EPC STORMWATER REVIEW COMMENTS
IN ORANGE BOXES WITH BLACK TEXT

EROSION CONTROL NOTES:

1. STORMWATER DISCHARGES FROM CONSTRUCTION SITES SHALL NOT CAUSE OR THREATEN TO CAUSE POLLUTION, CONTAMINATION, OR DEGRADATION OF STATE WATERS. ALL WORK AND EARTH DISTURBANCE SHALL BE DONE IN A MANNER THAT MINIMIZES POLLUTION OF ANY ON-SITE OR OFF-SITE WATERS, INCLUDING WETLANDS.

2. NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION, ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE, AND EROSION CONTROL SHALL CONFORM TO THE STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL, AND THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS FROM REGULATIONS AND STANDARDS MUST BE REQUESTED, AND APPROVED, IN WRITING.

3. A SEPARATE STORMWATER MANAGEMENT PLAN (SWMP) FOR THIS PROJECT SHALL BE COMPLETED AND AN EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP) ISSUED PRIOR TO COMMENCING CONSTRUCTION. MANAGEMENT OF THE SWMP DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE DESIGNATED QUALIFIED STORMWATER MANAGER OR CERTIFIED EROSION CONTROL INSPECTOR. THE SWMP SHALL BE LOCATED ON SITE AT ALL TIMES DURING CONSTRUCTION AND SHALL BE KEPT UP TO DATE WITH WORK PROGRESS AND CHANGES IN THE FIELD.

4. ONCE THE ESQCP IS APPROVED AND A "NOTICE TO PROCEED" HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL MEASURES AS INDICATED ON THE APPROVED GEC. A PRECONSTRUCTION MEETING BETWEEN THE CONTRACTOR, ENGINEER, AND EL PASO COUNTY WILL BE HELD PRIOR TO ANY CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE APPLICANT TO COORDINATE THE MEETING TIME AND PLACE WITH COUNTY STAFF.

5. CONTROL MEASURES MUST BE INSTALLED PRIOR TO COMMENCEMENT OF ACTIVITIES THAT COULD CONTRIBUTE POLLUTANTS TO STORMWATER. CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, AND DISTURBED LAND AREAS SHALL BE INSTALLED IMMEDIATELY UPON COMPLETION OF THE DISTURBANCE.

6. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE MAINTAINED AND REMAIN IN EFFECTIVE OPERATING CONDITION UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED AND FINAL STABILIZATION IS ESTABLISHED. ALL PERSONS ENGAGED IN LAND DISTURBANCE ACTIVITIES SHALL ASSESS THE ADEQUACY OF CONTROL MEASURES AT THE SITE AND IDENTIFY IF CHANGES TO THOSE CONTROL MEASURES ARE NEEDED TO ENSURE THE CONTINUED EFFECTIVE PERFORMANCE OF THE CONTROL MEASURES. ALL CHANGES TO TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES MUST BE INCORPORATED INTO THE STORMWATER MANAGEMENT PLAN.

7. TEMPORARY STABILIZATION SHALL BE IMPLEMENTED ON DISTURBED AREAS AND STOCKPILES WHERE GROUND-DISTURBING CONSTRUCTION ACTIVITY HAS PERMANENTLY CEASED OR TEMPORARILY CEASED FOR LONGER THAN 14 DAYS.

8. FINAL STABILIZATION MUST BE IMPLEMENTED AT ALL APPLICABLE CONSTRUCTION SITES. FINAL STABILIZATION IS ACHIEVED WHEN ALL GROUND-DISTURBING ACTIVITIES ARE COMPLETE AND ALL DISTURBED AREAS EITHER HAVE A UNIFORM VEGETATIVE COVER WITH INDIVIDUAL PLANT DENSITY OF 70 PERCENT OF PRE-DISTURBANCE LEVELS ESTABLISHED OR EQUIVALENT PERMANENT ALTERNATIVE STABILIZATION METHOD IS IMPLEMENTED. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE REMOVED UPON FINAL STABILIZATION AND BEFORE PERMIT CLOSURE.

9. ALL PERMANENT STORMWATER MANAGEMENT FACILITIES SHALL BE INSTALLED AS DESIGNED IN THE APPROVED PLANS. ANY PROPOSED CHANGES THAT EFFECT THE DESIGN OR FUNCTION OF PERMANENT STORMWATER MANAGEMENT STRUCTURES MUST BE APPROVED BY THE ECM ADMINISTRATOR PRIOR TO IMPLEMENTATION.

10. EARTH DISTURBANCES SHALL BE CONDUCTED IN SUCH A MANNER SO AS TO EFFECTIVELY MINIMIZE ACCELERATED SOIL EROSION AND RESULTING SEDIMENTATION. ALL DISTURBANCES SHALL BE DESIGNED, CONSTRUCTED, AND COMPLETED SO THAT THE EXPOSED AREA OF ANY DISTURBED LAND SHALL BE LIMITED TO THE SHORTEST PRACTICAL PERIOD OF TIME. PRE-EXISTING VEGETATION SHALL BE PROTECTED AND MAINTAINED WITHIN 50 HORIZONTAL FEET OF A WATERS OF THE STATE UNLESS SHOWN TO BE INFEASIBLE AND SPECIFICALLY REQUESTED AND APPROVED.

11. COMPACTION OF SOIL MUST BE PREVENTED IN AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES OR WHERE FINAL STABILIZATION WILL BE ACHIEVED BY VEGETATIVE COVER. AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES SHALL ALSO BE PROTECTED FROM SEDIMENTATION DURING CONSTRUCTION UNTIL FINAL STABILIZATION IS ACHIEVED. IF COMPACTION PREVENTION IS NOT FEASIBLE DUE TO SITE CONSTRAINTS, ALL AREAS DESIGNATED FOR INFILTRATION AND VEGETATION CONTROL MEASURES MUST BE LOOSENED PRIOR TO INSTALLATION OF THE CONTROL MEASURE(S).

12. ANY TEMPORARY OR PERMANENT FACILITY DESIGNED AND CONSTRUCTED FOR THE CONVEYANCE OF STORMWATER AROUND, THROUGH, OR FROM THE EARTH DISTURBANCE AREA SHALL BE A STABILIZED CONVEYANCE DESIGNED TO MINIMIZE EROSION AND THE DISCHARGE OF SEDIMENT OFF SITE.

13. CONCRETE WASH WATER SHALL BE CONTAINED AND DISPOSED OF IN ACCORDANCE WITH THE SWMP. NO WASH WATER SHALL BE DISCHARGE TO OR ALLOWED TO ENTER STATE WATERS, INCLUDING ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR FACILITIES. CONCRETE WASHOUTS SHALL NOT BE LOCATED IN AN AREA WHERE SHALLOW GROUNDWATER MAY BE PRESENT, OR WITHIN 50 FEET OF A SURFACE WATER BODY, CREEK, OR STREAM.

14. DURING DEWATERING OPERATIONS OF UNCONTAMINATED GROUND WATER MAY BE DISCHARGED ON SITE, BUT SHALL NOT LEAVE THE SITE IN THE FORM OF SURFACE RUNOFF UNLESS AN APPROVED STATE DEWATERING PERMIT IS IN PLACE.

15. EROSION CONTROL BLANKETING OR OTHER PROTECTIVE COVERING SHALL BE USED ON SLOPES STEEPER THAN 3:1.

16. CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL WASTES FROM THE CONSTRUCTION SITE FOR DISPOSAL IN ACCORDANCE WITH LOCAL AND STATE REGULATORY REQUIREMENTS. NO CONSTRUCTION DEBRIS, TREE SLASH, BUILDING MATERIAL WASTES, OR UNUSED BUILDING MATERIALS SHALL BE BURIED, DUMPED, OR DISCHARGED AT THE SITE.

17. WASTE MATERIALS SHALL NOT BE TEMPORARILY PLACED OR STORED IN THE STREET, ALLEY, OR OTHER PUBLIC WAY, UNLESS IN ACCORDANCE WITH AN APPROVED TRAFFIC CONTROL PLAN. CONTROL MEASURES MAY BE REQUIRED BY EL PASO COUNTY ENGINEERING IF DEEMED NECESSARY, BASED ON SPECIFIC CONDITIONS AND CIRCUMSTANCES.

18. TRACKING OF SOILS AND CONSTRUCTION DEBRIS OFF SITE SHALL BE MINIMIZED. MATERIALS TRACKED OFF SITE SHALL BE CLEANED UP AND PROPERLY DISPOSED OF IMMEDIATELY.

19. THE OWNER/DEVELOPER SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL CONSTRUCTION DEBRIS, DIRT, TRASH, ROCK, SEDIMENT, SOIL, AND SAND THAT MAY ACCUMULATE IN ROADS, STORM DRAINS, AND OTHER DRAINAGE CONVEYANCE SYSTEMS AND STORMWATER APPURTENANCES AS A RESULT OF SITE DEVELOPMENT.

20. THE QUANTITY OF MATERIALS STORED ON THE PROJECT SITE SHALL BE LIMITED, AS MUCH AS PRACTICAL, TO THAT QUANTITY REQUIRED TO PERFORM THE WORK IN AN ORDERLY SEQUENCE. ALL MATERIALS STORED ON SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER, IN THEIR ORIGINAL CONTAINERS, AND WITH ORIGINAL MANUFACTURER'S LABELS.

21. NO CHEMICAL(S) HAVING THE POTENTIAL TO BE RELEASED IN STORMWATER ARE TO BE STORED OR USED ON SITE UNLESS PERMISSION FOR THE USE OF SUCH CHEMICAL(S) IS GRANTED IN WRITING BY THE ECM ADMINISTRATOR. IN GRANTING APPROVAL FOR THE USE OF SUCH CHEMICAL(S), SPECIAL CONDITIONS AND MONITORING MAY BE REQUIRED.

22. BULK STORAGE OF ALLOWED PETROLEUM PRODUCTS OR OTHER ALLOWED LIQUID CHEMICALS IN EXCESS OF 55 GALLONS SHALL REQUIRE ADEQUATE SECONDARY CONTAINMENT PROTECTION TO CONTAIN ALL SPILLS ON SITE AND TO PREVENT ANY SPILLED MATERIALS FROM ENTERING STATE WATERS, ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM, OR OTHER FACILITIES.

23. NO PERSON SHALL CAUSE THE IMPEDIMENT OF STORMWATER FLOW IN THE CURB AND GUTTER OR DITCH EXCEPT WITH APPROVED SEDIMENT CONTROL MEASURES.

24. OWNER/DEVELOPER AND THEIR AGENTS SHALL COMPLY WITH THE "COLORADO WATER QUALITY CONTROL ACT" (TITLE 25, ARTICLE 8, CRS) AND THE "CLEAN WATER ACT" (33 USC 1344), IN ADDITION TO THE REQUIREMENTS OF THE LAND DEVELOPMENT CODE, DCM VOLUME II AND THE ECM APPENDIX I. ALL APPROPRIATE PERMITS MUST BE OBTAINED BY THE CONTRACTOR PRIOR TO CONSTRUCTION (1041, NPDES, FLOODPLAIN, 404, FUGITIVE DUST, ETC.). IN THE EVENT OF CONFLICTS BETWEEN THESE REQUIREMENTS AND OTHER LAWS, RULES, OR REGULATIONS OF OTHER FEDERAL, STATE, LOCAL, OR COUNTY AGENCIES, THE MOST RESTRICTIVE LAWS, RULES, OR REGULATIONS SHALL APPLY.

25. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE ONLY AT APPROVED CONSTRUCTION ACCESS POINTS.

26. PRIOR TO CONSTRUCTION, THE PERMITTEE SHALL VERIFY THE LOCATION OF EXISTING UTILITIES.

27. A WATER SOURCE SHALL BE AVAILABLE ON SITE DURING EARTHWORK OPERATIONS AND SHALL BE UTILIZED AS REQUIRED TO MINIMIZE DUST FROM EARTHWORK EQUIPMENT AND WIND.

28. THE SOILS REPORT FOR THIS SITE WAS PREPARED BY ENTECH ENGINEERING, INC. (DATED 02/26/2021) AND SHALL BE CONSIDERED A PART OF THESE PLANS.

29. AT LEAST TEN (10) DAYS PRIOR TO THE ANTICIPATED START OF CONSTRUCTION, FOR PROJECTS THAT WILL DISTURB ONE (1) ACRE OR MORE, THE OWNER OR OPERATOR OF CONSTRUCTION ACTIVITY SHALL SUBMIT A PERMIT APPLICATION FOR STORMWATER DISCHARGE TO THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT, WATER QUALITY DIVISION. THE APPLICATION CONTAINS CERTIFICATION OF COMPLETION OF A STORMWATER MANAGEMENT PLAN (SWMP), OF WHICH THIS GRADING AND EROSION CONTROL PLAN MAY BE A PART. FOR INFORMATION OR APPLICATIONS MATERIALS, CONTACT:

COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT
WATER QUALITY CONTROL DIVISION
WQCD - PERMITS
4300 CHERRY CREEK DRIVE SOUTH
DENVER, CO 80246-1530
ATTN: PERMITS UNIT

- Is "Pond I" still currently a TSB? Or has the EDB been fully constructed? Please make a note on the drawings and SWMP of the status that this Pond will be in during the construction of this filing.

- Also, note whether there will be (or currently are) any check dams in the roadside ditches along Zaragoza Trail.

- Please add this note to the GEC Plans and/or SWMP:
The contractor will be responsible for any re-excavation of sediment and debris that collects in the downstream Sediment Basin depression required to ensure that the basin meets the design grades following construction. The roadside ditches conveying site runoff to the basin shall also be cleaned and free of sediment once the site becomes stabilized.

Or if the FSD features (trickle channel, forebay, outlet structure, etc) have already been installed, the contractor will need to regularly clean them out.

TIMING, CONSTRUCTION STAGING, AND SEQUENCING:

EXPECTED START DATE: JULY 2021
INSTALL TEMPORARY EROSION CONTROL - 2 DAYS
- PERIMETER SILT FENCING
- VEHICLE TRACKING CONTROL PAD

ROUGH GRADING - 5 DAYS
INSTALL FINAL SITE IMPROVEMENTS - 10 MONTHS
REMOVE TEMPORARY EROSION CONTROL - 5 DAYS

MINIMUM BEST MANAGEMENT PRACTICES ELEMENTS:

STEP 1- EROSION AND SEDIMENT CONTROL
INSTALL SEDIMENT TRAPPING DEVICES (PERIMETER CONTROLS) PRIOR TO THE START OF CONSTRUCTION.
STEP 2- SPILL PREVENTION AND RESPONSE
STEP 3- MATERIAL MANAGEMENT
MATERIAL AND EQUIPMENT STORAGE AREAS SHALL BE SECURE AND CONTAINED TO PREVENT DISCHARGE OF ANY MATERIAL IN RUNOFF.
WASTE SHALL BE CONTAINED AND DISPOSED OF PROPERLY. MAINTAIN BMP'S DURING BUILDING AND UTILITY CONSTRUCTION.
STEP 4- INSPECTION AND MAINTENANCE (SEE EROSION CONTROL NOTES)
STEP 5- INSTALL FINAL STABILIZATION - BASE COURSE, LANDSCAPING, EROSION CONTROL BLANKETS, AND SEEDING.
STEP 6- REMOVE TEMPORARY CONTROLS - SILT FENCING AFTER PERMANENT FEATURES ARE INSTALLED.

FINAL STABILIZATION AND LONG-TERM STORMWATER MANAGEMENT:

FINAL STABILIZATION MEASURES INCLUDE BASE COURSE, PARTIAL LANDSCAPE, AND REVEGETATION

EARTHWORK SUMMARY:

PROPOSED SITE:
CUT - 5,993 CY
FILL - 6,072 (*1.15) = 6,983 CY
NET - 990 CY CUT/FILL

DISTURBED AREA - 179,904 SF, 4.13 AC

EROSION CONTROL FACILITIES:

SILT FENCE (SF) - 930 LF
VEHICLE TRACKING PAD (VT) - 1

COMMON NAME (N=NATIVE, I=INTRODUCED)		SCIENTIFIC NAME	LBS PLS/ ACRE
WHEATGRASS, SIBERIAN	I	AGROPYRON FRAGILE	2.04
WHEATGRASS, SLENDER	N	ELYMUS TRACHYCAULUS	10.90
WHEATGRASS, INTERMEDIATE	I	THINOPYRUM INTERMEDIUM	3.00
WILDRYE, RUSSIAN	I	PSATHYROSTACHYS JUNCEA	2.04
WHEATGRASS, WESTERN	N	PASCOPYRUM SMITHII	3.20
CLOVER, RED	I	TRIFOLIUM PRATENSE	0.40
FLAX, BLUE-APPAR	I	LINUM PERENNE	0.41
SULPHUR-FLOWER BUCKWHEAT	N	ERIOGONUM UMBELLATUM	0.55
TOTAL/POUNDS/ACRE			22.54

ENGINEER OF RECORD:

THESE DETAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECTION AND SUPERVISION. SAID PLANS AND SPECIFICATIONS HAVE BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR DETAILED ROADWAY, DRAINAGE, GRADING AND EROSION CONTROL PLANS AND SPECIFICATIONS, AND SAID PLANS AND SPECIFICATIONS ARE IN CONFORMITY WITH APPLICABLE MASTER DRAINAGE PLANS AND MASTER TRANSPORTATION PLANS. SAID PLANS AND SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR ROADWAY AND DRAINAGE FACILITIES ARE DESIGNED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS, OR OMISSIONS ON MY PART IN PREPAREATION OF THESE DETAILED PLANS AND SPECIFICATIONS.

RYAN M. MANGINO, PE #43304 — DATE

OWNER'S STATEMENT:

I, THE OWNER/DEVELOPER HAVE READ AND WILL COMPLY WITH THE REQUIREMENTS OF THE GRADING AND EROSION CONTROL PLAN AND ALL OF THE REQUIREMENTS SPECIFIED IN THESE DETAILED PLANS AND SPECIFICATIONS.

DATE

EL PASO COUNTY:

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY, THROUGH APPROVAL OF THIS DOCUMENT, ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

FILED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, DRAINAGE CRITERIA MANUAL VOLUMES 1 AND 2, AND ENGINEERING CRITERIA MANUAL, AS AMENDED.

IN ACCORDANCE WITH ECM SECTION 1.12, THESE CONSTRUCTION DOCUMENTS WILL BE VALID FOR CONSTRUCTION FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO COUNTY ENGINEER. IF CONSTRUCTION HAS NOT STARTED WITHIN THOSE 2 YEARS, THE PLANS WILL NEED TO BE RESUBMITTED FOR APPROVAL, INCLUDING PAYMENT OF REVIEW FEES AT THE PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION.

JENNIFER IRVINE, P.E.
COUNTY ENGINEER / ECM ADMINISTRATOR — DATE

PPR-21-020

PCD File No. PPR-21-

JDS-HYDRO CONSULTANTS, INC.
5540 TECH CENTER DR., SUITE 100
COLORADO SPRINGS, COLORADO 80919
(719) 227-0072

SADDLEHORN RANCH
OVERALL WATER SYSTEM
GRADING & EROSION CONTROL NOTES

REVISIONS

NO.	DESCRIPTION	BY	APP.	DATE
1				
2				
3				
4				
5				
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7				

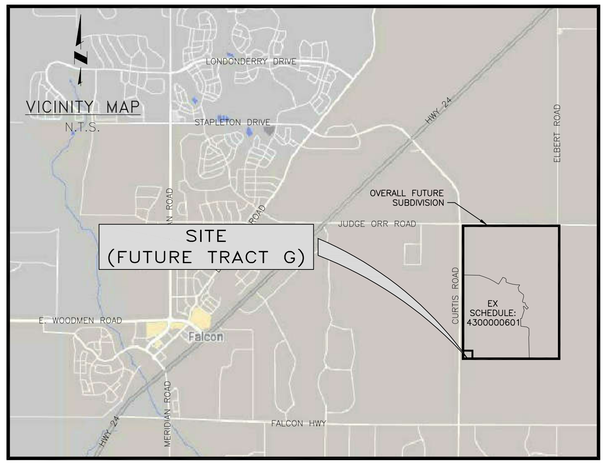
PRELIMINARY

Project No.: 311.02
Date: 04/01/21
Design: RMM
Drawn: SKG
Check: RMM

C11
SHEET 11 OF ----

LEGEND

EX PROPERTY LINE	PP VEHICLE TRACKING PAD (INITIAL)
EX RIGHT-OF-WAY	PP EROSION CONTROL SILT FENCE (INITIAL)
EX FENCE	STAGING AREA (INITIAL)
EX CONTOURS-MAJOR (5540)	CONCRETE WASHOUT (INITIAL)
EX CONTOURS-MINOR	RIPRAP (FINAL)
PP CONTOURS-MAJOR (5540)	PRE-DEVELOPED FLOW DIRECTION
PP CONTOURS-MINOR	DEVELOPED FLOW DIRECTION
PP YARD PIPING	CHECK DAM
PP FENCE	
AREA OF CUT	
AREA OF FILL	



FUTURE LOT (TRACT G) COVERAGE	
AREA OF TRACT	64,118 S.F.
GROSS FLOOR AREA OF TANK & BUILDING	20,625 S.F.
% TRACT COVERAGE WITH IMPROVEMENTS (INCLUDING FUTURE)	32%
AREA OF GRAVEL/DRIVEWAY	12,897 S.F.
% TRACT COVERAGE WITH DRIVEWAY	52%

0' 30' 60'
SCALE 11x17: 1" = 60'
SCALE 24x36: 1" = 30'

NOTES:
1. SEED AND MULCH ALL DISTURBED AREAS THAT WILL NOT HAVE IMPROVEMENTS (I.E. GRAVEL ROADS, RIPRAP, ETC.)
2. EROSION CONTROL BLANKETS ARE REQUIRED ON SLOPES 3:1 AND STEEPER. THERE ARE NO SLOPES ANTICIPATED TO BE 3:1 OR GREATER FOR THIS PROJECT.
3. EASEMENT BOUNDARY IS ALSO CONSIDERED THE CONSTRUCTION BOUNDARY AND LIMITS OF DISTURBANCE.
4. NO VEGETATION EXISTS ON THE SITE PRIOR TO CONSTRUCTION AS SITE WAS ALREADY STRIPPED.
5. NO BATCH PLANTS ARE PROPOSED AS A PART OF THIS PROJECT.
6. THERE ARE NO STREAM CROSSINGS WITHIN THE LIMITS OF THIS PROJECT.
7. ACCESS POINTS AND SITE NOT ACCESSIBLE TO PUBLIC.
8. ALL BMP'S ARE TEMPORARY AND MUST BE INSTALLED PRIOR TO LAND DISTURBANCE. NO BMP'S ARE PHASED FOR THIS PROJECT.
9. ENTIRE PARCEL SHALL BE USED AS A CONSTRUCTION BOUNDARY WITH LIMITS OF DISTURBANCE BEING PARCEL LINES.
10. THE PARTIES RESPONSIBLE FOR THIS PLAN HAVE FAMILIARIZED THEMSELVES WITH ALL CURRENT ACCESSIBILITY CRITERIA AND SPECIFICATIONS AND THE PROPOSED PLAN REFLECTS ALL SITE ELEMENTS REQUIRED BY THE APPLICABLE ADA DESIGN STANDARDS AND GUIDELINES AS PUBLISHED BY THE UNITED STATES DEPARTMENT OF JUSTICE. APPROVAL OF THIS PLAN BY EL PASO COUNTY DOES NOT ASSURE COMPLIANCE WITH THE ADA OR ANY REGULATIONS OR GUIDELINES ENACTED OR PROMULGATED UNDER OR WITH RESPECT TO SUCH LAWS.

Checklist Item "j" - make a note about FEMA 100-yr floodplain (ex: not within site boundaries).

PCD File No. PPR-21-

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5540 TECH CENTER DR., SUITE 100
COLORADO SPRINGS, COLORADO 80919
(719) 227-0072

SADDLEHORN RANCH

OVERALL WATER SYSTEM

GRADING & EROSION CONTROL PLAN

REVISIONS				
NO.	DESCRIPTION	BY	APP.	DATE
1				
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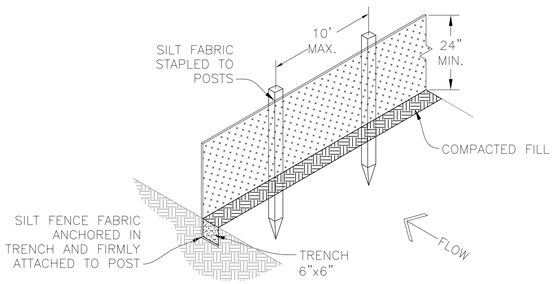
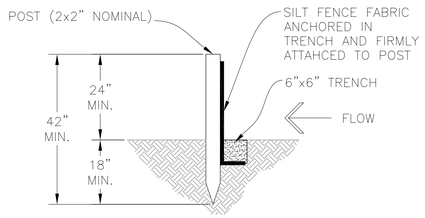
PRELIMINARY

Project No.: 311.02
Date: 04/01/21
Design: RMM
Drawn: SKG
Check: RMM

C12
SHEET 12 OF ----





SILT FENCE DETAIL

INSTALLATION REQUIREMENTS:

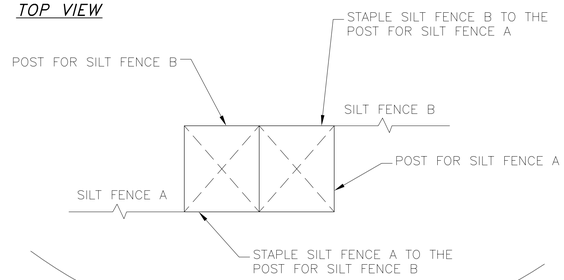
1. SILT FENCES SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.
2. WHEN JOINTS ARE NECESSARY, SILT FENCE GEOTEXTILE SHALL BE SPLICED TOGETHER ONLY AT SUPPORT POST AND SECURELY SEALED.
3. METAL POSTS SHALL BE "STUDDED TEE" OR "U" TYPE WITH MINIMUM WEIGHT OF 1.33 POUNDS PER LINEAR FOOT. WOOD POSTS SHALL HAVE A MINIMUM DIAMETER OR CROSS SECTION DIMENSION OF 2 INCHES.
4. THE FILTER MATERIAL SHALL BE FASTENED SECURELY TO METAL POSTS USING WIRE TIES, OR TO WOOD POSTS WITH 3/4" LONG #9 HEAVY-DUTY STAPLES. THE SILT FENCE GEOTEXTILE SHALL NOT BE STAPLED TO EXISTING TREES.
5. WHILE NOT REQUIRED, WIRE MESH FENCE MAY BE USED TO SUPPORT THE GEOTEXTILE. WIRE FENCE SHALL BE FASTENED SECURELY TO THE UPSLOPE SIDE OF THE POSTS USING HEAVY-DUTY WIRE STAPLES AT LEAST 3/4" LONG, TIE WIRES OR HOG RINGS. THE WIRE SHALL EXTEND INTO THE TRENCH A MINIMUM OF 6 INCHES AND SHALL NOT EXTEND MORE THAN 3 FEET ABOVE THE ORIGINAL GROUND SURFACE.
6. ALONG THE TOE OF FILLS, INSTALL THE SILT FENCE ALONG A LEVEL CONTOUR AND PROVIDE AN AREA BEHIND THE FENCE FOR RUNOFF TO POND AND SEDIMENT TO SETTLE. A MINIMUM DISTANCE OF 5 FEET FROM THE TOE OF THE FILL IS RECOMMENDED.
7. THE HEIGHT OF THE SILT FENCE FROM THE GROUND SURFACE SHALL BE MINIMUM OF 24 INCHES AND SHALL NOT EXCEED 36 INCHES. HIGHER FENCES MAY IMPOUND VOLUMES OF WATER SUFFICIENT TO CAUSE FAILURE OF THE STRUCTURE.

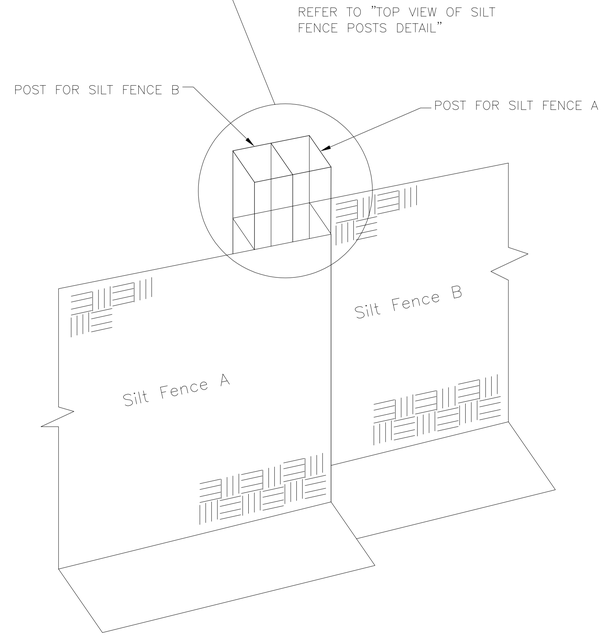
MAINTENANCE REQUIREMENTS:

1. CONTRACTOR SHALL INSPECT SILT FENCES IMMEDIATELY AFTER EACH RAINFALL, AT LEAST DAILY DURING PROLONGED RAINFALL, AND WEEKLY DURING PERIODS OF NO RAINFALL. DAMAGED, COLLAPSED, UNENTRENCHED OR INEFFECTIVE SILT FENCES SHALL BE PROMPTLY REPAIRED OR REPLACED.
2. SEDIMENT SHALL BE REMOVED FROM BEHIND SILT FENCE WHEN IT ACCUMULATES TO HALF THE EXPOSED GEOTEXTILE HEIGHT.
3. SILT FENCES SHALL BE REMOVED WHEN ADEQUATE VEGETATIVE COVER IS ATTAINED.

A / C13 SILT FENCE DETAIL
SCALE: N.T.S.

TOP VIEW





B / C13 SILT FENCE POSTS DETAIL
SCALE: N.T.S.

PCD File No. PPR-21-

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SADDLEHORN RANCH
OVERALL WATER SYSTEM
GRADING & EROSION CONTROL DETAILS 1

REVISIONS			
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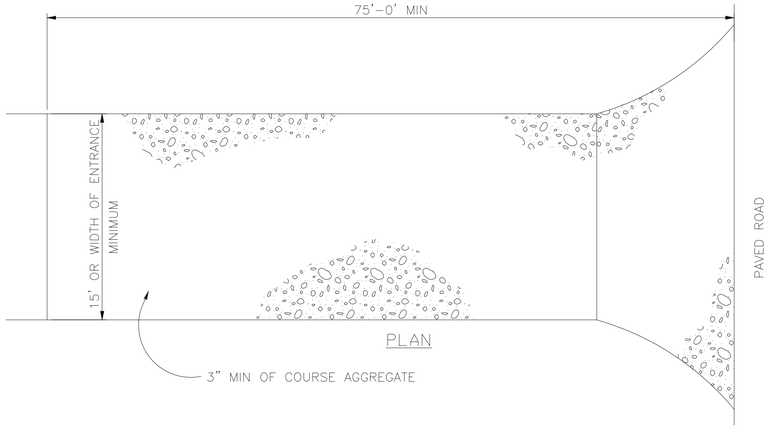
PRELIMINARY

Project No.: 311.02
Date: 04/01/21
Design: RMM
Drawn: SKG
Check: RMM

C13
SHEET 13 OF ----

2021/04/01 3:36 PM By: Shelby Gatlin J:\JDS-Hydro\Project Files\311 ROI Property Group\311.02 Saddlehorn Ranch Subdivision\Drawings\Working\311102_1_Civil.dwg



VEHICLE TRACKING PAD DETAIL

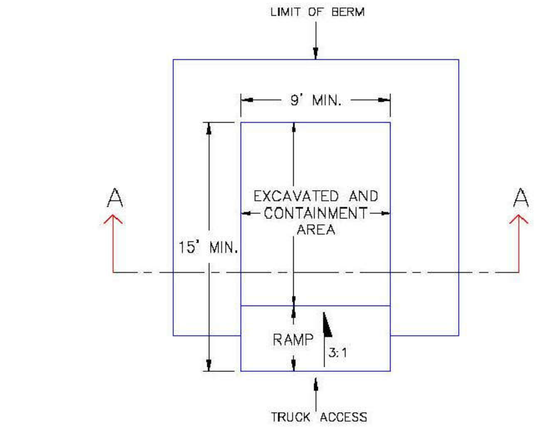
INSTALLATION REQUIREMENTS:

1. ALL ENTRANCES TO THE CONSTRUCTION SITE ARE TO BE STABILIZED PRIOR TO COMMENCEMENT OF CONSTRUCTION.
2. CONSTRUCTION ENTRANCES ARE TO BE BUILT WITH AN APRON TO ALLOW FOR TURNING TRAFFIC, BUT SHOULD NOT BE BUILT OVER EXISTING PAVEMENT EXCEPT FOR A SLIGHT OVERLAP.
3. AREAS TO BE STABILIZED ARE TO BE PROPERLY GRADED AND COMPACTED.
4. CONSTRUCTION ROADS, PARKING AREAS, LOADING/UNLOADING ZONES, STORAGE AREAS, AND STAGING AREAS ARE TO BE STABILIZED.
5. CONSTRUCTION ROADS ARE TO BE BUILT TO CONFORM TO SITE GRADES, BUT SHOULD NOT HAVE SIDE SLOPES OR ROAD GRADES THAT ARE EXCESSIVELY STEEP.

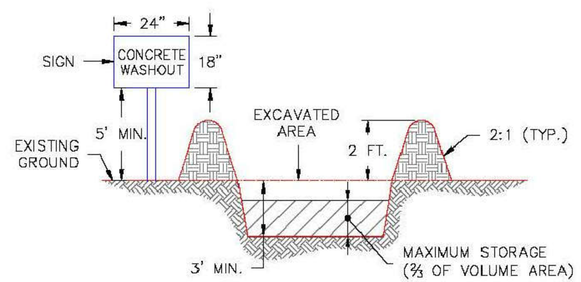
MAINTENANCE REQUIREMENTS

1. REGULAR INSPECTIONS ARE TO BE MADE OF ALL STABILIZED AREAS, ESPECIALLY AFTER STORM EVENTS.
2. STONES ARE TO BE REAPPLIED PERIODICALLY AND WHEN REPAIR IS NECESSARY.
3. SEDIMENT TRACKED ONTO PAVED ROADS IS TO BE REMOVED DAILY BY SHOVELING OR SWEEPING. SEDIMENT IS NOT TO BE WASHED DOWN STORM SEWER DRAINS.
4. OTHER ASSOCIATED SEDIMENT CONTROL MEASURES ARE TO BE INSPECTED TO ENSURE GOOD WORKING CONDITION.
5. TO BE REMOVED JUST PRIOR TO FINAL SURFACING AND STABILIZATION.

A / C14 VEHICLE TRACKING PAD DETAIL
SCALE: N.T.S.



PLAN VIEW



SECTION A-A

NOTES:
1. SIGN MATERIAL, EXCAVATION, AND RESTORATION ARE INCLUDED IN THE COST OF THE CONCRETE WASHOUT STRUCTURE.
2. EROSION BALES MAY BE USED AS AN ALTERNATIVE FOR THE BERM.

1/1/08 DATE APPROVED: John A. McCarty DEPARTMENT OF TRANSPORTATION	Concrete Washout Structure Standard Drawing REVISION DATE: 7/17/07 FILE NAME: SD_3-84	EL PASO COUNTY DEPARTMENT OF TRANSPORTATION

B / C14 CONCRETE WASHOUT STRUCTURE DETAIL
SCALE: N.T.S.

PCD File No. PPR-21-

JDS-HYDRO CONSULTANTS, INC.
5540 TECH CENTER DR., SUITE 100
COLORADO SPRINGS, COLORADO 80919
(719) 227-0072

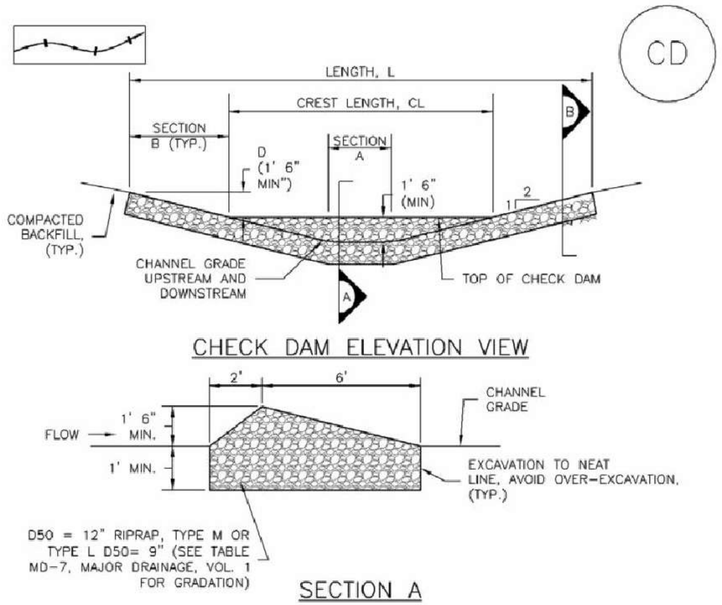
SADDLEHORN RANCH
OVERALL WATER SYSTEM
GRADING & EROSION CONTROL DETAILS 2

REVISIONS			
NO.	DESCRIPTION	BY	APP.	DATE
1				
2				
3				
4				
5				
6				
7				

PRELIMINARY

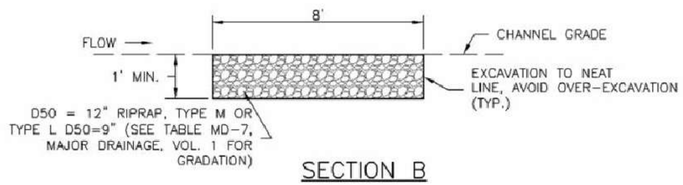
Project No.: 311.02
Date: 04/01/21
Design: RMM
Drawn: SKG
Check: RMM

C14
SHEET 14 OF ----

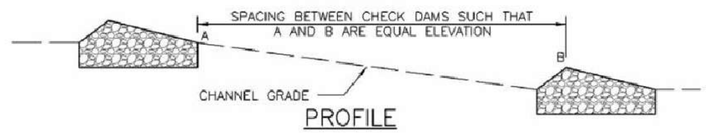


CHECK DAM ELEVATION VIEW

SECTION A



SECTION B



PROFILE

A / C15 CHECK DAM DETAIL
SCALE: N.T.S.

Include details for the following BMP's. Examples of acceptable details for each are provided:

	Detail # and Source			
BMP	ECM (Appendix F)	DCM (Vol 2: Chap 3.3)	MHFD (USDCM Vol 3: Chap 7)	CDOT Standard Plans on M-208-1
Mulching		MU-1	EC-4	
Seeding		TS-1	EC-2	
Stabilized Staging Area			SM-6	

CHECK DAM INSTALLATION NOTES:

1. SEE PLAN VIEW FOR LOCATION OF CHECK DAMS, CHECK DAM TYPE (CHECK DAM OR REINFORCED CHECK DAM), LENGTH (L), CREST LENGTH (CL), AND DEPTH (D).
2. CHECK DAMS INDICATED ON INITIAL SWMP SHALL BE INSTALLED AFTER CONSTRUCTION FENCE, BUT PRIOR TO ANY UPSTREAM LAND-DISTURBING ACTIVITIES
3. RIPRAP UTILIZED FOR CHECK DAMS SHOULD BE OF APPROPRIATE SIZE FOR THE APPLICATION. TYPICAL TYPES OF RIPRAP USED FOR CHECK DAMS ARE TYPE M (D50 12") OR TYPE L (D50 9").
4. RIPRAP PAD SHALL BE TRENCHED INTO THE GROUND A MINIMUM OF 1'.
5. THE ENDS OF THE CHECK DAM SHALL BE A MINIMUM OF 1'-6" HIGHER THAN THE CENTER OF THE CHECK DAM.

CHECK DAM MAINTENANCE NOTES:

1. INSPECT BMPs EACH WORKDAY AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPSs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION AND PERFORM ANY NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. SEDIMENT ACCUMULATED UPSTREAM OF THE CHECK DAMS SHALL BE REMOVED WHEN THE SEDIMENT DEPTH IS WITHIN 1/2 OF THE HEIGHT OF THE CREST.
5. CHECK DAMS ARE TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND APPROVED BY THE LOCAL JURISDICTION.
6. WHEN CHECK DAMS ARE REMOVED, EXCAVATIONS SHALL BE FILLED WITH SUITABLE COMPACTED BACKFILL. DISTURBED AREA SHALL BE SEEDED AND MULCHED AND COVERED WITH GEOTEXTILE OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE LOCAL JURISDICTION.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

PCD File No. PPR-21-

JDS-HYDRO CONSULTANTS, INC.
5540 TECH CENTER DR., SUITE 100
COLORADO SPRINGS, COLORADO 80919
(719) 227-0072

SADDLEHORN RANCH
OVERALL WATER SYSTEM
CHECK DAM DETAILS

REVISIONS

NO.	DESCRIPTION	BY	APP.	DATE
1				
2				
3				
4				
5				
6				
7				

PRELIMINARY

Project No.: 311.02
Date: 04/01/21
Design: RMM
Drawn: SKG
Check: RMM

C15
SHEET 15 OF ----

Appendix: Soils Report

ENTECH
ENGINEERING, INC.

505 ELKTON DRIVE
COLORADO SPRINGS, CO 80907
PHONE (719) 531-5599
FAX (719) 531-5238

SUBSURFACE SOIL INVESTIGATION
WATER STORAGE TANK AND
TREATMENT FACILITY
SADDLEHORN RANCH SUBDIVISION
EL PASO COUNTY, COLORADO

Prepared for:

William Guman and Associates, Ltd.
731 North Weber Street
Colorado Springs, Colorado 80903

Attn: Mr. Bill Guman

February 26, 2021

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Logan L. Langford, P.G.
Geologist

Reviewed by:

Joseph C. Goode, Jr., P.E.
President

COLORADO LICENSED
JOSEPH COLLIN GOODE JR
23725
PROFESSIONAL ENGINEER
3/1/21

LLL

Encl.

Entech Job No. 210181
AAprojects/2021/210181ssi

Table of Contents

SUBSURFACE SOIL INVESTIGATION
WATER STORAGE TANK AND
TREATMENT FACILITY
SADDLEHORN RANCH SUBDIVISION
EL PASO COUNTY, COLORADO

1.0 INTRODUCTION

William Guman and Associates, Ltd. are planning the construction of a water storage tank and water treatment facility for the proposed Saddlehorn Ranch Subdivision. A 42.5-foot round tank approximately 28-feet high, and a water treatment facility are proposed. The site is located east of Falcon, Colorado south and east of Judge Orr Road and Curtis Road in the eastern portion of El Paso County. The water wells are currently located on Tract A to the northeast of the proposed tank and treatment facility. The location of the project is shown on the Vicinity Map, Figure 1. The proposed site plan and the test boring locations are shown on the Test Boring Location Plan, Figure 2.

This report describes the subsurface soil conditions encountered in the test borings conducted on the site and provides recommendations for foundation design and construction. The Subsurface Soil Investigation included the drilling of two test borings within the proposed tank and treatment facility footprints, collecting samples of soil, and conducting a geotechnical evaluation of the investigation findings. All drilling and subsurface investigation activities were performed by Entech Engineering, Inc. (Entech). The contents of this report, including the geotechnical evaluation and recommendations, are subject to the limitations and assumptions presented in Section 6.0.

2.0 PROJECT AND SITE DESCRIPTION

The project will consist the construction of a water storage tank and water treatment facility for the proposed Saddlehorn Ranch Subdivision. A 42.5-foot round tank approximately 28-feet high, a water treatment facility, and associated site improvements. The site is currently undeveloped with vegetation consisting of grasses and weeds. Past land use has consisted of agricultural grazing. The water wells are currently located on Tract A to the northeast of the proposed tank and treatment facility, and the area around the water wells has previously been regraded. Topography of the site is gradually sloping to the east. The site is bordered by agricultural grazing land to the east and south, and rural residential properties to the west.

3.0 SUBSURFACE EXPLORATIONS AND LABORATORY TESTING

Subsurface conditions within the footprints of the proposed water tank and treatment facility were explored by drilling two test borings. Test Boring No. 1 was drilled at the water tank location, and Test Boring No. 2 was drilled at the treatment facility location. The test borings were drilled at the approximate locations shown on the Test Boring Location Plan, Figure 2. The borings were drilled to depths of 20 and 40 feet below the existing ground surface (bgs). The drilling was performed using a truck-mounted, continuous flight auger-drilling rig supplied and operated by Entech. Boring logs descriptive of the subsurface conditions encountered during drilling are presented in Appendix A. At the conclusion of drilling, observations for groundwater levels were made in the open boreholes.

Soil samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D-1586) using 2-inch O.D. split-barrel and California samplers. Results of the Standard Penetration Test (SPT) are included on the boring logs in terms of N-values expressed in blows per foot (bpf). Soil samples recovered from the borings were visually classified and recorded on the boring logs. The soil classifications were later verified utilizing laboratory testing and grouped by soil type. The soil type numbers are included on the boring logs. It should be understood that the soil descriptions shown on the boring logs may vary between boring location and sample depth. It should also be noted that the lines of stratigraphic separation shown on the boring logs represent

approximate boundaries between soil types and the actual stratigraphic transitions may be more gradual and vary with location.

Moisture content testing (ASTM D-2216) was performed on the samples recovered from the borings, and the results are shown on the boring logs. Grain-Size Analysis Testing (ASTM D-422) was performed on selected samples to assist in classifying the materials encountered in the borings. Volume change testing was performed on selected samples using the Swell/Consolidation Test (ASTM D-4318) in order to evaluate potential expansion/compression characteristics of the soil. The Laboratory Testing Results are summarized on Table 1 and are presented in Appendix B.

4.0 SUBSURFACE CONDITIONS

Two soil types and one bedrock type were encountered in the test borings drilled for the subsurface investigation: Type 1: slightly silty to silty sand (SM-SW, SM), and Type 2: sandy siltstone (ML). Bedrock was encountered in Test Boring No. 1 at 30 feet bgs. Each soil type was classified in accordance with the Unified Soil Classification System (USCS) using the laboratory testing results and the observations made during drilling.

4.1 Soil and Bedrock

Soil Type 1 classified as sand, slightly silty to silty sand (SM-SW, SM). The sand was encountered in both of the test borings at the existing surface extending to depths ranging from 20 to 30 feet bgs. Standard Penetration Testing resulted in SPT N-values of 14 to 50 bpf, indicating medium dense to dense states. Water content and grain size testing resulted in approximately 1 to 8 percent water content with approximately 8 to 19 percent of the soil size particles passing the No. 200 sieve.

Soil Type 2 classified as sandy siltstone (ML). The siltstone was encountered in Test Boring No. 1 at 30 feet and extended to the termination of the test boring (40 feet). Standard Penetration Testing resulted in SPT N-values of greater than 50 bpf, indicating hard consistencies. Water content and grain size testing resulted in approximately 15 to 16 percent water content with

approximately 85 percent of the soil size particles passing the No. 200 sieve. Swell/Consolidation Testing resulted in volume change of 0.0 percent, indicating low swell potentials.

4.2 Groundwater

Groundwater was not encountered in the test borings during or subsequent to drilling. It is anticipated groundwater will not affect construction of the water tank foundation depending on the final grading plans, however deeper excavations for utilities and potentially deep foundation members may encounter groundwater. Development of this and adjacent properties, as well as seasonal precipitation changes, and changes in runoff may affect groundwater elevations.

5.0 GEOTECHNICAL EVALUATION AND RECOMMENDATIONS

The following discussion is based on the subsurface conditions encountered in two borings drilled for the planned water storage tank and treatment facility for the proposed Saddlehorn Ranch Subdivision. If subsurface conditions different from those described herein are encountered during construction of the structure or if the project elements change from those described, Entech Engineering, Inc. should be notified so that the evaluation and recommendations presented can be reviewed and revised if necessary.

Subsurface soil conditions encountered in the test borings drilled within the proposed tank and treatment facility footprints consisted of slightly silty to silty sand with underlying sandy siltstone. The siltstone was encountered in Test Boring No. 1 at a depth of 30 feet bgs. SPT N-values measured in upper soil profiles indicated medium dense to dense states. The siltstone was encountered at hard states. The in-situ upper sands are considered moderate bearing soils and will provide adequate support for the proposed treatment facility. If higher bearing capabilities are required for the tank it is recommended the water tank foundation be overexcavated 3 feet and replaced with Class 6 Aggregate Basecourse to be compacted according to the "Structural Fill" paragraph. The overexcavation subgrade should be moisture-conditioned and recompacted prior to placement of the aggregate basecourse. A shallow foundation system in conjunction with overexcavation will provide a uniform bearing pad for the water tank. Foundation components for the treatment facility are suitable to bear on the medium dense native sands. Shallow foundations for the tank are anticipated to be concrete ring/footing configurations. Design considerations are discussed in the following sections.

5.1 Footing Subgrade Improvement and Bearing Capacity:

The sand soils were encountered at medium dense to dense states. The upper soils exhibit a low potential for consolidation and expansion. The native soils are suitable to support the treatment building and water tank, if the lower bearing capacity is suitable. At the tank location, the subgrade soils should be removed and recompacted to provide a uniform pad. The recompaction should be 2 feet below footing depth and 3 feet beyond the foundation perimeter. Due to anticipated loads of the water tank, to achieve higher bearing capacities, the soils can be overexcavated to a depth of 3 feet below the foundation subgrade and replaced with 3 feet of compacted Class 6 Aggregate Basecourse (ABC) as defined by the Colorado Department of Transportation (CDOT). Alternative aggregate sources should be approved prior to hauling to this site. The overexcavation width should extend a minimum of 3 feet beyond the footing perimeter to assist in distributing footing stresses. The subgrade of the overexcavated area should be scarified, moisture conditioned and compacted, to at least 95 percent of the soils maximum dry density as determined by the Modified Proctor Test ASTM D-1557. The ABC placed in the overexcavation should be compacted to at least 95 percent of its maximum Modified Proctor Dry Density (ASTM D-1557).

Provided the soils encountered in the footing excavations are consistent with the in-place conditions observed in the borings and the above mitigation recommended is implemented, a maximum allowable bearing capacity of 3,500 pounds per square foot (psf) is recommended for the tank foundation design. Fill placed in the overexcavation should consist of ABC, free of organic materials, debris and stone sizes greater than 3 inches in diameter. If the overexcavation/ base course is not used at the tank site, an allowable bearing capacity of 2600 psf can be used for recompacted site soils. Foundation components for the treatment facility are suitable to bear on the medium dense native sands; a maximum allowable bearing capacity of 2,400 psf is recommended. Fill placed below footings should be compacted to at least 95 percent of the soils' ASTM D-1557 maximum dry density and be placed in horizontal lifts not exceeding 6 inches in thickness after compaction. Frequent density tests should be performed at 12-inch intervals and at the final proposed footing subgrade elevation. Exterior footings should be embedded a minimum of 30 inches below the adjacent exterior site grade in order to provide frost protection.

Following the above foundation construction recommendations and adhering to the recommended maximum allowable soil bearing capacity is expected to result in a foundation

design which should limit total and differential settlements to 1 inch and ½ inch, or less, respectively. Given the site soil conditions as described herein, coupled with the use of ABC beneath the foundation footings, the bulk of the foundation settlement is expected to occur during the building construction period, upon initial filling of the tank, or shortly thereafter. Based on the use of ABC, a vertical modulus of subgrade reaction of 200 pounds per inch can be used.
To further support the foundation design, it is recommended that an Entech Engineer observe the foundation excavation and evaluate if the exposed native and filled subgrade(s) are consistent with those described in this report. Overexcavation recommendations should be finalized when the excavation observations are performed. The Entech Engineer should also provide recommendations for foundation drainage should conditions warrant.

5.2 Tank Floor-On-Grade-Slab

The floor of the water storage tank reportedly is expected to consist of a reinforced concrete slab-on-grade. To carry the expected heavy loads for a water storage tank, it is recommended that the sand soils be overexcavated to provide a uniform bearing pad as discussed above. The subgrade of the overexcavated zone should be scarified, moisture conditioned and compacted to at least 95 percent of the soils' maximum dry density as determined by the Modified Proctor Test ASTM D-1557 and within $\pm$ 2% of the optimum moisture content as determined by the proctor test. The overexcavation beneath the tank slab should be replaced with a minimum of 6 feet of ABC as defined by CDOT. The ABC should be compacted to 95% of its' ASTM D –1557 maximum dry density and within ± 2% of its' optimum moisture content as determined by the proctor test.

5.3 Seismic Site Classification

Based on the subsurface conditions encountered at the site and in accordance with Section 1613 of the 2015 International Building Code (IBC), the site meets the conditions of a Site Class D.

5.4 Surface and Subsurface Drainage

Positive surface drainage is recommended around the standpipe to minimize infiltration of surface water into the supporting foundation soils. A minimum ground surface slope of 5 percent in the first 10 feet adjacent to exterior foundation walls is recommended for unpaved areas. For paved areas and other impervious surfaces, a minimum slope of 2 percent is adequate. All tank overflow

piping should be extended to discharge well beyond the tank's foundation backfill zone or be connected to a storm sewer system.

A perimeter foundation drain will not be required for the water tank.

5.5 Concrete Degradation Due to Sulfate Attack

Type II cement is recommended for concrete at this site. To further avoid concrete degradation during construction it is recommended that concrete not be placed on frozen or wet ground. Care should be taken to prevent the accumulation or ponding of water in the foundation excavation prior to the placement of concrete. If standing water is present in the foundation excavation, it should be removed by ditching to sumps and pumping the water away from the foundation area prior to concrete placement. If concrete is placed during periods of cold temperatures, the concrete must be kept from freezing. This may require covering the concrete with insulated blankets and adding heat to prohibit freezing.

5.6 Foundation Excavation Observation

Subgrade preparation for tank foundation should be observed by Entech prior to construction in order to verify that (1) no anomalies are present, (2) materials of the proper bearing capacity have been encountered or placed, and (3) no soft spots, expansive or organic soil, or debris are present in the foundation area prior to concrete placement or backfilling. Entech should make any additional recommendations for over-excavation during scheduled site visits.

5.7 Structural Fill

The structural fill for the water tank shall consist of Class 6 Aggregate Basecourse (ABC) as defined by the Colorado Department of Transportation (CDOT). The basecourse should be approved by Entech, and compacted to a minimum of 95 percent of the soils maximum dry density as determined by the Modified Proctor Test (ASTM D-1557), usually within ±2 percent of the optimum water content. Fill material should be placed in horizontal lifts such that each finished lift has a compacted thickness of six inches or less. Fill should be placed at water contents conducive to achieving adequate compaction. The overexcavation subgrade should be scarified a minimum of 12 inches, and be compacted to a minimum of 95 percent compaction, at ±2 percent

optimum moisture content, utilizing a Modified Proctor Dry Density ASTM D-1557. No water flooding techniques of any type should be used for compaction or placement of foundation or floor slab fill material. Entech should approve any imported fill to be used within the foundation area prior to delivery to the site.

5.8 Utility Trench Backfill

Fill placed in utility trenches should be compacted according to local specifications. Fill should be placed in horizontal lifts having a compacted thickness of six inches or less and at a water content conducive to adequate compaction, within ±2 percent of optimum water content. Mechanical methods should be used for fill placement; however, heavy equipment should be kept at a distance from foundation walls. No water flooding techniques of any type should be used for compaction or placement of utility trench fill.

Trench backfill placement should be performed in accordance with the Town of Sheridan Lake and Kiowa County specifications. All excavation and excavation shoring/bracing should be performed in accordance with OSHA guidelines. Groundwater may be encountered in the excavations.

5.9 General Backfill

Any areas to receive fill outside the foundation limits should have all topsoil, organic material, and debris removed. Fill must be properly benched into existing slopes in order to be adequately compacted. The fill receiving surface should be scarified to a depth of 12-inches and moisture conditioned to ±2 percent of the optimum water content, and compacted to a minimum of 95 percent of Standard Proctor, ASTM D-698 (for cohesive soils) and Modified Proctor ASTM D-1557 (for cohesionless soils) maximum Dry Density before the addition of new fill. Fill should be placed in thin lifts not to exceed 6 inches in thickness after compaction while maintaining at least 95 percent of Standard Proctor, ASTM D-698 (for cohesive soils) and Modified Proctor, ASTM D-1557 (for cohesionless soils) maximum Dry Density. Fill material should be free of vegetation and other unsuitable material and shall not contain rocks or fragments greater than 3-inches. Topsoil and strippings should be segregated from all other fill sources on the site. Fill placement and compaction beneath and around foundations, in utility trenches, beneath roadways or other structural features of the project should be observed and tested by Entech during construction.

5.10 Excavation Stability

Excavation sidewalls must be properly sloped, benched and/or otherwise supported in order to maintain stable conditions. All excavation openings and work completed therein shall conform to OSHA Standards as put forward in CFR 29, Part 1926.650-652, (Subpart P).

5.11 Winter Construction

In the event construction of the planned facility occurs during winter, foundations and subgrades should be protected from freezing conditions. Concrete should not be placed on frozen soil and once concrete has been placed, it should not be allowed to freeze. Similarly, once exposed, the foundation subgrade should not be allowed to freeze. During site grading and subgrade preparation, care should be taken to avoid burial of snow, ice or frozen material within the planned construction area.

5.12 Construction Observations

It is recommended that Entech observe and document the following activities during construction of the building foundations.

- Excavated subgrades and subgrade preparation.
- Placement of foundation perimeter drains (if installed).
- Placement/compaction of fill material for the foundation components and floor slab.
- Placement/compaction of utility bedding and trench backfill.

6.0 CLOSURE

The subsurface Investigation, geotechnical evaluation and recommendations presented in this report are intended for use by William Guman and Associates, Ltd., with application to the planned water tank land treatment facility at the proposed Saddlehorn Ranch Subdivision in eastern El Paso County, Colorado, southeast of Judge Orr Road and Curtis Road. Based on the results of this investigation, the site is suitable for the proposed development, if the improvements are implemented as discussed in the report. In conducting the subsurface investigation, laboratory testing, engineering evaluation and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in same locality and under similar conditions. No other warranty, expressed or implied is made. During final development design and prior to construction, additional investigation is recommended after site grading to provide final recommendations for the building site.

If there are any questions regarding the information provided herein or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.

TABLE

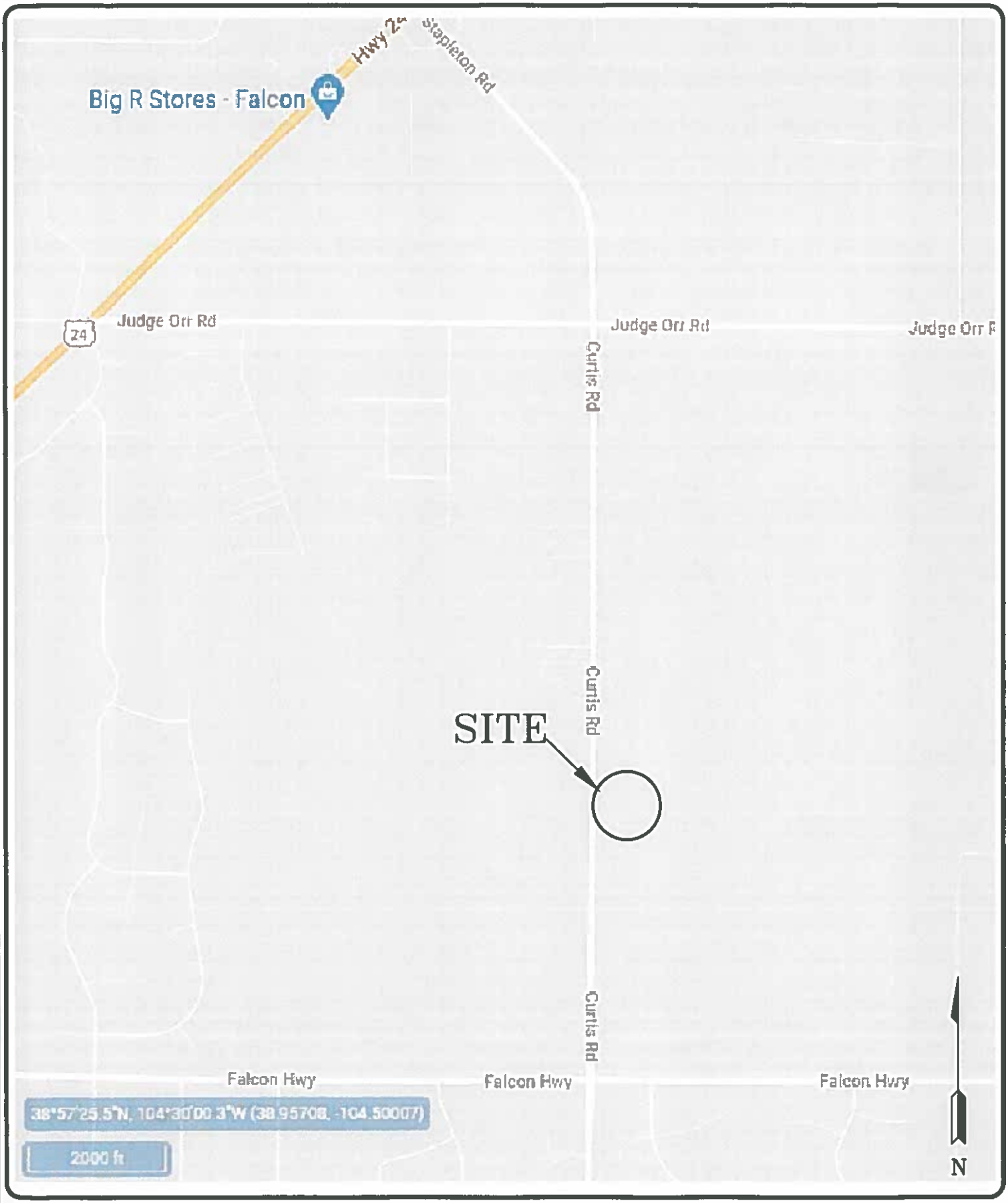
TABLE 1

SUMMARY OF LABORATORY TEST RESULTS

CLIENT WILLIAM GUMAN
PROJECT CURTIS ROAD
JOB NO. 210181

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/ CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	1	2-3			7.9						SM-SW	SAND, SLIGHTLY SILTY
1	1	20			19.0						SM	SAND, SILTY
1	2	5			19.2						SM	SAND, SILTY
2	1	35	16.8	112.2	81.1					0.0	ML	SILTSTONE, SANDY

FIGURES



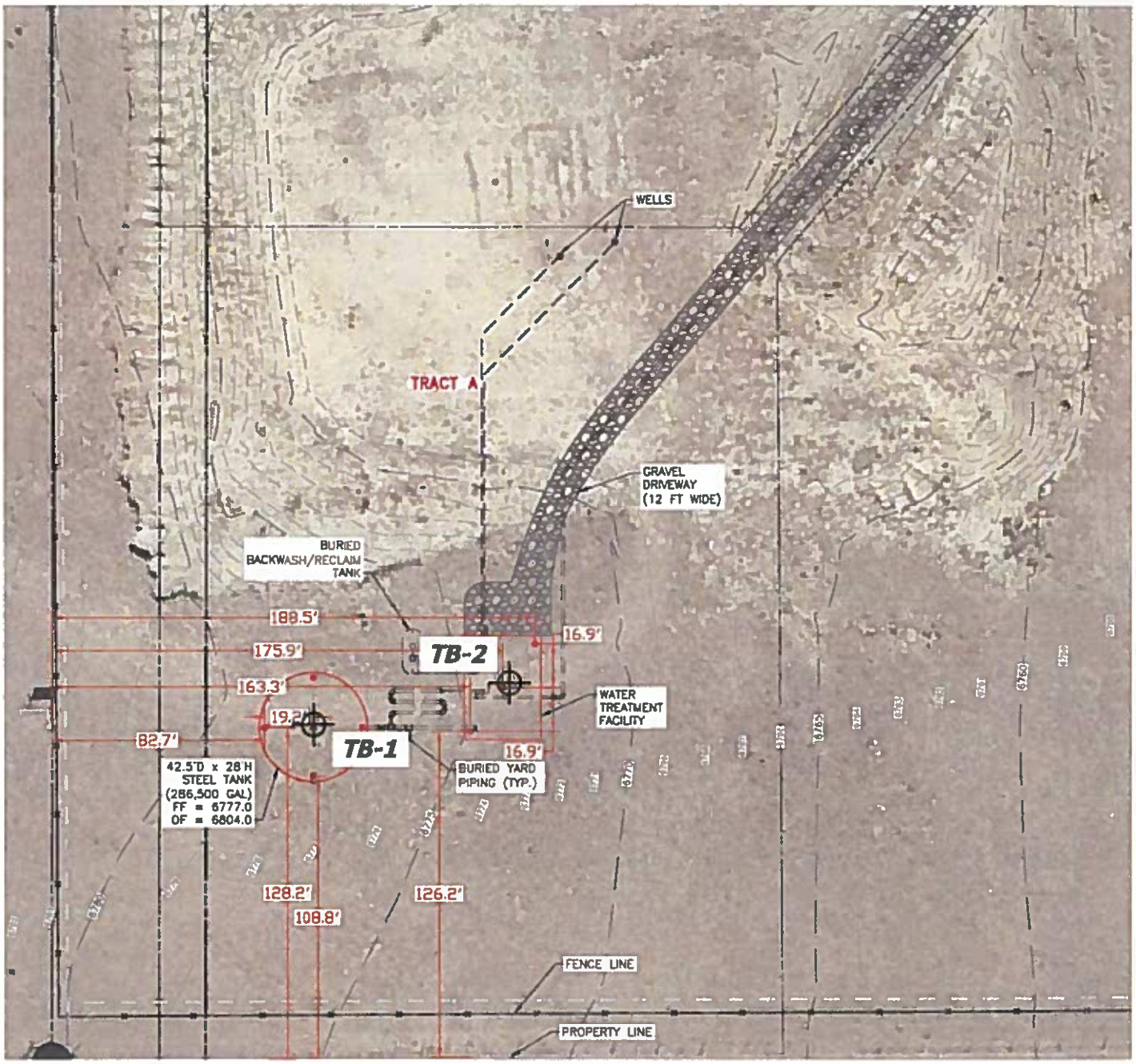
ENTECH ENGINEERING, INC.
505 ELKTON DRIVE
COLORADO SPRINGS, CO 80907 (719) 531-5599

VICINITY MAP
SADDLEHORN RANCH SUBIDVISION
WATER TANK AND TREATMENT FACILITY
EL PASO COUNTY, COLORADO
FOR: WILLIAM GUMAN AND ASSOCIATES, LTD.

DRAWN:	DATE:	CHECKED:	DATE:
LLL	2/12/21		

JOB NO.: 210181

FIG NO.: 1



⊕ **TB- APPROXIMATE TEST BORING LOCATION AND NUMBER**

N

ENTECH ENGINEERING, INC.
395 ELKTON DRIVE
COLORADO SPRINGS, CO. 80907 (719) 531-5599

SITE PLAN/TEST BORING LOCATION MAP
SADDLEHORN RANCH SUBIDVISION
WATER TANK AND TREATMENT FACILITY
EL PASO COUNTY, COLORADO
FOR: WILLIAM GUMAN AND ASSOCIATES, LTD.

DRAWN:	DATE:	CHECKED:	DATE:
LLL	2/12/21		

JOB NO.: 210181

FIG NO.: 2

APPENDIX A: Test Boring Logs

TEST BORING NO. 1
DATE DRILLED 2/5/2021
Job # 210181

REMARKS

DRY TO 38', 2/17/21

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
	SAND, SLIGHTLY SILTY TO SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE TO VERY DENSE, DRY TO MOIST		20	1.2	1
5			38	6.2	1
10			44	5.7	1
15			35	4.6	1
20			50	7.4	1
25			43	4.7	1
30			38	5.3	1
	SILTSTONE, SANDY, BLUE GRAY, HARD, MOIST				
35			50/10"	15.9	2
40			50/5"	15.2	2

TEST BORING NO. 2
DATE DRILLED 2/5/2021
CLIENT WILLIAM GUMAN
LOCATION CURTIS ROAD

REMARKS

DRY TO 19.5', 2/17/21

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
	SAND, SLIGHTLY SILTY TO SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE TO DENSE, MOIST		14	5.6	1
5			23	6.2	1
10			36	3.3	1
15			34	7.7	1
20			35	5.7	1

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TEST BORING LOG

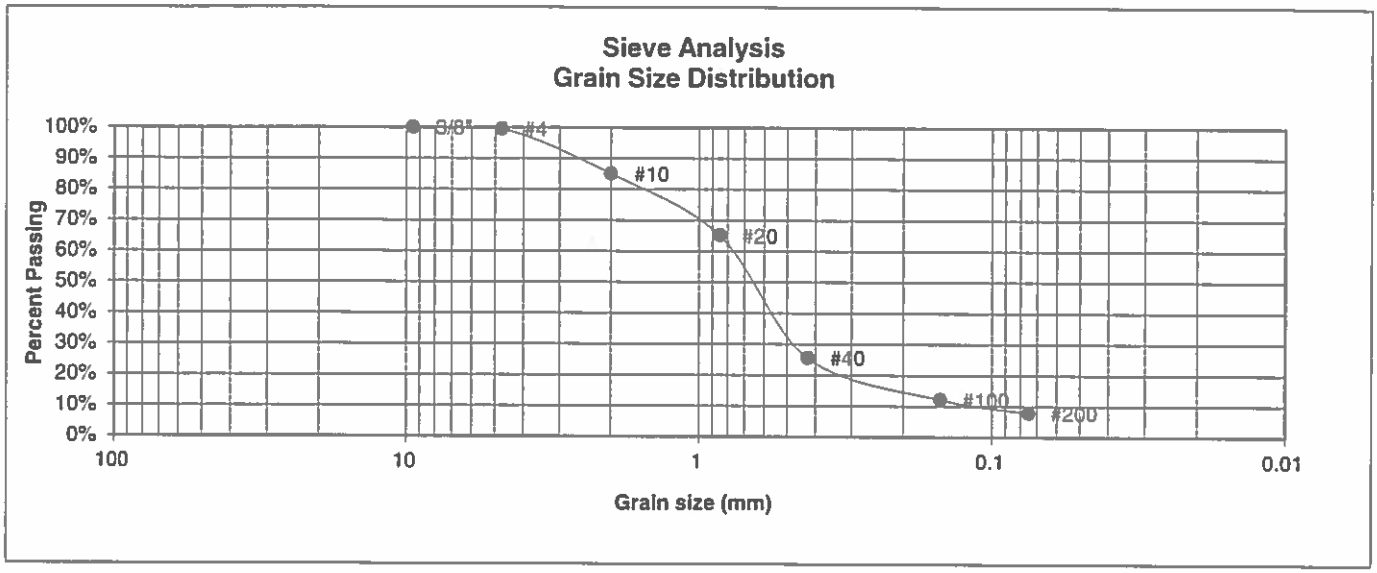
DRAWN:	DATE	CHECKED:	DATE:
		LLL	2/12/21

JOB NO: 210181

FIG NO: A-1

APPENDIX B: Laboratory Test Results

BORING NO.	1	UNIFIED CLASSIFICATION	SM-SW	TEST BY	BL
DEPTH(ft)	2-3	AASHTO CLASSIFICATION		JOB NO.	210181
CLIENT	WILLIAM GUMAN				
PROJECT	CURTIS ROAD				



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.5%
10	85.1%
20	65.1%
40	25.6%
100	12.1%
200	7.9%

Atterberg Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)

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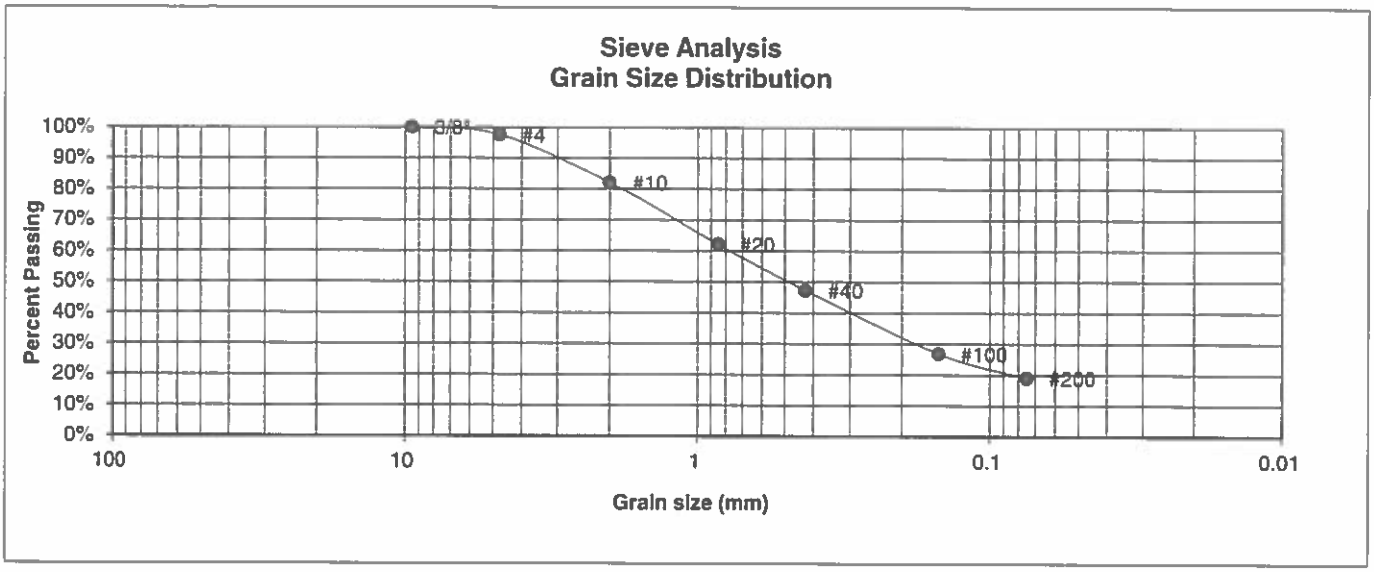
LABORATORY TEST RESULTS

DRAWN:	DATE:	CHECKED:	DATE:
		LLL	2/12/21

JOB NO.: 210181

FIG NO.: B-1

BORING NO.	1	UNIFIED CLASSIFICATION	SM	TEST BY	BL
DEPTH(ft)	20	AASHTO CLASSIFICATION		JOB NO.	210181
CLIENT	WILLIAM GUMAN				
PROJECT	CURTIS ROAD				



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	97.6%
10	82.1%
20	62.1%
40	47.3%
100	26.9%
200	19.0%

Atterberg Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)

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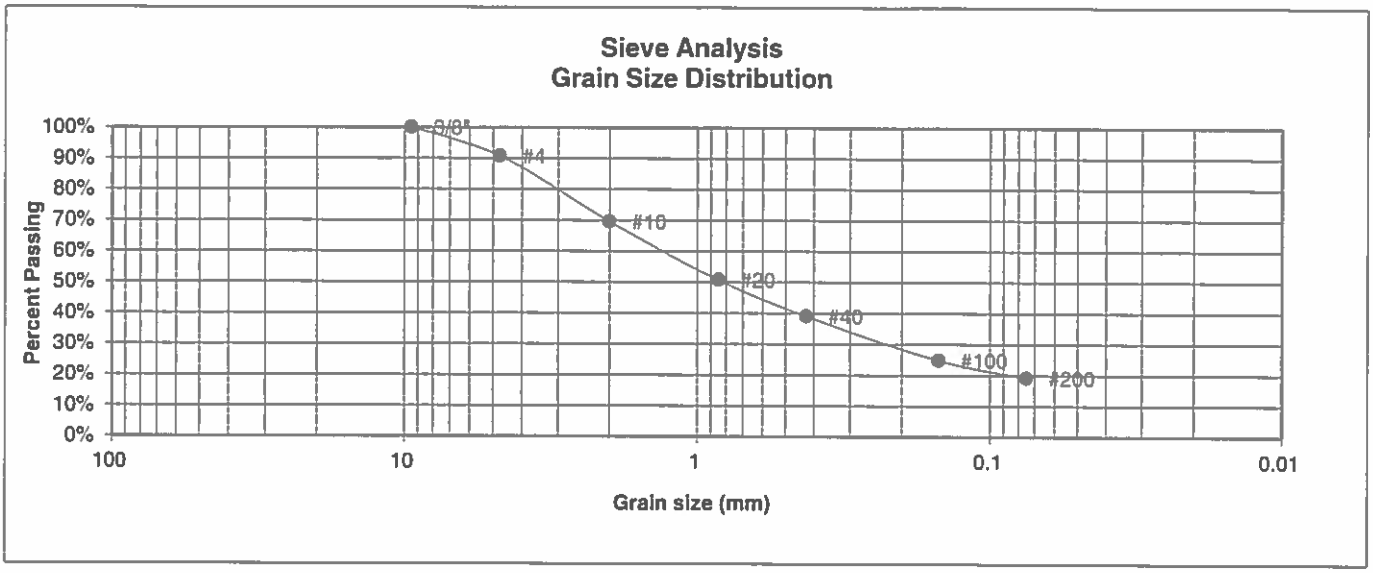
LABORATORY TEST RESULTS

DRAWN:	DATE:	CHECKED:	DATE:
		LLL	2/12/21

JOB NO: 210181

FIG NO: B-2

BORING NO.	2	UNIFIED CLASSIFICATION	SM	TEST BY	BL
DEPTH(ft)	5	AASHTO CLASSIFICATION		JOB NO.	210181
CLIENT	WILLIAM GUMAN				
PROJECT	CURTIS ROAD				



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	90.8%
10	69.6%
20	50.8%
40	39.1%
100	24.9%
200	19.2%

Atterberg Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)

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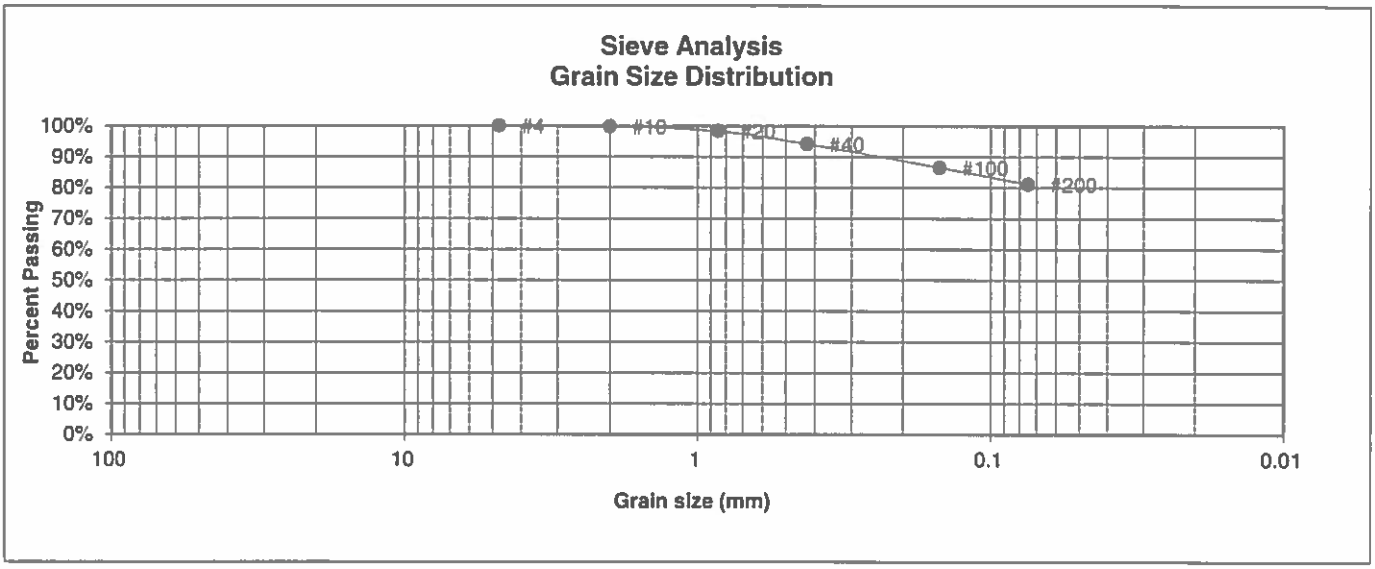
LABORATORY TEST RESULTS

DRAWN:	DATE:	CHECKED:	DATE:
		LLL	2/12/24

JOB NO.: 210181

FIG NO.: B-3

BORING NO.	1	UNIFIED CLASSIFICATION	ML	TEST BY	BL
DEPTH(ft)	35	AASHTO CLASSIFICATION		JOB NO.	210181
CLIENT	WILLIAM GUMAN				
PROJECT	CURTIS ROAD				



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.8%
20	98.3%
40	94.1%
100	86.5%
200	81.1%

Atterberg Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)

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LABORATORY TEST RESULTS

DRAWN:	DATE:	CHECKED:	DATE:
		LLL	2/12/21

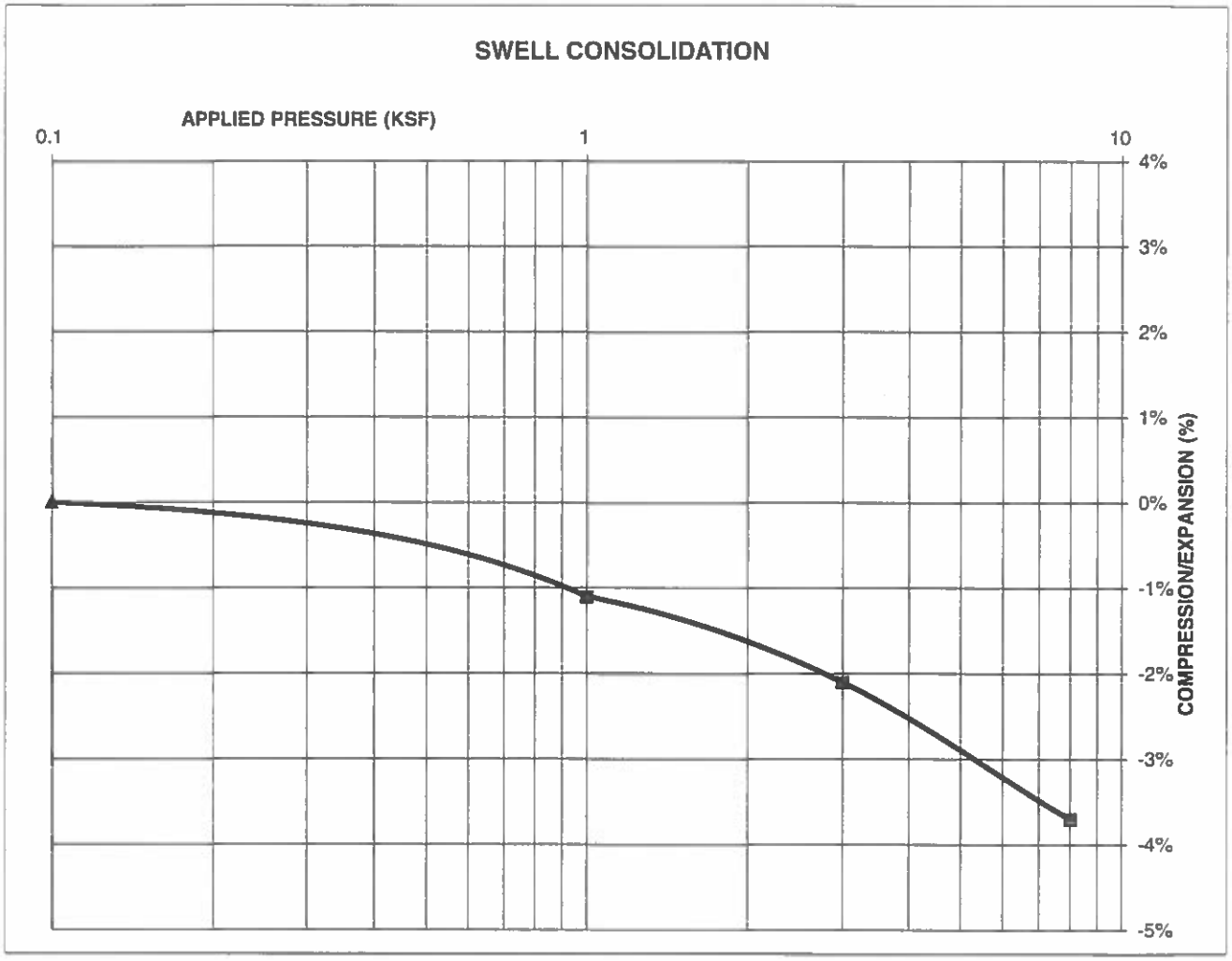
JOB NO.: 210181

FIG NO.: B-4

CONSOLIDATION TEST RESULTS

SAMPLE FROM:	1	DEPTH(ft)	35
DESCRIPTION	SILTSTONE, SANDY		
NATURAL UNIT DRY WEIGHT (PCF)			112
NATURAL MOISTURE CONTENT			16.8%
SWELL/CONSOLIDATION (%)			0.0%

JOB NO. 210181
CLIENT WILLIAM GUMAN
PROJECT CURTIS ROAD



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SWELL CONSOLIDATION
TEST RESULTS

DRAWN:	DATE:	CHECKED:	DATE
		LLL	2/12/21

JOB NO: 210181
FIG NO: B-5