MEMORANDUM

TO: Watermark Residential

111 Monument Circle, Suite 1600 Indianapolis, Indiana 46204

FROM: Adam Maxwell, PE, PTOE

Brandon Wilson SM ROCHA, LLC

DATE: April 1, 2021

SUBJECT: Watermark Akers Drive—Transportation Memorandum

Engineering Review

05/14/2021 5:07:19 PM dsdrice

JeffRice@elpasoco.com (719) 520-7877

EPC Planning & Community Development Department

See comment memo also.

Analysis Objective

As requested by El Paso County, this memorandum and attached information is provided to summarize our additional analysis and discussion regarding:

- County's site distance and access spacing requirements along Akers Drive with respect to two proposed site access drive locations.
- Traffic operations at the Hunter Jumper Drive intersection with Akers Drive, if the northern
 access is the only public access to the development. It is understood that with this scenario
 the secondary access to the south would be emergency access only and gated.
- Cost sharing information for the future eastbound left turn lane improvements at the Akers Drive / Constitution Avenue intersection.

Site Description

The proposed development is understood to entail the new construction of 300 multifamily dwelling units accommodated within multiple three-story buildings and supported by various property amenities including a clubhouse and swimming pool.

The approved Watermark Akers Drive traffic impact study¹ (TIS provided in Appendix B) outlines that proposed access to the development is provided at the following locations: one full-movement access onto Akers Drive, serving as an extension of Hunter Jumper Drive east of Akers Drive (referred to as Access A), and one right-in/right-out access onto Akers Drive (referred to as Access B). Access B is approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive, centerline to centerline.

Site Distance and Access Spacing

Access spacing requirements along Akers Drive are to be based on the County's Engineering Criteria Manual (ECM).

An evaluation of access spacing requirements, pursuant to Section 2.4.1 and Tables 2-34 through 2-36 of the County's ECM, reveals that minimum access spacing along Akers Drive shall be equal to the entering site distance for a posted speed limit of 35 MPH, or 350 feet. Considering the uphill grade that exists along Akers Drive, this minimum length may be adjusted pursuant to adjustment factors in Table 2-34 of the County's ECM.

The above standards were then applied to the two proposed site plan access locations.

• Access A (Hunter Jumper Drive extension)

Is there an adjustment for southbound traffic on Akers?

- Per Table 2-35 of the County's ECM, access spacing standard met for 35 MPH posted speed limit and without adjustment factors.
- Access B (right-in/right-out)
 - Centennial Boulevard and Akers Drive intersection is visible from proposed rightin/right-put access.
 - Considering average vehicle turning speeds and the uphill grade along Akers Drive, motorists turning from Centennial Boulevard onto northbound Akers Drive are expected to be traveling slower than the posted speed of 35 MPH. Per Table 2-35 of the County's ECM, access spacing standard of 250 feet can be met for 25 MPH travel speed conditions without considering grade adjustment factors.

Considering the items mentioned above, it is believed that adequate access spacing is provided along Akers Drive.

Proposed site access spacing is shown in Figure 1.

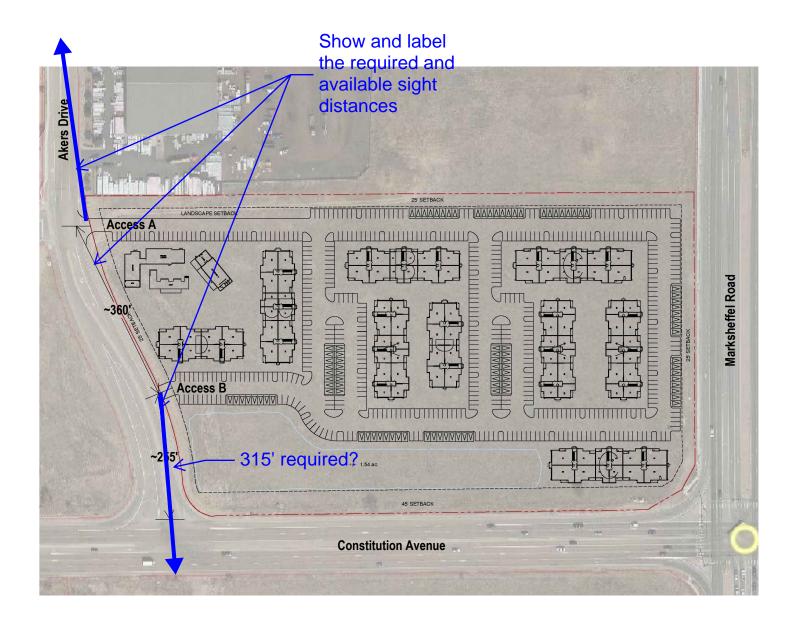
What about through-traffic when the intersection is completed?

Constitution Ave.

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¹ Watermark Akers Drive: Traffic Impact Study, SM ROCHA, LLC, February 2021.







Additional Analysis - Traffic Operations with One Site Access

At the request of County Staff, the analysis presented in this memorandum considers the Akers Drive intersection with Access A (extension of Hunter Jumper Drive) as the only public access for the proposed development. The proposed southern right-in/right-out access will be considered as a gated emergency-only access and not considered within this analysis.

Trip Generation

Trip generation information from the approved traffic study is shown in Table 1 below.

Table 1 – Trip Generation Summary

			TOTAL TRIPS GENERATED							
ITE		24	AM	PEAK HO	DUR	PM	PEAK HO	DUR		
CODE	LAND USE	SIZE	HOUR	ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL	
221	Multifamily Housing (Mid-Rise)	300 DU	1,632	28	80	108	81	51	132	
		Total:	1,632	28	80	108	81	51	132	

Note: All data and calculations above are subject to being rounded to nearest value.

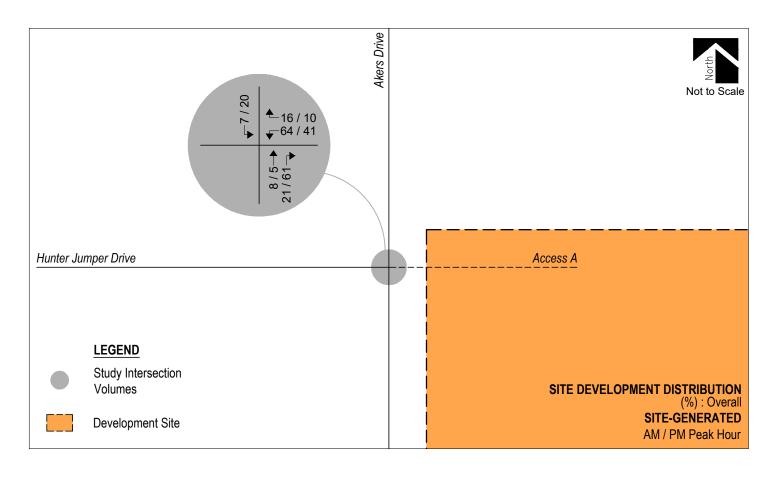
Upon build-out, Table 1 illustrates that the proposed development has the potential to generate approximately 1,632 daily trips with 108 of those occurring during the morning peak hour and 132 during the afternoon peak hour.

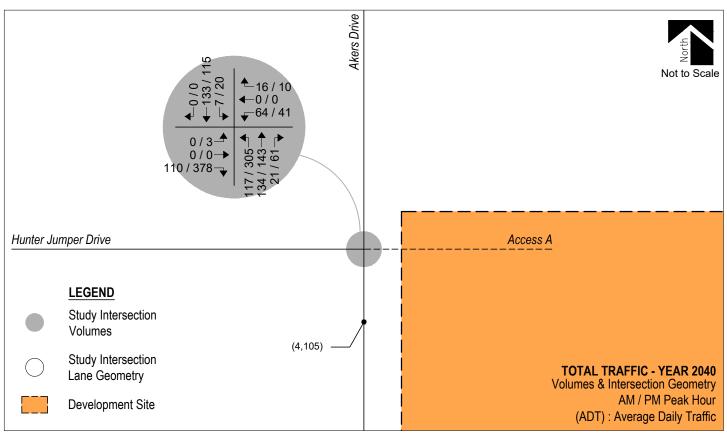
Trip Distribution and Re-Assignment

The overall directional distribution of site-generated traffic was based on the distribution presented in the approved Watermark Akers Drive TIS and re-assigned to utilize one site access at the Hunter Jumper Drive intersection.

Applying re-evaluated trip distribution patterns to site-generated traffic, re-assignment of site traffic is shown on Figure 2.

Site-generated traffic was then added to Year 2040 background traffic in order to develop total traffic projections. Projected Year 2040 total traffic volumes and intersection geometry are shown in Figure 2.







Peak Hour Intersection Levels of Service

The analyses and procedures described in this memorandum were performed in accordance with the Highway Capacity Manual (HCM) and are based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, the study intersections are likely to operate with traffic conditions better than those described within this memorandum, which represent the peak hours of weekday operations only.

Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers.

The operations of the study intersection were analyzed under projected total traffic conditions using the SYNCHRO computer program. Analysis included heavy vehicle percentages as shown in the peak hour traffic count data for the intersection. AM peak hour data shows 9.5% and 22.1% heavy vehicles for the northbound and southbound through movements, respectively. PM peak hour data shows 26.2% and 1.4% heavy vehicles for the northbound and southbound through movements, respectively.

Year 2040 total traffic level of service analysis results are summarized in Table 2. Intersection capacity worksheets are provided in Appendix A.

Table 2 – Intersection Capacity Analysis Summary – Total Traffic – Year 2040

INTERSECTION	LEVEL OF	SERVICE
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR
Hunter Jumper Drive / Akers Drive (Stop-Controlled)		
Eastbound Left	Α	D
Eastbound Through	Α	Α
Eastbound Right	Α	В
Westbound Left	С	F
Westbound Through and Right	A	Α
Northbound Left	Α	Α
Southbound Left	A	Α

Key: Stop-Controlled Intersection: Level of Service

Total Traffic Analysis Results Upon Development Build-Out

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at or better than LOS C during the morning peak traffic hour and LOS D or better during the afternoon peak traffic hour. The exception includes the westbound left turning movement which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

Compared to the Year 2040 total traffic intersection capacity analysis summary within the approved Watermark Akers Drive TIS, removal or restriction of the southern access is not expected to significantly change the operations of the Hunter Jumper Drive and Akers Drive intersection. Heavy vehicles were considered in the operational analysis are not shown to have a negative impact at the northern site access location

In review of the site plan for the propose development, it is not expected that many of the vehicle trips exiting at the right-in/right-out access would be returning to Constitution Avenue. Motorists desiring to access Constitution Avenue are anticipated to exit the site at the Hunter Jumper Drive extension and turn southbound onto Akers Drive to access Constitution Avenue.

It is not recommended that development access onto Akers Drive be more limited than that proposed with two locations. Limited access will interfere with the development's ability to equally distribute traffic within the site and out to available roadways, thus possibly impacting existing and future traffic in the surrounding area. This may have the potential to cause the adjacent roadway network to be used in a manner not intended for or cause additional delay that could impact emergency response times.

- this doesn't make sense

Turn Lane Improvements, Estimated Construction Costs and Contribution

Referencing Table 8 from the approved Watermark Akers Drive TIS, the existing eastbound left turn lane along Constitution Avenue at Akers Drive is recommended to be extended when 95th percentile queuing exceeds the existing turn lane length.

Vehicle storage for the eastbound left turn lane can be increased by extending the left turn bay more to the west. Based upon available aerial imagery, approximately 135 additional feet of turn bay could be constructed while maintaining the minimum median width required for the roadway section.

Estimated construction costs for removal of a portion of the existing roadway median and construction of the eastbound turn lane extension is shown below. Cost information was provided by Watermark Residential.

• Traffic Control = \$5,000

Qemolition = \$4,500

Asphalt Patching/Pavement = \$2,000

Concrete Curb and Gutter = \$3,000

Striping \$1,000 Total =\$15,500 These seem low. Use the FAE form and include all items with standard costs (separate from the FAE form for actual construction).

In coordination with County Staff, developer contribution for the improvement shall be based on a vehicle volume perspective. Comparison of site-generated traffic to background traffic for the eastbound to northbound left turn movement indicates an approximate 12 percent construction contribution/responsibility for the eastbound left turn lane improvement. This equates to a value of \$1,860.

Analysis Conclusion

Staff will verify whether escrow will be required for such a small contribution.

This memorandum assessed site distance and access spacing requirements along Akers Drive with proposed site accesses, provided additional analysis to evaluate intersection operations at the Akers Drive and Hunter Jumper Drive intersection considering changes to proposed site access drives, and considered potential impacts to the adjacent roadway network.

When considering the site sensitive traffic conditions presented in this memorandum, it is our professional opinion that the proposed site access drives meet access spacing requirements defined with the County's ECM. Analysis of future traffic conditions indicates that the conditions assumed are expected to create no negative impact to traffic operations at the study intersection. Conclusions and recommendations as presented in the approved Watermark Akers Drive TIS remain valid.

We trust that our findings will assist in the planning and approval of traffic items addressed in this memorandum related to the Watermark Akers Drive development. Please contact us should further assistance be needed.

Sincerely, Adam Maxwell, PE, PTOE Senior Traffic Engineer

SM ROCHA, LLC

Traffic and Transportation Consultants



APPENDIX A Capacity Worksheets

The following information can be found in the <u>Highway Capacity Manual</u>, Transportation Research Board, 2016: Chapter 19 – Signalized Intersections and Chapter 20 – Two-Way Stop Controlled Intersections.

<u>Automobile Level of Service (LOS) for Signalized Intersections</u>

Levels of service are defined to represent reasonable ranges in control delay.

LOS A

Describes operations with a control delay of 10 s/veh or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

LOS C

Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual *cycle failures* (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

LOS D

Describes operations with control delay between 35 and 55 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F

Describes operations with control delay exceeding 80 s/veh or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

Level of Service (LOS) for Unsignalized TWSC Intersections

Level of Service (v/c ≤ 1.0)	Average Control Delay (s/veh)
А	0 - 10
В	> 10 - 15
С	> 15 - 25
D	> 25 - 35
E	> 35 - 50
F	> 50

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u></u>	7	ሻ	֔		ሻ	<u></u>	7	ሻ	<u></u>	7
Traffic Vol, veh/h	0	0	110	64	0	16	117	126	21	7	133	0
Future Vol, veh/h	0	0	110	64	0	16	117	126	21	7	133	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	10	2	2	22	2
Mvmt Flow	0	0	120	70	0	17	127	137	23	8	145	0
Major/Minor I	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	572	575	145	612	552	137	145	0	0	160	0	0
Stage 1	161	161	-	391	391	-	-	-	-	-	-	-
Stage 2	411	414	-	221	161	-	-	-	-	-	-	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	448	438	902	419	452	972	1437	-	-	1439	-	-
Stage 1	841	765	-	655	616	-	-	-	-	-	-	_
Stage 2	638	600	-	781	765	-	-	-	-	-	-	-
Platoon blocked, %	1	1		1	1	1		-	-	1	-	_
Mov Cap-1 Maneuver	408	397	902	337	410	972	1437	-	-	1439	-	-
Mov Cap-2 Maneuver	408	397	-	337	410	-	-	-	-	-	-	-
Stage 1	767	760	-	598	561	-	-	-	-	-	-	-
Stage 2	571	547	-	674	760	-	-	-	-	-	-	-
J -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.6			16.5			3.4			0.4		
HCM LOS	7.0 A			C			J.∃T			J.7		
TOW LOO	A											
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	FRI n1	FRI n2	FRI n31	WBLn1V	VRI n2	SBL	SBT	SBR
Capacity (veh/h)	II.	1437	-	NDK	LDLIII	LDLIIZ	902	337	972	1439	JD I	JUK
HCM Lane V/C Ratio		0.088		-	-	-		0.206			-	-
		7.7	-	-	0		9.6	18.4	8.8	7.5		-
HCM Control Delay (s) HCM Lane LOS		7.7 A	-	-	A	0	9.6 A	18.4 C	8.8 A		-	-
HCM 95th %tile Q(veh	١	0.3	-	-	A -	A	0.5	0.8	0.1	A 0	-	-
HOW FOUT WITH Q(VEH)		0.3	-	-	-	-	0.5	0.0	0.1	U		-

Intersection													
Int Delay, s/veh	11.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ሻ	<u></u>	7	ሻ	(î		ሻ	<u></u>	7	ሻ	<u></u>	7	
Traffic Vol, veh/h	3	0	378	41	0	10	305	138	61	20	115	0	
uture Vol, veh/h	3	0	378	41	0	10	305	138	61	20	115	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
T Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-	
eh in Median Storage	.,# -	0	-	-	0	-	-	0	-	-	0	-	
rade, %	-	0	-	-	0	-	-	0	-	-	0	-	
eak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
eavy Vehicles, %	2	2	2	2	2	2	2	26	2	2	2	2	
mt Flow	3	0	411	45	0	11	332	150	66	22	125	0	
VIIIC I IOVV	J	· ·		10	U		002	100	00	22	120	U	
ajor/Minor N	Minor2			Minor1		1	Major1			Major2			
onflicting Flow All	1022	1049	125	1189	983	150	125	0	0	216	0	0	
Stage 1	169	169	123	814	814	130	125	-	-	210	-	-	
Stage 2	853	880	-	375	169	-	-	-	-	-	-	-	
tical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12		-	
,	6.12	5.52	0.22		5.52	0.22	4.12	-	-		-	-	
tical Hdwy Stg 1			-	6.12		-	-	-	-	-	-	-	
tical Hdwy Stg 2	6.12	5.52	2 24 0	6.12	5.52	- 2.210	- 210	-	-	-	-	-	
llow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
t Cap-1 Maneuver	210	219	926	156	242	*975	1462	-	-	1374	-	-	
Stage 1	833	759	-	369	381	-	-	-	-	-	-	-	
Stage 2	349	353	•	646	759	-	-	-	-	-	-	-	
atoon blocked, %	1	1		1	1	1		-	-	1	-	-	
ov Cap-1 Maneuver	169	167	926	71	184	*975	1462	-	-	1374	-	-	
ov Cap-2 Maneuver	169	167	-	71	184	-	-	-	-	-	-	-	
Stage 1	644	747	-	285	295	-	-	-	-	-	-	-	
Stage 2	267	273	-	354	747	-	-	-	-	-	-	-	
pproach	EB			WB			NB			SB			
CM Control Delay, s	12.1			96.6			5			1.1			
CM LOS	В			F									
inor Lane/Major Mvm	ıt	NBL	NBT	NBR	EBLn1	EBLn2	EBLn3V	VBLn1V	VBLn2	SBL	SBT	SBR	
apacity (veh/h)		1462	-	-	169	-	926	71	975	1374	-	-	
CM Lane V/C Ratio		0.227	-	-	0.019	-		0.628		0.016	-	-	
CM Control Delay (s)		8.2	-	-	26.7	0	12	118	8.7	7.7	-	-	
CM Lane LOS		A	_	_	D	A	В	F	A	A	_	-	
CM 95th %tile Q(veh)		0.9	-	-	0.1	-	2.3	2.8	0	0	-	-	
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April 2021

APPENDIX B

Approved Watermark Akers Drive Traffic Impact Study

TRAFFIC IMPACT STUDY

For

Watermark Akers Drive Colorado Springs, Colorado

December 2020 Revised: February 2021

Prepared for:

Watermark Residential 111 Monument Circle, Suite 1600 Indianapolis, Indiana 46204

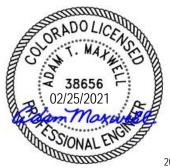
Prepared by:



8703 Yates Drive, Suite 210 Westminster, Colorado 80031 (303) 458-9798

6 South Tejon Street, Suite 515 Colorado Springs, Colorado 80903 (719) 203-6639

> Traffic Engineers: Brandon Wilson Adam Maxwell, PE, PTOE



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I. Introduction

Project Overview

This traffic impact study addresses the capacity, geometric, and control requirements associated with the development entitled Watermark Akers Drive.

This traffic impact study has been revised to address El Paso County review comments dated 02/08/2021 regarding various review comments throughout.

This proposed residential development consists of various multifamily residential buildings. The development is located on the northwest corner of the Constitution Avenue with Marksheffel Road intersection in Colorado Springs, Colorado.

Study Area Boundaries

The study area to be examined in this analysis encompasses the Akers Drive intersections with Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, as well as the Marksheffel Road intersections with Constitution Avenue and Electronic Drive, and intersections with proposed site accesses.

Figure 1 illustrates location of the site and study intersections.

Site Description

Land for the development is currently vacant and surrounded by a mix of open space, residential, light industrial, and commercial land uses. It is understood land for the development is currently within the jurisdiction of El Paso County.

The proposed development is understood to entail the new construction of 300 multifamily dwelling units accommodated within multiple three-story buildings and supported by various property amenities including a clubhouse and swimming pool.

Proposed access to the development is provided at the following locations: one full-movement access onto Akers Drive, serving as an extension of Hunter Jumper Drive east of Akers Drive (referred to as Access A), and one right-in/right-out access onto Akers Drive (referred to as Access B). Access B is approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive.

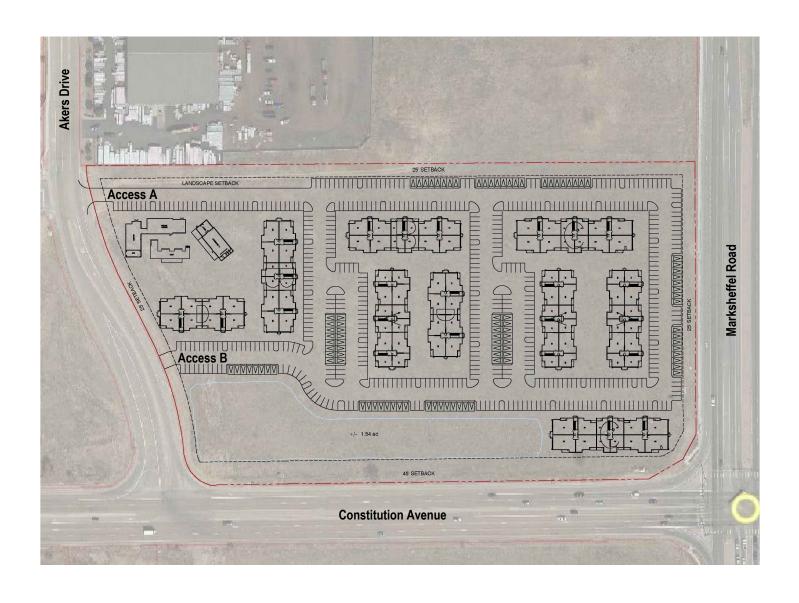
For purposes of this study, it is anticipated that development construction would be completed by end of Year 2022. A conceptual site plan, as prepared by Watermark Residential, is shown on Figure 2. This plan is provided for illustrative purposes.

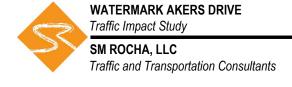




WATERMARK AKERS DRIVE *Traffic Impact Study* Figure 1
SITE LOCATION







Existing and Committed Surface Transportation Network

Within the study area, Constitution Avenue and Marksheffel Road are the primary roadways that will accommodate traffic to and from the proposed development. Secondary roadways include Akers Drive, Electronic Drive, and Hunter Jumper Drive. A brief description of each roadway, based on the City's major transportation thoroughfare plan¹ and design standards², and the County's 2016 Major Transportation Corridors Plan (MTCP)³ and Engineering Criteria Manual (ECM)⁴, is provided below:

<u>Marksheffel Road</u> is a north-south principal arterial roadway having four through lanes (two lanes in each direction) with exclusive turn lanes at the intersections within the study area. Marksheffel Road provides a posted speed limit of 50 MPH.

<u>Constitution Avenue</u> is an east-west principal arterial roadway having a varying number of through lanes (two to three lanes in each direction) with exclusive turn lanes at the intersections within the study area. Constitution Avenue provides a posted speed limit of 50 MPH.

Akers Drive is a north-south roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. Akers Drive provides a posted speed limit of 35 MPH. Akers Drive is unclassified in both the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards, the roadway's estimated ROW width, and connection to Constitution Avenue, Akers Drive is assumed to be classified as a collector roadway within Colorado Springs. Pursuant to Section 2.2.4 of the County's ECM, Akers Drive is assumed to be classified as an urban nonresidential collector roadway within El Paso County.

<u>Hunter Jumper Drive</u> is an east-west roadway having two through lanes (one lane in each direction) with exclusive turn lanes at the intersection within the study area. Hunter Jumper Drive is unclassified in the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards and the roadway's estimated ROW width, Hunter Jumper Drive is assumed to be classified as a collector roadway within Colorado Springs with a posted speed limit of 30 MPH. Pursuant to Section 2.2.4 of the County's ECM, Hunter Jumper Drive is assumed to be classified as an urban residential collector roadway within El Paso County with a posted speed limit of 35 MPH.

¹ Major Thoroughfare Plan, City of Colorado Springs, August 2011.

² Engineering Criteria Manual, Section III: Traffic Criteria Manual, City of Colorado Springs City Engineering, July 2010.

³ El Paso County 2016 Major Transportation Corridors Plan Update, Felsburg Holt & Ullevig, December 2016.

⁴ El Paso County Engineering Criteria Manual, El Paso County, December 2016.

Electronic Drive is an east-west roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. Electronic Drive provides a posted speed limit of 30 MPH. Electronic Drive is unclassified in the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards, the roadway's estimated ROW width, connection to Marksheffel Road, and service to various industrial land uses, Electronic Drive is assumed to be classified as an industrial roadway within Colorado Springs. Pursuant to Section 2.2.4 of the County's ECM, Electronic Drive is assumed to be classified as a rural local roadway within El Paso County.

The study intersection of Marksheffel Road with Constitution Avenue is signalized. All other study intersections operate under a stop-controlled condition. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

Comparison of existing roadway cross-sections of Marksheffel Road and Constitution Avenue to the City's design standards concludes that both roadways are not built to their ultimate width for accommodation of future regional transportation demands. Both roadways are envisioned to become six-lane roadways (three through lanes in each direction).

II. Existing Traffic Conditions

Morning (AM) and afternoon (PM) peak hour traffic counts were collected at the Akers Drive intersections of Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, and at the Marksheffel Road intersections of Constitution Avenue and Electronic Drive. Average daily (24-hour) traffic volumes were collected on Marksheffel Road, Constitution Avenue, and Akers Drive. These counts are shown on Figure 3.

It should be noted that due to the effects of the COVID-19 pandemic, traffic volumes collected may not accurately represent peak hour and 24-hour traffic volumes under normal conditions. Therefore, in order to more accurately represent existing traffic volumes under normal conditions, average daily traffic volumes along Marksheffel Road, provided from the City's GIS web mapping application⁵, were referenced and grown at a two percent annual growth rate to Year 2020. Comparing the calculated 24-hour volume to the collected count data concludes that the collected count data represents higher traffic volumes. Therefore, collected count data is considered to adequately represent traffic volumes under normal conditions. These counts are shown on Figure 3.

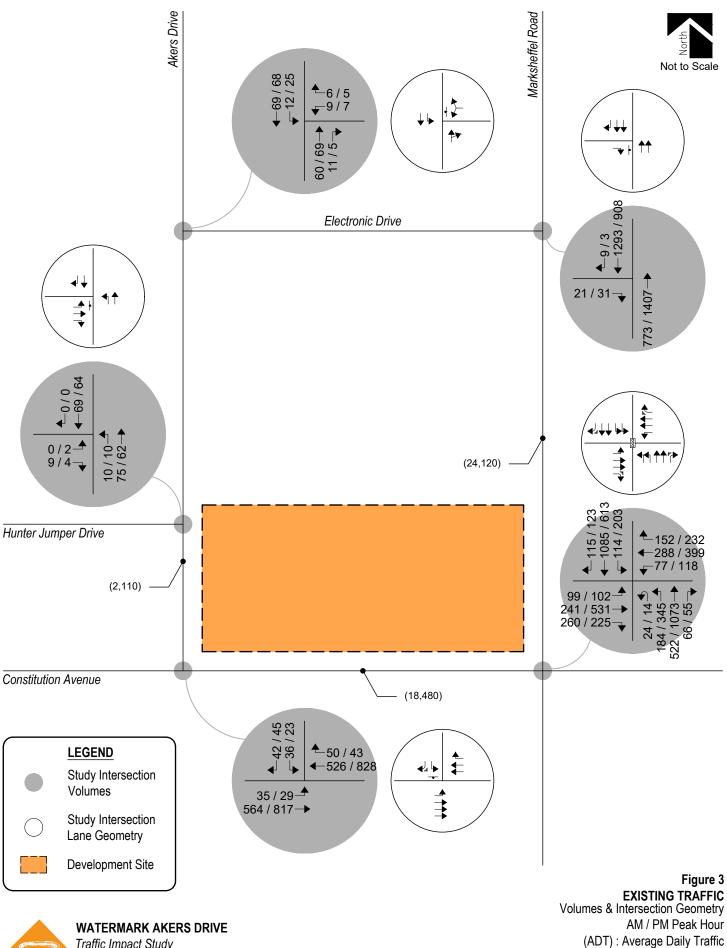
It should also be noted that a significant number of U-turn vehicles are present at the intersection of Constitution Avenue with Marksheffel Road. It is observed that these U-turns utilize the existing northbound left turn lane. These U-turn movements were individually analyzed within this analysis and are shown separately in Figure 3.

Traffic count data is included for reference in Appendix A.

Existing signal timing parameters for the intersection of Marksheffel Road and Constitution Avenue were obtained from City Staff and used throughout this study to the best extent possible in order to remain consistent with existing signal coordination plans. City signal timing information received is included for reference in Appendix A.

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⁵ Web Mapping Application, City of Colorado Springs GIS, January 2019.





The Signalized and Unsignalized Intersection Analysis techniques, as published in the Highway Capacity Manual (HCM) by the Transportation Research Board and as incorporated into the SYNCHRO computer program, were used to analyze the study intersections for existing traffic conditions. These nationally accepted techniques allow for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers. These levels of service grades with brief descriptions of the operating condition, for unsignalized and signalized intersections, are included for reference in Appendix B and have been used throughout this study.

The level of service analyses results for existing conditions are summarized in Table 1.

Intersection capacity worksheets developed for this study are provided in Appendix C.

Table 1 – Intersection Capacity Analysis Summary – Existing Traffic

INTERSECTION	LEVEL OF	SERVICE
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR
Constitution Avenue / Marksheffel Road (Signalized)	C (30.7)	D (41.7)
Constitution Avenue / Akers Drive (Stop-Controlled) Eastbound Left Southbound Left Southbound Right	A B A	A B A
Hunter Jumper Drive / Akers Drive (Stop-Controlled) Eastbound Left Eastbound Right Northbound Left	A A A	A A A
Electronic Drive / Akers Drive (Stop-Controlled) Westbound Left and Right Southbound Left	A A	A A
Electronic Drive / Marksheffel Road (Stop-Controlled) Eastbound Right	С	В

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

Existing Traffic Analysis Results

Under existing conditions, operational analysis shows that the signalized intersection of Constitution Avenue with Marksheffel Road has overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The unsignalized intersection of Constitution Avenue with Akers Drive has turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road has turn movement operations at LOS C during the morning peak traffic hour and LOS B during the afternoon peak traffic hour.

III. Future Traffic Conditions Without Proposed Development

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by development of vacant parcels in the area.

To account for projected increases in background traffic for Years 2022 and 2040, a compounded annual growth rate of approximately two percent was applied to existing traffic volumes. This annual growth rate is consistent with regional growth projections and the level of in-fill development expected within the area.

To account for projected traffic from adjacent developments not yet built, trip generations from The Sands traffic impact study⁶ were added to Year 2022 background traffic volumes, while Filings 5-8 and Phases 9-10 from the Hannah Ridge at Feathergrass traffic impact studies^{7,8} were added to Year 2040 background traffic volumes. It should be noted that the Hannah Ridge at Feathergrass traffic impact study, dated September 2017, originally assumed various retail land uses for the same development area currently proposed with this project, and included a right-in/right-out access into the site. This access is no longer proposed with this development, therefore ingress and egress traffic volumes originally anticipated to utilize the right-in/right-out access were not added to background traffic volumes.

A signal warrant analysis, using 2040 background traffic volumes and upon the assumed extension of Akers Drive south of Constitution Avenue, was conducted for the Akers Drive intersection with Constitution Avenue in order to review potential for traffic signal control. Analysis results conclude that the intersection was found to be above the minimum vehicle volumes required to meet Warrant 3 – Peak Hour, from the Manual on Uniform Traffic Control Devices (MUTCD), for the installation of a traffic signal. As such, and consistent with assumptions defined within the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study, the intersection was analyzed under traffic signal control by Year 2040. Warrant study worksheets are provided for reference in Appendix D.

Warrant 3 is intended for use at locations where traffic conditions are such that for a minimum of one hour on an average day, the minor-street (Akers Drive) traffic suffers undue delay when entering or crossing the major street (Constitution Avenue). This assumption provides for a conservative analysis. Said intersection should be monitored further by County Staff as area development occurs to determine when signalization installation is appropriate.

Pursuant to the non-committed area roadway improvements discussed in Section I, Year 2022 background traffic conditions assumes no roadway improvements to accommodate regional transportation demands.

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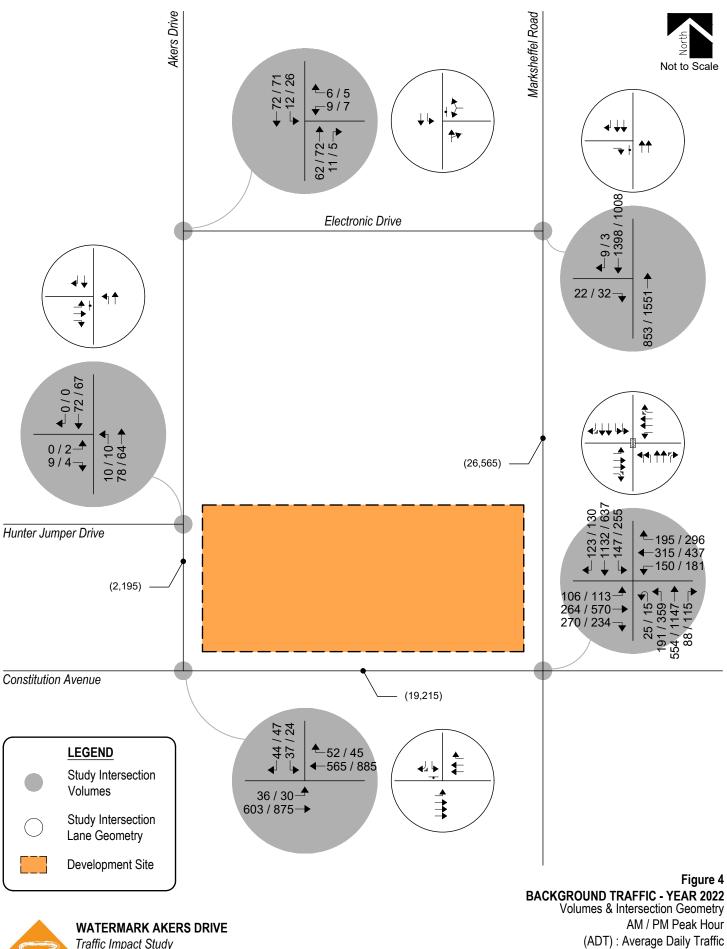
⁶ The Sands, LSC Transportation Consultants, Inc., May 2016.

⁷ Hannah Ridge at Feathergrass Filings 5, 6, and 7, LSC Transportation Consultants, Inc., March 2019.

⁸ Hannah Ridge at Feathergrass Filing Nos. 3 and 4, LSC Transportation Consultants, Inc., September 2017.

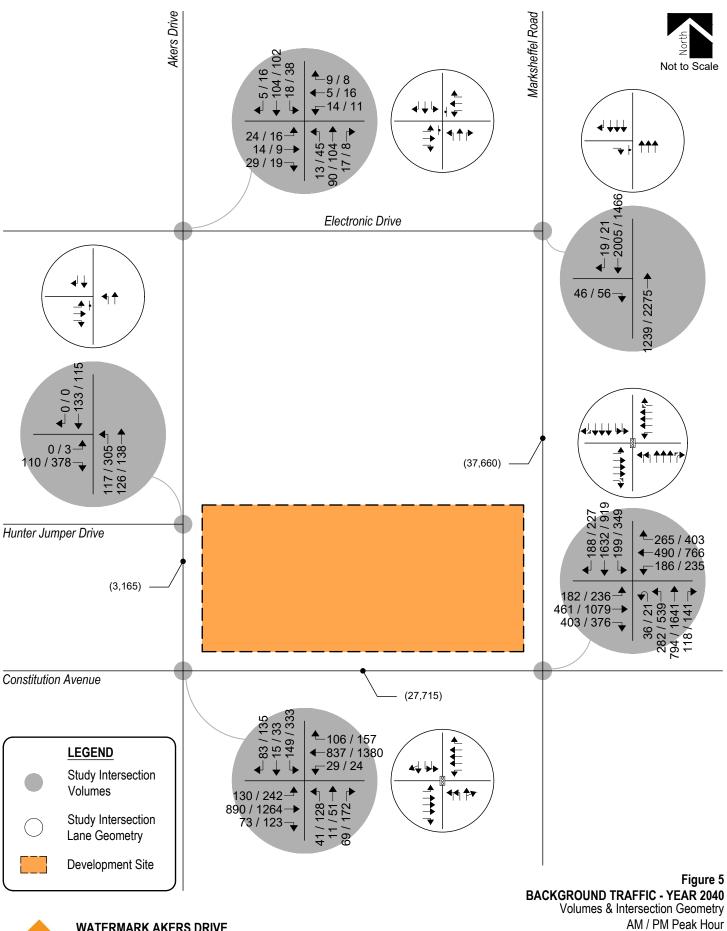
Year 2040 background traffic conditions assume Marksheffel Road and Constitution Avenue will be built out to their ultimate widths to accommodate regional transportation demands. This is consistent with assumptions defined within The Sands traffic impact study. Additionally, pursuant to the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study, it is assumed Akers Drive is planned to extend south of Constitution Avenue in order to serve future commercial land uses within Phase 10 of the Hannah Ridge at Feathergrass development. Similarly, it is assumed Electronic Drive will extend west of Akers Drive to provide access to the future residential land uses within Phases 5 through 8 of said future development. Year 2040 also assumes existing signal timing parameters for the Marksheffel Road and Constitution Avenue intersection with optimized intersection splits in effort to better long-term intersection performance. This assumption provides for a conservative analysis.

Projected background traffic volumes and intersection geometry for Years 2022 and 2040 are shown on Figure 4 and Figure 5, respectively.





Traffic Impact Study





WATERMARK AKERS DRIVE

Traffic Impact Study

(ADT): Average Daily Traffic

As with existing traffic conditions, the operations of study intersections were analyzed under background conditions, without the proposed development, using the SYNCHRO computer program.

Background traffic level of service analysis results for Year 2022 are listed in Table 2. Year 2040 operational results are summarized in Table 3.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 2 – Intersection Capacity Analysis Summary – Background Traffic – Year 2022

INTERSECTION	LEVEL OF	SERVICE
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR
Constitution Avenue / Marksheffel Road (Signalized)	C (33.1)	D (44.9)
Constitution Avenue / Akers Drive (Stop-Controlled) Eastbound Left Southbound Left Southbound Right	A B A	A B A
Hunter Jumper Drive / Akers Drive (Stop-Controlled) Eastbound Left Eastbound Right Northbound Left	A A A	A A A
Electronic Drive / Akers Drive (Stop-Controlled) Westbound Left and Right Southbound Left	A A	A A
Electronic Drive / Marksheffel Road (Stop-Controlled) Eastbound Right	С	С

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

Background Traffic Analysis Results – Year 2022

Year 2022 background traffic analysis indicates that the signalized intersection of Constitution Avenue with Marksheffel Road experiences overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The unsignalized intersection of Constitution Avenue with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive shows turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS C during both morning and afternoon peak traffic hours.

Table 3 – Intersection Capacity Analysis Summary – Background Traffic – Year 2040

INTERSECTION	LEVEL OF	SERVICE		
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR		
Constitution Avenue / Marksheffel Road (Signalized)	D (39.4)	E (56.8)		
Constitution Avenue / Akers Drive (Signalized)	B (13.6)	D (54.3)		
Hunter Jumper Drive / Akers Drive (Stop-Controlled) Eastbound Left Eastbound Right Northbound Left	A A A	C B A		
Electronic Drive / Akers Drive (Stop-Controlled) Eastbound Left Eastbound Right Westbound Left Westbound Through Westbound Right Northbound Left Southbound Left	B B A B B A A	B B A B A A		
Electronic Drive / Marksheffel Road (Stop-Controlled) Eastbound Right	D	С		

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

Background Traffic Analysis Results – Year 2040

By Year 2040 and without the proposed development, the study intersection of Constitution Avenue with Marksheffel Road projects overall operations at LOS D during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during the afternoon peak traffic period is attributed to eastbound and westbound through volumes, as well as left turning movements in all directions.

The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at LOS A during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour.

The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

IV. Proposed Project Traffic

Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, 10th Edition, were applied to the proposed land use in order to estimate average daily traffic (ADT), AM Peak Hour, and PM Peak Hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from a point of origin to a point of destination.

The ITE land use code 221 (Multifamily (Mid-Rise)) was used for estimating trip generation because of its best fit to the proposed land use description.

Trip generation rates used in this study are presented in Table 4.

Table 4 – Trip Generation Rates

			TRIP GENERATION RATES							
ITE		24	AM	PEAK HO	DUR	PM	PEAK HO	OUR		
CODE	LAND USE	UNIT	HOUR	ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL	
221	Multifamily Housing (Mid-Rise)	DU	5.44	0.09	0.27	0.36	0.27	0.17	0.44	

Key: DU = Dw elling Units.

Note: All data and calculations above are subject to being rounded to nearest value.

Table 5 illustrates projected average daily traffic (ADT), AM Peak Hour, and PM Peak Hour traffic volumes likely generated by the proposed development upon build-out.

Table 5 – Trip Generation Summary

			TOTAL TRIPS GENERATED						
ITE			24	AM	PEAK HO	OUR	PM	PEAK HO	DUR
CODE	LAND USE	SIZE	HOUR	ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL
221	Multifamily Housing (Mid-Rise)	300 DU	1,632	28	80	108	81	51	132
		Total:	1,632	28	80	108	81	51	132

Note: All data and calculations above are subject to being rounded to nearest value.

Upon build-out, Table 5 illustrates that the proposed development has the potential to generate approximately 1,632 daily trips with 108 of those occurring during the morning peak hour and 132 during the afternoon peak hour.

Adjustments to Trip Generation Rates

A development of this type is not likely to attract trips from within area land uses nor pass-by or diverted link trips from the adjacent roadway system, therefore no trip reduction was taken in this analysis.

Trip Distribution

The overall directional distribution of site-generated traffic was determined based on the location of development site within the City and County, proposed and existing area land uses, allowed turning movements, available roadway network, and in reference to the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study.

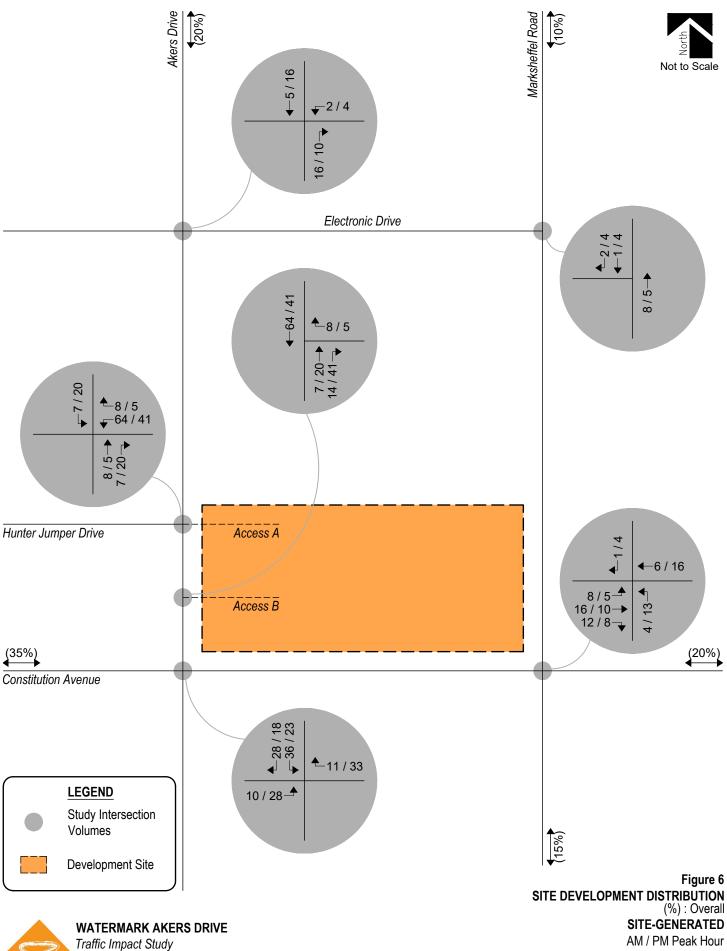
Phase 10 of the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic study anticipated site-generated traffic to travel north and south through the Constitution Avenue and Akers Drive intersection. For purposes of this analysis, consideration of site-generated traffic volumes accessing the assumed future retail south of Constitution Avenue is not considered. Moreover, northbound and southbound site-generated trips along Akers Drive across Constitution Avenue could be considered as internal capture within the overall Hannah Ridge at Feathergrass development. Therefore, site-generated traffic volumes are assumed to travel to and from the greater area beyond the overall Hannah Ridge at Feathergrass development area.

Overall trip distribution patterns for the development are shown on Figure 6.

Trip Assignment

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying trip distribution patterns to site-generated traffic provides the overall site-generated trip assignments shown on Figure 6.



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V. Future Traffic Conditions With Proposed Developments

Site-generated traffic was added to background traffic projections for Years 2022 and 2040 to develop total traffic projections. For analysis purposes, it was assumed that development construction would be completed by end of Year 2022.

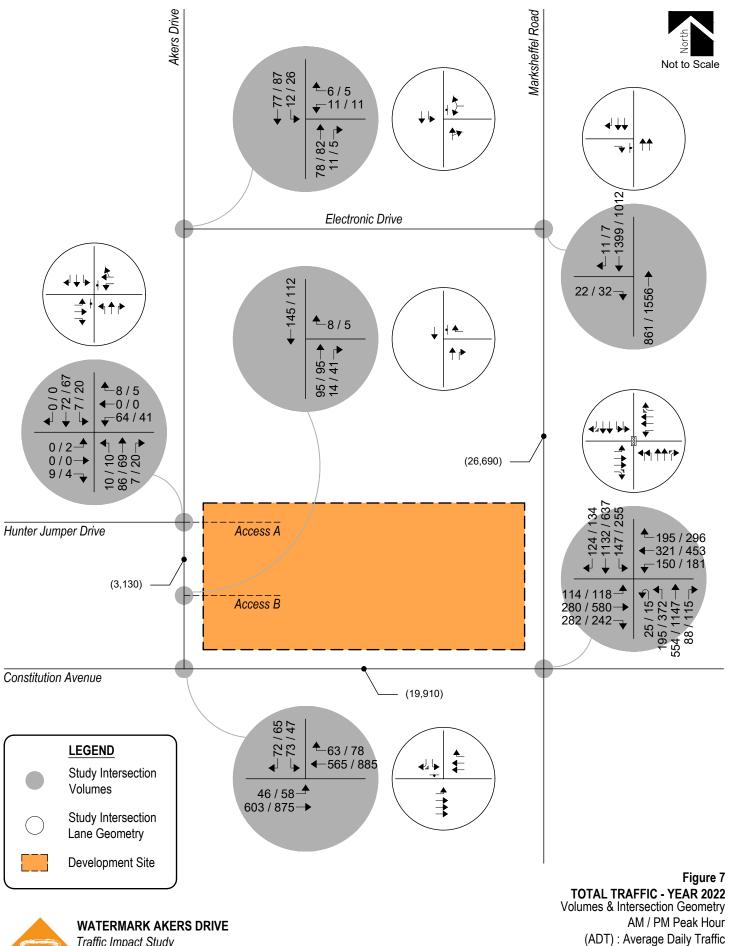
Pursuant to area roadway improvement discussions provided in Section III, Year 2022 and Year 2040 total traffic conditions assume no additional roadway improvements to accommodate regional transportation demands. However, 20 feet of right-of-way will be dedicated along the north side of Constitution Avenue as part of this development. Additional roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

A preliminary site distance evaluation, pursuant to Section 2.4, Tables 2-34, 2-35, and 2-36 of the County's ECM, was evaluated for Access B. As mentioned in Section I, Access B is located approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive. Considering the multifamily land use being proposed and the approximate grade and posted speed limit along Akers Drive, preliminary evaluation indicates that access spacing requirements are satisfied for Access B.

A signal warrant analysis, using 2022 and 2040 total traffic volumes, was conducted for the Akers Drive with Hunter Jumper Drive intersection to review potential for traffic signal control. Analysis results conclude that the intersection does not have the minimum volume required to meet Warrant 3 – Peak Hour, from the MUTCD, for the installation of a traffic signal. As such, the intersection remained a stop-controlled condition. Warrant study worksheets are provided for reference in Appendix D.

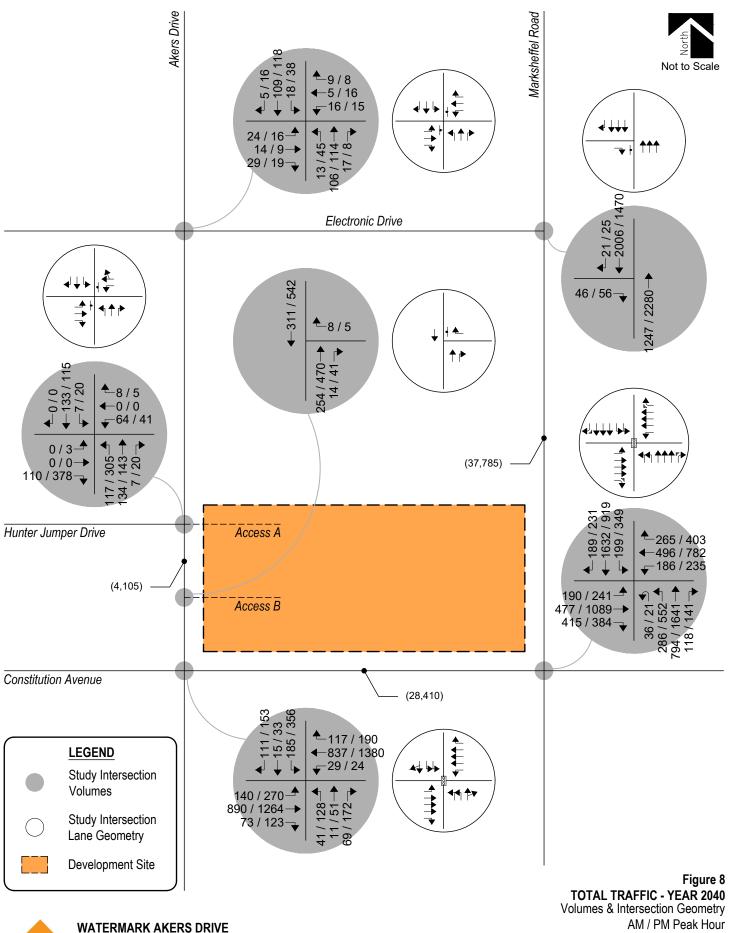
Projected Year 2022 total traffic volumes and intersection geometry are shown in Figure 7.

Figure 8 shows projected total traffic volumes and intersection geometry for Year 2040.





Traffic Impact Study





Traffic Impact Study

(ADT): Average Daily Traffic

VI. Project Impacts

The analyses and procedures described in this study were performed in accordance with the Highway Capacity Manual (HCM) and are based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this study, which represent the peak hours of weekday operations only.

Peak Hour Intersection Levels of Service

As with background traffic, the operations of the study intersections were analyzed under projected total traffic conditions using the SYNCHRO computer program. Total traffic level of service analysis results for Years 2022 and 2040 are summarized in Table 6 and Table 7, respectively.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 6 – Intersection Capacity Analysis Summary – Total Traffic – Year 2022

INTERSECTION	LEVEL OF	SERVICE
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR
Constitution Avenue / Marksheffel Road (Signalized)	C (33.5)	D (45.2)
Constitution Avenue / Akers Drive (Stop-Controlled)		
Eastbound Left	Α	Α
Southbound Left	В	В
Southbound Right	Α	Α
Hunter Jumper Drive / Akers Drive (Stop-Controlled)		
Eastbound Left	Α	В
Eastbound Through	Α	Α
Eastbound Right	Α	Α
Westbound Left	В	В
Westbound Through and Right	Α	Α
Northbound Left	Α	Α
Southbound Left	A	A
Electronic Drive / Akers Drive (Stop-Controlled)		
Westbound Left and Right	Α	Α
Southbound Left	А	А
Electronic Drive / Marksheffel Road (Stop-Controlled)		
Eastbound Right	С	В
Access B / Akers Drive (Stop-Controlled)		
Westbound Right	Α	А

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

Table 7 – Intersection Capacity Analysis Summary – Total Traffic – Year 2040

INTERSECTION	LEVEL OF	SERVICE
LANE GROUPS	AM PEAK HOUR	PM PEAK HOUR
Constitution Avenue / Marksheffel Road (Signalized)	D (39.5)	D (53.8)
Constitution Avenue / Akers Drive (Signalized)	B (14.2)	E (65.9)
Hunter Jumper Drive / Akers Drive (Stop-Controlled) Eastbound Left	A	D
Eastbound Through Eastbound Right Westbound Left	A A C	A B F
Westbound Through and Right Northbound Left Southbound Left	A A A	A A A
Electronic Drive / Akers Drive (Stop-Controlled) Eastbound Left Eastbound Through Eastbound Right Westbound Left Westbound Through Westbound Right Northbound Left Southbound Left	B B A B A A	B B A B A A
Electronic Drive / Marksheffel Road (Stop-Controlled) Eastbound Right	D	С
Access B / Akers Drive (Stop-Controlled) Westbound Right	А	В

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

Total Traffic Analysis Results Upon Development Build-Out

Table 7 illustrates how, by Year 2040 and upon development build-out, the signalized intersection of Constitution Avenue with Marksheffel Road shows an overall LOS D operation during both morning and afternoon peak traffic hours. Compared to the background traffic analysis, the LOS during the afternoon peak traffic hour is shown to improve. This is due to optimized intersection split assumptions mentioned in Section III.

The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during the afternoon peak traffic period is attributed to the eastbound left turn movement, as well as northbound and southbound left and through volumes. In order to mitigate the anticipated LOS E operation, it is recommended allowing for a protective/permissive turn type for the eastbound left turn movement. This is shown to provide overall intersection operations of LOS D during the afternoon peak traffic hour.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at or better than LOS C during the morning peak traffic hour and LOS D or better during the afternoon peak traffic hour. Exceptions would include the westbound left turning movement which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

The unsignalized intersection of Access B with Akers Drive anticipates turn movement operations at LOS A during the morning peak traffic hour and LOS B or better during the afternoon peak traffic hour.

These intersection operations are similar to background conditions.

Pedestrian and Bicycle Accommodations

Watermark Akers Drive development would accommodate pedestrians and bicyclists with the following improvements:

- Pedestrian sidewalks along Constitution Avenue and Akers Drive adjacent to the site in accordance with local jurisdictional standards.
- Sidewalks along all internal local roadways in accordance with local jurisdictional standards.

Queue Length Analysis

Queue lengths for proposed site access intersections were analyzed using Year 2040 total traffic conditions. The analysis yields estimate of 95th percentile queue lengths, which have only a five percent probability of being exceeded during the analysis time period. Queue lengths were modeled and are included with the Synchro worksheets in Appendix C.

No significant queuing at the proposed site accesses was indicated. The greatest on-site queue length anticipated at Access A occurs during the afternoon peak hour. The queue length is approximately three vehicles for the eastbound right and westbound left turn movements.

At the intersection of Constitution Avenue and Akers Drive, afternoon peak hour queuing for the eastbound left turn movement is anticipated to exceed the existing storage length. It is recommended lengthening the existing back-to-back eastbound left turn lane to its maximum allowable length of approximately 450 feet. This is expected to be long enough to accommodate the anticipated 95th percentile queuing. It is emphasized that the anticipated queuing is a result of conservative trips generated by the proposed development and surrounding area, and may only be exceed five percent of the time during the afternoon peak traffic period.

Auxiliary Lane Analysis

Auxiliary lanes for site development accesses were based on the County's ECM and Section III of the City's design standards.

Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7 of the County's standards and Section 8.0, Table 2, of the City's design standards, reveals that left and right turn deceleration lanes are not required along Akers Drive. However, pursuant to existing striping patterns, it is assumed restriping will occur to allow for a northbound right turn deceleration lane at Access B, and right and left turn decelerations lanes at Access A, in order to provide consistency with the existing lane geometry on the west side of Akers Drive.

An evaluation of auxiliary lane requirements for existing turn lanes at the remaining study intersections indicates that exclusive turn lane requirements meet City and County minimum requirements and that no changes are recommended.

Recommended Improvements

Table 8 illustrates the recommended roadway and intersection control improvements associated with the proposed Watermark Akers Drive development and adjacent area.

Table 8 – Recommended Roadway & Intersection Improvements

IMPROVEMENT	TYPE	TIMING	RESPONSIBILITY
Lengthen eastbound left turn lane on Constitution Avenue at Akers Drive	Auxiliary Lane and Median Modification	When 95th Percentile Queuing Exceeds Existing Lane Length	Developments and other trip generators along Akers Drive north of Constitution Avenue
Signalization of Constitution Avenue / Akers Drive	Traffic Signal	When Warranted	Whoever warrants the need; i.e. County, City, or Developer
Construct northbound right turn lanes on Akers Drive at Access A and Access B	Auxiliary Lane	With Final Plat Application(s) / Site Development	Applicant
Construct southbound left turn lane on Akers Drive at Access A	Auxiliary Lane	With Final Plat Application(s) / Site Development	Applicant
Widen Constitution Avenue and Marksheffel Road to six-lane cross-section	Roadway Segment	Shown on MTCP by 2040	Master planned
Extend Akers Drive south of Constitution Avenue	Roadway Segment	With Future Development South of Consitution Avenue	Developments and other trip generators along Akers Drive south of Constitution Avenue
Extend Electronic Drive west of Akers Drive	Roadway Segment	With Future Development West of Akers Drive	Developments and other trip generators along Electronic Drive west of Akers Drive

VII. Conclusion

This traffic impact study addressed the capacity, geometric, and control requirements associated with the development entitled Watermark Akers Drive. This proposed residential development consists of various multifamily residential buildings. The development is located on the northwest corner of the Constitution Avenue with Marksheffel Road intersection in Colorado Springs, Colorado.

The study area examined in this analysis encompassed the Akers Drive intersections with Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, the Marksheffel Road intersections with Constitution Avenue and Electronic Drive, and intersections with proposed site accesses.

A preliminary site distance evaluation for Access B was evaluated. Considering the multifamily land use being proposed and the approximate grade and posted speed limit along Akers Drive, preliminary evaluation indicates that access spacing requirements are satisfied.

Analysis was conducted for critical AM Peak Hour and PM Peak Hour traffic operations for existing traffic conditions, Year 2022 and Year 2040 background traffic conditions, and Year 2022 and Year 2040 total traffic conditions.

Analysis of existing traffic conditions indicates that the signalized intersection of Constitution Avenue with Marksheffel Road has overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The unsignalized intersection of Constitution Avenue with Akers Drive has turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road has turn movement operations at LOS C during the morning peak traffic hour and LOS B during the afternoon peak traffic hour.

Without the proposed development, Year 2022 background operational analysis shows that the signalized intersection of Constitution Avenue with Marksheffel Road experiences overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The unsignalized intersection of Constitution Avenue with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive shows turn movement operations at LOS A during both morning and afternoon peak traffic hours. The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS C during both morning and afternoon peak traffic hours.

By Year 2040 and without the proposed development, the study intersection of Constitution Avenue with Marksheffel Road projects overall operations at LOS D during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during afternoon peak traffic periods is attributed to eastbound and westbound through volumes, as well as left turning movements in all directions. The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at LOS A during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour. The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon roadway and intersection control improvements assumed within this analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at future levels of service comparable to Year 2040 background traffic conditions. Proposed site accesses have long-term operations at LOS D or better during peak traffic periods and upon build-out. Exceptions would include the westbound left turning movement along Hunter Jumper Drive at Akers Drive which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

No significant queuing at the proposed site accesses was indicated. The greatest on-site queue length anticipated at Access A occurs during the afternoon peak hour. The queue length is approximately three vehicles for the eastbound right and westbound left turn movements. At the intersection of Constitution Avenue and Akers Drive, afternoon peak hour queuing for the eastbound left turn movement is anticipated to exceed the existing storage length. It is recommended lengthening the existing back-to-back eastbound left turn lane to its maximum allowable length of approximately 450 feet. This is expected to be long enough to accommodate the anticipated 95th percentile queuing. It is noted that the anticipated queuing is a result of conservative trips generated by the proposed development and surrounding area, and may only be exceed five percent of the time during afternoon peak traffic periods.

An evaluation of auxiliary lane requirements reveals that left and right turn deceleration lanes are not required along Akers Drive. However, pursuant to existing striping patterns, it is assumed restriping will occur to allow for a northbound right turn deceleration lane at Access B, and right and left turn decelerations lanes at Access A, in order to provide consistency with the existing lane geometry on the west side of Akers Drive. For existing turn lanes at the remaining study intersections, exclusive turn lane requirements meet City and County minimum requirements and no changes are recommended.

This site is subject to the El Paso County Road Impact Fee Program (Resolution 19-471), as amended. An option for payment will be selected at the final plat stage.

APPENDIX A

Traffic Count Data Signal Timing Information

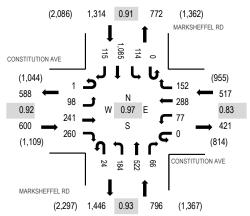


Location: 1 MARKSHEFFEL RD & CONSTITUTION AVE AM

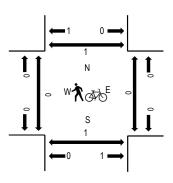
Date: Tuesday, August 11, 2020 **Peak Hour:** 07:00 AM - 08:00 AM

Peak 15-Minutes: 07:15 AM - 07:30 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	CON	STITU		AVE	CON	STITUT Westb	ΓΙΟΝ ΑV ound	Έ	MAI	RKSHEF Northbo		RD	MAI	RKSHE Southl	FFEL I	RD		Rolling	Ped	lestrian	n Crossin	ıas
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru F	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West			0
7:00 AM	0	23	50	67	0	16	68	38	4	34	141	18	0	36	281	29	805	3,227	0	0	0	0
7:15 AM	1	32	70	60	0	31	49	36	5	48	121	17	0	14	315	31	830	3,038	0	0	0	0
7:30 AM	0	19	64	75	0	21	85	49	7	49	145	12	0	30	240	32	828	2,799	0	0	0	0
7:45 AM	0	24	57	58	0	9	86	29	8	53	115	19	0	34	249	23	764	2,548	0	0	0	1
8:00 AM	0	22	64	44	0	17	46	32	2	40	99	15	0	28	177	30	616	2,290	0	0	1	0
8:15 AM	1	22	72	47	0	22	58	40	4	28	84	13	0	22	160	18	591		0	0	0	1
8:30 AM	0	11	66	34	0	10	64	34	4	48	95	13	0	26	148	24	577		0	0	0	0
8:45 AM	0	24	52	50	0	14	57	44	3	33	83	7	0	15	115	9	506		0	0	0	0
Count Total	2	177	495	435	0	140	513	302	37	333	883	114	0	205	1,685	196	5,517		0	0	1	2
Peak Hour	1	98	241	260	0	77	288	152	24	184	522	66	0	114	1,085	115	3,227	,	0	0	0	1

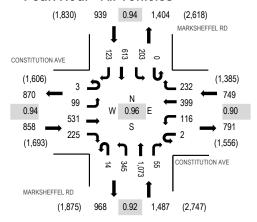


Location: 1 MARKSHEFFEL RD & CONSTITUTION AVE PM

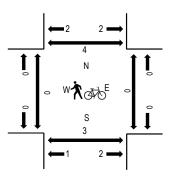
Date: Tuesday, August 11, 2020 **Peak Hour:** 04:15 PM - 05:15 PM

Peak 15-Minutes: 05:00 PM - 05:15 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	CON	STITU Eastb	TION A	AVE		STITUT Westb	TION A\ ound	VΕ	MAI	RKSHE Northb	FFEL Found	RD		RKSHE Southl	FFEL I	RD		Rolling	Ped	estrian	n Crossin	ıgs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru I	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South N	Vorth
4:00 PM	0	22	130	48	0	27	87	42	1	75	273	8	0	43	153	34	943	3,924	0	0	0	0
4:15 PM	0	28	149	50	0	24	105	54	1	88	252	14	0	54	155	28	1,002	4,033	0	0	0	0
4:30 PM	2	19	129	64	1	29	91	53	3	86	252	13	0	50	153	27	972	3,990	0	0	0	0
4:45 PM	0	23	130	51	1	28	114	66	7	98	253	15	0	55	133	33	1,007	3,930	0	0	0	0
5:00 PM	1	29	123	60	0	35	89	59	3	73	316	13	0	44	172	35	1,052	3,731	0	0	0	0
5:15 PM	0	27	141	63	0	25	96	52	4	71	244	15	0	48	147	26	959		0	0	0	0
5:30 PM	0	23	143	50	0	18	92	46	3	71	226	12	0	48	156	24	912		0	0	0	0
5:45 PM	0	26	116	46	0	29	83	39	4	51	194	8	0	53	133	26	808		0	0	0	0
Count Total	3	197	1,061	432	2	215	757	411	26	613	2,010	98	0	395	1,202	233	7,655		0	0	0	0
Peak Hour	3	99	531	225	2	116	399	232	14	345	1,073	55	0	203	613	3 123	3 4,033	}	0	0	0	0

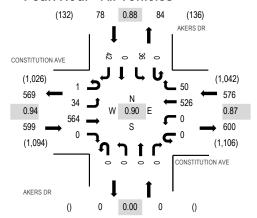


Location: 2 AKERS DR & CONSTITUTION AVE AM

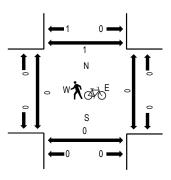
Date: Tuesday, August 11, 2020 **Peak Hour:** 07:00 AM - 08:00 AM

Peak 15-Minutes: 07:30 AM - 07:45 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	CON	STITU Eastb	TION A	AVE	CON	STITUT Westb	TION AV	Æ		AKERS Northb				AKER South				Rolling	Ped	lestrian	n Crossin	ıgs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru F	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South I	North
7:00 AM	0	10	135	0	0	0	115	12	0	0	0	0	0	4	0	8	284	1,253	0	0	0	0
7:15 AM	0	12	148	0	0	0	114	16	0	0	0	0	0	12	0	13	315	1,235	0	0	0	0
7:30 AM	0	5	154	0	0	0	153	13	0	0	0	0	0	11	0	13	349	1,172	0	0	0	0
7:45 AM	1	7	127	0	0	0	144	9	0	0	0	0	0	9	0	8	305	1,078	0	0	0	1
8:00 AM	0	9	115	0	0	0	115	5	0	0	0	0	0	13	0	9	266	1,015	0	0	0	0
8:15 AM	0	2	131	0	0	0	102	7	0	0	0	0	0	6	0	4	252		0	0	0	1
8:30 AM	0	3	107	0	0	0	122	13	0	0	0	0	0	5	0	5	255		0	0	0	1
8:45 AM	0	9	119	0	0	0	98	4	0	0	0	0	0	10	0	2	242		0	0	0	0
Count Total	1	57	1,036	0	0	0	963	79	0	0	0	0	0	70	0	62	2,268		0	0	0	3
Peak Hour	1	34	564	0	0	0	526	50	0	0	0	0	0	36	6 () 4:	2 1,253	}	0	0	0	1

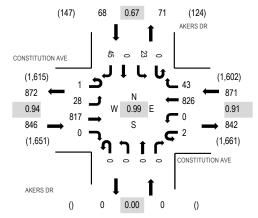


Location: 2 AKERS DR & CONSTITUTION AVE PM

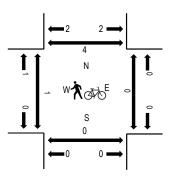
Date: Tuesday, August 11, 2020 **Peak Hour:** 04:15 PM - 05:15 PM

Peak 15-Minutes: 04:30 PM - 04:45 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

	CON	STITU	TION /	AVE	CON	STITU [*]	TION AV	/E		AKER:	SDR			AKER	SDR							
Interval		Eastb	ound			Westb	ound			Northb	ound			South	oound			Rolling	Ped	lestriar	n Crossir	ngs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru F	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South	North
 4:00 PM	0	5	190	0	0	0	180	14	0	0	0	0	0	8	0	21	418	1,754	0	0	0	0
4:15 PM	0	7	204	0	0	0	210	10	0	0	0	0	0	4	0	9	444	1,785	0	0	0	0
4:30 PM	1	5	214	0	1	0	192	13	0	0	0	0	0	11	0	16	453	1,757	0	0	0	0
4:45 PM	0	4	186	0	0	0	226	14	0	0	0	0	0	2	0	7	439	1,737	0	0	0	0
5:00 PM	0	12	213	0	1	0	198	6	0	0	0	0	0	6	0	13	449	1,646	0	0	0	0
5:15 PM	0	4	206	0	1	0	178	7	0	0	0	0	0	12	0	8	416		0	0	0	0
5:30 PM	0	4	214	0	0	0	182	12	0	0	0	0	0	7	0	14	433		0	0	0	0
5:45 PM	1	4	177	0	0	0	154	3	0	0	0	0	0	4	0	5	348		0	0	0	1
Count Total	2	45	1,604	0	3	0	1,520	79	0	0	0	0	0	54	0	93	3,400		0	0	0	1
Peak Hour	1	28	817	0	2	0	826	43	0	0	C	0	0	23	3 () 45	1,785		0	0	0	0

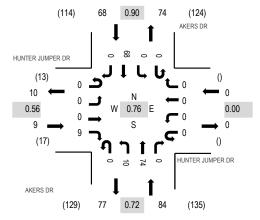


Location: 3 AKERS DR & HUNTER JUMPER DR AM

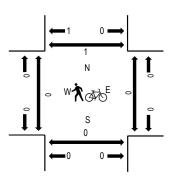
Date: Tuesday, August 11, 2020 **Peak Hour:** 07:00 AM - 08:00 AM

Peak 15-Minutes: 07:15 AM - 07:30 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	HUN	TER JU Eastb		R DR		ER JU Westb		DR		AKERS Northb				AKER South				Rolling	Ped	lestriar	Crossir	ıgs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South	North
7:00 AM	0	0	0	0	0	0	0	0	0	3	18	0	0	0	12	0	33	161	0	0	0	0
7:15 AM	0	0	0	4	0	0	0	0	0	3	26	0	0	0	20	0	53	160	0	0	0	0
7:30 AM	0	0	0	3	0	0	0	0	0	2	16	0	0	0	21	0	42	129	0	0	0	0
7:45 AM	0	0	0	2	0	0	0	0	0	2	14	0	0	0	15	0	33	114	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	12	0	0	0	20	0	32	105	0	0	0	0
8:15 AM	0	2	0	1	0	0	0	0	0	0	10	0	0	0	9	0	22		0	0	0	0
8:30 AM	0	0	0	2	0	0	0	0	0	2	13	0	0	0	10	0	27		0	0	0	0
8:45 AM	0	1	0	2	0	0	0	0	1	1	12	0	0	0	7	0	24		1	0	0	0
Count Total	0	3	0	14	0	0	(0	1	13	121	0	0	0	114	0	266		1	0	0	0
Peak Hour	0	0	0	9	0	0	0	0	0	10	74	0	0	C	68	3) 161	I	0	0	0	0

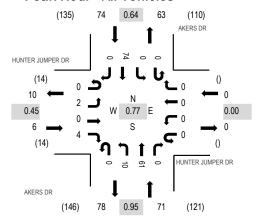


Location: 3 AKERS DR & HUNTER JUMPER DR PM

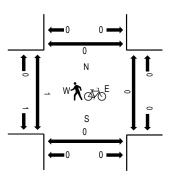
Date: Tuesday, August 11, 2020 **Peak Hour:** 04:00 PM - 05:00 PM

Peak 15-Minutes: 04:00 PM - 04:15 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	HUN	TER JU Eastb		R DR	HUN	ΓER JU Westb	MPER Dound	R		AKERS Northbo				AKER South				Rolling	Ped	estriar	Crossii	ngs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru R	ight	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South	North
4:00 PM	0	1	0	1	0	0	0	0	0	2	16	0	0	0	29	0	49	151	0	0	0	0
4:15 PM	0	0	0	1	0	0	0	0	0	2	15	0	0	0	11	0	29	140	0	0	0	0
4:30 PM	0	0	0	2	0	0	0	0	0	3	16	0	0	0	25	0	46	143	1	0	0	0
4:45 PM	0	1	0	0	0	0	0	0	0	3	14	0	0	0	9	0	27	129	0	0	0	0
5:00 PM	0	1	0	4	0	0	0	0	0	1	18	0	0	0	14	0	38	119	0	0	0	0
5:15 PM	0	0	0	1	0	0	0	0	0	1	10	0	0	0	20	0	32		0	0	0	0
5:30 PM	0	0	0	1	0	0	0	0	1	2	10	0	0	0	18	0	32		0	0	0	0
5:45 PM	0	1	0	0	0	0	0	0	0	0	7	0	0	0	9	0	17		0	0	0	0
Count Total	0	4	0	10	0	0	0	0	1	14	106	0	0	0	135	0	270		1	0	0	0
Peak Hour	0	2	0	4	0	0	0	0	0	10	61	0	0	0	74		0 151		1	0	0	0

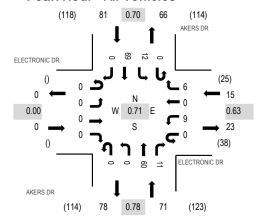


Location: 4 AKERS DR & ELECTRONIC DR AM

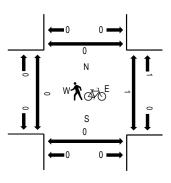
Date: Tuesday, August 11, 2020 **Peak Hour:** 07:15 AM - 08:15 AM

Peak 15-Minutes: 07:15 AM - 07:30 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	EL	ECTR(Eastb		DR		ECTRO Westb	NIC DR	?		AKERS Northb				AKER Southb				Rolling	Ped	estriar	n Crossin	ngs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru I	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South I	North
7:00 AM	0	0	0	0	0	1	0	0	0	0	14	4	0	5	10	0	34	161	0	0	0	0
7:15 AM	0	0	0	0	0	6	0	0	0	0	18	6	0	10	19	0	59	167	0	0	0	0
7:30 AM	0	0	0	0	0	2	0	1	0	0	17	1	0	2	19	0	42	130	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	14	1	0	0	11	0	26	111	0	0	0	0
8:00 AM	0	0	0	0	0	1	0	5	0	0	11	3	0	0	20	0	40	105	0	0	0	0
8:15 AM	0	0	0	0	0	4	0	0	0	0	8	0	0	3	7	0	22		0	0	0	0
8:30 AM	0	0	0	0	0	1	0	0	0	0	13	1	0	0	8	0	23		0	0	0	0
8:45 AM	0	0	0	0	0	3	0	1	0	0	12	0	0	2	2	0	20		0	0	0	0
Count Total	0	0	0	0	0	18	0	7	0	0	107	16	0	22	96	0	266		0	0	0	0
 Peak Hour	0	0	0	0	0	9	0	6	0	0	60	11	0	12	69) () 167	7	0	0	0	0

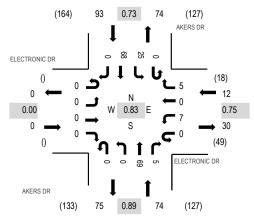


Location: 4 AKERS DR & ELECTRONIC DR PM

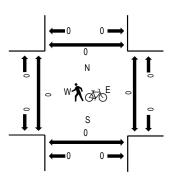
Date: Tuesday, August 11, 2020 **Peak Hour:** 04:00 PM - 05:00 PM

Peak 15-Minutes: 04:00 PM - 04:15 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	EL	ECTR(Eastb		DR	ELI	ECTRO Westb	NIC DR ound			AKERS Northbo				AKER Southb				Rolling	Ped	estriar	n Crossir	ngs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru Rio	ght	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South I	North
4:00 PM	0	0	0	0	0	2	0	2	0	0	16	2	0	9	23	0	54	179	0	0	0	0
4:15 PM	0	0	0	0	0	3	0	0	0	0	19	1	0	6	10	0	39	174	0	0	0	0
4:30 PM	0	0	0	0	0	1	0	3	0	0	16	1	0	7	23	0	51	167	0	0	0	0
4:45 PM	0	0	0	0	0	1	0	0	0	0	18	1	0	3	12	0	35	147	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	3	0	0	19	3	0	8	16	0	49	130	0	0	0	0
5:15 PM	0	0	0	0	0	2	0	0	0	0	12	0	0	3	15	0	32		0	0	0	0
5:30 PM	0	0	0	0	0	1	0	0	0	0	9	0	0	4	17	0	31		0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	10	0	0	1	7	0	18		0	0	0	0
Count Total	0	0	0	0	0	10	0	8	0	0	119	8	0	41	123	0	309		0	0	0	0
 Peak Hour	0	0	0	0	0	7	0	5	0	0	69	5	0	25	68	3	0 179	9	0	0	0	0

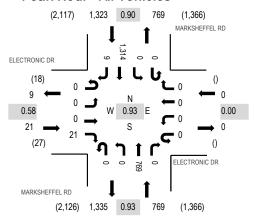


Location: 5 MARKSHEFFEL RD & ELECTRONIC DR AM

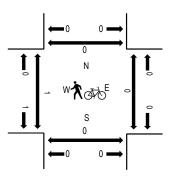
Date: Tuesday, August 11, 2020 **Peak Hour:** 07:00 AM - 08:00 AM

Peak 15-Minutes: 07:00 AM - 07:15 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Interval	EL	ECTR(Eastb		DR	ELI	ECTRC Westb	NIC DR		MA	RKSHE Northb		RD	MA	RKSHE South	EFFEL I	RD		Rolling	Ped	lestrian	n Crossin	nas
Start Time	U-Turn	Left	Thru	Right	U-Turn		Thru R	light	U-Turn	Left		Right	U-Turn	Left	Thru	Right	Total	Hour	West		South N	0
7:00 AM	0	0	0	9	0	0	0	0	0	0	193	0	0	0	366	1	569	2,113	0	0	0	0
7:15 AM	0	0	0	9	0	0	0	0	0	0	195	0	0	0	331	5	540	1,930	0	0	0	0
7:30 AM	0	0	0	2	0	0	0	0	0	0	207	0	0	0	317	3	529	1,763	0	0	0	0
7:45 AM	0	0	0	1	0	0	0	0	0	0	174	0	0	0	300	0	475	1,558	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	154	0	0	0	231	1	386	1,397	0	0	0	0
8:15 AM	0	0	0	2	0	0	0	0	0	0	153	0	0	0	214	4	373		0	0	0	0
8:30 AM	0	0	0	3	0	0	0	0	0	0	129	0	0	0	191	1	324		2	0	0	0
8:45 AM	0	0	0	1	0	0	0	0	0	0	161	0	0	0	149	3	314		0	0	0	0
Count Total	0	0	0	27	0	0	0	0	0	0	1,366	0	0	0	2,099	18	3,510		2	0	0	0
Peak Hour	0	0	0	21	0	0	0	0	0	0	769	0	0	(1,314		2,113	3	0	0	0	0

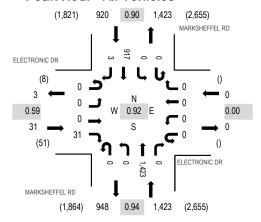


Location: 5 MARKSHEFFEL RD & ELECTRONIC DR PM

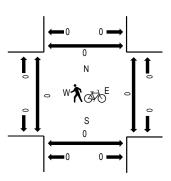
Date: Tuesday, August 11, 2020 **Peak Hour:** 04:30 PM - 05:30 PM

Peak 15-Minutes: 05:00 PM - 05:15 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

	EL	ECTR(ONIC E)R	ELE	CTRC	ONIC D)R	MA	RKSHE	FFEL F	RD	MA	RKSHE	FFELF	RD						
Interval		Eastb	ound			Westb	ound			Northb	ound			South	oound			Rolling	Ped	estrian	Crossin	gs
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour	West	East	South N	√orth
4:00 PM	0	0	0	9	0	0	0	0	0	0	348	0	0	0	249	2	608	2,307	0	0	0	0
4:15 PM	0	0	0	6	0	0	0	0	0	0	322	0	0	0	210	2	540	2,343	0	0	0	0
4:30 PM	0	0	0	14	0	0	0	0	0	0	336	0	0	0	237	1	588	2,374	0	0	0	0
4:45 PM	0	0	0	3	0	0	0	0	0	0	354	0	0	0	213	1	571	2,316	0	0	0	0
5:00 PM	0	0	0	10	0	0	0	0	0	0	377	0	0	0	257	0	644	2,220	0	0	0	0
5:15 PM	0	0	0	4	0	0	0	0	0	0	356	0	0	0	210	1	571		0	0	0	0
5:30 PM	0	0	0	4	0	0	0	0	0	0	288	0	0	0	237	1	530		0	0	0	0
5:45 PM	0	0	0	1	0	0	0	0	0	0	274	0	0	0	200	0	475		1	0	0	0
Count Total	0	0	0	51	0	0		0 0	0	0	2,655	0	0	0	1,813	8	4,527		1	0	0	0
Peak Hour	0	0	0	31	0	0	(0 0	0	0	1,423	0	0	(917	. 3	3 2,374		0	0	0	0

All Traffic Data Services www.alltrafficdata.net

Date Start: 11-Aug-20
CONSTITUTION AVE W.O. MARKSHEFFEL RD
Site Code: 6
Station ID:

	Total	49	32	31	43	110	282	821	1175	991	992	1072	1112	1313	1306	1234	1446	1702	1603	1101	810	583	320	201	119	18478		02:00	1175	16:00	1702	18478	
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!	WB	20	14	13	15	42	125	343	277	475	514	527	562	622	627	220	629	698	742	520	383	238	146	82	54	8739	47.3%	00:20	277	16:00	869	8739	47.3%
	EB	29	18	18	28	89	157	478	298	516	478	545	220	691	629	684	192	833	861	581	427	345	204	119	65	9739	52.7%	02:00	298	17:00	861	9739	52.7%
11-Aug-20	Tue																											•		•			
Start		12:00 AM	01:00	02:00	03:00	04:00	02:00	00:90	07:00	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	02:00	00:90	02:00	08:00	00:60	10:00	11:00	Total	Percent	AM Peak	Vol.	PM Peak	Vol.	Grand Total	Percent

AADT 18,478

ADT 18,478

ADT

All Traffic Data Services www.alltrafficdata.net

Date Start: 11-Aug-20
MARKSHEFFEL RD N.O. CONSTITUTION AVE
Site Code: 7
Station ID:

	Total	44	34	42	44	130	485	1577	2064	1424	1317	1336	1355	1492	1465	1489	1992	2250	2220	1295	861	533	313	220	139	24121		02:00	2064	16:00	2250	24121	
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	SB	23	14	21	29	75	316	912	1310	808	200	229	658	739	737	731	829	902	944	647	447	247	156	107	20	12116	50.2%	00:00	1310	17:00	944	12116	50.2%
	NB	21	20	21	15	22	169	999	754	615	611	629	269	753	728	758	1133	1348	1276	648	414	286	157	113	88	12005	49.8%	00:20	754	16:00	1348	12005	49.8%
11-Aug-20	Tue																											•	•	•	•		
		12:00 AM	01:00	05:00	03:00	04:00	02:00	00:90	00:20	08:00	00:60	10:00	11:00	12:00 PM	01:00	05:00	03:00	04:00	02:00	00:90	00:20	08:00	00:60	10:00	11:00	Total	Percent	AM Peak	Vol.	PM Peak	Vol.	Grand Total	Percent

AADT 24,121

ADT 24,121

ADT

All Traffic Data Services www.alltrafficdata.net

Date Start: 11-Aug-20
AKERS DR N.O. CONSTITUTION AVE
Site Code: 8
Station ID:

Tue	NB	SB							Total
	4	2							9
	က	0							3
	_	0							-
	_	2							3
	4	∞							22
	46	15							61
	98	46							132
	83	83							166
	56	54							110
	74	69							143
	99	71							137
	82	75							157
	88	06							179
	109	82							194
	80	72							152
	96	87							183
	82	79							161
	46	73							119
	30	30							09
	30	19							49
	22	တ							31
	6	12							21
	6	7							16
	က	_							4
	1121	686							2110
	53.1%	46.9%							
•	00:90	02:00	•	1	•		1		02:00
•	98	83	•		•				166
•	13:00	12:00	•		•				13:00
•	109	06	-	-	-	-	-	-	194
	1121	686							2110
	52 10/	76 9%							

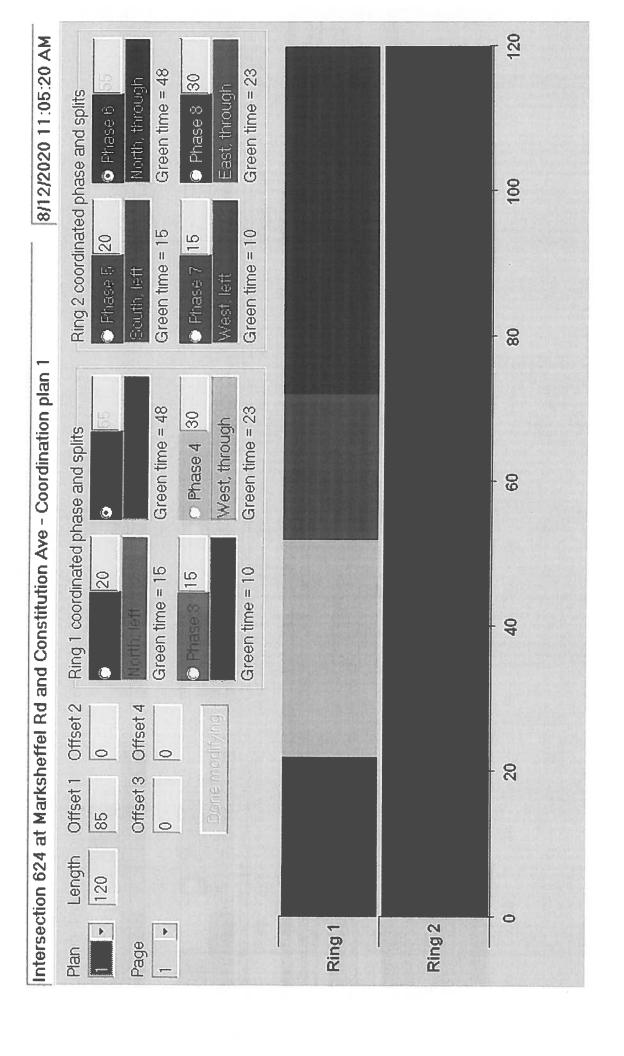
AADT 2,110

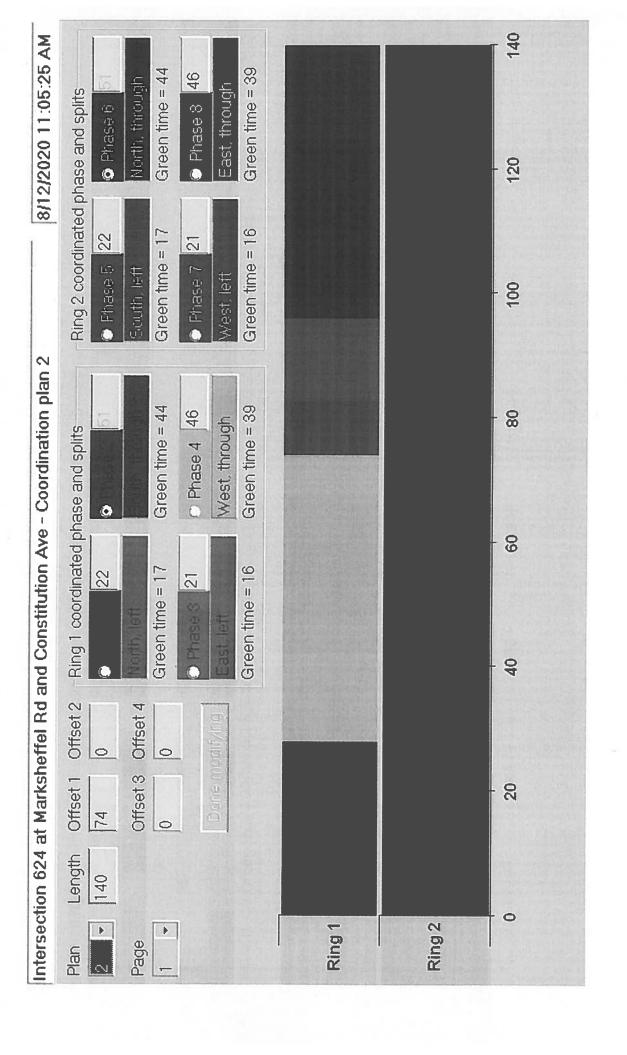
ADT 2,110

ADT

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	Intersection 624 at Marksheffel Rd and Constitution Ave - Plans schedule
-	_

	Sun 08/16	Free		Plan 1, Ofst 1	120, 85	Free										
	Sat 08/15	Free		Plan 1, Ofst 1 Plan 1, Ofst 1	120, 85	Free										
		12:00am	Cycle len, offset	7:30am	Cycle len,offset	7:00pm	Cycle len,offset									
Week of outvicuou	Fri 08/14	Free		Plan1, Ofst1	120, 85	Plan2,Ofst1	140,74	Free								
Week of	Thu 08/13	Free		Ofst1 Plan1, Ofst1 Plan1, Ofst1	120, 85	Plan2, Ofst1 Plan2, Ofst1 Plan2, Ofst1 Plan2, Ofst1	140, 74	Free								
	Wed 08/12	Free		Plan1,0fst1	120,85	Plan2,Ofst1	140, 74	Free								
Andinges	Tue 08/11	Free		Plan1, Ofst1	120, 85	Plan2,0fst1	140,74	Free								
raye or railyes	Mon 08/10	Free		Plan 1, Ofst 1 Plan 1, Ofst 1	120, 85	Plan2,Ofst1	140,74	Free								
ाणा आवातिक		12:00am	Cycle len, offset	6:30am	Cycle len, offset	4:00pm	Cycle len.offset	6:00pm	Cycle len.offset							





Intersection 624 at Marksheffel Rd and Constitution Ave - Timing table

Page 1	Phases							1				
	1	2	3	4	5	9	7	8	6	10	11	12
Min Green	4	4	4	4	4	4	4	4	0	0	0	0
Passage Time I	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	0.0	0.0	0.0
Passage Time II	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Green I	20	30	12	25	20	30	12	25	0	0	0	0
Max Green II	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Clearance	3.0	5.0	3.0	5.0	3.0	5.0	3.0	5.0	0.0	0.0	0.0	0.0
Red Clearance	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	0.0	0.0	0.0
Added Initial	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Added Initial	0	0	0	0	0	0	0	0	0	0	0	0
Time Before Reduction	0	0	0	0	0	0	0	0	0	0	0	0
Cars Before Reduction	0	0	0	0	0	0	0	0	0	0	0	0
Time To Reduce	0	0	0	0	0	0	0	0	0	0	0	0
Min Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Green Time	0	0	0	0	0	0	0	0	0	0	0	0
Red Revert Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Walk Time	0	7	0	7	0	7	0	7	0	0	0	0
Pedestrian Clearance	0	25	0	34	0	25	0	34	0	0	0	0
Handicap Walk	0	0	0	0	0	0	0	0	0	0	0	0
Handicap Ped Clearance	0	0	0	0	0	0	0	0	0	0	0	0
Marksheffel Rd	×	×			×	×						
Constitution Ave			×	×			×	×				
Compass Direction	z	S	Ш	Ν	S	Z	Λ	E				
Through, Turn or XPed	Left, prt	Thru	Left,p/p Thru	Thru	Left, prt	Thru	Left.p/p Thru	Thru				

APPENDIX B

Level of Service Definitions

The following information can be found in the <u>Highway Capacity Manual</u>, Transportation Research Board, 2010: Chapter 18 – Signalized Intersections and Chapter 19 – Two-Way Stop Controlled Intersections.

<u>Automobile Level of Service (LOS) for Signalized Intersections</u>

Levels of service are defined to represent reasonable ranges in control delay.

LOS A

Describes operations with a control delay of 10s/veh or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

LOS C

Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual *cycle failures* (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

LOS D

Describes operations with control delay between 35 and 55 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F

Describes operations with control delay exceeding 80 s/veh or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

Level of Service (LOS) for Unsignalized TWSC Intersections

Level of Service	Average Control Delay (s/veh)
А	0 - 10
В	> 10 - 15
C	> 15 - 25
D	> 25 - 35
E	> 35 - 50
F	> 50

APPENDIX C Capacity Worksheets

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	† †	7	٦	^	7		ሽኘ	^	7	ቪቪ	^
Traffic Volume (vph)	99	241	260	77	288	152	24	184	522	66	114	1085
Future Volume (vph)	99	241	260	77	288	152	24	184	522	66	114	1085
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.408			0.590				0.950			0.950	
Satd. Flow (perm)	760	3539	1583	1099	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			275			165				127		
Lane Group Flow (vph)	108	262	283	84	313	165	0	226	567	72	124	1179
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	15.0	30.0	30.0	15.0	30.0	30.0	20.0	20.0	55.0	55.0	20.0	55.0
Total Split (%)	12.5%	25.0%	25.0%	12.5%	25.0%	25.0%	16.7%	16.7%	45.8%	45.8%	16.7%	45.8%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	28.5	18.6	18.6	26.8	15.9	15.9		13.2	61.0	61.0	9.7	57.6
Actuated g/C Ratio	0.24	0.16	0.16	0.22	0.13	0.13		0.11	0.51	0.51	0.08	0.48
v/c Ratio	0.42	0.48	0.59	0.28	0.67	0.47		0.60	0.32	0.08	0.45	0.69
Control Delay	38.3	49.8	11.5	35.2	56.6	11.3		57.5	18.8	0.4	57.4	28.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	38.3	49.8	11.5	35.2	56.6	11.3		57.5	18.8	0.4	57.4	28.3
LOS	D	D	В	D	Ε	В		Ε	В	Α	Ε	С
Approach Delay		31.3			40.1				27.3			28.7
Approach LOS		С			D				С			С
Queue Length 50th (ft)	65	101	5	50	123	0		87	132	0	47	368
Queue Length 95th (ft)	109	140	86	88	165	61		125	193	3	78	510
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	265	684	527	311	678	436		439	1800	867	429	1697
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.41	0.38	0.54	0.27	0.46	0.38		0.51	0.32	0.08	0.29	0.69

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120 Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

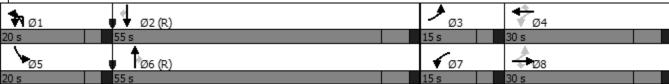


Lane Group	SBR
Lane Configurations	7
Traffic Volume (vph)	115
Future Volume (vph)	115
Satd. Flow (prot)	1583
Flt Permitted	1000
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	127
Lane Group Flow (vph)	125
Turn Type	Perm
Protected Phases	T CITI
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	55.0
Total Split (%)	45.8%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Actuated a/C Patio	57.6
Actuated g/C Ratio v/c Ratio	0.48 0.15
Control Delay	4.0
Queue Delay	0.0
Total Delay	4.0
LOS Approach Dolov	Α
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	0
Queue Length 95th (ft)	36
Internal Link Dist (ft)	0/5
Turn Bay Length (ft)	265
Base Capacity (vph)	825
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.15
Intersection Summary	

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 30.7 Intersection LOS: C
Intersection Capacity Utilization 76.3% ICU Level of Service D
Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



Intersection								
Int Delay, s/veh	0.5							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	ኝ	ተተተ	^	7	ሻ	7		
Traffic Vol, veh/h	35	564	526	50	36	42		
Future Vol, veh/h	35	564	526	50	36	42		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-			None	-	Free		
Storage Length	275	-	-	-	355	0		
Veh in Median Storage		0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	38	613	572	54	39	46		
WINTER TOW	- 30	013	312		- 37	- 10		
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	626	0	-	0	893	_		
Stage 1	-	-	-	-	572	-		
Stage 2		-	_	-	321	_		
Critical Hdwy	4.14	-	-	-	6.29	_		
Critical Hdwy Stg 1	-	-	_	-	5.84	_		
Critical Hdwy Stg 2	-	-		-	6.04	-		
Follow-up Hdwy	2.22	-	_	-	3.67	-		
Pot Cap-1 Maneuver	*1306	-	_	-	*767	0		
Stage 1	-	_	_	-	*790	0		
Stage 2	_	-	_	-	*671	0		
Platoon blocked, %	1	_	_	_	1			
Mov Cap-1 Maneuver	-	-	-		*744	_		
Mov Cap-1 Maneuver	1300	-		-	*744	-		
Stage 1	-	-			*767	_		
Stage 2		-			*671	-		
Jiaye Z					0/1	-		
Approach	EB		WB		SB			
HCM Control Delay, s	0.5		0		10.1			
HCM LOS	0.0				В			
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	SBLn1 S	SBLn2	
Capacity (veh/h)		* 1306		_	-	744	-	
HCM Lane V/C Ratio		0.029	_	-	-	0.053	-	
HCM Control Delay (s))	7.8		-	-	10.1	0	
HCM Lane LOS		Α.	_	-	-	В	A	
HCM 95th %tile Q(veh	1)	0.1	_	_	_	0.2	-	
	'/	0.1				0.2		
Notes	n o o!!	¢ D.	Jav	200 de 20	000	0	autotion Not Defined	*. All mader velous a les violes
~: Volume exceeds ca	pacity	\$: D6	eiay exc	ceeds 30	UUS	+: Comp	outation Not Defined	*: All major volume in platod

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						JDK 7
Lane Configurations	<u>ነ</u>	*	<u>ነ</u>	↑	↑	
Traffic Vol, veh/h	0	9	10	75	69	0
Future Vol, veh/h	0	9	10	75	69	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	100	0	130	-	-	120
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	76	76	76	76	76	76
Heavy Vehicles, %	2	2	2	2	2	2
	0	12	13	99	91	
Mvmt Flow	U	12	13	99	91	0
Major/Minor	Minor2	ľ	Major1	1	Major2	
Conflicting Flow All	216	91	91	0		0
Stage 1	91	71	-	-		-
	125	-	-		-	
Stage 2				-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	772	967	1504	-	-	-
Stage 1	933	-	-	-	-	-
Stage 2	901	-	-	-	_	_
Platoon blocked, %	, , ,			_	_	_
Mov Cap-1 Maneuver	765	967	1504	_		
	765	707	1304	_	_	_
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	925	-	-	-	-	-
Stage 2	901	-	-	-	-	-
Approach	EB	-	NB	-	SB	-
HCM Control Delay, s	8.8		0.9		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBL	MRT	EBLn1 E	-RI n2	SBT
	10					
Capacity (veh/h)		1504	-	-	, , ,	-
HCM Lane V/C Ratio		0.009	-		0.012	-
HCM Control Delay (s)		7.4	-	0	8.8	-
HCM Lane LOS		Α	-	Α	Α	-
HCM 95th %tile Q(veh	1)	0	-	-	0	-

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1		ሻ	<u></u>
Traffic Vol, veh/h	9	6	60	11	12	69
Future Vol, veh/h	9	6	60	11	12	69
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Jiop	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	71	71	71	71	71	71
						2
Heavy Vehicles, %	2	2	2	2	2	
Mvmt Flow	13	8	85	15	17	97
Major/Minor	Minor1	N	Major1	ľ	Major2	
Conflicting Flow All	224	93	0	0	100	0
Stage 1	93	-	-	-	-	-
Stage 2	131	_	_	-	_	_
Critical Hdwy	6.42	6.22	-	_	4.12	-
Critical Hdwy Stg 1	5.42	-	_	_	-	_
Critical Hdwy Stg 2	5.42	_		_	_	-
Follow-up Hdwy	3.518	3.318	-	_	2.218	_
Pot Cap-1 Maneuver	764	964	-	-	1493	-
	931	704	_	-	1493	-
Stage 1			-	-		
Stage 2	895	-	-	-	-	-
Platoon blocked, %	75 (0/4	-	-	1 100	-
Mov Cap-1 Maneuver	756	964	-	-	1493	-
Mov Cap-2 Maneuver	758	-	-	-	-	-
Stage 1	931	-	-	-	-	-
Stage 2	885	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		1.1	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	829	1493	-
HCM Lane V/C Ratio			-	0.025		
	\	-	-	9.5	7.4	-
HCM Control Delay (s HCM Lane LOS)	-	-			-
HCM 95th %tile Q(veh		-	-	Α	Α	-
	.)			0.1	0	_

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBF	. NBL	NBT	SBT	SBR
Lane Configurations	EDL	CDF		<u>NDI</u>	<u>⊅⊅1</u>	JDK 7
Traffic Vol, veh/h	0			TT 773	TT 1293	r
Future Vol, veh/h	0			773	1293	9
Conflicting Peds, #/hr	0			0	1293	0
Sign Control	Stop			Free	Free	Free
RT Channelized	310p				-	Free
Storage Length				NONE -	-	240
Veh in Median Storage,				0	0	240
Grade, %	, # 0			0	0	-
Peak Hour Factor	92			92	92	92
Heavy Vehicles, %	2			2	2	2
Mvmt Flow	0			840	1405	10
IVIVIIIL FIOW	U	23	0	840	1405	10
Major/Minor N	/linor2		Major1	N	Major2	
Conflicting Flow All	-	703	-	0	-	0
Stage 1	-		-	-	-	-
Stage 2	-		-	-	-	-
Critical Hdwy	-	6.94	. -	-	-	-
Critical Hdwy Stg 1	-		-	-	-	-
Critical Hdwy Stg 2	-		-	-	-	-
Follow-up Hdwy	-	3.32	! -	-	-	-
Pot Cap-1 Maneuver	0	380	0	-	-	0
Stage 1	0		. 0	-	-	0
Stage 2	0		. 0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	380	-	-	-	-
Mov Cap-2 Maneuver	-			-	-	-
Stage 1	-			-	-	-
Stage 2	-			-	-	-
A I.	FC		NE		65	
Approach	EB		NB		SB	
	15.1		0		0	
HCM Control Delay, s						
	C					
HCM Control Delay, s						
HCM Control Delay, s HCM LOS	С		EBLn1	SBT		
HCM Control Delay, s HCM LOS Minor Lane/Major Mvml	С		EBLn1 380	SBT -		
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	С	NBT	380	-		
HCM Control Delay, s HCM LOS Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio	С	NBT	380			
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm! Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	С	NBT	380 0.06 15.1	-		
HCM Control Delay, s HCM LOS Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio	C t	NBT	380 0.06 15.1	-		

	۶	→	•	•	←	•	₹I	4	†	<i>></i>	>	ļ
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	† †	7	ሻ	^	7		ሽኘ	^	7	ቪቪ	<u></u> ↑↑
Traffic Volume (vph)	102	531	225	118	399	232	14	345	1073	5 5	203	613
Future Volume (vph)	102	531	225	118	399	232	14	345	1073	55	203	613
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.368			0.193				0.950			0.950	
Satd. Flow (perm)	685	3539	1583	360	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			245			252				109		
Lane Group Flow (vph)	111	577	245	128	434	252	0	390	1166	60	221	666
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	21.0	46.0	46.0	21.0	46.0	46.0	22.0	22.0	51.0	51.0	22.0	51.0
Total Split (%)	15.0%	32.9%	32.9%	15.0%	32.9%	32.9%	15.7%	15.7%	36.4%	36.4%	15.7%	36.4%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	42.6	29.0	29.0	45.1	30.2	30.2		21.2	59.9	59.9	14.3	53.0
Actuated g/C Ratio	0.30	0.21	0.21	0.32	0.22	0.22		0.15	0.43	0.43	0.10	0.38
v/c Ratio	0.37	0.79	0.47	0.52	0.57	0.47		0.75	0.77	0.08	0.63	0.50
Control Delay	33.8	60.6	8.0	38.3	51.5	7.7		66.6	39.9	0.3	68.3	36.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	33.8	60.6	8.0	38.3	51.5	7.7		66.6	39.9	0.3	68.3	36.5
LOS	С	E	Α	D	D	Α		E	D	Α	E	D
Approach Delay		43.6			35.9				44.9			39.4
Approach LOS		D			D				D			D
Queue Length 50th (ft)	70	264	0	81	187	0		175	473	0	101	248
Queue Length 95th (ft)	104	312	68	118	229	68		#239	#717	2	141	336
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	351	985	617	283	985	622		519	1513	739	421	1338
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.32	0.59	0.40	0.45	0.44	0.41		0.75	0.77	0.08	0.52	0.50

Cycle Length: 140

Actuated Cycle Length: 140
Offset: 74 (53%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 70



Lana Craun	CDD
Lane Group	SBR
Lare Configurations	122
Traffic Volume (vph)	123
Future Volume (vph)	123
Satd. Flow (prot)	1583
Flt Permitted	1500
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	134
Lane Group Flow (vph)	134
Turn Type	Perm
Protected Phases	_
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	51.0
Total Split (%)	36.4%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	53.0
Actuated g/C Ratio	0.38
v/c Ratio	0.20
Control Delay	6.0
Queue Delay	0.0
Total Delay	6.0
LOS	Α
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	0
Queue Length 95th (ft)	49
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	681
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.20
Intersection Summary	
Intersection Summary	

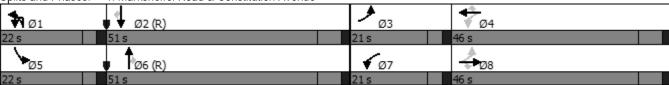
Maximum v/c Ratio: 0.79

Intersection Signal Delay: 41.7 Intersection LOS: D Intersection Capacity Utilization 76.7% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Intersection								
Int Delay, s/veh	0.3							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	ሻ	ተተተ	^	7	ኝ	7		
Traffic Vol, veh/h	29	817	828	43	23	45		
Future Vol, veh/h	29	817	828	43	23	45		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-		-	Free		
Storage Length	275	-		-	355	0		
Veh in Median Storage		0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	32	888	900	47	25	49		
1011	02	000	700	.,	20	17		
Major/Minor	Major1	N	Majora	N	/linor?			
Major/Minor Conflicting Flow All	<u>Major1</u> 947	0	Major2	0	<u>//inor2</u> 1319			
			-		900			
Stage 1	-	-	•	-		-		
Stage 2	-	-	-	-	419	-		
Critical Hdwy	4.14	-	-	-	6.29	-		
Critical Hdwy Stg 1	-	-	-	-	5.84	-		
Critical Hdwy Stg 2	-	-	-	-	6.04	-		
Follow-up Hdwy	2.22	-	-	-	3.67	-		
Pot Cap-1 Maneuver	*1120	-	•	-	*678	0		
Stage 1	-	-	-	-	*678	0		
Stage 2	-	-	-	-	*597	0		
Platoon blocked, %	1	-	-	-	1			
Mov Cap-1 Maneuver	*1120	-	-	-	*658	-		
Mov Cap-2 Maneuver	-	-	-	-	*658	-		
Stage 1	-	-	-	-	*658	-		
Stage 2	-	-	-	-	*597	-		
Approach	EB		WB		SB			
HCM Control Delay, s	0.3		0		10.7			
HCM LOS					В			
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR 9	SBLn1 S	SBLn2	
Capacity (veh/h)		* 1120		-	-	658	-	
HCM Lane V/C Ratio		0.028	_	-		0.038	-	
HCM Control Delay (s)		8.3		-	-	10.7	0	
HCM Lane LOS		0.5 A	-	-	-	В	A	
HCM 95th %tile Q(veh)	0.1	_		-	0.1	-	
	7	0.1				0.1		
Notes								
~: Volume exceeds ca	pacity	\$: D€	elay exc	ceeds 30	00s	+: Com	outation Not Defined	*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u> </u>	7	`	†		*
Traffic Vol, veh/h	2	4	10	62	64	0
Future Vol, veh/h	2	4	10	62	64	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	100	0	130	-	-	120
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	77	77	77	77	77	77
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	5	13	81	83	0
IVIVIIIL I IOVV	J	J	13	UT	- 03	
Major/Minor	Minor2		Major1	<u> </u>	Major2	
Conflicting Flow All	190	83	83	0	-	0
Stage 1	83	-	-	-	-	-
Stage 2	107	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-		-
Critical Hdwy Stg 1	5.42	0.22	4.12			_
		-		-	-	
Critical Hdwy Stg 2	5.42	2 210	2 210	-	-	-
Follow-up Hdwy	3.518	3.318		-	-	-
Pot Cap-1 Maneuver	799	976	1514	-	-	-
Stage 1	940	-	-	-	-	-
Stage 2	917	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	792	976	1514	-	-	-
Mov Cap-2 Maneuver	783	-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	917		_			_
Jugo 2	/ 1 /					
Approach	EB		NB		SB	
HCM Control Delay, s	9		1		0	
HCM LOS	A					
	, ,					
Minor Lane/Major Mvn	nt	NBL	NBT I	EBLn1 E	EBLn2	SBT
Capacity (veh/h)		1514	-	783	976	-
HCM Lane V/C Ratio		0.009	_	0.003		_
HCM Control Delay (s)		7.4		9.6	8.7	_
HCM Lane LOS		7.4 A	-	7.0 A	Α	-
HCM 95th %tile Q(veh	١	0	-	0	0	-
HOW YOU WILL U(VEN)	U		U	U	-

Intersection						
Int Delay, s/veh	1.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		אטוי		אטוז		
Lane Configurations	¥	-	}	_	\	†
Traffic Vol, veh/h	7	5	69	5	25	68
Future Vol, veh/h	7	5	69	5	25	68
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	6	83	6	30	82
IVIVIIIL FIOW	0	O	03	O	30	02
Major/Minor	Minor1	N	Major1	1	Major2	
Conflicting Flow All	228	86	0	0	89	0
Stage 1	86	-	-	-	-	-
Stage 2	142	_	-	_	_	_
Critical Hdwy	6.42	6.22			4.12	-
	5.42	0.22	-	-	4.12	-
Critical Hdwy Stg 1						
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-		2.218	-
Pot Cap-1 Maneuver	760	973	-	-	1506	-
Stage 1	937	-	-	-	-	-
Stage 2	885	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	745	973	-	-	1506	-
Mov Cap-2 Maneuver	748	_	-	-	-	-
Stage 1	937	-	_	_	_	_
Stage 2	867	-			_	
Stage 2	007	-		-		-
Approach	WB		NB		SB	
HCM Control Delay, s	9.4		0		2	
HCM LOS	A				_	
TOW LOO	, \					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	828	1506	-
				0.017	0.02	-
HCM Lane V/C Ratio		-	-	0.017		
)	-	-			-
HCM Control Delay (s))		-	9.4	7.4	-
		-	-			

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	EDK	NDL	<u>ND1</u>	<u>>D1</u>	JDK 7
Traffic Vol, veh/h	0	1 31	0	TT 1407	TT 908	
Future Vol, veh/h	0	31	0	1407	908	3
Conflicting Peds, #/hr	0	0	0	0	900	0
Sign Control RT Channelized	Stop	Stop	Free	Free	Free	Free
	-	Yield	-	None	-	Free
Storage Length	<u>-</u>	0	-	-	-	240
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	34	0	1529	987	3
Major/Minor M	linor2		Major1	N	Major2	
Conflicting Flow All	-	494	viajoi i	0	- viajoi z	0
Stage 1	-	474	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	_	0.94	-	-	-	-
Critical Hdwy Stg 2	-					
	-	2 22	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	521	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	521	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12.4		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	EBLn1	SBT		
Capacity (veh/h)			521	-		
HCM Lane V/C Ratio		_	0.065	-		
HCM Control Delay (s)			12.4	_		
HCM Lane LOS		_	В	-		
HCM 95th %tile Q(veh)			0.2	_		
			3.2			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	7	^	7	, j	† †	7		ሽኘ	^	7	1,1	^
Traffic Volume (vph)	106	264	270	150	315	195	25	191	554	88	147	1132
Future Volume (vph)	106	264	270	150	315	195	25	191	554	88	147	1132
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.414			0.480				0.950			0.950	
Satd. Flow (perm)	771	3539	1583	894	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			230			212				127		
Lane Group Flow (vph)	115	287	293	163	342	212	0	235	602	96	160	1230
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	15.0	30.0	30.0	15.0	30.0	30.0	20.0	20.0	55.0	55.0	20.0	55.0
Total Split (%)	12.5%	25.0%	25.0%	12.5%	25.0%	25.0%	16.7%	16.7%	45.8%	45.8%	16.7%	45.8%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	28.2	16.7	16.7	28.9	17.0	17.0		13.3	58.6	58.6	10.9	56.2
Actuated g/C Ratio	0.24	0.14	0.14	0.24	0.14	0.14		0.11	0.49	0.49	0.09	0.47
v/c Ratio	0.44	0.58	0.70	0.57	0.68	0.52		0.62	0.35	0.11	0.51	0.74
Control Delay	38.0	52.8	21.3	42.3	55.8	10.7		58.0	20.7	1.9	57.6	30.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	52.8	21.3	42.3	55.8	10.7		58.0	20.7	1.9	57.6	30.7
LOS	D	D	С	D	E	В		E	С	Α	E	С
Approach Delay		37.1			39.4				28.2			31.2
Approach LOS		D			D				С			С
Queue Length 50th (ft)	68	111	44	100	134	0		90	147	0	61	401
Queue Length 95th (ft)	112	150	135	152	176	66		130	218	18	95	553
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	267	678	489	289	678	474		435	1726	837	429	1656
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.43	0.42	0.60	0.56	0.50	0.45		0.54	0.35	0.11	0.37	0.74

Cycle Length: 120

Actuated Cycle Length: 120 Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 60

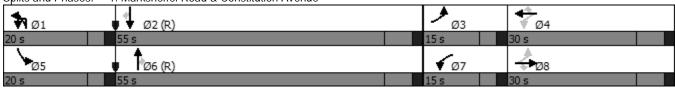


Lana Craun	CDD
Lane Group	SBR
Lare Configurations	
Traffic Volume (vph)	123
Future Volume (vph)	123
Satd. Flow (prot)	1583
Flt Permitted	1500
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	127
Lane Group Flow (vph)	134
Turn Type	Perm
Protected Phases	
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	55.0
Total Split (%)	45.8%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	56.2
Actuated g/C Ratio	0.47
v/c Ratio	0.17
Control Delay	4.8
Queue Delay	0.0
Total Delay	4.8
LOS	Α
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	3
Queue Length 95th (ft)	42
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	808
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.17
Intersection Summary	
intersection Summary	

AM Peak Hour - Year 2022

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 33.1 Intersection LOS: C
Intersection Capacity Utilization 82.5% ICU Level of Service E
Analysis Period (min) 15



Intersection								
Int Delay, s/veh	0.5							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	Ť	^	^	7	ሻ	7		
Traffic Vol, veh/h	36	603	565	52	37	44		
Future Vol, veh/h	36	603	565	52	37	44		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	Free		
Storage Length	275	-	-	-	355	0		
Veh in Median Storage	-, # -	0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	39	655	614	57	40	48		
Major/Minor N	Major1	Ŋ	Major2	ſ	Minor2			
Conflicting Flow All	671	0	-	0	954	-		
Stage 1	-	-	-	-	614	-		
Stage 2	-	-	-	-	340	-		
Critical Hdwy	4.14	-	-	-	6.29	-		
Critical Hdwy Stg 1	-	-	-	-	5.84	-		
Critical Hdwy Stg 2	-	-	-	-	6.04	-		
Follow-up Hdwy	2.22	-	-	-	3.67	-		
Pot Cap-1 Maneuver	*1306	-	-	-	*698	0		
Stage 1	-	-	-	-	*790	0		
Stage 2	-	-	-	-	*656	0		
Platoon blocked, %	1	-	-	-	1			
Mov Cap-1 Maneuver	*1306	-	-	-	*677	-		
Mov Cap-2 Maneuver	-	-	-	-	*677	-		
Stage 1	-	-	-	-	*766	-		
Stage 2	-	-	-	-	*656	-		
Approach	EB		WB		SB			
HCM Control Delay, s	0.4		0		10.7			
HCM LOS					В			
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR	SBLn1 S	SBLn2	
Capacity (veh/h)		* 1306	-	-	-	677	-	
HCM Lane V/C Ratio		0.03	-	-	-	0.059	-	
HCM Control Delay (s)		7.8	-	-	-	10.7	0	
HCM Lane LOS		A	-	-	-	В	A	
HCM 95th %tile Q(veh)		0.1	-	-	-	0.2	-	
Notes								
~: Volume exceeds cap	nacity	\$. Do	alay eye	ceeds 3	nns -	+· Com	outation Not Defined	*: All major volume in platoon
. Volume exceeds cal	Jacity	ψ. De	dy the	ccus 3	003	T. CUITI	Jatation Not Delineu	. All major volume in platoon

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	FDL.	EDR	NDL T	IND I	<u>301</u>	JDK 7
Traffic Vol, veh/h	<u>า</u> 0		1 0	7 78	T 72	r 0
Future Vol, veh/h	0	9	10	78 78	72	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Siop -		riee -	None	riee -	None
	100	0	130	None -	-	120
Storage Length						
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	- 7/
Peak Hour Factor	76	76	76	76	76	76
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	12	13	103	95	0
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	224	95	95	0	-	0
Stage 1	95	-	-	-	-	-
Stage 2	129	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	764	962	1499	-	-	-
Stage 1	929	-	-	_	-	_
Stage 2	897	-	-	-	_	-
Platoon blocked, %	077			_		_
Mov Cap-1 Maneuver	757	962	1499	_	_	_
Mov Cap 1 Maneuver	760	702	-	_	_	_
Stage 1	921	_	_	_	_	_
Stage 2	897	-	-	-	-	-
Staye 2	097	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		0.8		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt.	NBL	MDT	EBLn1 E	EDI n2	SBT
	III		INDI	CDLIII E		
Capacity (veh/h)		1499	-	-	962	-
HCM Lane V/C Ratio		0.009	-		0.012	-
HCM Control Delay (s))	7.4	-	0	8.8	-
HCM Lane LOS	.\	A	-	Α	A	-
HCM 95th %tile Q(veh	1)	0	-	-	0	-

AM Peak Hour - Year 2022

Interception						
Intersection	1 4					
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽		ሻ	
Traffic Vol, veh/h	9	6	62	11	12	72
Future Vol, veh/h	9	6	62	11	12	72
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	_	-	0
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	13	8	87	15	17	101
IVIVIIIL FIOW	13	0	07	10	17	101
Major/Minor	Minor1	N	Major1	1	Major2	
Conflicting Flow All	230	95	0	0	102	0
Stage 1	95	-	-	-	-	-
Stage 2	135	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	758	962	_	-	1490	_
Stage 1	929	-	_	_	-	_
Stage 2	891	-	_	_	_	-
Platoon blocked, %	071		_	_		_
Mov Cap-1 Maneuver	750	962	-	-	1490	-
Mov Cap-1 Maneuver	754	702	-	-	1470	_
	929		-	-		
Stage 1		-	-	-	-	-
Stage 2	881	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.5		0		1.1	
HCM LOS	А					
NA: 1 /NA: NA		NDT	NDDV	VDL 4	CDI	CDT
Minor Lane/Major Mvm	<u>nt </u>	NBT	INRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	020	1490	-
HCM Lane V/C Ratio		-	-	0.026		-
HCM Control Delay (s))	-	-	9.5	7.4	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)	-	-	0.1	0	-

November 2020

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7		^	^	7
Traffic Vol, veh/h	0	22	0	853	1398	9
Future Vol, veh/h	0	22	0	853	1398	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	24	0	927	1520	10

Major/Minor	Minor2	M	ajor1	Ma	jor2		
Conflicting Flow All	-	760	-	0	-	0	
Stage 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	
Critical Hdwy	-	6.94	-	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	
Follow-up Hdwy	-	3.32	-	-	-	-	
Pot Cap-1 Maneuver	0	349	0	-	-	0	
Stage 1	0	-	0	-	-	0	
Stage 2	0	-	0	-	-	0	
Platoon blocked, %				-	-		
Mov Cap-1 Maneuver		349	-	-	-	-	
Mov Cap-2 Maneuver	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	

Approach	EB	NB	SB
HCM Control Delay, s	16.1	0	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBT EBLn1	SBT	
Capacity (veh/h)	- 349	-	
HCM Lane V/C Ratio	- 0.069	-	
HCM Control Delay (s)	- 16.1	-	
HCM Lane LOS	- C	-	
HCM 95th %tile Q(veh)	- 0.2	-	

	۶	→	*	•	-	•	₽î	1	†	<i>></i>	/	↓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	^	7	٦	† †	7		ሽኘ	^	7	ቪቪ	^
Traffic Volume (vph)	113	570	234	181	437	296	15	359	1147	115	255	637
Future Volume (vph)	113	570	234	181	437	296	15	359	1147	115	255	637
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.370			0.165				0.950			0.950	
Satd. Flow (perm)	689	3539	1583	307	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			250			322				109		
Lane Group Flow (vph)	123	620	254	197	475	322	0	406	1247	125	277	692
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	21.0	46.0	46.0	21.0	46.0	46.0	22.0	22.0	51.0	51.0	22.0	51.0
Total Split (%)	15.0%	32.9%	32.9%	15.0%	32.9%	32.9%	15.7%	15.7%	36.4%	36.4%	15.7%	36.4%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	44.6	30.6	30.6	50.8	33.7	33.7		21.6	54.2	54.2	16.1	48.7
Actuated g/C Ratio	0.32	0.22	0.22	0.36	0.24	0.24		0.15	0.39	0.39	0.12	0.35
v/c Ratio	0.39	0.80	0.47	0.73	0.56	0.52		0.77	0.91	0.18	0.70	0.56
Control Delay	32.0	60.0	8.1	46.4	48.9	7.2		67.1	52.0	8.6	69.5	40.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	32.0	60.0	8.1	46.4	48.9	7.2		67.1	52.0	8.6	69.5	40.2
LOS	С	Е	Α	D	D	Α		Е	D	Α	E	D
Approach Delay		43.3			34.9				52.4			43.2
Approach LOS		D			С				D			D
Queue Length 50th (ft)	74	283	3	124	199	0		181	571	9	126	278
Queue Length 95th (ft)	111	333	71	175	248	75		#281	#827	58	173	351
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	362	985	621	279	985	673		529	1369	679	430	1230
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.34	0.63	0.41	0.71	0.48	0.48		0.77	0.91	0.18	0.64	0.56

Cycle Length: 140

Actuated Cycle Length: 140
Offset: 74 (53%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 90

November 2020



Lana Craun	CDD
Lane Group	SBR
Lare Configurations	120
Traffic Volume (vph)	130
Future Volume (vph)	130
Satd. Flow (prot)	1583
Flt Permitted	
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	141
Lane Group Flow (vph)	141
Turn Type	Perm
Protected Phases	
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	51.0
Total Split (%)	36.4%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	48.7
Actuated g/C Ratio	0.35
v/c Ratio	0.22
Control Delay	6.1
Queue Delay	0.0
Total Delay	6.1
LOS	Α
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	0
Queue Length 95th (ft)	49
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	642
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.22
Intersection Summary	
Intersection Summary	

1: Marksheffel Road & Constitution Avenue

PM Peak Hour - Year 2022

Maximum v/c Ratio: 0.91

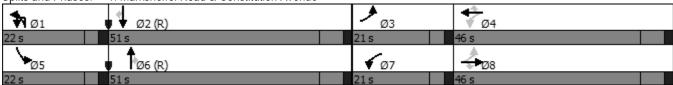
Intersection Signal Delay: 44.9 Intersection LOS: D Intersection Capacity Utilization 84.8% ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



November 2020 Synchro Report

Intersection									
Int Delay, s/veh	0.3								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	7	^	^	7	ሻ	7			
Traffic Vol, veh/h	30	875	885	45	24	47			
Future Vol, veh/h	30	875	885	45	24	47			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-		-	Free			
Storage Length	275	-	-	-	355	0			
Veh in Median Storage	e,# -	0	0	-	0	-			
Grade, %	-	0	0	-	0	-			
Peak Hour Factor	92	92	92	92	92	92			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	33	951	962	49	26	51			
Major/Minor	Moior1		//olor)	N	/linar)				
	Major1		Major2		Minor2				
Conflicting Flow All	1011	0	-	0	1408	-			
Stage 1	-	-	-	-	962	-			
Stage 2	-	-	-	-	446	-			
Critical Hdwy	4.14	-	-	-	6.29	-			
Critical Hdwy Stg 1	-	-	-	-	5.84	-			
Critical Hdwy Stg 2	-	-	-	-	6.04	-			
Follow-up Hdwy	2.22	-	-	-	3.67	-			
Pot Cap-1 Maneuver	*1087	-	-	-	*658	0			
Stage 1	-	-	-	-	*658	0			
Stage 2	-	-	-	-	*578	0			
Platoon blocked, %	1	-	-	-	1				
Mov Cap-1 Maneuver		-	-	-	*638	-			
Mov Cap-2 Maneuver		-	-	-	*638	-			
Stage 1	-	-	-	-	*638	-			
Stage 2	-	-	-	-	*578	-			
Approach	EB		WB		SB				
HCM Control Delay, s	0.3		0		10.9				
HCM LOS					В				
Minor Long/Major Mun	w.t	EDI	EDT	WDT	WDD	SBLn1 S	בת וחב		
Minor Lane/Major Mvr		EBL	EBT	WBT					
Capacity (veh/h)		* 1087	-	-	-	638	-		
HCM Cantral Dalay (,	0.03	-	-		0.041	-		
HCM Control Delay (s)	8.4	-	-	-	10.9	0		
HCM Lane LOS		A	-	-	-	В	A		
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.1	-		
Notes									
~: Volume exceeds ca	pacity	\$: De	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major volume in platoon	
			<i>j</i>					,	

Synchro Report SM ROCHA, LLC November 2020

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T T	LDK.	NDL T	†	<u>361</u>	3DK
Traffic Vol, veh/h	2	4	10	64	67	0
Future Vol, veh/h	2	4	10	64	67	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	100	0	130	-	-	120
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	77	77	77	77	77	77
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	5	13	83	87	0
Major/Minor	Minor2	1	Major1	N	Major2	
Conflicting Flow All	196	87	87	0	-	0
Stage 1	87	-	-	-	-	-
Stage 2	109	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	793	971	1509	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	916	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	786	971	1509	-	-	-
Mov Cap-2 Maneuver	779	-	-	-	-	-
Stage 1	928	-	-	-	-	-
Stage 2	916	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9		1		0	
HCM LOS	А					
Minor Lanc/Major Mun	nt .	NBL	NDT	EBLn1 E	EDI n2	SBT
Minor Lane/Major Mvn	It					SBI
Capacity (veh/h)		1509	-	779 0.003	971	-
HCM Lane V/C Ratio HCM Control Delay (s)		0.009 7.4		9.6	8.7	-
HCM Lane LOS			-	9.0 A	8.7 A	-
HCM 95th %tile Q(veh)	A 0	-	0	0	-
HOW FOUT WILLE U(VEH)	U	-	U	U	-

Synchro Report SM ROCHA, LLC November 2020

_	PM Peak Hour - Year 2022
	PIVI Peak Hour - Year 2022

Intersection						
Int Delay, s/veh	1.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥*				ኝ	<u> </u>
Traffic Vol, veh/h	7	5	72	5	26	71
Future Vol, veh/h	7	5	72	5	26	71
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized		None		None		None
	-	none	-		- 1E0	
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	6	87	6	31	86
N A = 1 = 1/N A1 = = 1	N:		1-11		4-10	
	Minor1		Major1		Major2	
Conflicting Flow All	238	90	0	0	93	0
Stage 1	90	-	-	-	-	-
Stage 2	148	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	750	968	-	-	1501	-
Stage 1	934	-	_	_	-	_
Stage 2	880		_	_	_	_
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	734	968	-	-	1501	-
Mov Cap-2 Maneuver	741	-	-	-	-	-
Stage 1	934	-	-	-	-	-
Stage 2	862	-	-	-	-	-
Approach	WB		NB		SB	
					2	
HCM Control Delay, s	9.5		0		2	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)				821	1501	
HCM Lane V/C Ratio		-		0.018		-
HCM Control Delay (s)		-	_	9.5	7.5	
HCM Lane LOS		-	-			-
		-	-	Α	A	-
HCM 95th %tile Q(veh)		-	-	0.1	0.1	-

November 2020

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7		^	^	7
Traffic Vol, veh/h	0	32	0	1551	1008	3
Future Vol, veh/h	0	32	0	1551	1008	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage,	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	35	0	1686	1096	3
N 4 - 1 (N 41	A' O		1-1-1		4-1-0	
	/linor2		Major1		Major2	
Conflicting Flow All	-	548	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	480	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	480	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
olago 2						
Approach	EB		NB		SB	
HCM Control Delay, s	13.1		0		0	
HCM LOS	В					
Minor Lanc/Major Mumi	+	NIDT	DI n1	CDT		
Minor Lane/Major Mvmt	l	NBT E		SBT		
Capacity (veh/h)		-	480	-		
HCM Cantral Datas (2)			0.072	-		
HCM Control Delay (s)		-	13.1	-		
LICALLAGALOC				-		
HCM Lane LOS HCM 95th %tile Q(veh)		-	0.2	-		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	ተተተ	7	ሻ	ተተተ	7		ሽኘ	ተተተ	7	ቪቪ	*
Traffic Volume (vph)	182	461	403	186	490	265	36	282	794	118	199	1632
Future Volume (vph)	182	461	403	186	490	265	36	282	794	118	199	1632
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	0	3433	5085	1583	3433	5085
Flt Permitted	0.271			0.458				0.950			0.950	
Satd. Flow (perm)	505	5085	1583	853	5085	1583	0	3433	5085	1583	3433	5085
Satd. Flow (RTOR)			147			270				173		
Lane Group Flow (vph)	198	501	438	202	533	288	0	346	863	128	216	1774
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	19.0	34.0	34.0	13.0	28.0	28.0	18.0	18.0	56.0	56.0	17.0	55.0
Total Split (%)	15.8%	28.3%	28.3%	10.8%	23.3%	23.3%	15.0%	15.0%	46.7%	46.7%	14.2%	45.8%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	40.5	26.0	26.0	30.9	20.9	20.9		13.3	50.7	50.7	11.3	48.7
Actuated g/C Ratio	0.34	0.22	0.22	0.26	0.17	0.17		0.11	0.42	0.42	0.09	0.41
v/c Ratio	0.64	0.46	0.96	0.72	0.60	0.58		0.91	0.40	0.17	0.67	0.86
Control Delay	35.4	38.5	60.0	48.7	48.9	11.7		81.1	25.2	1.6	62.9	38.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	35.4	38.5	60.0	48.7	48.9	11.7		81.1	25.2	1.6	62.9	38.0
LOS	D	D	Е	D	D	В		F	C	Α	Е	D
Approach Delay		46.2			38.4				37.4			37.5
Approach LOS	100	D	242	111	D	10		120	D	0	0.4	D
Queue Length 50th (ft)	120	127	242	114	140	12		139	172	0	84	457
Queue Length 95th (ft)	188	165	#442	#193	181	95		#230	209	16	125	525
Internal Link Dist (ft)	425	1022	200	225	405	225		425	707	225	/70	1957
Turn Bay Length (ft)	435	1111	200	225	007	235		425	21.47	325	670	2042
Base Capacity (vph)	320	1144	470	280	897	501		381	2147	768	343	2063
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0 63	0	0 02	0 72	0.50	0.57		0.01	0 40	0 17	0 62	0 06
Reduced v/c Ratio	0.62	0.44	0.93	0.72	0.59	0.57		0.91	0.40	0.17	0.63	0.86

Cycle Length: 120

Actuated Cycle Length: 120 Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 90



Lano Croun	SBR
Lane Group	
Lane Configurations	
Traffic Volume (vph)	188
Future Volume (vph)	188
Satd. Flow (prot)	1583
Flt Permitted	1500
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	173
Lane Group Flow (vph)	204
Turn Type	Perm
Protected Phases	_
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	55.0
Total Split (%)	45.8%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	48.7
Actuated g/C Ratio	0.41
v/c Ratio	0.27
Control Delay	6.2
Queue Delay	0.0
Total Delay	6.2
LOS	Α
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	14
Queue Length 95th (ft)	63
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	745
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.27
Intersection Summary	
intersection summary	

1: Marksheffel Road & Constitution Avenue

AM Peak Hour - Year 2040

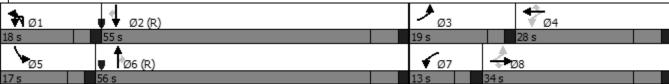
Maximum v/c Ratio: 0.96

Intersection Signal Delay: 39.4 Intersection LOS: D
Intersection Capacity Utilization 95.9% ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



AM Peak Hour - Year 2040

	•	→	•	•	←	•	4	†	/	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	ተተተ	7	۲	ተተተ	7	1/4	f)		1,1	f)	
Traffic Volume (vph)	130	890	73	29	837	106	41	11	69	149	15	83
Future Volume (vph)	130	890	73	29	837	106	41	11	69	149	15	83
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	1622	0	3433	1626	0
Flt Permitted	0.293			0.274			0.950			0.950		
Satd. Flow (perm)	546	5085	1583	510	5085	1583	3433	1622	0	3433	1626	0
Satd. Flow (RTOR)			82			115		75			90	
Lane Group Flow (vph)	141	967	79	32	910	115	45	87	0	162	106	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Split	NA		Split	NA	
Protected Phases		4			8		2	2		6	6	
Permitted Phases	4		4	8		8						
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	24.0	24.0	24.0	24.0	24.0	24.0		24.0	24.0	
Total Split (s)	72.0	72.0	72.0	72.0	72.0	72.0	24.0	24.0		24.0	24.0	
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	20.0%	20.0%		20.0%	20.0%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	83.3	83.3	83.3	83.3	83.3	83.3	7.7	7.7		11.0	11.0	
Actuated g/C Ratio	0.69	0.69	0.69	0.69	0.69	0.69	0.06	0.06		0.09	0.09	
v/c Ratio	0.37	0.27	0.07	0.09	0.26	0.10	0.20	0.50		0.52	0.46	
Control Delay	12.0	7.5	1.7	11.7	10.7	5.0	54.4	25.4		57.5	20.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	12.0	7.5	1.7	11.7	10.7	5.0	54.4	25.4		57.5	20.6	
LOS	В	A	Α	В	В	A	D	C		E	C	
Approach Delay		7.7			10.1			35.3			42.9	
Approach LOS	40	A	•	10	В	40	47	D		/0	D	
Queue Length 50th (ft)	40	91	0	10	112	12	17	9		62	12	
Queue Length 95th (ft)	98	136	17	m23	m171	m32	36	59		95	65	
Internal Link Dist (ft)	075	734	005	005	1022	075	400	327		055	581	
Turn Bay Length (ft)	275	2520	235	235	2520	275	120	207		355	220	
Base Capacity (vph)	379	3530	1124	354	3530	1134	514	307		514	320	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0 07	0	0	0 10	0	0		0	0	
Reduced v/c Ratio	0.37	0.27	0.07	0.09	0.26	0.10	0.09	0.28		0.32	0.33	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

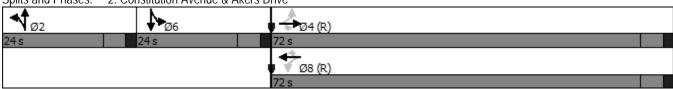
Natural Cycle: 80

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 13.6	Intersection LOS: B
Intersection Capacity Utilization 49.3%	ICU Level of Service A
Analysis Period (min) 15	

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Constitution Avenue & Akers Drive



Intersection							
Int Delay, s/veh	4						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	7	ሻ	<u></u>	<u></u>	7	
Traffic Vol, veh/h	0	110	117	126	133	0	
Future Vol, veh/h	0	110	117	126	133	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	100	0	130	-	-	120	
Veh in Median Storage		-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	120	127	137	145	0	
Major/Minor	Minor2		Major1	N	/lajor2		
Conflicting Flow All	536	145	145	0	-	0	
Stage 1	145	-	-	-	-	-	
Stage 2	391	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy		3.318		-	-	-	
Pot Cap-1 Maneuver	520	902	1437	-	-	-	
Stage 1	882	-	-	-	-	-	
Stage 2	695	-	-	-	-	-	
Platoon blocked, %	1	6.5.5	4 / 5 =	-	-	-	
Mov Cap-1 Maneuver	474	902	1437	-	-	-	
Mov Cap-2 Maneuver	553	-	-	-	-	-	
Stage 1	804	-	-	-	-	-	
Stage 2	695	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	9.6		3.7		0		
HCM LOS	А						
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1 E	FRI n2	SBT	SBR
Capacity (veh/h)	TC .	1437	-	-	902	- 100	
HCM Lane V/C Ratio		0.088	-		0.133		-
HCM Control Delay (s)	\	7.7	-	0	9.6	-	-
HCM Lane LOS		7.7 A	-	A	9.0 A	-	-
HCM 95th %tile Q(veh	1)	0.3	-	- A	0.5	-	-
1161VI 75111 701116 Q(VEII)	0.3	-	-	0.5	-	-

HCM 95th %tile Q(veh)

-													
Intersection													
Int Delay, s/veh	3.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ች	<u></u>	7	ኘ	<u></u>	7	ች	<u></u>	7	ች	<u></u>	7	
Traffic Vol, veh/h	24	14	29	14	5	9	13	90	17	18	104	5	
Future Vol, veh/h	24	14	29	14	5	9	13	90	17	18	104	5	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	115	-	115	115	-	115	120	-	120	150	-	120	
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	26	15	32	15	5	10	14	98	18	20	113	5	
Major/Minor I	Minor2			Minor1			Major1			Major2			
Conflicting Flow All	296	297	113	305	284	98	118	0	0	116	0	0	
Stage 1	153	153	-	126	126	-	-	-	-	-	-	-	
Stage 2	143	144	-	179	158	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	688	634	940	679	646	1003	1470	-	-	1488	-	-	
Stage 1	849	771	-	914	808	-	-	-	-	-	-	-	
Stage 2	894	794	-	823	767	-	-	-	-	-	-	-	
Platoon blocked, %	1	1		1	1	1		-	-	1	-	-	
Mov Cap-1 Maneuver	666	620	940	633	631	1003	1470	-	-	1488	-	-	
Mov Cap-2 Maneuver	666	620	-	633	631	-	-	-	-	-	-	-	
Stage 1	841	761	-	904	800	-	-	-	-	-	-	-	
Stage 2	871	786	-	769	757	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	10			10.1			0.8			1.1			
HCM LOS	В			В									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2	EBLn3V	VBLn1V	VBLn2V	VBLn3	SBL	SBT	
Capacity (veh/h)		1470			666	620	940	633	631	1003	1488		
HCM Lane V/C Ratio		0.01	-	_			0.034			0.01	0.013	-	
HCM Control Delay (s)		7.5	-		10.6	11	9	10.8	10.8	8.6	7.5		
HCM Lane LOS		7.5 A	-	_	В	В	A	В	В	Α	7.5 A	-	
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August 2020

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Interception						
Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7		ተተተ	ተተተ	7
Traffic Vol, veh/h	0	46	0	1239	2005	19
Future Vol, veh/h	0	46	0	1239	2005	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage	2, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	50	0	1347	2179	21

Major/Minor	Minor2	N	lajor1	Ma	jor2	
Conflicting Flow All	-	1090	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	-	-	-	-
Pot Cap-1 Maneuver	0	181	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuve		181	-	-	-	-
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	32.3	0	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBT EBLn1	SBT	
Capacity (veh/h)	- 181	-	
HCM Lane V/C Ratio	- 0.276	-	
HCM Control Delay (s)	- 32.3	-	
HCM Lane LOS	- D	-	
HCM 95th %tile Q(veh)	- 1.1	-	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	ተተተ	7	ሻ	ተተተ	7		ሽኘ	ተተተ	7	1,1	^
Traffic Volume (vph)	236	1079	376	235	766	403	21	539	1641	141	349	919
Future Volume (vph)	236	1079	376	235	766	403	21	539	1641	141	349	919
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	0	3433	5085	1583	3433	5085
Flt Permitted	0.158			0.128				0.950			0.950	
Satd. Flow (perm)	294	5085	1583	238	5085	1583	0	3433	5085	1583	3433	5085
Satd. Flow (RTOR)			288			251				134		
Lane Group Flow (vph)	257	1173	409	255	833	438	0	609	1784	153	379	999
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	22.0	40.0	40.0	20.0	38.0	38.0	33.0	33.0	59.0	59.0	21.0	47.0
Total Split (%)	15.7%	28.6%	28.6%	14.3%	27.1%	27.1%	23.6%	23.6%	42.1%	42.1%	15.0%	33.6%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	51.8	33.0	33.0	48.2	31.2	31.2		27.3	52.0	52.0	16.0	40.7
Actuated g/C Ratio	0.37	0.24	0.24	0.34	0.22	0.22		0.20	0.37	0.37	0.11	0.29
v/c Ratio	0.90	0.98	0.69	1.04	0.74	0.80		0.91	0.94	0.23	0.97	0.68
Control Delay	66.1	74.3	20.8	105.3	55.1	33.6		73.9	54.0	7.3	99.0	46.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	66.1	74.3	20.8	105.3	55.1	33.6		73.9	54.0	7.3	99.0	46.7
LOS	E	E (1.2	С	F	E	С		E	D	Α	F	D
Approach Delay		61.3			57.3				55.9			52.8
Approach LOS	1/Γ	E	00	100	E	171		200	E	11	170	D
Queue Length 50th (ft)	165	391	98	~195	260	171		280	573	11	179	297
Queue Length 95th (ft)	#323	#494	225	#376	312	#324		#380	#676	58	#283	350
Internal Link Dist (ft)	42E	1022	200	225	405	225		42E	707	225	470	1957
Turn Bay Length (ft)	435	1100	200	225	1122	235		425	1000	325	670	1470
Base Capacity (vph)	288	1198	593	245	1132	547		686	1888	672	392	1478
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn Reduced v/c Ratio	0 0 0	0	0 60	1.04	0.74	0 00		0 00	0 04	0.23	0 07	0.68
Neulleu WC Rallu	0.89	0.98	0.69	1.04	0.74	0.80		0.89	0.94	0.23	0.97	0.00

Cycle Length: 140

Actuated Cycle Length: 140
Offset: 74 (53%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 110



Traffic Volume (vph) 227 Future Volume (vph) 227 Satd. Flow (prot) 1583 Flt Permitted Satd. Flow (perm) 1583 Satd. Flow (perm) 1583 Satd. Flow (perm) 247 Lane Group Flow (vph) 247 Turn Type Perm Protected Phases Permitted Phases 2 Detector Phase 3 Minimum Initial (s) 4.0 Minimum Split (s) 11.0 Total Split (s) 47.0 Total Split (%) 33.6% Yellow Time (s) 5.0 All-Red Time (s) 2.0 Lost Time Adjust (s) 7.0 Lead/Lag Lag Lead-Lag Optimize? Yes Recall Mode C-Max Act Effct Green (s) 40.7 Actuated g/C Ratio 0.29 v/c Ratio 0.29 v/c Ratio 0.39 Control Delay 6.2 Queue Delay 0.0 Total Delay 6.2 LOS AApproach LOS Queue Length 95th (ft) 0 Queue Length 95th (ft) 0 Queue Length 95th (ft) 0 Starvation Cap Reductn 0 Spillback Cap Reductn 0 Spillback Cap Reductn 0 Storage Cap Reductn 0 Reduced v/c Ratio 0.39	Lane Group	SBR
Traffic Volume (vph) 227 Future Volume (vph) 227 Satd. Flow (prot) 1583 Flt Permitted Satd. Flow (perm) 1583 Satd. Flow (RTOR) 247 Lane Group Flow (vph) 247 Turn Type Perm Protected Phases Permitted Phases 2 Detector Phase 3 Switch Phase 4 Minimum Initial (s) 4.0 Minimum Split (s) 11.0 Total Split (s) 47.0 Total Split (s) 33.6% Yellow Time (s) 5.0 All-Red Time (s) 2.0 Lost Time Adjust (s) 7.0 Lead/Lag Lag Lead-Lag Optimize? Yes Recall Mode C-Max Act Effct Green (s) 40.7 Actuated g/C Ratio 0.29 v/c Ratio 0.29 v/c Ratio 0.39 Control Delay 6.2 Queue Delay 0.0 Total Delay 6.2 Cueue Length 50th (ft) 0 Queue Length 95th (ft) 0 Queue Length 95th (ft) 1 Turn Bay Length (ft) 65 Internal Link Dist (ft) Turn Bay Length (ft) 635 Starvation Cap Reductn 0 Spillback Cap Reductn 0 Storage Cap Reductn 0 Reduced v/c Ratio 0.39		
Future Volume (vph) Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Perm Protected Phases Permitted Phases Permitted Phases Permitted Phases Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/C Ratio O.29 V/C Ratio Control Delay Control Delay Control Delay Control Delay Coueue Delay Approach LOS Queue Length 50th (ft) Oueue Length 95th (ft) Internal Link Dist (ft) Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn O Reduced v/C Ratio O.39 Cotrage Cap Reductn O Reduced v/C Ratio O.39 Cotrage Cap Reductn O O O O O O O O O O O O O		
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Switch Phase Minimum Initial (s) 4.0 Minimum Split (s) 11.0 Total Split (s) 47.0 Total Split (%) 33.6% Yellow Time (s) 5.0 All-Red Time (s) 2.0 Lost Time Adjust (s) 7.0 Lead/Lag Lag Lead-Lag Optimize? Yes Recall Mode C-Max Act Effet Green (s) 40.7 Actuated g/C Ratio 0.29 v/c Ratio 0.39 Control Delay 6.2 Queue Delay 0.0 Total Delay 6.2 LOS A Approach Delay 6.2 LOS A Approach LOS Queue Length 50th (ft) 0 Queue Length 95th (ft) 65 Internal Link Dist (ft) Turn Bay Length (ft) 265 Base Capacity (vph) 635 Starvation Cap Reductn 0 Spillback Cap Reductn 0 Storage Cap Reductn 0 Reduced v/c Ratio 0.39		
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Spillback Cap Reductn0Storage Cap Reductn0Reduced v/c Ratio0.39		0
Storage Cap Reductn 0 Reduced v/c Ratio 0.39		0
Reduced v/c Ratio 0.39		
		0.39
Intersection Summary	Intersection Summary	

1: Marksheffel Road & Constitution Avenue

PM Peak Hour - Year 2040

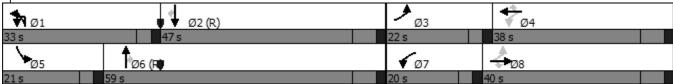
Maximum v/c Ratio: 1.04

Intersection Signal Delay: 56.8 Intersection LOS: E
Intersection Capacity Utilization 95.5% ICU Level of Service F

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



PM Peak Hour - Year 2040

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ተተተ	7	7	ተተተ	7	ሻሻ	î,		ሻሻ	1>	
Traffic Volume (vph)	242	1264	123	24	1380	157	128	51	172	333	33	135
Future Volume (vph)	242	1264	123	24	1380	157	128	51	172	333	33	135
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	1647	0	3433	1639	0
Flt Permitted	0.122			0.147			0.950			0.950		
Satd. Flow (perm)	227	5085	1583	274	5085	1583	3433	1647	0	3433	1639	0
Satd. Flow (RTOR)			134			171		39			91	
Lane Group Flow (vph)	263	1374	134	26	1500	171	139	242	0	362	183	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Split	NA		Split	NA	
Protected Phases		4			8		2	2		6	6	
Permitted Phases	4		4	8		8						
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	24.0	24.0	24.0	24.0	24.0	24.0		24.0	24.0	
Total Split (s)	72.0	72.0	72.0	72.0	72.0	72.0	24.0	24.0		24.0	24.0	
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	20.0%	20.0%		20.0%	20.0%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	68.2	68.2	68.2	68.2	68.2	68.2	17.2	17.2		16.6	16.6	
Actuated g/C Ratio	0.57	0.57	0.57	0.57	0.57	0.57	0.14	0.14		0.14	0.14	
v/c Ratio	2.05	0.48	0.14	0.17	0.52	0.18	0.28	0.90		0.76	0.60	
Control Delay	518.8	16.4	2.5	16.8	17.0	2.3	47.2	76.3		60.9	32.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	518.8	16.4	2.5	16.8	17.0	2.3	47.2	76.3		60.9	32.7	
LOS	F	В	Α	В	В	Α	D	E		E	С	
Approach Delay		89.9			15.5			65.7			51.5	
Approach LOS		F			В			E			D	
Queue Length 50th (ft)	~229	229	0	10	258	0	50	157		139	65	
Queue Length 95th (ft)	#401	269	28	28	302	31	81	#303		191	143	
Internal Link Dist (ft)		734			1022			309			581	
Turn Bay Length (ft)	275		235	235		275	120			355		
Base Capacity (vph)	128	2889	957	155	2889	973	514	280		514	323	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	2.05	0.48	0.14	0.17	0.52	0.18	0.27	0.86		0.70	0.57	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 48 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 150 Control Type: Actuated-Coordinated

2: Constitution Avenue & Akers Drive

PM Peak Hour - Year 2040

Maximum v/c Ratio: 2.05

Intersection Signal Delay: 54.3 Intersection LOS: D
Intersection Capacity Utilization 82.8% ICU Level of Service E

Analysis Period (min) 15

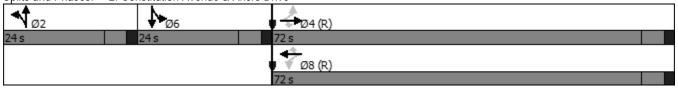
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Constitution Avenue & Akers Drive



August 2020

Intersection							
Int Delay, s/veh	7.5						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	EBL	EBR	NBL T	ND ↑	<u>361</u>	SBR 7	
Traffic Vol., veh/h	ገ 3	378	305	T 138	T 115		
Future Vol, veh/h	3	378	305	138	115	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	100	0	130	-	-	120	
Veh in Median Storage		-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	3	411	332	150	125	0	
Major/Minor	Minor2		Major1	N	Major2		
Conflicting Flow All	939	125	125	0	-	0	
Stage 1	125	-	-	-	-	-	
Stage 2	814	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	287	926	1462	-	-	-	
Stage 1	901	-	-	-	-	-	
Stage 2	425	-	-	-	-	-	
Platoon blocked, %	1			-	-	-	
Mov Cap-1 Maneuver	222	926	1462	-	-	-	
Mov Cap-2 Maneuver	332	-	-	-	-	-	
Stage 1	696	-	-	-	-	-	
Stage 2	425	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	12		5.6		0		
HCM LOS	В						
Minor Lane/Major Mvm	nt	NBL	NBT I	EBLn1 E	FBI n2	SBT	SBR
Capacity (veh/h)		1462	-	332	926	-	-
HCM Lane V/C Ratio		0.227	-		0.444	_	_
HCM Control Delay (s)		8.2	-	16	12	-	-
HCM Lane LOS		A	-	C	В	-	-
HCM 95th %tile Q(veh)	0.9	-	0	2.3	-	-
	,						

Intersection														
Int Delay, s/veh	3.8													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	ሻ	<u></u>	7	ሻ	<u></u>	7	ሻ	†	7	ሻ	<u></u>	7		
Traffic Vol, veh/h	16	9	19	11	16	8	45	104	8	38	102	16		
Future Vol, veh/h	16	9	19	11	16	8	45	104	8	38	102	16		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None		
Storage Length	115	-	115	115	-	115	120	-	120	150	-	120		
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-		
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-		
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2		
Mvmt Flow	17	10	21	12	17	9	49	113	9	41	111	17		
Major/Minor I	Minor2			Minor1			Major1			Major2				
Conflicting Flow All	422	413	111	428	421	113	128	0	0	122	0	0		
Stage 1	193	193	-	211	211	-	-	-	-	-	-	-		
Stage 2	229	220	-	217	210	-	-	-	-	-	-	-		
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-		
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-		
Pot Cap-1 Maneuver	562	542	942	558	537	983	1458	-	-	1481	-	-		
Stage 1	809	741	-	819	740	-	-	-	-	-	-	-		
Stage 2	800	734	-	785	728	-	-	-	-	-	-	-		
Platoon blocked, %	1	1		1	1	1		-	-	1	-	-		
Mov Cap-1 Maneuver	518	509	942	512	504	983	1458	-	-	1481	-	-		
Mov Cap-2 Maneuver	518	509	-	512	504	-	-	-	-	-	-	-		
Stage 1	781	720	-	791	715	-	-	-	-	-	-	-		
Stage 2	748	709	-	736	708	-	-	-	-	-	-	-		
Approach	EB			WB			NB			SB				
HCM Control Delay, s	10.8			11.5			2.2			1.8				
HCM LOS	В			В										
Minor Lanc/Major Mum	nt .	NBL	NIDT	MPD	ERI n1	FRI n2	FRI n2\	VBLn1V	V/RI n2V	MRI n2	SBL	SBT	SBR	
Minor Lane/Major Mvm	IL		NBT	NDK								SDI	SDK	
Capacity (veh/h)		1458	-	-	518	509	942	512 0.023	504	983	1481	-	-	
HCM Control Dolay (s)		0.034 7.6	-	-		12.2	8.9				0.028	-	-	
HCM Control Delay (s) HCM Lane LOS			-		12.2			12.2	12.4	8.7	7.5	-	-	
HCM 95th %tile Q(veh)	١	0.1	-	-	0.1	0.1	0.1	0.1	0.1	A 0	0.1	-	-	
HOW YOU WILL U(VEI)		0.1	-		0.1	0.1	0.1	0.1	0.1	U	U. I	-	-	

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Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	LDL	- LDIK	NDL			JDIK
Lane Configurations				ተተተ	^	ľ
Traffic Vol, veh/h	0	56	0	2275	1466	21
Future Vol, veh/h	0	56	0	2275	1466	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92

Major/Minor	Minor2	N	lajor1	Ma	jor2	
Conflicting Flow All	-	797	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	-	-	-	-
Pot Cap-1 Maneuver	0	283	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuve		283	-	-	-	-
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	21.2	0	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBT EBLn1	SBT	
Capacity (veh/h)	- 283	-	
HCM Lane V/C Ratio	- 0.215	-	
HCM Control Delay (s)	- 21.2	-	
HCM Lane LOS	- C	-	
HCM 95th %tile Q(veh)	- 0.8	-	

Heavy Vehicles, %
Mvmt Flow

	۶	→	•	•	←	•	₹I	1	†	<i>></i>	/	+
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	¥	† †	7	7	^	7		ሽኘ	^	7	ቪቪ	† †
Traffic Volume (vph)	114	280	282	150	321	195	25	195	554	88	147	1132
Future Volume (vph)	114	280	282	150	321	195	25	195	554	88	147	1132
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.405			0.459				0.950			0.950	
Satd. Flow (perm)	754	3539	1583	855	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			228			212				127		
Lane Group Flow (vph)	124	304	307	163	349	212	0	239	602	96	160	1230
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	15.0	30.0	30.0	15.0	30.0	30.0	20.0	20.0	55.0	55.0	20.0	55.0
Total Split (%)	12.5%	25.0%	25.0%	12.5%	25.0%	25.0%	16.7%	16.7%	45.8%	45.8%	16.7%	45.8%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	28.7	17.2	17.2	29.3	17.5	17.5		13.1	58.1	58.1	10.9	55.8
Actuated g/C Ratio	0.24	0.14	0.14	0.24	0.15	0.15		0.11	0.48	0.48	0.09	0.46
v/c Ratio	0.48	0.60	0.73	0.57	0.68	0.52		0.64	0.35	0.12	0.51	0.75
Control Delay	38.5	52.8	23.9	42.1	55.1	10.4		58.9	21.1	2.0	57.6	31.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	38.5	52.8	23.9	42.1	55.1	10.4		58.9	21.1	2.0	57.6	31.0
LOS	D	D	С	D	Е	В		E	С	Α	E	С
Approach Delay		38.3			39.1				28.8			31.5
Approach LOS	7.	D	.	100	D	•		00	C	•		C
Queue Length 50th (ft)	74	117	56	100	137	0		92	148	0	61	404
Queue Length 95th (ft)	117	156	152	150	177	65		133	222	18	95	553
Internal Link Dist (ft)	405	1022	000	005	405	005		105	707	0.05	(70	1957
Turn Bay Length (ft)	435	/70	200	225	/70	235		425	4740	325	670	4/1/
Base Capacity (vph)	267	678	487	286	678	474		429	1712	831	429	1646
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.46	0.45	0.63	0.57	0.51	0.45		0.56	0.35	0.12	0.37	0.75

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green
Natural Cycle: 65

Control Type: Actuated-Coordinated



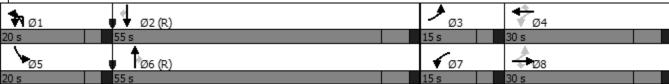
Lana Craun	CDD
Lane Group	SBR
Lare Configurations	124
Traffic Volume (vph)	124
Future Volume (vph)	124
Satd. Flow (prot)	1583
Flt Permitted	4500
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	127
Lane Group Flow (vph)	135
Turn Type	Perm
Protected Phases	
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	55.0
Total Split (%)	45.8%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	55.8
Actuated g/C Ratio	0.46
v/c Ratio	0.17
Control Delay	4.9
Queue Delay	0.0
Total Delay	4.9
LOS	А
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	3
Queue Length 95th (ft)	43
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	804
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.17
Intersection Summary	
Intersection Summary	

AM Peak Hour - Year 2022

Maximum v/c Ratio: 0.75

Intersection Signal Delay: 33.5 Intersection LOS: C
Intersection Capacity Utilization 83.3% ICU Level of Service E
Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



2: Constitution Avenue & Akers Drive

Intersection									
Int Delay, s/veh	0.9								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	ሻ	^ ^	^	7	ř	7			
Traffic Vol, veh/h	46	603	565	63	73	72			
Future Vol, veh/h	46	603	565	63	73	72			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-	None	-	Free			
Storage Length	275	-	-	-	355	0			
Veh in Median Storage		0	0	-	0	-			
Grade, %	-	0	0	-	0	-			
Peak Hour Factor	92	92	92	92	92	92			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	50	655	614	68	79	78			
WWW.	- 30	- 000	017	00		70			
Mainu/Minna	Ma!au1		1-:0		/! ?	_			
	Major1		Major2		Minor2				
Conflicting Flow All	682	0	-	0	976	-			
Stage 1	-	-	-	-	614	-			
Stage 2	-	-	-	-	362	-			
Critical Hdwy	4.14	-	-	-	6.29	-			
Critical Hdwy Stg 1	-	-	-	-	5.84	-			
Critical Hdwy Stg 2	-	-	-	-	6.04	-			
Follow-up Hdwy	2.22	-	-	-	3.67	-			
Pot Cap-1 Maneuver	*1306	-	-	-	*675	0			
Stage 1	-	-	-	-	*790	0			
Stage 2	-	-	-	-	*639	0			
Platoon blocked, %	1	-	-	-	1				
Mov Cap-1 Maneuver	*1306	-	-	-	*649	-			
Mov Cap-2 Maneuver	-	-	-	-	*649	-			
Stage 1	-	-	-	-	*760	-			
Stage 2	-	-	-	-	*639	-			
J. T. J.									
Approach	EB		WB		SB				
HCM Control Delay, s	0.6		0		11.3				
HCM LOS	0.0				В				
Minor Long /Mailer Md		EDI	EDT	WDT	MDD	CDL 1 (2DI 2		
Minor Lane/Major Mvm		EBL	EBT	WBT	WRK:	SBLn1 S	PRFU5		
Capacity (veh/h)		* 1306	-	-	-	649	-		
HCM Lane V/C Ratio		0.038	-	-	-	0.122	-		
HCM Control Delay (s)		7.9	-	-	-	11.3	0		
HCM Lane LOS		Α	-	-	-	В	Α		
HCM 95th %tile Q(veh))	0.1	-	-	-	0.4	-		
Notes									
~: Volume exceeds cap	nacity	\$· De	elav evo	ceeds 30	20s	+. Com	putation Not Defined	*: All major volume in platoon	
. Volume exceeds ca	pacity	ψ. DC	hay CAC	ccus si	503	i. Com	patation Not Delineu	. All major volume in platoon	

Intersection												
Int Delay, s/veh	3.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘ	<u></u>	7	ኝ	1		ኝ	<u></u>	7	ሻ	†	7
Traffic Vol, veh/h	0	0	9	64	0	8	10	86	7	7	72	0
Future Vol, veh/h	0	0	9	64	0	8	10	86	7	7	72	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	10	70	0	9	11	93	8	8	78	0
Major/Minor I	Minor2			Minor1			Major1		[Major2		
Conflicting Flow All	218	217	78	214	209	93	78	0	0	101	0	0
Stage 1	94	94	-	115	115	-	-	-	-	-	-	-
Stage 2	124	123	-	99	94	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	738	681	983	743	688	964	1520	-	-	1491	-	-
Stage 1	913	817	-	890	800	-	-	-	-	-	-	-
Stage 2	880	794	-	907	817	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	724	673	983	729	680	964	1520	-	-	1491	-	-
Mov Cap-2 Maneuver	724	673	-	729	680	-	-	-	-	-	-	-
Stage 1	907	813	-	884	794	-	-	-	-	-	-	-
Stage 2	866	788	-	893	813	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.7			10.3			0.7			0.7		
HCM LOS	Α			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2	EBLn3\	WBLn1V	VBLn2	SBL	SBT	SBR
Capacity (veh/h)		1520	-	-		-	983	729	964	1491	-	
HCM Lane V/C Ratio		0.007	_	-	_	_				0.005	-	-
HCM Control Delay (s)		7.4	-	-	0	0	8.7	10.5	8.8	7.4	-	-
HCM Lane LOS		A	-	-	A	A	A	В	A	A	-	_
HCM 95th %tile Q(veh))	0	-	-	-	-	0	0.3	0	0	-	-

Intersection						
Int Delay, s/veh	1.3					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	\	,	}	11	<u>ነ</u>	
Traffic Vol, veh/h	11	6	78	11	12	77
Future Vol, veh/h	11	6	78	11	12	77
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	7	85	12	13	84
Major/Minor	Minor1	N	Major1		Major2	
Conflicting Flow All	201	91	0	0	97	0
Stage 1	91	91	-	U	97	-
				-	-	
Stage 2	110	-	-	-		-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-		-
Pot Cap-1 Maneuver	788	967	-	-	1496	-
Stage 1	933	-	-	-	-	-
Stage 2	915	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	781	967	-	-	1496	-
Mov Cap-2 Maneuver	776	-	-	-	-	-
Stage 1	933	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Annroach	\A/D		NID		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	9.4		0		1	
HCM LOS	Α					
	nt .	NBT	NBRV	VBLn1	SBL	SBT
Minor Lane/Maior Myn		1101			1496	-
Minor Lane/Major Mvn		_		(1.14	1470	
Capacity (veh/h)		-	-			_
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.022	0.009	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		-	-	0.022 9.4	0.009 7.4	-
Capacity (veh/h) HCM Lane V/C Ratio)		-	0.022	0.009	

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	7	1100	↑ ↑	† †	7
Traffic Vol, veh/h	0	22	0	861	1398	11
Future Vol, veh/h	0	22	0	861	1398	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Otop	Yield		None	-	Free
Storage Length	_	0	_	-	_	240
Veh in Median Storage		-	_	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	24	0	936	1520	12
WWW. LIOW	U	27	U	700	1320	12
	Minor2		/lajor1		Major2	
Conflicting Flow All	-	760	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	349	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	349	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	_
g · -						
	F.D.		NE		65	
Approach	EB		NB		SB	
HCM Control Delay, s	16.1		0		0	
HCM LOS	С					
Minor Lane/Major Mvm	t	NBT E	EBLn1	SBT		
Capacity (veh/h)		-		-		
HCM Lane V/C Ratio			0.069	-		
HCM Control Delay (s)		_		_		
HCM Lane LOS		_	C	-		
HCM 95th %tile Q(veh)		-	0.2	-		
HOW /SUT /OUIC Q(VCH)			0.2			

Intersection						
Intersection Int Delay, s/veh	0.3					
		WED	NET	NDD	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		_ 7		₹		
Traffic Vol, veh/h	0	8	95	14	0	145
Future Vol, veh/h	0	8	95	14	0	145
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	120	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	9	103	15	0	158
IVIVIIICI IOW	- 0	/	103	10	- 0	100
	linor1		Major1	N	/lajor2	
Conflicting Flow All	-	103	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	_	-	-	-	_	_
Critical Hdwy Stg 2					_	-
Follow-up Hdwy		3.318	-	-	-	-
	0	952	-	-		-
Pot Cap-1 Maneuver			-		0	
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	952	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
J .						
Annroach	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	952	-	
HCM Cartral Dalay (a)		-	-	0.009	-	
HCM Control Delay (s)		-	-	8.8	-	
HCM Lane LOS		-	-	A	-	
HCM 95th %tile Q(veh)		-	-	0	-	

	٠	→	•	•	←	4	₹î	1	†	<i>></i>	/	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	7	† †	7	J.	† †	7		ሽኘ	^	7	ቪቪ	^
Traffic Volume (vph)	118	580	242	181	453	296	15	372	1147	115	255	637
Future Volume (vph)	118	580	242	181	453	296	15	372	1147	115	255	637
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	0	3433	3539	1583	3433	3539
Flt Permitted	0.351			0.166				0.950			0.950	
Satd. Flow (perm)	654	3539	1583	309	3539	1583	0	3433	3539	1583	3433	3539
Satd. Flow (RTOR)			254			322				109		
Lane Group Flow (vph)	128	630	263	197	492	322	0	420	1247	125	277	692
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	21.0	46.0	46.0	21.0	46.0	46.0	22.0	22.0	51.0	51.0	22.0	51.0
Total Split (%)	15.0%	32.9%	32.9%	15.0%	32.9%	32.9%	15.7%	15.7%	36.4%	36.4%	15.7%	36.4%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	45.4	31.1	31.1	51.2	34.0	34.0		22.3	53.7	53.7	16.0	47.4
Actuated g/C Ratio	0.32	0.22	0.22	0.37	0.24	0.24		0.16	0.38	0.38	0.11	0.34
v/c Ratio	0.41	0.80	0.48	0.73	0.57	0.51		0.77	0.92	0.19	0.71	0.58
Control Delay	32.1	59.4	8.4	45.8	49.0	7.1		66.6	53.2	8.6	69.8	41.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	32.1	59.4	8.4	45.8	49.0	7.1		66.6	53.2	8.6	69.8	41.2
LOS	С	E	Α	D	D	Α		E	D	Α	E	D
Approach Delay		42.9			35.1				53.2			43.7
Approach LOS		D		400	D				D			D
Queue Length 50th (ft)	77	288	6	123	207	0		187	574	9	126	282
Queue Length 95th (ft)	115	337	76	174	257	75		#301	#827	58	174	351
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	357	985	624	280	985	673		546	1357	674	428	1199
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.36	0.64	0.42	0.70	0.50	0.48		0.77	0.92	0.19	0.65	0.58

Intersection Summary

Cycle Length: 140

Actuated Cycle Length: 140
Offset: 74 (53%), Referenced to phase 2:SBT and 6:NBT, Start of Green

Natural Cycle: 90 Control Type: Actuated-Coordinated



Lane Group	SBR
Lare Configurations	7
Traffic Volume (vph)	134
Future Volume (vph)	134
Satd. Flow (prot)	1583
Flt Permitted	
Satd. Flow (perm)	1583
Satd. Flow (RTOR)	146
Lane Group Flow (vph)	146
Turn Type	Perm
Protected Phases	
Permitted Phases	2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	51.0
	36.4%
Total Split (%)	
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	47.4
Actuated g/C Ratio	0.34
v/c Ratio	0.23
Control Delay	6.1
Queue Delay	0.0
Total Delay	6.1
LOS	А
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	0
Queue Length 95th (ft)	50
Internal Link Dist (ft)	30
Turn Bay Length (ft)	265
Base Capacity (vph)	632
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.23
Intersection Summary	

PM Peak Hour - Year 2022

Maximum v/c Ratio: 0.92

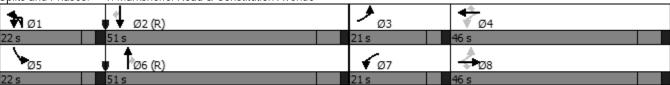
Intersection Signal Delay: 45.2 Intersection LOS: D Intersection Capacity Utilization 85.0% ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



Synchro Report August 2020

Int Delay, s/veh									
Movement	Intersection								
Lane Configurations	Int Delay, s/veh	0.5							
Lane Configurations	Movement	FBI	FBT	WBT	WBR	SBI	SBR		
Traffic Vol. veh/h 58 875 885 78 47 65 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Stop RT Channelized None - None - None - Storage Length 275 355 0 Veh in Median Storage, # - 0 0 0 - 0 - 0 Feak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 MmI Flow All 1047 0 - 0 1468 - Stage 1 962 Stage 1 962 - Stage 2 506 - 2 Critical Hdwy Stg 1 5084									
Future Vol, veh/h 58 875 885 78 47 65 Conflicting Peds, #hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Free Fre									
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0									
Sign Control Free RT Channelized Free None Free None Stop Stop RT Channelized None - None - Free Free Free Free Storage Length 275 - 0 0 - 0 - 0 - 0 Peak Hour Factor 92 92 92 92 92 92 Peak Hour Factor 92 29 92 92 92 92 Heavy Vehicles, % 2 3 3 7									
RT Channelized - None - None - Free Storage Length 275 355 0 Verb in Median Storage, # - 0 0 0 - 0 - Grade, % - 0 0 0 - 0 - Peak Hour Factor 92 92 92 92 92 92 92 92 Peak Hour Factor 92 92 92 92 92 92 92 Peak Hour Factor 92 93 95 962 85 51 71 Verbin Flow 63 951 962 85 51 71 Verbin Flow 63 951 962 85 51 71 Verbin Flow Major 1 Major 2 Minor 2 Verbin Flow Major 1 Major 2 Minor 2 Verbin Flow Major 1 Major 2 Minor 2 Verbin Flow Major 1 Major 2 Verbin Flow Major 1 Major 2 Verbin Flow Major 1 Major 2 Verbin Flow Major 2 Verbin Flow Major 2 Verbin Flow Major 3 Verbin Flow Major 3 Verbin Flow Major 3 Verbin Flow Major 3 Verbin Flow Major 4 Ve									
Storage Length									
Veh in Median Storage, # - 0 0 0 - 0 - 0 - 0 Grade, % - 0 0 0 - 0 0 - 0 - 0 Grade, % - 0 0 0 - 0 0 - 0 Grade, % - 0 0 0 - 0 0 - 0 0 - 0 Grade, % - 0 0 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0		275		-		355			
Grade, %			0	0	-				
Peak Hour Factor 92 93 94	Grade, %		0	0	-	0	-		
Mymit Flow 63 951 962 85 51 71 Major/Minor Major1 Major2 Minor2 Conflicting Flow All 1047 0 - 0 1468 - Stage 1 - - - 962 - - Stage 2 - - - 506 - Critical Hdwy 4.14 - - 6.29 - Critical Hdwy Stg 1 - - - 5.84 - Critical Hdwy Stg 2 - - 6.04 - Follow-up Hdwy 2.22 - - 3.67 - Follow-up Hdwy 2.22 - - 3.67 - Pot Cap-1 Maneuver *1087 - - 658 0 Stage 1 - - - *658 0 Stage 2 - - *620 - Stage 1 - - *620 -	Peak Hour Factor	92	92	92	92	92	92		
Major/Minor Major1 Major2 Minor2 Conflicting Flow All 1047 0 - 0 1468 - Stage 1 962 - 506 - Critical Hdwy 4.14 6.29 - Critical Hdwy Stg 1 5.84 - Critical Hdwy Stg 1 5.84 - Critical Hdwy Stg 2 3.67 - Follow-up Hdwy 2.22 3.67 - Follow-up Hdwy 2.22 3.67 - Follow-up Hdwy 2.22 6.04 - Follow-up Hdwy 2.22 6.58 0 Stage 1 658 0 Stage 2 7537 0 Platoon blocked, % 1 1 Mov Cap-1 Maneuver *1087 *620 - Follow-up Hdwy Cap-2 Maneuver *	Heavy Vehicles, %	2	2	2	2	2	2		
Conflicting Flow All 1047 0 - 0 1468 - Stage 1 962 - Stage 2 506 - Critical Hdwy Val. 4.14 6.29 - Critical Hdwy Stg 1 6.29 - Critical Hdwy Stg 2 6.04 - Follow-up Hdwy	Mvmt Flow	63	951	962	85	51	71		
Conflicting Flow All 1047 0 - 0 1468 - Stage 1 962 - Stage 2 506 - Critical Hdwy Val. 4.14 6.29 - Critical Hdwy Stg 1 6.29 - Critical Hdwy Stg 2 6.04 - Follow-up Hdwy									
Conflicting Flow All 1047 0 - 0 1468 - Stage 1 962 - Stage 2 506 - Critical Hdwy Val. 4.14 6.29 - Critical Hdwy Stg 1 6.29 - Critical Hdwy Stg 2 6.04 - Follow-up Hdwy	Major/Minor	Major1	N	Major2		Minor?			
Stage 1							_		
Stage 2				-					
Critical Hdwy									
Critical Hdwy Stg 1									
Critical Hdwy Stg 2				_					
Follow-up Hdwy 2.22 3.67 - Pot Cap-1 Maneuver *1087 *658 0 Stage 1 *658 0 Stage 2 *537 0 Platon blocked, % 1 *620 - Mov Cap-1 Maneuver *1087 *620 - Stage 1 *620 - Stage 1 *537 - Mov Cap-2 Maneuver *620 - Stage 1 *537 - Mov Cap-2 Maneuver *620 - Stage 2 *537 - Approach EB WB SB HCM Control Delay, \$ 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) *1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes			-						
Pot Cap-1 Maneuver *1087 *658 0 Stage 1 *658 0 Stage 2 *537 0 Platoon blocked, % 1 1 Mov Cap-1 Maneuver *1087 *620 - Mov Cap-2 Maneuver *620 - Stage 1 *620 - Stage 2 *537 - Mov Cap-2 Maneuver *620 - Stage 1 *620 - Stage 2 *537 - Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) * 1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A - B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes			_	-					
Stage 1 *658			-	_	_				
Stage 2 - - - *537 0 Platoon blocked, % 1 - - 1 Mov Cap-1 Maneuver *1087 - - *620 - Mov Cap-2 Maneuver - - - *620 - Stage 1 - - - *620 - Stage 2 - - - *537 - Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) * 1087 - - 620 - HCM Lane V/C Ratio 0.058 - - 0.082 - HCM Control Delay (s) 8.5 - - 11.3 0 HCM Lane LOS A - - - - - - - - - - - -	•	-	-	-	_				
Platoon blocked, % 1 1 Mov Cap-1 Maneuver *1087 *620 - Mov Cap-2 Maneuver *620 - Stage 1 *620 - Stage 2 *537 - Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) * 1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A - B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes		-	-		-				
Mov Cap-1 Maneuver *1087 *620 - Mov Cap-2 Maneuver *620 - Stage 1 *620 - Stage 2 *537 - Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) *1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A - B A HCM 95th %tile Q(veh) 0.2 - 0.3 - Notes		1	-	-	-				
Mov Cap-2 Maneuver - - *620 - Stage 1 - - - *537 - Stage 2 - - - *537 - Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) * 1087 - - 620 - HCM Lane V/C Ratio 0.058 - - 0.082 - HCM Control Delay (s) 8.5 - - 11.3 0 HCM Lane LOS A - - B A HCM 95th %tile Q(veh) 0.2 - - 0.3 - Notes		*1087	-	-	-	*620	-		
Stage 1			-	-	-		-		
Stage 2	•	-	-	-	-		-		
Approach EB WB SB HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) * 1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 0.3 -		-	-	-	-		-		
HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) *1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 - 0.3 - Notes									
HCM Control Delay, s 0.5 0 11.3 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) *1087 620 - HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 - 0.3 - Notes	Annroach	FR		\M/R		SB			
Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2									
Minor Lane/Major Mvmt		0.5		U					
Capacity (veh/h) * 1087 - - 620 - HCM Lane V/C Ratio 0.058 - - 0.082 - HCM Control Delay (s) 8.5 - - 11.3 0 HCM Lane LOS A - - B A HCM 95th %tile Q(veh) 0.2 - - 0.3 - Notes	TIGIVI EUS					В			
Capacity (veh/h) * 1087 - - 620 - HCM Lane V/C Ratio 0.058 - - 0.082 - HCM Control Delay (s) 8.5 - - 11.3 0 HCM Lane LOS A - - B A HCM 95th %tile Q(veh) 0.2 - - 0.3 - Notes			==-		14/5-	14/5=	001 1		
HCM Lane V/C Ratio 0.058 0.082 - HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes				EBT	WBT			SBLn2	
HCM Control Delay (s) 8.5 11.3 0 HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes				-					
HCM Lane LOS A B A HCM 95th %tile Q(veh) 0.2 0.3 - Notes				-					
HCM 95th %tile Q(veh) 0.2 0.3 - Notes)							
Notes		\		-					
	HCM 95th %tile Q(veh	1)	0.2	-	-	-	0.3	•	
	Notes								
	~: Volume exceeds ca	pacity	\$: De	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major volume in platoon

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Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u></u>	7	ሻ	֔		ř		7	ķ		7
Traffic Vol, veh/h	2	0	4	41	0	5	10	69	20	20	67	0
Future Vol, veh/h	2	0	4	41	0	5	10	69	20	20	67	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	0	4	45	0	5	11	75	22	22	73	0
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	228	236	73	216	214	75	73	0	0	97	0	0
Stage 1	117	117	-	97	97	-	-	-	-	-	-	-
Stage 2	111	119	-	119	117	-	-	-	-	_	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52		6.12	5.52		-	-	_		-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	_	2.218	-	-
Pot Cap-1 Maneuver	727	665	989	740	684	986	1527	-	-	1496	-	-
Stage 1	888	799	-	910	815	-		-	_	-	-	-
Stage 2	894	797	-	885	799	-	-	-	-	-	-	-
Platoon blocked, %	3,1	.,,		300				-			-	-
Mov Cap-1 Maneuver	711	650	989	724	669	986	1527	-	-	1496	-	-
Mov Cap-2 Maneuver	711	650	-	724	669	-		-	-	-	-	-
Stage 1	882	787	-	904	809	-	-	-	-	-	-	-
Stage 2	883	791	-	868	787	-	-	-	-	_	-	_
3 g												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.2			10.1			0.7			1.7		
HCM LOS	Α.Σ			В			0.7			117		
	, (
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	FBI n1	FBI n2	FBI n3\	NBLn1V	VRI n2	SBL	SBT	SBR
Capacity (veh/h)		1527	-	NDIX	711	-	989	724	986	1496	- 201	JUN
HCM Lane V/C Ratio		0.007	-		0.003			0.062			-	-
HCM Control Delay (s)		7.4	-	-	10.1	0	8.7	10.3	8.7	7.4	-	-
HCM Lane LOS		7.4 A	-	-	В	A	0.7 A	10.3 B	Α.7	7.4 A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	-	0	0.2	0	0	-	_
HOW FOUT FOUTE QIVET	1						0	0.2		0		

Intersection						
Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥*		1		ኝ	<u> </u>
Traffic Vol, veh/h	11	5	82	5	26	87
Future Vol, veh/h	11	5	82	5	26	87
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	5	89	5	28	95
WWW. Cow	12	J	07		20	70
	Minor1		Major1		Major2	
Conflicting Flow All	243	92	0	0	94	0
Stage 1	92	-	-	-	-	-
Stage 2	151	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-	2.218	-
Pot Cap-1 Maneuver	745	965	-	-	1500	-
Stage 1	932	-	-	-	-	-
Stage 2	877	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	731	965	-	-	1500	-
Mov Cap-2 Maneuver	739	-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	860	-	-	-	-	-
Annragah	MD		ND		CD	
Approach Dalama	WB		NB		SB	
HCM Control Delay, s	9.6		0		1.7	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	797	1500	
HCM Lane V/C Ratio		-		0.022		-
HCM Control Delay (s)		-	-	9.6	7.4	-
HCM Lane LOS		_	_	Α.	Α	-
HCM 95th %tile Q(veh))	-	-	0.1	0.1	-
				J. 1	3.1	

Intersection						
Int Delay, s/veh	0.2					
		FDD	NDL	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	0	*	0	^	^	7
Traffic Vol, veh/h	0	32	0	1556	1012	7
Future Vol, veh/h	0	32	0	1556	1012	7
Conflicting Peds, #/hr	0	0	0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield		None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	35	0	1691	1100	8
Major/Minor	linor?		Anior1	N	Anior?	
	1inor2		/lajor1		Major2	
Conflicting Flow All	-	550	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	479	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	479	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	_		_	_	_	_
Approach	EB		NB		SB	
HCM Control Delay, s	13.1		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT E	ERI n1	SBT		
Capacity (veh/h)		-	.,,	-		
HCM Lane V/C Ratio			0.073	-		
HCM Control Delay (s)		-	13.1	-		
HCM Lane LOS		-	В	-		
HCM 95th %tile Q(veh)		-	0.2	_		

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		_ ₹		_ 7		
Traffic Vol, veh/h	0	5	95	41	0	112
Future Vol, veh/h	0	5	95	41	0	112
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	120	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	5	103	45	0	122
WWW.CT IOW	U	U	100	10	U	122
	1inor1		/lajor1		/lajor2	
Conflicting Flow All	-	103	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	_	_	-	-
Pot Cap-1 Maneuver	0	952	_	-	0	-
Stage 1	0	-	_	_	0	_
Stage 2	0	-		-	0	-
Platoon blocked, %	U	-	-		U	
		በርን	-	-		-
Mov Cap-1 Maneuver	-	952	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		0	
HCM LOS	0.6 A		U		U	
HCIVI LU3	А					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)			-	952	-	
HCM Lane V/C Ratio		-	_	0.006	-	
HCM Control Delay (s)				8.8	-	
HCM Lane LOS		-	_	Α	-	
HCM 95th %tile Q(veh)		-	-	0	-	
HOW 95th %the Q(ven)			-	U	-	

	•	→	•	•	•	•	₹I	~	†	/	>	ļ
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	ተተተ	7	Ť	ተተተ	7		ሽኘ	ተተተ	7	ቪቪ	*
Traffic Volume (vph)	190	477	415	186	496	265	36	286	794	118	199	1632
Future Volume (vph)	190	477	415	186	496	265	36	286	794	118	199	1632
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	0	3433	5085	1583	3433	5085
Flt Permitted	0.278			0.450				0.950			0.950	
Satd. Flow (perm)	518	5085	1583	838	5085	1583	0	3433	5085	1583	3433	5085
Satd. Flow (RTOR)			142			264				128		
Lane Group Flow (vph)	207	518	451	202	539	288	0	350	863	128	216	1774
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	16.0	35.0	35.0	11.0	30.0	30.0	18.0	18.0	57.0	57.0	17.0	56.0
Total Split (%)	13.3%	29.2%	29.2%	9.2%	25.0%	25.0%	15.0%	15.0%	47.5%	47.5%	14.2%	46.7%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	40.2	27.2	27.2	30.2	22.2	22.2		13.4	51.5	51.5	11.3	49.4
Actuated g/C Ratio	0.34	0.23	0.23	0.25	0.18	0.18		0.11	0.43	0.43	0.09	0.41
v/c Ratio	0.72	0.45	0.96	0.79	0.57	0.57		0.91	0.40	0.17	0.67	0.85
Control Delay	41.9	37.6	61.6	57.4	47.1	11.7		82.0	24.5	4.2	62.9	36.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	41.9	37.6	61.6	57.4	47.1	11.7		82.0	24.5	4.2	62.9	36.8
LOS	D	D	E	E	D	В		F	С	Α	E	D
Approach Delay		47.5			39.2				37.6			36.5
Approach LOS		D			D				D			D
Queue Length 50th (ft)	129	132	258	116	139	15		140	169	0	84	450
Queue Length 95th (ft)	#211	171	#396	#212	179	97		#233	206	37	125	517
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	288	1186	478	257	974	516		383	2181	752	343	2094
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	0.72	0.44	0.94	0.79	0.55	0.56		0.91	0.40	0.17	0.63	0.85

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green
Natural Cycle: 90

Control Type: Actuated-Coordinated



Lano Croun	SBR
Lane Group	28K
Land Configurations Traffic Volume (vph)	r
Traffic Volume (vph)	189
Future Volume (vph)	
Satd. Flow (prot) Flt Permitted	1583
	1500
Satd. Flow (perm)	1583
Satd. Flow (RTOR) Lane Group Flow (vph)	176 205
Turn Type	Perm
Protected Phases	2
Permitted Phases Detector Phase	2 2
	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	56.0
Total Split (%)	46.7%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	49.4
Actuated g/C Ratio	0.41
v/c Ratio	0.27
Control Delay	5.9
Queue Delay	0.0
Total Delay	5.9
LOS	A
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	13
Queue Length 95th (ft)	61
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	755
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.27
Intersection Summary	
g	

1: Marksheffel Road & Constitution Avenue

Maximum v/c Ratio: 0.96

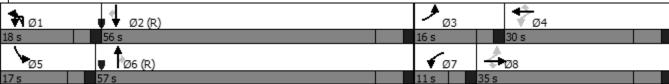
Intersection Signal Delay: 39.5 Intersection LOS: D Intersection Capacity Utilization 96.7% ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



	•	→	•	•	←	•	4	†	/	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ŋ.	ተተተ	7	ř	ተተተ	7	1/4	f)		1,1	₽	
Traffic Volume (vph)	140	890	73	29	837	117	41	11	69	185	15	111
Future Volume (vph)	140	890	73	29	837	117	41	11	69	185	15	111
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	1622	0	3433	1617	0
Flt Permitted	0.292			0.272			0.950			0.950		
Satd. Flow (perm)	544	5085	1583	507	5085	1583	3433	1622	0	3433	1617	0
Satd. Flow (RTOR)			82			127		75			121	
Lane Group Flow (vph)	152	967	79	32	910	127	45	87	0	201	137	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Split	NA		Split	NA	
Protected Phases		4			8		2	2		6	6	
Permitted Phases	4		4	8		8						
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	24.0	24.0	24.0	24.0	24.0	24.0		24.0	24.0	
Total Split (s)	72.0	72.0	72.0	72.0	72.0	72.0	24.0	24.0		24.0	24.0	
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	20.0%	20.0%		20.0%	20.0%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	82.0	82.0	82.0	82.0	82.0	82.0	7.7	7.7		12.3	12.3	
Actuated g/C Ratio	0.68	0.68	0.68	0.68	0.68	0.68	0.06	0.06		0.10	0.10	
v/c Ratio	0.41	0.28	0.07	0.09	0.26	0.11	0.20	0.50		0.57	0.50	
Control Delay	13.6	8.1	1.8	10.7	10.1	4.3	54.4	25.4		57.6	18.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	13.6	8.1	1.8	10.7	10.1	4.3	54.4	25.4		57.6	18.0	
LOS	В	Α	Α	В	В	Α	D	С		E	В	
Approach Delay		8.4			9.4			35.3			41.5	
Approach LOS		A	•	10	A	10	47	D			D	
Queue Length 50th (ft)	46	95	0	10	112	13	17	9		77	11	
Queue Length 95th (ft)	114	142	17	m21	m153	m26	36	59		113	71	
Internal Link Dist (ft)	075	734	005	005	1022	075	400	276		055	237	
Turn Bay Length (ft)	275	0.47.4	235	235	0.47.4	275	120	007		355	0.45	
Base Capacity (vph)	371	3474	1107	346	3474	1121	514	307		514	345	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0 11	0	0		0	0	
Reduced v/c Ratio	0.41	0.28	0.07	0.09	0.26	0.11	0.09	0.28		0.39	0.40	

Intersection Summary

Cycle Length: 120

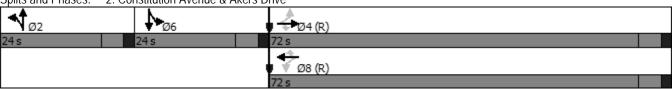
Actuated Cycle Length: 120
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green Natural Cycle: 90
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 14.2	Intersection LOS: B	
Intersection Capacity Utilization 55.7%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Constitution Avenue & Akers Drive



Intersection												
Int Delay, s/veh	5.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	ሻ	- ↑		ሻ	<u></u>	7	ሻ	<u></u>	7
Traffic Vol, veh/h	0	0	110	64	0	8	117	134	7	7	133	0
Future Vol, veh/h	0	0	110	64	0	8	117	134	7	7	133	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	120	70	0	9	127	146	8	8	145	0
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	570	569	145	621	561	146	145	0	0	154	0	0
Stage 1	161	161	-	400	400	-	-	-	-	-	-	-
Stage 2	409	408	-	221	161	-		-	-	-	-	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_		4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-		_			_	_
Critical Hdwy Stg 2	6.12	5.52		6.12	5.52	_	-	_		-	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2 218	_		2.218	_	_
Pot Cap-1 Maneuver	450	441	902	413	446	960	1437	_		1446	_	_
Stage 1	841	765	-	648	609	-	-	_	_	-	_	_
Stage 2	640	604	-	781	765	-	-	-	-	_	-	-
Platoon blocked, %	1	1		1	1	1		_	_	1	_	_
Mov Cap-1 Maneuver	414	400	902	332	404	960	1437	-		1446	-	-
Mov Cap-2 Maneuver	414	400	-	332	404	-		-	-	-	_	-
Stage 1	767	760	-	591	556	-	-	-	-	-	-	-
Stage 2	578	551	-	674	760	-	-	-	-	-	-	-
g												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.6			17.6			3.5			0.4		
HCM LOS	Α.			C			3.0			J, i		
	, (
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	FBI n1	FBI n2	EBLn3V	VBI n1V	VBI n2	SBL	SBT	SBR
Capacity (veh/h)		1437	-	TVDIC			902	332	960	1446	- 201	JUIN
HCM Lane V/C Ratio		0.088	-	-	-	_	0.133				-	_
HCM Control Delay (s)		7.7	-	-	0	0	9.6	18.7	8.8	7.5	-	-
HCM Lane LOS		Α.	-	-	A	A	9.0 A	C	0.0 A	7.5 A	-	-
HCM 95th %tile Q(veh)	0.3	-	-	-	-	0.5	0.8	0	0	-	-
How but build Qiven	1	0.5					0.0	0.0		0		

Intersection													
Int Delay, s/veh	3.3												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ሻ		7	ሻ	†	7	ሻ		7	ሻ	↑	7	
Traffic Vol, veh/h	24	14	29	16	5	9	13	106	17	18	109	5	
Future Vol, veh/h	24	14	29	16	5	9	13	106	17	18	109	5	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	115	-	115	115	-	115	120	-	120	150	-	120	
Veh in Median Storage		0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	_	-	0	-	-	0	-	_	0	_	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	26	15	32	17	5	10	14	115	18	20	118	5	
IVIVIIIL I IOVV	20	- 13	- 32	17		10	17	113	10	20	110		
Major/Minor I	Minor2			Minor1			Major1		1	Major2			
Conflicting Flow All	318	319	118	327	306	115	123	0	0	133	0	0	
Stage 1	158	158	110	143	143	-	123	-	-	133	-	-	
Stage 2	160	161	-	184	163	-	-	-	-	-	_	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-		4.12		-	
•	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52		-			-	-		
Critical Hdwy Stg 2			2 210			2 210	2 210	-	-	2 210	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	680	626	934	670	637	*1001	1464	-	-	1474	-	-	
Stage 1	844	767	-	911	803	-	-	-	-	-	-	-	
Stage 2	891	788	-	818	763	-	-	-	-	-	-	-	
Platoon blocked, %	1	1	20.4	1	1	1	44/4	-	-	1	-	-	
Mov Cap-1 Maneuver	657	611	934	624	622	*1001	1464	-	-	1474	-	-	
Mov Cap-2 Maneuver	657	611	-	624	622	-	-	-	-	-	-	-	
Stage 1	836	756	-	902	795	-	-	-	-	-	-	-	
Stage 2	868	780	-	764	752	-	-	-	-	-	-	-	
Annraach	ED			MD			NID			CD			
Approach	EB			WB			NB			SB			
HCM Control Delay, s	10			10.2			0.7			1			
HCM LOS	В			В									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR			EBLn3V				SBL	SBT	SBR
Capacity (veh/h)		1464	-	-	657	611	934	624	622	1001	1474	-	-
HCM Lane V/C Ratio		0.01	-	-	0.04	0.025	0.034	0.028	0.009	0.01	0.013	-	-
HCM Control Delay (s)		7.5	-	-	10.7	11	9	10.9	10.8	8.6	7.5	-	-
HCM Lane LOS		Α	-	-	В	В	Α	В	В	Α	Α	-	-
HCM 95th %tile Q(veh))	0	-	-	0.1	0.1	0.1	0.1	0	0	0	-	-
Notes													
	o o city	¢. D.	Nov. ov.	annda 2	000	Com	nutatio	a Not D	ofinad	*. AII	major	/oluma	in plataan
~: Volume exceeds cap	vacity	\$: D6	elay exc	ceeds 3	UUS	+: Com	putation	ט זסעו נו	elinea	: All	major v	volume	in platoon

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Intersection						
Int Delay, s/veh	0.5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	0	7	0	^	^	*
Traffic Vol, veh/h	0	46	0	1247	2006	21
Future Vol, veh/h	0	46	0	1247	2006	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	50	0	1355	2180	23
, ,						
	/linor2		/lajor1		Major2	
Conflicting Flow All	-	1090	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	-	-	-	-
Pot Cap-1 Maneuver	0	181	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %	-			-	_	
Mov Cap-1 Maneuver	-	181	-	-	-	-
Mov Cap-2 Maneuver	-	-	_	_		_
Stage 1	_	-			_	
Stage 2	-	-	-	-	-	-
Staye 2			-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	32.3		0		0	
HCM LOS	D				-	
Minor Lane/Major Mvm	t	NBT E		SBT		
Capacity (veh/h)			181	-		
HCM Lane V/C Ratio		-	0.276	-		
HCM Control Delay (s)		-	32.3	-		
HCM Lane LOS		-	D	-		
HCM 95th %tile Q(veh)		-	1.1	-		

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Laterra etter						
Intersection	0.1					
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	†	7		<u></u>
Traffic Vol, veh/h	0	8	254	14	0	311
Future Vol, veh/h	0	8	254	14	0	311
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	120	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	9	276	15	0	338
IVIVIII(I IOW	U	,	210	13	U	330
Major/Minor M	linor1	N	/lajor1	N	Major2	
Conflicting Flow All	-	276	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	-	-	-	-
Pot Cap-1 Maneuver	0	874	-	-	0	-
Stage 1	0	-	-		0	-
Stage 2	0	_	_	_	0	_
Platoon blocked, %	- 0	1	_	_	- 0	_
Mov Cap-1 Maneuver		874	-	_	-	_
Mov Cap-1 Maneuver	-	074	-	-	-	-
	-	-	_	-	_	-
Stage 1		-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.2		0		0	
HCM LOS	Α					
	,,					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	874	-	
HCM Lane V/C Ratio		-	-	0.01	-	
HCM Control Delay (s)		-	-	9.2	-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	0	-	
HCM 95th %tile Q(veh)		-	-	0	-	

	۶	→	•	•	←	•	₹I	4	†	<i>></i>	>	ļ
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	^	7	٦	ተተተ	7		ሽኘ	ተተተ	7	1,1	ተተተ
Traffic Volume (vph)	241	1089	384	235	782	403	21	552	1641	141	349	919
Future Volume (vph)	241	1089	384	235	782	403	21	552	1641	141	349	919
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	0	3433	5085	1583	3433	5085
Flt Permitted	0.148			0.160				0.950			0.950	
Satd. Flow (perm)	276	5085	1583	298	5085	1583	0	3433	5085	1583	3433	5085
Satd. Flow (RTOR)			314			240				153		
Lane Group Flow (vph)	262	1184	417	255	850	438	0	623	1784	153	379	999
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	Prot	NA	Perm	Prot	NA
Protected Phases	3	8		7	4		1	1	6		5	2
Permitted Phases	8		8	4		4				6		
Detector Phase	3	8	8	7	4	4	1	1	6	6	5	2
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	11.0	11.0	9.0	11.0	11.0	9.0	9.0	11.0	11.0	9.0	11.0
Total Split (s)	18.0	34.0	34.0	16.0	32.0	32.0	30.0	30.0	52.0	52.0	18.0	40.0
Total Split (%)	15.0%	28.3%	28.3%	13.3%	26.7%	26.7%	25.0%	25.0%	43.3%	43.3%	15.0%	33.3%
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	5.0	5.0	3.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	7.0	7.0	5.0	7.0	7.0		5.0	7.0	7.0	5.0	7.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max
Act Effct Green (s)	42.0	27.0	27.0	38.0	25.0	25.0		24.3	45.0	45.0	13.0	33.7
Actuated g/C Ratio	0.35	0.22	0.22	0.32	0.21	0.21		0.20	0.38	0.38	0.11	0.28
v/c Ratio	1.02	1.03	0.70	1.11	0.80	0.84		0.90	0.94	0.22	1.02	0.70
Control Delay	93.1	68.6	9.8	123.5	51.9	36.3		63.3	46.6	4.7	105.0	41.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	93.1	68.6	9.8	123.5	51.9	36.3		63.3	46.6	4.7	105.0	41.9
LOS	F	Е	Α	F	D	D		Е	D	Α	F	D
Approach Delay		58.8			59.3				48.2			51.5
Approach LOS		Ε			Ε				D			D
Queue Length 50th (ft)	~151	~364	29	~170	231	155		242	482	0	~160	256
Queue Length 95th (ft)	m#309	#462	m98	#342	282	#333		#336	#587	43	#260	308
Internal Link Dist (ft)		1022			405				707			1957
Turn Bay Length (ft)	435		200	225		235		425		325	670	
Base Capacity (vph)	258	1144	599	229	1059	519		715	1906	689	371	1428
Starvation Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0	0	0	0	0
Reduced v/c Ratio	1.02	1.03	0.70	1.11	0.80	0.84		0.87	0.94	0.22	1.02	0.70

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 85 (71%), Referenced to phase 2:SBT and 6:NBT, Start of Green Natural Cycle: 110
Control Type: Actuated-Coordinated



Lana Craun	CDD
Lane Group	SBR
Late Configurations Traffic Volume (vph)	r
Future Volume (vph)	231
	1583
Satd. Flow (prot) Flt Permitted	1583
	1502
Satd. Flow (perm)	1583 224
Satd. Flow (RTOR) Lane Group Flow (vph)	251
Turn Type	Perm
Protected Phases	2
Permitted Phases	2 2
Detector Phase	2
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	11.0
Total Split (s)	40.0
Total Split (%)	33.3%
Yellow Time (s)	5.0
All-Red Time (s)	2.0
Lost Time Adjust (s)	0.0
Total Lost Time (s)	7.0
Lead/Lag	Lag
Lead-Lag Optimize?	Yes
Recall Mode	C-Max
Act Effct Green (s)	33.7
Actuated g/C Ratio	0.28
v/c Ratio	0.41
Control Delay	8.6
Queue Delay	0.0
Total Delay	8.6
LOS	A
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	16
Queue Length 95th (ft)	83
Internal Link Dist (ft)	
Turn Bay Length (ft)	265
Base Capacity (vph)	605
Starvation Cap Reductn	0
Spillback Cap Reductn	0
Storage Cap Reductn	0
Reduced v/c Ratio	0.41
Intersection Summary	
y	

PM Peak Hour - Year 2040

Maximum v/c Ratio: 1.11

Intersection Signal Delay: 53.8 Intersection LOS: D
Intersection Capacity Utilization 95.7% ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

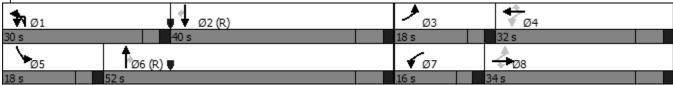
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Marksheffel Road & Constitution Avenue



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	PM	Peak	Hour -	Year	2040

	•	→	•	•	←	•	4	†	/	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, j	ተተተ	7	, j	ተተተ	7	1,1	f)		1,1	£	
Traffic Volume (vph)	270	1264	123	24	1380	190	128	51	172	356	33	153
Future Volume (vph)	270	1264	123	24	1380	190	128	51	172	356	33	153
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	1647	0	3433	1634	0
Flt Permitted	0.121			0.146			0.950			0.950		
Satd. Flow (perm)	225	5085	1583	272	5085	1583	3433	1647	0	3433	1634	0
Satd. Flow (RTOR)			134			207		37			91	
Lane Group Flow (vph)	293	1374	134	26	1500	207	139	242	0	387	202	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Split	NA		Split	NA	
Protected Phases		4		_	8	_	2	2		6	6	
Permitted Phases	4		4	8		8						
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	24.0	24.0	24.0	24.0	24.0	24.0		24.0	24.0	
Total Split (s)	72.0	72.0	72.0	72.0	72.0	72.0	24.0	24.0		24.0	24.0	
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	20.0%	20.0%		20.0%	20.0%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Lead/Lag Ontimize?												
Lead-Lag Optimize? Recall Mode	C May	C May	C May	C-Max	C May	C-Max	None	None		None	None	
Act Effct Green (s)	C-Max 67.7	C-Max 67.7	C-Max 67.7	67.7	C-Max 67.7	67.7	None 17.3	17.3		None 17.0	17.0	
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.56	0.56	0.14	0.14		0.14	0.14	
v/c Ratio	2.33	0.30	0.30	0.30	0.50	0.30	0.14	0.14		0.14	0.14	
Control Delay	636.7	16.6	2.5	18.9	18.9	5.9	47.2	77.5		62.8	36.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	636.7	16.6	2.5	18.9	18.9	5.9	47.2	77.5		62.8	36.9	
LOS	636.7	В	2.5 A	В	В	Α	D	77.5 E		02.0 E	D	
Approach Delay	<u>'</u>	116.4	,,		17.4	,,		66.4			53.9	
Approach LOS		F			В			E			D	
Queue Length 50th (ft)	~280	229	0	9	201	27	50	158		150	80	
Queue Length 95th (ft)	#459	269	28	m13	234	m40	81	#306		205	163	
Internal Link Dist (ft)	11 10 7	734	20	11110	1022	11110	01	276		200	237	
Turn Bay Length (ft)	275	701	235	235	1022	275	120	210		355	201	
Base Capacity (vph)	126	2869	951	153	2869	983	514	278		514	322	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	2.33	0.48	0.14	0.17	0.52	0.21	0.27	0.87		0.75	0.63	
Interception Commerce												

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

PM Peak Hour - Year 2040

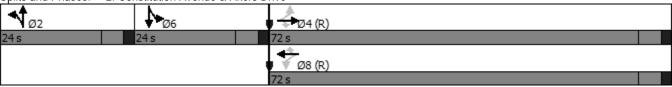
Maximum v/c Ratio: 2.33

Intersection Signal Delay: 65.9 Intersection LOS: E
Intersection Capacity Utilization 85.1% ICU Level of Service E

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
 - Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Constitution Avenue & Akers Drive



Intersection												
Int Delay, s/veh	11.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u></u>	7	ሻ	-		ሻ		7	ሻ	<u></u>	7
Traffic Vol, veh/h	3	0	378	41	0	5	305	143	20	20	115	0
Future Vol, veh/h	3	0	378	41	0	5	305	143	20	20	115	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	115	115	-	-	130	-	120	120	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	0	411	45	0	5	332	155	22	22	125	0
Major/Minor I	Minor2			Minor1		1	Major1			Major2		
Conflicting Flow All	1002	1010	125	1194	988	155	125	0	0	177	0	0
Stage 1	169	169	-	819	819	-	-	-	_	-	-	-
Stage 2	833	841	-	375	169	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	217	232	926	156	240	970	1462	-	-	1425	-	-
Stage 1	833	759	-	366	380	-	-	-	-	-	-	-
Stage 2	359	370	-	646	759	-	-	-	-	-	-	-
Platoon blocked, %	1	1		1	1	1		-	-	1	-	-
Mov Cap-1 Maneuver	176	177	926	71	183	970	1462	-	-	1425	-	-
Mov Cap-2 Maneuver	176	177	-	71	183	-	-	-	-	-	-	-
Stage 1	644	748	-	283	293	-	-	-	-	-	-	-
Stage 2	276	286	-	354	748	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.1			106.1			5.3			1.1		
HCM LOS	В			F			- 0.0					
Minor Lane/Major Mvm	nt	NBL	NBT	MRD	FRI n1	FRI n2 l	FRI n21	NBLn1V	MRI n2	SBL	SBT	SBR
	IL	1462			176		926	71	970	1425	<u> </u>	אמכ
Capacity (veh/h) HCM Lane V/C Ratio		0.227	-	-	0.019	-		0.628				-
		8.2	-		25.8	0	12	118	8.7	7.6	-	-
HCM Control Delay (s) HCM Lane LOS		8.2 A	-	-	25.8 D	A	12 B	118 F	8.7 A		-	-
HCM 95th %tile Q(veh))	0.9	-	-	0.1	- A	2.3	2.8	0	A 0	-	-
HOW FULL FORME Q(VEH)		0.9	-	-	0.1	-	2.3	2.0	U	U	-	-

Intersection															
Int Delay, s/veh	3.7														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR			
Lane Configurations	ሻ	<u></u>	7	ኘ	<u></u>	7	ኘ	<u></u>	7	ሻ	†	7			
Traffic Vol., veh/h	16	9	19	15	16	8	45	114	8	38	118	16			
uture Vol, veh/h	16	9	19	15	16	8	45	114	8	38	118	16			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free			
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None			
Storage Length	115	-	115	115	-	115	120	-	120	150	-	120			
eh in Median Storage	-, # -	0	-	-	0	-	-	0	-	-	0	-			
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-			
eak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92			
łeavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2			
/Ivmt Flow	17	10	21	16	17	9	49	124	9	41	128	17			
Major/Minor N	Minor2			Minor1			Major1			Major2					
Conflicting Flow All	450	441	128	456	449	124	145	0	0	133	0	0			
Stage 1	210	210	-	222	222	-	-	-	-	-	-	-			
Stage 2	240	231	-	234	227	-	-	-	-	-	-	-			
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-			
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-			
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-			
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-			
Pot Cap-1 Maneuver	547	528	922	543	522	989	1437	-	-	1474	-	-			
Stage 1	792	728	-	821	738	-	-	-	-	-	-	-			
Stage 2	801	731	-	769	716	-	-	-	-	-	-	-			
Platoon blocked, %	1	1		1	1	1		-	-	1	-	-			
Mov Cap-1 Maneuver	504	496	922	498	491	989	1437	-	-	1474	-	-			
Mov Cap-2 Maneuver	504	496	-	498	491	-	-	-	-	-	-	-			
Stage 1	765	708	-	793	713	-	-	-	-	-	-	-			
Stage 2	748	706	-	721	696	-	-	-	-	-	-	-			
Approach	EB			WB			NB			SB					
HCM Control Delay, s	10.9			11.8			2			1.7					
HCM LOS	В			В											
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR	EBLn1	EBLn2	EBLn3V	VBLn1V	VBLn2\	WBLn3	SBL	SBT	SBR		
Capacity (veh/h)		1437	-	-	504	496	922	498	491	989	1474	-			
HCM Lane V/C Ratio		0.034	_		0.035		0.022				0.028	_	-		
HCM Control Delay (s)		7.6	-	-		12.4	9	12.5	12.6	8.7	7.5	-	-		
HCM Lane LOS		Α.	_	-	В	В	Á	В	В	Α	Α.	_	-		
HCM 95th %tile Q(veh)		0.1	-	-	0.1	0.1	0.1	0.1	0.1	0	0.1	-	-		
									J.,						

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7		ተተተ	ተተተ	7
Traffic Vol, veh/h	0	56	0	2280	1470	25
Future Vol, veh/h	0	56	0	2280	1470	25
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Yield	-	None	-	Free
Storage Length	-	0	-	-	-	240
Veh in Median Storage,	, # 0		-	0	0	
Grade, %	0	-	-	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	61	0	2478	1598	27
IVIVIIILI IUW	- 0	01	U	2470	1370	ZI
Major/Minor N	/linor2		Najor1	N	Major2	
Conflicting Flow All	-	799	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	_	-	-	_
Pot Cap-1 Maneuver	0	282	0	-	-	0
Stage 1	0	-	0		-	0
Stage 2	0	_	0		_	0
Platoon blocked, %	U		U	_		U
Mov Cap-1 Maneuver	-	282			-	_
			-	_		
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	21.2		0		0	
HCM LOS	C		U		0	
TIGIVI LOS	C					
Minor Lane/Major Mvm	t	NBT E	EBLn1	SBT		
Capacity (veh/h)		-	282	-		
HCM Lane V/C Ratio		-	0.216	-		
HCM Control Delay (s)		-	21.2	-		
HCM Lane LOS		-	С	-		
HCM 95th %tile Q(veh)		-	0.8	-		
2 · 2 · · · · · · · · · · · · · ·			3.0			

Intersection						
Int Delay, s/veh	0					
		MDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	0	7	470	آم 11	0	†
Traffic Vol, veh/h Future Vol, veh/h	0	5 5	470 470	41 41	0	542 542
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None		None	-	
Storage Length	-	0	_	120	-	-
Veh in Median Storag	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	511	45	0	589
Major/Minor	Minor1		Major1	N	/lajor2	
Conflicting Flow All	-	511	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	-	-	-	-
Pot Cap-1 Maneuver	0	*693	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		*/02	-	-		-
Mov Cap-1 Maneuver		*693	-	-	-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		0	
HCM LOS	В					
Minor Lane/Major Mvi	mt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-		693	-	
HCM Lane V/C Ratio		-	_	0.008	-	
HCM Control Delay (s	5)	-	-	10.2	-	
HCM Lane LOS	•	-	-	В	-	
HCM 95th %tile Q(vel	n)	-	-	0	-	
Notes						
	nnacity	¢. Da	Nav ovo	onds 20)0c	L. Com
~: Volume exceeds ca	apacity	\$: De	eiay exc	eeds 30	JUS	+: Com

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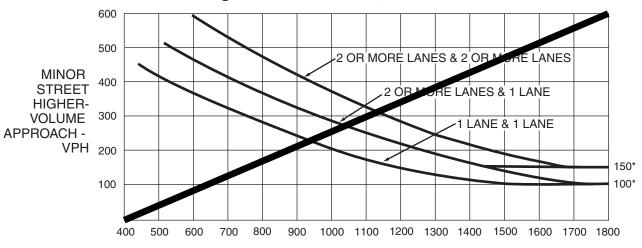
APPENDIX D

Warrant Analysis Forms

Project: WATERMARK AKERS DRIVE Background Traffic - Year 2040

Constitution Avenue & Akers Drive

Figure 4C-3. Warrant 3, Peak Hour

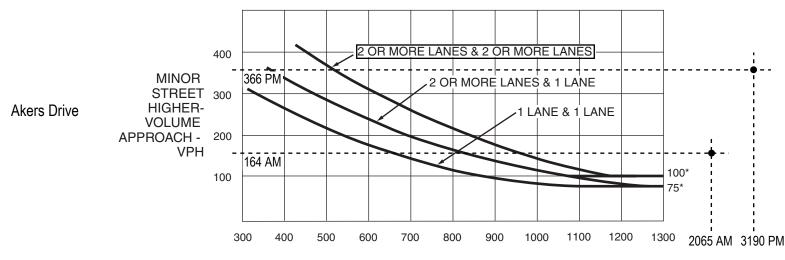


MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

Constitution Avenue = 50 MPH

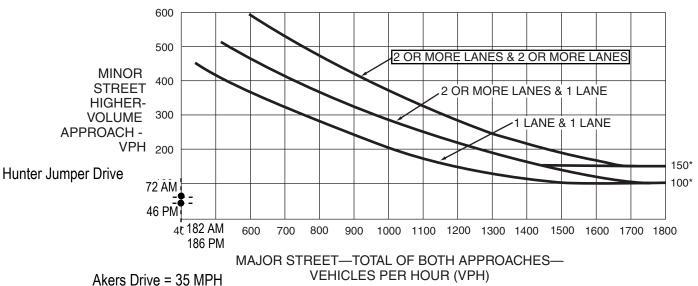
December 2009 Sect. 4C.04

Project: WATERMARK AKERS DRIVE

Total Traffic - Year 2022

Hunter Jumper Drive & Akers Drive

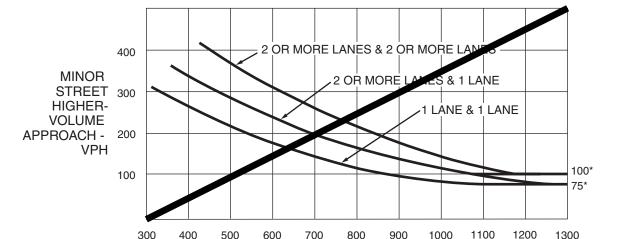
Figure 4C-3. Warrant 3, Peak Hour



VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

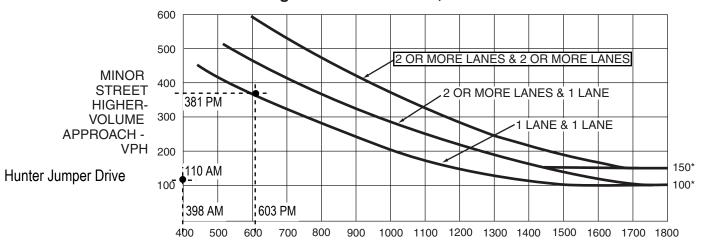
December 2009 Sect. 4C.04 Akers Drive = 35 MPH

Project: WATERMARK AKERS DRIVE

Total Traffic - Year 2040

Hunter Jumper Drive & Akers Drive

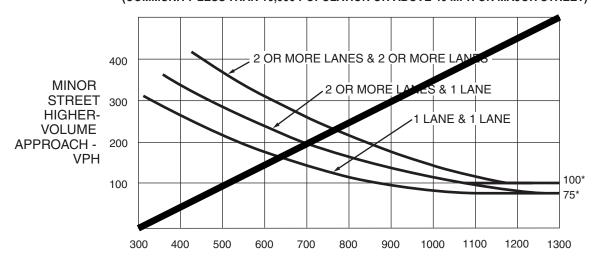
Figure 4C-3. Warrant 3, Peak Hour



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

December 2009 Sect. 4C.04