FINAL DRAINAGE REPORT

FOR

LOT 3 PADMARK BUSINESS PARK FILING NO. 1

(LOT 44 OF CLAREMONT BUSINESS PARK FILING NO. 2) EL PASO COUNTY, COLORADO

JUNE 2018

Prepared for:

Hammers Construction, Inc. 1411 Woolsey Heights Colorado Springs, CO 80915

Prepared by:



CIVIL CONSULTANTS, INC.

20 Boulder Crescent, Suite 110 Colorado Springs, CO 80903 (719) 955-5485

Project #44-031 PCD Project #PPR-18-000 PPR-18-034

FINAL DRAINAGE REPORT FOR LOT 3 PADMARK BUSINESS PARK FILING NO. 1 (Lot 44 of Claremont Business Park Filing No. 2)

DRAINAGE PLAN STATEMENTS

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Virgil A. Sanchez, P.E. #37160 For and on Behalf of M&S Civil Consultants, Inc

DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY:

TITLE:	
DATE:	

ADDRESS: Hammers Construction, LLC 1411 Woolsey Heights Colorado Springs, CO80915

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Manual, as amended.

BY:

DATE:

Jennifer Irvine, P.E. County Engineer

CONDITIONS:

FINAL DRAINAGE REPORT

FOR

LOT 3 PADMARK BUSINESS PARK FILING NO. 1 (Lot 44 of Claremont Business Park Filing No. 2)

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Vicinity Map Soils Map FIRM Panel W/Revised LOMR Hydrologic Calculations Hydraulic Calculations/SFB WQCV Calculations Proposed Drainage Map Existing Drainage Map Grading Erosion Control Plan

FINAL DRAINAGE REPORT FOR LOT 3 PADMARK BUSINESS PARK FILING NO. 1 (Lot 44 of Claremont Business Park Filing No. 2)

PURPOSE

This document is intended to serve as the Final Drainage Report for Lot 3 PADMARK BUSINESS PARK FILING NO. 1 formerly (Lot 44 of Claremont Business Park Filing No. 2). The purpose of this document is to identify and analyze the on and offsite drainage patterns and to ensure that post development runoff is routed through the site safely and in a manner that satisfies the requirements set forth by the El Paso County Drainage Criteria Manual. The proposed principal use for Lot 3 consists of all infrastructures typically associated with commercial building structures. The majority of the site will consist of asphalt, curb, lighting, a storm water quality facility and landscaping. The proposed use is a permissible use within the Commercial Service zoning criteria.

GENERAL LOCATION AND DESCRIPTION

PADMARK BUSINESS PARK FILING NO. 1 is located in the northeast quarter of the northeast quarter of Section 8, Township 14 South, Range 65 West of the 6th P.M. in El Paso County, Colorado. The site is bound on the northeast by a vacant parcel of land (Tract B, Claremont Commercial Sub No. 1). The site is bound on the northwest by the East Fork Sand Creek Channel. The property is bound to the southwest by an existing development of an office/warehouse/storage yard (Lot 2 Padmark Business Park). An existing access road runs along the eastern property boundary of Lot 2 adjacent to Meadowbrook Parkway, and currently provides access to both Lot 1, 2 & 3. The site lies within the Sand Creek Drainage Basin. Flows from this site are tributary to Sand Creek.

Lot 3 of Padmark Business Park is presently undeveloped and consists of 0.870 acres. Vegetation is sparse, consisting of native grasses. The site had experienced overlot grading activities within the last ten years. Existing site terrain generally slopes from northeast to southwest at grade rates that vary between 2% and 12%.

The PADMARK BUSINESS PARK FILING NO. 1 site is currently zoned "CS" and the proposed principal uses for Lot 3 will be an office/warehouse/light manufacturing. The majority of Lot 3 shall consist of warehouse building, asphalt, curb, lighting, a storm water quality facility and landscaping. A sand filter basin is proposed within the southwest portion of the lot and will function to provide water quality treatment for the site. Flows discharge from the sand filter basin through an outlet structure and eventually outfall directly to East Fork Sand Creek.

SOILS

Soils for this project are delineated by the map in the appendix as Ellicott Loamy Course Sand (28) and characterized as Hydrologic Soil Types "A". Soils in the study area are shown as mapped by S.C.S. in the "Soils Survey of El Paso County Area". Vegetation is sparse, consisting of native grasses and weeds.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The relevant data sheets are included in the appendix of this report.

FLOODPLAIN STATEMENT

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0756 F and Panel No. 08041C0752 F, effective date March 17, 1997 and revised to reflect LOMR, 06-08-B137P, dated December 13, 2006, the site lies adjacent to and is NOT impacted by a Zone "AE". An annotated FIRM Panel is included in the Appendix. The approximate BFE of the East Fork Creek adjacent to the proposed pond is approximately 6358. The proposed sand filter pond and emergency spillway is designed above the East Sand Creek BFE.

DRAINAGE CRITERIA

This drainage analysis has been prepared in accordance with the current City of Colorado Springs/El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres.

FOUR STEP PROCESS

- Step1 Employ Runoff Reduction Practices Approx. 0.116 ac of the proposed developed 0.870 ac of ground within the project is being set aside for Open Space/WQ facility. Roof drains will be directed to landscaped areas if possible to minimize direct connection of impervious surfaces.
- Step 2 Stabilize Drainageways The site is directly adjacent to the Sand Creek Channel. The "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc. (hence for referred to as "MDDP") has been designed to discharge developed flows via an existing public 48" RCP directly to the East Fork Sand Creek. The existing public 48" RCP is located south of Lot 44 Claremont Business Park Filing No. 2 as highlighted on the existing drainage map. Lot 3 proposes a Sand Filter Water Quality Facility before ultimately discharging to the existing public 48" RCP pipe. The outlet underdrain has been designed to drain the pond in a peak event within 12 hours, therefore is not anticipated to have negative effects on downstream drainageways.
- Step 3 Provide Water Quality Capture Volume A Sand Filter Basin water quality facility is proposed to provide WQCV.
- **Step4** Consider Need for Industrial and Commercial BMP's This submittal provides a final grading and erosion control plans with BMPs in place. The proposed project will use silt fence, a vehicle tracking control pad, concrete washout area, mulching and reseeding to mitigate the potential for erosion across the site.

EXISTING DRAINAGE CONDITIONS

The Lot 3 PADMARK BUSINESS PARK FILING NO. 1 site consists of 0.870 acres and is situated east of the East Fork Reach of the Sand Creek Watershed. This area was previously studied in the "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc. (hence for referred to as "MDDP") and was included within Sub-basin B5 (Lot 44). The MDDP calculations indicate that the total tributary area of Sub-basin B5 (4.0acres) would produce runoff of approximately Q5=12.0 cfs and Q100=24.1cfs. The MDDP illustrated that the basin watershed would drain and be collected by a10' Type R sump inlet which was proposed to be located at the southwest corner of the Lot 44. Flows from the sump inlet would be carried to the existing public 48" storm sewer before outfalling into East Fork Sand Creek.

In the existing condition, Lot 1 of Padmark Business Park is currently developed, and Lot 2 is currently under construction. The "Final Drainage Report for Padmark Business Park Filing No. 1", dated June 2017, by M & S Civil Consultants, Inc (hence for referred to as "FDR") details the subdivision of Lot 44 of Claremont Business Park Filing No. 2 into three lots and the development of Lot 1 within that subdivision. An existing access road runs along the eastern property boundary of Lot 2, adjacent to Meadowbrook Parkway, and currently provides access to Lots 1, 2 and 3 of the Padmark Business Park. Treated flows from Lots 1 & 2 are collected and discharged into an existing 30" RCP pipe that connects to the existing public 48" storm sewer, outfalling into East Fork Sand Creek as determined by the MDDP and implemented in the FDR. During development of Lot 2. During development of Lot 2, the existing 30" RCP pipe was connected to then extended to the WQ outfall manhole, then an 18" RCP was installed continually along the northwest property line of Lot 2 and the 18" RCP was capped for future connection for Lot 3. This provides a drainage connection upon the development

PROPOSED DRAINAGE CHARACTERISTICS

General Concept Drainage Discussion

Identify that the outlet box is designed to capture the 100yr developed runoff. The inline storm system for the three lots is not just for conveyance of

Runoff tributary to the southwestern boundary and sand filter basin the treated WQCV. BUSINESS PARK FILING NO. 1 site is produced within Basin A (See Proposed Drainage Map in the appendix).

Runoff collected (Basin A) and conveyed to the water quality facility (Design Point 1) is discharged from a proposed sand filter basin via a 2.91' X 2.91' CDOT outlet box and an 18" RCP to an inline system conveying all treated flows from Lot 3, 2 and 1 that ultimately outfalls to an existing public 48" storm sewer at the southwest corner of Lot 1. The proposed 18" RCP outlet pipe from the sand filter basin connects to a Type II manhole proposed at the southwest corner of Lot 3. The proposed Type II manhole connects with the existing 18" RCP was stubbed at the north end of Lot 2. The existing capped 18" RCP storm sewer along the west side of Lot 2 discharges to an existing Type II manhole at the southwest corner of Lot 2 and ties to an existing 30" RCP located at the southwest property boundary. An emergency overflow spillway section has been proposed in the event of blockage of the 2.91' X 2.91' CDOT inlet for Lot 3. All flows generated by Lot 3 Padmark Business Park will eventually be discharged to the existing public 48" storm sewer, outfalling into East Fork Sand Creek as determined by the MDDP. A detailed drainage discussion of each basin is presented below.

Detailed Drainage Discussion

Basin A, 0.78 acres, ($Q_5=3.3$ cfs, $Q_{100}=6.0$ cfs), consists of office/warehouse/light manufacturing building, asphalt paving, crushed asphalt, curb and gutter, and landscaping. Flows produced within the watershed

are routed as surface runoff to DP1 where they are conveyed to the onsite sand filter basin for water quality.

There are no planned or required improvements to the Sand Creek Drainage Channel with the development of the Lot 3 PADMARK BUSINESS PARK FILING NO. 1.

WATER QUALITY PROVISIONS AND MAINTENANCE

The proposed Sand Filter Basin functions to provide water quality for runoff produced on the Lot 3 PADMARK BUSINESS PARK FILING NO. 1 site (see Proposed Drainage Map). Lot 44 of Claremont Business Park Filing No. 2 has been replatted in to (3) lots. Each of the 3 lots shall be and has been responsible for each respective generated runoff. This water quality pond for Lot 3 is designed to treat approx 0.78 (Basin A) ac up to the 100 yr event, and provide 915 cubic-feet of water quality storage. The water quality basin will be private and shall be maintained by the property owner. Access shall be granted to the owner and El Paso County for access and maintenance of the private WQCV facility. A private maintenance agreement document shall accompany the submittal.

The subject site was previously analyzed within the Final Drainage Report for Claremont Business Park Filing No. 2 prepared by Matrix Design Group approved April 24, 2006. Per Resolution 16-426 of the BOCC, on-site WQCV is required but on-site stormwater detention is <u>not</u> required per the FDR for Claremont Business Park Fil. 2, (See Appendix). The water quality volume required for the site has been determined using the guidelines set forth in the City of Colorado Springs/El Paso County Drainage Criteria Manual - Volume II. Refer to the water quality facility sizing calculations (UD-Detention Version 3.07 & UD-BMP Version 3.06) located within the appendix of this report.

EROSION CONTROL

It is the policy of the El Paso County that we submit a grading and erosion control plan with the drainage report. Proposed silt fence, vehicle traffic control, and concrete washout area are proposed as erosion control measures.

CONSTRUCTION COST OPINION

Private Drainage Facilities NON-Reimbursable:

Item	Description	Quai	ntity	Unit C	Cost		Cost
1.	18" RCP	15	LF	\$40	/LF		\$600.00
2.	Type II Manhole (4' DIA)	1	EA	\$6,000	/EA		\$6,000.00
3.	WQ Sand Filter Basin	1	EA	\$6,000	/EA		\$6,000.00
4.	Rip Rap Protection	7	CY	\$60	/CY		\$420.00
5.	Spillway Protection (SC250 Mat)	94	SY	\$10	/SY		\$940.00
6.	Pond Outlet Structure	1	EA	\$5,000	/EA		\$5,000.00
						Total \$	\$18,960.00

DRAINAGE & BRIDGE FEES

The proposed replat of the Claremont Business Park Filing No. 2, Lot 44 into 3 commercial lots does not proposed a change to the zoning designation nor the impervious acreage, therefore no drainage fees are due.

M &S Civil Consultants, Inc. (M &S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above is only an estimate of the facility cost and drainage basin fee amounts in 2018.

SUMMARY

Development of Lot 3 PADMARK BUSINESS PARK FILING NO. 1 site will not adversely affect the surrounding development per this final drainage report with no negative impacts on surrounding developments and the East Fork Sand Creek channel. The proposed drainage facilities will adequately convey, detain and route runoff from the tributary onsite and existing offsite flows to the Sand Creek Drainage channel. All drainage facilities described herein and shown on the included drainage map are subject to change due to formal design considerations during the construction document preparation stage. Care will be taken to accommodate overland emergency flow routes on site and temporary drainage conditions. The development of Lot 3 PADMARK BUSINESS PARK FILING NO. 1, project shall not adversely affect adjacent or downstream property.

REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".
- 2.) "Urban Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Effective date March 17, 1997.
- 5.) "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc.
- 6.) "Final Drainage Report for Padmark Business Park Filing No. 1", dated June 2017, by M & S Civil Consultants, Inc.

APPENDIX

VICINITY MAP





SOILS MAP



National Cooperative Soil Survey

Conservation Service



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	0.0	0.0%
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	A	4.2	100.0%
Totals for Area of Intere	st		4.2	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



FIRM PANEL W/ REVISED LOMR



Federal Emergency Management Agency

Washington, D.C. 20472

NOV 13 2006

CERTIFIED MAIL RETURN RECEIPT REQUESTED

The Honorable Sallie Clark Chair, El Paso County Board of Commissioners 27 East Vermijo Avenue Colorado Springs, CO 80903

Dear Ms. Clark:

IN REPLY REFER TO: Case No.: 06-08-B137P Follows Conditional Case No.: 04-08-0469R Community Name: El Paso County, CO Community No.: 080059 Effective Date of This Revision: DEC 13 2006

The Flood Insurance Study Report and Flood Insurance Rate Map for your community have been revised by this Letter of Map Revision (LOMR). Please use the enclosed annotated map panel(s) revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals issued in your community.

Additional documents are enclosed which provide information regarding this LOMR. Please see the List of Enclosures below to determine which documents are included. Other attachments specific to this request may be included as referenced in the Determination Document. If you have any questions regarding floodplain management regulations for your community or the National Flood Insurance Program (NFIP) in general, please contact the Consultation Coordination Officer for your community. If you have any technical questions regarding this LOMR, please contact the Director, Federal Insurance and Mitigation Division of the Department of Homeland Security's Federal Emergency Management Agency (FEMA) in Denver, Colorado, at (303) 235-4830, or the FEMA Map Assistance Center, toll free, at 1-877-336-2627 (1-877-FEMA MAP). Additional information about the NFIP is available on our website at http://www.fema.gov/nfip.

Sincerely,

Kevin C Long

Kevin C. Long, CFM, Project Engineer Engineering Management Section Mitigation Division

List of Enclosures:

Letter of Map Revision Determination Document Annotated Flood Insurance Rate Map Annotated Flood Insurance Study Report

cc: Mr. Kevin Stilson, P.E., CFM Regional Floodplain Administrator

Central Marksheffel Business District

Matrix Design Group

For: William R. Blanton Jr., CFM, Chief Engineering Management Section Mitigation Division

Page 1 of 4	Issue Date:	NOV 1	3 2006	Effective Date	· NEV 1 3 2005	Case N	o.: 06-08-B137P	LOMR-APP
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				SUMMARY O	F REVISIONS			
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* BFEs - Base Floo	od Elevations							
				DETERM	INATION		· · · · · · · · · · · · · · · · · · ·	
This document p regarding a requ a revision to the warranted. This panels revised b This determination any questions abo	provides the de pest for a Lette flood hazards document rev by this LOMR for is based on the ut this document	etermination r of Map Re depicted in ises the eff or floodplain flood data pr b please con	n from the Depa evision (LOMR) the Flood Insu ective NFIP ma n management	artment of Homela for the area desc irance Study (FIS) ap, as indicated in purposes and for	Ind Security's Federal Emer ribed above. Using the info report and/or National Floc the attached documentation all flood insurance policies	rgency Ma rmation su od Insuran n. Please and renew	nagement Agency (FE ubmitted, we have dete ce Program (NFIP) ma use the enclosed anni vals in your community arding this determination.	MA) ermined that p is otated map '
LOMR Depot, 360	1 Eisenhower Av	venue, Alexa	ndria, VA 22304.	Additional Informat	ion about the NFIP is available	-orrena on our web	MAP) or by letter addres site at http://www.fema.g	sed to the ov/nfip.
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Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

COMMUNITY INFORMATION

APPLICABLE NFIP REGULATIONS/COMMUNITY OBLIGATION

We have made this determination pursuant to Section 206 of the Flood Disaster Protection Act of 1973 (P.L. 93-234) and in accordance with the National Flood Insurance Act of 1968, as amended (Title XIII of the Housing and Urban Development Act of 1968, P.L. 90-448), 42 U.S.C. 4001-4128, and 44 CFR Part 65. Pursuant to Section 1361 of the National Flood Insurance Act of 1968, as amended, communities participating in the NFIP are required to adopt and enforce floodplain management regulations that meet or exceed NFIP criteria. These criteria, including adoption of the FIS report and FIRM, and the modifications made by this LOMR, are the minimum requirements for continued NFIP participation and do not supersede more stringent State/Commonwealth or local requirements to which the regulations apply.

We provide the floodway designation to your community as a tool to regulate floodplain development. Therefore, the floodway revision we have described in this letter, while acceptable to us, must also be acceptable to your community and adopted by appropriate community action, as specified in Paragraph 60.3(d) of the NFIP regulations.

NFIP regulations Subparagraph 60.3(b)(7) requires communities to ensure that the flood-carrying capacity within the altered or relocated portion of any watercourse is maintained. This provision is incorporated into your community's existing floodplain management ordinances; therefore, responsibility for maintenance of the altered or relocated watercourse, including any related appurtenances such as bridges, culverts, and other drainage structures, rests with your community. We may request that your community submit a description and schedule of maintenance activities necessary to ensure this requirement.

COMMUNITY REMINDERS

We based this determination on the 1-percent-annual-chance flood discharges computed in the FIS for your community without considering subsequent changes in watershed characteristics that could increase flood discharges. Future development of projects upstream could cause increased flood discharges, which could cause increased flood hazards. A comprehensive restudy of your community's flood hazards would consider the cumulative effects of development on flood discharges subsequent to the publication of the FIS report for your community and could, therefore, establish greater flood hazards in this area.

Your community must regulate all proposed floodplain development and ensure that permits required by Federal and/or State/Commonwealth law have been obtained. State/Commonwealth or community officials, based on knowledge of local conditions and in the interest of safety, may set higher standards for construction or may limit development in floodplain areas. If your State/Commonwealth or community has adopted more restrictive or comprehensive floodplain management criteria, those criteria take precedence over the minimum NFIP requirements.

We will not print and distribute this LOMR to primary users, such as local insurance agents or mortgage lenders; instead, the community will serve as a repository for the new data. We encourage you to disseminate the information in this LOMR by preparing a news release for publication in your community's newspaper that describes the revision and explains how your community will provide the data and help interpret the NFIP maps. In that way, interested persons, such as property owners, insurance agents, and mortgage lenders, can benefit from the information.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at http://www.fema.gov/nfip.

Kevin C. Lon

Kevin C. Long, CFM, Project Engineer Engineering Management Section Mitigation Division



Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

We have designated a Consultation Coordination Officer (CCO) to assist your community. The CCO will be the primary liaison between your community and FEMA. For information regarding your CCO, please contact:

Ms. Jeanine D. Petterson Director, Federal Insurance and Mitigation Division Federal Emergency Management Agency, Region VIII Denver Federal Center, Building 710 P.O. Box 25267 Denver, CO 80225-0267 (303) 235-4830

STATUS OF THE COMMUNITY NFIP MAPS

We will not physically revise and republish the FIRM and FIS report for your community to reflect the modifications made by this LOMR at this time. When changes to the previously cited FIRM panel(s) and FIS report warrant physical revision and republication in the future, we will incorporate the modifications made by this LOMR at that time.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at http://www.fema.gov/nfip.

Kevin C. Long

Kevin C. Long, CFM, Project Engineer Engineering Management Section Mitigation Division



Federal Emergency Management Agency Washington, D.C. 20472

LETTER OF MAP REVISION **DETERMINATION DOCUMENT (CONTINUED)**

PUBLIC NOTIFICATION OF REVISION

PUBLIC	NOTIFICATION
--------	--------------

FLOODING SOURCE	LOCATION OF REFERENCED ELEVATION	BFE (FEET	NGVD 29)	MAP PANEL
····		EFFECTIVE	REVISED	NUMBER(S)
East Fork Sand Creek	Approximately 5,150 feet downstream of Marksheffel Road	6,316	6,315	08041C0752F
	Approximately 210 feet downstream of Marksheffel Road	6,381	6,379	08041C0756F

Within 90 days of the second publication in the local newspaper, a citizen may request that we reconsider this determination. Any request for reconsideration must be based on scientific or technical data. This revision will become effective 30 days from the date of this letter. However, until the 90-day period has elapsed, the revised BFEs presented in this LOMR may be changed.

A notice of changes will be published in the Federal Register. This information also will be published in your local newspaper on or about the dates listed below.

LOCAL NEWSPAPER

Name: El Paso County News Dates: 11/29/2006 and 12/06/2006

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMR Depot, 3601 Eisenhower Avenue, Alexandria, VA 22304. Additional Information about the NFIP is available on our website at http://www.fema.gov/nfip.

Kevin C. Long

Kevin C. Long, CFM, Project Engineer **Engineering Management Section** Mitigation Division

109770 10.3.1.0608B137 102-I-A-C

CHANGES ARE MADE IN DETERMINATIONS OF BASE FLOOD ELEVATIONS FOR THE UNINCORPORATED AREAS OF EL PASO COUNTY, COLORADO, UNDER THE NATIONAL FLOOD INSURANCE PROGRAM

On March 17, 1997, the Department of Homeland Security's Federal Emergency Management Agency identified Special Flood Hazard Areas (SFHAs) in the unincorporated areas of El Paso County, Colorado, through issuance of a Flood Insurance Rate Map (FIRM). The Mitigation Division has determined that modification of the elevations of the flood having a 1-percent chance of being equaled or exceeded in any given year (base flood) for certain locations in this community is appropriate. The modified Base Flood Elevations (BFEs) revise the FIRM for the community.

The changes are being made pursuant to Section 206 of the Flood Disaster Protection Act of 1973 (Public Law 93-234) and are in accordance with the National Flood Insurance Act of 1968, as amended (Title XIII of the Housing and Urban Development Act of 1968, Public Law 90-448), 42 U.S.C. 4001-4128, and 44 CFR Part 65.

A hydraulic analysis was performed to incorporate the effects of channel improvements along Sand Creek East Fork from approximately 5,250 feet downstream to just upstream of Marksheffel Road, and has resulted in a revised delineation of the regulatory floodway, an increase in SFHA width, a decrease in SFHA width, and decreased BFEs for Sand Creek East Fork. The aforementioned channelized portion of Sand Creek East Fork contains the base flood. The table below indicates existing and modified BFEs for selected locations along the affected lengths of the flooding source(s) cited above.

Location	Existing BFE (feet)*	Modified BFE (feet)*
Sand Creek East Fork		
Approximately 5,150 feet downstream of Marksheffel Road	6,316	6,315
Approximately 210 feet downstream of Marksheffel Road	6,381	6,379

*National Geodetic Vertical Datum, rounded to nearest whole foot

Under the above-mentioned Acts of 1968 and 1973, the Mitigation Division must develop criteria for floodplain management. To participate in the National Flood Insurance Program (NFIP), the community must use the modified BFEs to administer the floodplain management measures of the NFIP. These modified BFEs will also be used to calculate the appropriate flood insurance premium rates for new buildings and their contents and for the second layer of insurance on existing buildings and contents.

Upon the second publication of notice of these changes in this newspaper, any person has 90 days in which he or she can request, through the Chief Executive Officer of the community, that the Mitigation Division reconsider the determination. Any request for reconsideration must be based on knowledge of changed conditions or new scientific or technical data. All interested parties are on notice that until the 90-day period elapses, the Mitigation Division's determination to modify the BFEs may itself be changed.

Any person having knowledge or wishing to comment on these changes should immediately notify:

The Honorable Sallie Clark Chair, El Paso County Board of Commissioners 27 East Vermijo Avenue Colorado Springs, CO 80903

BASE FLOOD WATER SURFACE ELEVATION	MEAN VELOCITY VELOCITY REET PER REGULATORY FEET (NGVD) INCREASE	REVISED BY LOMIR DATED OCTOBER 07, 2004	11.9 6,038.7 6,038.7 6,038.7 0.0	12.2 // 6,054.3 6,054.3 6,054.3 0.0	12.0 / 6,069.9 6,069.9 6,069.9 0.0	12.1 6,085.1 6,085.1 6,085.1 0.0	12.0 6,095.2 6,095.2 6,095.2 0.0	8.9 6,118.4 6,118.4 6,118.9 0.5	10.3 6,128.1 6,128.1 6,129.1 1.0	10.6 6.168.8 6.168.8 6.168.8 6.158.2 0.0	12.0 6 ,188.4 6 ,188.4 6 ,188.4 0.0	11.5 / [6,196.2 6,196.2 6,196.2 0.0	10.2 6,207.3 6,207.3 6,207.3 0.0		/.0 0,228.8 0,228.8 0,228.8 0.1 10.0 6.241.7 6.241.7 0.0	11.1 6,257.9 6,257.9 6,257.9 0.0	8.9 6,259.9 6,259.9 6,259.9 1.0	9.2 6,268.7 0,268.7 6,268.7 0.0	7.9 6,277.3 6,277.3 6,277.5 0.2	1.1 0,291.4 6,291.4 6,292.0 0.6 8.0 6.791.4 6.201.4 6.292.0 0.6	3.3 6,293.4 6,293.4 6,294.0 0.6	7.8 6,307.2 6,307.2 6,307.6 0.4	11.7 6,327.8 6,327.8 6,328.4 0.6	11.0 6,348.8 6,348.8 6,349.4 0.6	10 0.358.0 0.309.0 0.0 0.0 2309.2 2309.2 7.0			LONR DATED OCTOBER 30, 2006 REFLECT LOMR	FLOODWAY LAFAFECTIVE DEC]	SAND CREEK EAST FORK
FLOODWAY	WIDTH (FEET) (SQUARE FEET)		100 455	100 446	100 450	100 449	100 451	250 602	150 518 175 77	150 505	100 443	115 465	166 525 173 637	367 600	188 570	125 479	109 671	286 877	300 678	326 667	388 1,598	367 683	103 575	142 506 145 506	418 3.156	132 452	112 419	d Creek REVISED BY	MENT AGENCY	AREAS
URCE	DISTANCE		1,100	2,400	3,330	4,240	4,870	5,820	0,090	8,665	9,675	10,565	225,11	12.610	13,720	14,805	14,885	Degici I	16,325	17.065	17,915	18,995	20,730	22,560	24.835	26,470	27,715	ence With San	ERGENCY MANAGE ASO COUNT	VCORPORATED
FLOODING SO	CROSS SECTION	Sand Creek East Fork	¥	Â	U I	a 1	<u>ب</u>	ž, (נד	1	در	××	1 Z	Z	0	<u>р</u> , 1	0 0	4 0	v F	EVISED U	NEA V	≥	× >		T AN	AB	AC	¹ Feet Above Conflu	FEDERAL EMI	AND II









HYDROLOGIC CALCULATIONS

LOT 3 PADMARK BUSINESS PARK FILING NO. 1 PROPOSED DRAINAGE CALCULATIONS (Area Runoff Coefficient Summary)

			STREE	ETS / DEVE	ELOPED	OVERI	AND / DEVE.	ELOPED	OVERLA	ND / UNDEV	WEIGHTED		
BASIN	TOTAL AREA (SF)	TOTAL AREA (Acres)	AREA (Acres)	C ₅	C ₁₀₀	AREA (Acres)	C ₅	C ₁₀₀	AREA (Acres)	C ₅	C ₁₀₀	C ₅	C ₁₀₀
A	34031.31	0.78	0.78	0.81	0.88	0.00	0.81	0.88	0.00			0.81	0.88

LOT 3 PADMARK BUSINESS PARK FILING NO. 1 FINAL DRAINAGE REPORT

(Area Drainage Summary)

From	Area Runoff Coe	fficient Summary			OVERLA	1ND		S7	REET / CH	IANNEL FLO)W	Time of T	Travel (T ₁)	INTEN	SITY *	TOTAL FLOWS	
BASIN	AREA TOTAL	C ₅	C ₁₀₀	C ₅	Length	Height	T _C	Length	Slope	Velocity	Tt	TOTAL	СНЕСК	I ₅	I ₁₀₀	Q5	Q ₁₀₀
	(Acres)	From DCM	1 Table 5-1		(ft)	(ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
A	0.78	0.81	0.88	0.81	45	1.0	2.7	200	1.0%	2.0	1.7	4.4	11.4	5.2	8.7	3.3	6.0

* Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CMN Date: 6/25/2018 Checked by: VAS

LOT 3 PADMARK BUSINESS PARK FILING NO. 1 FINAL DRAINAGE REPORT (Basin Routing Summary)

	From Area Runoff Coefficient Summary	,			OVE	ERLAND		PIPE	E / CHAI	NNEL FLO	W	Time of Travel (T_t)	INTEN	SITY *	TOTAL FLOWS		
DESIGN POINT	CONTRIBUTING BASINS	CA ₅	CA100	C ₅	Length	Height	T _C	Length	Slope	Velocity	T _t	TOTAL	I ₅	I ₁₀₀	Q5	Q ₁₀₀	COMMENTS
					(ft)	(ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)	
1	Α	0.63	0.69	0.81	45	1	2.7	200	1.0%	2.0	1.7	5.0	5.2	8.7	3.3	6.0	SAND FILTER BASIN AND
																	CDOT TYPE C
																	OUTLET STRUCTURE
													Calcu	lated by:	CMN		
														Deter	(125/2010)	

Date: <u>6/25/2018</u> Checked by: <u>VAS</u>

LOT 3 PADMARK BUSINESS PARK FILING NO. 1 FINAL DRAINAGE REPORT (Storm Sewer Routing Summary)

					Inten	sity*	Fl	ow
PIPE	Contributing Pipes/Design Points	Equivalent CA 5	Equivalent CA ₁₀₀	Maximum T _C	I 5	I 100	Q 5	Q 100
1	DP1	0.63	0.69	5.0	5.2	8.7	3.3	6.0

* Intensity equations assume a minimum travel time of 5 minutes.

DP - Design Point

EX - Existing Design Point

FB- Flow By from Design Point INT- Intercepted Flow from Design Point Calculated by: CMN Date: 6/25/2018

Checked by: VAS

HYDRAULIC CALCULATIONS / SFB WQCV CALCULATIONS

			SIN STAGE-S	TORAG	LIADL	DOILD						
		UD-Dete	ention, Version 3	3.07 (Febi	ruary 2017	7)						
Project: Racio ID:	Lot 3 Padmark Busine	ss Park Filing No. 1										
ZONE3	<u>A</u>											
	IONE 1	~										
VOLUME EURY WOCY												
	10	D-YEAR	Depth Increment =		ft							
PERMANENT ORIFIC	CES Configuration /P/	tention Bond)	Stage - Storage	Stage	Optional Override	Length	Width	Area	Optional Override	Area	Volume	
Example 2016	, comiguration (re	tention rond)	Description	(ft)	Stage (ft)	(ft)	(ft)	(ft'2)	Area (ft'2)	(acre)	(ft/3)	
Required Volume Calculation		Bttm of SFB 6361	Media Surface		0.00				409	0.009		
Selected BMP Type =	SF		6362		1.00				1,067	0.024	727	-
Watershed Area =	0.78 acres	Note: L / W Ratio < 1	6363		2.00			-	1,807	0.041	2,157	+
Watershed Slope =	0.015 ft/ft	L / W Ratio = 0.1	0303.20		2.20	-	-	-	1,810	0.044	2,000	+
Watershed Imperviousness =	95.00% percent											1
Percentage Hydrologic Soil Group A =	100.0% percent							-				
Percentage Hydrologic Soil Group B =	0.0% percent		-									+
Percentage Hydrologic Soll Groups C/D = Desired WOCV Drain Time =	12.0 bours		-									+
Location for 1-hr Rainfall Depths =	Denver - Capitol Buildir	g						-				+
Water Quality Capture Volume (WQCV) =	0.023 acre-fee	Optional User Override						-				
Excess Urban Runoff Volume (EURV) =	0.102 acre-fee	1-hr Precipitation										
2-yr Runoff Volume (P1 = 1.19 in.) =	0.071 acre-fee	t 1.19 inches	-									_
5-yr Runoff Volume (P1 = 1.5 in.) = 10-yr Runoff Volume (P1 = 1.75 in.) =	0.092 acre-fee	t 1.50 inches						-				-
25-yr Runoff Volume (P1 = 2 in.) =	0.128 acre-fee	2.00 inches						-				
50-yr Runoff Volume (P1 = 2.25 in.) =	0.144 acre-fee	t 2.25 inches						-				
100-yr Runoff Volume (P1 = 2.52 in.) =	0.163 acre-fee	2.52 inches										1
500-yr Runott Volume (P1 = 3.14 in.) = Anoroximate 2 yr Detection Velume =	0.208 acre-fee	inches			-							+
Approximate 2-yr Detention Volume =	0.088 acre-fee	t						-	1			+
Approximate 10-yr Detention Volume =	0.104 acre-fee	t						-				L
Approximate 25-yr Detention Volume =	0.122 acre-fee	t						-			1	Γ
Approximate 50-yr Detention Volume =	0.132 acre-fee	t					-		-			-
Approximate 100-yr Detention volume =	U. 141 acre-fee	L.						-				+
Stage-Storage Calculation								-				1
Zone 1 Volume (WQCV) =	0.023 acre-fee	t						-				
Select Zone 2 Storage Volume (Optional) =	acre-fee	Total detention volume										
Select Zone 3 Storage Volume (Optional) = Total Detention Basin Volume =	acre-fee	volume.						-				+
Initial Surcharge Volume (ISV) =	N/A ft/3	t.						-				
Initial Surcharge Depth (ISD) =	N/A ft							-				
Total Available Detention Depth (H _{total}) =	user ft							-				
Depth of Trickle Channel (H _{TC}) =	N/A ft							-				-
Slopes of Main Basin Sides (Smain) =	User U.V							-				
Basin Length-to-Width Ratio (R _{L/W}) =	user											
								-				
Initial Surcharge Area (A _{ISV}) =	user ft/2		-									_
Surcharge Volume Length (L _{ISV}) = Surcharge Volume Width (W _{ISV}) =	user ft							-				
Depth of Basin Floor (H _{FLOOR}) =	user ft							-				
Length of Basin Floor (L _{FLOOR}) =	user ft											
Width of Basin Floor (W _{FLOOR}) =	user ft							-				-
Volume of Basin Floor (V _{FLOOR}) =	user ft/2							-				
Depth of Main Basin (H _{MAIN}) =	user ft							-				+
Length of Main Basin (L _{MAIN}) =	user ft											Γ
Width of Main Basin (W _{MAIN}) =	user ft		-					-				+
Area of Main Basin (A _{MAIN}) =	user ft/2				-			-	-			+
Calculated Total Basin Volume (V _{total}) =	user acre-fee	1					-	-				+
		-			-			-				1
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UD-Detention, Version 3.07 (February 2017)



Weighted Percent Imperviousness of WQ SFB								
Contributing Basins	Area (Acres)	<i>C</i> 5	Impervious % (I)	(Acres)*(I)				
A	0.7813	0.81	95	74.22				
Totals	0.7813			74.22				
Imperviousness of WQ Pond 1	95.0							

	Detention Basin Outlet Structure Design								
Project:	Lot 3 Padmark Bus	iness Park Filing No	UD-Detention, Ve o. 1	rsion 3.07 (Februar	y 2017)				
Basin ID:	А								
ZONE 3 ZONE 2 ZONE 1	_	201							
100-YR				Stage (ft)	Zone Volume (ac-ft)	Outlet Type			
			Zone 1 (WQCV)	1.24	0.023	Filtration Media			
ZONE 1 AND 2	100-YEA ORIFICE	R	Zone 2						
PERMANENT ORIFICES	Configuration (Po	tention Bond)	Zone 3						
Example Zone	Configuration (Re	tention Pond)			0.023	Total			
User Input: Orifice at Underdrain Outlet (typically us	ed to drain WQCV in	a Filtration BMP)		()	Linda	Calculate	ed Parameters for Un	derdrain	
Underdrain Ornice Invert Depth =	0.73	inches	le filtration media sur	race)	Underdra	in Orifice Centroid =	0.03	ft feet	
User Input: Orifice Plate with one or more orifices of	r Elliptical Slot Weir	(typically used to dra	in WQCV and/or EUF	V in a sedimentation	n BMP)	Calcu	lated Parameters for	Plate	
Invert of Lowest Orifice =		ft (relative to basin b	oottom at Stage = 0 ft)	1	WQ O	rifice Area per Row =	N/A	ft ²	
Depth at top of Zone using Orifice Plate =		ft (relative to basin b	oottom at Stage = 0 ft)		E	lliptical Half-Width =	N/A	feet	
Orifice Plate: Orifice Vertical Spacing =	-	inches			Elli	ptical Slot Centroid =	N/A	feet	
Orifice Plate: Orifice Area per Row =		inches				Elliptical Slot Area =	N/A	ft	
User Input: Stage and Total Area of Each Orifice	Row (numbered fro	m lowest to highest)						
	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)	
Stage of Orifice Centroid (ft)									
Orifice Area (sq. inches)									
		[[]					[I
	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)	
Stage of Orifice Centroid (ft)	-				-	-	-		
Office Area (sq. incres)									
User Input: Vertical Orifice (Circ	ular or Rectangular)					Calculated	Parameters for Vert	ical Orifice	
	Not Selected	Not Selected]				Not Selected	Not Selected	
Invert of Vertical Orifice =			ft (relative to basin b	ottom at Stage = 0 ft) V	ertical Orifice Area =			ft ²
Depth at top of Zone using Vertical Orifice =	ft (relative to basin bottom at Stage = 0 ft) Vertical Orifice Centroid = feet								feet
Vertical Orifice Diameter =			inches						
User Input: Overflow Weir (Dropbox) and G	irate (Flat or Sloped)					Calculated	Parameters for Ove	rflow Weir	
User Input: Overflow Weir (Dropbox) and G	irate (Flat or Sloped) Not Selected	Not Selected]			Calculated	Parameters for Ove Not Selected	rflow Weir Not Selected	
User Input: Overflow Weir (Dropbox) and G	irate (Flat or Sloped) Not Selected 1.24	Not Selected	ft (relative to basin bo	ttom at Stage = 0 ft)	Height of Gr	Calculated are Upper Edge, $H_t =$	Parameters for Ove Not Selected 1.24	rflow Weir Not Selected	feet
User Input: Overflow Weir (Dropbox) and G Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length =	irate (Flat or Sloped) Not Selected 1.24 2.91	Not Selected	ft (relative to basin bo feet	ttom at Stage = 0 ft)	Height of Gr Over Flow	Calculated rate Upper Edge, H _t = Weir Slope Length =	Parameters for Ove Not Selected 1.24 2.91	rflow Weir Not Selected	feet feet
User Input: Overflow Weir (Dropbox) and G Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope =	irate (Flat or Sloped) Not Selected 1.24 2.91 0.00	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl	ttom at Stage = 0 ft) at grate)	Height of Gr Over Flow Grate Open Area /	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area =	Parameters for Ove Not Selected 1.24 2.91	rflow Weir Not Selected	feet feet should be ≥ 4
User Input: Overflow Weir (Dropbox) and O Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides =	rate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet	ttom at Stage = 0 ft) at grate)	Height of Gr Over Flow Grate Open Area / Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris =	Parameters for Ove Not Selected 1.24 2.91 5.93	rflow Weir Not Selected	feet feet should be \geq 4 ft ²
User Input: Overflow Weir (Dropbox) and O Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debric Florein &	rate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91 70%	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t «	ttom at Stage = 0 ft) at grate) otal area	Height of Gr Over Flow Grate Open Area / Overflow Grate Op Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris = pen Area w/ Debris =	Parameters for Ove Not Selected 1.24 2.91 5.93 2.96	rflow Weir Not Selected	feet feet should be ≥ 4 ft ²
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debris Clogging % =	rate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91 70% 50%	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t %	ttom at Stage = 0 ft) at grate) otal area	Height of Gr Over Flow Grate Open Area / Overflow Grate Op Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris = ben Area w/ Debris =	Parameters for Ove Not Selected 1.24 2.91 5.93 2.96	rflow Weir Not Selected	feet feet should be≥4 ft ² ft ²
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debris Clogging % = User Input: Outlet Pipe w/ Flow Restriction Plate (Ci	rate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91 70% 50% 	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t % gular Orifice)	ttom at Stage = 0 ft) at grate) otal area	Height of Gr Over Flow Grate Open Area / Overflow Grate Op Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris = ben Area w/ Debris = Calculated Parametei	Parameters for Ove Not Selected 1.24 2.91 5.93 2.96 rs for Outlet Pipe w/	rflow Weir Not Selected	feet feet should be≥4 ft ² e
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debris Clogging % = User Input: Outlet Pipe w/ Flow Restriction Plate (Cr	irate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91 70% 50% - rular Orifice, Restrict Not Selected	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t % ular Orifice)	ttom at Stage = 0 ft) at grate) otal area	Height of Gr Over Flow Grate Open Area / Overflow Grate Op Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris = ben Area w/ Debris = Calculated Parameter	Parameters for Ove Not Selected 1.24 2.91 5.93 2.96 s for Outlet Pipe w/ Not Selected	rflow Weir Not Selected	feet feet should be≥4 ft ² g
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debris Clogging % = User Input: Outlet Pipe w/ Flow Restriction Plate (Ci Depth to Invert of Outlet Pipe =	rate (Flat or Sloped) Not Selected 1.24 2.91 0.00 2.91 70% 50% - rcular Orifice, Restric Not Selected	Not Selected	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t % ular Orifice) ft (distance below basi	ttom at Stage = 0 ft) at grate) otal area n bottom at Stage = 0 1	Height of Gr Over Flow Grate Open Area / Overflow Grate Op Overflow Grate Op Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = en Area w/o Debris = ben Area w/ Debris = Calculated Parameter Outlet Orifice Area =	Parameters for Ove Not Selected 1.24 2.91 5.93 2.96 s for Outlet Pipe w/ Not Selected	rflow Weir Not Selected	feet feet should be \geq 4 ft ² e ft ²
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Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

	Storm Inflow H	ydrographs	UD-Det	ention, Versio	n 3.07 (Februa	ry 2017)				
	The user can o	verride the calcu	lated inflow hyd	trographs from t	his workbook wi	th inflow hydrog	raphs develope	d in a separate p	rogram.	
	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
Time Interval	TIME	WOCV [cfs]	FURV [cfs]	2 Vear [cfs]	5 Vear [cfs]	10 Year [cfs]	25 Vear [cfs]	50 Year [cfs]	100 Year [cfs]	500 Vear [cfs]
	0.00.00	WQCV [cl3]	Eonv [ei3]	2 1001 [013]	5 rear [ers]	io real [cl3]	25 (61)	So rear [ers]		500 (cai [ci3]
2.30 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:02:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:04:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:06:54	0.04	0.17	0.12	0.16	0.19	0.22	0.24	0.27	0.34
2.176	0:09:12	0.11	0.46	0.32	0.42	0.49	0.57	0.64	0.73	0.92
	0:11:30	0.28	1.18	0.83	1.07	1.27	1.48	1.65	1.87	2.36
	0:13:48	0.77	3.24	2.29	2.94	3.50	4.06	4.53	5.14	6.49
	0:16:06	0.89	3.76	2.65	3.41	4.07	4.72	5.28	6.00	7.59
	0:18:24	0.83	3.56	2.51	3.23	3.86	4.49	5.01	5.71	7.22
	0:20:42	0.75	3.24	2.28	2.94	3.51	4.08	4.56	5.19	6.57
	0:23:00	0.66	2.86	2.01	2.59	3.10	3.61	4.04	4.60	5.83
	0.23.16	0.55	2.44	1.71	2.21	2.64	3.08	3.45	3.93	4.99
	0:29:54	0.49	2.13	1.50	1.95	2.31	2.70	3.02	3.44	4.57
	0.23.34	0.44	1.95	1.55	1.75	2.09	1.09	2.73	3.11	3.95
	0:34:30	0.35	1.30	0.87	1.41	1.05	1.58	1 78	2.33	2.60
	0:36:48	0.20	0.93	0.64	0.84	1.01	1.55	1.78	1 53	1.96
	0:39:06	0.14	0.55	0.45	0.60	0.73	0.86	0.97	1.55	1.50
	0:41:24	0.11	0.49	0.45	0.45	0.54	0.63	0.71	0.82	1.45
	0:43:42	0.08	0.39	0.27	0.35	0.43	0.50	0.56	0.64	0.82
	0:46:00	0.07	0.32	0.22	0.29	0.35	0.41	0.46	0.53	0.68
	0:48:18	0.06	0.28	0.19	0.25	0.30	0.35	0.40	0.45	0.58
	0:50:36	0.05	0.24	0.17	0.22	0.27	0.31	0.35	0.40	0.51
	0:52:54	0.05	0.22	0.15	0.20	0.24	0.28	0.32	0.36	0.46
	0:55:12	0.05	0.21	0.14	0.19	0.23	0.26	0.30	0.34	0.43
	0:57:30	0.03	0.15	0.11	0.14	0.16	0.19	0.22	0.25	0.31
	0:59:48	0.02	0.11	0.08	0.10	0.12	0.14	0.16	0.18	0.23
	1:02:06	0.02	0.08	0.06	0.07	0.09	0.10	0.12	0.13	0.17
	1:04:24	0.01	0.06	0.04	0.05	0.06	0.07	0.08	0.10	0.12
	1:06:42	0.01	0.04	0.03	0.04	0.04	0.05	0.06	0.07	0.09
	1:09:00	0.01	0.03	0.02	0.03	0.03	0.04	0.04	0.05	0.06
	1:11:18	0.00	0.02	0.01	0.02	0.02	0.02	0.03	0.03	0.04
	1:13:36	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.03
	1:15:54	0.00	0.01	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	1:18:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	1:20:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:22:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:25:06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:27:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:29:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1.32.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1.34.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:38:54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:41:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:43:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:45:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:48:06	0,00	0,00	0.00	0,00	0,00	0,00	0.00	0.00	0.00
	1:50:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:52:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:57:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	1:59:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:01:54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:04:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:06:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:08.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:13:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:15:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:18:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:20:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:22:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:24.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:29:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:31:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:34:06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:36:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:38:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:41:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships
The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage	Stage	Area	Area	Volume	Volume	Outflow	
Description	[ft]	[ft^2]	[acres]	[ft^3]	[ac-ft]	[cfs]	
							Fan hant nanulta in sluda tha
						ł	For best results, include the
							stages of all grade slope
							from the S-A-V table on
							Shoot 'Pasin'
							Sheet basin.
							Also include the inverts of all
							outlets (e.g. vertical orifice
							overflow grate, and spillway.
							where applicable)
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PROPOSED DRAINAGE MAP







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(6920) - -

SURFACE DESIGN POINT

BASIN BOUNDARY

EXISTING CONTOUR

PROP CONTOUR

EXISTING GAS LINE

PROPOSED STORM SEWER PIPE

EXISTING STORM SEWER PIPE

CROSSPAN INLET

EXISTING FLOW DIRECTION

ARROW EMERGENCY OVERFLOW DIRECTION

FLOW DIRECTION

HIGH POINT

LOW POINT

SC250 PERM. EROSION CONTROL MAT

BASIN SUMMARY							
BASIN	AREA (ACRES)	Q_5	Q ₁₀₀				
A	0.78	3.3	6.0				

DESIGN POINT SUMMARY							
DESIGN POINT	Q ₅	Q ₁₀₀	BASIN	STRUCTURE			
1	3.3	6.0	А	WQ SAND FILTER BASIN & OUTLET STRUCTURE			

STORM SEWER SUMMARY							
PIPE RUN	Q ₅	Q ₁₀₀	PIPE SIZE	CONTRIBUTING PIPES			
1	3.3	6.0	18" RCP	DP1/SFB			

40'

WQCV SUMMARY							
EPC/URBAN DRAINA FILTER BASIN-SEE S	GE SAND STD. DET.						
WQCV REQUIRED	915 CF						
WQCV PROVIDED	1,002 CF						
AREA REQUIRED	402 SF						
AREA PROVIDED	409 SF						
EMERGENCY SPILLWAY CREST ELEVATION	6362.62 FT						
SPILLWAY DESIGN FLOW DEPTH	0.12 FT						
CDOT TYPE C OUTLET STRUCTURE TOB ELEVATION	6362.24 FT						



	20 BOULDER CRESCENT, SUITE 110	LOT J	3 P.	ADMA	RK BUSIN	VESS	PARK	FIL.	NO.	1
	COLORADO SPRINGS, CO 80903 PHONE: 719.955.5485	PROPOSED DRAINAGE MAP								
			NO. 4	4–031	SCALE:	DATE:	6/25/201	8		
ULTANTS, INC		DESIGNED DRAWN B CHECKED) BY: 3Y: 9 BY:	CMN CMN VAS	1"=20' VERTICAL: N/A	SHEET	5 1 OF 1		PDM	

EXISTING DRAINAGE MAP

FINAL DRAINAGE REPORT

For

"Claremont Business Park Filing No. 2"

Prepared for: El Paso County Department of Public Works Engineering Division

On Behalf of: Claremont Development, Inc.

Prepared by:



2435 Research Parkway, Suite 300 Colorado Springs, CO 80920 (719) 575-0100 fax (719) 572-0208

Revised November 2006

05.151.006

Engineer's Statement:

The *revisions* (changes made to the base Final Drainage Report since July, 2006) to the attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. The revisions encompassed adding additional right of way to the study area at the County's request, the handling of offsite drainage due to the additional right of way, a breakdown of private drainage within lot numbers 10 through 25 of Filing No. 2 due to cross-lot drainage (contrary to note # 25 on the recorded plat), profiling additional inlets along the channel edge, and rip-rap sizing for outlet structures along the channel. The Final Drainage Report dated July, 2006 was prepared under the direct supervision of Richard G. Gallegos, Jr. in July, 2006 and stamped (see next sheet).

The Final Drainage Report was prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing the *revisions* to this report.

Brady A. Shyrock Registered Professional Engineer State of Colorado No. 38164 SEAL



Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Richard G. Gallegos, Jr. Registered Professional Engineer State of Colorado No. 36247 Developer's Statement:

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

Claremont Developmen	at, Inc. /
Business Name	
	\mathcal{O}
By:	14
•	
Title	tun
Address: 3460 Capital F	
Address. <u>5400 Capital L</u>	
<u>Colorado Sprir</u>	<u>1gs, CO 80915</u>

El Paso County:

Filed/In accordance with Section 51.1 of the El Paso Land Development Code, as amended.

For Mr. John McCarty, County Engineer/Director

4/23/01 Date

Conditions:







GRADING EROSION CONTROL PLAN

RESOLUTION 16-426

60CL



RESOLUTION NO. 16-426

BOARD OF COUNTY COMMISSIONERS COUNTY OF EL PASO, STATE OF COLORADO

Resolution Denying an Appeal by Hammers Construction LLC (APP-16-002) of the Administrative Determination made by the Planning and Community Development Department Executive Director regarding the requirement for permanent/post construction Water Quality (permanent stormwater quality best management practices or BMP's).

WHEREAS, pursuant to §§30-11-101(1)(e) and 30-11-107(1)(e), C.R.S., the Board of County Commissioners of El Paso County, Colorado (hereinafter "Board) has the legislative authority to manage the concerns of El Paso County when deemed by the Board to be in the best interests of the County and its inhabitants; and

WHEREAS, after consultation with the County Attorney's Office, the Executive Director of Planning and Community Development on August 4, 2016 issued an administrative determination finding made an administrative determination that all undeveloped lots within the Claremont Business Park are subject to installation of permanent stormwater management best management practices (BMP's) associated with development, and that the terms of a 2008 approved deviation relieving the developer of the requirements have not been met.; and

WHEREAS, an appeal of the administrative determination was filed by Hammers Construction on August 10, 2016, and a hearing date was set for September 27, 2016 to hear the appeal; and

WHEREAS, the hearing was continued to a date certain of November 22, 2016; and

WHEREAS, at the Applicant's appeal hearing on November 22, 2016, testimony from the Applicant and the Applicant's representatives was heard by the Board in favor of the appeal, testimony from representatives of Planning and Community Development Department and was presented, and such testimony and associated evidence was weighed by the Board; and

Resolution No. 16-426 Page 2

WHEREAS, the Board, having reviewed the testimony and evidence, hereby finds and determines that the requested appeal of the administrative determination by the Planning and Community Development Executive Director by the Applicant did not satisfy the criteria of approval to overturn the administrative determination.

NOW, THEREFORE, BE IT RESOLVED that the Board of County Commissioners of El Paso County, Colorado, hereby denies the appeal of the administrative determination by Hammers Construction and determines that permanent stormwater management best management practices (BMP's) are required with new development within the Claremont Business Park: and

BE IT **FURTHER RESOLVED** that Sallie Clark, duly elected, qualified member and Chair of the Board of County Commissioners, or Darryl Glenn, duly elected, qualified member and Vice Chair of the Board of County Commissioners, be and is hereby authorized on behalf of the Board to execute any and all documents necessary to carry out the intent of the Board as described herein.

DONE THIS 22nd day of November, 2016, at Colorado Springs Colorado.



BOARD OF COUNTY COMMISSIONERS EL PASO COUNTY, COLORADO

e (Oal By:

Chair of the Board



DESIGN ENGINEER'S STATEMENT

THIS GRADING AND EROSION CONTROL PLAN WAS PREPARED UNDER MY DIRECTION AND SUPERVISION AND IS CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. SAID PLAN HAS BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR GRADING AND EROSION CONTROL PLANS. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARING THIS PLAN.

VIRGIL A. SANCHEZ, COLORADO P.E. #37160 D FOR AND ON BEHALF OF M & S CIVIL CONSULTANTS, INC.

OWNER/DEVELOPER'S STATEMENT:

I, THE OWNER/DEVELOPER HAVE READ AND WILL COMPLY WITH ALL OF THE REQUIREMENTS SPECIFIED IN THESE DETAILED PLANS AND SPECIFICATIONS.

NAME:

DATE

DBA: HAMMERS CONSTRUCTION

EL PASO COUNTY:

ADDRESS: 1411 WOOLSEY HEIGHTS COLORADO SPRINGS, 80915

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

FILED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, DRAINAGE CRITERIA, AND ENGINEERING CRITERIA MANUAL AS AMENDED.

IN ACCORDANCE WITH ECM SECTION 1.12, THESE CONSTRUCTION DOCUMENTS WILL BE VALID FOR CONSTRUCTION FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO COUNTY ENGINEER. IF CONSTRUCTION HAS NOT STARTED WITHIN THOSE 2 YEARS, THE PLANS WILL NEED TO BE RESUBMITTED FOR APPROVAL, INCLUDING PAYMENT OF REVIEW FEES AT THE PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION.



1.	CONSTRUCTION MAY NOT COMMENCE UNTIL A CONSTRUCTION PERMIT IS OBTAINED FROM DEVELOPMENT SERVICES AND A PRECONSTRUCTION CONFERENCE IS HELD WITH DEVELOPMENT SERVICES INSPECTIONS.
2.	STORMWATER DISCHARGES FROM CONSTRUCTION SITES SHALL NOT CAUSE OR THREATEN TO CAUSE POLLUTION, CONTAMINATION, OR DEGRADATION OF STATE WATERS. ALL WORK AND EARTH DISTURBANCE SHALL BE DONE IN A MANNER THAT MINIMIZES POLLUTION OF ANY ON-SITE OR OFF SITE WATERS, INCLUDING WETLANDS.
3.	NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION, ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE AND EROSION CONTROL SHALL CONFORM TO THE STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS TO REGULATIONS AND STANDARDS MUST BE REQUESTED AND APPROVED, IN WRITING.
4.	A SEPARATE STORMWATER MANAGEMENT PLAN (SMWP) FOR THIS PROJECT SHALL BE COMPLETED AND AN EROSION AN STORMWATER QUALITY CONTROL PERMIT (ESQCP) ISSUED PRIOR TO COMMENCING CONSTRUCTION. DURING CONSTRUCTION THE SWMP IS THE RESPONSIBILITY OF THE DESIGNATED STORMWATER MANAGER, SHALL BE LOCATED OF SITE AT ALL TIMES AND SHALL BE KEPT UP TO DATE WITH WORK PROGRESS AND CHANGES IN THE FIELD.
5.	ONCE THE ESQCP HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL BMPS AS INDICATED ON THE GEC. A PRECONSTRUCTION MEETING BETWEEN THE CONTRACTOR, ENGINEER, AND EL PASO COUNTY WILL BE HELD PRIOR TO ANY CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE APPLICANT T COORDINATE THE MEETING TIME AND PLACE WITH COUNTY DSD INSPECTIONS STAFF.
6.	SOIL EROSION CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, OR ANY DISTURBED LAND AREA SHALL BE COMPLETED WITHIN 21 CALENDAR DAYS AFTER FINAL GRADING, OR FINAL EARTH DISTURBANCE, HAS BEEN COMPLETED. DISTURBED AREAS AND STOCKPILES WHICH ARE NOT AT FINAL GRADE BUT WILL REMAIN DORMANT FOR LONGER THAN 30 DAYS SHALL ALSO BE MULCHED WITHIN 21 DAYS AFTER INTERIM GRADING. AN AREA THAT IS GOING TO REMAIN I AN INTERIM STATE FOR MORE THAN 60 DAYS SHALL ALSO BE SEEDED. ALL TEMPORARY SOIL EROSION CONTROL MEASURES AND BMPS SHALL BE MAINTAINED UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED AND ESTABLISHED.
7.	TEMPORARY SOIL EROSION CONTROL FACILITIES SHALL BE REMOVED AND EARTH DISTURBANCE AREAS GRADED AND STABILIZED WITH PERMANENT SOIL EROSION CONTROL MEASURES PURSUANT TO STANDARDS AND SPECIFICATION PRESCRIBED IN THE DCM VOLUME II AND THE ENGINEERING CRITERIA MANUAL (ECM) APPENDIX I.
8.	ALL PERSONS ENGAGED IN EARTH DISTURBANCE SHALL IMPLEMENT AND MAINTAIN ACCEPTABLE SOIL EROSION AND SEDIMENT CONTROL MEASURES INCLUDING BMPS IN CONFORMANCE WITH THE EROSION CONTROL TECHNICAL STANDARD OF THE DRAINAGE CRITERIA MANUAL (DCM) VOLUME II AND IN ACCORDANCE WITH THE STORMWATER MANAGEMENT PLAI (SWMP).
9.	ALL TEMPORARY EROSION CONTROL FACILITIES INCLUDING BMPS AND ALL PERMANENT FACILITIES INTENDED TO CONTRO EROSION OF ANY EARTH DISTURBANCE OPERATIONS, SHALL BE INSTALLED AS DEFINED IN THE APPROVED PLANS, THE SWMP AND THE DCM VOLUME II AND MAINTAINED THROUGHOUT THE DURATION OF THE EARTH DISTURBANCE OPERATION
10.	ANY EARTH DISTURBANCE SHALL BE CONDUCTED IN SUCH A MANNER SO AS TO EFFECTIVELY REDUCE ACCELERATED SOIL EROSION AND RESULTING SEDIMENTATION. ALL DISTURBANCES SHALL BE DESIGNED, CONSTRUCTED, AND COMPLETED SO THAT THE EXPOSED AREA OF ANY DISTURBED LAND SHALL BE LIMITED TO THE SHORTEST PRACTICAL PERIOD OF TIME.
11.	ANY TEMPORARY OR PERMANENT FACILITY DESIGNED AND CONSTRUCTED FOR THE CONVEYANCE OF STORMWATER AROUND, THROUGH, OR FROM THE EARTH DISTURBANCE AREA SHALL BE DESIGNED TO LIMIT THE DISCHARGE TO A NON-EROSIVE VELOCITY.
12.	CONCRETE WASH WATER SHALL BE CONTAINED AND DISPOSED OF IN ACCORDANCE WITH THE SWMP. NO WASH WATER SHALL BE DISCHARGED TO OR ALLOWED TO RUNOFF TO STATE WATERS, INCLUDING ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR FACILITIES.
13.	EROSION CONTROL BLANKETING IS TO BE USED ON SLOPES STEEPER THAN 3:1.
14.	BUILDING, CONSTRUCTION, EXCAVATION, OR OTHER WASTE MATERIALS SHALL NOT BE TEMPORARILY PLACED OR STORED IN THE STREET, ALLEY, OR OTHER PUBLIC WAY, UNLESS IN ACCORDANCE WITH AN APPROVED TRAFFIC CONTROL PLAN. BMP'S MAY BE REQUIRED BY EL PASO COUNTY ENGINEERING IF DEEMED NECESSARY, BASED ON SPECIFIC CONDITIONS AND CIRCUMSTANCES.
15.	. VEHICLE TRACKING OF SOILS AND CONSTRUCTION DEBRIS OFF-SITE SHALL BE MINIMIZED. MATERIALS TRACKED OFFSI SHALL BE CLEANED UP AND PROPERLY DISPOSED OF IMMEDIATELY.
16.	CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL WASTES FROM THE CONSTRUCTION SITE FOR DISPOSAL IN ACCORDANCE WITH LOCAL AND STATE REGULATORY REQUIREMENTS. NO CONSTRUCTION DEBRIS, TREE SLASH, BUILDING MATERIAL WASTES OR UNUSED BUILDING MATERIALS SHALL BE BURIED, DUMPED, OR DISCHARGED AT THE SITE.
17.	THE OWNER, SITE DEVELOPER, CONTRACTOR, AND/OR THEIR AUTHORIZED AGENTS SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL CONSTRUCTION DEBRIS, DIRT, TRASH, ROCK, SEDIMENT, AND SAND THAT MAY ACCUMULATE IN THE STORM SEWER OR OTHER DRAINAGE CONVEYANCE SYSTEM AND STORMWATER APPURTENANCES AS A RESULT OF SITE DEVELOPMENT.
18.	. THE QUANTITY OF MATERIALS STORED ON THE PROJECT SITE SHALL BE LIMITED, AS MUCH AS PRACTICAL, TO THAT QUANTITY REQUIRED TO PERFORM THE WORK IN AN ORDERLY SEQUENCE. ALL MATERIALS STORED ON—SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER, IN THEIR ORIGINAL CONTAINERS, WITH ORIGINAL MANUFACTURER'S LABELS.
19.	NO CHEMICALS ARE TO BE USED BY THE CONTRACTOR, WHICH HAVE THE POTENTIAL TO BE RELEASED IN STORMWATER UNLESS PERMISSION FOR THE USE OF A SPECIFIC CHEMICAL IS GRANTED IN WRITING BY THE ECM ADMINISTRATOR. I GRANTING THE USE OF SUCH CHEMICALS, SPECIAL CONDITIONS AND MONITORING MAY BE REQUIRED.
20	BULK STORAGE STRUCTURES FOR PETROLEUM PRODUCTS AND OTHER CHEMICALS SHALL HAVE ADEQUATE PROTECTION. SO AS TO CONTAIN ALL SPILLS AND PREVENT ANY SPILLED MATERIAL FROM ENTERING STATE WATERS, INCLUDING ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR FACILITIES.
21	. NO PERSON SHALL CAUSE THE IMPEDIMENT OF STORMWATER FLOW IN THE FLOW LINE OF THE CURB AND GUTTER OR IN THE DITCHLINE.
22	. INDIVIDUALS SHALL COMPLY WITH THE "COLORADO WATER QUALITY CONTROL ACT" (TITLE 25, ARTICLE 8, CRS), AND TH "CLEAN WATER ACT" (33 USC 1344), IN ADDITION TO THE REQUIREMENTS INCLUDED IN THE DCM VOLUME II AND THE ECM APPENDIX I. ALL APPROPRIATE PERMITS MUST BE OBTAINED BY THE CONTRACTOR PRIOR TO CONSTRUCTION (NPDES, FLOODPLAIN, 404, FUGITIVE DUST, ETC.). IN THE EVENT OF CONFLICTS BETWEEN THESE REQUIREMENTS AND LAWS, RULES, OR REGULATIONS OF OTHER FEDERAL, STATE, OR COUNTY AGENCIES, THE MORE RESTRICTIVE LAWS, RULES, OR REGULATIONS SHALL APPLY.
23	. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE AT APPROVED CONSTRUCTION ACCESS POINTS.
24	PRIOR TO ACTUAL CONSTRUCTION THE PERMITEE SHALL VERIFY THE LOCATION OF EXISTING UTILITIES.
25	. A WATER SOURCE SHALL BE AVAILABLE ON SITE DURING EARTHWORK OPERATIONS AND UTILIZED AS REQUIRED TO MINIMIZE DUST FROM EARTHWORK EQUIPMENT AND WIND.
26	THE SOILS REPORT FOR THIS SITE HAS BEEN PREPARED BY TERRACON # 23055071 MAY 30, 2006. AND SHALL BE CONSIDERED A PART OF THESE PLANS.
27	AI LEAST TEN DAYS PRIOR TO THE ANTICIPATED START OF CONSTRUCTION, FOR PROJECTS THAT WILL DISTURB 1 ACR OR MORE, THE OWNER OR OPERATOR OF CONSTRUCTION ACTIVITY SHALL SUBMIT A PERMIT APPLICATION FOR STORMWATER DISCHARGE TO THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT, WATER QUALITY DIVISION. THE APPLICATION CONTAINS CERTIFICATION OF COMPLETION OF A STORMWATER MANAGEMENT PLAN (SWMP), OF WHICH THIS GRADING AND EROSION CONTROL PLAN MAY BE A PART. FOR INFORMATION OR APPLICATION MATERIALS CONTACT:
	COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT WATER QUALITY CONTROL DIVISION WQCD – PERMITS 4300 CHERRY CREEK DRIVE SOUTH DENVER, CO 80246–1530 ATTN: PERMITS UNIT





WQCV SUMMARY	
EPC/URBAN DRAINAGE SAND FILTER BASIN-SEE STD DET.	
WQCV REQUIRED = 915 CF	
WQCV PROVIDED = 1002 CF	
AREA REQUIRED = 402 SF	
AREA PROVIDED = 409 SF	
100 YR OUTLET - CDOT TYPE C INLET TOP OF BOX=6362.24 100 YR WSE = 6362.62 EMERGENCY SPILLWAY EL = 6362.62	

GRADING & EROSION CONTROL PLAN DETAILS LOT 3, PADMARK BUSINESS PARK FIL. NO. 1 JOB NO. 44–031 DATE PREPARED: JUNE 29, 2018 DATE REVISED:





20 BOULDER CRESCENT, SUITE 110 COLORADO SPRINGS, CO 80903 PHONE: 719.955.5485

SHEET 2 OF 3



November 2010 Urban Drainage and Flood Control District

EC-2 Temporary and Permanent Seeding (TS/PS)

Urban Storm Drainage Criteria Manual Volume 3

Common ^ª Name	Botanical Name	Growth Season ^b	Growth Form	Seeds/ Pound	Pounds of PLS/acre
Alakali Soil Seed Mix					
Alkali sacaton	Sporobolus airoides	Cool	Bunch	1,750,000	0.25
Basin wildrye	Elymus cinereus	Cool	Bunch	165,000	2.5
Sodar streambank wheatgrass	Agropyron riparium 'Sodar'	Cool	Sod	170,000	2.5
Jose tall wheatgrass	Agropyron elongatum 'Jose'	Cool	Bunch	79,000	7.0
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	5.5
Total					17.75
Fertile Loamy Soil Seed Mix					
Ephriam crested wheatgrass	Agropyron cristatum 'Ephriam'	Cool	Sod	175,000	2.0
Dural hard fescue	Festuca ovina 'duriuscula'	Cool	Bunch	565,000	1.0
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Sodar streambank wheatgrass	Agropyron riparium 'Sodar'	Cool	Sod	170,000	2.5
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	7.0
Total					15.5
High Water Table Soil Seed Mix	κ.				
Meadow foxtail	Alopecurus pratensis	Cool	Sod	900,000	0.5
Redtop	Agrostis alba	Warm	Open sod	5,000,000	0.25
Reed canarygrass	Phalaris arundinacea	Cool	Sod	68,000	0.5
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Pathfinder switchgrass	Panicum virgatum 'Pathfinder'	Warm	Sod	389,000	1.0
Alkar tall wheatgrass	Agropyron elongatum 'Alkar'	Cool	Bunch	79,000	5.5
Total					10.75
Transition Turf Seed Mix ^c					
Ruebens Canadian bluegrass	Poa compressa 'Ruebens'	Cool	Sod	2,500,000	0.5
Dural hard fescue	Festuca ovina 'duriuscula'	Cool	Bunch	565,000	1.0
Citation perennial ryegrass	Lolium perenne 'Citation'	Cool	Sod	247,000	3.0
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Total					7.5

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Common Name	Botanical Name	Growth Season ^b	
Sandy Soil Seed Mix			
Blue grama	Bouteloua gracilis	Warm	
Camper little bluestem	Schizachyrium scoparium 'Camper'	Warm	
Prairie sandreed	Calamovilfa longifolia	Warm	
Sand dropseed	Sporobolus cryptandrus	Cool	
Vaughn sideoats grama	nghn sideoats grama <i>Waughn'</i>		
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	
Total			
Heavy Clay, Rocky Foothill Seed	Mix		
Ephriam crested wheatgrass ^d	Agropyron cristatum 'Ephriam'	Cool	
Oahe Intermediate wheatgrass	Agropyron intermedium 'Oahe'	Cool	
Vaughn sideoats grama ^e	Bouteloua curtipendula 'Vaughn'	Warm	
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	
Total			

If site is to be irrigated, the transition turf seed rates should be doubled.

TS/PS-4

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June 2012

CWA-3

GRADING & EROSION CONTROL PLAN DETAILS LOT 3, PADMARK BUSINESS PARK FIL. NO. 1 JOB NO. 44-031 DATE PREPARED: JUNE 29, 2018 DATE REVISED: EL PASO COUNTY FILE NO. PPR 18-000



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Markup Summary



veyed to the starter quality facility (Design Point 1) is discharged from 2014 2 527 (COT ordief box and an 18 R CP to an infine system Lot 3, 2 and 1 that ultimately confilis to an existing public 48° storm Lot 1. The proposed 18° RCP could rep from the sand filter basin posed at the southwest corter of Lot 3. The proposed Type II manhole P was subbed at the norther and f Lot 2. The existing caped 18° RCP of Lot 2 discharges to an existing Type II manhole at the southwest 4 discharges to an existing Type II manhole at the southwest