

December 20, 2023

El Paso County
Planning and Community Development
2880 International Circle, Suite 110
Colorado Springs, CO 80910



J-R ENGINEERING

Attn.: Mr. Brad Walters, Inspection Supervisor
RE: Solace Apartments Filing No. 1 – Pond Certifications

To whom it may concern,

This letter is intended to provide documentation with County Inspection Staff that the Pond facilities in Solace Apartments Filing No. 1 have been constructed within reasonable conformance to the design. The District owned pond facilities for Solace Apartments Filing No. 1 are described by the following:

- Pond A – north portion of Tract B (FSD Pond)
- Pond B – south portion of Tract B (FSD Pond)

JR Engineering reviewed the final constructed facilities and recently completed as-builts which confirm the appropriate size and design of the ponds.

The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

Based upon this information and information gathered during periodic site visits to the project under construction, JR Engineering is of the opinion that the stormwater BMPs have been constructed in general compliance with the approved Construction Plans, and Specifications as filed with El Paso County.

(See attached documents – UD Detention sheets and as-built drawings)

Statement Of Engineer In Responsible Charge:

To the best of my knowledge, information and belief, the referenced **Solace Apartments Filing No. 1** Pond facilities have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

Mike Bramlett, P.E.
Colorado No. 32314

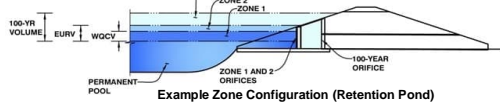


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.03 (May 2020)

Project: Solace Apartments

Basin ID: Pond A



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB
Watershed Area =	7.89 acres
Watershed Length =	790 ft
Watershed Length to Centroid =	340 ft
Watershed Slope =	0.020 ft/ft
Watershed Imperviousness =	49.43% percent
Percentage Hydrologic Soil Group A =	1.0% percent
Percentage Hydrologic Soil Group B =	99.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WOCV Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WOCV) =	0.135	acre-feet
Excess Urban Runoff Volume (EURV) =	0.417	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.382	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.546	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.691	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	0.887	acre-feet
50-yr Runoff Volume (P1 = 2.26 in.) =	1.052	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	1.247	acre-feet
500-yr Runoff Volume (P1 = 3.14 in.) =	1.654	acre-feet
Approximate 2-yr Detention Volume =	0.314	acre-feet
Approximate 5-yr Detention Volume =	0.430	acre-feet
Approximate 10-yr Detention Volume =	0.570	acre-feet
Approximate 25-yr Detention Volume =	0.626	acre-feet
Approximate 50-yr Detention Volume =	0.657	acre-feet
Approximate 100-yr Detention Volume =	0.732	acre-feet

Optional User Overrides

	acre-feet
	acre-feet
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.26	inches
2.52	inches
	inches

Define Zones and Basin Geometry

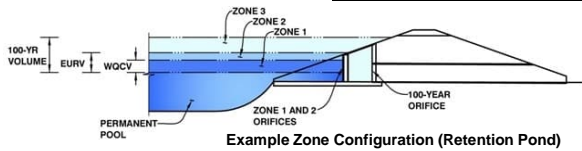
Zone 1 Volume (WOCV) =	0.135	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.282	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.315	acre-feet
Total Detention Basin Volume =	0.732	acre-feet
Initial Surcharge Volume (ISV) =	user	ft ³
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H _{total}) =	user	ft
Depth of Trickle Channel (H _{TC}) =	user	ft
Slope of Trickle Channel (S _{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S _{main}) =	user	H:V
Basin Length-to-Width Ratio (R _{L/W}) =	user	
Initial Surcharge Area (A _{ISV}) =	user	ft ²
Surcharge Volume Length (L _{ISV}) =	user	ft
Surcharge Volume Width (W _{ISV}) =	user	ft
Depth of Basin Floor (H _{FLOOR}) =	user	ft
Length of Basin Floor (L _{FLOOR}) =	user	ft
Width of Basin Floor (W _{FLOOR}) =	user	ft
Area of Basin Floor (A _{FLOOR}) =	user	ft ²
Volume of Basin Floor (V _{FLOOR}) =	user	ft ³
Depth of Main Basin (H _{MAIN}) =	user	ft
Length of Main Basin (L _{MAIN}) =	user	ft
Width of Main Basin (W _{MAIN}) =	user	ft
Area of Main Basin (A _{MAIN}) =	user	ft ²
Volume of Main Basin (V _{MAIN}) =	user	ft ³
Calculated Total Basin Volume (V _{total}) =	USER	acre-feet

Depth Increment =	ft									
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft ²)	Optional Override Area (ft ²)	Area (acre)	Volume (ft ³)	Volume (ac-ft)	
6251 Top of Micropool	--	0.00	--	--	--	10	0.000			
ELEV:6252	--	1.00	--	--	--	2,043	0.047	1,026	0.024	
ELEV:6253	--	2.00	--	--	--	5,660	0.130	4,878	0.112	
ELEV:6254	--	3.00	--	--	--	10,498	0.241	12,957	0.297	
ELEV:6255	--	4.00	--	--	--	14,961	0.343	25,687	0.590	
ELEV:6256	--	5.00	--	--	--	18,037	0.414	42,186	0.968	
ELEV:6257	--	6.00	--	--	--	20,826	0.478	61,617	1.415	

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention*, Version 4.03 (May 2020)

Project: Solace Apartments
Basin ID: Pond A



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WOCV)	2.17	0.135	Orifice Plate
Zone 2 (EURV)	3.46	0.282	Circular Orifice
Zone 3 (100-year)	4.40	0.315	Weir&Pipe (Restrict)
Total (all zones)		0.732	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = sq. inches (diameter = 3/4 inch)

Calculated Parameters for Plate
WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.55	1.40	2.10				
Orifice Area (sq. inches)	0.45	0.45	0.45	0.45				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	2.49	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	3.77	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	0.38	N/A	inches

Calculated Parameters for Vertical Orifice

	Zone 2 Circular	Not Selected	
Vertical Orifice Area =	0.00	N/A	ft ²
Vertical Orifice Centroid =	0.02	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.94	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	4.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	3.00	N/A	feet
Overflow Gate Open Area % =	70%	N/A	%, gate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Gate Upper Edge, H _i =	3.94	N/A	feet
Overflow Weir Slope Length =	3.00	N/A	feet
Gate Open Area / 100-yr Orifice Area =	28.73	N/A	
Overflow Gate Open Area w/o Debris =	8.40	N/A	ft ²
Overflow Gate Open Area w/ Debris =	4.20	N/A	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	4.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.29	N/A	ft ²
Outlet Orifice Centroid =	0.20	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	0.98	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = feet
Spillway End Slopes = H:V
Freeboard above Max Water Surface = feet

Calculated Parameters for Spillway

Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres
Basin Volume at Top of Freeboard = acre-ft

Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

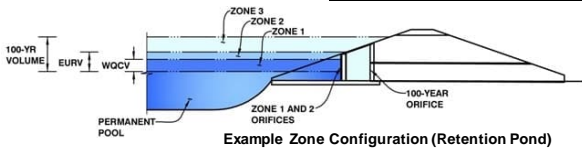
	WOCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period	N/A	N/A	1.19	1.50	1.75	2.00	2.26	2.52	3.14
One-Hour Rainfall Depth (in)	N/A	N/A	1.19	1.50	1.75	2.00	2.26	2.52	3.14
CUHP Runoff Volume (acre-ft)	0.135	0.417	0.382	0.546	0.691	0.887	1.052	1.247	1.654
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.382	0.546	0.691	0.887	1.052	1.247	1.654
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.9	2.7	4.0	7.2	9.1	11.2	15.7
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.12	0.34	0.51	0.91	1.15	1.42	1.99
Peak Inflow Q (cfs)	N/A	N/A	6.7	9.8	12.0	15.6	18.5	22.1	28.9
Peak Outflow Q (cfs)	0.1	0.1	0.1	0.1	1.3	2.8	3.0	3.1	11.6
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.0	0.3	0.4	0.3	0.3	0.7
Structure Controlling Flow	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	0.1	0.3	0.3	0.4	0.4
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	47	82	79	96	99	97	95	93	89
Time to Drain 99% of Inflow Volume (hours)	51	90	86	104	108	106	106	105	103
Maximum Ponding Depth (ft)	2.17	3.46	3.27	3.82	4.06	4.29	4.60	5.03	5.45
Area at Maximum Ponding Depth (acres)	0.15	0.29	0.27	0.32	0.35	0.36	0.39	0.42	0.44
Maximum Volume Stored (acre-ft)	0.136	0.419	0.364	0.526	0.607	0.692	0.808	0.977	1.157

The drain time exceeds the allowable limits for the 2-year and 5-year. The pond must be within allowable ranges for drain time.

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention, Version 4.03 (May 2020)*

Project: Solace Apartments
Basin ID: Pond B



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.86	0.264	Orifice Plate
Zone 2 (EURV)	2.87	0.482	Circular Orifice
Zone 3 (100-year)	3.91	0.666	Weir & Pipe (Restrict)
Total (all zones)		1.412	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft ²
Depth at top of Zone using Orifice Plate =	2.60	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	inches	Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.50	1.04	1.56	2.29			
Orifice Area (sq. inches)	0.56	0.56	0.56	0.52	0.52			

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected		Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	2.53	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.01	N/A
Depth at top of Zone using Vertical Orifice =	3.63	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.06	N/A
Vertical Orifice Diameter =	1.50	N/A	inches			

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

	Zone 3 Weir	Not Selected		Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.77	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Gate Upper Edge, Hi =	3.77	N/A
Overflow Weir Front Edge Length =	4.00	N/A	feet	Overflow Weir Slope Length =	3.00	N/A
Overflow Weir Gate Slope =	0.00	N/A	H:V	Gate Open Area / 100-yr Orifice Area =	28.73	N/A
Horiz. Length of Weir Sides =	3.00	N/A	feet	Overflow Gate Open Area w/o Debris =	8.40	N/A
Overflow Gate Open Area % =	70%	N/A	% , grate open area/total area	Overflow Gate Open Area w/ Debris =	4.20	N/A
Debris Clogging % =	50%	N/A	%			

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected		Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	0.29	N/A
Outlet Pipe Diameter =	18.00	N/A	inches	Outlet Orifice Centroid =	0.20	N/A
Restrictor Plate Height Above Pipe Invert =	4.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	0.98	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

			Calculated Parameters for Spillway
Spillway Invert Stage =	6.42	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth =
Spillway Crest Length =	50.00	feet	Stage at Top of Freeboard =
Spillway End Slopes =	10.00	H:V	Basin Area at Top of Freeboard =
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =

Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period	N/A	N/A	1.19	1.50	1.75	2.00	2.26	2.52	3.14
One-Hour Rainfall Depth (in)	N/A	N/A	1.19	1.50	1.75	2.00	2.26	2.52	3.14
CUHP Runoff Volume (acre-ft)	0.264	0.746	0.729	1.088	1.408	1.872	2.246	2.702	3.634
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.729	1.088	1.408	1.872	2.246	2.702	3.634
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	1.4	4.0	6.1	11.3	14.3	18.2	25.4
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.08	0.23	0.35	0.64	0.82	1.04	1.45
Peak Inflow Q (cfs)	N/A	N/A	8.4	12.8	16.1	23.1	27.6	32.7	43.5
Peak Outflow Q (cfs)	0.1	0.1	0.1	0.2	0.6	2.8	3.0	3.1	3.4
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.0	0.1	0.2	0.2	0.2	0.1
Structure Controlling Flow	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	0.0	0.3	0.3	0.3	0.4
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	68	118	117	>120	>120	>120	>120	>120	>120
Time to Drain 99% of Inflow Volume (hours)	74	>120	>120	>120	>120	>120	>120	>120	>120
Maximum Ponding Depth (ft)	1.86	2.87	2.78	3.37	4.20	4.20	4.60	5.12	6.12
Area at Maximum Ponding Depth (acres)	0.39	0.57	0.56	0.63	0.69	0.72	0.76	0.81	0.90
Maximum Volume Stored (acre-ft)	0.266	0.751	0.700	1.046	1.350	1.611	1.916	2.324	3.170

The drain time exceeds the allowable limits for the 2-year and 5-year. The pond must be within allowable ranges for drain time.

December 20, 2023

El Paso County
Development Services Division
2880 International Circle
Colorado Springs, CO 80910



RE: **Solace Apartments Filing 1 – Public Storm Sewer**

The public storm drainage facilities for Solace Apartments Filing No. 1 consist of the following facilities:

- | | | |
|------------------------|------------|---------------------|
| - (5) 10' Type R Inlet | - (117 LF) | 18" RCP Storm Drain |
| - (3) 15' Type R Inlet | - (44 LF) | 24" RCP Storm Drain |
| - (2) Storm Manholes | - (187 LF) | 36" RCP Storm Drain |
| | - (33 LF) | 42" RCP Storm Drain |

The above listed storm system was recently installed by CS Powers and Galley, LLC and per the approved drainage report, the storm system drains to Ponds A and B which are subject to a separate certification.

The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

Based upon information gathered during periodic site visits to the project and the as-built drawings, JR Engineering is of the opinion that the storm drainage facilities have been constructed in general compliance with the approved design plans and specifications as filed with the County.

On behalf of CS Powers and Galley, LLC, JR Engineering hereby requests probationary inspection of these facilities by County Engineering so that the warranty period can begin.

Statement Of Engineer In Responsible Charge:

To the best of my knowledge, information and belief, the referenced public storm drainage facilities have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

Respectfully submitted,

JR ENGINEERING, LLC

A handwritten signature in blue ink that reads "Mike Bramlett".

Mike Bramlett, PE
Colorado No. 32314



December 20, 2023



El Paso County
Development Services Division
2880 International Circle
Colorado Springs, CO 80910

RE: Solace Apartments Filing No. 1 – Paonia Street Improvements

The public Paonia Street improvements associated with Solace Apartments Filing 1 consist of the paving, curb and gutter, cross pans, sidewalk, pedestrian ramps and driveways to Solace Pond View and Tranquil Glen Grove which have been recently installed by CS Powers and Galley, LLC.

The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

Based upon information gathered during periodic site visits during the installation of the street improvements and the as-built drawings, JR Engineering is of the opinion that the street improvements have been constructed in general compliance with the approved design plans and specifications as filed with the County.

On behalf of CS Powers and Galley, LLC, JR Engineering hereby requests probationary inspection of these facilities by County Engineering so that the warranty period can begin.

STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

To the best of my knowledge, information and belief, the referenced public street improvements have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

Respectfully submitted,

JR ENGINEERING, LLC

A handwritten signature in black ink that reads "Mike Bramlett".

Mike Bramlett, PE
Colorado No. 32314

