rate Map, El Paso County, Colorado and Unincorporated Areas, Map Number 8041CO575F, Effective Date March 17, 1997.
- 6. Final Drainage Report Highway 105 Project A, by HDR, April 7, 2023

APPENDIX





USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





# Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	D	1.1	73.3%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	В	0.4	26.7%
Totals for Area of Intere	est	1.5	100.0%	

# Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

# **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



# National Flood Hazard Layer FIRMette



#### Legend

#### 104°50'58"W 39°5'52"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - - - - Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LIIII Levee, Dike, or Floodwall 70074GEET 20.2 Cross Sections with 1% Annual Chance 7112 FEET 17.5 Water Surface Elevation L PASO COUNTY **Coastal Transect** Zone/FFLOOT//AY Base Flood Elevation Line (BFE) 30059 Limit of Study T115 R07W S013 Jurisdiction Boundary **Coastal Transect Baseline** OTHER **Profile Baseline** FEATURES Hydrographic Feature **Digital Data Available** PROJECT SITE No Digital Data Available MAP PANELS Unmapped AREA OF MINIMAL FLOOD HAZARD The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/18/2024 at 1:06 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 104°50'21"W 39°5'24"N Feet 1:6,000 unmapped and unmodernized areas cannot be used for regulatory purposes. 250 500 1,000 1,500 2,000 n

Basemap Imagery Source: USGS National Map 2023

PROJECT INF	ORMATION						0
PROJECT:	LDS parking lot						
PROJECT NO:	21841-00						
DESIGN BY:	SBN					Drexel, B	arrell & Co.
REV. BY:	TDM						
AGENCY:	El Paso County						
REPORT TYPE:	Final						
DATE:	5/9/2024						
Soil Type: B & D							
			C5* (Type B)	C5* (Type D)	C100* (Type B)	C100* (Type D)	% IMPERV
Pasture/Meadow			0.08	0.15	0.35	0.50	0
Asphalt/Sidewalk			0.90	0.90	0.96	0.96	100
*C-Values and Basin Imper	viousness based on Table 6-	6, City of Colorado Spri	ings "Drainage Criteria N	lanual"			
EXISTING							
SUB-BASIN	SURFACE DESIGNAT	TION AREA	CC	OMPOSITE RUN	NOFF COEFFICIE	INTS	% IMPERV
		ACRE	C5 (Type B)	C5 (Type D)	C100* (Type B)	C100* (Type D)	
Α	Pasture/Meadow	0.00	0.08		0.35		0
	Pasture/Meadow	0.62		0.15		0.50	0
	Asphalt/Sidewalk		0.90		0.96		100
	Asphalt/Sidewalk	0.00		0.90		0.96	100
	WEIGHTED AVERAG	E		0.15		0.50	0%
TOTAL A		0.62					
В	Pasture/Meadow	0.28	0.08		0.35		0
	Pasture/Meadow	0.22		0.15		0.50	0
	Asphalt/Sidewalk	0.02	0.90		0.96		100
	Asphalt/Sidewalk	0.00		0.90		0.96	100
	WEIGHTED AVERAG	E		0.11		0.40	4%
TOTAL B		0.52					
TOTAL SITE		1.14		0.13		0.45	1.8%

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REPORT TYPE:	Final
DATE:	5/9/2024



#### RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF EXISTING TIME OF CONCENTRATION STANDARD FORM SF-2

SUB-BASIN				INITIAL/OVERLAND			TRAVEL TIME				TIME OF CONC.		FINAL			
		DATA			TIME (t <sub>i</sub> )				(t <sub>t</sub> )				t <sub>c</sub>		t <sub>c</sub>	
BASIN	DESIGN PT:	C <sub>5</sub>	C <sub>100</sub>	AREA	LENGTH	HT	SLOPE	ti	LENGTH	HT	SLOPE	VEL.	t	COMP.	MINIMUM	
				Ac	Ft	FT	%	Min	Ft	FT	%	FPS	Min	t <sub>c</sub>	tc	Min
А	1	0.15	0.50	0.62	145	13	9.0	10.3						10.3	5	10.3
В	2	0.11	0.40	0.52	155	6	3.9	14.7						14.7	5	14.7

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#### RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

EXISTING	RUNOFF	5	SYR STORI	N		P1=	1.50
			DIRECT RUNG	DFF			
BASIN (S)	DESIGN POINT	AREA (AC)	RUNOFF COEFF		C * A	I (IN/HR)	Q (CFS)
A	1	0.62	0.15	10.3	0.09	4.09	0.6
В	2	0.52	0.11	14.7	0.06	3.55	0.2



Drexel, Barrell & Co.

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#### RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

EXISTING	RUNOFF	10	0 YR STOF	RM		P1=	2.52
			DIRECT RUNC	DFF			
BASIN (S)	DESIGN POINT	AREA (AC)	RUNOFF COEFF	t <sub>c</sub> (MIN)	C * A	I (IN/HR)	Q (CFS)
A	1	0.62	0.50	10.3	0.31	6.86	3.4
В	2	0.52	0.40	14.7	0.21	5.96	1.2



Drexel, Barrell & Co.

<b>PROJECT INF</b>	ORMATION	N						
PROJECT:	LDS parking	g lot						
PROJECT NO:	21841-00	, ,						
DESIGN BY:	SBN						Drexel, Ba	rrell & Co.
REV. BY:	TDM							
AGENCY:	El Paso Col	unty						
REPORT TYPE:	Final							
DATE:	5/9/2024							
Soil Type: B & D								
				C5* (Type B)	C5* (Type D)	C100* (Type B)	C100* (Type D)	% IMPERV
Pasture/Meadow				0.08	0.15	0.35	0.50	0
Asphalt/Sidewalk	Asphalt/Sidewalk			0.90	0.90	0.96	0.96	100
*C-Values and Basin Imper	rviousness based o	n Table 6-6, City o	f Colorado Springs	"Drainage Criteria I	Manual"			
PROPOSED								
SUB-BASIN	SURFACE DE	SIGNATION	AREA	C	OMPOSITE RU	NOFF COEFFICIE	NTS	% IMPERV
			ACRE	C5 (Type B)	C5 (Type D)	C100* (Type B)	C100* (Type D)	
Α	Pasture/Mead	OW	0.06	0.08		0.35		0
	Pasture/Mead	OW	0.02		0.15		0.50	0
	Asphalt/Sidew	alk	0.14	0.90		0.96		100
	Asphalt/Sidew	alk	0.30		0.90		0.96	100
	WEIGHTED A	VERAGE			0.78		0.87	85%
TOTAL A			0.52					
В	Pasture/Mead	ow	0.09	0.08		0.35		0
	Pasture/Mead	OW	0.08		0.15		0.50	0
	Asphalt/Sidew	alk	0.00	0.90		0.96		100
	Asphalt/Sidew	alk	0.00		0.90		0.96	100
	WEIGHTED A	VERAGE			0.11		0.42	0%
TOTAL B			0.17					
C	Pasture/Mead	OW	0.01	0.08		0.35		0
	Pasture/Mead	ow	0.44		0.15		0.50	0
	Asphalt/Sidew	alk	0.00	0.90		0.96		100
	Asphalt/Sidew	alk	0.00		0.90		0.96	100
	WEIGHTED A	VERAGE	0.45		0.15		0.50	0%
TUTALC			0.45					
TOTAL					0.40		0.00	00.00/
IUIAL			1.14		0.43		0.66	38.6%

 PROJECT:
 LDS parking lot

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 El Paso County

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 Final

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#### RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF PROPOSED TIME OF CONCENTRATION STANDARD FORM SF-2

SUB-BASIN INITIAL/OVERLAND							)	TRAVEL TIME				PIPE TRAVEL TIME				TIME OF CONC.		FINAL		
	DATA TIME (t <sub>i</sub> )					(t <sub>t</sub> )				(t <sub>p</sub> )				tc		tc				
BASIN	DESIGN PT:	C <sub>5</sub>	C <sub>100</sub>	AREA	LENGTH	HT	SLOPE	ti	LENGTH	HT	SLOPE	VEL.	tt	LENGTH	SLOPE	VEL.	tt	COMP.	MINIMUM	
				Ac	Ft	FT	%	Min	Ft	FT	%	FPS	Min	Ft	%	FPS	Min	t <sub>c</sub>	t <sub>c</sub>	Min
А	1	0.78	0.87	0.52	100	3	3.0	4.2	175	5	2.9	10.0	0.3					4.5	5	5.0
В	2	0.11	0.42	0.17	40	1	2.5	8.6										8.6	5	8.6
С	3	0.15	0.50	0.45	85	8	9.4	7.8										7.8	5	7.8

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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

	• • • • • • • • • •						
PROPOSED	RUNOFF	5	SYR STOR		P1=	1.50	
			DIRECT RUNC	FF			
BASIN (S)	DESIGN POINT	AREA (AC)	RUNOFF COEFF	t <sub>c</sub> (MIN)	C * A	I (IN/HR)	Q (CFS)
A	1	0.52	0.78	5.0	0.40	5.17	2.1
В	2	0.17	0.11	8.6	0.02	4.36	0.1
С	3	0.45	0.15	7.8	0.07	4.51	0.3

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Drexel, Barrell & Co.

#### RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

PROPOSED	RUNOFF	10	0 YR STOF	RM		2.52				
			DIRECT RUNC	DFF				PIPE SIZING		G
BASIN (S)	DESIGN POINT	AREA (AC)	RUNOFF COEFF	t <sub>c</sub> (MIN)	C * A	I (IN/HR)	Q (CFS)	n	Slope (ft/ft)	Calculated Pipe Dia (in)
A	1	0.52	0.87	5.0	0.45	8.68	3.9	0.013		
B	2	0.17	0.42	8.6	0.07	7.31	0.5			
C	3	0.45	0.50	7.8	0.22	7.57	1.7			

Inlets



Figure 8-11. Inlet Capacity Chart Sump Conditions , Curb Opening (Type R) Inlet

Notes:

1. The standard inlet parameters must apply to use this chart.

8-16

City of Colorado Springs Drainage Criteria Manual, Volume 1 May 2014





Line No.	Flow Rate	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Up	HGL Dn	Minor Loss	HGL Jnct	Vel Ave	J-Loss Coeff	
	(cfs)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)	(ft/s)		
1	8.34	24	Cir	272.072	7113.29	7116.96	1.35	7117.99 j	7114.89	n/a	7117.99	4.11	0.84 z	
2	2.10	18	Cir	38.474	7117.32	7119.00	4.37	7119.55 j	7117.99	n/a	7119.55	3.18	1.00 z	
	Not	es: j-Li	ine cont	ains hyd. j	ump; z-Zer	o Junction	Loss							



# 100-yr

Line No.	Flow Rate	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Up	HGL Dn	Minor Loss	HGL Jnct	Vel Ave	J-Loss Coeff		
	(cfs)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)	(ft/s)			
1	18.30	24	Cir	272.072	7113.29	7116.96	1.35	7118.50 j	7115.29	n/a	7118.50	6.44	0.84 z		
2	3.90	18	Cir	38.474	7117.32	7119.00	4.37	7119.75 j	7118.50	n/a	7119.75	3.50	1.00 z		
	Note	es: j-Lii	ne conta	ains hyd. ju	imp; z-Zero	Junction	LOSS								

From: Alissa Werre <<u>AlissaWerre@elpasoco.com</u>> Sent: Wednesday, July 24, 2024 12:56 PM To: Tim McConnell <<u>tmcconnell@drexelbarrell.com</u>> Cc: Jeff Rice <<u>JeffRice@elpasoco.com</u>> Subject: [EXTERNAL]FW: Hwy 105 Project - LDS Church parking lot

Tim, Please see below.

Thank you,



Alissa Werre, PE Principal Engineer / Project Manager Department of Public Works 719.238.8080 AlissaWerre@elpasoco.com https://publicworks.elpasoco.com

From: Staten, Liz <<u>liz.staten@hdrinc.com</u>> Sent: Wednesday, July 24, 2024 10:31 AM To: Alissa Werre <<u>AlissaWerre@elpasoco.com</u>> Cc: Smith, Jamey <<u>Jamey.Smith@hdrinc.com</u>>; Wilkert, Joshua <<u>Joshua.Wilkert@hdrinc.com</u>>; Beasley, Cory <<u>cory.beasley@hdrinc.com</u>> Subject: RE: Hwy 105 Project - LDS Church parking lot

Hello Alissa,

I had to recalculate pond design to add in the additional impervious area. The 0.4 acres impervious added to the 17.52 acres currently going into the pond will stay below the spillway – as in we do have volume for it.

The 100-year WSE in the pond will be 6.69 ft deep. Current design is 6.67 ft The as constructed spillway is at 6.75 feet. These calculations include the adjacent future Maguire development

The 100-year release rate is 24.4 and is unchanged.

Liz Staten, PE (CO,NM) D 719.272.8839 M 719.291.7102

hdrinc.com/follow-us



	PREPARED BY:
↓-	DREXEL, BARRELL & CO. Engineers. Surveyors 101 SANWATCH ST. #100 COLORADO SPGS. COLORADO 80903 CONTACT: TIM D. McCONNELL, P.E. (719)260-0887 COLORADO SPRINGS. LAFAYETTE CLIENT: LDS CHURCH-REAL ESTATE
30 60 : 1"=30'	50 E NORTH TEMPLE #509-8866
AREA (AC)         % IMPERV         Q5 (cfs)         Q100 (cfs)           0.62         0%         0.6         3.4           0.52         4%         0.2         1.2	DRAINAGE PLANS FOR: LDS CHURCH PARKING ADDITION MONUMENT, COLORADO
	ISSUE DATE INITIAL ISSUE 1-11-2024
	DESIGNED BY: SBN DRAWN BY: SBN CHECKED BY: TDM FILE NAME: 21841-00 EXDR
	PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO. DRAWING SCALE: HORIZONTAL: 1"=30' VERTICAL: N/A
	EXISTING CONDITIONS DRAINAGE MAP
Know what's <b>below.</b>	PROJECT NO. 21841-00CSCV DRAWING NO.
CALL 3-BUSINESS DAYS IN ADVANCE DEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.	EX



BASIN	DP	AREA (AC)	% IMPERV	Q5 (cfs)	Q100 (cfs)
A	1	0.62	0%	0.6	3.4
В	2	0.52	4%	0.2	1.2



	DREXEL, BARRELL & CO Engineers Surveyors 101 SAHWATCH ST, #100 COLORADO SPGS, COLORADO 899 CONTACT: TM D. MCCONNELL, P. COLORADO SPRINGS • LAFAVETTE CLIENT: LDS CHURCH-REAL ESTATE DIVISION 50 E NORTH TEMPLE #509-8866 SALT LAKE CITY, UT 84150				
00 (cfs) 3.9 0.5 1.7	DRAINAGE PLANS FOR: LDS CHURCH PARKING ADDITION	MONUMENT, COLORADO			
	ISSUE INITIAL ISSUE LATEST ISSUE DESIGNED BY: DRAWN BY: CHECKED BY: FILE NAME: 2184	DATE 5/21/24 7/25/24 			
g.	PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO. DRAWING SCALE: HORIZONTAL: 1"=30' VERTICAL: N/A PROPOSED CONDITIONS DRAINAGE MAP PROJECT NO. 21841-00CSCV DRAWING NO.				
F S.	SHEET: 2	OF 2			



BASIN	DP	AREA (AC)	% IMPERV	Q5 (cfs)	Q100 (cfs)
A	1	0.52	85%	2.1	3.9
В	2	0.17	0%	0.1	0.5
С	3	0.45	0%	0.3	1.7

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Know what's below. Call before you dig

CALL 3-BUSINESS DAYS IN ADVA BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES