## MEMORANDUM

| TO： | Watermark Residential <br> 111 Monument Circle，Suite 1600 <br> Indianapolis，Indiana 46204 |
| :--- | :--- |
| FROM： | Adam Maxwell，PE，PTOE <br> Brandon Wilson <br> SM ROCHA，LLC |
| DATE： | May 24，2021 |
| SUBJECT： | Watermark Akers Drive－Transportation Memorandum |

This memorandum has been revised to address County review comments dated 05／14／2021．

## Analysis Objective

As requested by El Paso County，this memorandum and attached information is provided to summarize our additional analysis and discussion regarding：
－County＇s site distance and access spacing requirements along Akers Drive with respect to two proposed site access drive locations．
－Traffic operations at the Hunter Jumper Drive intersection with Akers Drive，if the northern access is the only public access to the development．It is understood that with this scenario the secondary access to the south would be emergency access only and gated．
－Cost sharing information for the future eastbound left turn lane improvements at the Akers Drive／Constitution Avenue intersection．

## Site Description

The proposed development is understood to entail the new construction of 300 multifamily dwelling units accommodated within multiple three－story buildings and supported by various property amenities including a clubhouse and swimming pool．

[^0]The approved Watermark Akers Drive traffic impact study ${ }^{1}$ (TIS provided in Appendix B) outlines that proposed access to the development is provided at the following locations: one full-movement access onto Akers Drive, serving as an extension of Hunter Jumper Drive east of Akers Drive (referred to as Access A), and one right-in/right-out access onto Akers Drive (referred to as Access B). Access B is approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive, centerline to centerline.

## Site Distance and Access Spacing

Access spacing requirements along Akers Drive are to be based on the County's Engineering Criteria Manual (ECM).

An evaluation of access spacing requirements, pursuant to Section 2.4.1 and Tables 2-34 through 2-36 of the County's ECM, reveals that minimum access spacing along Akers Drive shall be equal to the entering site distance for a posted speed limit of 35 MPH , or 350 feet. Considering the uphill grade that exists along Akers Drive, this minimum length may be adjusted pursuant to adjustment factors in Table 2-34 of the County's ECM.

The above standards were then applied to the two proposed site plan access locations.

- Access A (Hunter Jumper Drive extension)
- Per Table 2-35 of the County's ECM, access spacing standard met for 35 MPH posted speed limit and without adjustment factors.
- Access B (right-in/right-out)
- Constitution Avenue and Akers Drive intersection is visible from proposed right-in/right-out access.
- Considering average vehicle turning speeds and the uphill grade along Akers Drive, motorists turning from Constitution Avenue onto northbound Akers Drive are expected to be traveling slower than the posted speed of 35 MPH . Per Table 2-35 of the County's ECM, access spacing standard of 250 feet can be met for 25 MPH travel speed conditions without considering grade adjustment factors.

Considering the items mentioned above, it is believed that adequate access spacing is provided along Akers Drive.

Proposed site access spacing is shown in Figure 1.

[^1]

## Additional Analysis - Traffic Operations with One Site Access

At the request of County Staff, the analysis presented in this memorandum considers the Akers Drive intersection with Access A (extension of Hunter Jumper Drive) as the only public access for the proposed development. The proposed southern right-in/right-out access will be considered as a gated emergency-only access and not considered within this analysis.

## Trip Generation

Trip generation information from the approved traffic study is shown in Table 1 below.
Table 1 - Trip Generation Summary

| $\begin{gathered} \text { ITE } \\ \text { CODE } \end{gathered}$ | LAND USE | SIZE | TOTAL TRIPS GENERATED |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| 221 | Multifamily Housing (Mid-Rise) | 300 DU | 1,632 | 28 | 80 | 108 | 81 | 51 | 132 |
|  |  | Total: | 1,632 | 28 | 80 | 108 | 81 | 51 | 132 |

Note: All data and calculations above are subject to being rounded to nearest value.

Upon build-out, Table 1 illustrates that the proposed development has the potential to generate approximately 1,632 daily trips with 108 of those occurring during the morning peak hour and 132 during the afternoon peak hour.

## Trip Distribution and Re-Assignment

The overall directional distribution of site-generated traffic was based on the distribution presented in the approved Watermark Akers Drive TIS and re-assigned to utilize one site access at the Hunter Jumper Drive intersection.

Applying re-evaluated trip distribution patterns to site-generated traffic, re-assignment of site traffic is shown on Figure 2.

Site-generated traffic was then added to Year 2040 background traffic in order to develop total traffic projections. Projected Year 2040 total traffic volumes and intersection geometry are shown in Figure 2.



WATERMARK AKERS DRIVE
Memorandum
Figure 2

## Peak Hour Intersection Levels of Service

The analyses and procedures described in this memorandum were performed in accordance with the Highway Capacity Manual (HCM) and are based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, the study intersections are likely to operate with traffic conditions better than those described within this memorandum, which represent the peak hours of weekday operations only.

Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers.

The operations of the study intersection were analyzed under projected total traffic conditions using the SYNCHRO computer program. Analysis included heavy vehicle percentages as shown in the peak hour traffic count data for the intersection. AM peak hour data shows $9.5 \%$ and $22.1 \%$ heavy vehicles for the northbound and southbound through movements, respectively. PM peak hour data shows $26.2 \%$ and $1.4 \%$ heavy vehicles for the northbound and southbound through movements, respectively.

Year 2040 total traffic level of service analysis results are summarized in Table 2. Intersection capacity worksheets are provided in Appendix A.

Table 2 - Intersection Capacity Analysis Summary - Total Traffic - Year 2040

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) |  |  |
| Eastbound Left | A | D |
| Eastbound Through | A | A |
| Eastbound Right | A | B |
| Westbound Left | C | F |
| Westbound Through and Right | A | A |
| Northbound Left | A | A |
| Southbound Left | A | A |

[^2]
## Total Traffic Analysis Results Upon Development Build-Out

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at or better than LOS C during the morning peak traffic hour and LOS D or better during the afternoon peak traffic hour. The exception includes the westbound left turning movement which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

Compared to the Year 2040 total traffic intersection capacity analysis summary within the approved Watermark Akers Drive TIS, removal or restriction of the southern access is not expected to significantly change the operations of the Hunter Jumper Drive and Akers Drive intersection. Heavy vehicles were considered in the operational analysis are not shown to have a negative impact at the northern site access location

In review of the site plan for the propose development, it is not expected that many of the vehicle trips exiting at the right-in/right-out access would be returning to Constitution Avenue. Motorists desiring to access Constitution Avenue are anticipated to exit the site at the Hunter Jumper Drive extension and turn southbound onto Akers Drive to access Constitution Avenue.

It is not recommended that development access onto Akers Drive be more limited than that proposed with two locations.

## Turn Lane Improvements, Estimated Construction Costs and Contribution

Referencing Table 8 from the approved Watermark Akers Drive TIS, the existing eastbound left turn lane along Constitution Avenue at Akers Drive is recommended to be extended when $95^{\text {th }}$ percentile queuing exceeds the existing turn lane length.

Vehicle storage for the eastbound left turn lane can be increased by extending the left turn bay more to the west. Based upon available aerial imagery, approximately 135 additional feet of turn bay could be constructed while maintaining the minimum median width required for the roadway section.

Estimated construction costs for removal of a portion of the existing roadway median and construction of the eastbound turn lane extension are shown below. Cost information is taken directly from the Financial Assurance Estimate (FAE).

- Traffic Control $=\$ 5,000.00$
- Demolition $=\$ 4,500.00$
- Asphalt Patching/Pavement $=\$ 7,428.69$
- Concrete Curb and Gutter $=\$ 8,525.00$
- Striping $=\$ 1,000.00$
- As-Built Plans $=\$ 1,500.00$
- Pond/BMP Certification $=\$ 1,500.00$

Total $=\$ 29,453.69$
Pursuant to the County review comments, it is understood that County Staff will verify if escrow will be required.

## Analysis Conclusion

This memorandum assessed site distance and access spacing requirements along Akers Drive with proposed site accesses, provided additional analysis to evaluate intersection operations at the Akers Drive and Hunter Jumper Drive intersection considering changes to proposed site access drives, and considered potential impacts to the adjacent roadway network.

When considering the site sensitive traffic conditions presented in this memorandum, it is our professional opinion that the proposed site access drives meet access spacing requirements defined with the County's ECM. Analysis of future traffic conditions indicates that the conditions assumed are expected to create no negative impact to traffic operations at the study intersection. Conclusions and recommendations as presented in the approved Watermark Akers Drive TIS remain valid.

We trust that our findings will assist in the planning and approval of traffic items addressed in this memorandum related to the Watermark Akers Drive development. Please contact us should further assistance be needed.

Sincerely,
Adam Maxwell, PE, PTOE
Senior Traffic Engineer
SM ROCHA, LLC
Traffic and Transportation Consultants


## APPENDIX A

Capacity Worksheets

The following information can be found in the Highway Capacity Manual, Transportation Research Board, 2016: Chapter 19 - Signalized Intersections and Chapter 20 - Two-Way Stop Controlled Intersections.

## Automobile Level of Service (LOS) for Signalized Intersections

Levels of service are defined to represent reasonable ranges in control delay.

## LOS A

Describes operations with a control delay of $10 \mathrm{~s} / \mathrm{veh}$ or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

## LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

## LOS C

Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

## LOS D

Describes operations with control delay between 35 and $55 \mathrm{~s} / \mathrm{veh}$ and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

## LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F
Describes operations with control delay exceeding $80 \mathrm{~s} / \mathrm{veh}$ or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

## Level of Service (LOS) for Unsignalized TWSC Intersections

| Level of Service (v/c $\leq 1.0)$ | Average Control Delay (s/veh) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| D | $>25-35$ |
| E | $>35-50$ |
| F | $>50$ |






| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1 EBL2 | EBLn3WBLn1WBLn2 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1462 | - | -169 | - | 926 | 71 | 975 | 1374 | - | - |
| HCM Lane V/C Ratio | 0.227 | - | -0.019 | -0.444 | 0.628 | 0.011 | 0.016 | - | - |  |
| HCM Control Delay (s) | 8.2 | - | - | 26.7 | 0 | 12 | 118 | 8.7 | 7.7 | - |
| HCM Lane LOS | A | - | - | D | A | B | F | A | A | - |
| HCM 95th \%tile Q(veh) | 0.9 | - | - | 0.1 | - | 2.3 | 2.8 | 0 | 0 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

## APPENDIX B

## Approved Watermark Akers Drive Traffic Impact Study

# TRAFFIC IMPACT STUDY 

For<br>Watermark Akers Drive Colorado Springs, Colorado

December 2020
Revised:
February 2021

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## I. Introduction

## Project Overview

This traffic impact study addresses the capacity, geometric, and control requirements associated with the development entitled Watermark Akers Drive.

This traffic impact study has been revised to address El Paso County review comments dated 02/08/2021 regarding various review comments throughout.

This proposed residential development consists of various multifamily residential buildings. The development is located on the northwest corner of the Constitution Avenue with Marksheffel Road intersection in Colorado Springs, Colorado.

## Study Area Boundaries

The study area to be examined in this analysis encompasses the Akers Drive intersections with Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, as well as the Marksheffel Road intersections with Constitution Avenue and Electronic Drive, and intersections with proposed site accesses.

Figure 1 illustrates location of the site and study intersections.

## Site Description

Land for the development is currently vacant and surrounded by a mix of open space, residential, light industrial, and commercial land uses. It is understood land for the development is currently within the jurisdiction of El Paso County.

The proposed development is understood to entail the new construction of 300 multifamily dwelling units accommodated within multiple three-story buildings and supported by various property amenities including a clubhouse and swimming pool.

Proposed access to the development is provided at the following locations: one full-movement access onto Akers Drive, serving as an extension of Hunter Jumper Drive east of Akers Drive (referred to as Access A), and one right-in/right-out access onto Akers Drive (referred to as Access B). Access B is approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive.

For purposes of this study, it is anticipated that development construction would be completed by end of Year 2022. A conceptual site plan, as prepared by Watermark Residential, is shown on Figure 2. This plan is provided for illustrative purposes.



## Existing and Committed Surface Transportation Network

Within the study area, Constitution Avenue and Marksheffel Road are the primary roadways that will accommodate traffic to and from the proposed development. Secondary roadways include Akers Drive, Electronic Drive, and Hunter Jumper Drive. A brief description of each roadway, based on the City's major transportation thoroughfare plan ${ }^{1}$ and design standards², and the County's 2016 Major Transportation Corridors Plan (MTCP) ${ }^{3}$ and Engineering Criteria Manual (ECM) ${ }^{4}$, is provided below:

Marksheffel Road is a north-south principal arterial roadway having four through lanes (two lanes in each direction) with exclusive turn lanes at the intersections within the study area. Marksheffel Road provides a posted speed limit of 50 MPH.

Constitution Avenue is an east-west principal arterial roadway having a varying number of through lanes (two to three lanes in each direction) with exclusive turn lanes at the intersections within the study area. Constitution Avenue provides a posted speed limit of 50 MPH.

Akers Drive is a north-south roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. Akers Drive provides a posted speed limit of 35 MPH . Akers Drive is unclassified in both the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards, the roadway's estimated ROW width, and connection to Constitution Avenue, Akers Drive is assumed to be classified as a collector roadway within Colorado Springs. Pursuant to Section 2.2.4 of the County's ECM, Akers Drive is assumed to be classified as an urban nonresidential collector roadway within El Paso County.

Hunter Jumper Drive is an east-west roadway having two through lanes (one lane in each direction) with exclusive turn lanes at the intersection within the study area. Hunter Jumper Drive is unclassified in the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards and the roadway's estimated ROW width, Hunter Jumper Drive is assumed to be classified as a collector roadway within Colorado Springs with a posted speed limit of 30 MPH . Pursuant to Section 2.2.4 of the County's ECM, Hunter Jumper Drive is assumed to be classified as an urban residential collector roadway within El Paso County with a posted speed limit of 35 MPH .

[^3]Electronic Drive is an east-west roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. Electronic Drive provides a posted speed limit of 30 MPH . Electronic Drive is unclassified in the City's major transportation thoroughfare plan and the County's MTCP. However, per Sections 15.0 and 16.0 of the City's design standards, the roadway's estimated ROW width, connection to Marksheffel Road, and service to various industrial land uses, Electronic Drive is assumed to be classified as an industrial roadway within Colorado Springs. Pursuant to Section 2.2.4 of the County's ECM, Electronic Drive is assumed to be classified as a rural local roadway within El Paso County.

The study intersection of Marksheffel Road with Constitution Avenue is signalized. All other study intersections operate under a stop-controlled condition. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

Comparison of existing roadway cross-sections of Marksheffel Road and Constitution Avenue to the City's design standards concludes that both roadways are not built to their ultimate width for accommodation of future regional transportation demands. Both roadways are envisioned to become six-lane roadways (three through lanes in each direction).

## II. Existing Traffic Conditions

Morning (AM) and afternoon (PM) peak hour traffic counts were collected at the Akers Drive intersections of Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, and at the Marksheffel Road intersections of Constitution Avenue and Electronic Drive. Average daily (24-hour) traffic volumes were collected on Marksheffel Road, Constitution Avenue, and Akers Drive. These counts are shown on Figure 3.

It should be noted that due to the effects of the COVID-19 pandemic, traffic volumes collected may not accurately represent peak hour and 24 -hour traffic volumes under normal conditions. Therefore, in order to more accurately represent existing traffic volumes under normal conditions, average daily traffic volumes along Marksheffel Road, provided from the City's GIS web mapping application ${ }^{5}$, were referenced and grown at a two percent annual growth rate to Year 2020. Comparing the calculated 24-hour volume to the collected count data concludes that the collected count data represents higher traffic volumes. Therefore, collected count data is considered to adequately represent traffic volumes under normal conditions. These counts are shown on Figure 3.

It should also be noted that a significant number of U-turn vehicles are present at the intersection of Constitution Avenue with Marksheffel Road. It is observed that these U-turns utilize the existing northbound left turn lane. These U-turn movements were individually analyzed within this analysis and are shown separately in Figure 3.

Traffic count data is included for reference in Appendix A.
Existing signal timing parameters for the intersection of Marksheffel Road and Constitution Avenue were obtained from City Staff and used throughout this study to the best extent possible in order to remain consistent with existing signal coordination plans. City signal timing information received is included for reference in Appendix A.

[^4]

Figure 3
EXISTING TRAFFIC
Volumes \& Intersection Geometry
AM / PM Peak Hour
WATERMARK AKERS DRIVE
Traffic Impact Study
Average Daily Traffic
February 2021
Traffic and Transportation Consultants
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The Signalized and Unsignalized Intersection Analysis techniques, as published in the Highway Capacity Manual (HCM) by the Transportation Research Board and as incorporated into the SYNCHRO computer program, were used to analyze the study intersections for existing traffic conditions. These nationally accepted techniques allow for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from " $A$ " which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers. These levels of service grades with brief descriptions of the operating condition, for unsignalized and signalized intersections, are included for reference in Appendix B and have been used throughout this study.

The level of service analyses results for existing conditions are summarized in Table 1.
Intersection capacity worksheets developed for this study are provided in Appendix C.

Table 1 - Intersection Capacity Analysis Summary - Existing Traffic

| INTERSECTION <br> LANE GROUPS | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Constitution Avenue / Marksheffel Road (Signalized) | $\mathrm{C}(30.7)$ | $\mathrm{D}(41.7)$ |
| Constitution Avenue / Akers Drive (Stop-Controlled) |  |  |
| Eastbound Left | A | A |
| Southbound Left | B | B |
| Southbound Right | A | A |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) | A | A |
| Eastbound Left | A | A |
| Eastbound Right | A | A |
| Northbound Left | A | A |
| Electronic Drive / Akers Drive (Stop-Controlled) | A | A |
| Westbound Left and Right | C | B |
| Southbound Left |  |  |
| Electronic Drive / Marksheffel Road (Stop-Controlled) |  |  |
| Eastbound Right |  |  |

[^5]
## Existing Traffic Analysis Results

Under existing conditions, operational analysis shows that the signalized intersection of Constitution Avenue with Marksheffel Road has overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The unsignalized intersection of Constitution Avenue with Akers Drive has turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road has turn movement operations at LOS C during the morning peak traffic hour and LOS B during the afternoon peak traffic hour.

## III. Future Traffic Conditions Without Proposed Development

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by development of vacant parcels in the area.

To account for projected increases in background traffic for Years 2022 and 2040, a compounded annual growth rate of approximately two percent was applied to existing traffic volumes. This annual growth rate is consistent with regional growth projections and the level of in-fill development expected within the area.

To account for projected traffic from adjacent developments not yet built, trip generations from The Sands traffic impact study ${ }^{6}$ were added to Year 2022 background traffic volumes, while Filings 5-8 and Phases 9-10 from the Hannah Ridge at Feathergrass traffic impact studies ${ }^{7,8}$ were added to Year 2040 background traffic volumes. It should be noted that the Hannah Ridge at Feathergrass traffic impact study, dated September 2017, originally assumed various retail land uses for the same development area currently proposed with this project, and included a right-in/right-out access into the site. This access is no longer proposed with this development, therefore ingress and egress traffic volumes originally anticipated to utilize the right-in/right-out access were not added to background traffic volumes.

A signal warrant analysis, using 2040 background traffic volumes and upon the assumed extension of Akers Drive south of Constitution Avenue, was conducted for the Akers Drive intersection with Constitution Avenue in order to review potential for traffic signal control. Analysis results conclude that the intersection was found to be above the minimum vehicle volumes required to meet Warrant 3 Peak Hour, from the Manual on Uniform Traffic Control Devices (MUTCD), for the installation of a traffic signal. As such, and consistent with assumptions defined within the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study, the intersection was analyzed under traffic signal control by Year 2040. Warrant study worksheets are provided for reference in Appendix D.

Warrant 3 is intended for use at locations where traffic conditions are such that for a minimum of one hour on an average day, the minor-street (Akers Drive) traffic suffers undue delay when entering or crossing the major street (Constitution Avenue). This assumption provides for a conservative analysis. Said intersection should be monitored further by County Staff as area development occurs to determine when signalization installation is appropriate.

Pursuant to the non-committed area roadway improvements discussed in Section I, Year 2022 background traffic conditions assumes no roadway improvements to accommodate regional transportation demands.

[^6]Year 2040 background traffic conditions assume Marksheffel Road and Constitution Avenue will be built out to their ultimate widths to accommodate regional transportation demands. This is consistent with assumptions defined within The Sands traffic impact study. Additionally, pursuant to the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study, it is assumed Akers Drive is planned to extend south of Constitution Avenue in order to serve future commercial land uses within Phase 10 of the Hannah Ridge at Feathergrass development. Similarly, it is assumed Electronic Drive will extend west of Akers Drive to provide access to the future residential land uses within Phases 5 through 8 of said future development. Year 2040 also assumes existing signal timing parameters for the Marksheffel Road and Constitution Avenue intersection with optimized intersection splits in effort to better long-term intersection performance. This assumption provides for a conservative analysis.

Projected background traffic volumes and intersection geometry for Years 2022 and 2040 are shown on Figure 4 and Figure 5, respectively.


Figure 4
BACKGROUND TRAFFIC - YEAR 2022
Volumes \& Intersection Geometry
AM / PM Peak Hour
WATERMARK AKERS DRIVE
Traffic Impact Study


Development Site

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Figure 5
BACKGROUND TRAFFIC - YEAR 2040
Volumes \& Intersection Geometry
AM / PM Peak Hour
WATERMARK AKERS DRIVE
Traffic Impact Study
(ADT) : Average Daily Traffic
February 2021
Traffic and Transportation Consultants

As with existing traffic conditions, the operations of study intersections were analyzed under background conditions, without the proposed development, using the SYNCHRO computer program.

Background traffic level of service analysis results for Year 2022 are listed in Table 2. Year 2040 operational results are summarized in Table 3.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 2 - Intersection Capacity Analysis Summary - Background Traffic - Year 2022

| INTERSECTION <br> LANE GROUPS | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Constitution Avenue / Marksheffel Road (Signalized) | $\mathrm{C} \mathrm{(33.1)}$ | $\mathrm{D}(44.9)$ |
| Constitution Avenue / Akers Drive (Stop-Controlled) |  |  |
| Eastbound Left | A | A |
| Southbound Left | B | B |
| Southbound Right | A | A |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) | A | A |
| Eastbound Left | A | A |
| Eastbound Right | A | A |
| Northbound Left | A | A |
| Electronic Drive / Akers Drive (Stop-Controlled) | A | A |
| Westbound Left and Right | C | C |
| Southbound Left |  |  |
| Electronic Drive / Marksheffel Road (Stop-Controlled) |  |  |
| Eastbound Right |  |  |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Lev el of Service

## Background Traffic Analysis Results - Year 2022

Year 2022 background traffic analysis indicates that the signalized intersection of Constitution Avenue with Marksheffel Road experiences overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The unsignalized intersection of Constitution Avenue with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive shows turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS C during both morning and afternoon peak traffic hours.

Table 3 - Intersection Capacity Analysis Summary - Background Traffic - Year 2040

| INTERSECTION <br> LANE GROUPS | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Constitution Avenue / Marksheffel Road (Signalized) | $\mathrm{D}(39.4)$ | $\mathrm{E}(56.8)$ |
| Constitution Avenue / Akers Drive (Signalized) | $\mathrm{B}(13.6)$ | $\mathrm{D}(54.3)$ |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) |  |  |
| Eastbound Left | A | C |
| Eastbound Right | A | B |
| Northbound Left | A | A |
| Electronic Drive / Akers Drive (Stop-Controlled) | B | B |
| Eastbound Left | B | B |
| Eastbound Through | A | A |
| Eastbound Right | B | B |
| Westbound Left | B | B |
| Westbound Through | A | A |
| Westbound Right | A | A |
| Northbound Left | A | A |
| Southbound Left |  |  |
| Electronic Drive / Marksheffel Road (Stop-Controlled) | D | C |
| Eastbound Right |  |  |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Lev el of Serv ice

## Background Traffic Analysis Results - Year 2040

By Year 2040 and without the proposed development, the study intersection of Constitution Avenue with Marksheffel Road projects overall operations at LOS D during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during the afternoon peak traffic period is attributed to eastbound and westbound through volumes, as well as left turning movements in all directions.

The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS D during the afternoon peak traffic hour.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at LOS A during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour.

The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

## IV. Proposed Project Traffic

## Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, 10 th Edition, were applied to the proposed land use in order to estimate average daily traffic (ADT), AM Peak Hour, and PM Peak Hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from a point of origin to a point of destination.

The ITE land use code 221 (Multifamily (Mid-Rise)) was used for estimating trip generation because of its best fit to the proposed land use description.

Trip generation rates used in this study are presented in Table 4.
Table 4 - Trip Generation Rates

| $\begin{array}{\|c\|} \hline \text { ITE } \\ \text { CODE } \end{array}$ | LAND USE | UNIT | TRIP GENERATION RATES |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
|  | Multifamily Housing (Mid-Rise) | DU | 5.44 | 0.09 | 0.27 | 0.36 | 0.27 | 0.17 | 0.44 |

Key: $\quad$ DU $=$ Dw elling Units.
Note: All data and calculations above are subject to being rounded to nearest value.

Table 5 illustrates projected average daily traffic (ADT), AM Peak Hour, and PM Peak Hour traffic volumes likely generated by the proposed development upon build-out.

Table 5 - Trip Generation Summary

| $\begin{gathered} \text { ITE } \\ \text { CODE } \end{gathered}$ | LAND USE | SIZE | TOTAL TRIPS GENERATED |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| 221 | Multifamily Housing (Mid-Rise) | 300 DU | 1,632 | 28 | 80 | 108 | 81 | 51 | 132 |
|  |  | Total: | 1,632 | 28 | 80 | 108 | 81 | 51 | 132 |

Note: All data and calculations above are subject to being rounded to nearest value.

Upon build-out, Table 5 illustrates that the proposed development has the potential to generate approximately 1,632 daily trips with 108 of those occurring during the morning peak hour and 132 during the afternoon peak hour.

## Adjustments to Trip Generation Rates

A development of this type is not likely to attract trips from within area land uses nor pass-by or diverted link trips from the adjacent roadway system, therefore no trip reduction was taken in this analysis.

## Trip Distribution

The overall directional distribution of site-generated traffic was determined based on the location of development site within the City and County, proposed and existing area land uses, allowed turning movements, available roadway network, and in reference to the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic impact study.

Phase 10 of the Hannah Ridge at Feathergrass Filing Nos. 3 and 4 traffic study anticipated sitegenerated traffic to travel north and south through the Constitution Avenue and Akers Drive intersection. For purposes of this analysis, consideration of site-generated traffic volumes accessing the assumed future retail south of Constitution Avenue is not considered. Moreover, northbound and southbound site-generated trips along Akers Drive across Constitution Avenue could be considered as internal capture within the overall Hannah Ridge at Feathergrass development. Therefore, sitegenerated traffic volumes are assumed to travel to and from the greater area beyond the overall Hannah Ridge at Feathergrass development area.

Overall trip distribution patterns for the development are shown on Figure 6.

## Trip Assignment

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying trip distribution patterns to site-generated traffic provides the overall site-generated trip assignments shown on Figure 6.


Figure 6
SITE DEVELOPMENT DISTRIBUTION
(\%) : Overall
SITE-GENERATED
WATERMARK AKERS DRIVE
AM / PM Peak Hour
Traffic Impact Study

## V. Future Traffic Conditions With Proposed Developments

Site-generated traffic was added to background traffic projections for Years 2022 and 2040 to develop total traffic projections. For analysis purposes, it was assumed that development construction would be completed by end of Year 2022.

Pursuant to area roadway improvement discussions provided in Section III, Year 2022 and Year 2040 total traffic conditions assume no additional roadway improvements to accommodate regional transportation demands. However, 20 feet of right-of-way will be dedicated along the north side of Constitution Avenue as part of this development. Additional roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

A preliminary site distance evaluation, pursuant to Section 2.4, Tables 2-34, 2-35, and 2-36 of the County's ECM, was evaluated for Access B. As mentioned in Section I, Access B is located approximately 300 feet north of Constitution Avenue and 375 feet south of Hunter Jumper Drive. Considering the multifamily land use being proposed and the approximate grade and posted speed limit along Akers Drive, preliminary evaluation indicates that access spacing requirements are satisfied for Access B.

A signal warrant analysis, using 2022 and 2040 total traffic volumes, was conducted for the Akers Drive with Hunter Jumper Drive intersection to review potential for traffic signal control. Analysis results conclude that the intersection does not have the minimum volume required to meet Warrant 3 - Peak Hour, from the MUTCD, for the installation of a traffic signal. As such, the intersection remained a stopcontrolled condition. Warrant study worksheets are provided for reference in Appendix D.

Projected Year 2022 total traffic volumes and intersection geometry are shown in Figure 7.
Figure 8 shows projected total traffic volumes and intersection geometry for Year 2040.


Figure 7

WATERMARK AKERS DRIVE
Traffic Impact Study

TOTAL TRAFFIC - YEAR 2022 Volumes \& Intersection Geometry

AM / PM Peak Hour
eometry
(ADT) : Average Daily Traffic


Figure 8
TOTAL TRAFFIC - YEAR 2040 Volumes \& Intersection Geometry

AM / PM Peak Hour
WATERMARK AKERS DRIVE
Traffic Impact Study
(ADT) : Average Daily Traffic
February 2021
Traffic and Transportation Consultants

## VI. Project Impacts

The analyses and procedures described in this study were performed in accordance with the Highway Capacity Manual (HCM) and are based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this study, which represent the peak hours of weekday operations only.

## Peak Hour Intersection Levels of Service

As with background traffic, the operations of the study intersections were analyzed under projected total traffic conditions using the SYNCHRO computer program. Total traffic level of service analysis results for Years 2022 and 2040 are summarized in Table 6 and Table 7, respectively.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 6 - Intersection Capacity Analysis Summary - Total Traffic - Year 2022

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Constitution Avenue / Marksheffel Road (Signalized) | C (33.5) | D (45.2) |
| Constitution Avenue / Akers Drive (Stop-Controlled) |  |  |
| Eastbound Left | A | A |
| Southbound Left | B | B |
| Southbound Right | A | A |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) | A | B |
| Eastbound Left | A | A |
| Eastbound Through | A | A |
| Eastbound Right | B | B |
| Westbound Left | A | A |
| Westbound Through and Right | A | A |
| Northbound Left | A | A |
| Southbound Left | A | A |
| Electronic Drive / Akers Drive (Stop-Controlled) | A |  |
| Westbound Left and Right | C | A |
| Southbound Left | A | B |
| Electronic Drive / Marksheffel Road (Stop-Controlled) | A |  |
| Eastbound Right |  |  |
| Access B / Akers Drive (Stop-Controlled) |  |  |
| Westbound Right |  |  |

[^7]Table 7 - Intersection Capacity Analysis Summary - Total Traffic - Year 2040

| INTERSECTION <br> LANE GROUPS | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Constitution Avenue / Akers Drive (Signalized) | $\mathrm{D}(39.5)$ | $\mathrm{D}(53.8)$ |
| Hunter Jumper Drive / Akers Drive (Stop-Controlled) | $\mathrm{B}(14.2)$ | $\mathrm{E}(65.9)$ |
| Eastbound Left |  |  |
| Eastbound Through | A | D |
| Eastbound Right | A | A |
| Westbound Left | A | B |
| Westbound Through and Right | C | F |
| Northbound Left | A | A |
| Southbound Left | A | A |
| Electronic Drive / Akers Drive (Stop-Controlled) | A | A |
| Eastbound Left | B | B |
| Eastbound Through | B | B |
| Eastbound Right | A | A |
| Westbound Left | B | B |
| Westbound Through | B | B |
| Westbound Right | A | A |
| Northbound Left | A | A |
| Southbound Left | A | A |
| Electronic Drive / Marksheffel Road (Stop-Controlled) |  | C |
| Eastbound Right | D |  |
| Access B / Akers Drive (Stop-Controlled) | A | B |
| Westbound Right |  |  |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Lev el of Service

## Total Traffic Analysis Results Upon Development Build-Out

Table 7 illustrates how, by Year 2040 and upon development build-out, the signalized intersection of Constitution Avenue with Marksheffel Road shows an overall LOS D operation during both morning and afternoon peak traffic hours. Compared to the background traffic analysis, the LOS during the afternoon peak traffic hour is shown to improve. This is due to optimized intersection split assumptions mentioned in Section III.

The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during the afternoon peak traffic period is attributed to the eastbound left turn movement, as well as northbound and southbound left and through volumes. In order to mitigate the anticipated LOS E operation, it is recommended allowing for a protective/permissive turn type for the eastbound left turn movement. This is shown to provide overall intersection operations of LOS D during the afternoon peak traffic hour.

The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at or better than LOS C during the morning peak traffic hour and LOS D or better during the afternoon peak traffic hour. Exceptions would include the westbound left turning movement which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours.

The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

The unsignalized intersection of Access B with Akers Drive anticipates turn movement operations at LOS A during the morning peak traffic hour and LOS B or better during the afternoon peak traffic hour.

These intersection operations are similar to background conditions.

## Pedestrian and Bicycle Accommodations

Watermark Akers Drive development would accommodate pedestrians and bicyclists with the following improvements:

- Pedestrian sidewalks along Constitution Avenue and Akers Drive adjacent to the site in accordance with local jurisdictional standards.
- Sidewalks along all internal local roadways in accordance with local jurisdictional standards.


## Queue Length Analysis

Queue lengths for proposed site access intersections were analyzed using Year 2040 total traffic conditions. The analysis yields estimate of $95^{\text {th }}$ percentile queue lengths, which have only a five percent probability of being exceeded during the analysis time period. Queue lengths were modeled and are included with the Synchro worksheets in Appendix C.

No significant queuing at the proposed site accesses was indicated. The greatest on-site queue length anticipated at Access A occurs during the afternoon peak hour. The queue length is approximately three vehicles for the eastbound right and westbound left turn movements.

At the intersection of Constitution Avenue and Akers Drive, afternoon peak hour queuing for the eastbound left turn movement is anticipated to exceed the existing storage length. It is recommended lengthening the existing back-to-back eastbound left turn lane to its maximum allowable length of approximately 450 feet. This is expected to be long enough to accommodate the anticipated $95^{\text {th }}$ percentile queuing. It is emphasized that the anticipated queuing is a result of conservative trips generated by the proposed development and surrounding area, and may only be exceed five percent of the time during the afternoon peak traffic period.

## Auxiliary Lane Analysis

Auxiliary lanes for site development accesses were based on the County's ECM and Section III of the City's design standards.

Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7 of the County's standards and Section 8.0, Table 2, of the City's design standards, reveals that left and right turn deceleration lanes are not required along Akers Drive. However, pursuant to existing striping patterns, it is assumed restriping will occur to allow for a northbound right turn deceleration lane at Access B, and right and left turn decelerations lanes at Access $A$, in order to provide consistency with the existing lane geometry on the west side of Akers Drive.

An evaluation of auxiliary lane requirements for existing turn lanes at the remaining study intersections indicates that exclusive turn lane requirements meet City and County minimum requirements and that no changes are recommended.

## Recommended Improvements

Table 8 illustrates the recommended roadway and intersection control improvements associated with the proposed Watermark Akers Drive development and adjacent area.

Table 8 - Recommended Roadway \& Intersection Improvements

| IMPROVEMENT | TYPE | TIMING | RESPONSIBILITY |
| :--- | :--- | :--- | :--- |
| Lengthen eastbound left turn lane on <br> Constitution Avenue at Akers Drive | Auxiliary Lane and <br> Median Modification | When 95th Percentile Queuing <br> Exceeds Existing Lane Length | Developments and other trip generators along <br> Akers Drive north of Constitution Avenue |
| Signalization of Constitution Avenue / Akers <br> Drive | Traffic Signal | When Warranted | Whoever warrants the need; i.e. County, City, or <br> Developer |
| Construct northbound right turn lanes on <br> Akers Drive at Access A and Access B | Auxiliary Lane | With Final Plat Application(s) / <br> Site Development | Applicant |
| Construct southbound left turn lane on Akers <br> Drive at Access A | Auxiliary Lane | With Final Plat Application(s) / <br> Site Development | Applicant |
| Widen Constitution Avenue and Marksheffel <br> Road to six-lane cross-section | Roadway Segment | Shown on MTCP by 2040 | Master planned |
| Extend Akers Drive south of Constitution <br> Avenue | Roadway Segment | With Future Development South <br> of Consitution Avenue | Developments and other trip generators along <br> Akers Drive south of Constitution Avenue |
| Extend Electronic Drive west of Akers Drive | Roadway Segment | With Future Development West <br> of Akers Drive | Developments and other trip generators along <br> Electronic Drive west of Akers Drive |

## VII. Conclusion

This traffic impact study addressed the capacity, geometric, and control requirements associated with the development entitled Watermark Akers Drive. This proposed residential development consists of various multifamily residential buildings. The development is located on the northwest corner of the Constitution Avenue with Marksheffel Road intersection in Colorado Springs, Colorado.

The study area examined in this analysis encompassed the Akers Drive intersections with Constitution Avenue, Hunter Jumper Drive, and Electronic Drive, the Marksheffel Road intersections with Constitution Avenue and Electronic Drive, and intersections with proposed site accesses.

A preliminary site distance evaluation for Access B was evaluated. Considering the multifamily land use being proposed and the approximate grade and posted speed limit along Akers Drive, preliminary evaluation indicates that access spacing requirements are satisfied.

Analysis was conducted for critical AM Peak Hour and PM Peak Hour traffic operations for existing traffic conditions, Year 2022 and Year 2040 background traffic conditions, and Year 2022 and Year 2040 total traffic conditions.

Analysis of existing traffic conditions indicates that the signalized intersection of Constitution Avenue with Marksheffel Road has overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The unsignalized intersection of Constitution Avenue with Akers Drive has turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road has turn movement operations at LOS C during the morning peak traffic hour and LOS B during the afternoon peak traffic hour.

Without the proposed development, Year 2022 background operational analysis shows that the signalized intersection of Constitution Avenue with Marksheffel Road experiences overall operations at LOS C during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The unsignalized intersection of Constitution Avenue with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive shows turn movement operations at LOS A during both morning and afternoon peak traffic hours. The unsignalized intersection of Electronic Drive with Akers Drive has turn movement operations at LOS A during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS C during both morning and afternoon peak traffic hours.

By Year 2040 and without the proposed development, the study intersection of Constitution Avenue with Marksheffel Road projects overall operations at LOS D during the morning peak traffic hour and LOS E during the afternoon peak traffic hour. The LOS E operation anticipated during afternoon peak traffic periods is attributed to eastbound and westbound through volumes, as well as left turning movements in all directions. The signalized intersection of Constitution Avenue with Akers Drive anticipates overall operations at LOS B during the morning peak traffic hour and LOS D during the afternoon peak traffic hour. The stop-controlled intersection of Hunter Jumper Drive with Akers Drive expects turn movement operations at LOS A during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour. The unsignalized intersection of Electronic Drive with Akers Drive shows turn movement operations at or better than LOS B during both morning and afternoon peak traffic hours. The stop-controlled intersection of Electronic Drive with Marksheffel Road experiences turning movement operations at LOS D during the morning peak traffic hour and LOS C during the afternoon peak traffic hour.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon roadway and intersection control improvements assumed within this analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at future levels of service comparable to Year 2040 background traffic conditions. Proposed site accesses have longterm operations at LOS D or better during peak traffic periods and upon build-out. Exceptions would include the westbound left turning movement along Hunter Jumper Drive at Akers Drive which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Akers Drive and the stop-controlled nature of the intersection. No reasonable mitigation measures can be recommended to improve the delay for this movement. Moreover, no mitigation is necessary as the poor level of service occurs on-site and is not expected to negatively impact operations of adjacent roadways or intersections.

No significant queuing at the proposed site accesses was indicated. The greatest on-site queue length anticipated at Access A occurs during the afternoon peak hour. The queue length is approximately three vehicles for the eastbound right and westbound left turn movements. At the intersection of Constitution Avenue and Akers Drive, afternoon peak hour queuing for the eastbound left turn movement is anticipated to exceed the existing storage length. It is recommended lengthening the existing back-to-back eastbound left turn lane to its maximum allowable length of approximately 450 feet. This is expected to be long enough to accommodate the anticipated $95^{\text {th }}$ percentile queuing. It is noted that the anticipated queuing is a result of conservative trips generated by the proposed development and surrounding area, and may only be exceed five percent of the time during afternoon peak traffic periods.

An evaluation of auxiliary lane requirements reveals that left and right turn deceleration lanes are not required along Akers Drive. However, pursuant to existing striping patterns, it is assumed restriping will occur to allow for a northbound right turn deceleration lane at Access B, and right and left turn decelerations lanes at Access A, in order to provide consistency with the existing lane geometry on the west side of Akers Drive. For existing turn lanes at the remaining study intersections, exclusive turn lane requirements meet City and County minimum requirements and no changes are recommended.

This site is subject to the El Paso County Road Impact Fee Program (Resolution 19-471), as amended. An option for payment will be selected at the final plat stage.

APPENDIX A
Traffic Count Data
Signal Timing Information
(303) 216-2439 www.alltrafficdata.net

Date: Tuesday, August 11, 2020
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:15 AM - 07:30 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | CONSTITUTION AVE <br> Eastbound |  |  |  | CONSTITUTION AVE <br> Westbound |  |  |  | MARKSHEFFEL RD Northbound |  |  |  | MARKSHEFFEL RD Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 23 | 50 | 67 | 0 | 16 | 68 | 38 | 4 | 34 | 141 | 18 | 0 | 36 | 281 | 29 | 805 | 3,227 | 0 | 0 | 0 | 0 |
| 7:15 AM | 1 | 32 | 70 | 60 | 0 | 31 | 49 | 36 | 5 | 48 | 121 | 17 | 0 | 14 | 315 | 31 | 830 | 3,038 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 19 | 64 | 75 | 0 | 21 | 85 | 49 | 7 | 49 | 145 | 12 | 0 | 30 | 240 | 32 | 828 | 2,799 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 24 | 57 | 58 | 0 | 9 | 86 | 29 | 8 | 53 | 115 | 19 | 0 | 34 | 249 | 23 | 764 | 2,548 | 0 | 0 | 0 | 1 |
| 8:00 AM | 0 | 22 | 64 | 44 | 0 | 17 | 46 | 32 | 2 | 40 | 99 | 15 | 0 | 28 | 177 | 30 | 616 | 2,290 | 0 | 0 | 1 | 0 |
| 8:15 AM | 1 | 22 | 72 | 47 | 0 | 22 | 58 | 40 | 4 | 28 | 84 | 13 | 0 | 22 | 160 | 18 | 591 |  | 0 | 0 | 0 | 1 |
| 8:30 AM | 0 | 11 | 66 | 34 | 0 | 10 | 64 | 34 | 4 | 48 | 95 | 13 | 0 | 26 | 148 | 24 | 577 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 24 | 52 | 50 | 0 | 14 | 57 | 44 | 3 | 33 | 83 | 7 | 0 | 15 | 115 | 9 | 506 |  | 0 | 0 | 0 | 0 |
| Count Total | 2 | 177 | 495 | 435 | 0 | 140 | 513 | 302 | 37 | 333 | 883 | 114 | 0 | 205 | 1,685 | 196 | 5,517 |  | 0 | 0 | 1 | 2 |
| Peak Hour | 1 | 98 | 241 | 260 | 0 | 77 | 288 | 152 | 24 | 184 | 522 | 66 | 0 | 114 | 1,085 | 115 | 3,227 |  | 0 | 0 | 0 | 1 |

Location: 1 MARKSHEFFEL RD \& CONSTITUTION AVE PM
Date: Tuesday, August 11, 2020
Peak Hour: 04:15 PM - 05:15 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 05:00 PM - 05:15 PM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | CONSTITUTION AVE <br> Eastbound |  |  |  | CONSTITUTION AVE Westbound |  |  |  | MARKSHEFFEL RD <br> Northbound |  |  |  | MARKSHEFFEL RD Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 22 | 130 | 48 | 0 | 27 | 87 | 42 | 1 | 75 | 273 | 8 | 0 | 43 | 153 | 34 | 943 | 3,924 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 28 | 149 | 50 | 0 | 24 | 105 | 54 | 1 | 88 | 252 | 14 | 0 | 54 | 155 | 28 | 1,002 | 4,033 | 0 | 0 | 0 | 0 |
| 4:30 PM | 2 | 19 | 129 | 64 | 1 | 29 | 91 | 53 | 3 | 86 | 252 | 13 | 0 | 50 | 153 | 27 | 972 | 3,990 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 23 | 130 | 51 | 1 | 28 | 114 | 66 | 7 | 98 | 253 | 15 | 0 | 55 | 133 | 33 | 1,007 | 3,930 | 0 | 0 | 0 | 0 |
| 5:00 PM | 1 | 29 | 123 | 60 | 0 | 35 | 89 | 59 | 3 | 73 | 316 | 13 | 0 | 44 | 172 | 35 | 1,052 | 3,731 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 27 | 141 | 63 | 0 | 25 | 96 | 52 | 4 | 71 | 244 | 15 | 0 | 48 | 147 | 26 | 959 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 23 | 143 | 50 | 0 | 18 | 92 | 46 | 3 | 71 | 226 | 12 | 0 | 48 | 156 | 24 | 912 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 26 | 116 | 46 | 0 | 29 | 83 | 39 | 4 | 51 | 194 | 8 | 0 | 53 | 133 | 26 | 808 |  | 0 | 0 | 0 | 0 |
| Count Total | 3 | 197 | 1,061 | 432 | 2 | 215 | 757 | 411 | 26 | 613 | 2,010 | 98 | 0 | 395 | 1,202 | 233 | 7,655 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 3 | 99 | 531 | 225 | 2 | 116 | 399 | 232 | 14 | 345 | 1,073 | 55 | 0 | 203 | 613 | 123 | 4,033 |  | 0 | 0 | 0 | 0 |

aLL TRAFFIC DATA SERVICES
(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval <br> Start Time | CONSTITUTION AVE Eastbound |  |  |  | CONSTITUTION AVE <br> Westbound |  |  |  | AKERS DR <br> Northbound |  |  |  | AKERS DR <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | eft | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 10 | 135 | 0 | 0 | 0 | 115 | 12 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 8 | 284 | 1,253 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 12 | 148 | 0 | 0 | 0 | 114 | 16 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 13 | 315 | 1,235 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 5 | 154 | 0 | 0 | 0 | 153 | 13 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 13 | 349 | 1,172 | 0 | 0 | 0 | 0 |
| 7:45 AM | 1 | 7 | 127 | 0 | 0 | 0 | 144 | 9 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 8 | 305 | 1,078 | 0 | 0 | 0 | 1 |
| 8:00 AM | 0 | 9 | 115 | 0 | 0 | 0 | 115 | 5 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 9 | 266 | 1,015 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 2 | 131 | 0 | 0 | 0 | 102 | 7 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 4 | 252 |  | 0 | 0 | 0 | 1 |
| 8:30 AM | 0 | 3 | 107 | 0 | 0 | 0 | 122 | 13 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 255 |  | 0 | 0 | 0 | 1 |
| 8:45 AM | 0 | 9 | 119 | 0 | 0 | 0 | 98 | 4 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 2 | 242 |  | 0 | 0 | 0 | 0 |
| Count Total | 1 | 57 | 1,036 | 0 | 0 | 0 | 963 | 79 | 0 | 0 | 0 | 0 | 0 | 70 | 0 | 62 | 2,268 |  | 0 | 0 | 0 | 3 |
| Peak Hour | 1 | 34 | 564 | 0 | 0 | 0 | 526 | 50 | 0 | 0 | 0 | 0 | 0 | 36 | 0 | 42 | 1,253 |  | 0 | 0 | 0 | 1 |

Location: 2 AKERS DR \& CONSTITUTION AVE PM
Date: Tuesday, August 11, 2020
Peak Hour: 04:15 PM - 05:15 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 04:30 PM - 04:45 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | CONSTITUTION AVE Eastbound |  |  |  | CONSTITUTION AVE <br> Westbound |  |  |  | AKERS DR <br> Northbound |  |  |  | AKERS DR <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | eft | Thru R |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 5 | 190 | 0 | 0 | 0 | 180 | 14 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 21 | 418 | 1,754 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 7 | 204 | 0 | 0 | 0 | 210 | 10 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 9 | 444 | 1,785 | 0 | 0 | 0 | 0 |
| 4:30 PM | 1 | 5 | 214 | 0 | 1 | 0 | 192 | 13 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 16 | 453 | 1,757 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 4 | 186 | 0 | 0 | 0 | 226 | 14 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 7 | 439 | 1,737 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 12 | 213 | 0 | 1 | 0 | 198 | 6 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 13 | 449 | 1,646 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 4 | 206 | 0 | 1 | 0 | 178 | 7 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 8 | 416 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 4 | 214 | 0 | 0 | 0 | 182 | 12 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 14 | 433 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 1 | 4 | 177 | 0 | 0 | 0 | 154 | 3 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 348 |  | 0 | 0 | 0 | 1 |
| Count Total | 2 | 45 | 1,604 | 0 | 3 | 0 | 1,520 | 79 | 0 | 0 | 0 | 0 | 0 | 54 | 0 | 93 | 3,400 |  | 0 | 0 | 0 | 1 |
| Peak Hour | 1 | 28 | 817 | 0 | 2 | 0 | 826 | 43 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 045 | 1,785 |  | 0 | 0 | 0 | 0 |

(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval Start Time | HUNTER JUMPER DR Eastbound |  |  |  | HUNTER JUMPER DR Westbound |  |  |  | AKERS DR <br> Northbound |  |  |  | AKERS DR <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 18 | 0 | 0 | 0 | 12 | 0 | 33 | 161 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 3 | 26 | 0 | 0 | 0 | 20 | 0 | 53 | 160 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 2 | 16 | 0 | 0 | 0 | 21 | 0 | 42 | 129 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 | 14 | 0 | 0 | 0 | 15 | 0 | 33 | 114 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 0 | 20 | 0 | 32 | 105 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 9 | 0 | 22 |  | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 | 13 | 0 | 0 | 0 | 10 | 0 | 27 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 1 | 1 | 12 | 0 | 0 | 0 | 7 | 0 | 24 |  | 1 | 0 | 0 | 0 |
| Count Total | 0 | 3 | 0 | 14 | 0 | 0 | 0 | 0 | 1 | 13 | 121 | 0 | 0 | 0 | 114 | 0 | 266 |  | 1 | 0 | 0 | 0 |
| Peak Hour | 0 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 10 | 74 | 0 | 0 | 0 | 68 | 0 | 0161 |  | 0 | 0 | 0 | 0 |

(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts


Location: 4 AKERS DR \& ELECTRONIC DR AM
Date: Tuesday, August 11, 2020
Peak Hour: 07:15 AM - 08:15 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 07:15 AM - 07:30 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | ELECTRONIC DR <br> Eastbound |  |  |  | ELECTRONIC DR <br> Westbound |  |  |  | AKERS DR <br> Northbound |  |  |  | AKERS DR <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn |  | Thru R |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 14 | 4 | 0 | 5 | 10 | 0 | 34 | 161 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 18 | 6 | 0 | 10 | 19 | 0 | 59 | 167 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 17 | 1 | 0 | 2 | 19 | 0 | 42 | 130 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 1 | 0 | 0 | 11 | 0 | 26 | 111 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 5 | 0 | 0 | 11 | 3 | 0 | 0 | 20 | 0 | 40 | 105 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 3 | 7 | 0 | 22 |  | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 13 | 1 | 0 | 0 | 8 | 0 | 23 |  | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 1 | 0 | 0 | 12 | 0 | 0 | 2 | 2 | 0 | 20 |  | 0 | 0 | 0 |
| Count Total | 0 | 0 | 0 | 0 | 0 | 18 | 0 | 7 | 0 | 0 | 107 | 16 | 0 | 22 | 96 | 0 | 266 |  | 0 | 0 | 0 |
| Peak Hour | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 6 | 0 | 0 | 60 | 11 | 0 | 12 | 69 | 0 | - 167 |  | 0 | 0 | 0 |

Location: 4 AKERS DR \& ELECTRONIC DR PM
Date: Tuesday, August 11, 2020
Peak Hour: 04:00 PM - 05:00 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 04:00 PM - 04:15 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

(303) 216-2439 www.alltrafficdata.net

Location: 5 MARKSHEFFEL RD \& ELECTRONIC DR AM
Date: Tuesday, August 11, 2020
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:00 AM - 07:15 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts


Location: 5 MARKSHEFFEL RD \& ELECTRONIC DR PM
Date: Tuesday, August 11, 2020
Peak Hour: 04:30 PM - 05:30 PM
Peak 15-Minutes: 05:00 PM - 05:15 PM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

All Traffic Data Services
Date Start: 11-Aug-20
CONSTITUTION AVE W.O. MARKSHEFFELRD
Site Code: 6
:al uoluets
路

All Traffic Data Services
www．alltrafficdata．net
Date Start：11－Aug－20
MARKSHEFFEL RD N．O．CONSTITUTION AVE
Site Code： 7
：al uollets

してトレて

All Traffic Data Services
www.alltrafficdata.net
Date Start: 11-Aug-20
AKERS DR N.O. CONSTITUTION AVE
Site Code: 8
:al uollels
8 :əpoう ə!!s


812/2020 11:05:08 AM
Overlap A Parents
Overlap B Parents
Overlap C Parents
Overlap D Parents
Constitution Ave

Intersection 624 at Marksheffel Rd and Constitution Ave - Phases and overlaps

812/2020 11:05:14 AM

Intersection 624 at Marksheffel Rd and Constitution Ave - Plans schedule

| - Plan Changes | $\bigcirc$ Page Changes |  |  | Week of | 8/10/2020 | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mon 08110 | Tue 08/11 | Wed 08,12 | Thu 08/13 | Fri 08/14 |  |
| 12:00am | Fire | Free | Free | Free | Free |  |
| Cycle len.offset |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 6:30am | Plan1.Ofst1 Plan1.Ofst1 Plan 1.Ofst1 Plan 1,Ofst1 Plan 1.Ofst1 |  |  |  |  |  |
| Cycle len.offset | 120.85 | 120.85 | 120.85 | 120.85 | 120.85 |  |
| 4:00pm | Plan2.Ofst1 Plan2.Ofst1 Plan2.Ofst1 Plan2.Ofst1 Plan2.Ofst1 |  |  |  |  |  |
| Cycle len.offset | 140.74 | 140.74 | 140.74 | 140.74 | 140.74 |  |
| 6:00pm | Free | Free | Free | Free | Free |  |
| Cycle len.offset |  |  |  |  |  |  |

Intersection 624 at Marksheffel Rd and Constitution Ave - Timing table


## APPENDIX B

Level of Service Definitions

The following information can be found in the Highway Capacity Manual, Transportation Research Board, 2010: Chapter 18 - Signalized Intersections and Chapter 19 - Two-Way Stop Controlled Intersections.

## Automobile Level of Service (LOS) for Signalized Intersections

Levels of service are defined to represent reasonable ranges in control delay.

## LOS A

Describes operations with a control delay of 10 s/veh or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

## LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

## LOS C

Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

## LOS D

Describes operations with control delay between 35 and $55 \mathrm{~s} /$ veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

## LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F
Describes operations with control delay exceeding $80 \mathrm{~s} / \mathrm{veh}$ or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

## Level of Service (LOS) for Unsignalized TWSC Intersections

| Level of Service | Average Control Delay (s/veh) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| D | $>25-35$ |
| E | $>35-50$ |
| F | $>50$ |

## APPENDIX C

## Capacity Worksheets

1：Marksheffel Road \＆Constitution Avenue

|  | 4 |  |  |  |  |  | 71 | 4 | $\uparrow$ |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | 个 $\uparrow$ | F | \％ | 个4 | 「 |  | ${ }^{\text {a }}$ | 个4 | 「 | ${ }^{1 *}$ | 个4 |
| Traffic Volume（vph） | 99 | 241 | 260 | 77 | 288 | 152 | 24 | 184 | 522 | 66 | 114 | 1085 |
| Future Volume（vph） | 99 | 241 | 260 | 77 | 288 | 152 | 24 | 184 | 522 | 66 | 114 | 1085 |
| Satd．Flow（prot） | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Flt Permitted | 0.408 |  |  | 0.590 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 760 | 3539 | 1583 | 1099 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Satd．Flow（RTOR） |  |  | 275 |  |  | 165 |  |  |  | 127 |  |  |
| Lane Group Flow（vph） | 108 | 262 | 283 | 84 | 313 | 165 | 0 | 226 | 567 | 72 | 124 | 1179 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ s ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 15.0 | 30.0 | 30.0 | 15.0 | 30.0 | 30.0 | 20.0 | 20.0 | 55.0 | 55.0 | 20.0 | 55.0 |
| Total Split（\％） | 12．5\％ | 25．0\％ | 25．0\％ | 12．5\％ | 25．0\％ | 25．0\％ | 16．7\％ | 16．7\％ | 45．8\％ | 45．8\％ | 16．7\％ | 45．8\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 28.5 | 18.6 | 18.6 | 26.8 | 15.9 | 15.9 |  | 13.2 | 61.0 | 61.0 | 9.7 | 57.6 |
| Actuated g／C Ratio | 0.24 | 0.16 | 0.16 | 0.22 | 0.13 | 0.13 |  | 0.11 | 0.51 | 0.51 | 0.08 | 0.48 |
| v／c Ratio | 0.42 | 0.48 | 0.59 | 0.28 | 0.67 | 0.47 |  | 0.60 | 0.32 | 0.08 | 0.45 | 0.69 |
| Control Delay | 38.3 | 49.8 | 11.5 | 35.2 | 56.6 | 11.3 |  | 57.5 | 18.8 | 0.4 | 57.4 | 28.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 38.3 | 49.8 | 11.5 | 35.2 | 56.6 | 11.3 |  | 57.5 | 18.8 | 0.4 | 57.4 | 28.3 |
| LOS | D | D | B | D | E | B |  | E | B | A | E | C |
| Approach Delay |  | 31.3 |  |  | 40.1 |  |  |  | 27.3 |  |  | 28.7 |
| Approach LOS |  | C |  |  | D |  |  |  | C |  |  | C |
| Queue Length 50th（ft） | 65 | 101 | 5 | 50 | 123 | 0 |  | 87 | 132 | 0 | 47 | 368 |
| Queue Length 95th（t） | 109 | 140 | 86 | 88 | 165 | 61 |  | 125 | 193 | 3 | 78 | 510 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（tt） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 265 | 684 | 527 | 311 | 678 | 436 |  | 439 | 1800 | 867 | 429 | 1697 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.41 | 0.38 | 0.54 | 0.27 | 0.46 | 0.38 |  | 0.51 | 0.32 | 0.08 | 0.29 | 0.69 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 85 （71\％），Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 60 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


| 4 |  |
| :---: | :---: |
| Lane Group | SBR |
| Lalteonfigurations | \% |
| Traffic Volume (vph) | 115 |
| Future Volume (vph) | 115 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 127 |
| Lane Group Flow (vph) | 125 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial (s) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 55.0 |
| Total Split (\%) | 45.8\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 57.6 |
| Actuated g/C Ratio | 0.48 |
| v/c Ratio | 0.15 |
| Control Delay | 4.0 |
| Queue Delay | 0.0 |
| Total Delay | 4.0 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 0 |
| Queue Length 95th (tt) | 36 |
| Internal Link Dist (t) |  |
| Turn Bay Length ( t ) | 265 |
| Base Capacity (vph) | 825 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.15 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.69
Intersection Signal Delay: 30.7 Intersection LOS: C

Intersection Capacity Utilization 76.3\% ICU Level of Service D
Analysis Period (min) 15




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.5 | 0 | 10.1 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | $* 1306$ | - | - | - | 744 | - |
| HCM Lane V/C Ratio | 0.029 | - | - | -0.053 | - |  |
| HCM Control Delay (s) | 7.8 | - | - | - | 10.1 | 0 |
| HCM Lane LOS | A | - | - | - | B | A |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.2 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | 1 | $\mathbf{7}$ |  | $\mathbf{4}$ | 4 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 0 | 9 | 10 | 75 | 69 | 0 |
| Future Vol, veh/h | 0 | 9 | 10 | 75 | 69 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 100 | 0 | 130 | - | - | 120 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 76 | 76 | 76 | 76 | 76 | 76 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 12 | 13 | 99 | 91 | 0 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| $l$ |  |  |  |  |  |  |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |


| Major/Minor | Minor2 |  |  |  |  |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | ---: | :--- | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | - | 703 | - | 0 | - |  |  |  |  |  |
| $\quad$ Stage 1 | - | - | - | - | - |  |  |  |  |  |
| $\quad$ Stage 2 | - | - | - | - | - |  |  |  |  |  |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 15.1 | 0 | 0 |
| HCM LOS | C |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -380 | - |
| HCM Lane V/C Ratio | - | 0.06 |

1：Marksheffel Road \＆Constitution Avenue

|  |  |  |  |  |  |  |  | 4 | $\uparrow$ |  |  | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | 个个 | 「 | \％ | 个4 | 「 |  | $\mathbf{H}^{4}$ | 个个 | 「 | \％${ }^{1 \times}$ | 个4 |
| Traffic Volume（vph） | 102 | 531 | 225 | 118 | 399 | 232 | 14 | 345 | 1073 | 55 | 203 | 613 |
| Future Volume（vph） | 102 | 531 | 225 | 118 | 399 | 232 | 14 | 345 | 1073 | 55 | 203 | 613 |
| Satd．Flow（prot） | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Flt Permitted | 0.368 |  |  | 0.193 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 685 | 3539 | 1583 | 360 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Satd．Flow（RTOR） |  |  | 245 |  |  | 252 |  |  |  | 109 |  |  |
| Lane Group Flow（vph） | 111 | 577 | 245 | 128 | 434 | 252 | 0 | 390 | 1166 | 60 | 221 | 666 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 21.0 | 46.0 | 46.0 | 21.0 | 46.0 | 46.0 | 22.0 | 22.0 | 51.0 | 51.0 | 22.0 | 51.0 |
| Total Split（\％） | 15．0\％ | 32．9\％ | 32．9\％ | 15．0\％ | 32．9\％ | 32．9\％ | 15．7\％ | 15．7\％ | 36．4\％ | 36．4\％ | 15．7\％ | 36．4\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 42.6 | 29.0 | 29.0 | 45.1 | 30.2 | 30.2 |  | 21.2 | 59.9 | 59.9 | 14.3 | 53.0 |
| Actuated g／C Ratio | 0.30 | 0.21 | 0.21 | 0.32 | 0.22 | 0.22 |  | 0.15 | 0.43 | 0.43 | 0.10 | 0.38 |
| v／c Ratio | 0.37 | 0.79 | 0.47 | 0.52 | 0.57 | 0.47 |  | 0.75 | 0.77 | 0.08 | 0.63 | 0.50 |
| Control Delay | 33.8 | 60.6 | 8.0 | 38.3 | 51.5 | 7.7 |  | 66.6 | 39.9 | 0.3 | 68.3 | 36.5 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 33.8 | 60.6 | 8.0 | 38.3 | 51.5 | 7.7 |  | 66.6 | 39.9 | 0.3 | 68.3 | 36.5 |
| LOS | C | E | A | D | D | A |  | E | D | A | E | D |
| Approach Delay |  | 43.6 |  |  | 35.9 |  |  |  | 44.9 |  |  | 39.4 |
| Approach LOS |  | D |  |  | D |  |  |  | D |  |  | D |
| Queue Length 50th（tt） | 70 | 264 | 0 | 81 | 187 | 0 |  | 175 | 473 | 0 | 101 | 248 |
| Queue Length 95th（tt） | 104 | 312 | 68 | 118 | 229 | 68 |  | \＃239 | \＃717 | 2 | 141 | 336 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（t） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 351 | 985 | 617 | 283 | 985 | 622 |  | 519 | 1513 | 739 | 421 | 1338 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.32 | 0.59 | 0.40 | 0.45 | 0.44 | 0.41 |  | 0.75 | 0.77 | 0.08 | 0.52 | 0.50 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $74(53 \%)$ ，Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 70 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


| $\downarrow$ |  |
| :---: | :---: |
| Lane Group | SBR |
| La\|*eonfigurations | F |
| Traffic Volume (vph) | 123 |
| Future Volume (vph) | 123 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 134 |
| Lane Group Flow (vph) | 134 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 51.0 |
| Total Split (\%) | 36.4\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 53.0 |
| Actuated g/C Ratio | 0.38 |
| v/c Ratio | 0.20 |
| Control Delay | 6.0 |
| Queue Delay | 0.0 |
| Total Delay | 6.0 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 0 |
| Queue Length 95th (t) | 49 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (t) | 265 |
| Base Capacity (vph) | 681 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.20 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.79
Intersection Signal Delay: 41.7 Intersection LOS: D

Intersection Capacity Utilization 76.7\% ICU Level of Service D
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road \& Constitution Avenue




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.3 | 0 | 10.7 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | *1120 | - | - | - | 658 | - |
| HCM Lane V/C Ratio | 0.028 | - | - | -0.038 | - |  |
| HCM Control Delay (s) | 8.3 | - | - | - | 10.7 | 0 |
| HCM Lane LOS | A | - | - | - | B | A |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.1 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | $\mathbf{7}$ | $\mathbf{r}$ | $\mathbf{1}$ | 个 | 个 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 2 | 4 | 10 | 62 | 64 | 0 |
| Future Vol, veh/h | 2 | 4 | 10 | 62 | 64 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 100 | 0 | 130 | - | - | 120 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 5 | 13 | 81 | 83 | 0 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement V | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | $\hat{\beta}$ |  | ${ }^{7}$ | 4 |
| Traffic Vol, veh/h | 7 | 5 | 69 | 5 | 25 | 68 |
| Future Vol, veh/h | 7 | 5 | 69 | 5 | 25 | 68 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 150 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 83 | 83 | 83 | 83 | 83 | 83 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 6 | 83 | 6 | 30 | 82 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor |  | Minor2 |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- | :---: |
| Conflicting Flow All | - | 494 | - | 0 | - | 0 |  |
| $\quad$ Stage 1 | - | - | - | - | - | - |  |
| $\quad$ Stage 2 | - | - | - | - | - | - |  |
| Critical Hdwy | - | 6.94 | - | - | - | - |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |  |
| Follow-up Hdwy | - | 3.32 | - | - | - | - |  |
| Pot Cap-1 Maneuver | 0 | 521 | 0 | - | - | 0 |  |
| $\quad$ Stage 1 | 0 | - | 0 | - | - | 0 |  |
| $\quad$ Stage 2 | 0 | - | 0 | - | - | 0 |  |
| Platoon blocked, \% |  |  |  | - | - |  |  |
| Mov Cap-1 Maneuver | - | 521 | - | - | - | - |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |  |
| Stage 1 | - | - | - | - | - | - |  |
| Stage 2 | - | - | - | - | - | - |  |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 12.4 | 0 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -521 | - |
| HCM Lane V/C Ratio | -0.065 | - |
| HCM Control Delay (s) | -12.4 | - |
| HCM Lane LOS | - | $B$ |
| HCM 95th \%tile Q(veh) | - | - |
| H.2 | - |  |


|  | $\rangle$ | $\rightarrow$ |  | $\checkmark$ |  |  | $\dagger$ | 4 | $\dagger$ |  |  | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 个个 | ${ }^{7}$ | ${ }^{7}$ | 个4 | 「 |  | 甸＊ | 坐 | 「 | \％${ }^{1 / 4}$ | 个4 |
| Traffic Volume（vph） | 106 | 264 | 270 | 150 | 315 | 195 | 25 | 191 | 554 | 88 | 147 | 1132 |
| Future Volume（vph） | 106 | 264 | 270 | 150 | 315 | 195 | 25 | 191 | 554 | 88 | 147 | 1132 |
| Satd．Flow（prot） | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| FIt Permitted | 0.414 |  |  | 0.480 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 771 | 3539 | 1583 | 894 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Satd．Flow（RTOR） |  |  | 230 |  |  | 212 |  |  |  | 127 |  |  |
| Lane Group Flow（vph） | 115 | 287 | 293 | 163 | 342 | 212 | 0 | 235 | 602 | 96 | 160 | 1230 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 15.0 | 30.0 | 30.0 | 15.0 | 30.0 | 30.0 | 20.0 | 20.0 | 55.0 | 55.0 | 20.0 | 55.0 |
| Total Split（\％） | 12．5\％ | 25．0\％ | 25．0\％ | 12．5\％ | 25．0\％ | 25．0\％ | 16．7\％ | 16．7\％ | 45．8\％ | 45．8\％ | 16．7\％ | 45．8\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 28.2 | 16.7 | 16.7 | 28.9 | 17.0 | 17.0 |  | 13.3 | 58.6 | 58.6 | 10.9 | 56.2 |
| Actuated g／C Ratio | 0.24 | 0.14 | 0.14 | 0.24 | 0.14 | 0.14 |  | 0.11 | 0.49 | 0.49 | 0.09 | 0.47 |
| v／c Ratio | 0.44 | 0.58 | 0.70 | 0.57 | 0.68 | 0.52 |  | 0.62 | 0.35 | 0.11 | 0.51 | 0.74 |
| Control Delay | 38.0 | 52.8 | 21.3 | 42.3 | 55.8 | 10.7 |  | 58.0 | 20.7 | 1.9 | 57.6 | 30.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 38.0 | 52.8 | 21.3 | 42.3 | 55.8 | 10.7 |  | 58.0 | 20.7 | 1.9 | 57.6 | 30.7 |
| LOS | D | D | C | D | E | B |  | E | C | A | E | C |
| Approach Delay |  | 37.1 |  |  | 39.4 |  |  |  | 28.2 |  |  | 31.2 |
| Approach LOS |  | D |  |  | D |  |  |  | C |  |  | C |
| Queue Length 50th（ft） | 68 | 111 | 44 | 100 | 134 | 0 |  | 90 | 147 | 0 | 61 | 401 |
| Queue Length 95th（ft） | 112 | 150 | 135 | 152 | 176 | 66 |  | 130 | 218 | 18 | 95 | 553 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（ t ） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 267 | 678 | 489 | 289 | 678 | 474 |  | 435 | 1726 | 837 | 429 | 1656 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.43 | 0.42 | 0.60 | 0.56 | 0.50 | 0.45 |  | 0.54 | 0.35 | 0.11 | 0.37 | 0.74 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 85 （71\％），Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 60 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
| Lal**enfigurations | 「 |
| Traffic Volume (vph) | 123 |
| Future Volume (vph) | 123 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 127 |
| Lane Group Flow (vph) | 134 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 55.0 |
| Total Split (\%) | 45.8\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 56.2 |
| Actuated g/C Ratio | 0.47 |
| v/c Ratio | 0.17 |
| Control Delay | 4.8 |
| Queue Delay | 0.0 |
| Total Delay | 4.8 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 3 |
| Queue Length 95th (tt) | 42 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (tt) | 265 |
| Base Capacity (vph) | 808 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.17 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.74
Intersection Signal Delay: 33.1 Intersection LOS: C

Intersection Capacity Utilization 82.5\% ICU Level of Service E Analysis Period (min) 15




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.4 | 0 | 10.7 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | $* 1306$ | - | - | - | 677 | - |
| HCM Lane V/C Ratio | 0.03 | - | - | -0.059 | - |  |
| HCM Control Delay (s) | 7.8 | - | - | - | 10.7 | 0 |
| HCM Lane LOS | A | - | - | - | B | A |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.2 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL |  |
| Lane Configurations | * |  | $\uparrow$ |  | ${ }^{1}$ | 4 |
| Traffic Vol, veh/h | 9 | 6 | 62 | 11 | 12 | 72 |
| Future Vol, veh/h | 9 | 6 | 62 | 11 | 12 | 72 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 150 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 71 | 71 | 71 | 71 | 71 | 71 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 8 | 87 | 15 | 17 | 101 |





| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 16.1 | 0 | 0 |
| HCM LOS | C |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -349 | - |
| HCM Lane V/C Ratio | -0.069 | - |
| HCM Control Delay (s) | -16.1 | - |
| HCM Lane LOS | - | C |
| HCM 95th \%tile Q(veh) | - | - |


|  |  |  |  | 7 |  |  |  | 4 | $\uparrow$ |  |  | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 个个 | 「 | \％ | 个4 | 「 |  | $\mathbf{H}^{4}$ | 个个 | 「 | \％${ }^{1 / 4}$ | 个4 |
| Traffic Volume（vph） | 113 | 570 | 234 | 181 | 437 | 296 | 15 | 359 | 1147 | 115 | 255 | 637 |
| Future Volume（vph） | 113 | 570 | 234 | 181 | 437 | 296 | 15 | 359 | 1147 | 115 | 255 | 637 |
| Satd．Flow（prot） | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Flt Permitted | 0.370 |  |  | 0.165 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 689 | 3539 | 1583 | 307 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Satd．Flow（RTOR） |  |  | 250 |  |  | 322 |  |  |  | 109 |  |  |
| Lane Group Flow（vph） | 123 | 620 | 254 | 197 | 475 | 322 | 0 | 406 | 1247 | 125 | 277 | 692 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 21.0 | 46.0 | 46.0 | 21.0 | 46.0 | 46.0 | 22.0 | 22.0 | 51.0 | 51.0 | 22.0 | 51.0 |
| Total Split（\％） | 15．0\％ | 32．9\％ | 32．9\％ | 15．0\％ | 32．9\％ | 32．9\％ | 15．7\％ | 15．7\％ | 36．4\％ | 36．4\％ | 15．7\％ | 36．4\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 44.6 | 30.6 | 30.6 | 50.8 | 33.7 | 33.7 |  | 21.6 | 54.2 | 54.2 | 16.1 | 48.7 |
| Actuated g／C Ratio | 0.32 | 0.22 | 0.22 | 0.36 | 0.24 | 0.24 |  | 0.15 | 0.39 | 0.39 | 0.12 | 0.35 |
| v／c Ratio | 0.39 | 0.80 | 0.47 | 0.73 | 0.56 | 0.52 |  | 0.77 | 0.91 | 0.18 | 0.70 | 0.56 |
| Control Delay | 32.0 | 60.0 | 8.1 | 46.4 | 48.9 | 7.2 |  | 67.1 | 52.0 | 8.6 | 69.5 | 40.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 32.0 | 60.0 | 8.1 | 46.4 | 48.9 | 7.2 |  | 67.1 | 52.0 | 8.6 | 69.5 | 40.2 |
| LOS | C | E | A | D | D | A |  | E | D | A | E | D |
| Approach Delay |  | 43.3 |  |  | 34.9 |  |  |  | 52.4 |  |  | 43.2 |
| Approach LOS |  | D |  |  | C |  |  |  | D |  |  | D |
| Queue Length 50th（tt） | 74 | 283 | 3 | 124 | 199 | 0 |  | 181 | 571 | 9 | 126 | 278 |
| Queue Length 95th（tt） | 111 | 333 | 71 | 175 | 248 | 75 |  | \＃281 | \＃827 | 58 | 173 | 351 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（ t ） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 362 | 985 | 621 | 279 | 985 | 673 |  | 529 | 1369 | 679 | 430 | 1230 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.34 | 0.63 | 0.41 | 0.71 | 0.48 | 0.48 |  | 0.77 | 0.91 | 0.18 | 0.64 | 0.56 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $74(53 \%)$ ，Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


| $\downarrow$ |  |
| :---: | :---: |
| Lane Group | SBR |
| La\|*eonfigurations | F |
| Traffic Volume (vph) | 130 |
| Future Volume (vph) | 130 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 141 |
| Lane Group Flow (vph) | 141 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 51.0 |
| Total Split (\%) | 36.4\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 48.7 |
| Actuated g/C Ratio | 0.35 |
| v/c Ratio | 0.22 |
| Control Delay | 6.1 |
| Queue Delay | 0.0 |
| Total Delay | 6.1 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 0 |
| Queue Length 95th (t) | 49 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (t) | 265 |
| Base Capacity (vph) | 642 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.22 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.91
Intersection Signal Delay: $44.9 \quad$ Intersection LOS: D

Intersection Capacity Utilization 84.8\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road \& Constitution Avenue



| Major/Minor | Major1 | Major2 |  | Minor2 |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Conflicting Flow All | 1011 | 0 | - | 0 | 1408 |
| $\quad$ Stage 1 | - | - | - | - | 962 |
| $\quad$ Stage 2 | - | - | - | - | 446 |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.3 | 0 | 10.9 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | *1087 | - | - | -638 | - |
| HCM Lane V/C Ratio | 0.03 | - | - | -0.041 | - |
| HCM Control Delay (s) | 8.4 | - | - | - | 10.9 |
| HCM Lane LOS | A | - | - | - | B |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.1 |
| H | - |  |  |  |  |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon




| Major/Minor | Minor1 | Major1 |  |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 238 | 90 | 0 | 0 | 93 | 0 |  |
| Stage 1 | 90 | - | - | - | - | - |  |
| Stage 2 | 148 | - | - | - |  | - |  |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | - |  | 2.218 | - |  |
| Pot Cap-1 Maneuver | 750 | 968 | - | - | 1501 | - |  |
| Stage 1 | 934 | - | - | - | - | - |  |
| Stage 2 | 880 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  | - | - |  | - |  |
| Mov Cap-1 Maneuver | 734 | 968 | - | - | 1501 | - |  |
| Mov Cap-2 Maneuver | 741 | - | - | - | - | - |  |
| Stage 1 | 934 | - |  | - | - | - |  |
| Stage 2 | 862 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |  |
| HCM Control Delay, s | 9.5 |  | 0 |  | 2 |  |  |
| HCM LOS | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRW | BLn1 | SBL | SBT |  |
| Capacity (veh/h) |  | - | - | 821 | 1501 | - |  |
| HCM Lane V/C Ratio |  | - |  | 0.018 | 0.021 | - |  |
| HCM Control Delay (s) |  |  |  | 9.5 | 7.5 | - |  |
| HCM Lane LOS |  | - | - | A | A | - |  |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0.1 | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 13.1 | 0 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -480 | - |
| HCM Lane V/C Ratio | -0.072 | - |
| HCM Control Delay (s) | -13.1 | - |
| HCM Lane LOS | - | $B$ |
| HCM 95th \%tile Q(veh) | - | - |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
|  | F |
| Traffic Volume (vph) | 188 |
| Future Volume (vph) | 188 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 173 |
| Lane Group Flow (vph) | 204 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 55.0 |
| Total Split (\%) | 45.8\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 48.7 |
| Actuated g/C Ratio | 0.41 |
| v/c Ratio | 0.27 |
| Control Delay | 6.2 |
| Queue Delay | 0.0 |
| Total Delay | 6.2 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 14 |
| Queue Length 95th (tt) | 63 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (tt) | 265 |
| Base Capacity (vph) | 745 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.27 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.96
Intersection Signal Delay: $39.4 \quad$ Intersection LOS: D

Intersection Capacity Utilization 95.9\%
ICU Level of Service F
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road \& Constitution Avenue


2: Constitution Avenue \& Akers Drive

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Maximum v/c Ratio: 0.52
Intersection Signal Delay: 13.6 Intersection LOS: B

Intersection Capacity Utilization 49.3\% ICU Level of Service A Analysis Period (min) 15
$m$ Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: $\quad 2:$ Constitution Avenue \& Akers Drive


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{1}$ | 个 | $\mathbf{4}$ | $\mathbf{7}$ |
| Traffic Vol, veh/h | 0 | 110 | 117 | 126 | 133 | 0 |
| Future Vol, veh/h | 0 | 110 | 117 | 126 | 133 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 100 | 0 | 130 | - | - | 120 |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 120 | 127 | 137 | 145 | 0 |






| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- |
| Conflicting Flow All | - | 1090 | - | 0 | - | 0 |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 7.14 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | - | 3.92 | - | - | - | - |
| Pot Cap-1 Maneuver | 0 | 181 | 0 | - | - | 0 |
| $\quad$ Stage 1 | 0 | - | 0 | - | - | 0 |
| $\quad$ Stage 2 | 0 | - | 0 | - | - | 0 |
| Platoon blocked, \% |  |  |  | - | - |  |
| Mov Cap-1 Maneuver | - | 181 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 32.3 | 0 | 0 |
| HCM LOS | D |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -181 | - |
| HCM Lane V/C Ratio | -0.276 | - |
| HCM Control Delay (s) | -32.3 | - |
| HCM Lane LOS | - | $D$ |
| HCM 95th \%tile Q(veh) | - | 1.1 |


|  | 4 |  |  | $\checkmark$ |  |  | 71 | 4 | $\dagger$ | \% |  | $\frac{1}{1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 444 | 7 | ${ }^{7}$ | 444 | 「 |  | ** | 444 | F | \% 1 | 444 |
| Traffic Volume (vph) | 236 | 1079 | 376 | 235 | 766 | 403 | 21 | 539 | 1641 | 141 | 349 | 919 |
| Future Volume (vph) | 236 | 1079 | 376 | 235 | 766 | 403 | 21 | 539 | 1641 | 141 | 349 | 919 |
| Satd. Flow (prot) | 1770 | 5085 | 1583 | 1770 | 5085 | 1583 | 0 | 3433 | 5085 | 1583 | 3433 | 5085 |
| Flt Permitted | 0.158 |  |  | 0.128 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd. Flow (perm) | 294 | 5085 | 1583 | 238 | 5085 | 1583 | 0 | 3433 | 5085 | 1583 | 3433 | 5085 |
| Satd. Flow (RTOR) |  |  | 288 |  |  | 251 |  |  |  | 134 |  |  |
| Lane Group Flow (vph) | 257 | 1173 | 409 | 255 | 833 | 438 | 0 | 609 | 1784 | 153 | 379 | 999 |
| Turn Type | pm+pt | NA | Perm | pm+pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split (s) | 22.0 | 40.0 | 40.0 | 20.0 | 38.0 | 38.0 | 33.0 | 33.0 | 59.0 | 59.0 | 21.0 | 47.0 |
| Total Split (\%) | 15.7\% | 28.6\% | 28.6\% | 14.3\% | 27.1\% | 27.1\% | 23.6\% | 23.6\% | 42.1\% | 42.1\% | 15.0\% | 33.6\% |
| Yellow Time (s) | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C-Max | C-Max | None | C-Max |
| Act Effct Green (s) | 51.8 | 33.0 | 33.0 | 48.2 | 31.2 | 31.2 |  | 27.3 | 52.0 | 52.0 | 16.0 | 40.7 |
| Actuated g/C Ratio | 0.37 | 0.24 | 0.24 | 0.34 | 0.22 | 0.22 |  | 0.20 | 0.37 | 0.37 | 0.11 | 0.29 |
| v/c Ratio | 0.90 | 0.98 | 0.69 | 1.04 | 0.74 | 0.80 |  | 0.91 | 0.94 | 0.23 | 0.97 | 0.68 |
| Control Delay | 66.1 | 74.3 | 20.8 | 105.3 | 55.1 | 33.6 |  | 73.9 | 54.0 | 7.3 | 99.0 | 46.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 66.1 | 74.3 | 20.8 | 105.3 | 55.1 | 33.6 |  | 73.9 | 54.0 | 7.3 | 99.0 | 46.7 |
| LOS | E | E | C | F | E | C |  | E | D | A | F | D |
| Approach Delay |  | 61.3 |  |  | 57.3 |  |  |  | 55.9 |  |  | 52.8 |
| Approach LOS |  | E |  |  | E |  |  |  | E |  |  | D |
| Queue Length 50th (ft) | 165 | 391 | 98 | ~195 | 260 | 171 |  | 280 | 573 | 11 | 179 | 297 |
| Queue Length 95th (ft) | \#323 | \#494 | 225 | \#376 | 312 | \#324 |  | \#380 | \#676 | 58 | \#283 | 350 |
| Internal Link Dist (ft) |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length (ft) | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity (vph) | 288 | 1198 | 593 | 245 | 1132 | 547 |  | 686 | 1888 | 672 | 392 | 1478 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.89 | 0.98 | 0.69 | 1.04 | 0.74 | 0.80 |  | 0.89 | 0.94 | 0.23 | 0.97 | 0.68 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 74 (53\%), Referenced to phase 2:SBT and 6:NBT, Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 110 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
|  | 「 |
| Traffic Volume (vph) | 227 |
| Future Volume (vph) | 227 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 247 |
| Lane Group Flow (vph) | 247 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial (s) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 47.0 |
| Total Split (\%) | 33.6\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 40.7 |
| Actuated g/C Ratio | 0.29 |
| v/c Ratio | 0.39 |
| Control Delay | 6.2 |
| Queue Delay | 0.0 |
| Total Delay | 6.2 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 0 |
| Queue Length 95th (tt) | 65 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (tt) | 265 |
| Base Capacity (vph) | 635 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.39 |
| Intersection Summary |  |

Maximum v/c Ratio: 1.04
Intersection Signal Delay: 56.8 Intersection LOS: E

Intersection Capacity Utilization 95.5\% ICU Level of Service F
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 1: Marksheffel Road \& Constitution Avenue


2: Constitution Avenue \& Akers Drive

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

2: Constitution Avenue \& Akers Drive
Maximum v/c Ratio: 2.05
Intersection Signal Delay: $54.3 \quad$ Intersection LOS: D

Intersection Capacity Utilization 82.8\% ICU Level of Service E
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Constitution Avenue \& Akers Drive


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{1}$ | 4 | 4 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 3 | 378 | 305 | 138 | 115 | 0 |
| Future Vol, veh/h | 3 | 378 | 305 | 138 | 115 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 100 | 0 | 130 | - | - | 120 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 411 | 332 | 150 | 125 | 0 |


| Major/Minor | Minor2 | Major1 |  | Major2 |  |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- |
| Conflicting Flow All | 939 | 125 | 125 | 0 | - | 0 |
| $\quad$ Stage 1 | 125 | - | - | - | - | - |
| $\quad$ Stage 2 | 814 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 287 | 926 | 1462 | - | - | - |
| $\quad$ Stage 1 | 901 | - | - | - | - | - |
| $\quad$ Stage 2 | 425 | - | - | - | - | - |
| Platoon blocked, \% | 1 |  |  | - | - | - |
| Mov Cap-1 Maneuver | 222 | 926 | 1462 | - | - | - |
| Mov Cap-2 Maneuver | 332 | - | - | - | - | - |
| Stage 1 | 696 | - | - | - | - | - |
| Stage 2 | 425 | - | - | - | - | - |
|  |  |  |  |  |  |  |


| Approach | EB | NB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 12 | 5.6 | 0 |

HCM LOS B

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 EBLn2 | SBT | SBR |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1462 | - | 332 | 926 | - |
| - |  |  |  |  |  |
| HCM Lane V/C Ratio | 0.227 | - | 0.01 | 0.444 | - |
| HCM Control Delay (s) | 8.2 | - | 16 | 12 | - |
| HCM Lane LOS | A | - | C | B | - |
| HCM 95th \%tile Q(veh) | 0.9 | - | 0 | 2.3 | - |



HCM LOS B B

| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1 EBLn2 | EBLn3WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1458 | - | - | 518 | 509 | 942 | 512 | 504 | 983 | 1481 |




| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 21.2 | 0 | 0 |
| HCM LOS | C |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -283 | - |
| HCM Lane V/C Ratio | -0.215 | - |
| HCM Control Delay (s) | -21.2 | - |
| HCM Lane LOS | - | C |
| HCM 95th \%tile Q(veh) | - | - |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
| Lal**enfigurations | 「 |
| Traffic Volume (vph) | 124 |
| Future Volume (vph) | 124 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 127 |
| Lane Group Flow (vph) | 135 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 55.0 |
| Total Split (\%) | 45.8\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 55.8 |
| Actuated g/C Ratio | 0.46 |
| v/c Ratio | 0.17 |
| Control Delay | 4.9 |
| Queue Delay | 0.0 |
| Total Delay | 4.9 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 3 |
| Queue Length 95th (tt) | 43 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (tt) | 265 |
| Base Capacity (vph) | 804 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.17 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.75
Intersection Signal Delay: 33.5 Intersection LOS: C

Intersection Capacity Utilization 83.3\% ICU Level of Service E Analysis Period (min) 15




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 0.6 | 0 | 11.3 |

HCM LOS B

| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | *1306 | - | - | -649 | - |
| HCM Lane V/C Ratio | 0.038 | - | - | -0.122 | - |
| HCM Control Delay (s) | 7.9 | - | - | - | 11.3 |
| HCM Lane LOS | A | - | - | - | B |
| HCM A5th \%tile Q(veh) | 0.1 | - | - | - | 0.4 |
| HCM |  |  |  |  |  |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon


| Major/Minor | Minor2 |  | Minor1 |  |  | Major1 |  |  | Major2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 218 | 217 | 78 | 214 | 209 | 93 | 78 | 0 | 0 | 101 | 0 | 0 |  |
| Stage 1 | 94 | 94 | - | 115 | 115 | - | - | - |  |  | - | - |  |
| Stage 2 | 124 | 123 | - | 99 | 94 | - |  | - | - |  | - | - |  |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | 4.12 | - |  | 4.12 | - | - |  |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - | - | - |  | - | - |  |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | 2.218 | - |  | 2.218 | - | - |  |
| Pot Cap-1 Maneuver | 738 | 681 | 983 | 743 | 688 | 964 | 1520 | - |  | 1491 | - | - |  |
| Stage 1 | 913 | 817 | - | 890 | 800 | - |  | - |  |  | - | - |  |
| Stage 2 | 880 | 794 | - | 907 | 817 | - | - | - |  |  | - | - |  |
| Platoon blocked, \% |  |  |  |  |  |  |  | - | - |  | - | - |  |
| Mov Cap-1 Maneuver | 724 | 673 | 983 | 729 | 680 | 964 | 1520 | - |  | 1491 | - | - |  |
| Mov Cap-2 Maneuver | 724 | 673 | - | 729 | 680 | - | - | - |  |  | - | - |  |
| Stage 1 | 907 | 813 | - | 884 | 794 |  |  | - |  |  | - | - |  |
| Stage 2 | 866 | 788 | - | 893 | 813 | - | - | - |  |  | - | - |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 8.7 |  |  | 10.3 |  |  | 0.7 |  |  | 0.7 |  |  |  |
| HCM LOS | A |  |  | B |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvm |  | NBL | NBT | NBR | EBLn1 | EBLn2 | EBLn3 | WBLn1 | WBLn2 | SBL | SBT | SBR |  |
| Capacity (veh/h) |  | 1520 | - |  | - | - | 983 | 729 | 964 | 1491 | - | - |  |
| HCM Lane V/C Ratio |  | 0.007 | - | - | - | - | 0.01 | 0.095 | 0.009 | 0.005 | - | - |  |
| HCM Control Delay (s) |  | 7.4 | - |  | 0 | 0 | 8.7 | 10.5 | 8.8 | 7.4 | - | - |  |
| HCM Lane LOS |  | A | - | - | A | A | A | B | A | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | - | 0 | 0.3 | 0 | 0 | - | - |  |





| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- |
| Conflicting Flow All | - | 760 | - | 0 | - | 0 |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| $\quad$ Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.94 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | - | 3.32 | - | - | - | - |
| Pot Cap-1 Maneuver | 0 | 349 | 0 | - | - | 0 |
| $\quad$ Stage 1 | 0 | - | 0 | - | - | 0 |
| $\quad$ Stage 2 | 0 | - | 0 | - | - | 0 |
| Platoon blocked, \% |  |  |  | - | - |  |
| Mov Cap-1 Maneuver | - | 349 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 16.1 | 0 | 0 |
| HCM LOS | C |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -349 | - |
| HCM Lane V/C Ratio | -0.069 | - |
| HCM Control Delay (s) | -16.1 | - |
| HCM Lane LOS | - | C |
| HCM 95th \%tile Q(veh) | - | - |



| Major/Minor | Minor1 | Major1 | Major2 |  |  |  |
| :--- | ---: | ---: | ---: | ---: | :--- | :--- |
| Conflicting Flow All | - | 103 | 0 | 0 | - | - |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | - | 3.318 | - | - | - | - |
| Pot Cap-1 Maneuver | 0 | 952 | - | - | 0 | - |
| $\quad$ Stage 1 | 0 | - | - | - | 0 | - |
| Stage 2 | 0 | - | - | - | 0 | - |
| Platoon blocked, \% |  | - | - |  | - |  |
| Mov Cap-1 Maneuver | - | 952 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 8.8 | 0 | 0 |
| HCM LOS | A |  |  |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBT |
| :--- | ---: | ---: | ---: |
| Capacity (veh/h) | - | -952 | - |
| HCM Lane V/C Ratio | - | -0.009 | - |
| HCM Control Delay (s) | - | -8.8 | - |
| HCM Lane LOS | - | - | A |
| HCM 95th \%tile Q(veh) | - | - | 0 |


|  | 4 |  |  |  |  |  | 71 | 4 | $\uparrow$ |  |  | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | ¢ $\uparrow$ | 7 | \％ | 个4 | F |  | ${ }^{\text {a }}$ | 个 $\uparrow$ | 「 | \％${ }^{1+1}$ | ¢ ${ }^{\text {¢ }}$ |
| Traffic Volume（vph） | 118 | 580 | 242 | 181 | 453 | 296 | 15 | 372 | 1147 | 115 | 255 | 637 |
| Future Volume（vph） | 118 | 580 | 242 | 181 | 453 | 296 | 15 | 372 | 1147 | 115 | 255 | 637 |
| Satd．Flow（prot） | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Flt Permitted | 0.351 |  |  | 0.166 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 654 | 3539 | 1583 | 309 | 3539 | 1583 | 0 | 3433 | 3539 | 1583 | 3433 | 3539 |
| Satd．Flow（RTOR） |  |  | 254 |  |  | 322 |  |  |  | 109 |  |  |
| Lane Group Flow（vph） | 128 | 630 | 263 | 197 | 492 | 322 | 0 | 420 | 1247 | 125 | 277 | 692 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 21.0 | 46.0 | 46.0 | 21.0 | 46.0 | 46.0 | 22.0 | 22.0 | 51.0 | 51.0 | 22.0 | 51.0 |
| Total Split（\％） | 15．0\％ | 32．9\％ | 32．9\％ | 15．0\％ | 32．9\％ | 32．9\％ | 15．7\％ | 15．7\％ | 36．4\％ | 36．4\％ | 15．7\％ | 36．4\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 45.4 | 31.1 | 31.1 | 51.2 | 34.0 | 34.0 |  | 22.3 | 53.7 | 53.7 | 16.0 | 47.4 |
| Actuated g／C Ratio | 0.32 | 0.22 | 0.22 | 0.37 | 0.24 | 0.24 |  | 0.16 | 0.38 | 0.38 | 0.11 | 0.34 |
| v／c Ratio | 0.41 | 0.80 | 0.48 | 0.73 | 0.57 | 0.51 |  | 0.77 | 0.92 | 0.19 | 0.71 | 0.58 |
| Control Delay | 32.1 | 59.4 | 8.4 | 45.8 | 49.0 | 7.1 |  | 66.6 | 53.2 | 8.6 | 69.8 | 41.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 32.1 | 59.4 | 8.4 | 45.8 | 49.0 | 7.1 |  | 66.6 | 53.2 | 8.6 | 69.8 | 41.2 |
| LOS | C | E | A | D | D | A |  | E | D | A | E | D |
| Approach Delay |  | 42.9 |  |  | 35.1 |  |  |  | 53.2 |  |  | 43.7 |
| Approach LOS |  | D |  |  | D |  |  |  | D |  |  | D |
| Queue Length 50th（ft） | 77 | 288 | 6 | 123 | 207 | 0 |  | 187 | 574 | 9 | 126 | 282 |
| Queue Length 95th（t） | 115 | 337 | 76 | 174 | 257 | 75 |  | \＃301 | \＃827 | 58 | 174 | 351 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（ t ） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 357 | 985 | 624 | 280 | 985 | 673 |  | 546 | 1357 | 674 | 428 | 1199 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.36 | 0.64 | 0.42 | 0.70 | 0.50 | 0.48 |  | 0.77 | 0.92 | 0.19 | 0.65 | 0.58 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $74(53 \%)$ ，Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


| $\downarrow$ |  |
| :---: | :---: |
| Lane Group | SBR |
| La\|*eonfigurations | F |
| Traffic Volume (vph) | 134 |
| Future Volume (vph) | 134 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 146 |
| Lane Group Flow (vph) | 146 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 51.0 |
| Total Split (\%) | 36.4\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 47.4 |
| Actuated g/C Ratio | 0.34 |
| v/c Ratio | 0.23 |
| Control Delay | 6.1 |
| Queue Delay | 0.0 |
| Total Delay | 6.1 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 0 |
| Queue Length 95th (t) | 50 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (t) | 265 |
| Base Capacity (vph) | 632 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.23 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.92
Intersection Signal Delay: $45.2 \quad$ Intersection LOS: D

Intersection Capacity Utilization 85.0\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road \& Constitution Avenue


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Conflicting Flow All | 1047 | 0 | - | 0 | 1468 |
| $\quad$ Stage 1 | - | - | - | - | 962 |
| $\quad$ Stage 2 | - | - | - | - | 506 |


| Approach | EB | WB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, $s$ | 0.5 | 0 | 11.3 |

HCM LOS B

| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | *1087 | - | - | -620 | - |
| HCM Lane V/C Ratio | 0.058 | - | - | -0.082 | - |
| HCM Control Delay (s) | 8.5 | - | - | - | 11.3 |
| HCM Lane LOS | A | - | - | - | B |
| HCM A5th \%tile Q(veh) | 0.2 | - | - | - | 0.3 |
| HC | - |  |  |  |  |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad *:$ All major volume in platoon



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL |  |
| Lane Configurations | * |  | $\uparrow$ |  | $\dagger$ | 4 |
| Traffic Vol, veh/h | 11 | 5 | 82 | 5 | 26 | 87 |
| Future Vol, veh/h | 11 | 5 | 82 | 5 | 26 | 87 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 150 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 5 | 89 | 5 | 28 | 95 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 243 | 92 | 0 | 0 | 94 | 0 |
| Stage 1 | 92 | - |  | - | - | - |
| Stage 2 | 151 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 745 | 965 | - | - | 1500 | - |
| Stage 1 | 932 | - | - | - | - | - |
| Stage 2 | 877 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 731 | 965 | - | - | 1500 | - |
| Mov Cap-2 Maneuver | 739 | - | - | - | - | - |
| Stage 1 | 932 | - | - | - | - | - |
| Stage 2 | 860 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 9.6 |  | 0 |  | 1.7 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - |  | 797 | 1500 | - |
| HCM Lane V/C Ratio |  | - | - | 0.022 | 0.019 | - |
| HCM Control Delay (s) |  | - | - | 9.6 | 7.4 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - |  | 0.1 | 0.1 | - |



| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- |
| Conflicting Flow All | - | 550 | - | 0 | - | 0 |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| $\quad$ Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.94 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | - | 3.32 | - | - | - | - |
| Pot Cap-1 Maneuver | 0 | 479 | 0 | - | - | 0 |
| $\quad$ Stage 1 | 0 | - | 0 | - | - | 0 |
| $\quad$ Stage 2 | 0 | - | 0 | - | - | 0 |
| Platoon blocked, \% |  |  |  | - | - |  |
| Mov Cap-1 Maneuver | - | 479 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 13.1 | 0 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -479 | - |
| HCM Lane V/C Ratio | -0.073 | - |
| HCM Control Delay (s) | -13.1 | - |
| HCM Lane LOS | - | $B$ |
| HCM 95th \%tile Q(veh) | - | - |



| Major/Minor | Minor1 | Major1 | Major2 |  |  |  |
| :--- | ---: | ---: | ---: | ---: | :--- | :--- |
| Conflicting Flow All | - | 103 | 0 | 0 | - | - |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | - | 3.318 | - | - | - | - |
| Pot Cap-1 Maneuver | 0 | 952 | - | - | 0 | - |
| $\quad$ Stage 1 | 0 | - | - | - | 0 | - |
| Stage 2 | 0 | - | - | - | 0 | - |
| Platoon blocked, \% |  | - | - |  | - |  |
| Mov Cap-1 Maneuver | - | 952 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 8.8 | 0 | 0 |
| HCM LOS | A |  |  |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBT |
| :--- | ---: | ---: | ---: |
| Capacity (veh/h) | - | -952 | - |
| HCM Lane V/C Ratio | - | -0.006 | - |
| HCM Control Delay (s) | - | -8.8 | - |
| HCM Lane LOS | - | - | A |
| HCM 95th \%tile Q(veh) | - | - | 0 |


|  | $\rangle$ |  |  |  |  |  | $\dagger$ | 4 | $\dagger$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | 种 | 「 | ${ }^{7}$ | 恌 | 「 |  | $\mathbf{H}^{*}$ | 恌 | F | \％${ }^{1+1}$ | ¢4ヶ |
| Traffic Volume（vph） | 190 | 477 | 415 | 186 | 496 | 265 | 36 | 286 | 794 | 118 | 199 | 1632 |
| Future Volume（vph） | 190 | 477 | 415 | 186 | 496 | 265 | 36 | 286 | 794 | 118 | 199 | 1632 |
| Satd．Flow（prot） | 1770 | 5085 | 1583 | 1770 | 5085 | 1583 | 0 | 3433 | 5085 | 1583 | 3433 | 5085 |
| Flt Permitted | 0.278 |  |  | 0.450 |  |  |  | 0.950 |  |  | 0.950 |  |
| Satd．Flow（perm） | 518 | 5085 | 1583 | 838 | 5085 | 1583 | 0 | 3433 | 5085 | 1583 | 3433 | 5085 |
| Satd．Flow（RTOR） |  |  | 142 |  |  | 264 |  |  |  | 128 |  |  |
| Lane Group Flow（vph） | 207 | 518 | 451 | 202 | 539 | 288 | 0 | 350 | 863 | 128 | 216 | 1774 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | Prot | NA | Perm | Prot | NA |
| Protected Phases | 3 | 8 |  | 7 | 4 |  | 1 | 1 | 6 |  | 5 | 2 |
| Permitted Phases | 8 |  | 8 | 4 |  | 4 |  |  |  | 6 |  |  |
| Detector Phase | 3 | 8 | 8 | 7 | 4 | 4 | 1 | 1 | 6 | 6 | 5 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ s ） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 | 11.0 | 9.0 | 9.0 | 11.0 | 11.0 | 9.0 | 11.0 |
| Total Split（s） | 16.0 | 35.0 | 35.0 | 11.0 | 30.0 | 30.0 | 18.0 | 18.0 | 57.0 | 57.0 | 17.0 | 56.0 |
| Total Split（\％） | 13．3\％ | 29．2\％ | 29．2\％ | 9．2\％ | 25．0\％ | 25．0\％ | 15．0\％ | 15．0\％ | 47．5\％ | 47．5\％ | 14．2\％ | 46．7\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 |  | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lead | Lag | Lag | Lead | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 40.2 | 27.2 | 27.2 | 30.2 | 22.2 | 22.2 |  | 13.4 | 51.5 | 51.5 | 11.3 | 49.4 |
| Actuated g／C Ratio | 0.34 | 0.23 | 0.23 | 0.25 | 0.18 | 0.18 |  | 0.11 | 0.43 | 0.43 | 0.09 | 0.41 |
| v／c Ratio | 0.72 | 0.45 | 0.96 | 0.79 | 0.57 | 0.57 |  | 0.91 | 0.40 | 0.17 | 0.67 | 0.85 |
| Control Delay | 41.9 | 37.6 | 61.6 | 57.4 | 47.1 | 11.7 |  | 82.0 | 24.5 | 4.2 | 62.9 | 36.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 41.9 | 37.6 | 61.6 | 57.4 | 47.1 | 11.7 |  | 82.0 | 24.5 | 4.2 | 62.9 | 36.8 |
| LOS | D | D | E | E | D | B |  | F | C | A | E | D |
| Approach Delay |  | 47.5 |  |  | 39.2 |  |  |  | 37.6 |  |  | 36.5 |
| Approach LOS |  | D |  |  | D |  |  |  | D |  |  | D |
| Queue Length 50th（ft） | 129 | 132 | 258 | 116 | 139 | 15 |  | 140 | 169 | 0 | 84 | 450 |
| Queue Length 95th（ft） | \＃211 | 171 | \＃396 | \＃212 | 179 | 97 |  | \＃233 | 206 | 37 | 125 | 517 |
| Internal Link Dist（tt） |  | 1022 |  |  | 405 |  |  |  | 707 |  |  | 1957 |
| Turn Bay Length（ t ） | 435 |  | 200 | 225 |  | 235 |  | 425 |  | 325 | 670 |  |
| Base Capacity（vph） | 288 | 1186 | 478 | 257 | 974 | 516 |  | 383 | 2181 | 752 | 343 | 2094 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.72 | 0.44 | 0.94 | 0.79 | 0.55 | 0.56 |  | 0.91 | 0.40 | 0.17 | 0.63 | 0.85 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 85 （71\％），Referenced to phase 2：SBT and 6：NBT，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
| L性象onfigurations | F |
| Traffic Volume (vph) | 189 |
| Future Volume (vph) | 189 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 176 |
| Lane Group Flow (vph) | 205 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial (s) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 56.0 |
| Total Split (\%) | 46.7\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Efftt Green (s) | 49.4 |
| Actuated g/C Ratio | 0.41 |
| v/c Ratio | 0.27 |
| Control Delay | 5.9 |
| Queue Delay | 0.0 |
| Total Delay | 5.9 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th ( t ) | 13 |
| Queue Length 95th ( t ) | 61 |
| Internal Link Dist (ft) |  |
| Turn Bay Length (t) | 265 |
| Base Capacity (vph) | 755 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.27 |
| Intersection Summary |  |

Maximum v/c Ratio: 0.96
Intersection Signal Delay: $39.5 \quad$ Intersection LOS: D

Intersection Capacity Utilization 96.7\% ICU Level of Service F
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Marksheffel Road \& Constitution Avenue


2: Constitution Avenue \& Akers Drive

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Maximum v/c Ratio: 0.57
Intersection Signal Delay: 14.2 Intersection LOS: B

Intersection Capacity Utilization 55.7\% ICU Level of Service B
Analysis Period (min) 15
$m$ Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: $\quad 2:$ Constitution Avenue \& Akers Drive




| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1 EBLn2 EBLn3WBLn1WBLn2 | SBL | SBT | SBR |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1437 | - | - | - | - | 902 | 332 | 960 | 1446 | - | - |
| HCM Lane V/C Ratio | 0.088 | - | - | - | - | 0.133 | 0.21 | 0.009 | 0.005 | - | - |
| HCM Control Delay (s) | 7.7 | - | - | 0 | 0 | 9.6 | 18.7 | 8.8 | 7.5 | - | - |
| HCM Lane LOS | A | - | - | $A$ | $A$ | A | C | A | A | - | - |
| HCM 95th \%tile Q(veh) | 0.3 | - | - | - | - | 0.5 | 0.8 | 0 | 0 | - | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1 EBLn2 | EBLn3WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1464 | - | - | 657 | 611 | 934 | 624 | 622 | 1001 | 1474 | - |
| HCM Lane V/C Ratio | 0.01 | - | - | 0.04 | 0.025 | 0.034 | 0.028 | 0.009 | 0.01 | 0.013 | - |
| HCM Control Delay (s) | 7.5 | - | - | 10.7 | 11 | 9 | 10.9 | 10.8 | 8.6 | 7.5 | - |
| HCM Lane LOS | A | - | - | B | B | A | B | B | A | A | - |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0.1 | 0.1 | 0.1 | 0.1 | 0 | 0 | 0 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined $\quad$ : All major volume in platoon



| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 32.3 | 0 | 0 |
| HCM LOS | D |  |  |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -181 | - |
| HCM Lane V/C Ratio | -0.276 | - |
| HCM Control Delay (s) | -32.3 | - |
| HCM Lane LOS | - | $D$ |
| HCM 95th \%tile Q(veh) | - | 1.1 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{F}$ |  | A |
| Traffic Vol, veh/h | 0 | 8 | 254 | 14 | 0 | 311 |
| Future Vol, veh/h | 0 | 8 | 254 | 14 | 0 | 311 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | 120 | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 9 | 276 | 15 | 0 | 338 |


| Major/Minor | Minor1 | Major1 |  | Major2 |  |  |
| :--- | ---: | ---: | ---: | ---: | :--- | :--- |
| Conflicting Flow All | - | 276 | 0 | 0 | - | - |
| $\quad$ Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |
| Follow-up Hdwy | -3.318 | - | - | - | - |  |
| Pot Cap-1 Maneuver | 0 | 874 | - | - | 0 | - |
| $\quad$ Stage 1 | 0 | - | - | - | 0 | - |
| Stage 2 | 0 | - | - | - | 0 | - |
| Platoon blocked, \% |  | 1 | - | - |  | - |
| Mov Cap-1 Maneuver | - | 874 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |


| Approach | WB | NB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 9.2 | 0 | 0 |
| HCM LOS | A |  |  |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBT |  |
| :--- | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | - | -874 | - |  |
| HCM Lane V/C Ratio | - | - | 0.01 | - |
| HCM Control Delay (s) | - | - | 9.2 | - |
| HCM Lane LOS | - | - | A | - |
| HCM 95th \%tile Q(veh) | - | - | 0 | - |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\downarrow$ |
| :---: | :---: |
| Lane Group | SBR |
|  | F |
| Traffic Volume (vph) | 231 |
| Future Volume (vph) | 231 |
| Satd. Flow (prot) | 1583 |
| Flt Permitted |  |
| Satd. Flow (perm) | 1583 |
| Satd. Flow (RTOR) | 224 |
| Lane Group Flow (vph) | 251 |
| Turn Type | Perm |
| Protected Phases |  |
| Permitted Phases | 2 |
| Detector Phase | 2 |
| Switch Phase |  |
| Minimum Initial ( $s$ ) | 4.0 |
| Minimum Split (s) | 11.0 |
| Total Split (s) | 40.0 |
| Total Split (\%) | 33.3\% |
| Yellow Time (s) | 5.0 |
| All-Red Time (s) | 2.0 |
| Lost Time Adjust (s) | 0.0 |
| Total Lost Time (s) | 7.0 |
| Lead/Lag | Lag |
| Lead-Lag Optimize? | Yes |
| Recall Mode | C-Max |
| Act Effct Green (s) | 33.7 |
| Actuated g/C Ratio | 0.28 |
| v/c Ratio | 0.41 |
| Control Delay | 8.6 |
| Queue Delay | 0.0 |
| Total Delay | 8.6 |
| LOS | A |
| Approach Delay |  |
| Approach LOS |  |
| Queue Length 50th (tt) | 16 |
| Queue Length 95th (t) | 83 |
| Internal Link Dist (tt) |  |
| Turn Bay Length (t) | 265 |
| Base Capacity (vph) | 605 |
| Starvation Cap Reductn | 0 |
| Spillback Cap Reductn | 0 |
| Storage Cap Reductn | 0 |
| Reduced v/c Ratio | 0.41 |
| Intersection Summary |  |

Maximum v/c Ratio: 1.11
Intersection Signal Delay: $53.8 \quad$ Intersection LOS: D

Intersection Capacity Utilization 95.7\% ICU Level of Service F
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 1: Marksheffel Road \& Constitution Avenue


2：Constitution Avenue \＆Akers Drive

|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 |  |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个中4 | F | ${ }^{7}$ | 恌 | F | \％＊ |  |  | ${ }^{7 *}$ | F |  |
| Traffic Volume（vph） | 270 | 1264 | 123 | 24 | 1380 | 190 | 128 | 51 | 172 | 356 | 33 | 153 |
| Future Volume（vph） | 270 | 1264 | 123 | 24 | 1380 | 190 | 128 | 51 | 172 | 356 | 33 | 153 |
| Satd．Flow（prot） | 1770 | 5085 | 1583 | 1770 | 5085 | 1583 | 3433 | 1647 | 0 | 3433 | 1634 | 0 |
| Flt Permitted | 0.121 |  |  | 0.146 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 225 | 5085 | 1583 | 272 | 5085 | 1583 | 3433 | 1647 | 0 | 3433 | 1634 | 0 |
| Satd．Flow（RTOR） |  |  | 134 |  |  | 207 |  | 37 |  |  | 91 |  |
| Lane Group Flow（vph） | 293 | 1374 | 134 | 26 | 1500 | 207 | 139 | 242 | 0 | 387 | 202 | 0 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Split | NA |  | Split | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  | 2 | 2 |  | 6 | 6 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 |  |  |  |  |  |  |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 2 | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Minimum Split（s） | 24.0 | 24.0 | 24.0 | 24.0 | 24.0 | 24.0 | 24.0 | 24.0 |  | 24.0 | 24.0 |  |
| Total Split（s） | 72.0 | 72.0 | 72.0 | 72.0 | 72.0 | 72.0 | 24.0 | 24.0 |  | 24.0 | 24.0 |  |
| Total Split（\％） | 60．0\％ | 60．0\％ | 60．0\％ | 60．0\％ | 60．0\％ | 60．0\％ | 20．0\％ | 20．0\％ |  | 20．0\％ | 20．0\％ |  |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time（s） | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | C－Max | C－Max | C－Max | C－Max | C－Max | C－Max | None | None |  | None | None |  |
| Act Efft Green（s） | 67.7 | 67.7 | 67.7 | 67.7 | 67.7 | 67.7 | 17.3 | 17.3 |  | 17.0 | 17.0 |  |
| Actuated g／C Ratio | 0.56 | 0.56 | 0.56 | 0.56 | 0.56 | 0.56 | 0.14 | 0.14 |  | 0.14 | 0.14 |  |
| v／c Ratio | 2.33 | 0.48 | 0.14 | 0.17 | 0.52 | 0.21 | 0.28 | 0.90 |  | 0.80 | 0.65 |  |
| Control Delay | 636.7 | 16.6 | 2.5 | 18.9 | 18.9 | 5.9 | 47.2 | 77.5 |  | 62.8 | 36.9 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 636.7 | 16.6 | 2.5 | 18.9 | 18.9 | 5.9 | 47.2 | 77.5 |  | 62.8 | 36.9 |  |
| LOS | F | B | A | B | B | A | D | E |  | E | D |  |
| Approach Delay |  | 116.4 |  |  | 17.4 |  |  | 66.4 |  |  | 53.9 |  |
| Approach LOS |  | F |  |  | B |  |  | E |  |  | D |  |
| Queue Length 50th（ft） | $\sim 280$ | 229 | 0 | 9 | 201 | 27 | 50 | 158 |  | 150 | 80 |  |
| Queue Length 95th（ft） | \＃459 | 269 | 28 | m13 | 234 | m40 | 81 | \＃306 |  | 205 | 163 |  |
| Internal Link Dist（tt） |  | 734 |  |  | 1022 |  |  | 276 |  |  | 237 |  |
| Turn Bay Length（ t ） | 275 |  | 235 | 235 |  | 275 | 120 |  |  | 355 |  |  |
| Base Capacity（vph） | 126 | 2869 | 951 | 153 | 2869 | 983 | 514 | 278 |  | 514 | 322 |  |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | ， | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 2.33 | 0.48 | 0.14 | 0.17 | 0.52 | 0.21 | 0.27 | 0.87 |  | 0.75 | 0.63 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $0(0 \%)$ ，Referenced to phase 4：EBTL and 8：WBTL，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 150 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |

Maximum v/c Ratio: 2.33
Intersection Signal Delay: $65.9 \quad$ Intersection LOS: E

Intersection Capacity Utilization 85.1\% ICU Level of Service E
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum atter two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
$m$ Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Constitution Avenue \& Akers Drive




| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1 EBL2 | EBLn3WBLn1WBLn2 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1462 | - | -176 | - | 926 | 71 | 970 | 1425 | - | - |
| HCM Lane V/C Ratio | 0.227 | - | -0.019 | -0.444 | 0.628 | 0.006 | 0.015 | - | - |  |
| HCM Control Delay (s) | 8.2 | - | - | 25.8 | 0 | 12 | 118 | 8.7 | 7.6 | - |
| HCM Lane LOS | A | - | - | D | A | B | F | A | A | - |
| HCM 95th \%tile Q(veh) | 0.9 | - | - | 0.1 | - | 2.3 | 2.8 | 0 | 0 | - |



HCM LOS B B

| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1 EBLn2 EBLn3WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1437 | - | - | 504 | 496 | 922 | 498 | 491 | 989 | 1474 | - |
| HCM Lane V/C Ratio | 0.034 | - | -0.035 | 0.02 | 0.022 | 0.033 | 0.035 | 0.009 | 0.028 | - | - |
| HCM Control Delay (s) | 7.6 | - | - | 12.4 | 12.4 | 9 | 12.5 | 12.6 | 8.7 | 7.5 | - |
| HCM Lane LOS | A | - | - | B | B | A | B | B | A | A | - |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0 | 0.1 | - |




| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, S | 21.2 | 0 | 0 |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT |
| :--- | ---: | ---: |
| Capacity (veh/h) | -282 | - |
| HCM Lane V/C Ratio | -0.216 | - |
| HCM Control Delay (s) | -21.2 | - |
| HCM Lane LOS | - | C |
| HCM 95th \%tile Q(veh) | - | - |




| Approach | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 10.2 | 0 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBT |  |
| :--- | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | - | -693 | - |  |
| HCM Lane V/C Ratio | - | -0.008 | - |  |
| HCM Control Delay (s) | - | -10.2 | - |  |
| HCM Lane LOS | - | - | $B$ | - |
| HCM 95th \%ttile Q(veh) | - | - | 0 | - |
| Notes |  |  |  |  |
| $:$ Volume exceeds capacity | $\$:$ Delay exceeds 300s | + : Computation Not Defined | $*:$ All major volume in platoon |  |

## APPENDIX D

Warrant Analysis Forms

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower
threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70\% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

## Akers Drive


*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-3. Warrant 3, Peak Hour


Figure 4C-4. Warrant 3, Peak Hour (70\% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-3. Warrant 3, Peak Hour


Figure 4C-4. Warrant 3, Peak Hour (70\% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.


[^0]:    6 SaUTH TEJロN STREET，SUITE 515 CロLロRADロ SPRINGS，LロLロRADロ Bロ9ロ3（719）2ロ3－6639 87ロ3 YATES DRIVE，SUITE 21 （WESTMINSTER，CロLロRADロ Bロロ31（3ロ3） $458-9798$ WWW．SMRロCHA．CIM

[^1]:    ${ }^{1}$ Watermark Akers Drive: Traffic Impact Study, SM ROCHA, LLC, February 2021.

[^2]:    Key: Stop-Controlled Intersection: Level of Service

[^3]:    ${ }^{1}$ Major Thoroughfare Plan, City of Colorado Springs, August 2011.
    ${ }^{2}$ Engineering Criteria Manual, Section III: Traffic Criteria Manual, City of Colorado Springs City Engineering, July 2010.
    ${ }^{3}$ El Paso County 2016 Major Transportation Corridors Plan Update, Felsburg Holt \& Ullevig, December 2016.
    ${ }^{4}$ El Paso County Engineering Criteria Manual, El Paso County, December 2016.

[^4]:    ${ }^{5}$ Web Mapping Application, City of Colorado Springs GIS, January 2019.

[^5]:    Key: Signalized Intersection: Level of Service (Control Delay in sec/veh) Stop-Controlled Intersection: Lev el of Service

[^6]:    ${ }^{6}$ The Sands, LSC Transportation Consultants, Inc., May 2016.
    ${ }^{7}$ Hannah Ridge at Feathergrass Filings 5, 6, and 7, LSC Transportation Consultants, Inc., March 2019.
    ${ }^{8}$ Hannah Ridge at Feathergrass Filing Nos. 3 and 4, LSC Transportation Consultants, Inc., September 2017.

[^7]:    Key: Signalized Intersection: Lev el of Service (Control Delay in sec/veh)
    Stop-Controlled Intersection: Lev el of Serv ice

