Deviations are needed

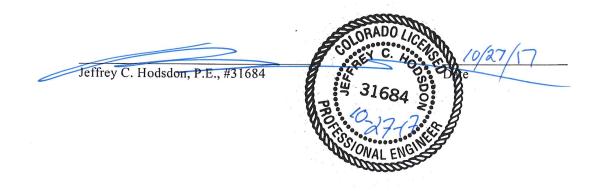


LSC TRANSPORTATION CONSULTANTS, INC. 545 East Pikes Peak Avenue, Suite 210 Colorado Springs, CO 80903 (719) 633-2868 FAX (719) 633-5430 E-mail: <u>lsc@lsctrans.com</u> Website: http://www.lsctrans.com

Falcon Marketplace Driveway Permit Resubmittal Updated Traffic Impact Analysis (LSC #164350) October 23, 2017

#### **Traffic Engineer's Statement**

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



#### **Developer's Statement**

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

201 Date

## Falcon Marketplace Driveway Permit Resubmittal

## **Updated Traffic Impact Analysis**

Prepared for:

Hummel Investments, LLC 8117 Preston Road, Suite 120 Dallas, TX 75225 (214) 416-9820

Prepared by:

LSC Transportation Consultants, Inc. 545 East Pikes Peak Avenue Colorado Springs, CO 80903 (719) 633-2868

LSC #164350

October 23, 2017

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#### **TRAFFIC REPORT**

- This updated traffic report has been prepared to address staff comments.
- This report presents analysis of the proposed right-in-only access to Woodmen Road combined with a proposed roundabout intersection with the Woodmen Frontage Road in the southwest corner of the site.
- The report also includes, for comparison purposes, analysis assuming no right-in-only access.
- This report contains short- and long-term traffic projections for Falcon Marketplace, other area properties, and study area roadways and intersections. Traffic analysis has been performed based on the projected volumes.
- Based on the analysis, detailed findings and study area roadway improvements have been presented for both "with" and "without" the proposed right-in access (and accompanying roundabout) scenarios.
- The overall report recommendation is to approve the proposed right-in-only access from Woodmen Road and accompanying roundabout. Analysis results indicate significantly improved area traffic circulation and benefits to operations at the study area intersections.

#### MATERIAL CHANGE TO THE RIGHT-IN DESIGN AND BENEFITS OF NEW DESIGN

- The inclusion of a roundabout into the design of the right-in access will provide for public access to the westbound frontage road—a significant change to the previous design. This enhancement will not only provide better access for these nearby residential and non-residential developments, but will also offer superior access for emergency response vehicles thereby reducing their response times to all properties on the north side of Woodmen between Meridian and Golden Sage—a significant and meaningful change.
- The new design for the right-in with a roundabout **allows for direct public access to the Woodmen Frontage Road** for passenger vehicles, trucks, and buses, as well as fire and emergency response vehicles as requested by the Falcon Fire Protection District. This accommodation will significantly improve the access to the Courtyards residential development, Mountain View Electric Association (MVEA), Falcon School District 49 (and other properties), which currently have poor access.
- Due to its configuration, the previously requested right-in-only access would have essentially served only the proposed Falcon Marketplace development as there was no public access from the right-in to the Woodmen Frontage Road. Additionally, the new configuration includes changing the required internal roadway (required by the 2008 BOCC-approved rezoning of the

site), which extends from Eastonville to the Woodmen Frontage Road, from a private access easement drive to a public right-of-way.

- The previously requested right-in-only was configured to direct traffic from westbound Woodmen northward into the site only, with no "direct" access for westbound travel to the neighboring residential and non-residential properties along the Woodmen Frontage Road west of the proposed Falcon Marketplace development. In the previous design, access for westbound travel along the Woodmen Frontage Road would have required a circuitous route into Falcon Marketplace in order to reverse direction.
- With this new design of the right-in combined with a roundabout, residents, employees, and other motorists traveling to the properties along the frontage road from westbound Woodmen or northbound Meridian (many traveling from eastbound US Highway 24) would have **a new direct connection to the east end of the Woodmen Frontage Road** and will no longer need to do either of the following to access their destination:
  - travel west for over a mile along Woodmen Road to the Golden Sage/Woodmen intersection, make a 180-degree turn and travel back to the east along the Woodmen Frontage Road, or alternatively,
  - travel north to the Eastonville/Meridian intersection, turn left and travel through the proposed Falcon Marketplace development, a route of over one-half mile including a traffic signal.
- The proposed right-in-only with the roundabout would also **benefit operations at the Woodmen/Golden Sage and Woodmen Frontage Road/Golden Sage** intersection by reducing existing and future traffic turning movements from these closely spaced intersections.
- This site is within the commercial "node" of Falcon as defined in the Falcon/Peyton Small Area Master Plan and an access from Woodmen Road at the proposed location is reasonable for a regional commercial development as allowed by the Commercial Regional zoning approved for the site in 2008. Furthermore, additional access points to Woodmen were contemplated by the BOCC-approved resolution adopting the Woodmen Road Access Management Plan.

#### SUMMARY OF TRAFFIC OPERATIONS ANALYSIS AND COMPARISON

- The three key intersections within this study are (a) the southwest roundabout at the proposed right-in-only access from Woodmen, (b) the meridian/Eastonville intersection, and (3) the Meridian/Woodmen intersection.
- The comparison between the two analysis scenarios (with and without the proposed right-inonly access from Woodmen) with respect to operations at the Meridian/Eastonville intersection shows significantly better operations with the proposed Woodmen access/roundabout. In addition to the intersection analysis, the right-in-only access would significantly reduce overall travel times and emergency response times for those traveling to destinations along the frontage road and Falcon Marketplace.

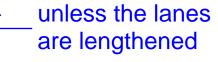
#### **Right-In-Only Access/Roundabout**

- The continuous lane along westbound Woodmen Road between Meridian and the access is projected to operate at weaving LOS C during the morning peak hour and LOS B during the afternoon peak hour.
- Multiple methods of analysis indicate level of service A for all roundabout approaches during the peak hours **based on 2040 volumes**.
- The southwest roundabout will see **minimal queuing** for traffic exiting Woodmen Road and entering the roundabout. A maximum queue during the peak hour of about 100-120 feet is projected and even this maximum queue will clear quickly as the queue will be more of a **"rolling" queue**.
- The proposed Woodmen access will have **little effect** on the operation of Woodmen Road as the turning movements will be right-turn in-only from westbound Woodmen Road with a continuous acceleration/deceleration lane between Meridian and the point of right-turn entry into the site.
- The roundabout has been designed to accommodate large tractor-trailer trucks/semis.

#### Meridian/Eastonville Intersection

- The northbound left turn at this intersection is projected to operate at LOS F during the afternoon peak hour without the right-in-only access off Woodmen. However, it will operate at LOS D during the afternoon peak hour with the right-in-only access off of Woodmen with the roundabout.
- The northbound left-turn queue would be significantly longer without the Woodmen-access scenario. Without the right-in-only access off of Woodmen the projected maximum queue will fill the dual left-turn lanes (457-foot queue within the lanes) and will overspill into the adjacent northbound through lane during the peak analysis interval.

#### **Meridian/Woodmen Intersection**



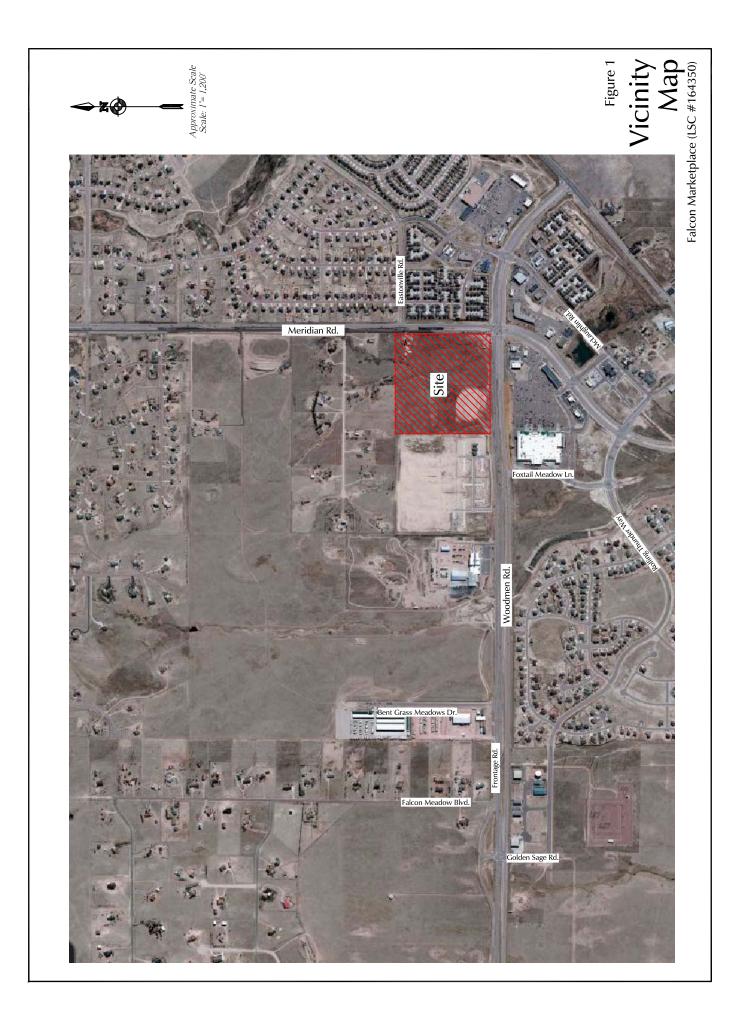
- The addition of the proposed right-in-only access off of Woodmen will provide motorists approaching from the south the option to utilize the existing dual left-turn lanes to go westbound on Woodmen to enter the project site. This allows motorists to adjust to the path of least congestion and will benefit the overall intersection. Providing this option will reduce the quantity of northbound through traffic. This will be especially helpful during the afternoon peak hour. Although the overall intersection delays shown in the table are comparable, the analysis shows failure of the northbound through movement without the right-in-only access off of Woodmen.
- Without the requested right-in-only from Woodmen, the northbound through movement is projected to operate at LOS F during the 2040 afternoon peak hour.

LSC Transportation Consultants, Inc. has prepared this updated traffic impact analysis for the 36-acre Falcon Marketplace to be located west of Meridian Road and north of Woodmen Road in the Falcon area of El Paso County, Colorado. The site is planned to be developed for a mixture of commercial and medical office land uses. Figure 1 shows the site location. Access is proposed to Meridian Road and the Woodmen Road Frontage Road. The previous report date was August 7, 2017.

The report has been updated to include analysis of a right-in-only access from Woodmen Road combined with a roundabout intersection with the Woodmen Frontage Road in the southwest corner of the site. This new right-in-only access with the proposed roundabout intersection connecting to the Woodmen Frontage Road would significantly improve access not only to the site, but also to the properties to the west along the Woodmen Frontage Road. This new provision for public access from westbound Woodmen Road to the westbound Woodmen Frontage Road is a <u>significant change</u> from the previously proposed right-in-only access configuration, which essentially only served the proposed Falcon Marketplace.

The salient points of the significant change are:

- Allows direct public access to east end of the frontage road.
- Provides badly needed access for emergency response vehicles.
- Takes traffic off of the Golden Sage intersection by providing access from the east.
- Addition of roundabout keeps traffic moving at a steady flow.
- Reduces potential backup on the frontage road.



#### **REPORT CONTENTS**

The report contains the following:

- The proposed site land uses and circulation plan for the site.
- The proposed plan to allow for a public street connection through the site between the current terminus of the Woodmen Frontage Road and Meridian Road.
- The existing and planned roadways in the study area including the number of lanes, classifications, posted speed limits, lane geometries, traffic controls, etc.
- Traffic volumes for the Meridian/Woodmen and Meridian/Eastonville intersections plus added traffic count data for the intersection of Woodmen Road and Golden Sage Road.
- The projected future peak-hour traffic volumes for the access points, internal intersections and the intersections adjacent to the site, the intersection of Golden Sage/Woodmen and along the Woodmen Frontage Road with and without the proposed right-in-only access from Woodmen Road.
- The resulting traffic impacts. The traffic impacts have been quantified by determining the future levels of service at the access points, internal intersections and adjacent intersections with and without the proposed right-in-only access from Woodmen Road.
- Recommended improvements.

# Site Land Use and Access/Circulation Plan

#### SITE CONTEXT

The site is located within the downtown Falcon commercial/service area. This site is part of the Falcon "commercial node." Several shopping centers exist south and southeast of this site on the south side of Woodmen Road. The Safeway shopping center is located to the east, and the Bent Grass Commercial Center is directly north of the site and the Owl Lane area.

#### LAND USE

The 36-acre site is located north of Woodmen Road and west of Meridian Road. The site is planned to contain a large grocery store anchor with associated gas station. The peripheral development lots are planned to include a pet supply store, in-line retail buildings, three free-standing fast-food restaurants, a coffee shop with drive-through, and an urgent/primary care clinic.

#### ACCESS AND CIRCULATION

Full-movement site access is proposed from Meridian Road aligning with Eastonville Road and via a connection to the current terminus of the Woodmen Frontage Road. A right-in/right-out access to Meridian Road is also proposed between Eastonville and Woodmen. In addition to the connection to the current terminus of the Woodmen Frontage Road, a right-in-only access from westbound Woodmen Road is also proposed in the southwest corner of the site.

Figures 2 and 3 show the access/intersection spacing for Woodmen Road and Meridian Road, respectively.

The site plan also shows a street stub to the property to the north to allow for a planned future connection to Bent Grass Meadows Drive. The access points and the proposed public street connection through the site is also shown on the site plan.

#### INTERSECTION/ACCESS SIGHT DISTANCE ANALYSIS

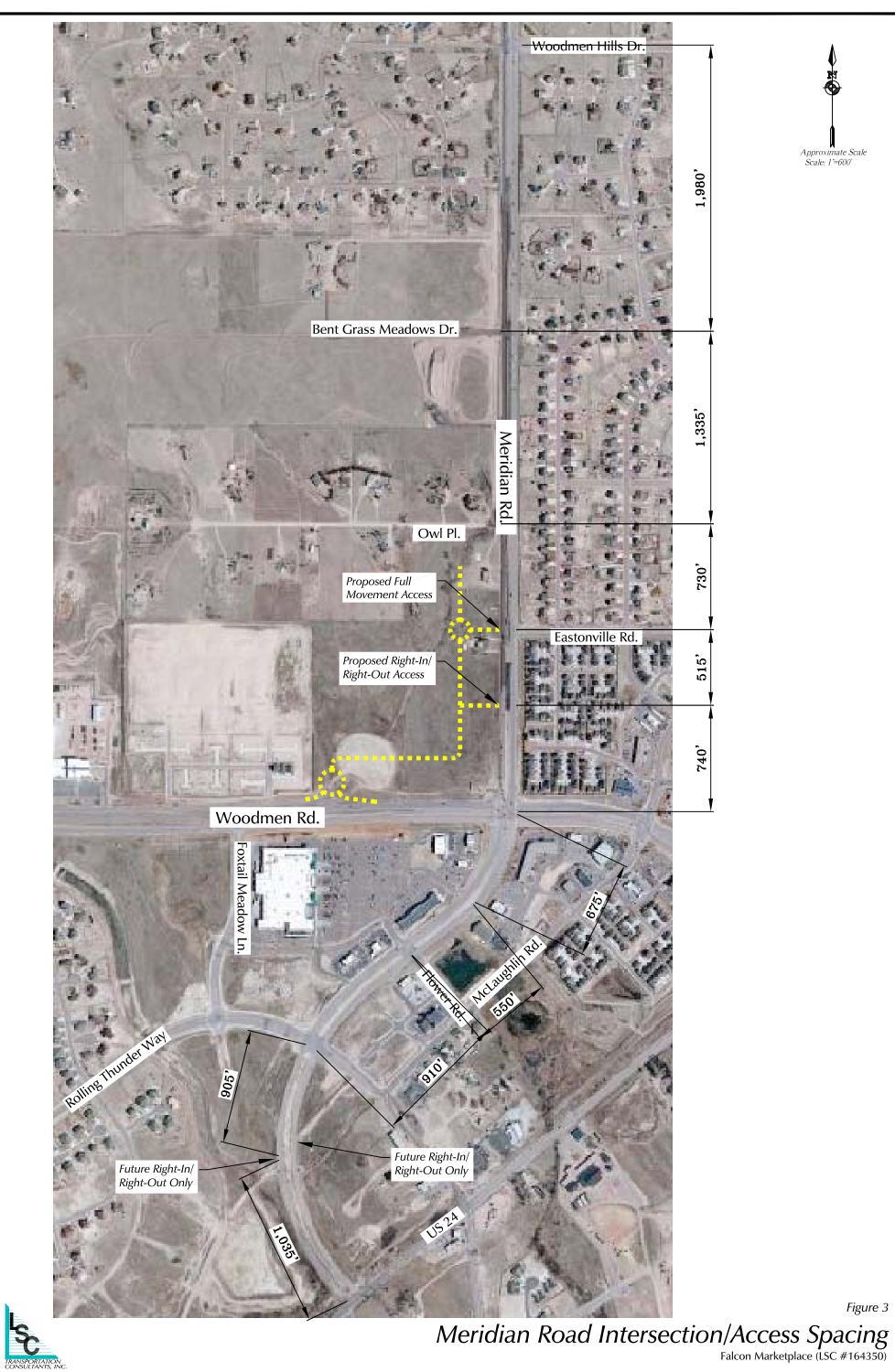
Figure 4 shows the sight distance analysis for the Meridian Road access points. There are currently no posted speed limit signs for southbound traffic on the approach to Eastonville Road and the speed limit to the north is 55 miles per hour (mph). This analysis assumes (following development of the site) a future posted speed limit of 45 mph (design speed of 50 mph) for southbound Meridian in the vicinity of and adjacent to the site. This is based on the *Meridian Road North Corridor Plan* dated December 2009.

Sight distance analysis for the internal intersections within the Preliminary Plan is included with the deviation request for Falcon Market Place.

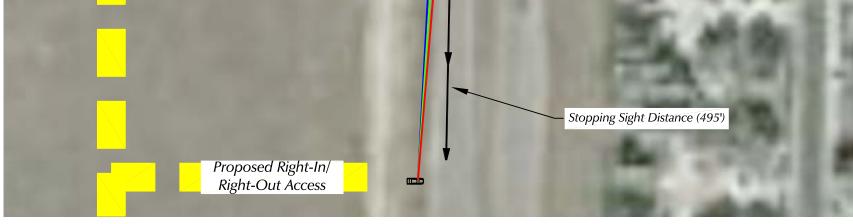
#### **Truck Turning Analysis**

Truck turning analysis using AutoTurn for the internal intersections within the Preliminary Plan is included with the deviation request for Falcon Market Place. The truck turning analysis for the roundabout proposed for the southwest corner of the site is included in this report as it is directly associated with the driveway permit application.

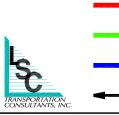








#### LEGEND:



- Required intersection sight distance for passenger cars
- Required intersection sight distance for single-unit trucks
- Required intersection sight distance for multi-unit trucks
- Required stopping sight distance =

Based on an anticipated post-development speed limit of 45mph (50 mph design speed)

Figure 4

# Sight Distance for Meridian Road Falcon Marketplace (LSC #164350)

## **Existing Roadway and Traffic Conditions**

#### AREA ROADWAYS

The roadways in the study area are shown on Figure 1 and are described below.

- US Highway (US) 24 is generally a two-lane State Highway extending east/west across Colorado connecting the Buena Vista, Colorado Springs, and Limon areas. US 24 is planned to be widened to four lanes through the Falcon area. US 24 is classified as an Expressway by the Colorado Department of Transportation (CDOT) and the *El Paso County Major Transportation Corridors Plan (MTCP)*. The posted speed limit on US 24 in the vicinity of Woodmen Road is 50 mph.
- Woodmen Road is shown on the *El Paso County 2040 Major Transportation Corridors Plan* and the *Preserved Corridor Network Plan* as a four-lane Expressway adjacent to and in the vicinity of the site. The posted speed limit on Woodmen Road adjacent to the site is 45 mph. The posted speed limit on Woodmen Road adjacent to the site is 55 mph.
- Woodmen Frontage Road is a paved two-lane frontage road along the north side of Woodmen Road. The Woodmen Frontage Road extends west from this site to its current terminus west of Golden Sage Road. The posted speed limit on the Woodmen Frontage Road is 30 mph.
- Meridian Road is shown on the MTCP as a four-lane Principal Arterial adjacent to the site. Meridian Road is currently four lanes plus some auxiliary turn lanes at intersections north of Rolling Thunder. There is a center median adjacent to the site. There are no speed limit signs specifically for the section of Meridian adjacent to the site. However, the posted limit on the section to the north is 55 mph. Meridian Road south of Rolling Thunder is not currently open and the road does not connect to US Highway 24. However, Meridian Road is planned to be opened south from Rolling Thunder to a new intersection with US 24 and extended south to Falcon Highway in the near future.
- **Eastonville Road** is a two-lane roadway extending northeast from Meridian Road to past Hodgen Road. It is shown as a two-lane Minor Arterial on the MTCP. The intersection of Meridian Road and Eastonville Road is currently stop-sign controlled.

#### **EXISTING TRAFFIC CONDITIONS**

Figure 5 shows the morning and afternoon peak-hour traffic volumes at the intersections of Woodmen Road/Meridian Road, Woodmen Road/Golden Sage Road, and Eastonville Road/Meridian Road based on counts conducted by LSC in September 2015, February 2016, March 2017, and June 2017. The traffic count reports are attached. The traffic volumes at Woodmen/Meridian have been adjusted to balance with more recent counts to the west and north.

#### ACCIDENT/CRASH HISTORY

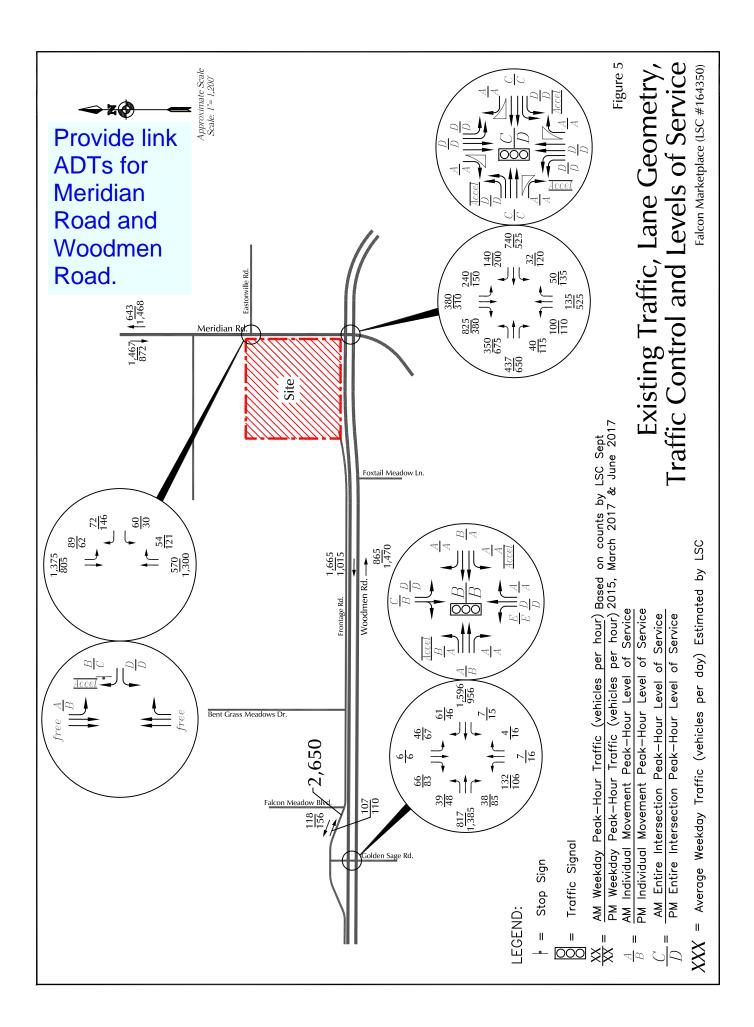
Traffic accident/crash data at study area intersections are attached. These have been provided by the Colorado State Patrol. The data show crashes by calendar year for 2014, 2015, and 2016 as well as year-to-date crashes for 2017.

There were seven reported crashes at the Woodmen/Golden Sage intersection in 2016. Five of the seven crashes in 2016 were non-intersection related. The remaining two crashes were rear-end crashes involving eastbound vehicles. The first crash resulted in property damage only and the second crash resulted in two injuries. There have also been seven crashes in 2017 (from January 1 to July 26). Of these crashes, two were non-intersection related. One involved an eastbound left-turning vehicle hitting a westbound through vehicle. This crash resulted in a fatality. The remaining four crashes were rear-end crashes (two involving westbound vehicles and two involving eastbound vehicles). These rear-end crashes all resulted in property damage only.

There were six reported crashes at the Woodmen/Meridian intersection in 2016. One of the crashes was non-intersection related. This was the only crash at this intersection in 2016 and 2017 that resulted in an injury. Three of the 2016 crashes were rear-end crashes (two involved vehicles in the westbound through lanes and one involved vehicles in the southbound to westbound acceleration lane). One 2016 crash involved a westbound vehicle that failed to stop for the red light and hit a northbound vehicle. The final 2016 crash involved a single vehicle traveling the wrong direction in the westbound acceleration lane hitting the pedestrian island in the northwest corner of the intersection. Based on the data provided, there has been one reported accident in 2017 (from January 1 to July 26). This crash was a rear-end accident involving vehicles in the eastbound left-turn lanes.

There were two reported crashes at the Eastonville/Meridian intersection in 2016. In the first crash, two vehicles were attempting to turn left from Eastonville Road onto Meridian Road. The first vehicle attempted a two-stage left turn, stopping in the median on Meridian Road, and was rear-ended by the second vehicle. This crash resulted in property damage only. The second crash involved an eastbound left-turn vehicle and a bicycle in the northbound shoulder. This crash resulted in an injury. There has been one crash at this intersection recorded in 2017 (from January 1 to July 26). This injury crash involved a southbound left-turning vehicle turning in front of a northbound through vehicle.

The estimated three-year crash/accident rate at Woodmen/Meridian is 0.46. This is based on an estimated average of 33,500 entering vehicles per day and 17 total intersection-related crashes in three years. **This is a relatively low accident rate.** For comparison, the Powers and Barnes intersection, which is one of the City of Colorado Springs' high accident locations, had a 2016 accident rate of about 2.25.



#### EXISTING LEVELS OF SERVICE

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. For reference, Table 1 shows the level of service delay ranges.

Table 1           Intersection Levels of Service Delay Ranges											
	Signalized Inte	rsections	Unsignalized Intersections								
Level of Service	Average Control Delay (seconds per vehicle)	<b>V/C</b> <sup>(1)</sup>	Average Control Delay (seconds per vehicle) <sup>(2)</sup>								
A	10.0 sec or less	less than 0.60	10.0 sec or less								
В	10.1-20.0 sec	0.60-0.69	10.1-15.0 sec								
С	20.1-35.0 sec	0.70-0.79	15.1-25.0 sec								
D	35.1-55.0 sec	0.80-0.89	25.1-35.0 sec								
E	55.1-80.0 sec	0.90-0.99	35.1-50.0 sec								
F	80.1 sec or more	1.00 and greater	50.1 sec or more								
(2) For unsignaliz	portation Research Circular ed intersections if V/C ratio i the projected average contro	s greater than 1.0 the	level of service is LOS F								

The intersections of Woodmen Road/Meridian Road and Woodmen Road/Golden Sage Road were analyzed to determine the existing levels of service using Synchro. The intersection of Eastonville Road/ Meridian Road was analyzed based on the unsignalized method of analysis procedures found in the *Highway Capacity Manual*, 6<sup>th</sup> Edition by the Transportation Research Board. As shown on Figure 5, these intersections are operating at acceptable levels of service during peak periods. **The detailed level of service analysis reports are attached**.

# **Projected Future Background Traffic Conditions**

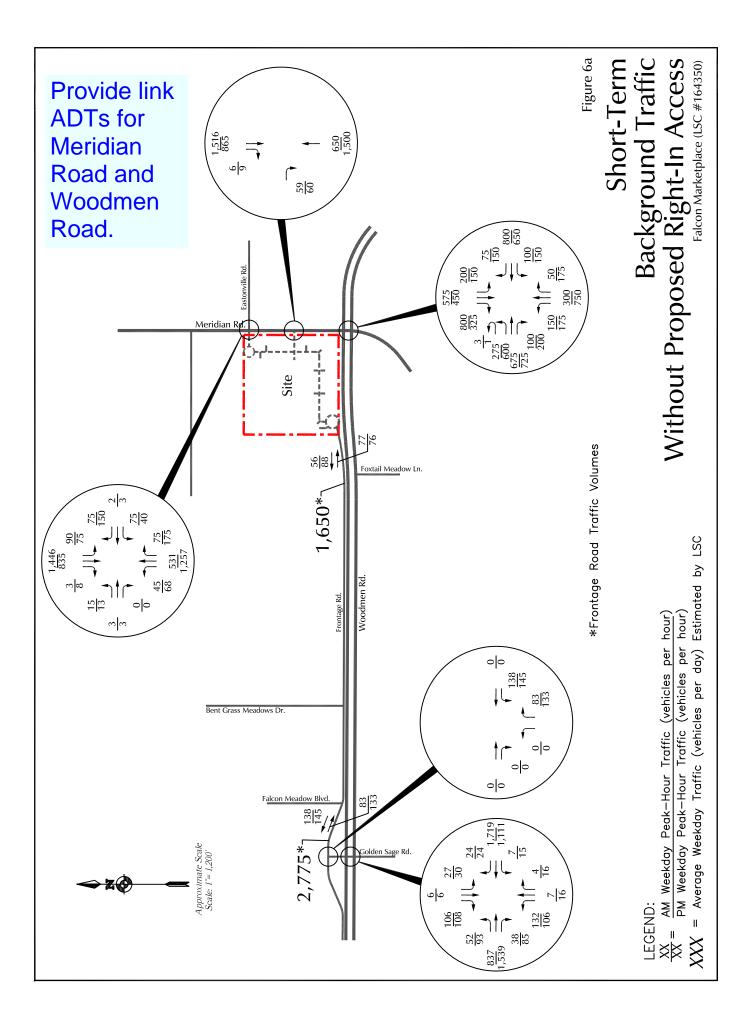
Baseline or "background" traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation and resulting added traffic volumes (site traffic is presented in the following chapter). New baseline/background traffic is the difference between future background traffic and existing traffic.

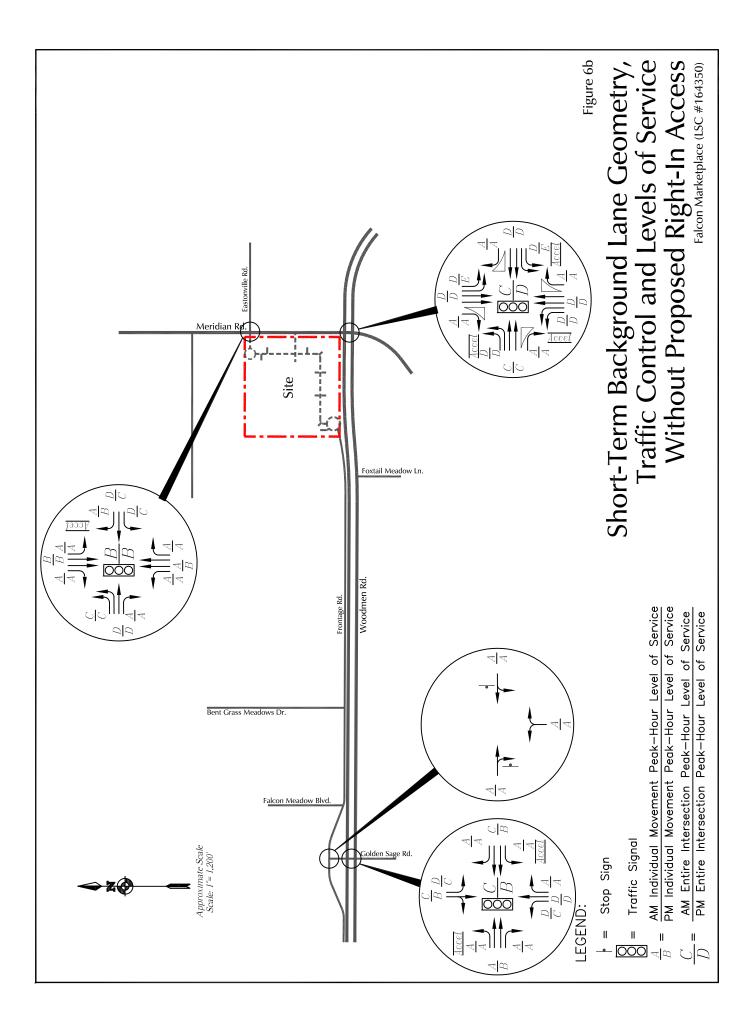
The background traffic analysis hypothetically assumes the site street connections and access points to be in place (but without site traffic). Background traffic includes the through traffic and the traffic generated by nearby developments, but hypothetically assumes zero traffic generated by the site for analysis purposes.

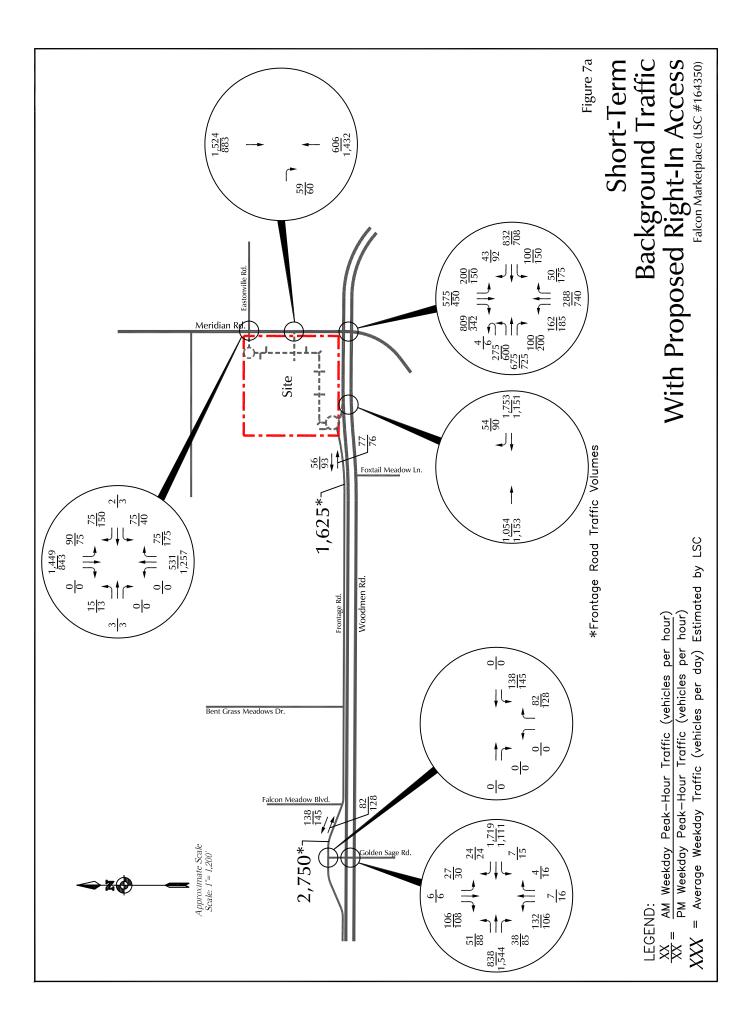
#### SHORT-TERM BACKGROUND TRAFFIC

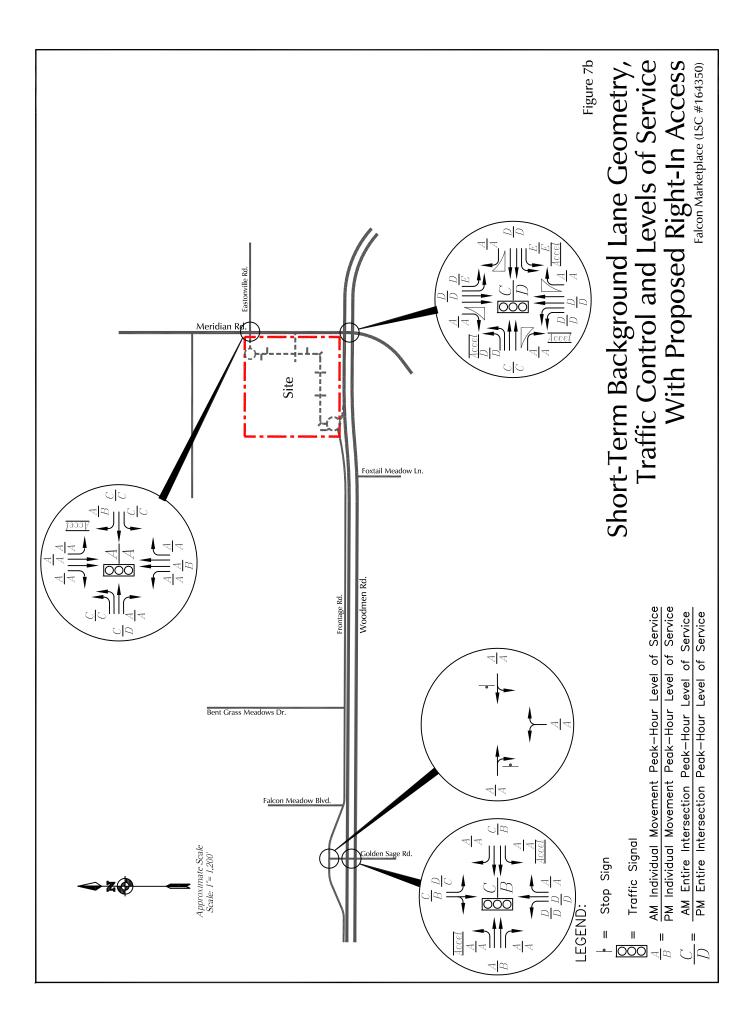
Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation and resulting site-generated traffic volumes. However, the site street connections and access points were assumed to be in place. Background traffic includes the through traffic and the traffic generated by nearby developments, but hypothetically assumes zero traffic generated by the site for analysis purposes. Figure 6a and 7a show the background traffic volumes traveling through the site and in the vicinity of the site for the short term. The short-term background traffic volumes were based on some growth in existing traffic volumes shown in Figure 5, with some adjustments to the existing traffic patterns due to the planned Meridian Road project to the south, the proposed vehicular connection through the site (between the end of the Woodmen Frontage Road and Eastonville Road) via access easement(s) and internal commercial drives, and the new west leg of the intersection of Meridian Road and Eastonville Road. The volumes shown in Figure 6a assume no access to Woodmen Road and the volumes shown in Figure 7a assume the proposed right-in-only access from Woodmen Road. The short-term background traffic volumes assume some additional traffic due to buildout of the Woodmen Courtyards development just west of the site.

Figures 6b and 7b show the lane geometry, traffic control, and level of service at the key intersections based on the short-term background volumes.





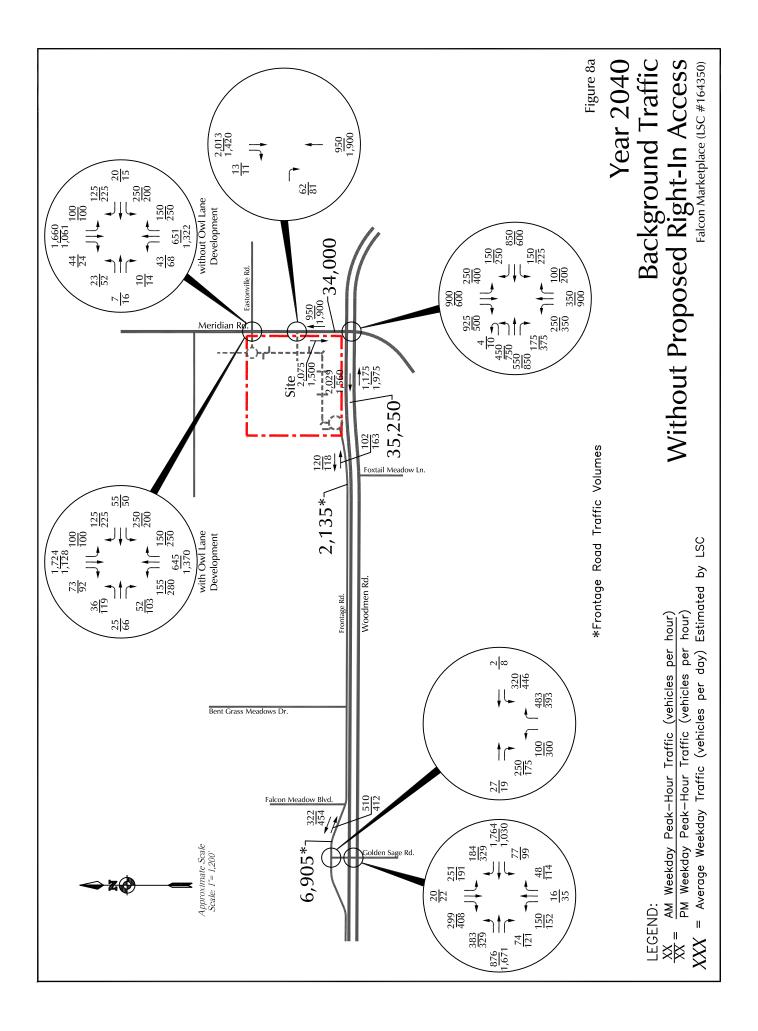


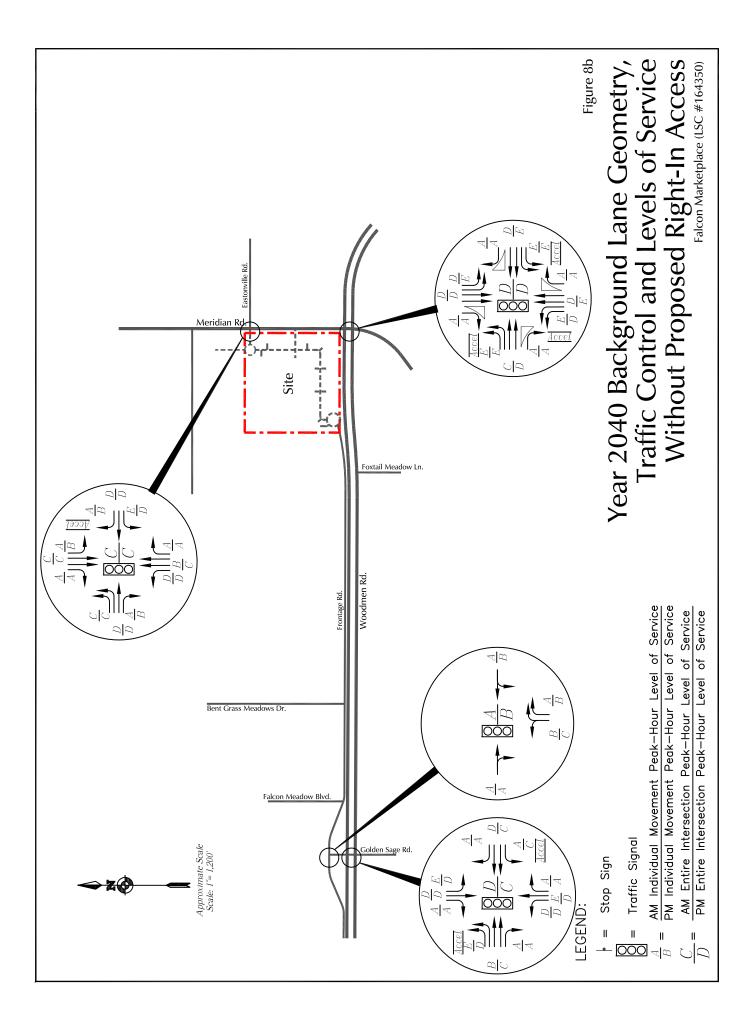


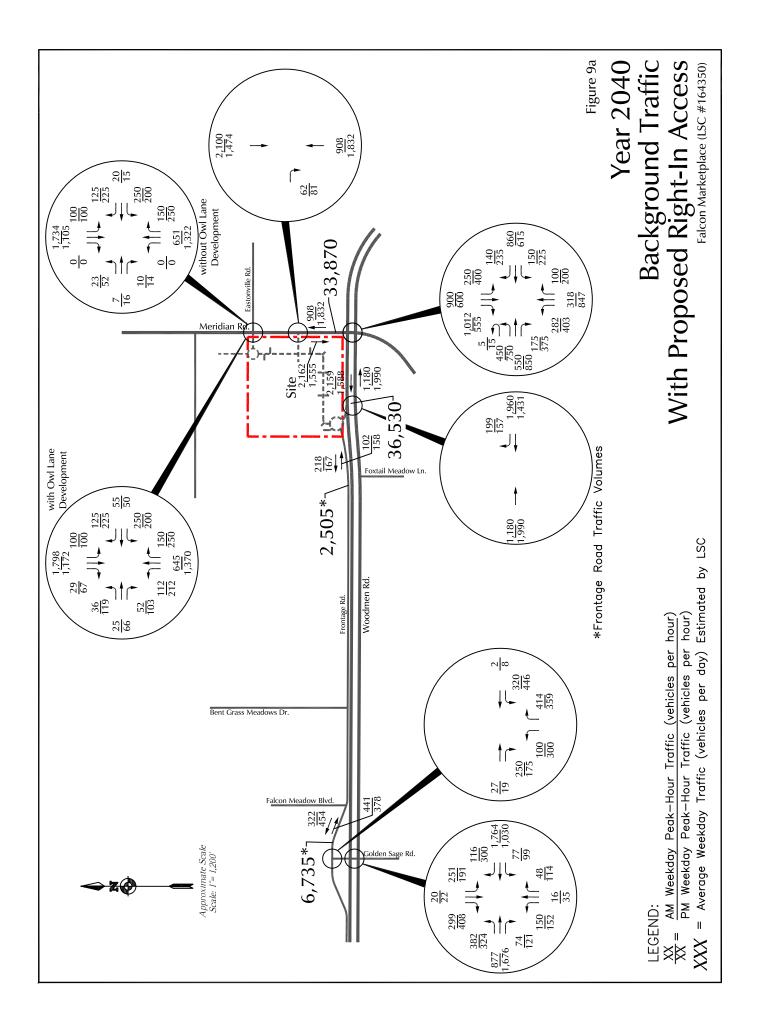
#### 2040 BACKGROUND TRAFFIC

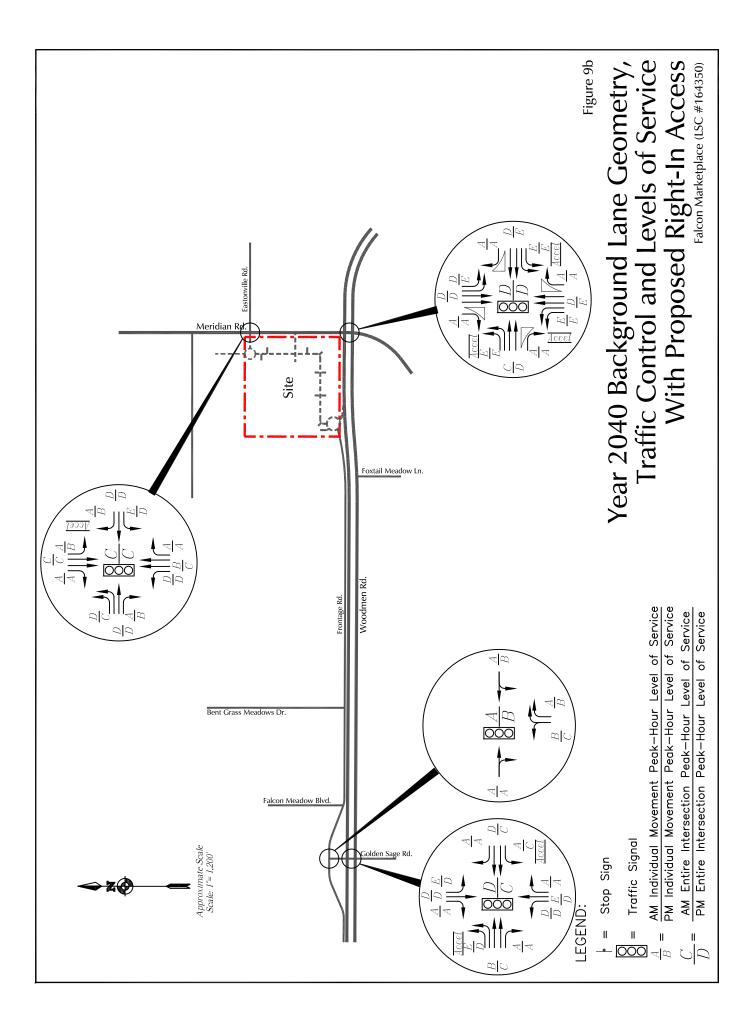
Figures 8a and 9a show the background traffic volumes for the year 2040. The volumes shown in Figure 8a assume no access from Woodmen Road and the volumes shown in Figure 9a assume the proposed right-in-only access. The 2040 background traffic volume estimates were based on the *El Paso County Major Transportation Corridors Plan (MTCP) 2040* and previous work completed in the area by LSC, including the Bent Grass Subdivision PUD/Preliminary Plan Updated Traffic Impact Study and the previous studies for this site, other area traffic studies, and traffic count data. The 2040 background traffic includes buildout of the Bent Grass subdivision, the Latigo site northeast of Bent Grass Meadows Drive/Woodmen Frontage Road (assuming the current I-2 industrial zoning -- although previous reports have been prepared contemplating rezoning to commercial/shopping center land uses), and potential Owl Lane redevelopment for commercial land uses with the planned north/south street connection between Eastonville and Bent Grass Meadows Drive. Increases in through traffic are also included. The 2040 background traffic estimates also take into account the Stapleton Drive extension to the west to the Briargate Parkway/Black Forest Road intersection.

Figures 8b and 9b show the lane geometry, traffic control, and level of service at the key intersections based on the 2040 background volumes.









# Trip Generation, Distribution, and Traffic Volume Estimates

#### **TRIP GENERATION**

Estimates of the traffic volumes expected to be generated by the existing and proposed land uses within the study area were made using the nationally published trip generation rates found in *Trip Generation*, *9th Edition, 2012* by the Institute of Transportation Engineers (ITE). Table 2 shows the trip generation estimates.

The total number of vehicle-trips generated by the land uses has been reduced to account for the internal vehicle-trips made within the site between land uses, without use of the external streets surrounding the site. Table 2 shows the number of internal trips assumed for each land use. The internal trip reduction is an estimate by LSC based on National Highway Cooperative Highway Research Program (NCHRP) Report 684 *Enhancing Internal Trip Capture Estimation for Mixed-Use Developments*. The results of the spreadsheet model are attached.

The total number of external new impact vehicle-trips generated by the retail land uses has been reduced to take into account the "pass-by" and "diverted link" phenomena. A pass-by trip is made by a motorist who would already be on the adjacent roadways regardless of the proposed development, but who stops in at the site while passing by. The motorist would then continue on his or her way to a final destination in the original direction. The pass-by percentages shown on Table 2 are from the *Trip Generation Handbook - An ITE Proposed Recommended Practice, 3rd Edition, 2014* by ITE. A diverted link trip is one made by a motorist who would already be traveling on a nearby (but not adjacent) roadway regardless of this development who now uses another roadway to access the site before continuing on his or her way to a final destination in the original direction. Diverted link trips are included in the distribution percentages.

The site is projected to generate about 9,558 new external vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24-hour period. Figure 8a

											-														New Externel Tri
					Trip Ger	neration R	ates <sup>(1)</sup>		Total Trips Generated					Internal Trips <sup>(7)</sup>					Total External Trips Generated						New External Trips Generated
	Land	se Use	Trip	Average	U		Afternoon Peak Hour		Average			Afternoon Peak Hour		Average Weekday	Morning Peak Hour		Afternoon	rnoon	Average	Morning		Afternoon		_	Average
	Use		Generation	Weekday					Weekday								Peak Hour		Weekday	Peak Hour		Peak Hour		Pass-By	New Weekday
Lot	Code		Units	Traffic	In	Out	In	Out	Traffic	In	Out	In	Out	Traffic	In	Out	In	Out	Traffic In Out In	Out	Dut Trips <sup>(2)</sup>	Traffic			
	Trip Ge	eneration Estimate Based on the Currently Proposed	Plan																						
1	866	Pet Supply Superstore <sup>(3)</sup>	15 KSF <sup>(4)</sup>	38.24	0.53	0.33	1.69	1.69	574	8	5	25	25	54	1	2	3	2	520	7	3	22	23	10%	468
2	850	Supermarket	123 KSF	78.26	2.11	1.29	3.76	3.62	9,626	259	159	463	445	909	17	26	48	37	8,717	242	133	415	408	36%	5,579
3	944	Gasoline/Service Station	18 VFP <sup>(5)</sup>	168.56	6.20	5.96	6.94	6.94	3,034	112	107	125	125	286	5	8	15	12	2,748	107	99	110	113	56%	1,209
4	934	Fast-Food Restaurant with Drive-Through Window <sup>(6)</sup>	2.5 KSF	496.12	0.42	0.39	16.98	15.67	1,240	1	1	42	39	380	0	0	12	17	860	1	1	30	22	50%	430
5	820	Shopping Center	5 KSF	55.14	0.77	0.47	2.36	2.51	276	4	2	12	13	26	1	0	1	1	250	3	2	11	12	34%	165
6	848	Tire Store	7.72 KSF	24.87	1.82	1.07	1.78	2.37	192	14	8	14	18	18	0	1	1	1	174	14	7	13	17	28%	125
7	934	Fast-Food Restaurant with Drive-Through Window	3.5 KSF	496.12	23.16	22.26	16.98	15.67	1,736	81	78	59	55	532	26	12	17	24	1,204	55	66	42	31	50%	602
8	934	Fast-Food Restaurant with Drive-Through Window <sup>(6)</sup>	2.5 KSF	496.12	0.42	0.39	16.98	15.67	1,240	1	1	42	39	380	0	0	12	17	860	1	1	30	22	50%	430
9	610	Clinic	7.8 KSF	31.45	2.19	2.19	2.12	3.06	245	17	17	17	24	40	3	16	10	5	205	14	1	7	19	0%	205
10	820	Shopping Center	8 KSF	55.14	0.77	0.47	2.36	2.51	441	6	4	19	20	42	1	1	2	2	399	5	3	17	18	34%	263
11	937	Coffee/Donut Shop With Drive-Through Window	1.3 KSF	818.58	51.30	49.28	21.40	21.40	1,064	67	64	28	28	326	21	10	9	12	738	46	54	19	16	89%	81
									19,669	570	446	846	831	2,993	75	76	130	130	16,676	495	370	716	701	-	9,558

(2) Source: "Trip Generation Handbook - An ITE Proposed Recommended Practice" 3rd Edition, 2014

(3) Daily and morning peak-hour trip generation rates for Pet Supply Superstore are estimates by LSC

(4) KSF = 1,000 square feet of floor space

(5) VFP = vehicle fueling position

(6) The AM peak-hour trip generation rates have been reduced by LSC as the proposed fast-food restaurant does not serve breakfast

(6) See attached NCHRP 684 Internal Trip Capture Estimate Tool Sheets

Source: LSC Transportation Consultants, Inc.

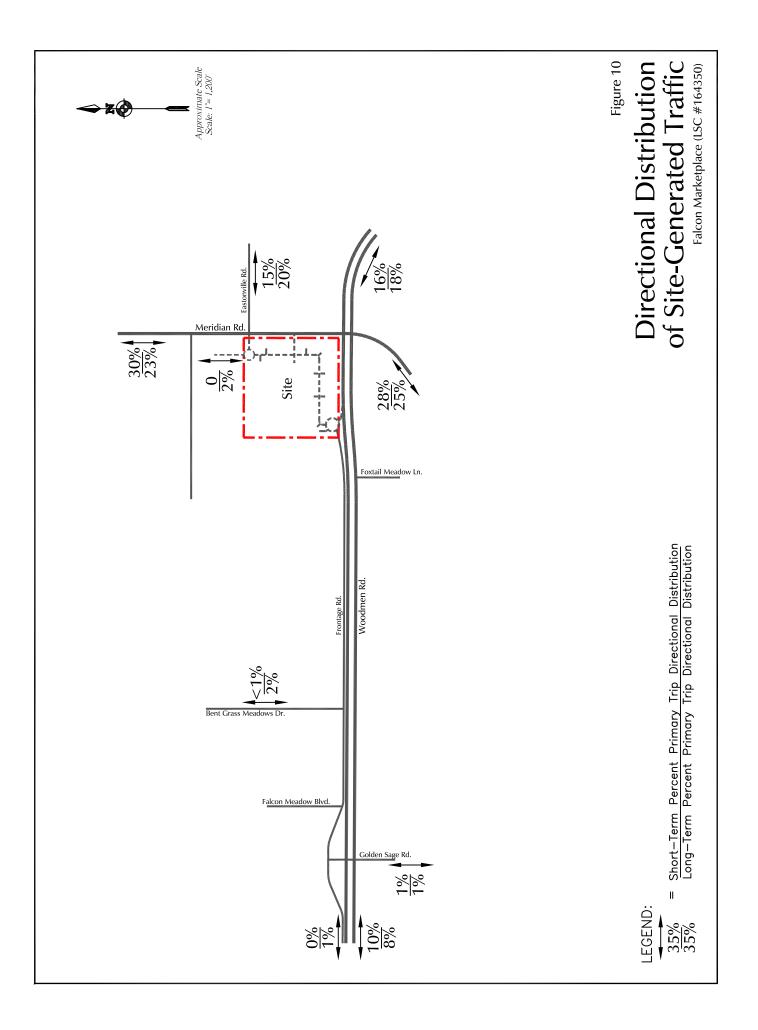
During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 495 vehicles would enter and 370 vehicles would exit the site.

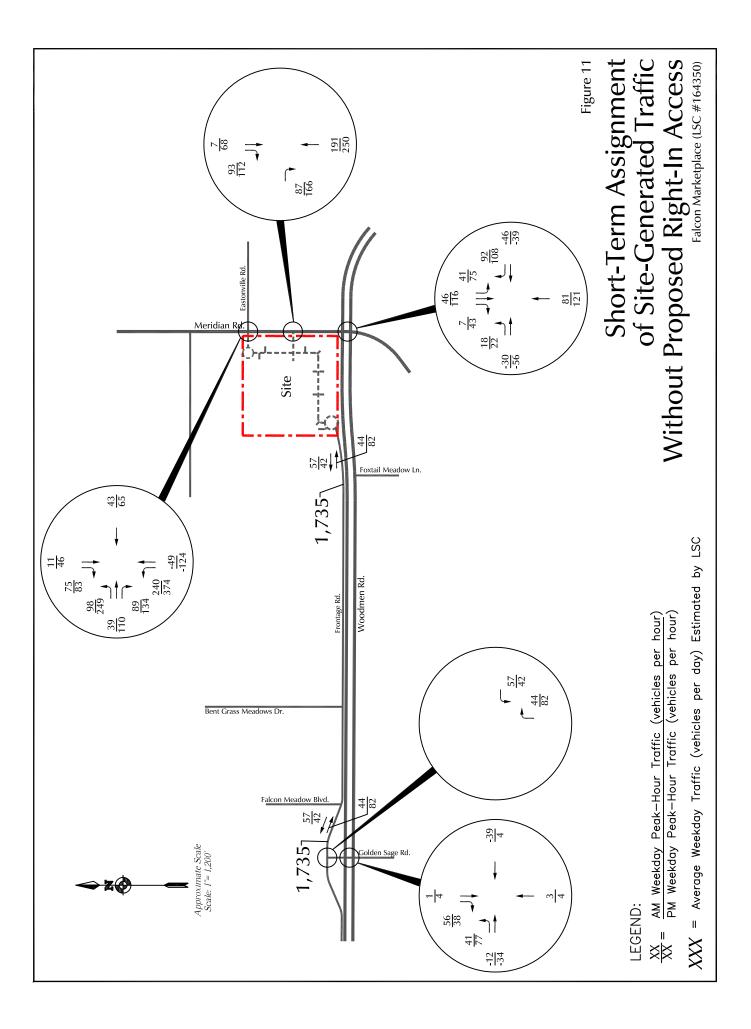
During the afternoon peak hour, which generally occurs for one hour between 4:15 and 6:15 p.m., about 716 vehicles would enter and 701 vehicles would exit the site.

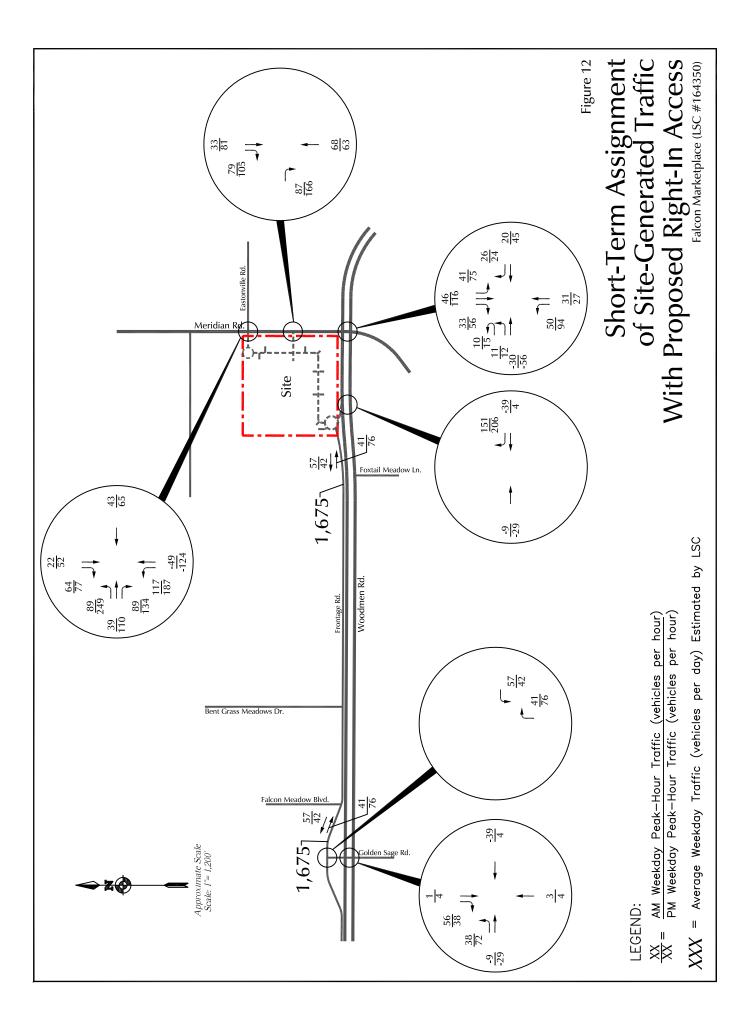
#### TRIP DISTRIBUTION AND ASSIGNMENT

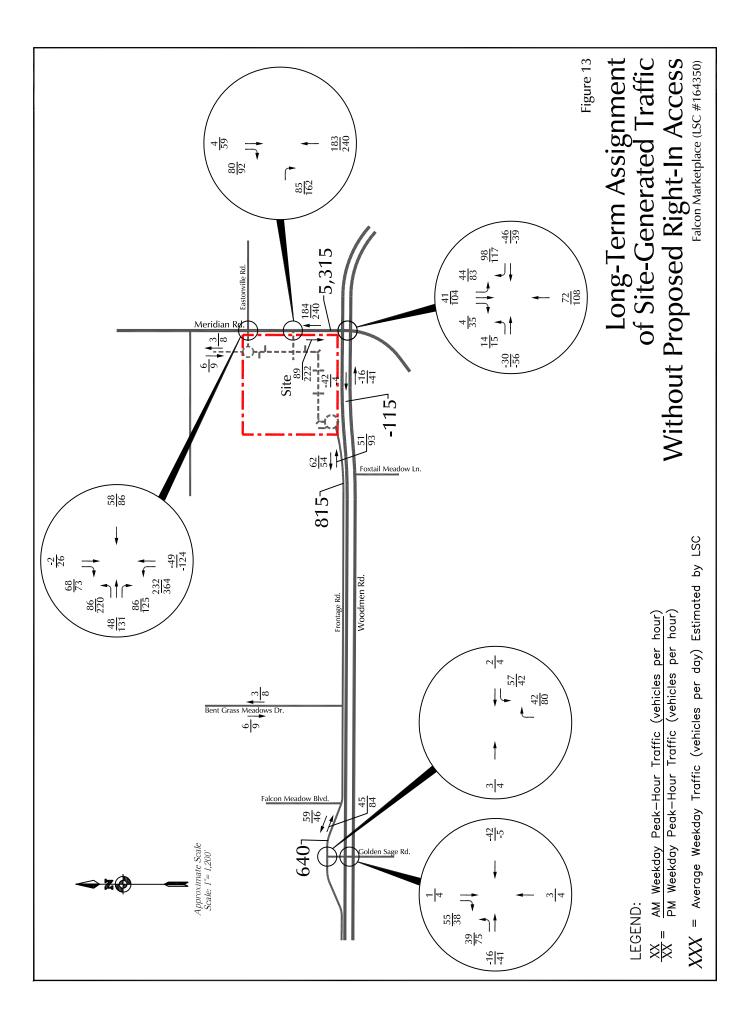
The estimated directional distribution of the site-generated traffic volumes on the adjacent roadways is an important factor in determining the site's traffic impacts. Figure 10 shows the directional distribution estimates for the primary site-generated traffic. The estimates have been based on the following factors: the site's location with respect to the Falcon area's residential, employment, and commercial areas; the balance of the northeast Colorado Springs metropolitan area and the rural areas of the county to the east; the site's proposed land uses; the site's proposed access and circulation system; and the roadway system serving the site. The short-term distribution estimate assumes the existing street network plus the Meridian Road project, and the long-term estimate assumes the future Stapleton extension to the west and additional east-west potential connections west of US 24 through Banning Lewis Ranch such as Dublin Boulevard. The pass-by trips and diverted trips were assigned based in large part on the magnitude and direction of the existing and projected background traffic volumes on the adjacent roadways.

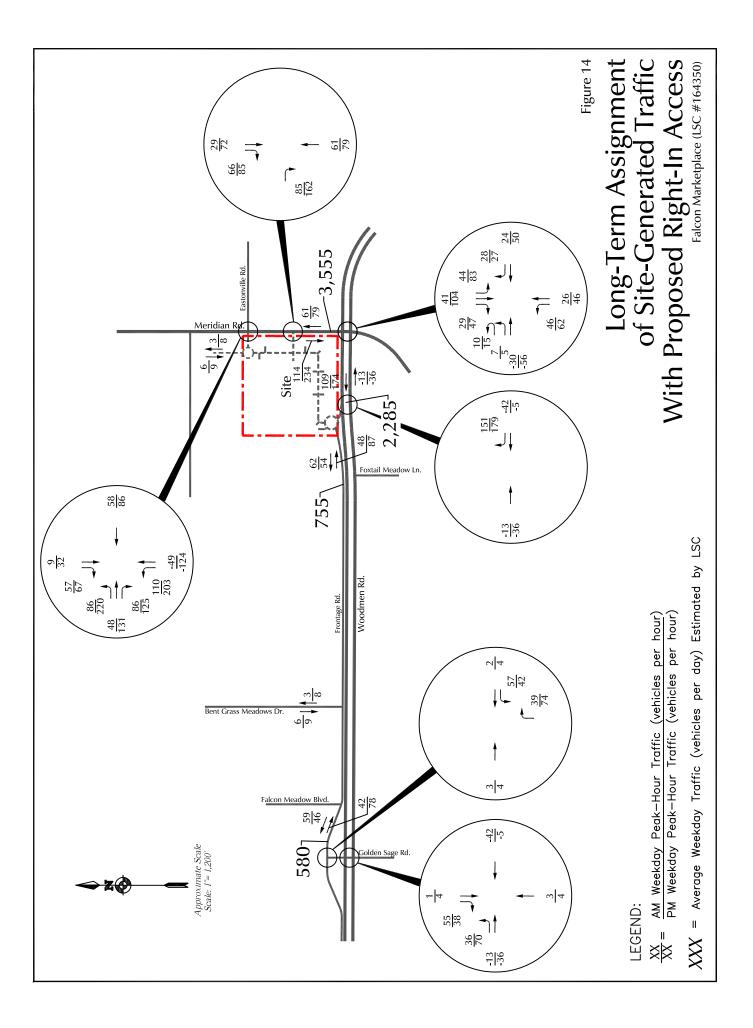
When the distribution percentages (from Figure 10) were applied to the trip generation estimates (from Table 2), the site-generated traffic volumes on the area roadways were determined. Figures 11 and 12 show the short-term site-generated traffic volumes without and with the proposed right-in-only access from Woodmen Road, respectively. Figures 13 and 14 show the long-term site-generated traffic volumes with no access to Woodmen Road and with the proposed right-in-only access from Woodmen Road, respectively.







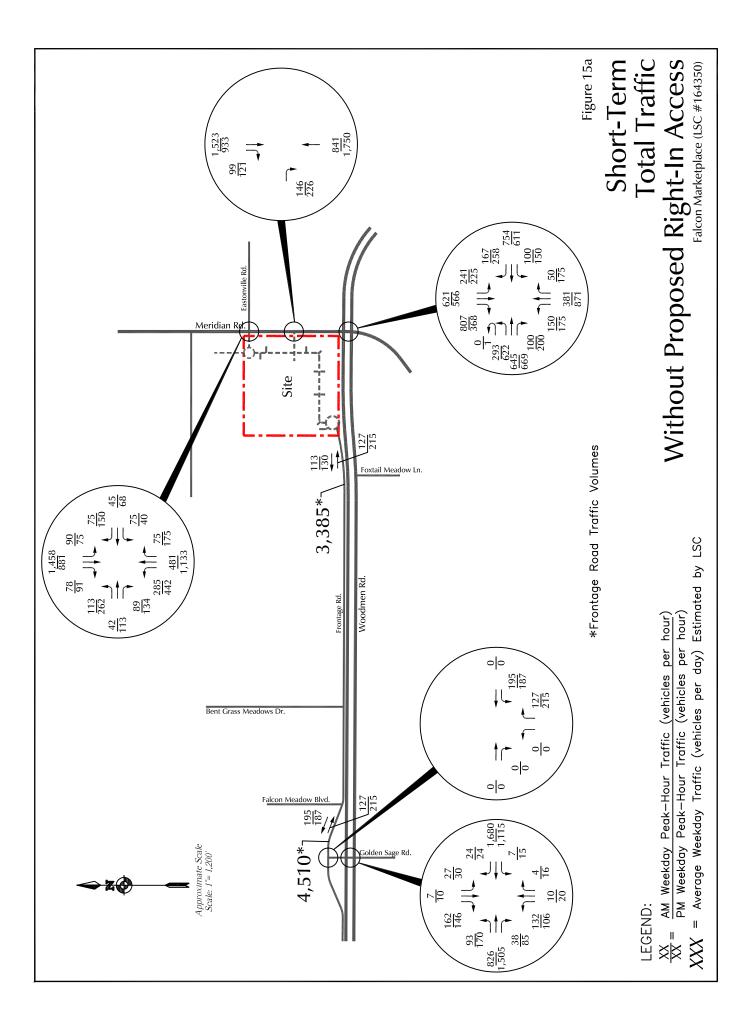


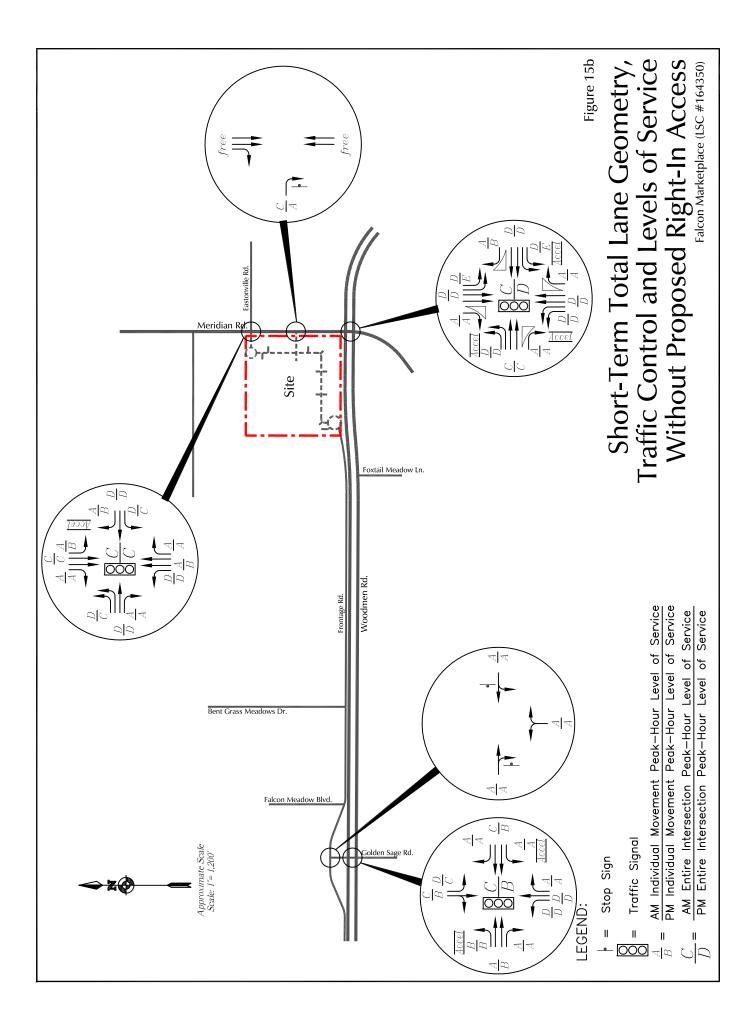


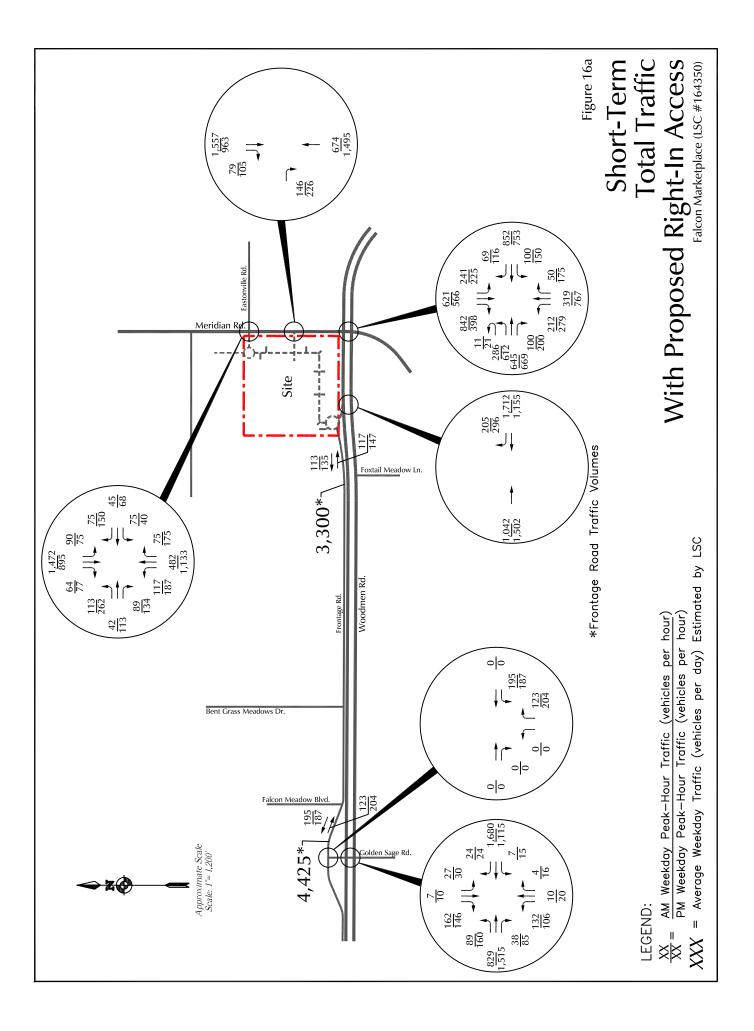
#### SHORT-TERM TOTAL TRAFFIC

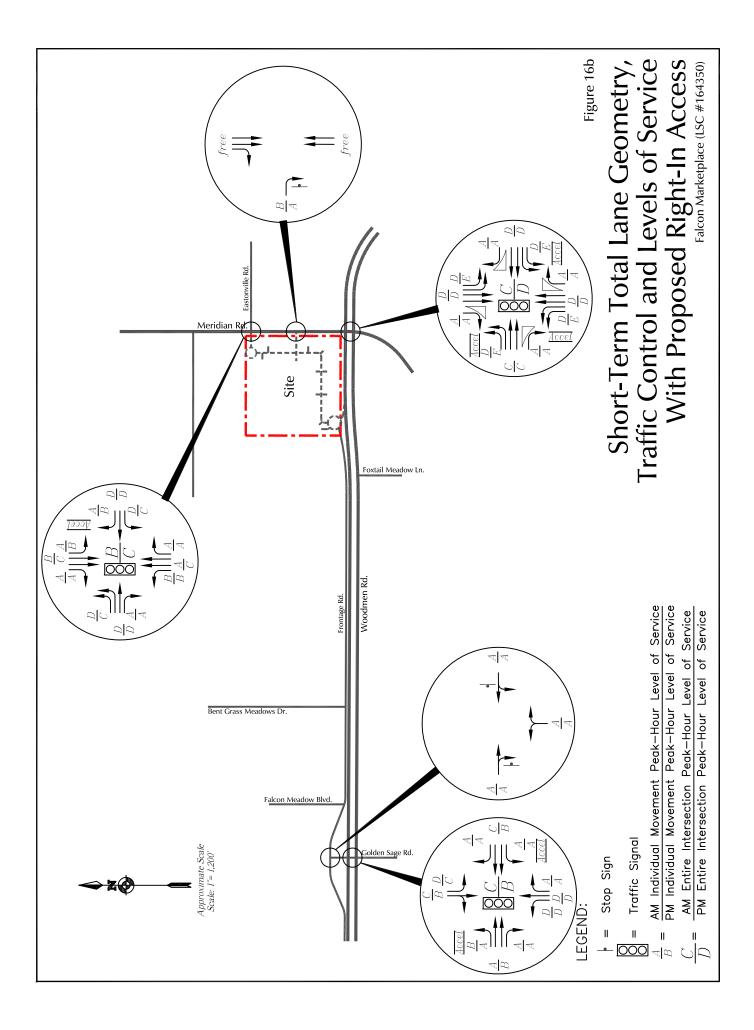
Figures 15a and 16a show the short-term total traffic volumes at the access points and key intersections adjacent to the site with no access to Woodmen Road and with the proposed right-in-only access from Woodmen Road, respectively. The volumes are the sum of the short-term background traffic volumes from Figures 6a and 7a, plus the short-term site-generated traffic volumes from Figures 11 and 12. The volumes shown in Figures 15a and 16a represent the short-term impacts of the development.

Figures 15b and 16b show the lane geometry, traffic control, and level of service at the key intersections based on the short-term total volumes.





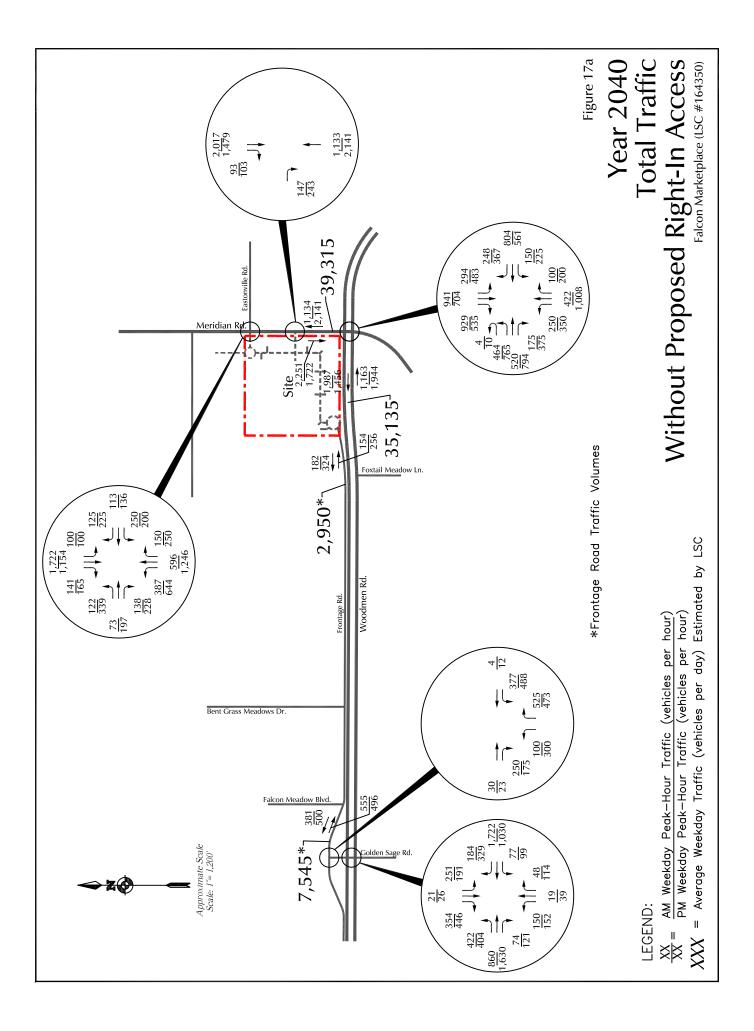


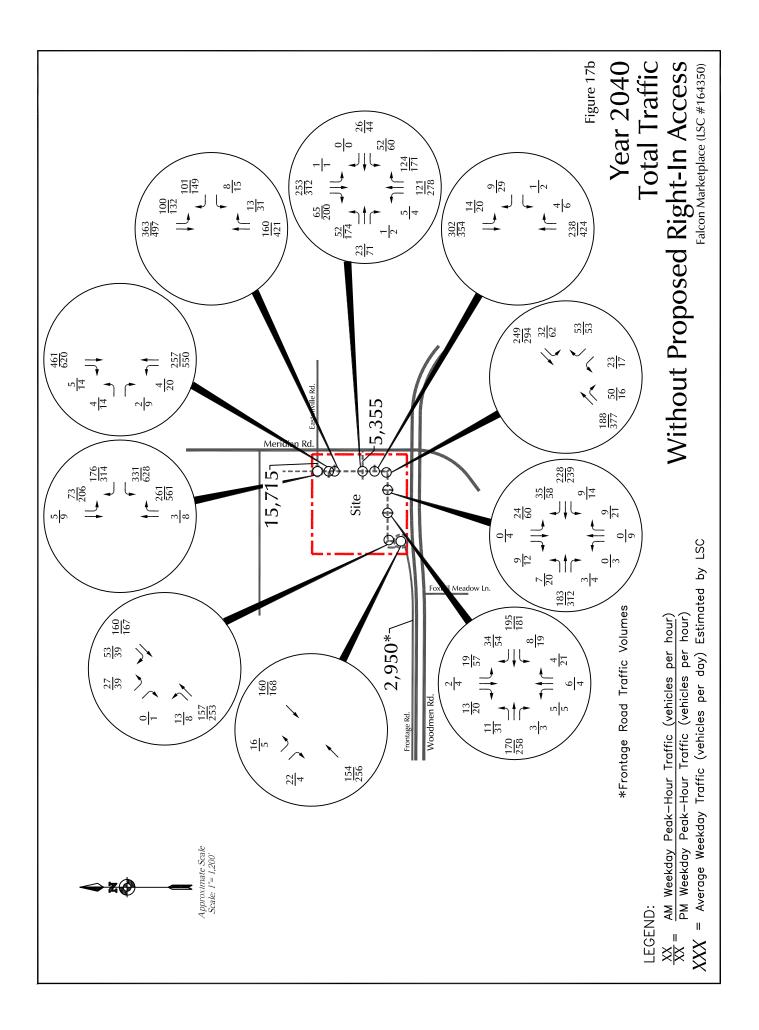


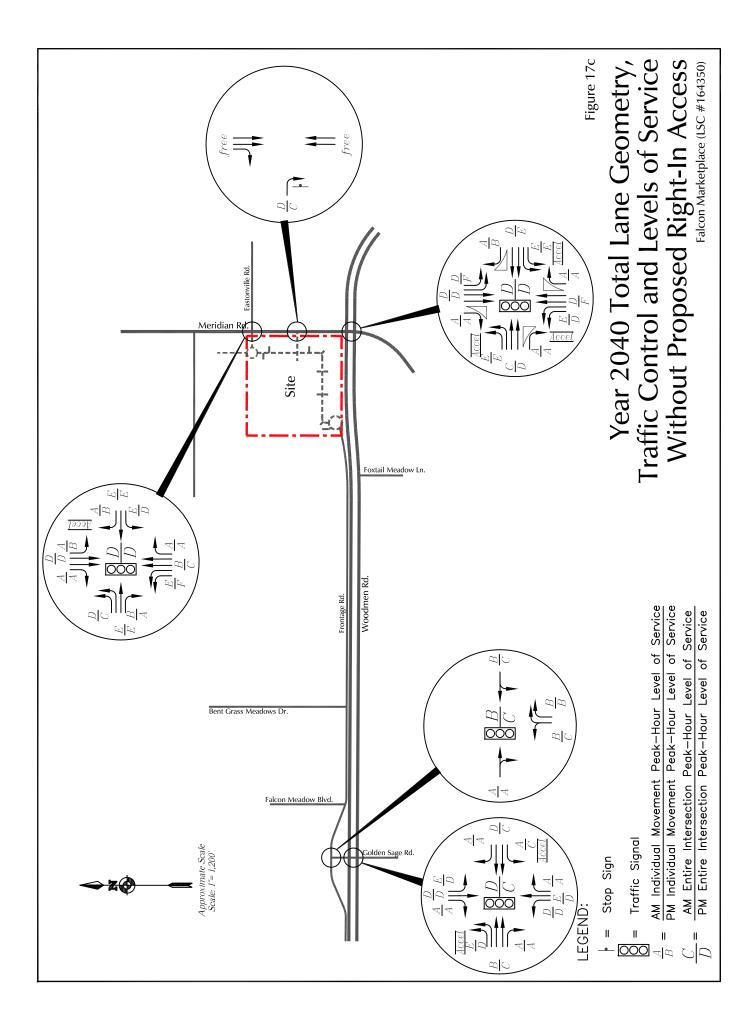
#### **2040 TOTAL TRAFFIC**

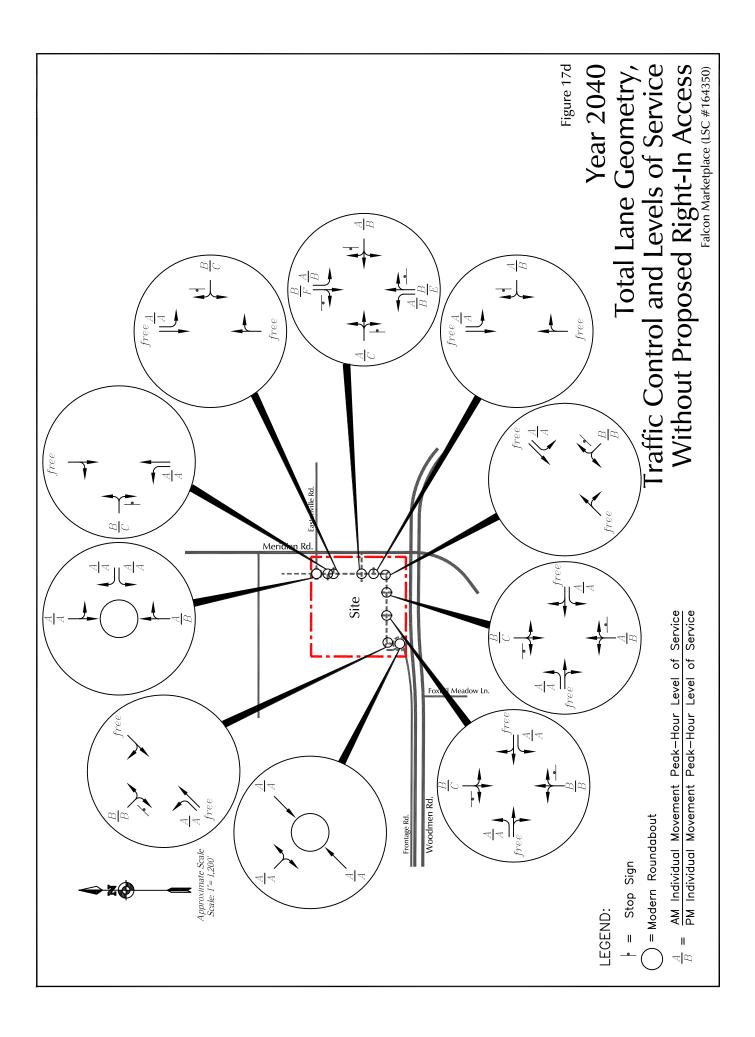
Figures 17a and 18a show the 2040 total traffic volumes at the site access points and key intersections adjacent to the site with no access to Woodmen Road and with the proposed right-in-only access from Woodmen Road, respectively. The volumes are the sum of the 2040 background traffic volumes from Figures 8a and 9a, plus the long-term site-generated traffic volumes from Figures 13 and 14. Figures 17b and 18b show the 2040 total traffic volumes at all of the proposed access points to the public internal road, which extends from the terminus of the Woodmen Frontage Road to Eastonville Road.

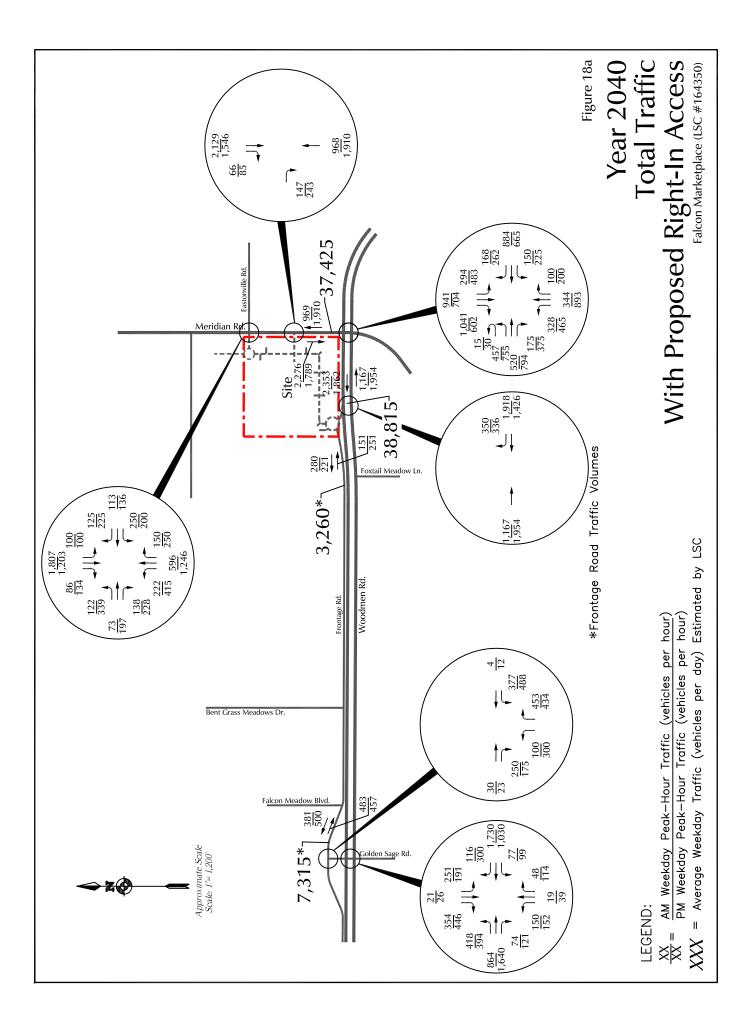
Figures 17c and 18c show the lane geometry, traffic control, and level of service at the site access points and key intersections adjacent to the site based on the 2040 total volumes. Figures 17d and 18d show the lane geometry, traffic control, and level of service at the site access points and key intersections adjacent to the site based on the 2040 total volumes. Figures 17d and 18d show the lane geometry, traffic control, and level of service at the site access points and key intersections adjacent to the site based on the 2040 total volumes. Figures 17d and 18d show the lane geometry, traffic control, and level of service at all of the proposed access points to the public internal road, which extends from the terminus of the Woodmen Frontage Road to Eastonville Road.

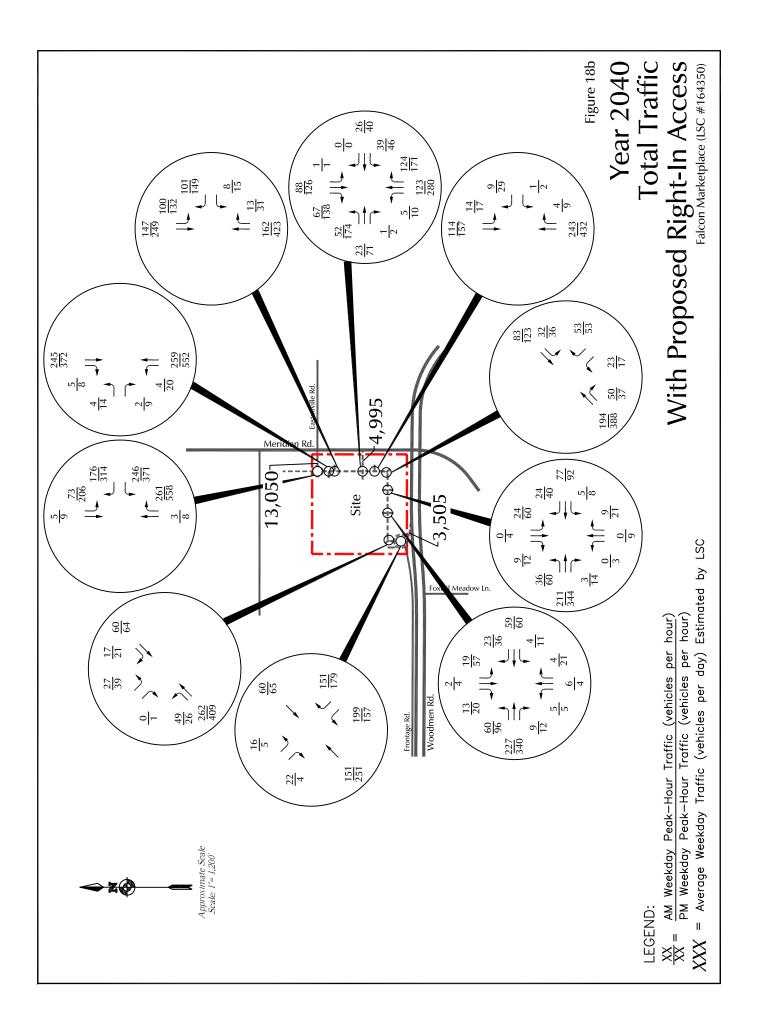


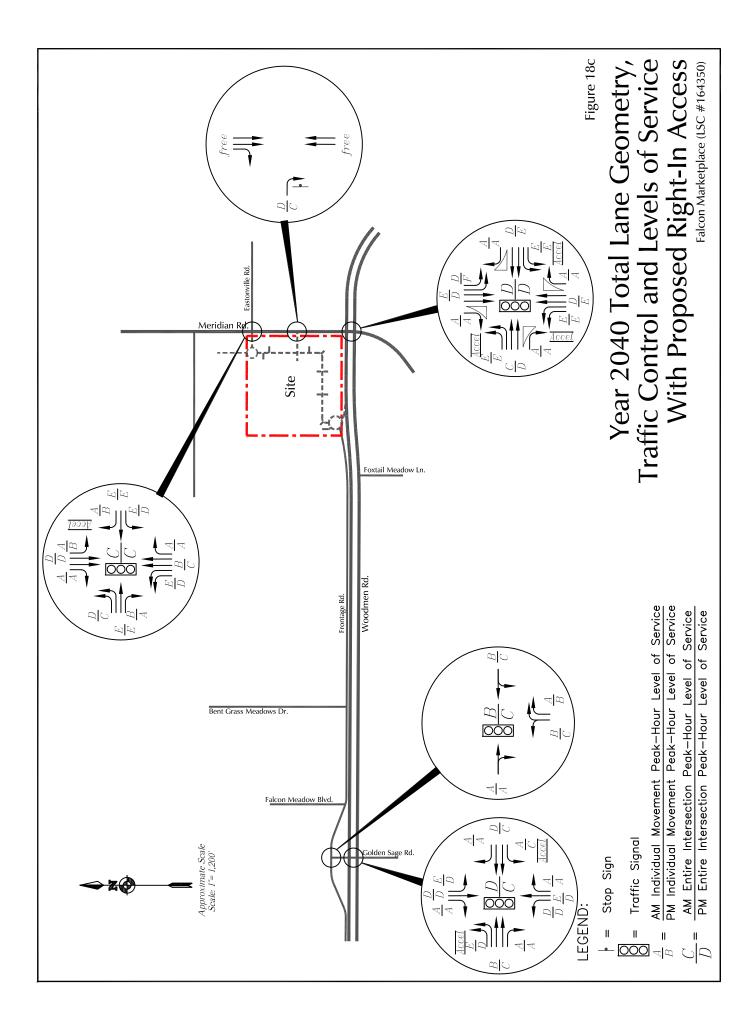


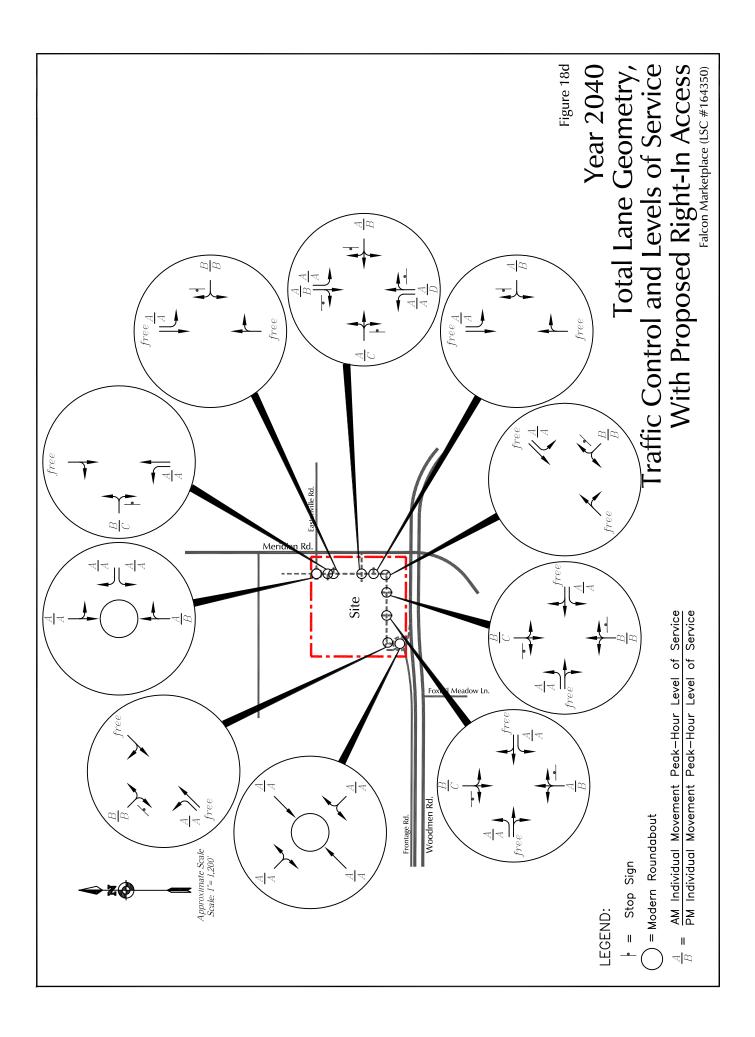












# **Traffic Operations Analysis**

#### **PROJECTED LEVELS OF SERVICE**

#### **Intersection Levels of Service**

The key area intersections were analyzed to determine the projected levels of service for the short-term and 2040 total traffic volumes with and without the proposed right-in-only access from Woodmen Road. Figures 6b, 7b, 8b, 9b, 15b, 16b, 17c, 17d, 18c, and 18d show the level of service analysis results. The signalized intersections were analyzed using Synchro. The right-in/right-out-only access point to Meridian Road was analyzed using SimTraffic simulations to better analyze the operational effects of adjacent signal-controlled intersections. The proposed access points to the internal public road were analyzed based on the unsignalized method of analysis procedures found in the *Highway Capacity Manual*, 6<sup>th</sup> Edition</sup> by the Transportation Research Board. The level of service (LOS) reports are attached. Tables 3 and 4 show the projected level of service, delay, and volume-to-capacity ratio for movements projected to operate below a LOS D based on the short-term and 2040 total traffic volumes, respectively.

	Short-Term		c Level of Se on Marketplac		ements			
Intersection		A	M			F	M	
Movement	Volume	LOS	Delay	V/C	Volume	LOS	Delay	V/C
Vithout Proposed Right-in Acce	ss to Woodmen Road							
Woodmen Road/Meridian Road								
Eastbound Left	293	D	47.4	0.59	623	D	54.3	0.85
Westbound Left	100	D	52.4	0.37	150	E	57.1	0.52
Northbound Left	150	D	48.9	0.42	175	D	54.0	0.51
Southbound Left	241	D	49.0	0.56	225	E	66.9	0.75
Overall		C	28.4			D	38.2	
	o Woodmen Road	<u>с</u> 	50.1	0.62	• • •	E	56.8	0.86
Vith Proposed Right-in Access t Woodmen Road/Meridian Road	o Woodmen Road							
Vith Proposed Right-in Access t Woodmen Road/Meridian Road Eastbound Left	o Woodmen Road	D	50.1	0.62	633	E	56.8	0.86
Vith Proposed Right-in Access t Woodmen Road/Meridian Road Eastbound Left Westbound Left	o Woodmen Road d 297 100	D	50.1 54.8	0.62 0.39	633 150	E	56.8 58.6	0.86

## Add a note that this is only for two through lanes.

1

Interesstic :-							DM	
Intersection Movement	Volume	LOS	AM Delay	V/C	Volume	LOS	PM Delay	V/C
enario: <u>Without</u> Proposed Rig	ht-in Access	to Woodm	nen Road					
Voodmen Road/Meridian Road								
Eastbound Left	468	<u> </u>	70.1	0.90	775	E	73.7	0.98
Vestbound Left	150	E	55.1	0.46	225	E	60.2	0.64
Vestbound Through	804	D	48.1	0.83	561	E	58.8	0.84
Northbound Left	250	E	61.4	0.69	350	D. F*V	54.9	0.69
Northbound Through Southbound Left	422 294	D	38.6 54.1	0.46	1008 483	F	103.4 80.5	<b>1.10*</b> 0.96
Southbound Through	941	D	54.1	0.03	704	 D	48.1	0.90
Overall	941	D	37.4			D	53.2	
Eastonville Road/Meridian Road Eastbound Through	73	E	54.9	0.45	197	E	56.1	0.68
Vestbound Left	250	E	61.8	0.82	200	 D	47.0	0.00
Vestbound Through	113	E	55.9	0.50	136	E	55.7	0.57
Northbound Left	387	E	70.3	0.87	644	F*	106.7	1.09*
Dverall		 D	38.2			D	40.0	
	•						•	
Voodmen Road/Golden Sage Rd Eastbound Left	400		70.0	0.04			50.0	0.70
	422	E	79.2	0.94	404	D	50.3	0.72
	10			0 4 7 1	20			
Northbound Through	19	E	56.4	0.17	39	D	52.8	
Northbound Through Southbound Left	251	E	63.9	0.73	191	D	53.2	0.69
Iorthbound Through Southbound Left Southbound Through Overall	251 21 	E D D	63.9 52.7 <b>38.6</b>					
Northbound Through Southbound Left Southbound Through Overall cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road	251 21 	E D D Voodmen	63.9 52.7 <b>38.6</b> Road	0.73 0.15	191 26 	D D <b>C</b>	53.2 53.1 <b>24.6</b>	0.69 0.19
Northbound Through Southbound Left Southbound Through <b>Dverall</b> cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left	251 21  in Access to V	E D D Voodmen E	63.9 52.7 <b>38.6</b> Road	0.73 0.15 	191 26  785	D D C	53.2 53.1 24.6 78.0	0.69 0.19 
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left	251 21  in Access to V 472 150	E D D Voodmen E E	63.9 52.7 <b>38.6</b> Road 74.5 55.7	0.73 0.15  0.92 0.47	191 26  785 225	D C C E E	53.2 53.1 24.6 78.0 60.4	0.69 0.19  1.00 0.64
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through	251 21  in Access to V 472 150 884	E D D Voodmen E E D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5	0.73 0.15  0.92 0.47 0.90	191           26              785           225           665	D C C E E E	53.2 53.1 24.6 78.0 60.4 77.9	0.69 0.19  1.00 0.64 0.98
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left Nestbound Through Northbound Left	251 21  in Access to V 472 150 884 328	E D D Voodmen E E D E	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3	0.73 0.15  0.92 0.47 0.90 0.89	191           26              785           225           665           465	D D C E E E E	53.2           53.1           24.6           78.0           60.4           77.9           60.4	0.69 0.19  1.00 0.64 0.98 0.82
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left Nestbound Through Northbound Left Northbound Through	251 21  in Access to V 472 150 884 328 344	E D D Voodmen E E D E D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5	0.73 0.15  0.92 0.47 0.90 0.89 0.37	191 26  785 225 665 465 893	D D C E E E E E E	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7	0.69 0.19  1.00 0.64 0.98 0.82 0.98
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Northbound Left Northbound Left Southbound Left	251 21  in Access to V 472 150 884 328 344 294	E D D E E D E D D D D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64	191 26  225 665 465 893 483	D D C E E E E F	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7	0.69 0.19  1.00 0.64 0.98 0.82 0.98 0.98
Iorthbound Through Southbound Left Southbound Through Overall Cenario: <u>With</u> Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Left Jorthbound Through Southbound Left Southbound Left Southbound Through	251 21  in Access to V 472 150 884 328 344	E D D Voodmen E E D E D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5	0.73 0.15  0.92 0.47 0.90 0.89 0.37	191 26  785 225 665 465 893	D D C E E E E E E	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7	0.69 0.19  1.00 0.64 0.98 0.82 0.98
Iorthbound Through Southbound Left Southbound Through Overall Cenario: <u>With</u> Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Left Jorthbound Left Jorthbound Left Southbound Left Southbound Left Southbound Through Southbound Through Southbound Through	251 21  in Access to V 472 150 884 328 344 294 941	E D D Voodmen E E D E D D E D E	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93	191           26              785           225           665           465           893           483           704	D D C E E E E E F D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0	0.69 0.19  1.00 0.64 0.98 0.82 0.98 0.98 0.96 0.83
Iorthbound Through Southbound Left Southbound Through Overall Internatio: With Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Jorthbound Left Jorthbound Left Southbound Through Southbound Through Southbound Through Dverall Eastonville Road/Meridian Road	251 21  in Access to V 472 150 884 328 344 294 941	E D D Voodmen E E D E D D E D E	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93	191           26              785           225           665           465           893           483           704	D D C E E E E E F D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0	0.69 0.19  1.00 0.64 0.98 0.82 0.98 0.98 0.96 0.83
Vorthbound Through Southbound Left Southbound Through Overall Venario: With Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Southbound Left Southbound Left Southbound Left Southbound Through Southbound Through Dverall Eastonville Road/Meridian Road Eastbound Through	251 21  in Access to V 472 150 884 328 344 294 941 	E D D Voodmen E E D E D D E D D E D D D D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b>	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93 	191           26              785           225           665           465           893           483           704	D D C E E E E F D D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8	0.69 0.19  1.00 0.64 0.98 0.98 0.98 0.98 0.98 0.83 
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Northbound Through Southbound Left Southbound Through Dverall Eastonville Road/Meridian Road Eastbound Through Vestbound Left	251 21  in Access to V 472 150 884 328 344 294 941  73	E D D Voodmen E D E D D E D D E D D E D	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b>	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93  0.45	191           26              785           225           665           465           893           483           704              197	D D C E E E F D D D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8           55.6	0.69 0.19  0.64 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98
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Verthbound Through Southbound Left Southbound Through Dverall Seenario: With Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Southbound Left Southbound Left Southbound Through Dverall Eastonville Road/Meridian Road Eastbound Through Vestbound Left Vestbound Left Vestbound Left Vestbound Left Dverall Dverall Dverall	251 21  in Access to V 472 150 884 328 344 294 941  73 250 113 222	E D D Voodmen E D E D E E D E E E E E	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b> 59.3 61.4 55.8 62.5	0.73 0.15  0.47 0.90 0.89 0.37 0.64 0.93  0.45 0.82 0.50 0.69	191           26              785           225           665           465           893           483           704              197           200           136           415	D D C E E E E E D D D D D D D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8           55.6           46.3           55.6           54.5	0.69 0.19  0.64 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98
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Verthbound Through Southbound Left Southbound Through Overall Seenario: With Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Southbound Left Southbound Through Southbound Through Overall Eastonville Road/Meridian Road Eastbound Through Vestbound Left Vestbound Left Vestbound Left Overall Southbound Left Doverall Southbound Left Doverall Southbound Left Doverall Southbound Left Doverall Southbound Left Doverall Southbound Left Doverall	251 21  in Access to V 472 150 884 328 344 294 941  73 250 113 222  73 250 113 222 	E D Voodmen E E D E D D E D E E E C C	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b> 59.3 61.4 55.8 62.5 <b>33.9</b> 77.7	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93  0.69 0.50 0.69  0.93	191         26            225         665         465         893         483         704            197         200         136         415            394	D D C E E E F D D D C C	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8           55.6           54.5           32.4           50.1           52.9	0.69 0.19  1.00 0.64 0.98 0.98 0.98 0.98 0.96 0.83  0.71 0.56 0.76  0.71
Verthbound Through Southbound Left Southbound Through Overall Cenario: With Proposed Right- Voodmen Road/Meridian Road Eastbound Left Vestbound Left Vestbound Through Southbound Left Southbound Left Southbound Through Overall Cestbound Through Overall Cestbound Through Vestbound Left Vestbound Left Vestbound Left Vestbound Left Overall Coveral Coveral Coveral Coveral Coveral Cov	251 21 21  in Access to V 472 150 884 328 344 294 941  73 250 113 222  418	E D D Voodmen E E D D E D D E E E E C C	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b> 55.9 <b>40.3</b> 55.3 61.4 55.8 61.4 55.8 62.5 <b>33.9</b> 77.7 56.4	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93  0.45 0.82 0.50 0.69  0.69 	191         26            25         665         465         893         483         704            197         200         136         415            394         39	D D C E E E E D D E D C C D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8           55.6           54.5           32.4           50.1	0.69 0.19  1.00 0.64 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98
Northbound Through Southbound Left Southbound Through Dverall Cenario: <u>With</u> Proposed Right- Noodmen Road/Meridian Road Eastbound Left Vestbound Left Nestbound Through Northbound Left	251 21  in Access to V 472 150 884 328 344 294 941  73 250 113 250 113 222  418 19 251	E D Voodmen E E D D E D D E E E E C C	63.9 52.7 <b>38.6</b> <b>Road</b> 74.5 55.7 53.5 78.3 37.5 54.9 55.9 <b>40.3</b> 55.9 <b>40.3</b> 55.9 <b>40.3</b> 55.9 <b>40.3</b> 77.7 56.4 63.9	0.73 0.15  0.92 0.47 0.90 0.89 0.37 0.64 0.93  0.45 0.82 0.50 0.69  0.69  0.93 0.17 0.73	191         26            225         665         465         893         483         704            197         200         136         415            394         39         191	D D C E E E E D D E D C C D D D D D	53.2           53.1           24.6           78.0           60.4           77.9           60.4           69.7           81.7           53.0           50.8           55.6           46.3           55.6           54.5           32.4           50.1           52.9           53.5	0.69 0.19  1.00 0.64 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98

### Woodmen/Meridian

The intersection of Woodmen/Meridian is projected to operate at an overall level of service (LOS) D or better based on the short-term and 2040 total traffic volumes with and without the proposed right-in-only access from Woodmen Road.

The eastbound left-turn movement at the Woodmen Road/Meridian Road intersection currently operates at LOS D. Given the high existing and projected background traffic demand for this turning movement, it is projected to operate at LOS E during the afternoon peak hour by 2040. Important note: This condition has little to do with this site, rather it is primarily due to the background traffic demand. The northbound left-turn movement is projected to operate at LOS E during the morning and LOS D during the afternoon peak hour assuming no access to Woodmen. The northbound left-turn movement is projected to operate at LOS E during and afternoon peak hours assuming the right-in access. The northbound through movement is projected to operate at LOS F during the 2040 afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS F during the 2040 afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS F during the 2040 afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS F during the afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS F during the 2040 afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS F during the afternoon peak hour assuming no site access to Woodmen. The northbound through movement is projected to operate at LOS E during the afternoon peak hour based on the projected 2040 total traffic volumes with the proposed right-in-only access.

## Meridian/Eastonville

The intersection of Meridian/Eastonville was assumed to be signalized once the site is built out. As a signalized intersection, it is projected to operate at an overall LOS C or better based on the short-term volumes with or without the proposed right-in-only access from Woodmen Road. By 2040, the northbound left-turn movement is projected to operate at LOS E during the morning peak hour and LOS F during the afternoon peak hour without the proposed right-in-only access from Woodmen Road. This movement is projected to operate at LOS E during the morning peak hour and LOS D during the afternoon peak hour based on the projected 2040 total traffic volumes with the proposed right-in-only access.

## Meridian/Right-In/Right-Out-Only Site Access

The exiting (eastbound) right-turn movement at the proposed right-in/right-out access to Meridian Road is projected to operate at LOS D or better during the morning peak hour based on the projected short-term and 2040 total traffic volumes with and without access to Woodmen Road. The afternoon peak-hour projected LOS is C under either scenario.

## Woodmen Frontage Road Intersections

The southbound (exiting) approaches to the access point intersections for Mountain View Electric and the Courtyards at Woodmen Hills to the Woodmen Frontage Road are projected to operate at LOS A or B based on projected total 2040 traffic.

## Roundabout Level of Service

The **southwest roundabout** has been analyzed for level of service using three different methods—HCM, Rodel, and SimTraffic. All methods indicate level of service A for all approaches during the peak hours

based on 2040 volumes. The southwest roundabout has been analyzed in more detail in this Woodmen Road Driveway Permit report to address the comments on the August 7, 2017 version of the report.

The **Eastonville roundabout** in the northeast part of the site has been analyzed using the HCM method of analysis and the results are shown in the figures. A second analysis using Rodel will be included with the resubmittal of the Preliminary Plan or with the Plat.

#### **Internal Public Road Intersections**

All of the access points to the internal public road (which is planned to extend from the proposed roundabout at the existing terminus of the Woodmen Frontage Road to the proposed roundabout at Eastonville Road) are planned to be two-way stop-sign-controlled intersections. The exception is the intersection that will align with the proposed right-in/right-out intersection to Meridian Road. This intersection is shown as all-way, stop-sign-controlled. All movements at the two-way, stop-sign-controlled intersections are projected to operate at LOS C or better based on the 2040 total traffic volumes with or without the proposed right-in access to Woodmen Road. The northbound and southbound through movements at the proposed all-way, stop-sign-controlled intersection are projected to operate at LOS D or better based on the 2040 total traffic volumes with the proposed right-in-only access. The northbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to operate at LOS E and the southbound through movement is projected to

All movements at the proposed roundabouts at the terminus of the Woodmen Frontage Road and at the intersection of the internal public road and Eastonville Road are projected to operate at LOS B or better during the peak hours based on the projected 2040 total traffic volumes with and without the proposed right-in-only access to Woodmen Road.

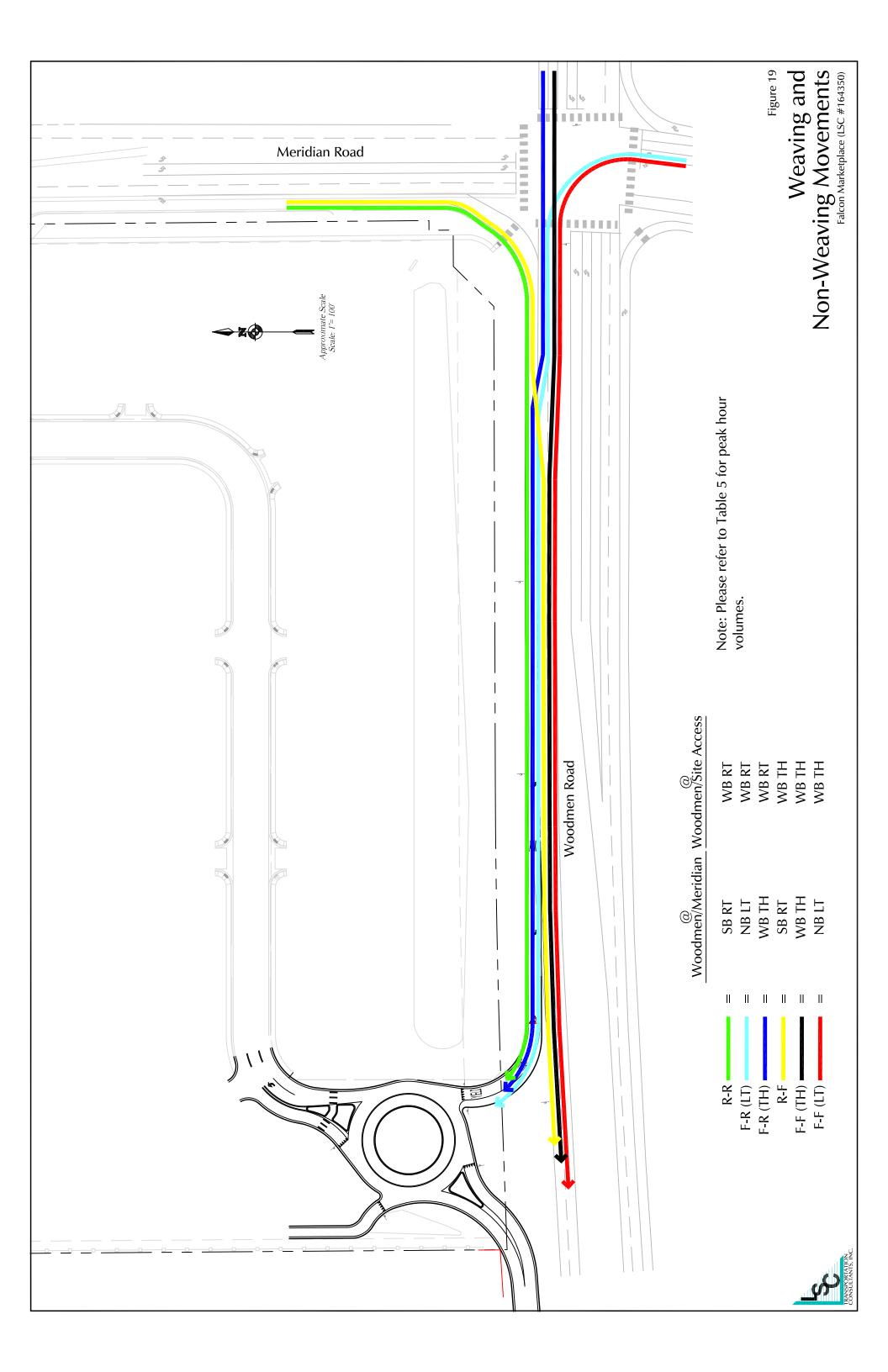
#### Weaving Section Level of Service

A weaving level of service analysis has been completed for the section of westbound Woodmen Road between the Meridian Road intersection and the proposed right-in-only site access. This section has been analyzed as a Type A weaving segment in order to determine the projected weaving area levels of service based on the freeway weaving operational method of analysis procedures from the *Highway Capacity Manual, 2010 Edition*. Table 5 shows a summary of the weaving movement volumes by zone and Figure 19 shows the path for each weaving movement. The weaving LOS reports are attached.

This weaving segment is projected to operate at LOS C during the morning peak hour and LOS B during the afternoon peak hour based on the projected future total traffic volumes. Note: This weaving segment would not operate as bona fide freeway weaving areas per the *Highway Capacity Manual, 2010 Edition* due to several operational and geometric differences between an urbanized corridor with intersections and traffic signals and a true freeway weaving section.

The following has been added since the previous version of the report to address staff comments regarding "*any potential real-world issues not anticipated by the modeling*."

		v	Tab Veaving Tra		05				
		v		ffic Volum	20				
					63				
			Falcon Ma	irketplace					
			AM Pea						
			-		ffic Volume	(vehicles/ho	our)		
		R-R		F-R		R-F		F-F	
		SB RT to	NB LT to	EB U- turn to	WB TH to	SB RT to	WB TH to	EB U- turn to	NB LT to
Zone		Right-In	Right-In	Right-in	Right-In	WB TH	WB TH	WB TH	WB TH
Mountain View Electric		4	10	0	4	0	0	0	0
Courtyards		5	13	1	4	0	0	0	0
Latigo Industrial		39	20	0	20	6	0	0	0
Latigo Business Center		0	10	0	10	0	0	0	0
Future MVE		12	12	0	35	0	0	0	0
Site		25	46	10	70	13	0	0	0
Other Background		0	0	0	0	937	741	4	217
т	otal	85	111	11	143	956	741	4	217
				265				958	
Site Access V	VB RT		350	)					
_									
	SB RT	1041							
Meridian/ Woodmen	BUT	15							
	VB TH	884							
N	IB LT	328							
			PM Pea						
			-		ffic Volume	(vehicles/ho	our)		
		R-R		F-R EB U-		R-F		F-F	Т
		SB RT to	NB LT to	turn to	WB TH to	SB RT to	WB TH to	EB U- turn to	NB LT to
Zone		Right-In	Right-In	Right-in	Right-In	WB TH	WB TH	WB TH	WB TH
Mountain View Electric		2	4	0	1	0	0	0	0
Courtyards		15	45	5	14	0	0	0	0
Latigo Industrial		25	13	0	13	4	0	0	0
Latigo Business Center		0	3	0	2	0	0	0	0
Future MVE		9	3	0	3	0	0	0	0
Site		13	62	15	89	33	0	0	0
Other Background		0	0	0	0	501	543	10	335
т	otal	64	130	20	122	538	543	10	335
				272				878	
Site Access V	VB RT		336	6					
		0.65							
	B RT	602							
Meridian/ Woodmen -	BUT	30							
	VB TH	665							
N Source: LSC Transportation Cor	IB LT	465							



The southbound right turn at the Woodmen/Meridian intersection is currently a channelized "free" right turn. There is no signal control or Stop/Yield control for this movement as vehicles are channelized into the westbound acceleration lane on Woodmen Road. The configuration of this free-right into the acceleration/deceleration lane (with the addition of the right-in access) has been simulated using SimTraffic in addition to the weaving section on Woodmen Road, which has been modeled using the HCM weaving procedures. This is because there is no traffic control (Yield, Stop, or signal control) for this movement and the plan does propose to change this condition. There will be future pedestrian ramps from the raised island to the inside curb radius to allow pedestrians to cross. There will likely be occasional pedestrians at this location, but the pedestrian volume will likely be low. Pedestrians have the potential to momentarily stop the flow of southbound right-turning traffic. The modeling of the "free right" in SimTraffic does not allow for the modeling of crossing pedestrians. However, observations at similar free-right conditions at other intersections in the Pikes Peak region indicate that pedestrians typically must wait for a gap in the right-turning traffic stream to cross because motorists using free right turns will most often not yield to pedestrians.

The simulation does not indicate that drivers will likely stop at the corner at the entry to the acceleration lane when there is a platoon of westbound traffic entering the weaving area on Woodmen Road (once the signal releases westbound through traffic). The phenomenon of right-turning motorists stopping to wait even when there is an acceleration lane is often the case at intersections on urban arterial streets at signalized intersections. That said, field observations at this particular location indicate a low relative percentage of motorists stopping to wait in this situation. This is likely because of several factors unique to this location—the relatively large radius, a raised/curbed right-turn island, and relatively long acceleration lane. Also, unlike many locations in the Pikes Peak region, there is not another intersection a short distance downstream west on Woodmen Road. The next intersection with a westbound left turn is Golden Sage, which is 1.3 miles to the west.

Should southbound right-turning motorists pause at the entry to the acceleration/deceleration lane for either an occasional pedestrian or to allow passage of a platoon of vehicles on westbound Woodmen Road, there is the potential for a queue to form to the north in the southbound right-turn deceleration lane. Such a queue is not reflected in the SimTraffic simulation as this was appropriately modeled as a free right. Should the occasional queue form, it would most likely occur when southbound through traffic at the Meridian/Woodmen intersection is stopped at the red signal, arriving vehicles from the north will be slowing, and queues will be forming in the southbound through lanes of Meridian Road as well. Once the westbound Woodmen platoon of traffic clears the merge area, any southbound right-turn queue will begin to clear and will clear completely with the southbound through green signal at Woodmen/Meridian (if not before).

The weaving analysis was completed using the HCM weaving analysis procedures. These do not assume upstream traffic signals as is commonly the case of weaving areas on urban arterials. In this particular situation,

- No weaving will occur during the north/south green signal phase for Meridian Road as no vehicles will enter the weaving section.
- During the eastbound left-turn phase (which is a relatively long phase), there will be very minimal weaving as only the occasional U-turning vehicle from the eastbound Woodmen left-turn lanes will

enter the weaving area. These U-turning vehicles will also be entering the weaving area at a slow speed after having executed a U-turn.

- During the northbound left-turn phase, vehicles will enter the weaving area from northbound Meridian Road. These vehicles will enter the weaving area at a speed comparable to the southbound right-turning traffic rather than 45 to 50 mph. This will allow for acceptance of shorter gaps for weaving and merging.
- Shortly after the start of the westbound through green phase, a platoon of westbound Woodmen traffic will enter the weaving area. The first part of this platoon will not be traveling 45 or 50 mph as these vehicles will be accelerating from a stop condition. Once westbound traffic reaches saturation flow, the remainder of the vehicles released by the signal from the westbound approach will enter the merge area at higher-than-startup speeds.

## VEHICLE QUEUING ANALYSIS

A queuing analysis was performed using Synchro/SimTraffic for the key approach turning movements at the study area intersections to determine the projected queue lengths based on the projected total traffic volumes. The short-term and 2040 total peak-hour traffic volumes with and without the proposed right-in-only access were entered into the Synchro model. The simulation was run five times. The queuing reports are attached. These queuing results have been used to develop auxiliary turn lane recommendations. The results of the analysis are shown in Table 6.

#### **Roundabout Queuing Analysis**

The **southwest roundabout** has been analyzed for queuing using three different methods—HCM, Rodel, and SimTraffic. The southwest roundabout has been analyzed in more detail in this report for the Woodmen Road Driveway Permit to address the comments on the May 15, 2017 version of the report. All methods indicate short queues for all approaches during the peak hours based on 2040 volumes. The most important queue length is the one on Approach 2—the right-in from Woodmen Road. The HCM analysis reports indicate 95<sup>th</sup> percentile queue length of one vehicle during the morning peak hour and two vehicles during the afternoon peak hour. The Rodel analysis reports indicate queue lengths of 1.31 vehicles during the morning peak hour and 1.34 vehicles during the afternoon peak hour. The SimTraffic analysis indicates a maximum queue of 102 feet during the morning peak hour and 119 feet during the afternoon peak hour. The simulation shows this to be more of a "rolling" queue that quickly shortens from the maximum reported length.

The **Eastonville roundabout** in the northeast part of the site has been analyzed for queuing using the HCM method of analysis and the results are shown in the HCM analysis printouts. Analysis using Rodel will be included with the resubmittal of the Preliminary Plan or with the Plat.

					-	l Queue Le n Marketpla	•					
			Pr	ojected Qu Short	eue Length	IS			F	Projected Qu	eue Lenath	16
		Backgrou	nd Traffic	onon	Total	Traffic			1	2040 Tot		13
Intersection			Right-In	Without	Right-In	With R	light-In		Without	Right-In	With R	light-In
Movement	Lane Length	AM PM		AM PM		AM PM		Lane Length	AM PM		AM PI	
Eastonville Road/	Meridian Road											
Northbound Left	425'	58	59	357	415	128	174	425'	259	430 <sup>(1)</sup>	148	236
Woodmen Road/	/leridian Road											
Eastbound Left	500' dual section + 215' (single)	168	361	193	368	173	443	Modeled with sufficient length to determine needed length	382	865 <sup>(2)</sup>	358	840
Westbound Left	435'	108	160	193	150	173	443 146	435'	129	177	217	318
Northbound Left	Modeled with sufficient length to determine needed length	142	140	139	163	162	220	Modeled with sufficient length to determine needed length	217	392	281	346
	315' dual section +145' Single	161	176	167	176	196	275	(400' dual section + 360' single)	243	929	307	391
Southbound Left	475'	147	137	151	152	184	202	475'	171	407	170	440
Woodmen Road/	Golden Sage Road											
Eastbound Left	465'	90	107	122	149	108	145	465'	421	297	437	238

Source: LSC Transportation Consultants, Inc.

## ROUNDABOUT DESIGN VEHICLE AND FASTEST PATH ANALYSIS

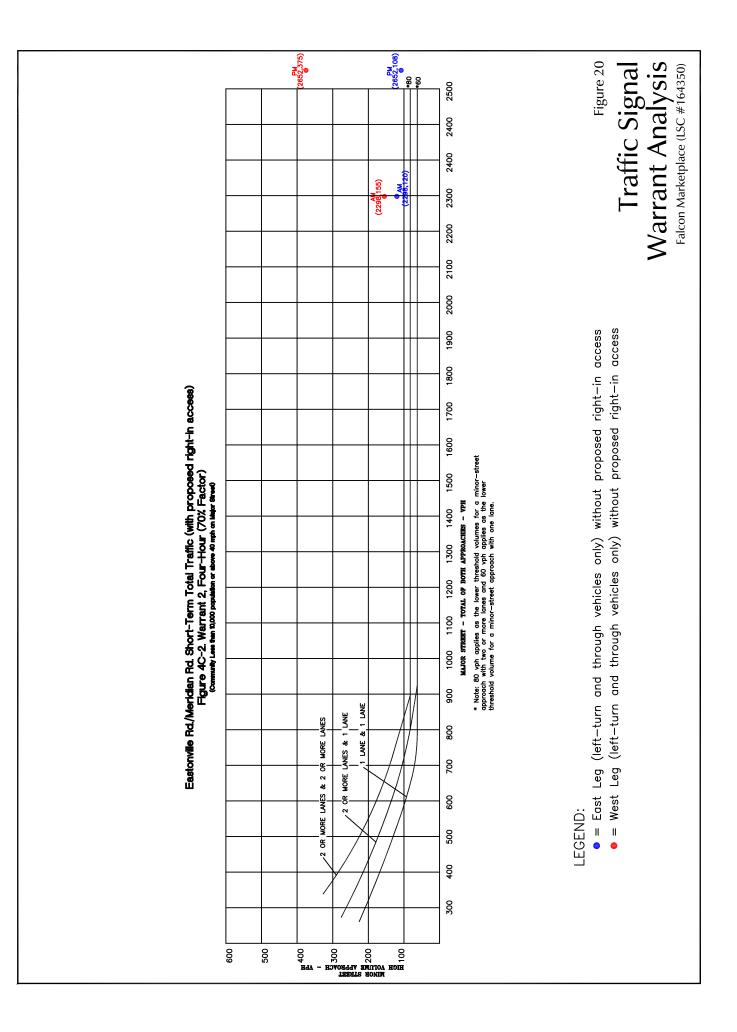
This report includes a detailed analysis of the truck turning movements, fastest-path, and geometry at the proposed southwest roundabout. This roundabout has been redesigned since the previous submittal to address staff comments. The roundabout geometric and operational analysis exhibits are attached. Complete roundabout analysis of the northeast/Eastonville roundabout will be included with the resubmittal of the Preliminary Plan or with the Plat.

The fastest-path analysis of the southwest roundabout indicates acceptable fastest-path speed on each of the four approaches at the proposed southwest roundabout.

## TRAFFIC SIGNAL WARRANT ANALYSIS

#### It is expected that a traffic signal would be warranted at the intersection of Eastonville Road and Meridian Road with the addition of the west leg of the intersection and traffic from this proposed development. Furthermore, the traffic signal was a condition to the 2008 BOCC rezoning of the property to Commercial Regional.

The intersection has been analyzed to determine if a Four-Hour Vehicular Volume Traffic Signal Warrant threshold would be reached or exceeded based on the projected short-term morning and afternoon peak-hour total traffic volumes. The results of the analysis are shown in Figure 20. The traffic volumes shown are based on the short-term total traffic volumes with the proposed right-in-only access to Woodmen Road shown in Figure 16a. As shown in Figure 20, the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant are projected to be exceeded based on the morning and afternoon peak hours. This analysis using the peak hours is intended to provide an indication that a warrant may be met or is close to being met. For a Four-Hour Traffic Signal Warrant to be satisfied, the volume threshold would need to be met for two additional hours of the day. For example, the four-hour warrant would be satisfied with the volume thresholds met for one hour in the morning, two hours (instead of the one-hour peak) during the afternoon peak period, and an hour during the mid-afternoon. Based on this analysis and our experience, it is likely that the volume thresholds would be met or exceeded for at least two additional hours of the day.



# **Comparison of Analysis Scenarios**

## MERIDIAN/EASTONVILLE INTERSECTION OPERATIONS

The comparison between the two analysis scenarios (with and without the proposed site access from Woodmen) with respect to operations at the Meridian/Eastonville intersection shows significantly better operations with the scenario that includes the proposed Woodmen access in combination with the proposed roundabout. This is because without the Woodmen access and the ability to enter the site from a second adjacent roadway (Woodmen Road), the northbound left-turn volume at Eastonville/Meridian would be significantly higher. The northbound left turn at this intersection is projected to operate at LOS F during the afternoon peak hour without the right-in from Woodmen and LOS D during the afternoon peak hour with the right-in from Woodmen with the roundabout. Another point is that the roundabout is projected to reduce the northbound left-turn delay by about five seconds per vehicle at the Meridian/Eastonville intersection during the afternoon peak hour over the previous configuration without the roundabout. This difference is significant. Without the right-in, additional northbound leftturn green signal phase time would be needed to adequately serve this left-turn demand and prevent queues from overflowing the northbound left-turn lane. This would negatively impact the intersection overall by reducing the allocation of southbound through green signal time. Creating this situation is unnecessary because with the addition of the proposed Woodmen access, motorists arriving from the east on Woodmen Road and from the south from Meridian Road (including the significant component of traffic arriving from eastbound US Highway 24 in the afternoon peak hour) would have the additional option of entering using the proposed Woodmen access instead of the single option of the access at Meridian/Eastonville via the northbound left turn at this intersection. The addition of the right-in-only with the roundabout would remove **background** traffic volumes of about 40 morning peak-hour trips and 70 afternoon peak-hour trips from the Meridian/Eastonville intersection (specifically the projected heavy northbound left-turn movement). In addition to these background traffic reductions, the site traffic component of this northbound left turn would be reduced by 112 morning peak-hour trips and 160 afternoon peak-hour trips.

The northbound left-turn queue would be significantly longer under the no-Woodmen-access scenario. The projected maximum 2040 afternoon peak-hour northbound left-turn queue is projected to be 236 feet under the with-right-in from Woodmen scenario. Without the right-in from Woodmen the projected maximum queue would fill the dual left-turn lanes (457-foot queue within the lanes) and would overspill into the adjacent northbound through lane during the peak analysis interval).

The lower northbound left-turn movement volume at Meridian/Eastonville under the proposed accessto-Woodmen scenario would likely allow the option to operate the northbound left-turn movement as a protected-permissive phase single left-turn movement for a significantly longer period, if not in perpetuity. This type of left-turn movement can often operate more efficiently and with less delay than protected-only dual left-turn lanes, especially when considering off-peak hours or most hours in a day and on weekends. The specific phasing and operation of the turn movement would be up to El Paso County, but a northbound left turn with a significantly reduced volume will likely allow for greater flexibility for better traffic operations.

## MERIDIAN/WOODMEN INTERSECTION OPERATIONS

The eastbound left-turn movement at this intersection is projected to operate at LOS E during the 2040 peak hours with or without this development (due to background traffic). However, the addition of the access from westbound Woodmen Road would improve overall operations at this intersection. This is because the addition of the proposed Woodmen access would allow the option for use of the existing dual left-turn lanes and a shift in approaching traffic from the adjacent high (critical) volume northbound through lanes into the adjacent dual left-turn lanes. This will be especially helpful during the afternoon peak hour. Although the overall intersection delays shown in the table are comparable, the analysis shows failure of the northbound through movement. Although the overall intersection delays are comparable at Meridian/Woodmen, consideration also needs to be given to the significantly higher travel time for site and background motorists that would use the right-in access, if provided (see paragraph below entitled "Woodmen Frontage Road Access." The right-in will significantly reduce travel times for motorists who would use it.

The afternoon peak-hour northbound through movement level of service would improve to LOS E (with a non-failing volume-to-capacity ratio) with the addition of the right-in-only from Woodmen and the roundabout. This is significant as a heavy afternoon northbound through volume is projected with the Meridian connection to US 24.

## WOODMEN ROAD OPERATIONS

The proposed Woodmen access will have little effect on the operation of Woodmen Road as the turning movements will be right-turn in-only from westbound Woodmen Road with a continuous acceleration/ deceleration lane between Meridian and the point of right-turn entry into the site. This site is within the commercial "node" of Falcon and an access at the proposed location would not be unexpected.

## WOODMEN FRONTAGE ROAD ACCESS

The right-in-only access with the proposed roundabout intersection with the Woodmen Frontage Road would also significantly improve access to the properties to the west along the Woodmen Frontage Road. This represents a **significant change** from the previous right-in-only access configuration. The prior right-in-only access would have essentially served only this site due to its configuration as there was no public access from the previously proposed right-in-only to the Woodmen Frontage Road. The previously proposed right-in-only was configured to direct traffic from westbound Woodmen north into the site only with no option for access for westbound travel to residential and non-residential properties along the Woodmen Frontage Road.

The addition of the roundabout intersection in the southwest corner of the site within to-be-dedicated public right-of-way is a significant change as it now allows for public access from westbound Woodmen Road to the Woodmen Frontage Road and properties along the North Frontage Road. The proposed roundabout allows for this access to the west for passenger vehicles, trucks, buses, and fire and emergency response vehicles. This accommodation will significantly improve the access to the Courtyards, MVEA (and other properties), which currently have poor access. With this proposed right-in-only access, residents, employees and other motorists traveling to these properties from westbound Woodmen or northbound Meridian (many traveling from eastbound US Highway 24) will no longer need to travel west for more than a mile along Woodmen Road to the Golden Sage/Woodmen

intersection and backtrack along the Woodmen Frontage Road (or travel north to Eastonville/Meridian to turn left at this intersection and travel through the Falcon Marketplace site) to access their destinations. This would significantly reduce travel times and emergency response times.

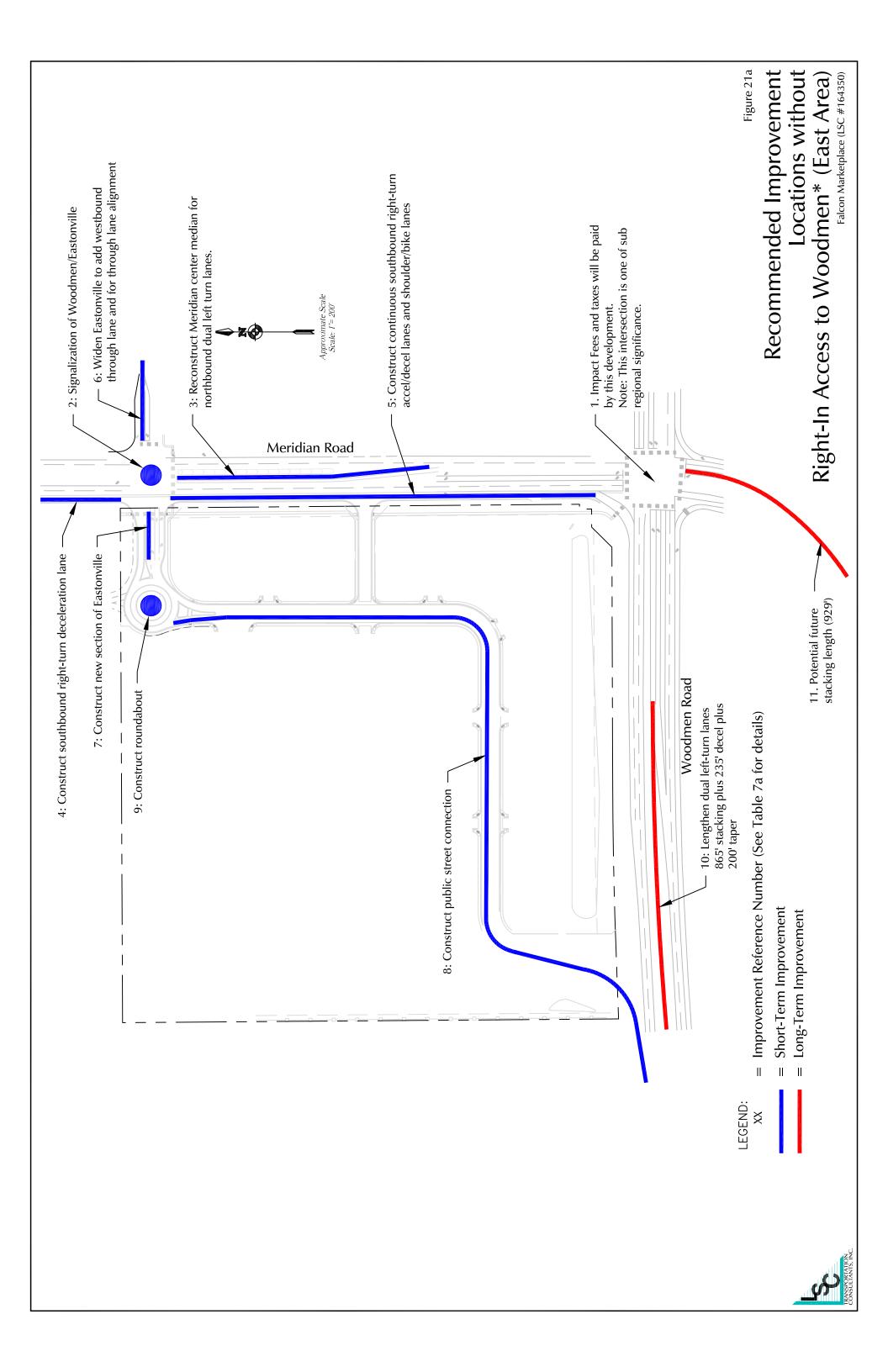
The proposed right-in-only with the roundabout would also benefit operations at the Woodmen/Golden Sage and Woodmen Frontage Road/Golden Sage intersection by removing existing and future traffic turning movements from these closely spaced intersections. The addition of the right-in-only combined with the roundabout would remove **background** traffic volumes of about 70 morning peak-hour trips and 30 afternoon peak-hour trips from **both** the Woodmen/Golden Sage and Woodmen Frontage Road/Golden Sage intersections

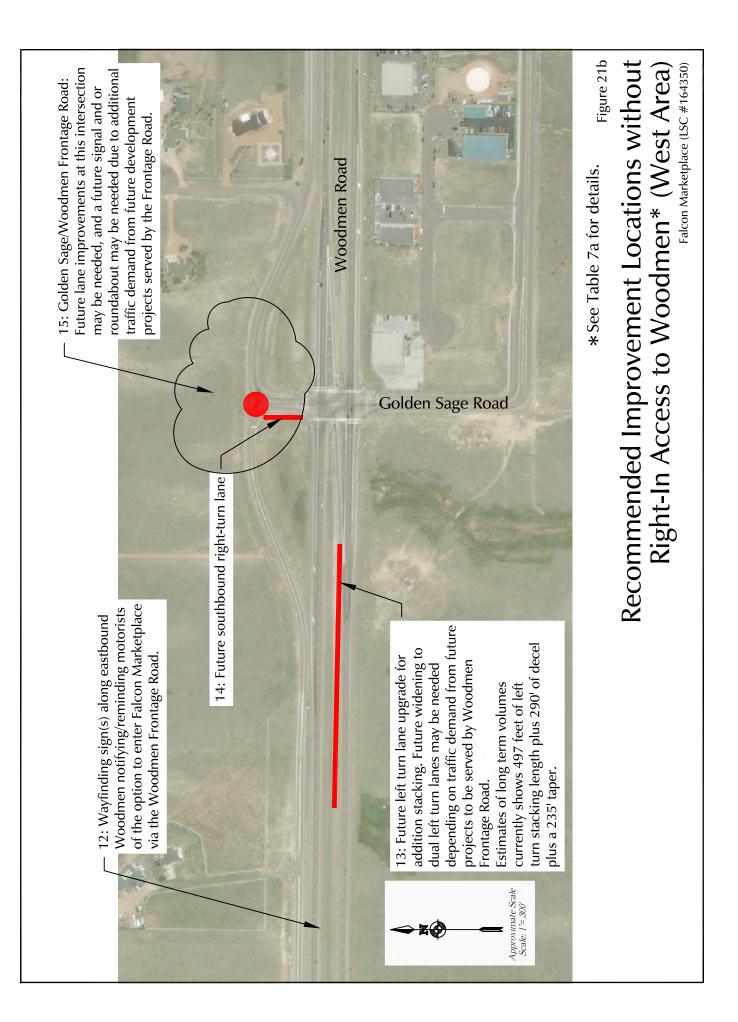
The following highlights the anticipated study area roadway and intersection improvement due to a combination of existing deficiencies, future background traffic and projected site traffic. A list of all improvements in the vicinity and assessment of responsibility is presented in Tables 7a and 7b. Table 7a shows improvements assuming no access to Woodmen Road (improvement locations are shown in Figures 21a and 21b). Table 7b shows the improvements assuming the proposed right-in access to Woodmen Road (improvement locations are shown in Figures 22a and 22b).

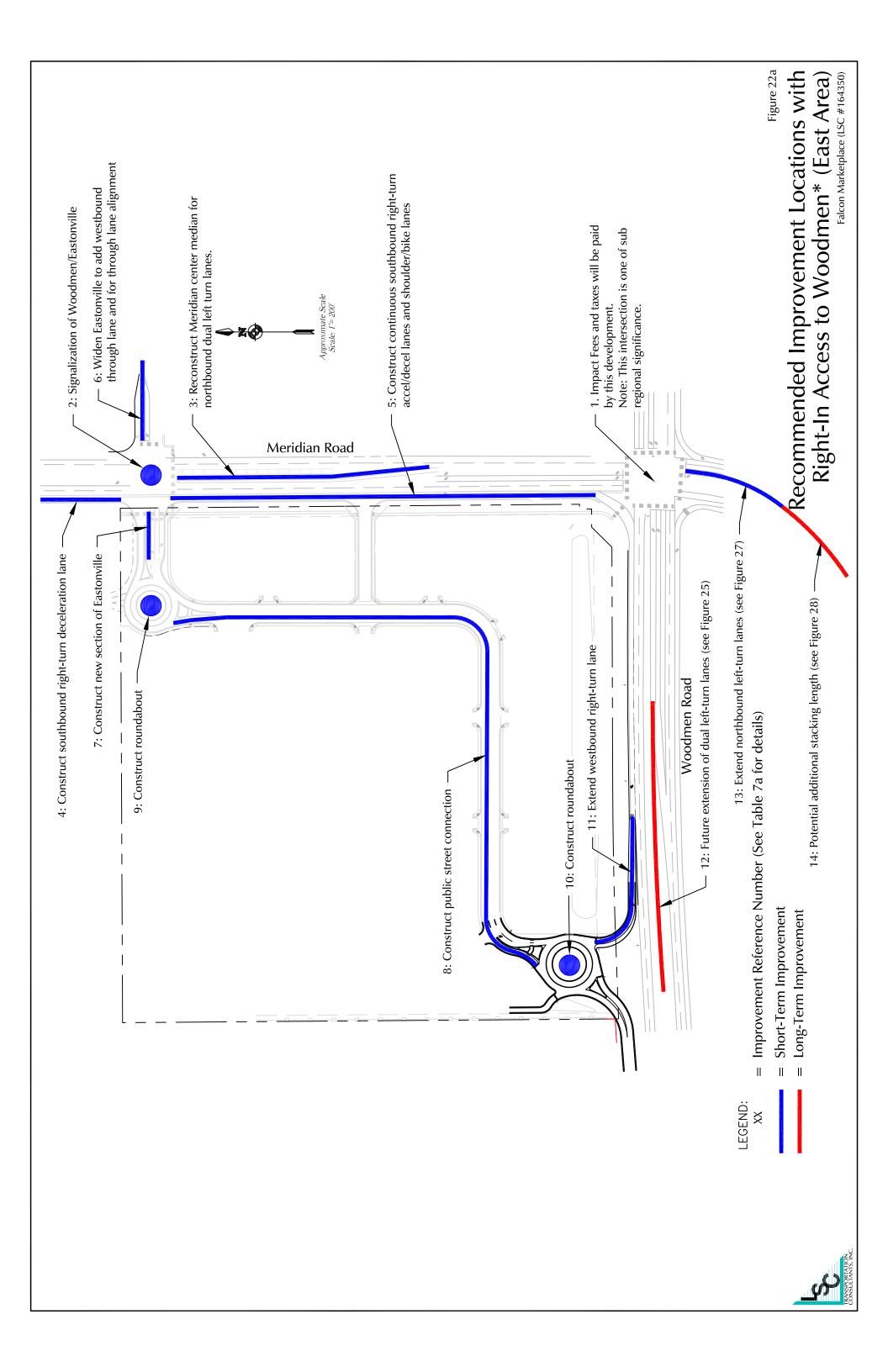
	Roadway Im	Table 7a Falcon Marketplace provements Without Proposed Right-In Access to Woodmen R	Road
Item #	Improvement	Timing	Responsibility
1	Woodmen Road Metropolitan District fees and taxes to be paid in lieu of Countywide Road Impact Fee Program fees and taxes. Woodmen Road has already been completed, but this project iss joining the district.	Countywide Road Impact Fee Program Fees and Taxes District fees payable at platting	Falcon Marketplace
2	Meridia	an/Eastonville and Merician Right-In/Right-Out Intersections Design and installation with the development of Falcon Marketplace once allowed by El Paso County.	Falcon Marketplace
3	Reconstruction of the Meridian center median south of Eastonville Road to achieve major street left-turn sight distance for the option of northbound/southbound interim protected/permissive left-turn signal phasing with interim single left-turn lanes northbound and southbound and to accommodate northbound dual left-turn lanes through restriping once needed. This lane should be 425 feet long plus a 200-foot taper. The taper would be back-to-back with the southbound dual left-turn taper.	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
4	Southbound right-turn deceleration lane on Meridian Road approaching Eastonville Road. This lane should be 235 feet long plus a 200-foot taper.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
5	Design and construction of continuous southbound right-turn lanes and shoulder/bike lane on Meridian Road from Eastonville Road south to the proposed right-in/right-out and from the right-in/right-out south to Woodmen Road.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
6	Widening of Eastonville Road east of Meridian Road to add a westbound through lane. Width would also be added as feasible between the westbound left-turn lane and the westbound through lane due to the proposed dual left-turn lanes on the west side of the intersection. This added width would allow for through lane alignment (with an acceptable offset across the intersection).	Design and installation with the development of Falcon Marketplace. Extent of this off-site improvement may be limited by available right-of-way and/or other existing constraints.	Falcon Marketplace
7	Design and construction of the proposed extension of Eastonville Road between Meridian and the proposed roundabout.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
		On-Site Improvements	l
8	Design and construction of the public street connection through the site (Falcon Market Place).	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
9	west of the Meridian/Eastonville intersection. This would include a "stub" to the north for the anticipated future street connection north to Bent Grass Meadows Drive.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
		Woodmen/Meridian Intersection	I
10	Woodmen eastbound dual left-turn lanes. This lane should be extended to provide a total of 865 feet of storage plus 235' deceleration length and a 200' taper.	Future extension of existing dual left-turn lanes once traffic queues regularly extend beyond the 480-foot stacking distance.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
11	Meridian northbound dual left-turn lanes LONG TERM: Potentially additional growth in the Falcon Area and east along Falcon Highway will add left turning traffic demand. Long term estimated queue would be up to about 475 feet in the inside left turn lane. Therefore the potential long term need may be 475' of stacking distance plus the 235 feet of deceleration distance and the 200-foot taper.	Future (if necessary Add additional lane length beyond #13 to provide additional stacking if/when needed (as shown in Figure 26).	By other future developments impacting this turn lane. Potentia for fee program credit for improvements completed as this is a regional intersection.
		Golden Sage Intersections	
12	Directional wayfinding sign(s) on eastbound Woodmen Road upstream of Golden Sage - notifying/reminding eastbound motorists of the option to enter Falcon Marketplace via the Woodmen Frontage Road.	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
13	Lengthening of the current eastbound single left-turn deceleration lane on Woodmen approaching Golden Sage Road to provide a 240-foot transition taper (20:1 taper ratio), 290 feet of deceleration distance plus sufficient vehicle stacking distance. CURRENT: 175-foot taper, plus a 465-foot left-turn lane which translates to a 175-foot taper, 290-foot deceleration distance, and 175 feet of stacking distance. SHORT TERM: Adequate stacking is available in the current turn lane - calculated queue length 141'. LONG TERM: lengthen single left-turn lane and/or future implementation of dual left-turn lanes (if capacity needs dictate) to maintain 290 feet of deceleration length, a 240-foot lane taper (20:1 taper ratio) plus provide sufficient vehicle stacking length - model indicates 497 feet of dual left stacking distance based on morning peak hour projected volumes. If a dual left is implemented in the future, consideration will need to be given to the configuration on Golden Sage and at the Golden Sage/Woodmen Frontage Road intersection to receive the dual left-turn movement.	Short Term: The existing lane is adequate based on the short term analysis. Long Term: Future with additional development served by the north frontage road - extension of existing single left-turn lane and potentially widening in the median to provide dual left turn lanes to provide additional vehicle stacking distance as described in the column to the left.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
14	Southbound exclusive right-turn lane on Golden Sage Road approaching Woodmen Road (a continuous right-turn lane within the 150 feet between the Woodmen Frontage Road and Woodmen Road).	lf/when needed to maintain acceptable level of service/traffic operations and/or to control vehicle queues.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
15	Signalization of Golden Sage Road/Woodmen Frontage Road or reconstruction as a modern roundabout; Future additional laneage may be necessary at this intersection to accommodate vehicle queues and for traffic operations.	lf/when needed to maintain acceptable level of service/traffic operations and/or to control vehicle queues.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.

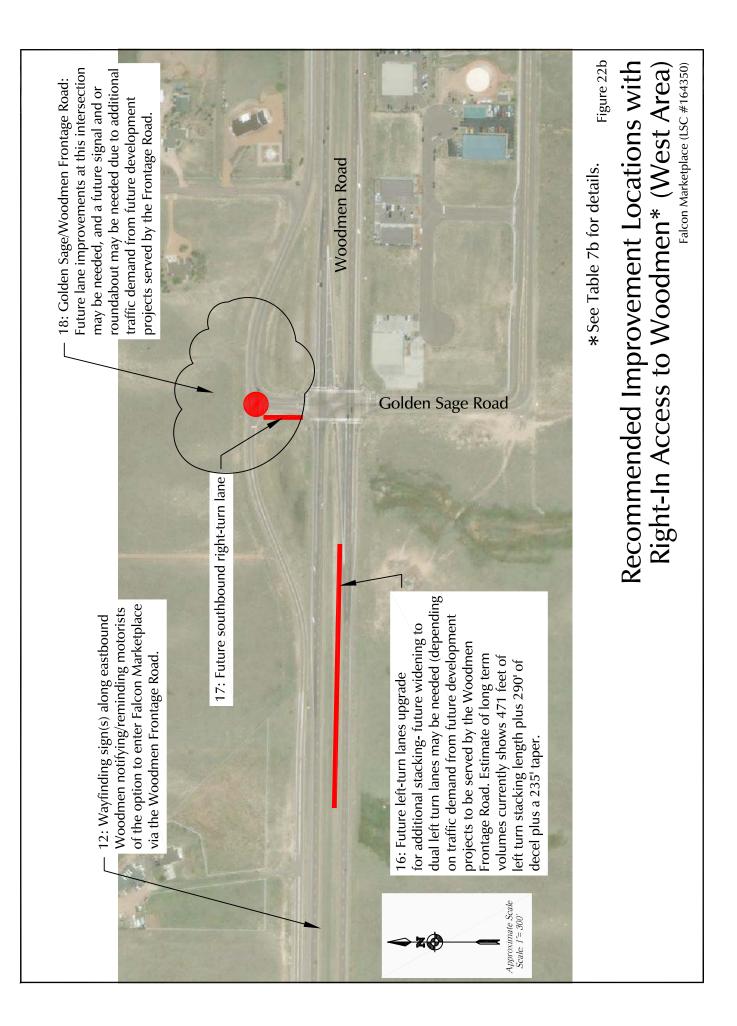
		Table 7b Falcon Marketplace	
Item #	Roadway Improvement	ents With Proposed Right-In Access to Woodmen Road	Responsibility
iteriti #		I iming le Road Impact Fee Program Fees and Taxes	
1	Woodmen Road Metropolitan District fees and taxes to be paid in lieu of Countywide Road Impact Fee Program fees and taxes. Woodmen Road has already been completed, but this project is joining the district.	District fees payable at platting	Falcon Marketplace
2	Meridian/Easton Signalization of Meridian Road/Eastonville Road intersection.	ville and Meridian Right-In/Right-Out Intersections Design and installation with the development of Falcon Marketplace once allowed by El Paso County.	Falcon Marketplace
3	Reconstruction of the Meridian center median south of Eastonville Road to achieve major street left-turn sight distance for the option of northbound/southbound protected/permissive left-turn signal phasing with interim single left-turn lanes northbound and southbound and to accommodate future northbound dual left-turn lanes f needed in the future. This lane should be 425 feet long plus a 200-foot taper. The laper would be back-to-back with the southbound dual left-turn taper.	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
4	Southbound right-turn deceleration lane on Meridian Road approaching Eastonville Road. This lane should be 235 feet long plus a 200-foot taper.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
5	Design and construction of continuous southbourd right-turn lanes and shoulder/bike lane on Meridian Road from Eastonville Road south to the proposed right-in/right-out and from the right-in/right-out south to Woodmen Road. (Note: Also please refer to related item #16 below.)	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
6	Widening of Eastorwile Road east of Meridian Road to add a westbound through lane and add width as feasible between the westbound left-turn lane and the westbound through lane due to the proposed dual left-turn lanes on the west side of the intersection. This adde width would allow for through lane alignment (with an acceptable offset across the intersection).	Design and installation with the development of Falcon Marketplace. Extent of this off-site improvement may be limited by available right-of-way and/or other existing constraints.	Falcon Marketplace
7	Design and construction of the proposed extension of Eastonville Road between Meridian and the proposed roundabout.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
	Design and construction of the public short successful of the succ	On-Site Improvements	
8	Design and construction of the public street connection through the site (Falcon Market Place). Design and construction of the proposed roundabout on-site west of the	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
9	Meridian/Eastonville intersection. This would include a "stub" to the north for the anticipated future street connection north to Bent Grass Meadows Drive.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
10	Design and construction of the proposed roundabout on-site at the east terminus of the Woodmen Frontage Road.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
	Extend eviating weathering dight two accounting two internet in the second state	Woodmen/Meridian Intersection	
11	Extend existing westbound right-turn acceleration lane on Woodmen Road at Meridian Road to provide a continuous right-turn lane between Meridian Road and the proposed right-in-only access.	Design and installation with the develoment of Falcon Marketplace.	Falcon Marketplace
12	Lengthening of Woodmen esatbound dual left-turn lanes. Lengthening of eastbound left-turn lanes and potential further furthening to provide a 24/boto tane transition taper. 290 feet of deceleration distance plus sufficient vehicle stacking distance. CURRENT lane length: Sol0 feet of dual left-turn lanes and 25-foot taper. Of this taper. 215 feet is full-width 'decard' distance. Remaining 20 feet of the required 235- food decel distance occurs in the fluxt 20 feet of the 50-food dual left turn lanes. The remainder of the dual left-turn lanes is stacking distance - 480 feet. ShORT TERN: Based on the short-term analysis, the existing lane provides sufficient stacking distance. LONG TERN. This lane will likely need to be extended to provide a total of 840 feet of dual left storage distance plus 235 feet of deceleration length plus a 200-foot taper.	Future extension of existing dual left-turn lanes once traffic queues regularly extend beyond the 480-foot stacking distance.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
13	Meridian northbound dual left-turn lanes: Lengthening of northbound left-turn lanes and potential turther future lengthening to provide a 200-foot lane transition taper, 235 feet of deceleration distance plus sufficient vehicle stacking distance. CURRENT lane configuration: 315 feet of dual left-turn lane length, 145 feet of single left-turn lane length, 145 feet of single left-turn lane. This provides a 150-foot taper, 235 feet of deceleration distance, and 225 feet of dual left stacking. SHORT TERM: The 315-foot dual left-turn lanes (235 feet plus a 200-foot taper, will need to be provided south of the end of the existing 315-foot dual left stacking lanes. For the deceleration distance, 235 feet plus a 200-foot taper will need to be provided south of the end of the existing dual lane width to the north. South of the deceleration distance 201 feet plus will need to be added in the median. These modifications will involve median reconstruction and restrips osuth of the existing dual left-turn stacking. 235-foot deceleration potion of the lane, a standard 200-foot taper, 235-foot deceleration distance, and 315 feet of dual left-turn stacking.	With the development of Falcon Marketplace, extend the northbound left-turn lane as described in the column to the left and as shown in Figure 25.	Short Term - Falcon Marketplace
14	Meridian northbound dual left-turn lanes. LONG TERM: Potentially, additional growth in the Falcon Area and east along Falcon Highway will add left-turning traffic demand. Long-term analysis indicates the potential future need for 400 feet of stacking distance plus the 235 feet of deceleration distance and the 200-foot taper.	Future (if necessary – Add additional lane length beyond #13 to provide additional stacking if/when needed (as shown in Figure 26).	By other future developments impacting this turn lane. Potential for fe program credit for improvements completed as this is a regional intersection.
		Golden Sage Intersections	
15	Directional wayfinding sign(s) on eastbound Woodmen Road upstream of Golden Sage - notifying/reminding eastbound motorists of the option to enter Falcon Marketplace via the Woodmen Frontage Road.	Design and installation with the development of Falcon Marketplace.	Falcon Marketplace
16	Lengthening of the current eastbound single left-turn deceleration lane on Woodmen approaching Golden Sage Road to provide a 240-foot transition taper (20:1 taper ratio). 200 test of deceleration distance plus sufficient vehicle stacking distance. <b>CURRENT</b> : 175-foot taper, plus a 465-foot left-turn lane which translates to a 175-foot taper, 290-foot deceleration distance, and 175 feet of stacking distance. <b>SHORT TERM</b> : Adequate stacking is available in the current turn lane - calculated queue length 141 feet. <b>LONG</b> <b>TERM</b> : Lengthen single left-turn lane and/or future implementation of dual left-turn lanes (if capacity needs dictate) to maintiain 290 feet of deceleration length, a 240-foot lane laper (20:1 taper ratio) plus provide sufficient vehicle stacking length - model indicates 471 feet of dual left stacking distance based on morming peak-hour projected volumes. I a dual left is implemented in the future, consideration will need to be given to the configuration on Golden Sage and the Golden Sage/Woodmen Frontage Road intersection to receive the dual left-turn movement.	Short Term: The existing lane is adequate based on the short- term analysis. Long Term: Future with additional development served by the north fontage road - extension of existing single left-turn lane and potentially widening in the median to provide dual left-turn insets to provide additional vehicle stacking distance as described in the column to the left.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
17	Southbound exclusive right-turn lane on Golden Sage Road approaching Woodmen Road (a continuous right-turn lane within the 150 feet between the Woodmen Frontage Road and Woodmen Road).	If/when needed to maintain acceptable level of service/traffic operations and/or to control vehicle queues.	Falcon Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of the improvements.
18	Signalization of Golden Sage Road/Woodmen Frontage Road or reconstruction as a modern roundabout; Future additional laneage may be necessary at this intersection to accommodate vehicle queues and for traffic operations.	If/when needed to maintain acceptable level of service/traffic operations and/or to control vehicle queues.	Faton Marketplace will consent to inclusion in a special improvement district comprising properties benefited by this improvement and will pay its pro-rata share (based on total traffic volumes) of the cost of th improvements.
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Revise per discussions Feb-March 2018 (no special improvement district).





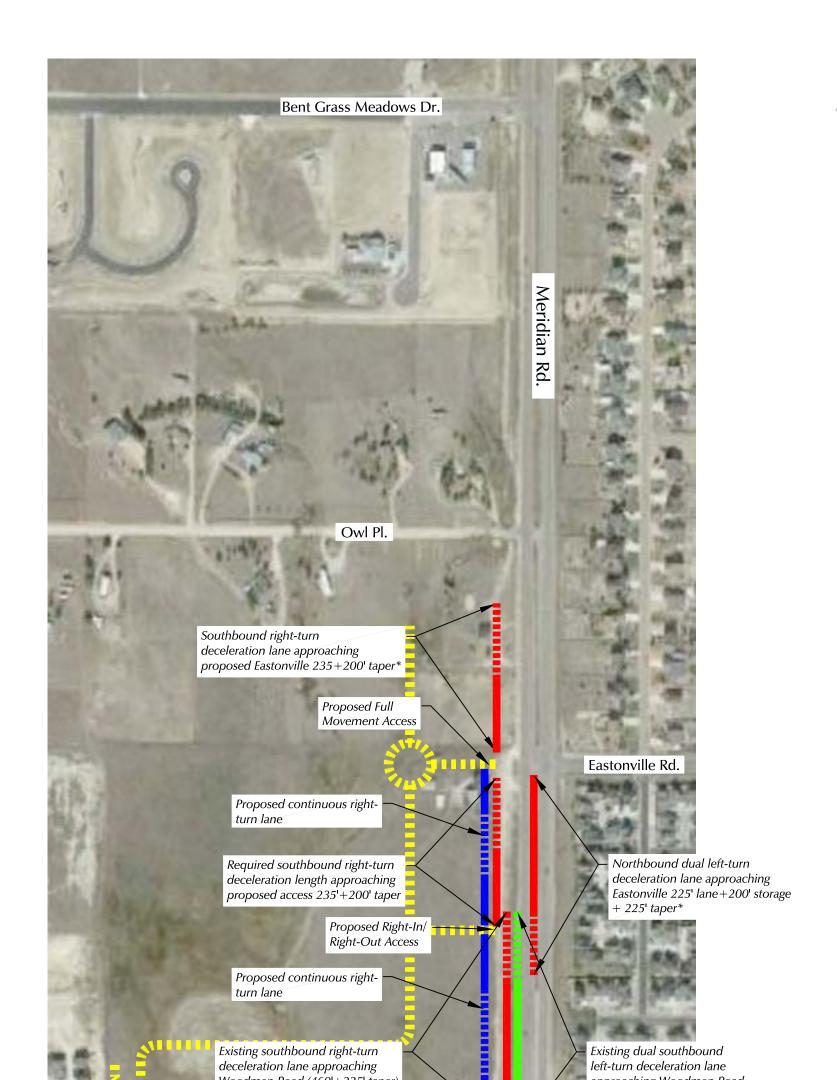




- Figure 23 shows the existing, proposed, and *Engineering Criteria Manual*-prescribed acceleration and deceleration lane lengths along Meridian Road between Eastonville and Woodmen Road, respectively. Figure 24 shows the existing, proposed and *Engineering Criteria Manual*-prescribed acceleration and deceleration lane lengths along Woodmen Road west of Meridian Road. Figure 25 shows additional detail for the continuous acceleration and deceleration lane based on the roundabout queuing analysis. The figure also addresses the Woodmen Road dual eastbound left-turn lanes—stacking lengths from the queueing analysis plus deceleration and taper distances. The existing dualwidth portion of eastbound left-turn lanes on Woodmen Road approaching Meridian Road is about 500 feet long. These lanes will likely need to be lengthened as shown in the figure to accommodate the projected 2040 eastbound left-turn queue.
- The existing westbound right-turn acceleration lane extending west from the intersection of Woodmen/Meridian should be extended west to the proposed right-in-only site access and restriped as a continuous acceleration/deceleration lane. Figure 26 shows the recommended signing and striping for the proposed acceleration/deceleration lane.
- Figure 27 shows the recommended short-term improvement to the northbound left-turn deceleration lane on Meridian Road approaching the Woodmen Road intersection. These are based on the short-term total traffic queuing analysis results and criteria in the ECM.
- Figure 28 shows the future stacking distances for the northbound left-turn lane at the Woodmen/ Meridian intersection from the queueing analysis plus the deceleration length and taper lengths prescribed by the ECM. These are based on long-term projected volumes. These included projected traffic from future commercial developments along Meridian Road south of Woodmen as well as potential future development within areas east southeast of the Falcon area using Falcon Highway to the new Meridian Road connection. The growth patterns in this area are subject to change. Also, the future Dublin Boulevard connection to Falcon Highway may alter some of these projections.
- A southbound right-turn deceleration lane should be provided on Meridian Road approaching the Meridian Road/Eastonville Road intersection. This lane should be 235 feet long plus a 200-foot taper (based on the anticipated post-development posted speed limit of 45 mph on the southbound approach to this intersection).
- Continuous southbound right-turn acceleration/deceleration lanes should be provided on Meridian Road between Eastonville and the proposed right-in/right-out access and between this proposed access and Woodmen Road.
- A northbound left-turn lane should be constructed within the existing center median on Meridian Road approaching the Meridian Road/Eastonville Road intersection. This lane should be 425 feet long plus a 200-foot taper. The median south of Eastonville should be reconstructed with a narrow six-foot-wide raised median nose with pavement for the remainder of the space between this new median nose and the northbound Meridian through lanes. This design is recommended for two reasons. The first is because of the current position of the southbound left-turn lane (immediately adjacent to the southbound through lanes). The second is that this design of the median modification would allow for conversion to a dual left-turn lane accomplished by restriping when needed in the future. Also, modification to the southbound left-turn lane on the north side of the intersection may

be needed to maintain sight distance depending on the signal phasing and/or to avoid conflicting leftturning vehicle paths. This will be evaluated further with the Preliminary Plan.

- The site plan shows the proposed connection to the frontage road, and the internal public street connection through the site between the east end of the frontage road and Meridian Road.
- The applicant will likely be required to widen Eastonville Road east of Meridian Road to add a westbound through lane and add width as feasible between the westbound left-turn lane and the westbound through lane due to the proposed dual left-turn lanes on the west side of the intersection. This added width would allow for through lane alignment (with an acceptable offset across the intersection). The extent of this offsite improvement may be limited by available right-of-way and/or other existing constraints.
- The development will be required to install a traffic signal (or escrow funds) for a traffic signal at the Meridian/Eastonville intersection. This project is planned to be developed in one phase, therefore the signal is planned to be installed once allowed by the County.







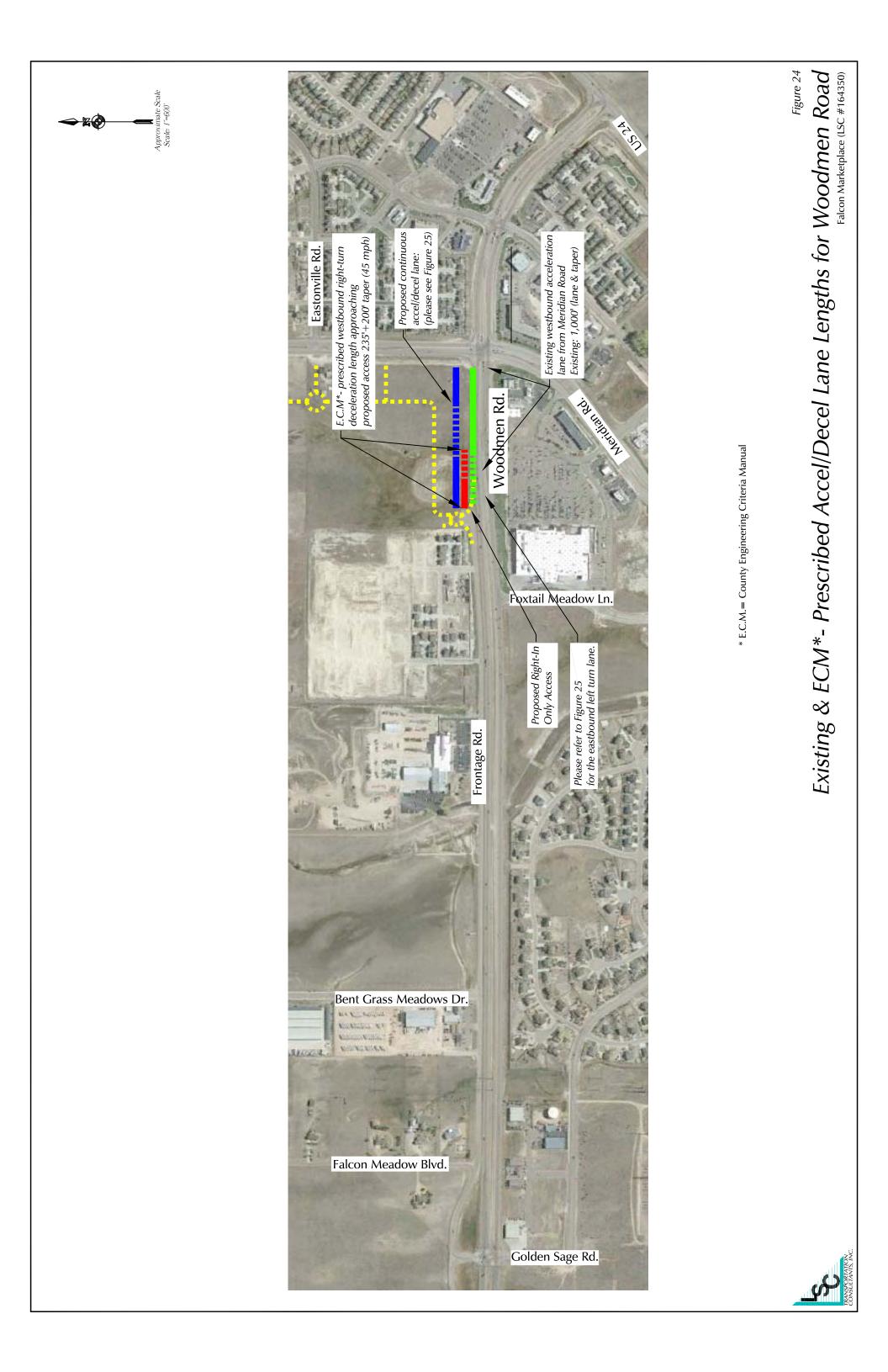
Note: Lane lengths from the Meridian Road (North) Corridor Plan (December 2009). Also assumes an anticipated post-development speed limit of 45mph (50mph design speed).

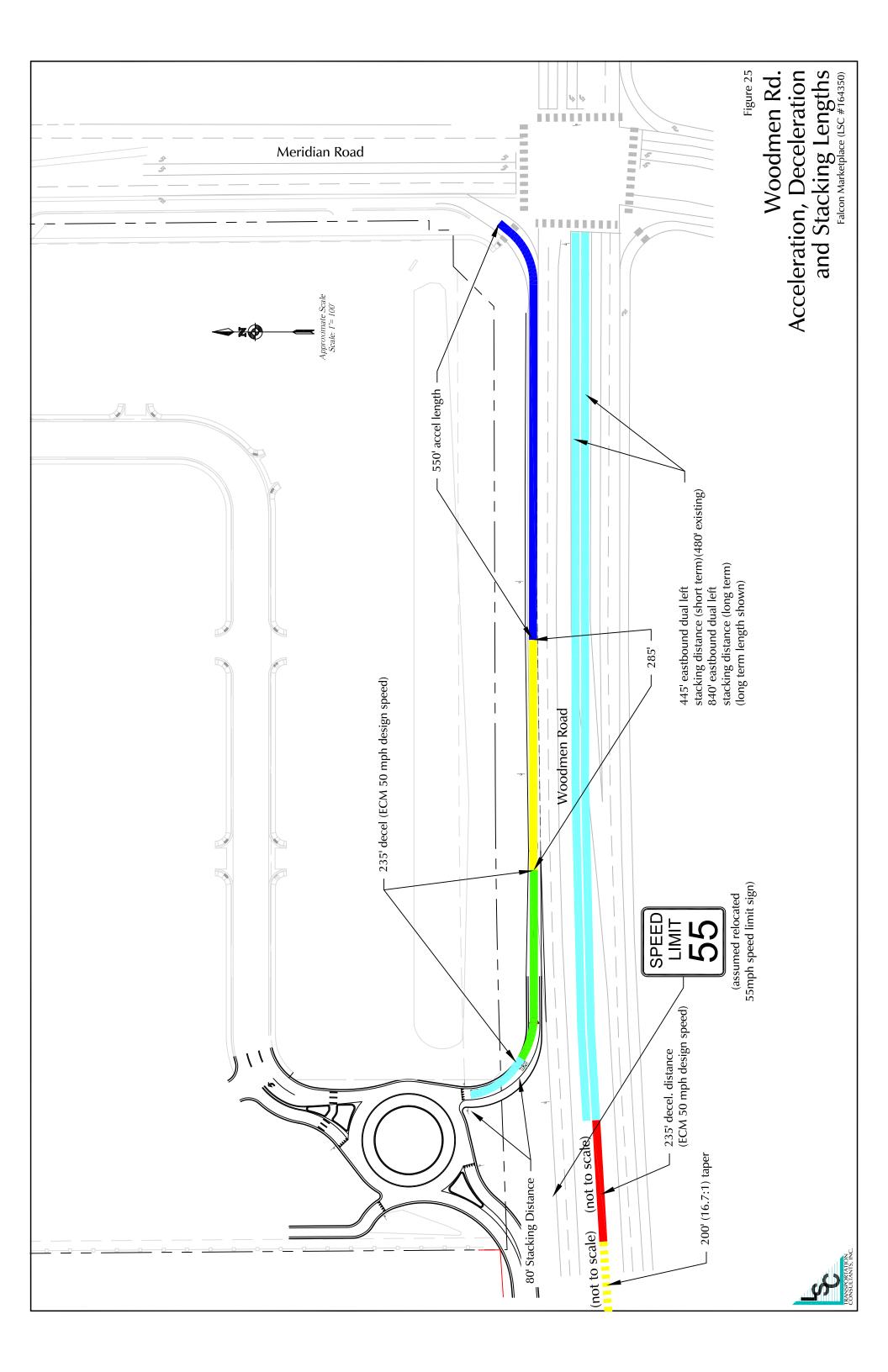
Figure 23

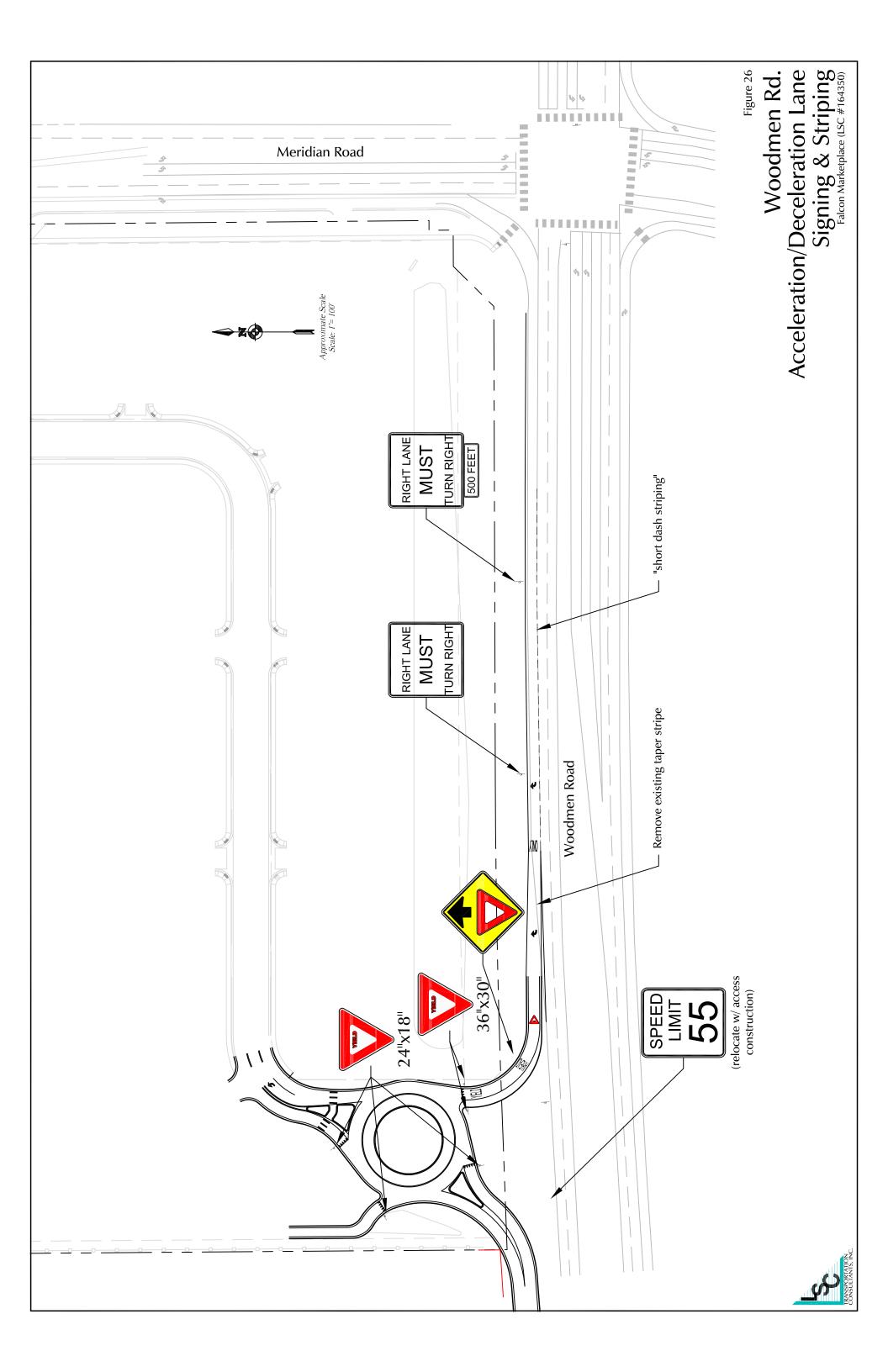


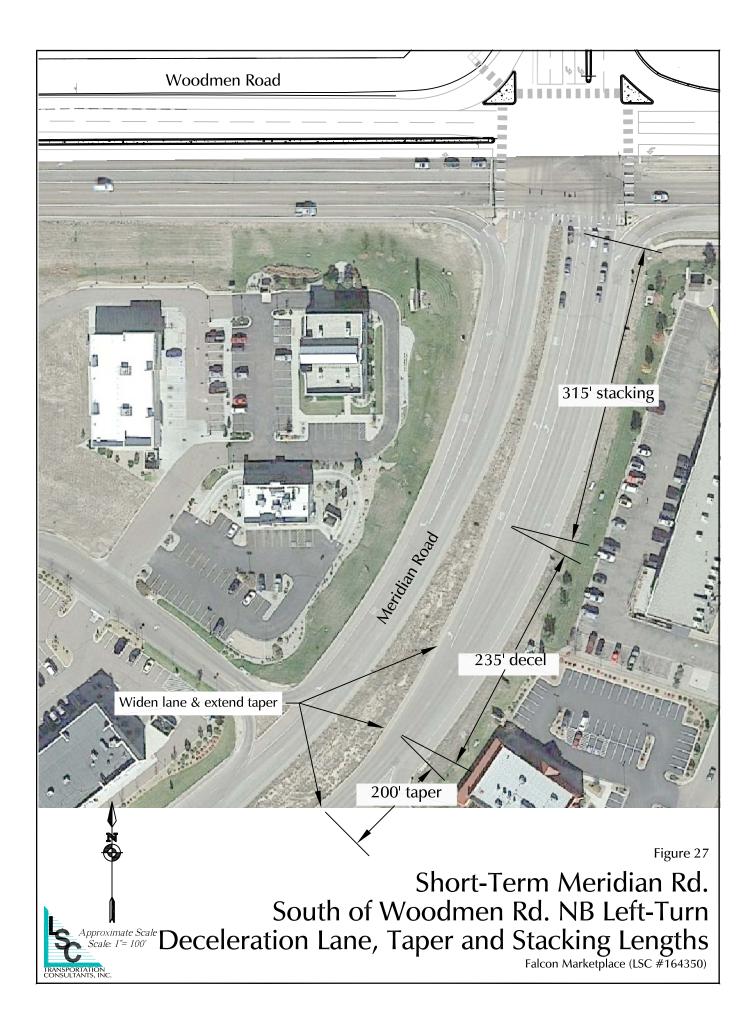
Accel/Decel Distances for Meridian Road North of Woodmen Road

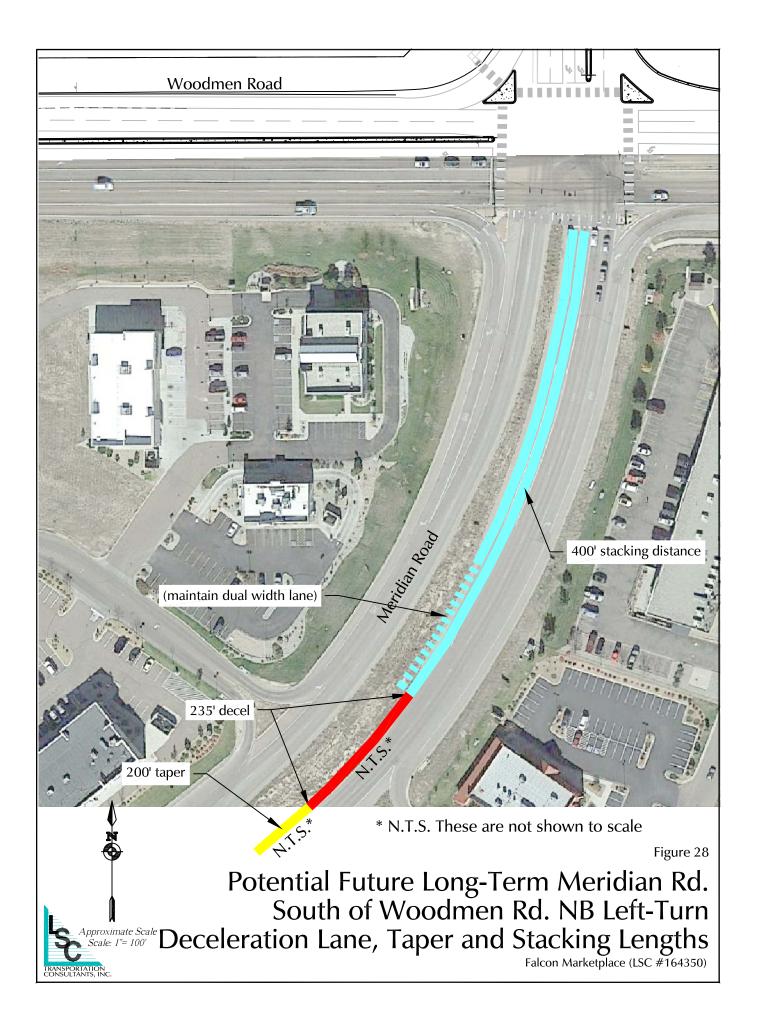
Falcon Marketplace (LSC #164350)











## **Summary and Conclusions**

### WOODMEN RIGHT-IN-ONLY ACCESS WITH ROUNDABOUT

The purpose of this updated traffic report is to analyze the currently proposed right-in-only access **combined with** a proposed roundabout intersection with the Woodmen Frontage Road, which would significantly improve access not only to the site, but also to the properties to the west along the Woodmen Frontage Road. This new provision for public access from westbound Woodmen Road to the westbound Woodmen Frontage Road is a <u>significant change</u> from the previously proposed right-in-only access configuration that essentially only served the proposed Falcon Marketplace. Please refer to the report section above for details.

### **TRIP GENERATION**

• The site is projected to generate about 9,558 new external vehicle-trips on the average week-day, with about half entering and half exiting the site during a 24-hour period. During the morning peak hour, about 495 vehicles would enter and 370 vehicles would exit the site. During the afternoon peak hour, about 716 vehicles would enter and 701 vehicles would exit the site.

### **PROJECTED LEVELS OF SERVICE**

- The eastbound left-turn movement at the Woodmen Road/Meridian Road intersection currently operates at LOS D. Given the high existing and projected background traffic demand for this turning movement, the LOS is projected to operate at LOS E during the afternoon peak hour by 2040 with or without the proposed right-in access to Woodmen Road. Important note: This condition has little to do with this site, rather it is primarily due to the background traffic demand. The northbound left-turn movement is projected to operate at LOS D during the morning peak hour and LOS E during the afternoon peak hour based on the projected 2040 total traffic volumes without the proposed right-in access. The northbound left-turn movement is projected to operate at LOS E during both the morning and afternoon peak hours based on the projected 2040 total traffic volumes with the right-in access. The northbound through movement is projected to operate at LOS F during the afternoon peak hour assuming no Woodmen access and LOS E assuming the proposed right-in access. The northbound through movement is projected to be about 115 vehicles per hour higher (afternoon peak hour) with the "no-right-in" scenario.
- The intersection of Meridian/Eastonville was assumed to be signalized once the site is built out. As a signalized intersection, it is projected to operate at an overall LOS C or better based on the short-term total traffic volumes.

By 2040 the overall intersection is projected to operate at LOS D during the afternoon peak hour. The northbound left-turn movement is projected to operate at LOS F during the afternoon peak hour assuming no Woodmen access and LOS D during the afternoon peak hour assuming the proposed right-in access. The northbound left-turn volume is projected to be about 230 vehicles per hour higher (afternoon peak hour) with the "no-right-in" scenario.

- Based on the projected short-term and 2040 total traffic volumes, all movements at the proposed right-in/right-out-only access to Meridian Road are projected to operate at LOS D or better during the peak hours as a <u>stop-sign-controlled intersection</u>.
- The proposed two-way stop-sign-controlled access points to the public internal road are projected to operate at LOS C or better for all movements during the peak hours based on the 2040 total traffic volumes with or without the proposed right-in-only access to Woodmen Road.
- The northbound and southbound through movements at the proposed all-way, stop-sign-controlled intersection are projected to operate at <u>LOS D</u> or better based on the 2040 total traffic volumes <u>with</u> the proposed right-in-only access. These movements are projected to operate at <u>LOS E and F</u>, respectively based on the 2040 afternoon peak hour <u>without</u> the proposed right-in access to Woodmen Road.
- All movements at the proposed roundabouts at the terminus of the Woodmen Frontage Road and at the intersection of the internal public road and Eastonville Road are projected to operate at LOS B or better based on the projected 2040 total traffic volumes with and without the proposed right-in-only access to Woodmen Road.
- This report also includes a weaving section level of service for westbound Woodmen Road between the Meridian Road intersection and the proposed right-in to the west. Please refer to the Level of Service section of this report for details.

## COMPARISON OF ANALYSIS SCENARIOS

• The analysis of the scenarios with and without the proposed Woodmen Road access clearly indicates better area intersection operations for the traveling public, including the portion of the traveling public that will shop/dine, etc. at this site, if the proposed Woodmen Road access is constructed. The access with the proposed roundabout would also significantly improve emergency vehicle and public access for the properties to the west along the Woodmen North Frontage Road. Please refer to the section above for details.

## **RECOMMENDED IMPROVEMENTS**

• Please refer to Chapter 8 which presents detailed recommendations for the access scenarios.

# Summary of Crash History





## 2014 – YTD 2017 Crash Data: Woodmen Rd, Meridian Rd, and Highway 67

Road	2014	2015	2016	2017	Total
Woodmen Rd	16	12	14	7	49
Golden Sage Rd	7	2	7	6	22
Meridian Rd	9	10	7	1	27
Total	16	12	14	7	49

#### Woodmen Rd Crashes 2014 – YTD 2017

#### Woodmen Rd Fatal & Injury Crashes 2014 – YTD 2017

Road	2014	2015	2016	2017	Total
Woodmen Rd	2	2	3	1	8
Golden Sage Rd	1		2	1	4
Meridian Rd	1	2	1		4
Total	2	2	3	1	8

#### Meridian Rd Crashes 2014 – YTD 2017\*

Road	2014	2016	2017	Total						
Meridian Road	4	2	1	7						
Eastonville Rd	3	2	1	6						
Owl Pl	1			1						
Total	4	2	1	7						
*Ne such as in these succes in 2015										

\*No crashes in these areas in 2015

#### Meridian Rd Fatal & Injury Crashes 2014 – YTD 2017\*

Road	2014	2016	2017	Total						
Meridian Road	1	1	1	3						
Eastonville Rd		1	1	2						
Owl Pl	1			1						
Total	1	1	1	3						
*Ne meshes in these energies 2015										

\*No crashes in these areas in 2015

#### Highway 67 Crashes 2014 – YTD 2017\*

Road	2016	Total
H67	1	1
FAIRFIELD LN	1	1
Total	1	1

\*No fatal and/or injury crashes at this area during this time period





Year	Month	l Accident Time	Number Killed	Number Injured		Road Description Code	Road Condition Code	Lighting Condition Code	Adverse Weather Condition Code	Direction Of Travel Code		Roadway Speed Limit	Estimated Vehicle Speed	Ejection	Suspected Alcohol	Suspected Drugs	Injury <u>Severity</u>	Accident Narrative
2014	2	3:09:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	East	Going	55	5	No	No	No	No Injury	Vehicle #1 was stopped at the red light of Woodmen Rd and Meridian Rd. Vehicle #2 was stopped at the red light of Woodmen Rd and
2014	4	5:45:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	East	Straight Going	45	30	No	No	Yes	No Injury	moved prior to investigation. Vehicle #2 was stopped in the right, left turn lane on eastbound Woodmen Rd to Meridian Rd. Vehicle #1 was traveling eastbound on \
					-1		,	., .			Straight						- ,- ,	into the rear of Vehicle #2. Both vehicle's were driven to rest. Vehicle #1 was traveling westbound Woodmen Rd approaching the intersection with Meridian Rd; Vehicle #2 was northbound Meridia
014	6	4:00:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	West	Going Straight	45	45	No	No	No	No Injury	lane, at the intersection with Woodmen Rd. The driver of vehicle # 1 failed to observe the red traffic light and collided its right front wi 3. Vehicle # 2 rotated clockwise approximately 48' and came to final rest, on all wheels, facing southeast. Vehicles # 1 and # 3 came to
14	6	5:10:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	West	Going Straight	35	15	No	No	No	No Injury	Vehicle #1 was westbound on Woodmen Road in the right lane. Vehicle #1 failed to stop for a red light and struck vehicle #2 with left fi northbound Meridian Road and was struck in the right front by vehicle #1. Vehicle #2 was driven to final rest.
14	6	1:10:00 PM	0	0	Property	At Intersection	Wet	Daylight	Fog	West	Going	45	40	No	No	No	Complaint of	Vehicle # 1 was westbound Woodmen Road, in the left lane, approaching the intersection with Meridian Road; Vehicle # 2 was westbou
								, ,	Ŭ		Straight						Injury	failed to observe stopped traffic and collided its front with the rear of vehicle # 2; vehicle # 2 then collided its front with the rear of veh Vehicle # 1 was westbound Woodmen Road, in the right lane, approaching the intersection with Meridian Road; Vehicle # 2 was south
014	6	11:25:00 AM	0	2	Injury	At Intersection	Dry	Dawn or Dusk	Wind	West	Going Straight	45	45	No	No	No	Evident - incapacitating	failed to observe its red traffic signal, entered the intersection, and collided its right front with the left side of vehicle # 2; the right rear intersection for approximately 159'7" before traveling off the southwest side of the roadway. Vehicle # 1 traveled approximately 18'3"
																		coming to final rest in the westbound Woodmen Road acceleration lane, on all wheels, facing northwest.
14	8	10:17:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	East	Going Straight	45	10	No	No	No	No Injury	Vehicle #1 was eastbound on Woodmen Rd. Vehicle #2 was eastbound on Woodmen Rd, stopped at the traffic light. Vehicle #1's front l vehicles moved prior to investigation.
4	8	2:55:00 PM	0	0	Property	Intersection Related	Dry	Daylight	None	East	Going Straight	45	20	No	No	No	No Injury	Vehicle 1 was eastbound on Woodmen approaching Meridian in the right side left turn lane for northbound Meridian. Vehicle 2 was ea vehicle 1 and stopped for a red signal light. Vehicle 1 struck vehicle 2 on the rear with its front. Both vehicles moved from rest onto the
14	11	7:00:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	North	Slowing	45	20	No	No	No	No Injury	Vehicle #1 and Vehicle #2 where in the right turn lane from E. Woodmen Rd to N. Meridian Rd. Vehicle #2 was stopped yielding to an o were driven to rest.
																		Vehicle #1 was traveling eastbound on Woodmen Road and was approaching the stop light, which was red. Vehicle #2 was stopped at
15	2	2:00:00 AM	0	0	Property	At Intersection	Dry	Dark - Lighted	None	East	Going Straight	45	10	No	No	No	No Injury	#2 began to make a left turn. Vehicle #1 ran the stop light and collided its front with the right side of vehicle #2. After impact, vehicle
						Intersection	-						_					Both vehicles were moved prior to arrival and point of impact could not be determined. Vehicle 1 was westbound on Woodmen Road in the right turn lane. Vehicle 2 was westbound on Woodmen Road, in the right turn lane
15	2	10:20:00 PM	0	0	Property	Related	Dry	Daylight	None	West	Slowing Going	45	5	No	No	No	No Injury	Both vehicles were moved from the scene prior to officer arrival.
15	4	2:20:00 AM	0	0	Property	Intersection Related	Dry	Dark - Unlighted	None	East	Straight	45	5	No	No	No	No Injury	Vehicle's 1 and 2 were eastbound on Woodmen Rd waiting to make a left turn onto northbound Meridian Rd. Vehicle 1 moved prior to
5	7	10:55:00 PM	0	0	Property	Intersection Related	Dry	Daylight	None	East	Going Straight	45	5	No	No	No	No Injury	Vehicles 1 and 2 were eastbound on Woodmen Rd approaching Meridian Rd in the left lane. Both vehicles were stopped for traffic aher were moved prior to investigation.
5	7	6:45:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	West	Making Left Turn	45	15	No	No	No	No Injury	Vehicle #1 was in the left, left turn lane on Woodmen Rd at Meridian Rd. Vehicle #2 was in the right, left turn lane on Woodmen Rd and the right lane colliding its right front with the left rear of Vehicle #2. Both vehicle's were moved prior to my investigation.
5	-	44 50 00 014				<b>A 1 1 1 1 1 1 1 1 1 1</b>		D. P.L		<b>C</b> 11	Going		45				Evident - non-	The traffic lights at the intersection at Woodmen Rd and Meridian Rd were not working (No lights were illuminated). Vehicle #1 was so
	/	11:50:00 PM	0	1	Injury	At Intersection	Dry	Daylight	None	South	Straight	55	15	No	No	No	incapacitating	through lane. Vehicle #2 started forward in the intersection. Vehicle #1 did not stop and vehicle #2 collided its front with the driver's s vehicles moved prior to investigation.
		42.55.00.014						D. P.L		<b>F</b> 1	Changing		20	X			Evident - non-	Vehicle 1, a bicycle, was eastbound Woodmen Rd, on the right paved shoulder. Vehicle 2 was eastbound Woodmen Rd, in the right the through lane and in the path of Vehicle 2. Vehicle 2 struck its driver's side front with the bicycle and its rider. The point of impact occu
	9	12:55:00 PM	0	1	Injury	Non-Intersection	Dry	Daylight	None	East	Lanes	55	20	Yes - Full	No	No	incapacitating	hood of Vehicle 2 and were carried. Vehicle 2 came to rest 120'7" east of the point of impact, 8'6" to the left of the road edge. The bic rest 160'9" east of the point of impact and 17'1" to the left of the right road edge. One tire from the bicycle became lodged under Veh
																		Vehicle #1 was traveling westbound on Woodmen Road in the number three lane and was approaching a red light at the intersection w
																		intersection with Meridian Road. Vehicle #2 had a green turn arrow and proceeded to turn left onto Meridian Road. Vehicle #1 disreg vehicle #2, 44.2 feet west and 29.8 feet south of the reference point. Both vehicles then moved from final rest to a safe location.
	9	2:05:00 AM	0	0	Property	At Intersection	Dry	Dark - Lighted	None	West	Going Straight	45	25	No	No	No	No Injury	
																		The reference point was the northeast traffic control signal pole.
																		Both vehicles were moved from final rest prior to arrival.
.5	10	7:30:00 PM	0	0	Property	Intersection	Dry	Daylight	None	East	Drove	45	55					Vehicle 1 was traveling eastbound in the westbound lanes on Woodmen approaching Meridian. Vehicle 2 was southbound on Meridian traveled for 360 feet leaving rolling tire marks on the shoulder, and then went northbound in the southbound merge lane of Meridian
			-	-	,	Related	,				Wrong Way							north 25 feet and came to rest facing northeast on its wheels in the southbound merge lane of Meridian. Both occupants of vehicle 1, v southwest on the left gore of southbound Meridian to westbound Woodmen lane.
15	12	9:45:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	Northwest	Going	45	5	No	No	No	No Injury	Vehicle # 1 was westbound E. Woodmen Road, in the right turn lane, approaching the intersection with Meridian Road; vehicle # 2 was with the rear of vehicle # 2. Both vehicle's were moved from final rest prior to my arrival.
.6	4	4:30:00 PM	0	1	Iniury	Non-Intersection	lcy	Daylight	now/Sleet/Ha	i West	Slowing	50	35	No	No	No	No Injury	VEHICLES 1 AND 2 WERE WESTBOUND ON WOODMEN RD, IN THE RIGHT LANE. VEHICLE 2 STOPPED IN TRAFFIC. THE FRONT OF VEHICL
							.,	., .									- ,- ,	DRIVEN TO REST ON THE RIGHT SHOULDER PRIOR TO ARRIVAL. EXACT POINT OF IMPACT UNKNOWN. Vehicle 2 was westbound Woodmen Rd, in the right most though lane, stopped in traffic, at the red light, at the intersection of Meridia
16	6	7:10:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	West	Slowing	45	20	No	No	No	No Injury	2. Vehicle 1 failed to safely stop behind Vehicle 2. Vehicle 1 struck its front with the rear of Vehicle 2. The point of impact occurred in were driven to final rest.
.6	6	12:49:00 AM	0	0	Property	Intersection	Dry	Dark - Lighted	None	East	Going	45	30	No	Yes	No	No Injury	Vehicle 1 was traveling east in the westbound merge lane from the intersection of Meridian road. Vehicle 1 did not turn or stop and co
6	8	6:45:00 PM	0	0	Property	Related At Intersection	Dry	Daylight	None	West	Straight Going	45	5	No	No	No	No Injury	and struck the utility pole with the right front fender. Vehicle 1 continued, making a left turn and stopping in the south bound turn land VEHICLES 1 AND 2 WERE WESTBOUND ON WOODMEN RD. VEHICLE 2 WAS STOPPED IN TRAFFIC. THE FRONT OF VEHICLE 1 IMPACTED 1
	-										Straight Going						Complaint of	MOVED OUT OF TRAFFIC PRIOR TO ARRIVAL. EXACT POINT OF IMPACT UNKNOWN. Vehicle #1 was in the right through lane westbound Woodmen Rd approaching Meridian Rd. Vehicle #2 was in the right through lane m
16	10	1:45:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	West	Straight Going	45	45	No	No	No	Injury Complaint of	to stop for the red light and collided its front with the right side of Vehicle #2. Both vehicles were driven to rest. Vehicle 1 was southbound on Meridian Road, in the right turn lane. Vehicle 2 was southbound on Meridian Road, in the right turn lane
016	11	3:35:00 PM	0	0	Property	Related	Dry	Daylight	None	West	Straight	45	10	No	No	No	Injury	Vehicle 1 followed Vehicle 2. Vehicle 1 stopped for traffic. Vehicle 1 struck Vehicle 2, in the rear, with its front. Both vehicles were move
017	5	7:30:00 AM	0	0	Property	Intersection Related	Dry	Dark - Lighted	None	East	Making Left Turn	45	15	No	Yes	Unknown	No Injury	Vehicle 1 and vehicle 2 were both stopped on eastbound E Woodmen Rd in the #2 left turn lane to N Meridian Rd with vehicle 1 behind came to final rest on their wheels facing east.

nd Meridian Rd. Vehicle #1's began to roll forward and its front bumper impacted the rear of Vehicle #2. Both vehicles

Woodmen Rd in the right, left turn lane approaching the intersection. Vehicle #1 failed to stop and collided its front

idian Rd, in the right lane, at the intersection with Woodmen Rd; Vehicle # 3 was northbound Meridian Rd, in the left with the right rear of vehicle # 2. Vehicle # 1 continued southwest and collided its front with the right side of vehicle # to final rest, on all wheels, facing north.

ft front of vehicle #1. Vehicle #1 was driven to final rest. Vehicle #2 was eastbound on Woodmen Road turning left to

bound Woodmen Road, in the left lane, and stopped at the red light at Meridian Road behind vehicle # 3. Vehicle # 1 rehicle # 3. All vehicles were moved from final rest prior to my arrival.

thbound Meridian Road, in the right lane, approaching the intersection with Woodmen Road. The driver of vehicle # 1 ear side of vehicle # 1 then collided with the left rear side of vehicle # 2. Vehicle # 1 traveled southwest through the '3" off road before coming to final rest, on all wheels, facing southwest. Vehicle # 2 traveled approximately 132' before

nt bumper impacted the rear bumper of Vehicle #2, approximately 6' from the right side of the right turn lane. Both

eastbound on Woodmen approaching Meridian in the right side left turn lane for northbound Meridian in front of he northbound shoulder of Meridian prior to arrival on scene.

on coming vehicle. Vehicle #1 failed to stop and collided its right front into the left rear of Vehicle #2. Both vehicle's

at the westbound stop light on Woodmen Road, waiting to make a left turn. The green turn arrow changed and vehicle le #1 and vehicle #2 came to final rest in the intersection.

ne, ahead of Vehicle 1. Vehicle 2 stopped for traffic. Vehicle 1 slowed and struck Vehicle 2 in the rear with its front.

to vehicle 2 and collided its front with the rear of vehicle 2. Both vehicles were moved prior to investigation.

head. Vehicle 1 began to move before vehicle 2. Vehicle 1 collided its front with the rear of vehicle 2. Both vehicles

and Meridian Rd. Both vehicle's were making a left turn and Vehicle #1 failed to drive within its lane and traveled into

southbound on Meridian Rd in the left through lane. Vehicle #2 was stopped on westbound Woodmen Rd in the left s side of vehicle #1. Vehicle #1 rotated counter-clockwise and then clockwise, traveling up onto a raised median. Both

through lane, approaching Vehicle 1. As Vehicle 2 neared Vehicle 1, Vehicle 1 moved from the shoulder into the right curred 14'3" to the left of the right road edge and 1013.84 west of Meridian Rd. The bicycle and rider fell onto the bicycle came to rest 28'9" to the left of the right road edge and 136'3" east of the point of impact. The rider came to ehicle 2 at some point during the collision. The rider and bicycle were moved prior to investigation.

n with Meridian Road. Vehicle #2 was stopped in the number one lane on eastbound Woodmen Road at the egarded the red light and continued to travel through the intersection and collided its right front with the right side of

lian preparing to take a merge lane onto westbound Woodmen. Vehicle 1 went off the north side of the roadway and an to westbound Woodmen. Vehicle 1 then struck vehicle 2 on the right side with its right front. Vehicle 1 continued 1, which later was confirmed as stolen, fled the scene. Vehicle 2 continued southwest for 15 feet and came to rest facin

vas in front of vehicle # 1. The driver of vehicle # 1 failed to observe that vehicle # 2 was stopped and collided its front

ICLE 1 IMPACTED THE REAR OF VEHICLE 2, APPROXIMATELY .1 MILE WEST OF MERIDIAN RD. BOTH VEHICLES WERE

dian Rd. Vehicle 1 was westbound Woodmen Rd, in the right most through lane, approaching Meridian Rd and Vehicle in the right most through lane of westbound Woodmen Rd, just east of Meridian Rd. After the impact, both vehicles

collided with the raised curb of the pedestrian island on the northwest corner of the intersection. Vehicle 1 continued ane of Meridian Road to eastbound Woodmen Road. Vehicle 1 came to rest in it's wheels facing north. D THE REAR OF VEHICLE 2, APPROXIMATELY 15' EAST OF MERIDIAN RD, IN THE LEFT THRU LANE. BOTH VEHICLES WERE

e northbound Meridian Rd at Woodmen Rd. Vehicle #2 had green light and entered the intersection. Vehicle #1 failed

ne ahead of Vehicle 1. Vehicle 2 turned right onto Woodmen Road and traveled westbound in the acceleration lane. oved from the scene prior to officer arrival.

ind vehicle 2. Vehicle 1 suddenly drove forward and struck the rear of vehicle 2 with its front. Vehicles 1 and 2 both

	AGE/WOOI	। ः Accident	Number Killed	Number Injured	FIP	Road Description Code	Road Condition Code	Lighting Condition Code	Adverse Weather Condition Code		Vehicle Movement Code	Roadway Speed Limit	Estimated Vehicle Speed	Ejection	Suspected Alcohol	Suspected Drugs	Injury <u>Severity</u>	Accident Narrative
2014	1	11:58:00 PM	0	1	Injury	Non-Intersection	Dry	Dark - Unlighted	None	East	Going Straight	55	75	No	Yes	No	No Injury	Vehicles 1 and 2 were eastbound on Woodmen Road in the number two lane approaching the intersection of Golden Sage Road. Ve clockwise approximately 90 degrees across the right shoulder due to the impact before exiting the right side of the roadway while ir then began to roll while colliding with a barb wire fence with its top. Vehicle 2 continued southeast through a field for approximatel to a controlled stop onto the right shoulder approximately 460 feet from the initial point of impact.
2014	1	7:15:00 PM	0	0	Property	Intersection Related	Dry	Dark - Lighted	None	East	Going Straight	55	50	No	No	No	No Injury	VEHICLE #1 WAS TRAVELING EAST ON WOODMEN ROAD, APPROACHING GOLDEN SAGE ROAD, IN THE RIGHT LANE. VEHICLE #2 WA: IT AND COULD NOT STOP. VEHICLE #1 SWERVED TO THE RIGHT AND COLLIDED ITS DRIVER'S SIDE FRONT TO THE PASSENGER SIDE R
2014	1	2:30:00 AM	0	0	Property	Intersection	Dry	Dark - Lighted	None	East	Going	55	40	No	No	No	Complaint of	Vehicle 1 was traveling east on Woodmen Road. Vehicle 2 was stopped behind several vehicles in the left turn lane to Golden Sage
2014	8	9:31:00 PM	0	0	Property	Related Non-Intersection	Dry	Daylight	None	West	Straight Going	55	55	No	No	No	Injury No Injury	partially off road and into the median when I arrived on scene. Vehicle 1 was westbound on Woodmen Road. The driver of Vehicle 1 fell asleep at the wheel and drove off the right side of the road
	0			0		Intersection					Straight Going							controlled stop, on its wheels, and stopped facing west.
014	9	9:40:00 PM	0	0	Property	Related	Dry	Daylight	None	East	Straight	55	10	No	No	No	No Injury	Vehicles 1 and 2 were eastbound on Woodmen Rd stopped at Golden Sage Rd in the right lane. Vehicle 1 began to move before veh Vehicle #1 was traveling eastbound on E Woodmen Rd approaching Golden Sage Rd. Vehicle #2 was northbound on Golden Sage Rd
2014	9	1:00:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	East	Going Straight	55	55	No	No	No	Complaint of Injury	Vehicle #1 failed to stop for a solid red light and entered into the intersection. Driver #1 stated that the sun was in her eyes and she approximately 70.5' before coming to rest in the westbound lanes of traffic facing east. Vehicle #1 traveled approximately 19.4' before
2014	12	1:20:00 PM	0	0	Property	At Intersection	Dry	Dark - Lighted	None	East	Making Left Turn	55	15	No	No	No	Complaint of Injury	Both driver's and passenger refused medical on scene. Vehicle 1 was eastbound on Woodmen Road in the left turn lane. Vehicle 2 was westbound on Woodmen approaching the intersect Golden Sage Road in front of traffic. The front end of Vehicle 2 collided with the right side of Vehicle 1 in the intersection. Vehicle 1 of the intersection where it came to final rest in the eastbound left turn lane facing west, on its wheels.
2015	1	12:15:00 PM	0	0	Property	Intersection Related	Dry	Dark - Lighted	None	South	Making Right Turn	55	25	No	No	No	Complaint of Injury	Vehicle #1 was stopped on Golden Sage Road southbound at Woodmen Road. Vehicle #2 was traveling westbound on Woodmen Ro Woodmen Road. Driver #1 then realized how close Vehicle #2 was to the intersection. Vehicle #1 moved into the left lane to avoid a traveling westbound in the left lane. Both vehicles pulled off the roadway after the crash.
2015	9	6:49:00 PM	0	0	Property	, Intersection Related	Dry	Daylight	None	West	Slowing	55	30	No	No	No	Complaint of Injury	Vehicle 1 was westbound on Woodmen Road, in the left traffic lane. Vehicle 2 was westbound on Woodmen Road, in the left traffic (Driver 1 stated that he had not allowed for traffic ahead of Vehicle 2 to be stopped).
2016	4	10:35:00 PM	0	0	Property	Non-Intersection	Dry	Daylight	None	East	Going Straight	55	55	No	Yes	Yes	No Injury	Vehicle #1 was traveling eastbound on Woodmen Road. Vehicle #1 traveled off the right side of the roadway for 260.0 feet. Vehicle of the roadway for 29.5 feet. Vehicle #1 then started to roll clockwise 1 1/2 roll, while traveling for 65.0 feet before coming to final
2016	6	10:00:00 PM	0	0	Property	Non-Intersection	Dry	Daylight	None	East	Going	55	55	No	No	No	No Injury	Vehicle #1 was traveling eastbound on Woodmen Rd in the left lane approximately .4 miles west of Golden Sage Rd. An antelope rai shoulder and stopped facing east.
2016	8	11:21:00 PM	0	0	Property	Intersection Related	Wet	Daylight	Rain	East	Going Straight	55	45	No	No	No	No Injury	Vehicle #1 and #2 were eastbound on E Woodmen Rd. The front of vehicle #1 collided with the rear of vehicle #2 approximately 200
016	10	6:40:00 AM	0	0	Property		Dry	Dark - Unlighted	None	East	Going Straight	55	55	No	No	No	No Injury	Vehicle 1 was traveling east on Woodmen Road in the right lane. Driver 1 fell asleep and vehicle 1 ran off the right side of the roadv Vehicle 1 rotated 1/2 times and struck a fence. Vehicle 1 continued under the fence and came to rest in the field off the south side
2016	10	6:45:00 AM	0	1	Injury	Non-Intersection	Dry	Dark - Unlighted	None	West	Going Straight	55	50	Yes - Full	No	No	Evident - non- incapacitating	Vehicle 1 was traveling west on Woodmen Road in the left lane. The animal was crossing the roadway from right to left. Vehicle 1 s to the left shoulder.
2016	12	10:18:00 PM	0	2	Injury	Intersection Related	Dry	Daylight	None	East	Going Straight	55	65	No	No	No	Evident - non- incapacitating	Vehicle 2 was eastbound in the right lane on Woodman Road, Stationary at a red light at the intersection with Golden Sage Road. Ve Vehicle 1 skidded for 35 feet before colliding with Vehicle 2. After impact Vehicle 2 travelled 118 feet coming to final rest within the before travelling off road to the right for 11 feet before colliding with a lamp pole. Vehicle 1 came to final rest on all four wheels age
2016	12	12:40:00 AM	0	0	Property	Non-Intersection	Dry	Dark - Unlighted	Wind	East	Going Straight	55	65	No	No	No	No Injury	Vehicle 2 was eastbound on Woodmen Road in the right lane of traffic. Vehicle 1 was eastbound on Woodmen road in the right lane Vehicle 1 continued an additional 140 feet on road before leaving the road to the right. Vehicle 1 drove off road for 35 feet before of Vehicle 2 is unknown as it was moved prior to State Patrol arrival.
2017	3	7:15:00 PM	0	0	Property	Non-Intersection	Dry	Daylight	Wind	West	Going Straight	55	50	No	No	No	No Injury	Vehicle 1 was westbound on Woodmen Road, in the right traffic lane. Vehicle 2 was westbound on Woodmen Road, in the left traffi westbound when a mattress and box spring spilled onto the roadway, into the left traffic lane, from its bed. Vehicle 2 swerved and s prior to officer arrival.
2017	4	12:40:00 AM	1	0	Fatal	Intersection Related	Dry	Daylight	None	East	Making Right Turn	55	25	No	Yes	No	No Injury	Vehicle #1 was eastbound on Woodmen Road making a left turn in front of approaching traffic. Vehicle #2 was westbound on Wood traveled off the right side of the road and came to rest facing south.
2017	4	1:45:00 PM	0	0	Property	Intersection Related	Dry	Daylight	None	West	Going Straight	55	35	No	No	No	Complaint of Injury	Vehicle #1 was traveling westbound on Woodmen Road in the right lane approaching Golden Sage Road. Vehicle #2 was stopped on marks before its front left struck Vehicle #2's right rear. Both vehicles moved from final rest onto Golden Sage Road prior to my arriv
2017	4	7:35:00 PM	0	0	Property	Intersection Related	Dry	Daylight	Wind	East	Entering/ Leaving Parked Position	55	15	No	No	No	No Injury	VEHICLES 1 AND 2 WERE EASTBOUND ON WOODMEN RD, IN THE LEFT LANE, STARTING FROM A GREEN LIGHT, AT GOLDEN SAGE RE THE LEFT LANE. BOTH VEHICLES WERE DRIVEN TO REST IN THE MEDIAN, OUT OF TRAFFIC, PRIOR TO ARRIVAL. EXACT POINT OF IMP
2017	6	10:07:00 PM	0	0	Property	At Intersection	Dry	Daylight	None	East	Going Straight	55	35	No	No	No	No Injury	Vehicle #1 was eastbound on Woodmen Rd. Vehicle #2 was stopped at the intersection of Golden Sage Rd and Woodmen Rd. Vehicl bumper impacted Vehicle #2's driver side. Both vehicles moved prior to investigation.
2017	6	11:45:00 PM	0	0	Property	Non-Intersection	Dry	Daylight	None	East	Going Straight	55	60	No	No	No	No Injury	Vehicle 4 was stopping in traffic on Woodmen Road just west of Golden Sage facing east. Vehicle 1, 2, and 3 were traveling east on slowing, vehicle 1 could not stop and collided front to rear with vehicle 2. Vehicle 1 continued and came to rest in the left lane facin the roadway. Vehicle 3 collided with the rear of vehicle 4. Vehicle 3 came to rest on it's wheels facing east on the roadway. Vehicle 4.
2017	7	12:25:00 AM	0	0	Property	Intersection	Dry	Daylight	None	West	Going	55	15	No	No	No	No Injury	Vehicles 1 and 2 were westbound on Woodmen Rd approaching Golden Sage Rd in the left lane. Vehicle 2 stopped for traffic ahead.

MERIDIAN	V/EASTONV	ILLE																	
		। ः Accident	Numbe				Road Description	Road Condition	Lighting	Adverse Weather Condition		Movement	Roadway Speed	Estimated Vehicle		Suspected			
Year	Month	1 Time	Killed	Inj	ured	FIP	Code	Code	Condition Code	Code	Code	Code	Limit	Speed	Ejection	Alcohol	Drugs	Injury Severity	Accident Narrative
2014	8	12:20:00 AM	1 0		0	Property	At Intersection	Dry	Daylight	None	West	Making Left Turn	25	10	No	No	No	No Injury	Vehicle #1 was stopped on Eastonville Rd at Meridian Rd facing west. Vehicle #2 was northbound on Meridian Rd approaching Eastonville Vehicle #1 attempted to turn left onto southbound Meridian Rd. from Eastonville Rd in front of vehicle #2. Vehicle #1's left front was struc
																			Both vehicles were moved out of the intersection prior to my arrival.
2014	9	11:15:00 AM	10		0	Property	At Intersection	Dry	Dark - Unlighted	None	South	Making Left Turn	55	15	No	No	No	No Injury	Vehicle #1 was on Eastonville Rd at N Meridian Rd. Vehicle #2 a semi truck and trailer was traveling southbound in the left lane on N Meric to yield right-of-way to Vehicle #2. Vehicle #2 attempted to avoid Vehicle #1 and swerved to the right. Vehicle #2's trailer left side collided
2014	12	10:55:00 PM	1 0		0	Property	At Intersection	Dry	Daylight	None	West	Making Left Turn	35	20	No	No	No	No Injury	Vehicle #1 was stopped at the intersection of Eastonville Rd and Meridian Rd. Vehicle #2 was northbound on Meridian Rd. Vehicle #1 faile the Vehicle #2's passenger side. Both vehicles moved prior to investigation.
2016	5	10:30:00 PM	I 0		0	Property	At Intersection	Dry	Daylight	None	West	Making Left Turn		15	No	No	No	No Injury	Vehicles #1 and #2 were stopped at a stop sign on Eastonville Road westbound at Meridian Road. Vehicle #2 proceeded from the stop sign northbound lanes. Vehicle #1 proceeded across the northbound lanes as well, and it's front left struck Vehicle #2's right rear. Vehicle #1 fit stopped.
2016	6	4:45:00 PM	0		1	Injury	At Intersection	Dry	Daylight	None	East	Making Left Turn	30	2	No	No	No	No Injury	Vehicle 1 was stopped at the stop sign on Eastonville Road at the intersection with Meridian Road. Bicycle 1 was northbound on Meridian proceeded into the intersection and collided its front with the bicycle, knocking its rider to the ground at low speed. Both Vehicle 1 and Bi
2017	5	3:15:00 PM	0		2	Injury	At Intersection	Dry	Daylight	None	East	Making Left Turn	55	20	No	No	No	Evident - non- incapacitating	Vehicle #1 was turning eastbound from southbound Meridian Rd. Vehicle #2 was traveling northbound on Meridian Rd. Vehicle #1 turned in the northbound traffic lane of Meridian Rd. Vehicle #2 came to rest in the dich, facing northeast, off the east side of Meridian Rd. Vehicle #1 vision obstructed by large dump truck.

hicle 2 was ahead of vehicle 1. Vehicle 1 struck the left rear of vehicle 2 with its right front. Vehicle 2 began to rotate a broadside skid. Vehicle 2 continued off road broadside for approximately 18 feet before hitting a trip point. Vehicle 2 y 208 feet where it rolled an additional 2.5 times before coming to rest on its wheels facing south. Vehicle 1 was driven

S STOPPED ON WOODMEN ROAD AT GOLDEN SAGE, IN THE RIGHT LANE, AT A RED LIGHT. VEHICLE #1'S BRAKES FAILED AR OF VEHICLE #2. BOTH VEHICLE MOVED PRIOR TO MY ARRIVAL. on Woodmen Road eastbound. Vehicle 1 collided with the rear of vehicle 2 with its front. Both vehicles were driven

. The front end of Vehicle 1 collided with a barbed wire fence. Vehicle 1 traveled back onto the roadway and drove to a

icle 2 and collided its front with the rear of vehicle 2. Both vehicles were moved prior to investigation.

in the left turn lane at E Woodmen Rd. Vehicle #2 had a solid green light to left onto westbound E Woodmen Rd. did not see that the light was red. Vehicle #1 collided its front with the left side of Vehicle #2. Vehicle #2 traveled for ore coming to rest in the intersection facing north.

ion of Golden Sage Road. The traffic light was yellow in both directions. Vehicle 1 made a left turn onto northbound was moved out of the lane of traffic and drove to final rest on the north side of the intersection. Vehicle 2 traveled west

ad in the right lane approaching Golden Sage Road. Vehicle #1 turned right into the right lane and proceeded west on crash. Vehicle #2 also moved left to avoid a crash. Vehicle #2's front struck Vehicle #1's rear while both vehicles were

lane ahead of Vehicle 1. Vehicle 2 stopped for traffic. Vehicle 1 slowed and struck Vehicle 2 in the rear with its front

#1 rotated counterclockwise 1/2 time and traveled back on the roadway for 55.0 feet before traveling off the left side rest on its roof, 53.0 feet from the roadway edge.

in front of vehicle #1; vehicle #1 struck the antelope with the front of the vehicle. Vehicle #1 merged to the left

' west of Golden Sage Rd. Vehicles were moved prior to investigation.

vay. Driver 1 attempted to correct, vehicle 1 returned to the roadway and yawed off the right side of the roadway. of the roadway on its wheels facing north.

truck the animal. Vehicle 1 overturned approximately 2 times and came to rest in the left lane. Vehicle 1 was moved

hicle 1 was eastbound in the right lane on Woodman Road approaching the intersection with Golden Sage Road. intersection on all four wheels facing east. After impact Vehicle 1 travelled an additional 140 feet on the roadway ainst the light pole, partially on the south side of the road facing southeast. e of traffic, directly behind Vehicle 2. Driver of Vehicle 1 fell asleep and collided with the rear of Vehicle 2. After impact

olliding with a fence. Vehicle 1 came to final rest against the fence on all four wheels facing southeast. Final rest of

c lane. Vehicle 3 was westbound on Woodmen Road, in the left traffic lane behind Vehicle 2. Vehicle 1 continued lowed. Vehicle 3 slowed and struck Vehicle 2, in the rear, with its front. All three vehicles were moved from the scene

lmen Road. Vehicle #1 collided its front with the left side of Vehicle #2. Vehicle #1 came to rest facing north. Vehicle #:

Woodmen Road at Golden Sage Road in the right lane. Vehicle #1 attempted to stop leaving 88 feet of braking tire al on scene.

D. THE FRONT OF VEHICLE 1 IMPACTED THE REAR OF VEHICLE 2, APPROXIMATELY 20' EAST OF THE INTERSECTION, IN ACT UNKNOWN.

e #1 failed to stop for the red light. Vehicle #2 began to turn left onto Woodmen Rd. Vehicle #1's passenger front

Woodmen Road just west of Golden Sage. From west to east, vehicle 1, vehicle 2, vehicle 3 and vehicle 4. Traffic was g east on the roadway. Vehicle 2 collided with the rear of vehicle 3. Vehicle 2 came to rest on it's wheels facing east on 4 was driven to the left shoulder

Vehicle 1 failed to stop and collided its front with the rear of vehicle 2. Both vehicles were moved prior to investigation

ville Rd in the right lane.

struck by the right front/side of vehicle #2.

Aridian Rd approaching Eastonville Rd. Vehicle #1 was attempting to make a left turn onto N Meridian Rd and failed ided with Vehicle #1 right front. Both vehicle's where driven to rest.

failed to yield the right of way and began to make a left turn onto Meridian Rd. Vehicle #1's front bumper impacted

sign to make a left turn onto southbound Meridian Road, stopping in the center of the intersection after crossing the 1 fled the scene. Vehicle #2 followed Vehicle #1 to Woodmen Road and Golden Sage Road where both vehicles

dian Road on the shoulder of the right turn lane. Bicycle 1 continued straight through the intersection. Vehicle 1 d Bicycle 1 were moved prior to investigation. Irrned in front of oncoming traffic and collided with the front of vehicle #2. Vehicle #1 came to rest, facing northeast,

## **Internal Trip Calculations**



	NCHRP 684 Internal Trip Capture Estimation Tool													
Project Name:	Project Name: Falcon Marketplace Organization: LSC													
Project Location:	El Paso County, CO		Performed By:	KDF										
Scenario Description:	Buildout		Date:	6/2/2017										
Analysis Year:	2040		Checked By:											
Analysis Period:	AM Street Peak Hour	Ì	Date:											

	Table 1-	A: Base Vehicle	-Trip Generation	Est	imates (Single-Use Si	te Estimate)				
Land Use	Developme	ent Data ( <i>For Info</i>	ormation Only)		Estimated Vehicle-Trips <sup>3</sup>					
Lanu Use	ITE LUCs <sup>1</sup> Quantity Units				Total	Entering	Exiting			
Office					34	17	17			
Retail					688	403	285			
Restaurant					294	150	144			
Cinema/Entertainment					0					
Residential					0					
Hotel					0					
All Other Land Uses <sup>2</sup>					0					
					1,016	570	446			

		Table 2-A:	Mode Split and Vehi	cle Oc	cupancy Estimates	3			
Land Use		Entering Tri	ps		Exiting Trips				
Land Use	Veh. Occ.4	% Transit	% Non-Motorized		Veh. Occ. <sup>4</sup>	% Transit	% Non-Motorized		
Office									
Retail									
Restaurant									
Cinema/Entertainment									
Residential									
Hotel									
All Other Land Uses <sup>2</sup>									

	Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)												
Origin (From)	Destination (To)												
Ongin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel							
Office													
Retail													
Restaurant													
Cinema/Entertainment													
Residential													
Hotel													

Table 4-A: Internal Person-Trip Origin-Destination Matrix*											
Origin (From)		Destination (To)									
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		5	11	0	0	0					
Retail	1		37	0	0	0					
Restaurant	2	20		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	0	0	0	0		0					
Hotel	0	0	0	0	0						

Table 5-A:	Computatio	ns Summary	Table 6-A: Internal Trip Capture Percentages by Land Use			
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Trips
All Person-Trips	1,016	570	446	Office	18%	94%
Internal Capture Percentage	15%	13%	17%	Retail	6%	13%
				Restaurant	32%	15%
External Vehicle-Trips <sup>5</sup>	864	494	370	Cinema/Entertainment	N/A	N/A
External Transit-Trips <sup>6</sup>	0	0	0	Residential	N/A	N/A
External Non-Motorized Trips <sup>6</sup>	0	0	0	Hotel	N/A	N/A

<sup>1</sup>Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

<sup>2</sup>Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator. <sup>3</sup>Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

<sup>4</sup>Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

<sup>5</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

<sup>6</sup>Person-Trips

\*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Project Name:	Falcon Marketplace
Analysis Period:	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends									
Land Use	Tab	le 7-A (D): Enter	ing Trips		-	Table 7-A (O): Exiting Trips			
	Veh. Occ.	Vehicle-Trips	Person-Trips*		Veh. Occ.	Vehicle-Trips	Person-Trips*		
Office	1.00	17	17		1.00	17	17		
Retail	1.00	403	403		1.00	285	285		
Restaurant	1.00	150	150		1.00	144	144		
Cinema/Entertainment	1.00	0	0		1.00	0	0		
Residential	1.00	0	0		1.00	0	0		
Hotel	1.00	0	0		1.00	0	0		

	Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)										
Origin (From)		Destination (To)									
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		5	11	0	0	0					
Retail	83		37	0	40	0					
Restaurant	45	20		0	6	4					
Cinema/Entertainment	0	0	0		0	0					
Residential	0	0	0	0		0					
Hotel	0	0	0	0	0						

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)											
Origin (From)		Destination (To)									
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		129	35	0	0	0					
Retail	1		75	0	0	0					
Restaurant	2	32		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	1	69	30	0		0					
Hotel	1	16	9	0	0						

	Table 9-A (D): Internal and External Trips Summary (Entering Trips)								
Destination Land Llas		Person-Trip Esti	mates		External Trips by Mode*				
Destination Land Use	Internal	External	Total		Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>		
Office	3	14	17		14	0	0		
Retail	25	378	403		378	0	0		
Restaurant	48	102	150		102	0	0		
Cinema/Entertainment	0	0	0		0	0	0		
Residential	0	0	0		0	0	0		
Hotel	0	0	0		0	0	0		
All Other Land Uses <sup>3</sup>	0	0	0		0	0	0		

	Table 9-A (O): Internal and External Trips Summary (Exiting Trips)								
		Person-Trip Esti	mates		External Trips by Mode*				
Origin Land Use	Internal	External	Total		Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>		
Office	16	1	17		1	0	0		
Retail	38	247	285		247	0	0		
Restaurant	22	122	144		122	0	0		
Cinema/Entertainment	0	0	0		0	0	0		
Residential	0	0	0		0	0	0		
Hotel	0	0	0		0	0	0		
All Other Land Uses <sup>3</sup>	0	0	0		0	0	0		

<sup>1</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A
<sup>2</sup>Person-Trips
<sup>3</sup>Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator
\*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Project Name: Falcon Marketplace Organization: LSC									
Project Location:	El Paso County, CO		Performed By:	KDF						
Scenario Description:	Buildout		Date:	6/2/2017						
Analysis Year:	2040		Checked By:							
Analysis Period:	PM Street Peak Hour		Date:							

	Table 1-	P: Base Vehicle	-Trip Generation	Est	timates (Single-Use Si	te Estimate)			
Land Use	Developme	Development Data (For Information Only)				Estimated Vehicle-Trips <sup>3</sup>			
	ITE LUCs <sup>1</sup>	Quantity	Units		Total	Entering	Exiting		
Office					41	17	24		
Retail					1,304	658	646		
Restaurant				Ī	333	172	161		
Cinema/Entertainment					0				
Residential					0				
Hotel					0				
All Other Land Uses <sup>2</sup>					0				
					1,678	847	831		

	Table 2-P: Mode Split and Vehicle Occupancy Estimates								
Land Use		Entering Tri	ps			Exiting Trips			
Land Use	Veh. Occ.4	% Transit	% Non-Motorized		Veh. Occ.4	% Transit	% Non-Motorized		
Office									
Retail									
Restaurant									
Cinema/Entertainment									
Residential									
Hotel									
All Other Land Uses <sup>2</sup>									

	Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)		Destination (To)								
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		600	200							
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-P: Internal Person-Trip Origin-Destination Matrix*										
Origin (From)		Destination (To)								
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		4	1	0	0	0				
Retail	5		50	0	0	0				
Restaurant	5	66		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	0	0	0	0		0				
Hotel	0	0	0	0	0					

Table 5-P	: Computatio	ns Summary	Table 6-P: Internal Trip Capture Percentages by Land Use			
	Total Entering Exiting		Land Use	Entering Trips	Exiting Trips	
All Person-Trips	1,678	847	831	Office	59%	21%
Internal Capture Percentage	16%	15%	16%	Retail	11%	9%
				Restaurant	30%	44%
External Vehicle-Trips <sup>5</sup>	1,416	716	700	Cinema/Entertainment	N/A	N/A
External Transit-Trips <sup>6</sup>	0	0	0	Residential	N/A	N/A
External Non-Motorized Trips <sup>6</sup>	0	0	0	Hotel	N/A	N/A

<sup>1</sup>Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

<sup>2</sup>Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

<sup>3</sup>Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

<sup>4</sup>Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be <sup>5</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P. <sup>5</sup>Person-Trips

\*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Project Name:	Falcon Marketplace
Analysis Period:	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends									
Land Use	Table	Table 7-P (D): Entering Trips				Table 7-P (O): Exiting Trips			
	Veh. Occ.	Vehicle-Trips	Person-Trips*		Veh. Occ.	Vehicle-Trips	Person-Trips*		
Office	1.00	17	17		1.00	24	24		
Retail	1.00	658	658		1.00	646	646		
Restaurant	1.00	172	172		1.00	161	161		
Cinema/Entertainment	1.00	0	0		1.00	0	0		
Residential	1.00	0	0		1.00	0	0		
Hotel	1.00	0	0		1.00	0	0		

Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)										
Origin (From)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		4	1	0	0	0				
Retail	13		187	26	168	32				
Restaurant	5	66		13	29	11				
Cinema/Entertainment	0	0	0		0	0				
Residential	0	0	0	0		0				
Hotel	0	0	0	0	0					

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)										
Origin (From)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		46	3	0	0	0				
Retail	5		50	0	0	0				
Restaurant	5	329		0	0	0				
Cinema/Entertainment	1	26	5		0	0				
Residential	10	66	24	0		0				
Hotel	0	13	9	0	0					

Table 9-P (D): Internal and External Trips Summary (Entering Trips)								
Destination Land Use	Person-Trip Estimates				External Trips by Mode*			
	Internal	External	Total		Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>	
Office	10	7	17		7	0	0	
Retail	70	588	658		588	0	0	
Restaurant	51	121	172		121	0	0	
Cinema/Entertainment	0	0	0		0	0	0	
Residential	0	0	0		0	0	0	
Hotel	0	0	0		0	0	0	
All Other Land Uses <sup>3</sup>	0	0	0		0	0	0	

Table 9-P (O): Internal and External Trips Summary (Exiting Trips)								
Origin Land Use	Person-Trip Estimates				External Trips by Mode*			
	Internal	External	Total		Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>	
Office	5	19	24		19	0	0	
Retail	55	591	646		591	0	0	
Restaurant	71	90	161		90	0	0	
Cinema/Entertainment	0	0	0		0	0	0	
Residential	0	0	0		0	0	0	
Hotel	0	0	0		0	0	0	
All Other Land Uses <sup>3</sup>	0	0	0		0	0	0	

<sup>1</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

<sup>2</sup>Person-Trips

<sup>3</sup>Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator \*Indicates computation that has been rounded to the nearest whole number.



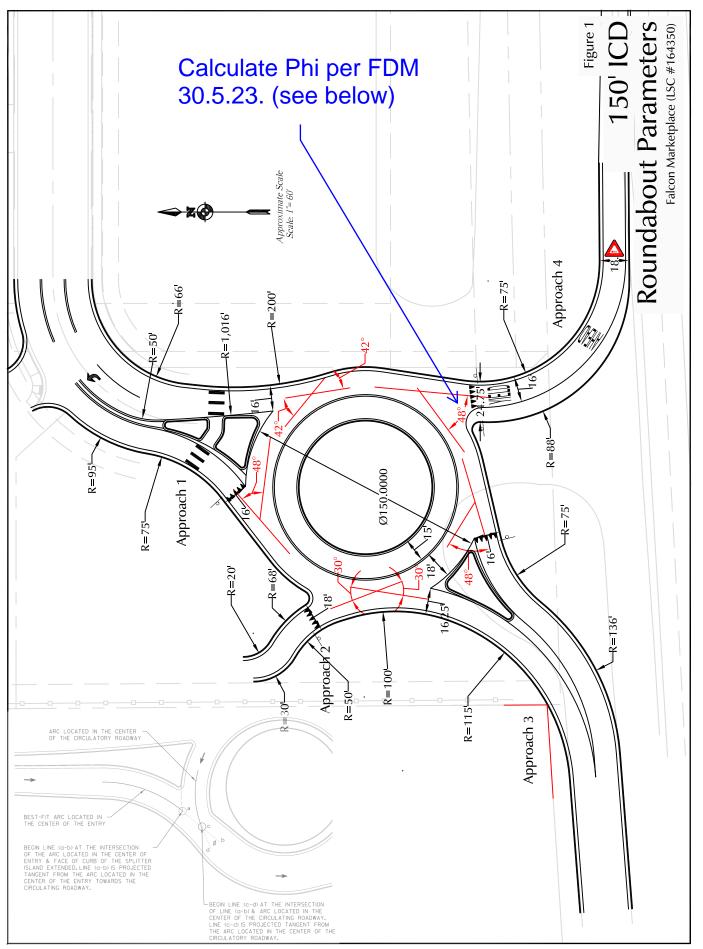
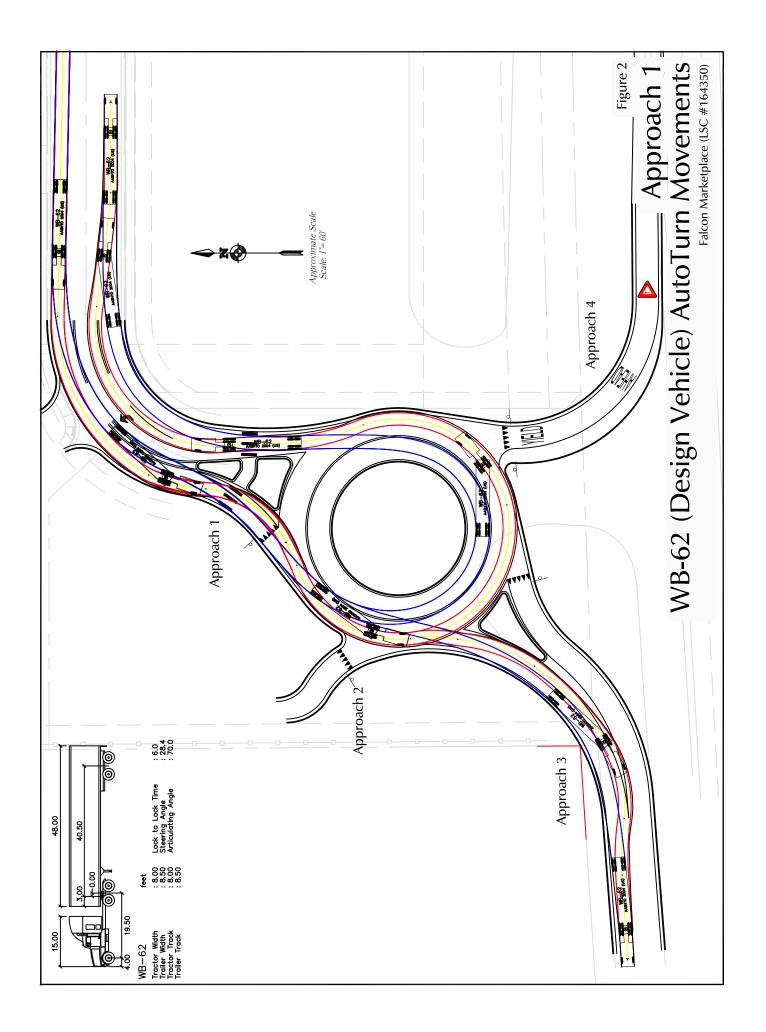
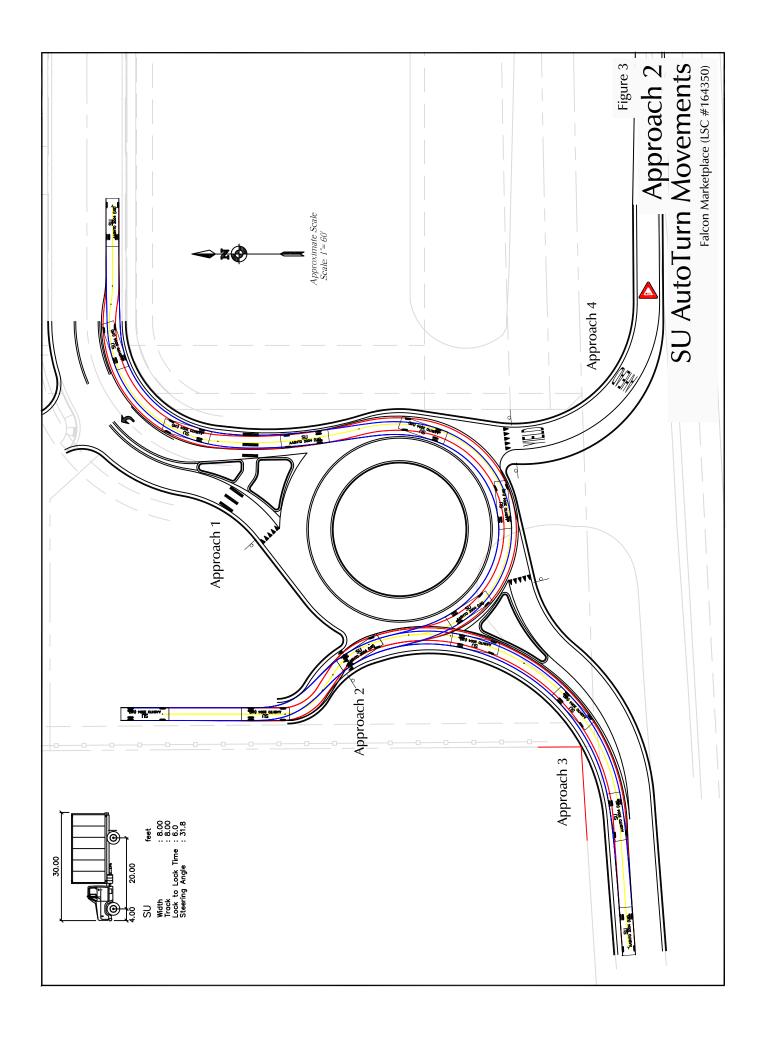
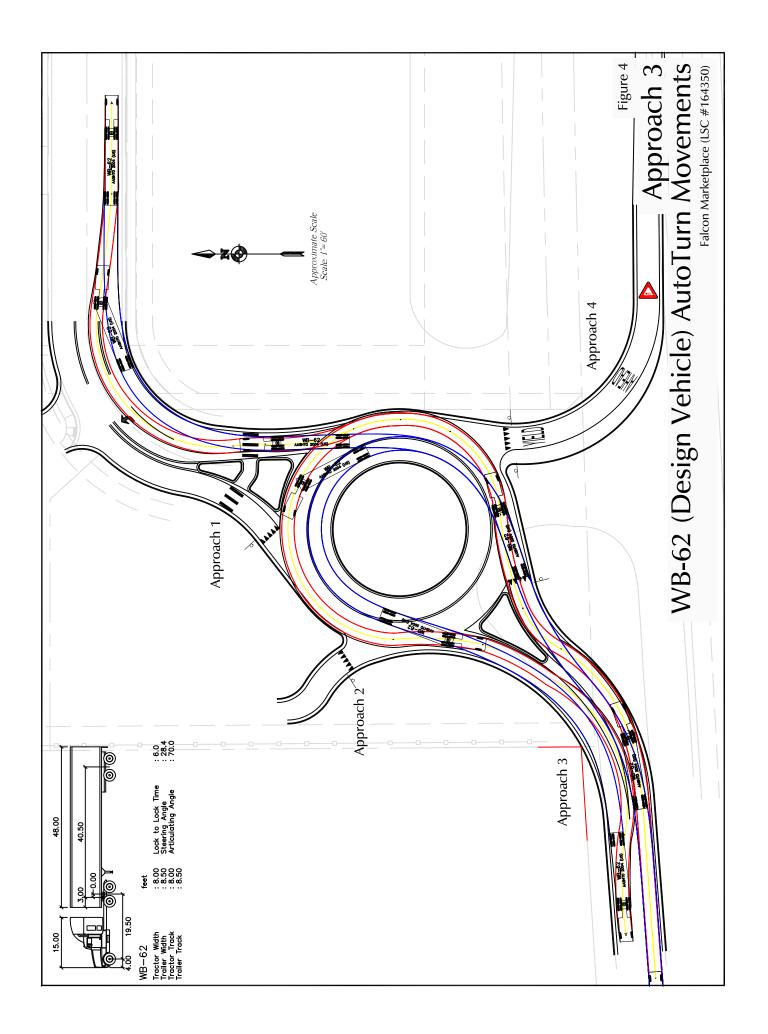
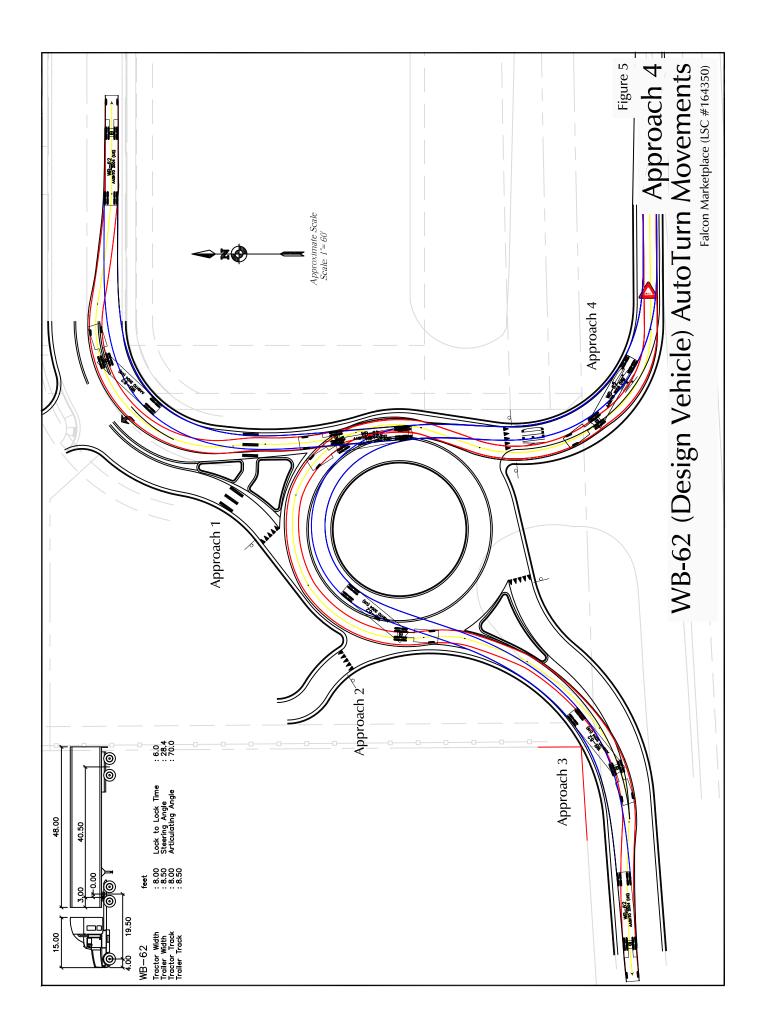


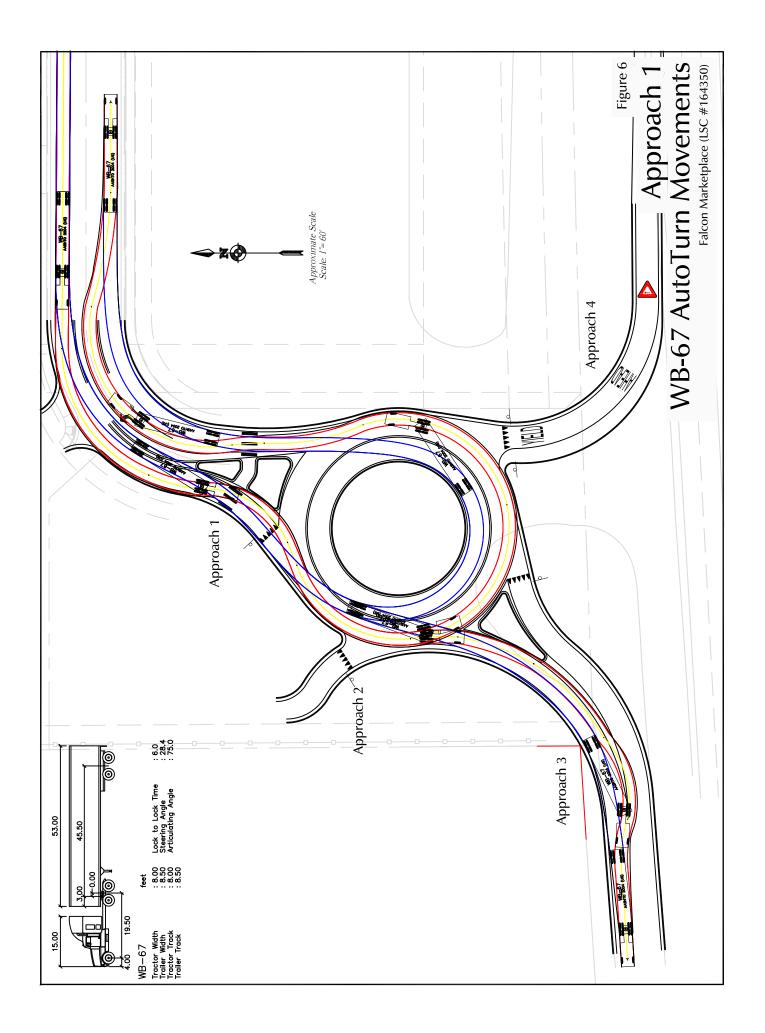
Figure 30.22 Method 2 Phi Measurement

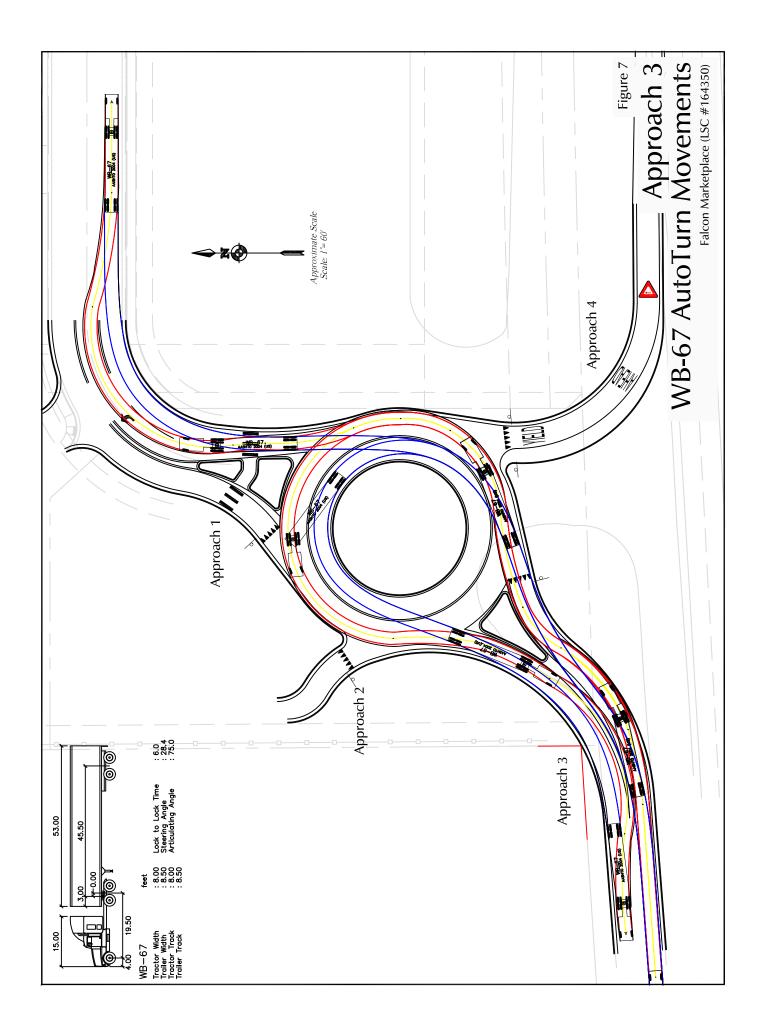


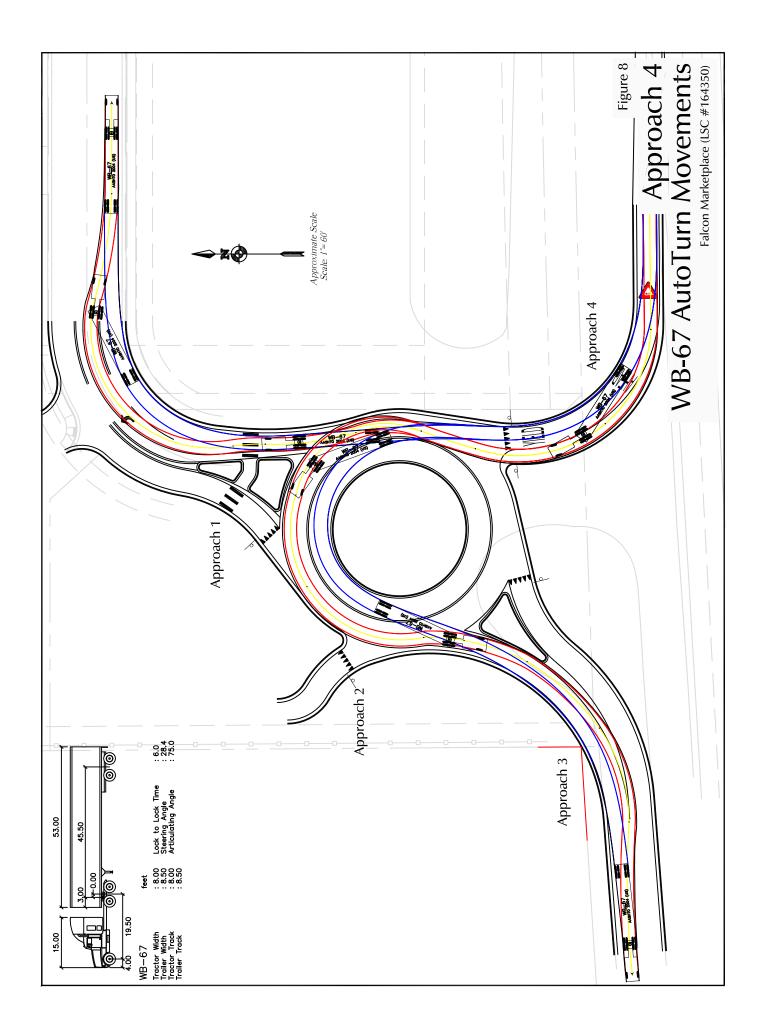


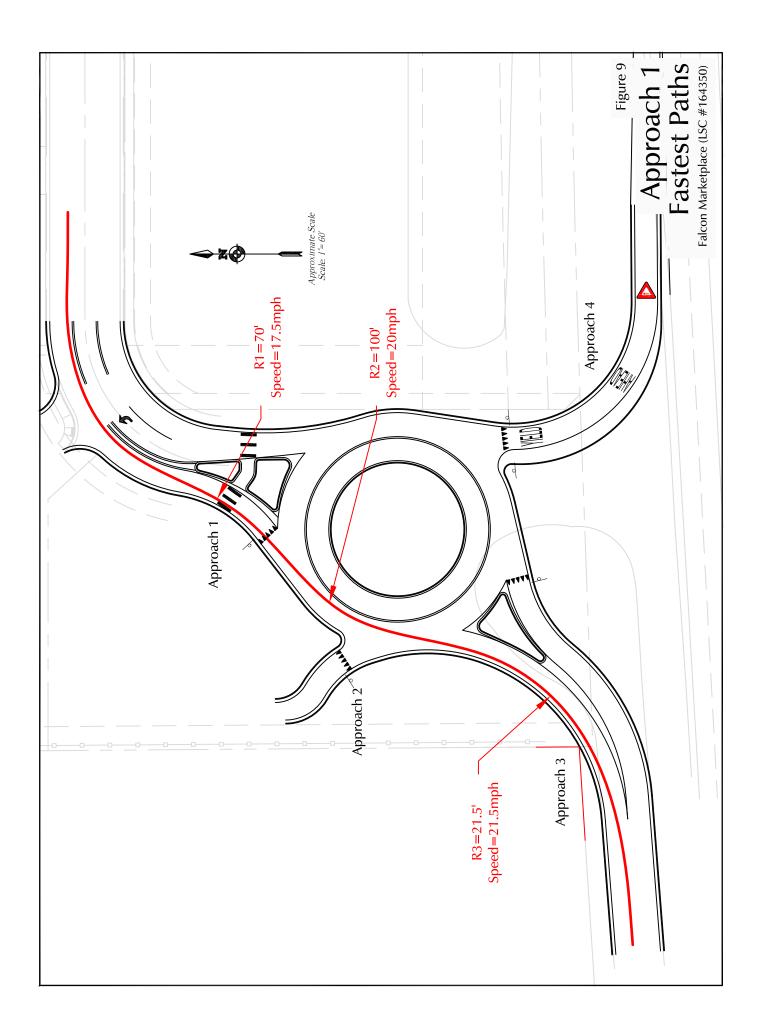


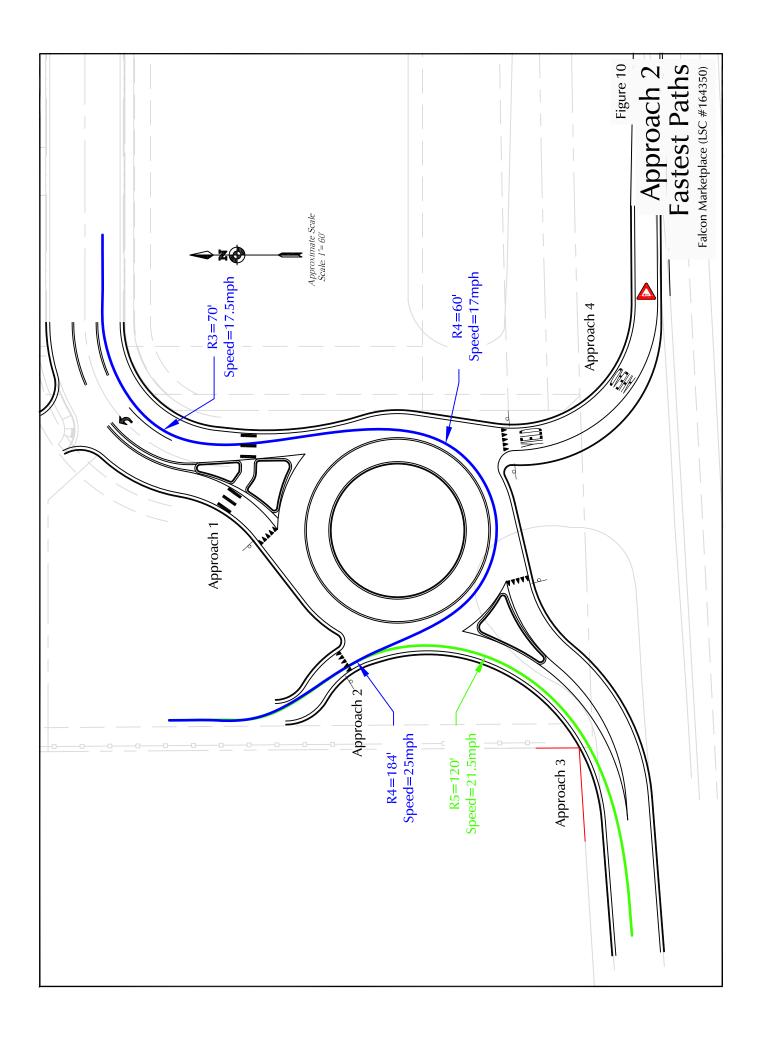


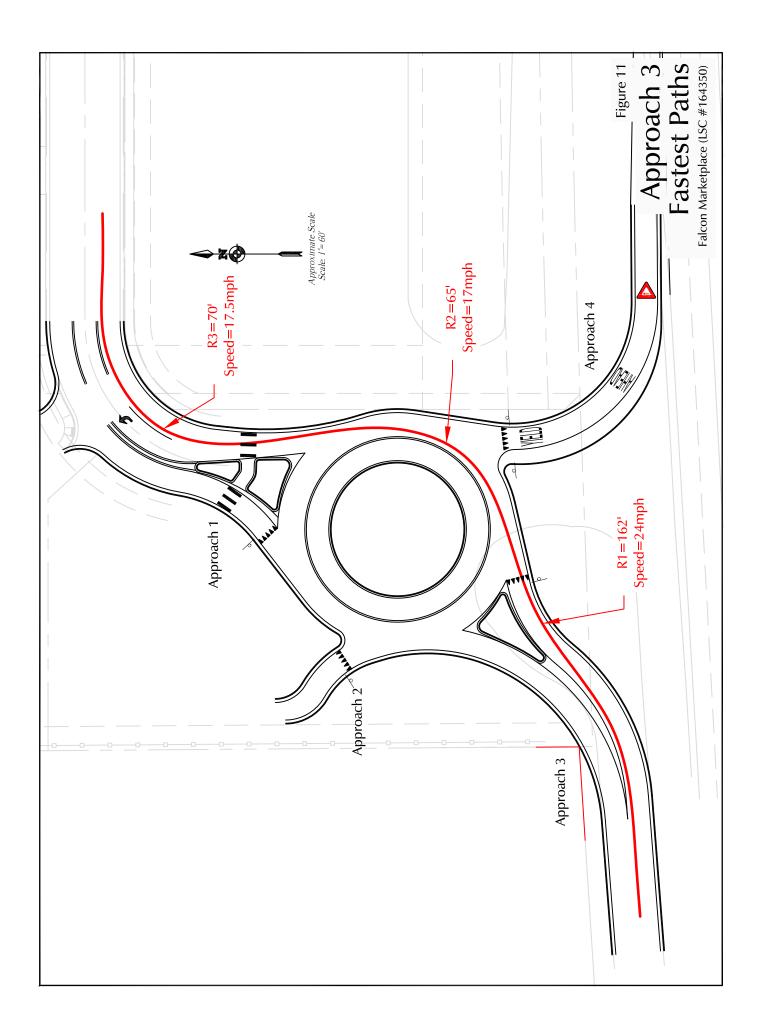


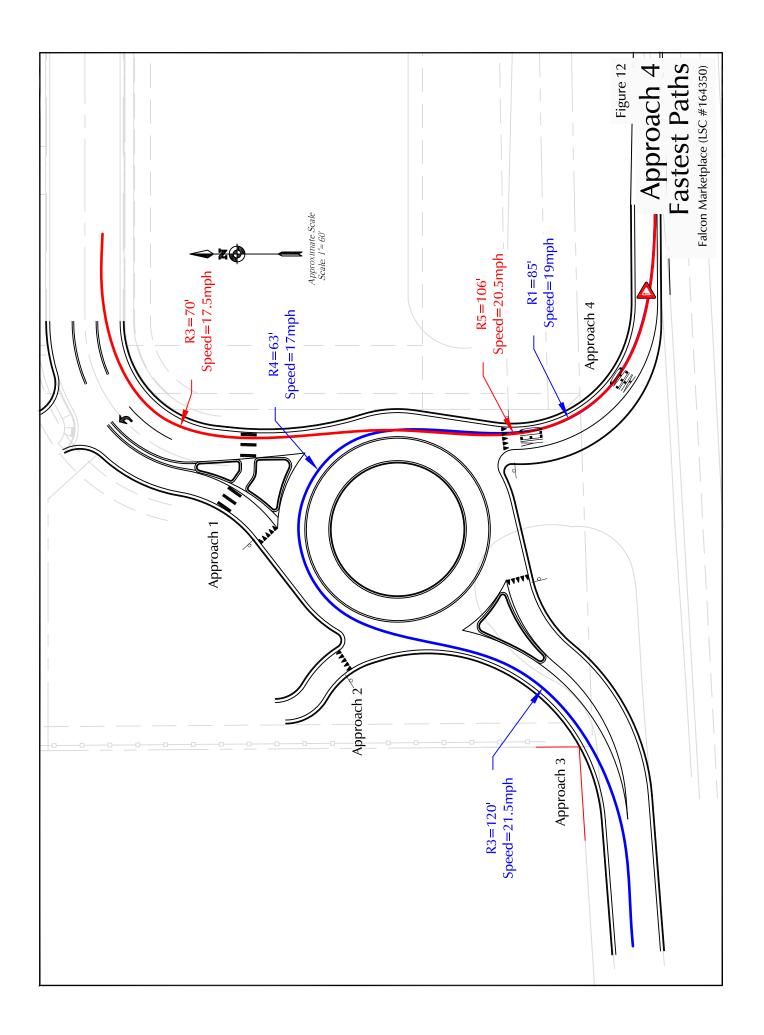












## **Rodel Analysis Reports**



# **Operational Results**

# 2040 AM Peak - 15 minutes

### **Flows and Capacity**

				Fl	ows (veh/l	nr)			Capacity	(veh/hr)	
Leg	Leg Names	Bypass Type	Arriva	al Flow	Opposi	ing Flow	Exit	Сар	acity	Averag	je VCR
		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Entry	Bypass	Entry	Bypass	Flow	Entry	Bypass	Entry	Bypass
1	Approach 1	None	65		216		346	1087		0.0604	
2	Approach 2	None	41		281		0	1281		0.0324	
3	Approach 3	None	164		17		305	1203		0.1374	
4	Approach 4	None	380		181		0	1107		0.3470	

### **Delays, Queues and Level of Service**

Log	Log Nomoo	Bypass	Ave	erage Delay (s	ec)	95% Qu	eue (veh)	L	evel of Servic	e
Leg	Leg Names	Туре	Entry	Bypass	Leg	Entry	Bypass	Entry	Bypass	Leg
1	Approach 1	None	3.63		3.63	0.16		А		А
2	Approach 2	None	2.91		2.91	0.09		А		A
3	Approach 3	None	3.95		3.95	0.41		А		A
4	Approach 4	None	5.56		5.56	1.31		А		А

# **Global Results**

## **Performance and Accidents**

### 2040 AM Peak Global Performance

Parameter	Units	Entries	Bypasses	Total
Arrive Flows	veh/hr	599		599
Capacity	veh/hr	4712		4712
Average Delay	sec/veh	5.08		5.08
L.O.S. (Signal)	A – F	А		А
L.O.S. (Unsig)	A – F	A		А
Total Delay	veh.hrs	0.85		0.85

# **Operational Results**

# 2040 PM Peak - 15 minutes

### **Flows and Capacity**

				Fl	ows (veh/l	nr)			Capacity	(veh/hr)	
Leg	Leg Names	Bypass Type	Arriva	al Flow	Oppos	ing Flow	Exit	Сар	acity	Averag	je VCR
		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Entry	Bypass	Entry	Bypass	Flow	Entry	Bypass	Entry	Bypass
1	Approach 1	None	71		171		473	1113		0.0639	
2	Approach 2	None	10		241		0	1307		0.0075	
3	Approach 3	None	273		5		246	1210		0.2272	
4	Approach 4	None	365		278		0	1050		0.3513	

### **Delays, Queues and Level of Service**

Log	Log Nomoo	Bypass	Ave	erage Delay (s	ec)	95% Qu	eue (veh)	L	evel of Servic	e
Leg	Leg Names	Туре	Entry	Bypass	Leg	Entry	Bypass	Entry	Bypass	Leg
1	Approach 1	None	3.58		3.58	0.17		А		А
2	Approach 2	None	0.04		0.04	0.00		А		А
3	Approach 3	None	4.59		4.59	0.74		А		А
4	Approach 4	None	5.82		5.82	1.34		А		А

# **Global Results**

## **Performance and Accidents**

### 2040 PM Peak Global Performance

Parameter	Units	Entries	Bypasses	Total
Arrive Flows	veh/hr	661		661
Capacity	veh/hr	4714		4714
Average Delay	sec/veh	5.32		5.32
L.O.S. (Signal)	A – F	А		А
L.O.S. (Unsig)	A – F	A		А
Total Delay	veh.hrs	0.98		0.98

# **Traffic Count Reports**



# LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO File Name : Meridian Rd - Eastonville Rd AM (719) 633-2868 Site Code : 00154450 Start Pate : 09/09/2015

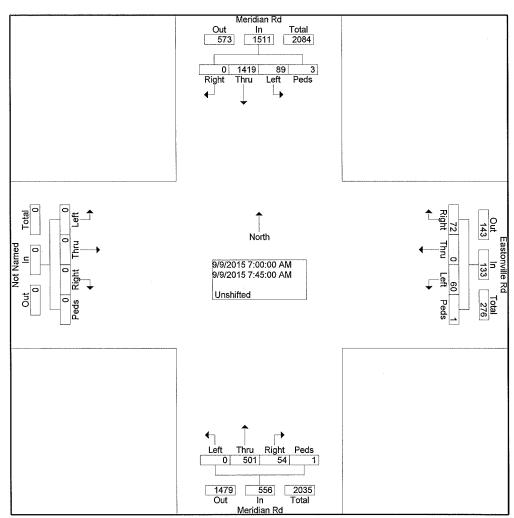
### LSC Transportation Consultants, Inc.

# Start Date : 09/09/2015 Page No : 1

											rage	NO	11				
							Groups	Printed-	Unshifte	ed	-						
		Meridia	an Rd			Eastony	ille Rd			Meridia	an Rd						
		From	North			From	East			From S	South			From	Nest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Тс
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	0	291	22	0	4	0	15	0	3	70	0	0	0	0	0	0	2
06:45 AM	0	289	13	0	8	0	11	0	9	101	0	0	0	0	0	0	2
Total	0	580	35	0	12	0	26	0	12	171	0	0	0	0	0	0	{
07:00 AM	0	385	13	1	13	0	17	0	19	119	0	0	0	0	0	0	;
07:15 AM	0	375	23	1	18	0	13	0	9	118	0	1	0	0	0	0	
07:30 AM	0	387	24	1	27	0	18	1	13	146	0	0	0	0	0	0	(
07:45 AM	0	272	29	0	14	0	12	0	13	118	0	0	0	0	0	0	
Total	0	1419	89	3	72	0	60	1	54	501	0	1	0	0	0	0	2
08:00 AM	0	255	22	2	17	0	12	0	14	112	0	0	0	0	0	0	
08:15 AM	0	278	18	0.	21	0	12	0	10	99	0	0	0	0	0	0	
Grand Total	0	2532	164	5	122	0	110	1	90	883	0	1	0	0	0	0	3
Apprch %	0.0	93.7	6.1	0.2	52.4	0.0	47.2	0.4	9.2	90.7	0.0	0.1	0.0	0.0	0.0	0.0	
Total %	0.0	64.8	4.2	0.1	3.1	0.0	2.8	0.0	2.3	22.6	0.0	0.0	0.0	0.0	0.0	0.0	

File Name: Meridian Rd - Eastonville Rd AMSite Code: 00154450Start Date: 09/09/2015Page No: 2

			eridian om No					stonvill rom Ea					eridian om Sc				F	rom W	est		
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Int. Total
Peak Hour F			M to			harmon and the second			3	TULAI	m	u		3	Total		u		3	TULAT	TULAI
Intersecti on	07:00	) AM																			
Volume	0	141 9	89	3	1511	72	0	60	1	133	54	501	0	1	556	0	0	0	0	0	2200
Percent	0.0	93. 9	5.9	0.2		54. 1	0.0	45. 1	0.8		9.7	90. 1	0.0	0.2		0.0	0.0	0.0	0.0		
07:30 Volume Peak Factor	0	387	24	1	412	27	0	18	1	46	13	146	0	0	159	0	0	0	0	0	617 0.891
High Int. Volume Peak Factor	07:30 0		24	1	412 0.91 7	07:30 27	0 AM 0	18	1	46 0.72 3	07:30 13	) AM 146	0	0	159 0.87 4	6:15:	00 AN	/1			

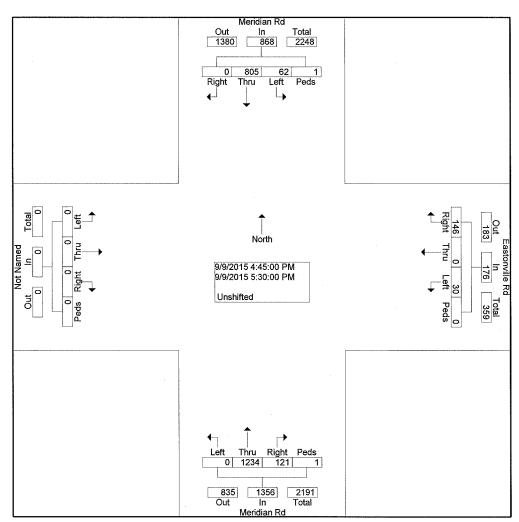


File Name : Meridian Rd - Eastonville Rd PM Site Code : 00154340 Start Date : 09/09/2015

											Page	No	: 1				
						(	Groups	Printed	- Unshif	ted							
		Meridi	an Rd			Easton	I			Meridia	an Rd						
		From	North			From	East			From S		1		From	West		
Oto at Time a	Dista	<b>T</b> I	1 - 4	Deale	D:14	These	1.4	Deale	D:	There	1.4			<b>T</b> 1	1 - 6	Bul	Int.
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:00 PM	0	182	27	0	43	0	14	0	22	238	0	0	0	0	0	0	526
04:15 PM	0	161	29	1	45	0	10	0	35	243	0	1	0	0	0	0	525
04:30 PM	0	150	14	0	40	0	5	0	21	270	0	0	0	0	0	0	500
04:45 PM	0	198	19	0	38	0	3	0	21	300	0	1	0	0	0	0	580
Total	0	691	89	1	166	0	32	0	99	1051	0	2	0	0	0	0	2131
05:00 PM	0	199	10	0	38	0	6	0	23	313	0	0	0	0	0	0	589
05:15 PM	Ō	207	20	Ō	39	Ō	10	Ō	35	290	Ō	Ō	Ō	Ō	Ō	Ō	601
05:30 PM	0	201	13	1	31	0	11	0	42	331	0	0	0	0	0	0	630
05:45 PM	0	183	15	0	30	0	10	0	39	301	0	1	0	0	0	0	579
Total	0	790	58	1	138	0	37	0	139	1235	0	1	0	0	0	0	2399
Grand Total	0	1481	147	2	304	0	69	0	238	2286	0	3	0	0	0	0	4530
Apprch %	0.0	90.9	9.0	0.1	81.5	0.0	18.5	0.0	9.4	90.5	0.0	0.1	0.0	0.0	0.0	0.0	
Total %	0.0	32.7	3.2	0.0	6.7	0.0	1.5	0.0	5.3	50.5	0.0	0.1	0.0	0.0	0.0	0.0	

File Name: Meridian Rd - Eastonville Rd PMSite Code: 00154340Start Date: 09/09/2015Page No: 2

			eridian om No					tonvill rom E					eridiar om So				Fi	om W	lest		
Start	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Int.
Time	ht	u	LOIL	S	Total	ht	u	LOIL	S	Total	ht	u	Lon	S	Total	ht	u	Len	S	Total	Total
Peak Hour F	From 0	)4:00 F	PM to (	05:45 I	PM - Pe	eak 1 c	of 1														
Intersecti on	04:45	5 PM																			
Volume	0	805	62	1	868	146	0	30	0	176	121	123 4	0	1	1356	0	0	0	0	0	2400
Percent	0.0	92. 7	7.1	0.1		83. 0	0.0	17. 0	0.0		8.9	91. 0	0.0	0.1		0.0	0.0	0.0	0.0		
05:30 Volume Peak Factor	0	201	13	1	215	31	0	11	0	42	42	331	0	0	373	0	0	0	0	0	630 0.952
High Int. Volume Peak Factor	05:15 0		20	0	227 0.95 6	05:15 39	PM 0	10	0	49 0.89 8	05:30 42	) PM 331	0	0	373 0.90 9	3:45:	00 PN	1			



File Name : Meridian Rd - Woodman Rd AM Site Code : 00154450 Start Date : 09/16/2015 Page No : 1

### Groups Printed- Unshifted Meridian Rd Woodmen Rd Meridian Rd Woodmen Rd From North From East From South From West Int. Peds Thru Left Peds Thru Left Peds Thru Left Peds Right Thru Left Start Time Right Right Right Total 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 Factor 06:30 AM 06:45 AM Total 07:00 AM 07:15 AM 07:30 AM 07:45 AM Total MA 00:80 08:15 AM Grand Total Apprch % 56.2 27.4 16.5 0.0 15.6 79.8 4.6 0.0 4.9 55.5 39.6 0.0 5.1 53.7 41.1 0.1 Total % 25.2 1.2 0.0 0.3 2.8 0.0 12.2 9.3 0.0 12.3 7.4 0.0 4.0 20.2 3.9 1.1

File Name: Meridian Rd - Woodman Rd AMSite Code: 00154450Start Date: 09/16/2015Page No: 2

	// ,, # , <sub>-</sub> /,		eridian rom No					odme rom E					eridiar om So					odme rom W			
Start Time	Rig ht	Thr u	Left	Peds	App. Total	Rig ht	Thr u	Left	Ped	App. Total	Rig ht	Thr u	Left	Ped	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Int. Total
Peak Hour F	rom 0	6:30 /	AM to		AM - Pe					10101		u		0	10101		ų	1		Total	Totai
on Volume	06:45 807	5 AM 371	238	0	1416	111	664	32	ο	807	8	106	79	0	193	30	355	271	0	656	3072
Percent	57. 0	26. 2	16. 8	0.0	1410	13. 8	82. 3	4.0	0.0	007	4.1	54. 9	40. 9	0.0	190	4.6	54. 1	41. 3	0.0	000	JU72
07:30 Volume	249	2 84	56	0	389	0 34	3 187	7	0	228	1	9 24	9 22	0	47	9	ı 98	90	0	197	861
Peak																				I	0.892
High Int. Volume Peak Factor			69	0	390 0.90 8	07:30 34	) AM 187	7	0	228 0.88 5	07:00 4	) AM 29	22	0	55 0.87 7	07:30 9	0 AM 98	90	0	197 0.83 2	
		Г							Out	Meridia In		otal							]		
									488			1904									
								[	807 Right	371 Thru	238 Left	0 Peds									
									€Ĵ	Ļ	4										
			90	Ξ.						•						<b>▲</b> 70		ll			
			Total 2206	271	Ĺ					Nori	h					Right	111	Out v			
		Mondman Dd	656	355 Thru					9/16/	2015 6:48						<b>←</b> <u>7</u>	664	Noodmu 80			
		Moode		30 20					9/16/	2015 7:30						↓ feft	32	50 R	)		
			Out 1550	0,00	200				Unst	nifted						Peds		d Total 1408			
				- نا													0				
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									79	106	8	0									
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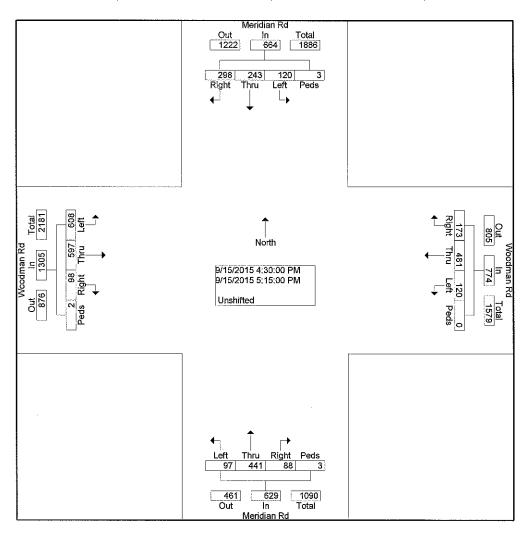
LSC Transportation Consultants, Inc. Colorado Sprin

### File Name : Meridian Rd - Woodman Rd PM Site Code : 00145450 Start Date : 09/15/2015 Page No : 1

						(	Groups	Printed	- Unshift	ted	-						
		Meridia				Woodm				Meridia				Woodn			
		From	North			From	East			From	South			From	VVest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:00 PM	74	48	29	0	24	111	40	0	10	92	19	1	23	130	149	0	750
04:15 PM	67	53	31	0	36	110	24	0	22	96	19	2	17	145	136	0	758
04:30 PM	84	63	27	3	43	141	29	0	20	110	19	2	18	143	131	0	833
04:45 PM	59	56	42	0	57	120	34	0	17	103	21	0	28	165	152	1	855
Total	284	220	129	3	160	482	127	0	69	401	78	5	86	583	568	1	3196
05:00 PM	72	71	26	0	38	125	27	0	21	113	25	0	23	130	162	0	833
05:15 PM	83	53	25	0	35	95	30	0	30	115	32	1	29	159	163	1	851
05:30 PM	81	69	26	0	44	116	30	0	21	106	21	3	24	145	131	0	817
05:45 PM	63	51	21	0	56	83	31	0	33	88	18	2	32	133	162	1	774
Total	299	244	98	0	173	419	118	0	105	422	96	6	108	567	618	2	3275
Grand Total	583	464	227	3	333	901	245	0	174	823	174	11	194	1150	1186	3	6471
Apprch %	45.7	36.3	17.8	0.2	22.5	60.9	16.6	0.0	14.7	69.6	14.7	0.9	7.7	45.4	46.8	0.1	
Total %	9.0	7.2	3.5	0.0	5.1	13.9	3.8	0.0	2.7	12.7	2.7	0.2	3.0	17.8	18.3	0.0	

File Name: Meridian Rd - Woodman Rd PMSite Code: 00145450Start Date: 09/15/2015Page No: 2

		Fr	eridian om No					odma rom E					eridiar om Sc					odma rom W			
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Int. Total
Peak Hour F	From 0	4:00 I	PM to	05:45	PM - Pe	eak 1 d	of 1							•							
Intersecti on	04:30	PM																			
Volume	298	243	120	3	664	173	481	120	0	774	88	441	97	3	629	98	597	608	2	1305	3372
Percent	44. 9	36. 6	18. 1	0.5		22. 4	62. 1	15. 5	0.0		14. 0	70. 1	15. 4	0.5		7.5	45. 7	46. 6	0.2		
04:45 Volume Peak	59	56	42	0	157	57	120	34	0	211	17	103	21	0	141	28	165	152	1	346	855 0.986
Factor High Int. Volume Peak	04:30 84	) PM 63	27	3	177 0.93	04:30 43	) PM 141	29	0	213 0.90	05:15 30	5 PM 115	32	1	178 0.88	05:18 29	5 PM 159	163	1	352 0.92	
Factor					8					8					3					7	



### LSC Transportation Consultants, Inc.

545 E. Pikes Peak Ave., #210

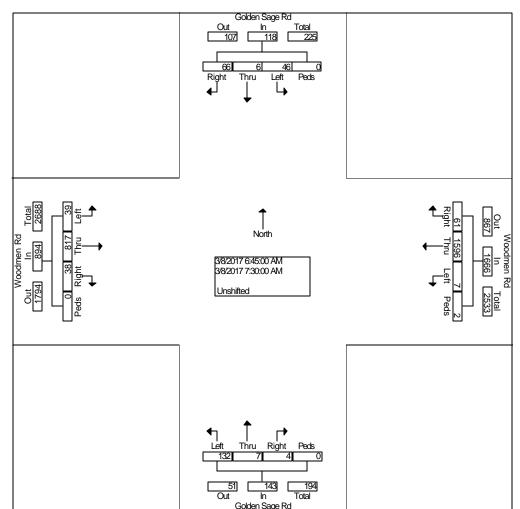
LSC Transportation Consultants, Inc. Colorado Springs, COF9998 : Golden Sage Rd - Woodmen Rd AM (719) 633-2868<sup>Site Code</sup> : 00164350 Start Date : 03/08/2017

Page No : 1

						G	Groups F	Printed-	Unshift	ted							
	Ċ	Golden S	Sage Ro	ł		Woodm	en Rd		G	olden S	age Rd		V	Voodme	en Rd		
		From I	North			From	East			From S	outh			From V	Vest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	10	0	11	0	32	368	0	0	1	4	18	0	3	151	27	0	625
06:45 AM	15	0	11	0	28	307	0	0	0	1	31	0	10	186	16	0	605
Total	25	0	22	0	60	675	0	0	1	5	49	0	13	337	43	0	1230
07:00 AM	13	2	11	0	6	459	1	0	2	3	34	0	10	212	10	0	763
07:15 AM	17	3	13	0	15	434	3	1	2	2	38	0	10	211	5	0	754
07:30 AM	21	1	11	0	12	396	3	1	0	1	29	0	8	208	8	0	699
07:45 AM	12	2	3	0	4	289	3	0	2	0	27	0	17	166	14	0	539
Total	63	8	38	0	37	1578	10	2	6	6	128	0	45	797	37	0	2755
08:00 AM	8	1	2	0	6	256	1	0	1	1	15	0	10	154	11	0	466
08:15 AM	9	0	8	0	16	326	3	0	0	0	17	0	2	153	18	0	552
Grand Total	105	9	70	0	119	2835	14	2	8	12	209	0	70	1441	109	0	5003
Apprch %	57.1	4.9	38.0	0.0	4.0	95.5	0.5	0.1	3.5	5.2	91.3	0.0	4.3	89.0	6.7	0.0	
Total %	2.1	0.2	1.4	0.0	2.4	56.7	0.3	0.0	0.2	0.2	4.2	0.0	1.4	28.8	2.2	0.0	

### LSC Transportation Consultants, Inc. 545 E. Pikes Peak Ave., #210 Colorado Springs, COF809903re : Golden Sage Rd - Woodmen Rd AM (719) 633-2868<sup>Site Code</sup> : 00164350 Start Date : 03/08/2017 Page No : 2

Golden Sage Rd Woodmen Rd Golden Sage Rd Woodmen Rd From North From East From South From West Pe App. Start Rig Thr Lef Rig Thr Lef Pe App. Rig Thr Lef Pe App. Rig Thr Lef Pe App. Int. ds Total ht ds Total ht ds ds Total Time ht u t u t u t Total ht u t Total Peak Hour From 06:30 AM to 08:15 AM - Peak 1 of 1 Intersecti 06:45 AM on 15 13 81 7 Volume 66 6 46 0 118 61 2 1666 4 0 143 38 39 0 894 2821 96 2 7 39. 95. 92. 91. 55. 3.7 Percent 5.1 0.0 0.4 0.1 2.8 4.9 0.0 4.3 4.4 0.0 3 9 0 8 4 07:00 45 21 13 2 11 0 26 6 1 0 466 2 3 34 0 39 10 10 0 232 763 Volume 9 2 0.924 Peak Factor 07:00 AM 07:15 AM 07:00 AM High Int. 07:15 AM 45 21 6 10 33 2 2 232 Volume 17 3 13 0 1 0 466 38 0 42 10 0 9 2 0.89 0.89 0.85 0.96 Peak 4 3 Factor 4 1



### LSC Transportation Consultants, Inc.

545 E. Pikes Peak Ave., #210

LSC Transportation Consultants, Inc. Colorado Springs, COF99903ne : Golden Sage Rd - Woodmen Rd PM (719) 633-2868<sup>Site Code</sup> : 00164350 Start Date : 03/07/2017

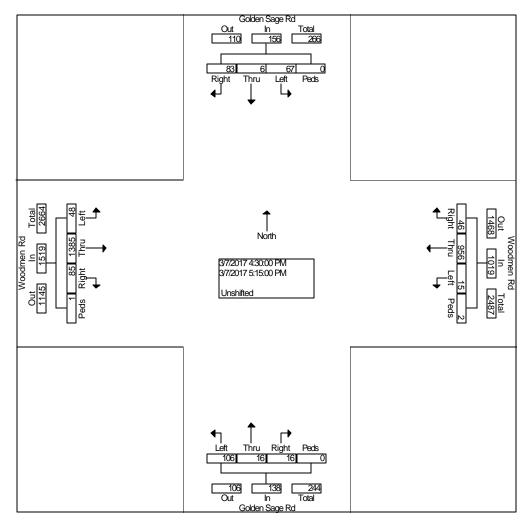
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										raye	NO						
						G	Groups I	Printed-	Unshift	ted							
	G	Golden S	Sage Ro	ł		Woodm	en Rd		G	olden Sa	age Rd		N	Noodme	en Rd		
		From	-			From	East			From S	-			From V	Vest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:00 PM	16	3	7	0	9	243	5	0	2	2	21	0	14	319	25	0	666
04:15 PM	19	4	18	0	10	234	1	1	3	2	16	0	28	324	15	0	675
04:30 PM	14	0	16	0	10	249	1	0	3	4	15	0	18	340	16	0	686
04:45 PM	21	1	15	0	16	233	2	1	3	4	27	0	20	313	10	0	666
Total	70	8	56	0	45	959	9	2	11	12	79	0	80	1296	66	0	2693
05:00 PM	17	3	15	0	14	264	2	0	2	3	24	0	20	355	13	0	732
05:15 PM	31	2	21	0	6	210	10	1	8	5	40	0	27	377	9	1	748
05:30 PM	30	2	40	0	7	178	2	1	5	3	14	0	17	381	6	0	686
05:45 PM	9	3	13	0	6	172	1	0	1	2	14	0	16	294	7	0	538
Total	87	10	89	0	33	824	15	2	16	13	92	0	80	1407	35	1	2704
Crand Total	157	18	145	0	78	1783	24	4	27	25	171	0	160	2703	101	1	5397
Grand Total	-	-	-	0	-					-		-					5397
Apprch %	49.1	5.6	45.3	0.0	4.1	94.4	1.3	0.2	12.1	11.2	76.7	0.0	5.4	91.2	3.4	0.0	
Total %	2.9	0.3	2.7	0.0	1.4	33.0	0.4	0.1	0.5	0.5	3.2	0.0	3.0	50.1	1.9	0.0	I

### LSC Transportation Consultants, Inc. 545 E. Pikes Peak Ave., #210 Colorado Springs, COBOOOR : Golden Sage Rd - Woodmen Rd PM (719) 633-2868<sup>Site</sup> Code : 00164350 Start Date : 03/07/2017

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				ige Rd	l			oodme					den S		d			oodm		I	
		Fr	om N	orth			F	rom E	ast			F	rom S					rom	West		
Start	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour I	From (	04:00	PM to	05:45	PM - F	Peak 1	of 1														
Intersecti on	04:30	D PM																			
Volume	83	6	67	0	156	46	95 6	15	2	1019	16	16	10 6	0	138	85	13 85	48	1	1519	2832
Percent	53. 2	3.8	42. 9	0.0		4.5	93. 8	1.5	0.2		11. 6	11. 6	76. 8	0.0		5.6	91. 2	3.2	0.1		
05:15 Volume	31	2	21	0	54	6	21 0	10	1	227	8	5	40	0	53	27	37 7	9	1	414	748
Peak Factor																					0.947
High Int.	05:15	5 PM				05:0	0 PM				05:1	5 PM				05:1	15 PM				.
Volume	31	2	21	0	54	14	26 4	2	0	280	8	5	40	0	53	27	37 7	9	1	414	
Peak					0.72					0.91					0.65					0.91	
Factor					2					0					1					7	

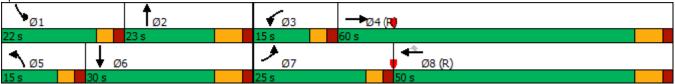




## Timings 1: Meridian & Woodmen

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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	NBR	SBL	• SBT	SBR
Lane Configurations	ካካ	<b>†</b> †	1	ኘኘ	<b>†</b> †	1	ኘኘ	<b>†</b> †	1	ኘኘ	<b>†</b> †	7
Traffic Volume (vph)	350	437	40	32	740	140	100	135	50	240	380	825
Future Volume (vph)	350	437	40	32	740	140	100	135	50	240	380	825
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			Free			8			Free			Free
Detector Phase	7	4		3	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Minimum Split (s)	9.0	21.0		9.0	21.0	21.0	9.0	21.0		9.0	21.0	
Total Split (s)	25.0	60.0		15.0	50.0	50.0	15.0	23.0		22.0	30.0	
Total Split (%)	20.8%	50.0%		12.5%	41.7%	41.7%	12.5%	19.2%		18.3%	25.0%	
Yellow Time (s)	3.0	5.0		3.0	5.0	5.0	3.0	5.0		3.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	6.0		4.0	6.0	6.0	4.0	6.0		4.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	Max	C-Max		Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	21.0	54.0	120.0	11.0	44.0	44.0	11.0	17.0	120.0	18.0	24.0	120.0
Actuated g/C Ratio	0.18	0.45	1.00	0.09	0.37	0.37	0.09	0.14	1.00	0.15	0.20	1.00
v/c Ratio	0.70	0.33	0.03	0.11	0.65	0.22	0.32	0.27	0.03	0.51	0.59	0.57
Control Delay	50.6	24.8	0.0	51.1	34.4	1.6	54.0	47.6	0.0	50.9	47.5	1.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.6	24.8	0.0	51.1	34.4	1.6	54.0	47.6	0.0	50.9	47.5	1.5
LOS	D	С	А	D	С	А	D	D	А	D	D	A
Approach Delay		34.6			30.0			41.5			21.8	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120	)											
Offset: 0 (0%), Referenced	to phase 4	EBT and	8:WBT, S	Start of G	reen, Mas	ter Inters	ection					
Natural Cycle: 65												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 2						n LOS: C						
Intersection Capacity Utiliza	ation 60.9%			10	CU Level	of Service	θB					
Analysis Period (min) 15												

Splits and Phases: 1: Meridian & Woodmen



### Intersection

Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	- 11	1	٦	- 11
Traffic Vol, veh/h	60	72	570	54	89	1375
Future Vol, veh/h	60	72	570	54	89	1375
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	100	0	-	400	385	-
Veh in Median Storage	, # 2	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	73	73	87	87	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	82	99	655	62	97	1495

Major/Minor	Minor1	N	lajor1	N	lajor2	
Conflicting Flow All	1597	328	0	0	717	0
Stage 1	655	-	-	-	-	-
Stage 2	942	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	97	668	-	-	880	-
Stage 1	479	-	-	-	-	-
Stage 2	340	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuve	r 86	668	-	-	880	-
Mov Cap-2 Maneuve	r 212	-	-	-	-	-
Stage 1	426	-	-	-	-	-
Stage 2	340	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	20.8	0	0.6
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1\	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	212	668	880	-	
HCM Lane V/C Ratio	-	-	0.388	0.148	0.11	-	
HCM Control Delay (s)	-	-	32.3	11.3	9.6	-	
HCM Lane LOS	-	-	D	В	А	-	
HCM 95th %tile Q(veh)	-	-	1.7	0.5	0.4	-	

# Timings 30: Golden Sage & Woodmen

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ľ	<u></u>	1	1	<u></u>	1	1	<b>†</b>	1	ľ	4Î	
Traffic Volume (vph)	39	817	38	7	1596	61	132	7	4	46	6	
Future Volume (vph)	39	817	38	7	1596	61	132	7	4	46	6	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	2	2	2	6	6	6	8	8	8	4	4	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Split (s)	11.0	11.0	11.0	11.0	11.0	11.0	9.5	9.5	9.5	9.5	9.5	
Total Split (s)	85.0	85.0	85.0	85.0	85.0	85.0	35.0	35.0	35.0	35.0	35.0	
Total Split (%)	70.8%	70.8%	70.8%	70.8%	70.8%	70.8%	29.2%	29.2%	29.2%	29.2%	29.2%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	5.5	5.5	5.5	5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	88.5	88.5	88.5	88.5	88.5	88.5	19.0	19.0	19.0	19.0	19.0	
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.74	0.74	0.16	0.16	0.16	0.16	0.16	
v/c Ratio	0.27	0.31	0.03	0.02	0.67	0.06	0.73	0.03	0.02	0.21	0.26	
Control Delay	13.4	6.3	2.0	4.4	12.7	0.8	66.7	38.9	0.0	43.6	29.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	13.4	6.3	2.0	4.4	12.7	0.8	66.7	38.9	0.0	43.6	29.2	
LOS	В	А	А	А	В	А	Е	D	А	D	С	
Approach Delay		6.4			12.3			63.3			34.8	
Approach LOS		А			В			E			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12	0											
Offset: 103 (86%), Referen	nced to phas	e 2:EBTL	and 6:W	BTL, Sta	rt of Gree	n						
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay:	14.2			h	ntersectio	n LOS: B						
Intersection Capacity Utiliz		)		l	CU Level	of Service	эC					
Analysis Period (min) 15												
		0.147										
Splits and Phases: 30: 0	Golden Sage	e & Wood	men									

## Timings 1: Meridian & Woodmen

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	ኘ	<u></u>	1	ኘ	<u></u>	1	ኘኘ	<u></u>	1	ኘኘ	<u></u>	1
Traffic Volume (vph)	675	650	115	120	525	200	110	525	135	150	310	380
Future Volume (vph)	675	650	115	120	525	200	110	525	135	150	310	380
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			Free			8			Free			Free
Detector Phase	7	4		3	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Minimum Split (s)	9.0	21.0		9.0	21.0	21.0	9.0	21.0		9.0	21.0	
Total Split (s)	33.0	58.0		16.0	41.0	41.0	15.0	29.0		17.0	31.0	
Total Split (%)	27.5%	48.3%		13.3%	34.2%	34.2%	12.5%	24.2%		14.2%	25.8%	
Yellow Time (s)	3.0	5.0		3.0	5.0	5.0	3.0	5.0		3.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	6.0		4.0	6.0	6.0	4.0	6.0		4.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	Max	C-Max		Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	29.0	52.0	120.0	12.0	35.0	35.0	11.0	23.0	120.0	13.0	25.0	120.0
Actuated g/C Ratio	0.24	0.43	1.00	0.10	0.29	0.29	0.09	0.19	1.00	0.11	0.21	1.00
v/c Ratio	0.87	0.45	0.08	0.38	0.55	0.35	0.35	0.77	0.09	0.40	0.42	0.24
Control Delay	52.9	25.6	0.1	54.0	38.4	5.9	54.6	54.8	0.1	53.5	43.3	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	52.9	25.6	0.1	54.0	38.4	5.9	54.6	54.8	0.1	53.5	43.3	0.4
LOS	D	С	А	D	D	А	D	D	А	D	D	A
Approach Delay		36.4			32.9			45.2			25.7	
Approach LOS		D			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12	0											
Offset: 0 (0%), Referenced	I to phase 4	EBT and	8:WBT, S	Start of G	reen, Mas	ter Inters	ection					
Natural Cycle: 70												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.87												
Intersection Signal Delay:	35.1			Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliz				10	CU Level	of Service	эC					
ntersection Capacity Utilization 69.2% ICU Level of Service C Analysis Period (min) 15												

Splits and Phases: 1: Meridian & Woodmen

Ø1	<b>1</b> Ø2	<b>√</b> Ø3	▶Ø4 (R) -	
17 s	29 s	16 s	58 s	
<b>▲</b> Ø5	↓ Ø6		● ● Ø8 (R)	
15 s	31s	33 s	41 s	

### Intersection

Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	- 11	1	٦	<b>^</b>
Traffic Vol, veh/h	30	146	1300	121	62	805
Future Vol, veh/h	30	146	1300	121	62	805
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	100	0	-	400	385	-
Veh in Median Storage	, # 2	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	93	93	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	30	146	1398	130	62	805

Major/Minor	Minor1	Ν	1ajor1	Ν	/lajor2	
Conflicting Flow All	1925	699	0	0	1528	0
Stage 1	1398	-	-	-	-	-
Stage 2	527	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	59	382	-	-	432	-
Stage 1	194	-	-	-	-	-
Stage 2	557	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	r 51	382	-	-	432	-
Mov Cap-2 Maneuver	r 152	-	-	-	-	-
Stage 1	166	-	-	-	-	-
Stage 2	557	-	-	-	-	-

Minor Lane/Major Mvmt	NBT	NBRWB	Ln1W	/BLn2	SBL	SBT	
Capacity (veh/h)	-	- '	152	382	432	-	
HCM Lane V/C Ratio	-	- 0.1	197	0.382	0.144	-	
HCM Control Delay (s)	-	- 3	4.4	20.1	14.7	-	
HCM Lane LOS	-	-	D	С	В	-	
HCM 95th %tile Q(veh)	-	-	0.7	1.8	0.5	-	

# Timings 30: Golden Sage & Woodmen

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	7	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	٦	<b>↑</b>	1	ሻ	el 👘	
Traffic Volume (vph)	48	1385	85	15	956	46	106	16	16	67	6	
Future Volume (vph)	48	1385	85	15	956	46	106	16	16	67	6	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	2	2	2	6	6	6	8	8	8	4	4	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Split (s)	11.0	11.0	11.0	11.0	11.0	11.0	9.5	9.5	9.5	9.5	9.5	
Total Split (s)	84.0	84.0	84.0	84.0	84.0	84.0	36.0	36.0	36.0	36.0	36.0	
Total Split (%)	70.0%	70.0%	70.0%	70.0%	70.0%	70.0%	30.0%	30.0%	30.0%	30.0%	30.0%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	5.5	5.5	5.5	5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	86.3	86.3	86.3	86.3	86.3	86.3	21.2	21.2	21.2	21.2	21.2	
Actuated g/C Ratio	0.72	0.72	0.72	0.72	0.72	0.72	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.13	0.59	0.08	0.08	0.38	0.04	0.80	0.08	0.08	0.38	0.33	
Control Delay	7.8	10.3	1.6	5.3	5.3	0.4	73.0	38.3	7.4	46.3	10.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.8	10.3	1.6	5.3	5.3	0.4	73.0	38.3	7.4	46.3	10.4	
LOS	А	В	А	А	А	А	E	D	А	D	В	
Approach Delay		9.8			5.1			61.2			25.8	
Approach LOS		А			А			E			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12	0											
Offset: 96 (80%), Reference	ced to phase	2:EBTL	and 6:WE	STL, Start	of Green							
Natural Cycle: 55												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.80												
Intersection Signal Delay:					ntersectio							
Intersection Capacity Utiliz	ation 61.2%			10	CU Level	of Service	θB					
Analysis Period (min) 15												
Splits and Phases: 30: 0	Golden Sage	& Wood	men									
opino anu mases. 30. C	JUILEII Jaye											

● Ø2 (R)	↓ Ø4
84 s	36 s
● ● Ø6 (R)	108
84 s	36 s

Timings				
1: Meridian	Rd	&	Woodmen	Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u>^</u>	1	ካካ	- <b>†</b> †	1	ሻሻ	<b>^</b>	1	ካካ	- <b>†</b> †	7
Traffic Volume (vph)	275	675	100	100	800	75	150	300	50	200	575	800
Future Volume (vph)	275	675	100	100	800	75	150	300	50	200	575	800
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	21.0	56.0		12.0	47.0	47.0	16.0	34.0		18.0	36.0	
Total Split (%)	17.5%	46.7%		10.0%	39.2%	39.2%	13.3%	28.3%		15.0%	30.0%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	14.3	40.3	98.7	8.0	31.0	31.0	10.6	21.7	98.7	12.0	23.2	98.7
Actuated g/C Ratio	0.14	0.41	1.00	0.08	0.31	0.31	0.11	0.22	1.00	0.12	0.24	1.00
v/c Ratio	0.57	0.48	0.06	0.37	0.73	0.13	0.42	0.39	0.03	0.49	0.71	0.52
Control Delay	46.6	24.0	0.1	51.9	35.3	0.4	48.4	35.9	0.0	47.7	40.9	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.6	24.0	0.1	51.9	35.3	0.4	48.4	35.9	0.0	47.7	40.9	1.2
LOS	D	С	А	D	D	Α	D	D	А	D	D	A
Approach Delay		27.7			34.3			36.1			21.6	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 98.	7											
Natural Cycle: 60												
Control Type: Actuated-Unc	coordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 2					ntersectio							
Intersection Capacity Utiliza	ation 66.1%	ı		10	CU Level	of Service	ЭC					
Analysis Period (min) 15												
		147	D.									

Splits and Phases: 1: Meridian Rd & Woodmen Rd

<b>√</b> Ø1 →Ø2			<b>1</b> ø3	↓ Ø4
12 s 56 s		16	5s 3	6 s
<b>≸</b> <sub>Ø5</sub>	<b>▲</b> Ø6		Ø7	<b>↑</b> ø8
21 s	47 s	18	3 s	34 s

Timings 2: Meridian Rd & Eastonville Rd

	۶	-	4	-	•	1	Ť	۲	1	Ļ	~	
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ካካ	•	٦	•	1	٦	<u></u>	1	۲	<b>^</b>	1	
Traffic Volume (vph)	15	3	75	2	75	45	531	75	90	1446	3	
Future Volume (vph)	15	3	75	2	75	45	531	75	90	1446	3	
Turn Type	pm+pt	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	
Protected Phases	7	4	3	8		5	2		1	6		
Permitted Phases	4		8		8	2		2	6		6	
Detector Phase	7	4	3	8	8	5	2	2	1	6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5	
Total Split (s)	10.0	14.0	10.0	14.0	14.0	18.0	86.0	86.0	10.0	78.0	78.0	
Total Split (%)	8.3%	11.7%	8.3%	11.7%	11.7%	15.0%	71.7%	71.7%	8.3%	65.0%	65.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5	
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None	None	Min	Min	None	Min	Min	
Act Effct Green (s)	7.9	7.6	9.7	7.6	7.6	50.4	44.2	44.2	48.3	45.5	45.5	
Actuated g/C Ratio	0.12	0.11	0.14	0.11	0.11	0.75	0.65	0.65	0.71	0.67	0.67	
v/c Ratio	0.04	0.01	0.32	0.01	0.24	0.13	0.23	0.07	0.14	0.62	0.00	
Control Delay	35.0	40.0	35.3	40.5	1.7	3.8	7.4	0.8	3.8	12.0	0.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	35.0	40.0	35.3	40.5	1.7	3.8	7.4	0.8	3.8	12.0	0.0	
LOS	С	D	D	D	А	А	А	А	А	В	А	
Approach Delay		35.8		18.8			6.4			11.4		
Approach LOS		D		В			А			В		
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 67.6												
Natural Cycle: 65												
Control Type: Actuated-Uncod	ordinated											
Maximum v/c Ratio: 0.62												
Intersection Signal Delay: 10.					ntersectio							
Intersection Capacity Utilization	on 66.2%			10	CU Level	of Service	ЭC					
Analysis Period (min) 15												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1 Ø2		<b>√</b> ø3	4 <sub>04</sub>
10 s 86 s		10 s	14 s
▲ Ø5	Ø6		₹ <mark>Ø</mark> 8
18 s 78	S	10 s	14 s

Timings St 25: Golden Sage Rd & Woodmen Rd

	۶	-	$\mathbf{F}$	4	+	•	•	1	1	1	Ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	۲	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	ef 👘	
Traffic Volume (vph)	52	837	38	7	1719	24	132	7	4	27	6	
Future Volume (vph)	52	837	38	7	1719	24	132	7	4	27	6	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	17.0	80.0	80.0	10.0	73.0	73.0	16.0	15.0	15.0	15.0	14.0	
Total Split (%)	14.2%	66.7%	66.7%	8.3%	60.8%	60.8%	13.3%	12.5%	12.5%	12.5%	11.7%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	73.3	69.5	69.5	67.5	60.9	60.9	20.7	15.5	15.5	17.5	7.9	
Actuated g/C Ratio	0.70	0.66	0.66	0.64	0.58	0.58	0.20	0.15	0.15	0.17	0.08	
v/c Ratio	0.25	0.36	0.04	0.02	0.85	0.03	0.55	0.03	0.01	0.10	0.53	
Control Delay	7.5	8.6	0.1	5.1	24.3	0.0	48.6	48.3	0.0	37.5	20.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.5	8.6	0.1	5.1	24.3	0.0	48.6	48.3	0.0	37.5	20.3	
LOS	А	А	A	А	С	А	D	D	А	D	С	
Approach Delay		8.2			23.9			47.3			23.6	
Approach LOS		А			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 104	.7											
Natural Cycle: 75												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.85												
Intersection Signal Delay: 2					ntersectio							
Intersection Capacity Utiliza	ation 69.8%			10	CU Level	of Service	ЭC					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

	<b>1</b> Ø3	<b>₽</b> ø4
10 s 80 s	16 s	14 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>	Ø7	<b>√</b> ø8
17 s 73 s	15 s 🛛	15 s

### Intersection

Int Delay, s/veh

5.7

		FDT			MOT			NDT		0.01	ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		- <b>î</b> >			- सी			- सी	1		- <b>4</b> >		
Traffic Vol, veh/h	0	0	0	138	0	0	0	0	83	0	0	0	
Future Vol, veh/h	0	0	0	138	0	0	0	0	83	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	145	0	0	0	0	87	0	0	0	

Major/Minor	Minor2			Minor1			Major1			Major2			
Conflicting Flow All	-	88	1	1	1	-	1	0	0	87	0	0	
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-	
Stage 2	-	87	-	1	1	-	-	-	-	-	-	-	
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	0	802	1084	1022	895	0	1622	-	-	1509	-	-	
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-	
Stage 2	0	823	-	1022	895	0	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	• -	802	1084	1022	895	-	1622	-	-	1509	-	-	
Mov Cap-2 Maneuver	• -	802	-	1022	895	-	-	-	-	-	-	-	
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-	
Stage 2	-	823	-	1022	895	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.1	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EB	Ln1W	/BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1509	-	-	
HCM Lane V/C Ratio	-	-	-	-	0.142	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.1	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.5	0	-	-	

Timings			
1: Meridian	Rd &	Woodmen	Rd

	۶	-	$\mathbf{i}$	1	+	*	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u></u>	1	ካካ	- <b>†</b> †	1	ሻሻ	<b>^</b>	1	ካካ	<u></u>	1
Traffic Volume (vph)	600	725	200	150	650	150	175	750	175	150	450	325
Future Volume (vph)	600	725	200	150	650	150	175	750	175	150	450	325
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	30.0	55.0		14.0	39.0	39.0	17.0	37.0		14.0	34.0	
Total Split (%)	25.0%	45.8%		11.7%	32.5%	32.5%	14.2%	30.8%		11.7%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	23.9	41.8	109.2	9.7	27.6	27.6	11.5	28.7	109.2	9.7	26.9	109.2
Actuated g/C Ratio	0.22	0.38	1.00	0.09	0.25	0.25	0.11	0.26	1.00	0.09	0.25	1.00
v/c Ratio	0.82	0.55	0.13	0.50	0.74	0.29	0.49	0.82	0.11	0.50	0.53	0.21
Control Delay	51.6	28.1	0.2	56.2	43.6	5.5	53.2	47.2	0.1	56.2	39.4	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.6	28.1	0.2	56.2	43.6	5.5	53.2	47.2	0.1	56.2	39.4	0.3
LOS	D	С	А	E	D	А	D	D	А	E	D	A
Approach Delay		33.7			39.6			40.6			28.4	
Approach LOS		С			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 109	.2											
Natural Cycle: 75												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.82												
Intersection Signal Delay: 3	5.5			li	ntersectio	n LOS: D						
Intersection Capacity Utiliza	tion 76.0%	ı		10	CU Level	of Service	Ð					
Analysis Period (min) 15												
			_									

Splits and Phases: 1: Meridian Rd & Woodmen Rd

<b>√</b> Ø1	<b>→</b> Ø2		<b>≜</b> Ø3		Ø4
14 s	55 s		17 s	34	s
<b>≯</b> <sub>Ø5</sub>		<b>4</b> <sup>♠</sup> _ Ø6	Ø7	_ <b>†</b>	Ø8
30 s		39 s	14 s	37 s	

Timings 2: Meridian Rd & Eastonville Rd

Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0       4.5         All-Red Time (s)       2.0		٦	-	4	-	•	1	1	1	1	ţ	~	
Traffic Volume (vph)       13       3       40       3       150       68       1257       175       75       835         Future Volume (vph)       13       3       40       3       150       68       1257       175       75       835         Future Volume (vph)       13       3       40       3       150       68       1257       175       75       835         Prometede Phases       7       4       3       8       5       2       1       6         Detector Phase       7       4       3       8       8       5       2       2       1       6         Switch Phase       7       4       3       8       5       2       2       1       6         Minimum Spitt (s)       10.0       10.0       10.0       10.0       10.0       11.0       11.5       11.0       10.0       11.0       11.0       11.0       11.0       11.0       11.0       11.0       11.0       11.0       11.0       11.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0 <t< th=""><th>ane Group</th><th></th><th>EBT</th><th></th><th>WBT</th><th>WBR</th><th></th><th></th><th>NBR</th><th></th><th></th><th>SBR</th><th></th></t<>	ane Group		EBT		WBT	WBR			NBR			SBR	
Future Volume (vph)         13         3         40         3         150         68         1257         175         75         835           Turn Type         pm+pt         NA         pm+pt         NA         pm+pt         NA         Perm         pm+pt         NA         Perm         NA         Perm         pm+pt         NA         Perm         Protected Phases         7         4         3         8         5         2         1         6           Permitted Phases         7         4         3         8         8         5         2         2         1         6           Switch Phase         7         4         3         8         8         5         2         2         1         6           Minimum Split (s)         10.0         10.0         10.0         10.0         10.0         11.5         11.0         11.5         10.0         11.0         11.5         10.0         11.5         10.0         11.5         10.0         11.5         10.0         11.5         10.0         11.5         10.0         11.5         10.0         11.0         10.0         10.0         10.0         10.0         10.0         10.0         10.5	<u> </u>	ካካ	<b>↑</b>	<u>۲</u>	<b>↑</b>	1	<u>۲</u>	- <b>††</b>	1	<u>۲</u>	- <b>†</b> †	1	
Turn Type         pm+pt         NA         pm+pt         NA         Perm         pm+pt         NA         Perm         pm+pt         NA           Protected Phases         7         4         3         8         5         2         1         6           Permitted Phases         4         8         8         2         2         6           Detector Phase         7         4         3         8         5         2         2         1         6           Switch Phase         7         4         3         8         5         2         2         1         6           Minimum Initial (s)         5.0 <td>raffic Volume (vph)</td> <td></td> <td>8</td> <td></td>	raffic Volume (vph)											8	
Protected Phases       7       4       3       8       5       2       1       6         Permitted Phases       4       8       8       2       2       6         Detector Phase       7       4       3       8       8       2       2       6         Detector Phase       7       4       3       8       8       2       2       1       6         Switch Phase       7       4       3       8       8       5       2       2       1       6         Switch Phase       7       4       3       8       8       5       2       2       1       6         Switch Phase       7       4       3       8       5       2       2       1       6         Minimum Ritial (s)       15.0       2.0       5.0		13		40		150	68		175	75		8	
Permitted Phases         4         8         8         2         2         6           Detector Phase         7         4         3         8         8         5         2         2         1         6           Switch Phase         7         4         3         8         5         2         2         1         6           Switch Phase         7         4         3         8         5         2         2         1         6           Minimum Split (s)         10.0         10.0         10.0         10.0         11.0         11.5         11.5         10.0         11.5           Total Split (s)         12.5%         20.0%         10.0%         17.5%         17.5%         19.2%         60.0%         60.0%         10.0%         50.8%           Yellow Time (s)         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         4.5         4.5         3.0         4.5           Vellow Time (s)         4.0         4.0         4.0         4.0         4.0         4.0         4.0         5.5         5.5         4.0		pm+pt		pm+pt	NA	Perm	pm+pt		Perm	pm+pt		Perm	
Detector Phase         7         4         3         8         8         5         2         2         1         6           Switch Phase         Minimum Initial (s)         5.0			4		8			2		•	6		
Switch Phase         Minimum Initial (s)       5.0		4				•				6		6	
Minimum Initial (s)       5.0<		7	4	3	8	8	5	2	2	1	6	6	
Minimum Split (s)       10.0       10.0       10.0       10.0       10.0       11.5       11.5       11.5       10.0       11.5         Total Split (s)       15.0       24.0       12.0       21.0       21.0       23.0       72.0       72.0       12.0       61.0         Total Split (%)       12.5%       20.0%       10.0%       17.5%       17.5%       19.2%       60.0%       60.0%       10.0%       50.8%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0       4.5         All-Red Time (s)       2.0													
Total Split (s)       15.0       24.0       12.0       21.0       23.0       72.0       72.0       12.0       61.0         Total Split (%)       12.5%       20.0%       10.0%       17.5%       17.5%       19.2%       60.0%       60.0%       10.0%       50.8%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0       4.5         All-Red Time (s)       2.0 <td< td=""><td>( )</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>5.0</td><td></td></td<>	( )											5.0	
Total Split (%)       12.5%       20.0%       10.0%       17.5%       17.5%       19.2%       60.0%       60.0%       10.0%       50.8%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0       4.5         All-Red Time (s)       2.0       2												11.5	
Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0       4.5         All-Red Time (s)       2.0												61.0	
All-Red Time (s)       2.0 <td></td> <td>50.8%</td> <td></td>												50.8%	
Lost Time Adjust (s)       -1.0 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>4.5</td><td></td></td<>												4.5	
Total Lost Time (s)       4.0       4.0       4.0       4.0       4.0       4.0       4.0       5.5       5.5       4.0       5.5         Lead/Lag       Lead       Lag       Lag <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>2.0</td><td></td></t<>												2.0	
Lead/Lag         Lead         Lag         Lag <thlag< th="">         Lag         <thlag< th=""> <thlag<< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-1.0</td><td></td></thlag<<></thlag<></thlag<>												-1.0	
Lead-Lag Optimize?         Yes		4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5	
Recall Mode         None         None         None         None         None         None         Min         Min         None         Min           Act Effct Green (s)         8.8         7.7         9.5         8.3         8.3         47.1         39.1         39.1         46.1         38.7           Actuated g/C Ratio         0.13         0.11         0.14         0.12         0.69         0.57         0.57         0.68         0.57           V/c Ratio         0.03         0.01         0.19         0.01         0.47         0.14         0.63         0.19         0.23         0.42           Control Delay         29.3         35.3         30.6         34.7         10.8         4.0         12.5         2.1         5.2         10.3           Queue Delay         0.0			Lag	Lead			Lead			Lead	Lag	Lag	
Act Effct Green (s)       8.8       7.7       9.5       8.3       8.3       47.1       39.1       39.1       46.1       38.7         Actuated g/C Ratio       0.13       0.11       0.14       0.12       0.12       0.69       0.57       0.57       0.68       0.57         v/c Ratio       0.03       0.01       0.19       0.01       0.47       0.14       0.63       0.19       0.23       0.42         Control Delay       29.3       35.3       30.6       34.7       10.8       4.0       12.5       2.1       5.2       10.3         Queue Delay       0.0										Yes		Yes	
Actuated g/C Ratio       0.13       0.11       0.14       0.12       0.12       0.69       0.57       0.57       0.68       0.57         v/c Ratio       0.03       0.01       0.19       0.01       0.47       0.14       0.63       0.19       0.23       0.42         Control Delay       29.3       35.3       30.6       34.7       10.8       4.0       12.5       2.1       5.2       10.3         Queue Delay       0.0												Min	
v/c Ratio       0.03       0.01       0.19       0.01       0.47       0.14       0.63       0.19       0.23       0.42         Control Delay       29.3       35.3       30.6       34.7       10.8       4.0       12.5       2.1       5.2       10.3         Queue Delay       0.0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>39.1</td><td></td><td></td><td>38.7</td><td>38.7</td><td></td></t<>								39.1			38.7	38.7	
Control Delay       29.3       35.3       30.6       34.7       10.8       4.0       12.5       2.1       5.2       10.3         Queue Delay       0.0												0.57	
Queue Delay         0.0 <th< td=""><td>/c Ratio</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>0.01</td><td></td></th<>	/c Ratio											0.01	
Total Delay       29.3       35.3       30.6       34.7       10.8       4.0       12.5       2.1       5.2       10.3         LOS       C       D       C       C       B       A       B       A       A       B         Approach Delay       30.4       15.3       10.9       9.8         Approach LOS       C       B       B       A       A         Intersection Summary       C       B       B       A         Cycle Length: 120       Actuated Cycle Length: 68.1       Natural Cycle: 60       C       Control Type: Actuated-Uncoordinated         Maximum v/c Ratio: 0.63       Intersection LOS: B       Intersection LOS: B       A	,											0.0	
LOSCDCCBABAABApproach Delay30.415.310.99.8Approach LOSCBBAIntersection SummaryCycle Length: 120Actuated Cycle Length: 68.1Natural Cycle: 60Control Type: Actuated-UncoordinatedMaximum v/c Ratio: 0.63Intersection Signal Delay: 11.0Intersection LOS: B	Queue Delay											0.0	
Approach Delay30.415.310.99.8Approach LOSCBBAIntersection SummaryCycle Length: 120Actuated Cycle Length: 68.1Natural Cycle: 60Control Type: Actuated-UncoordinatedMaximum v/c Ratio: 0.63Intersection Signal Delay: 11.0Intersection LOS: B												0.0	
Approach LOS C B B A Intersection Summary Cycle Length: 120 Actuated Cycle Length: 68.1 Natural Cycle: 60 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B		С		С		В	А		А	А		А	
Intersection Summary Cycle Length: 120 Actuated Cycle Length: 68.1 Natural Cycle: 60 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B													
Cycle Length: 120 Actuated Cycle Length: 68.1 Natural Cycle: 60 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B	pproach LOS		С		В			В			А		
Actuated Cycle Length: 68.1 Natural Cycle: 60 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B	ntersection Summary												
Natural Cycle: 60         Control Type: Actuated-Uncoordinated         Maximum v/c Ratio: 0.63         Intersection Signal Delay: 11.0         Intersection LOS: B	Cycle Length: 120												
Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B	ctuated Cycle Length: 68.	1											
Maximum v/c Ratio: 0.63 Intersection Signal Delay: 11.0 Intersection LOS: B													
Intersection Signal Delay: 11.0 Intersection LOS: B	Control Type: Actuated-Unc	oordinated											
	laximum v/c Ratio: 0.63												
	ntersection Signal Delay: 1	1.0			Ir	ntersectio	n LOS: B						
Intersection Capacity Utilization 59.5% ICU Level of Service B	ntersection Capacity Utiliza	tion 59.5%	)		(	CU Level	of Service	θB					
Analysis Period (min) 15	nalysis Period (min) 15												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1	1 ø2	✓ Ø3
12 s	72 s	12 s 24 s
▲ Ø5	<b>₽</b> Ø6	▶ <sub>Ø7</sub> ♥ <sub>Ø8</sub>
23 s	61s	15 s 21 s

Timings Shows Show

	٦	+	7	4	+	*	•	1	1	1	ŧ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	4	
Traffic Volume (vph)	93	1539	85	15	1111	24	106	16	16	30	6	
Future Volume (vph)	93	1539	85	15	1111	24	106	16	16	30	6	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	18.0	82.0	82.0	10.0	74.0	74.0	15.0	18.0	18.0	10.0	13.0	
Total Split (%)	15.0%	68.3%	68.3%	8.3%	61.7%	61.7%	12.5%	15.0%	15.0%	8.3%	10.8%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	58.1	53.1	53.1	53.0	46.2	46.2	19.0	13.4	13.4	12.5	7.8	
Actuated g/C Ratio	0.68	0.62	0.62	0.62	0.54	0.54	0.22	0.16	0.16	0.15	0.09	
v/c Ratio	0.28	0.72	0.09	0.07	0.59	0.03	0.40	0.06	0.04	0.14	0.48	
Control Delay	7.1	14.7	1.3	5.7	16.2	0.0	36.3	41.7	0.2	33.8	18.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.1	14.7	1.3	5.7	16.2	0.0	36.3	41.7	0.2	33.8	18.1	
LOS	А	В	А	А	В	А	D	D	А	С	В	
Approach Delay		13.6			15.7			32.7			21.4	
Approach LOS		В			В			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 85.6	5											
Natural Cycle: 65												
Control Type: Semi Act-Unco	oord											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay: 15	5.6			Ir	ntersectio	n LOS: B						
Intersection Capacity Utilizat	tion 70.9%			(	CU Level	of Service	ЭC					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

🖌 Ø1 🕹 Ø2		▲ Ø3		₽ <mark>∞</mark> 4	
10 s 82 s		15 s		13 s	
	∲ Ø6	Ø7	-	Ø8	
18 s	74 s	10 s	18 s		

### Intersection

Int Delay, s/veh

4.7

		FDT			MOT			NDT		0.01	ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		- <b>î</b> >			- सी			- सी	1		- <b>4</b> >		
Traffic Vol, veh/h	0	0	0	145	0	0	0	0	133	0	0	0	
Future Vol, veh/h	0	0	0	145	0	0	0	0	133	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	153	0	0	0	0	140	0	0	0	

Major/Minor	Minor2		l	Minor1			Major1		l	Major2			
Conflicting Flow All	-	141	1	1	1	-	1	0	0	140	0	0	
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-	
Stage 2	-	140	-	1	1	-	-	-	-	-	-	-	
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	0	750	1084	1022	895	0	1622	-	-	1443	-	-	
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-	
Stage 2	0	781	-	1022	895	0	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	• -	750	1084	1022	895	-	1622	-	-	1443	-	-	
Mov Cap-2 Maneuver	• -	750	-	1022	895	-	-	-	-	-	-	-	
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-	
Stage 2	-	781	-	1022	895	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.1	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EE	BLn1W	/BLn1	SBL	SBT	SBR
Capacity (veh/h)	1622	-	-	-	1022	1443	-	-
HCM Lane V/C Ratio	-	-	-	- (	0.149	-	-	-
HCM Control Delay (s)	0	-	-	0	9.1	0	-	-
HCM Lane LOS	А	-	-	Α	Α	Α	-	-
HCM 95th %tile Q(veh)	0	-	-	-	0.5	0	-	-

## Timings 1: Meridian Rd & Woodmen Rd

	≯	-	$\mathbf{F}$	•	-	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u></u>	1	ካካ	- <b>†</b> †	1	ሻሻ	<u></u>	1	ካካ	<u></u>	7
Traffic Volume (vph)	275	675	100	100	832	43	162	288	50	200	575	809
Future Volume (vph)	275	675	100	100	832	43	162	288	50	200	575	809
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	21.0	56.0		12.0	47.0	47.0	16.0	34.0		18.0	36.0	
Total Split (%)	17.5%	46.7%		10.0%	39.2%	39.2%	13.3%	28.3%		15.0%	30.0%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	14.9	41.1	104.8	8.0	34.2	34.2	11.0	23.7	104.8	12.4	25.1	104.8
Actuated g/C Ratio	0.14	0.39	1.00	0.08	0.33	0.33	0.10	0.23	1.00	0.12	0.24	1.00
v/c Ratio	0.62	0.53	0.07	0.42	0.78	0.07	0.49	0.39	0.03	0.53	0.74	0.56
Control Delay	50.4	26.1	0.1	55.6	38.0	0.2	52.1	37.3	0.0	51.2	43.7	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.4	26.1	0.1	55.6	38.0	0.2	52.1	37.3	0.0	51.2	43.7	1.4
LOS	D	С	А	Е	D	А	D	D	А	D	D	А
Approach Delay		30.0			38.1			38.4			23.0	
Approach LOS		С			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 104	1.8											
Natural Cycle: 65												
Control Type: Actuated-Und	coordinated											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay: 3	0.3			I	ntersectio	n LOS: C						
Intersection Capacity Utiliza		ı		[	CU Level	of Service	ЭC					
Analysis Period (min) 15												
,												

Splits and Phases: 1: Meridian Rd & Woodmen Rd

<b>√</b> Ø1 →Ø2		<b>▲</b> ø3	▼ Ø4
12 s 56 s		16 s	36 s
<b>≸</b> <sub>Ø5</sub>	<b>4</b> <sup>♠</sup> Ø6	Ø7	¶ø8
21 s	47 s	18 s	34 s

Timings 2: Meridian Rd & Eastonville Rd

	۶	-	4	-	•	1	۲	1	ţ		
Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	NBR	SBL	SBT	Ø5	
Lane Configurations	ካካ	<b>†</b>	ሻ	<b>†</b>	1	<u></u>	1	٦	<u></u>		
Traffic Volume (vph)	15	3	75	2	75	531	75	90	1449		
Future Volume (vph)	15	3	75	2	75	531	75	90	1449		
Turn Type	pm+pt	NA	pm+pt	NA	Perm	NA	Perm	pm+pt	NA		
Protected Phases	7	4	3	8		2		1	6	5	
Permitted Phases	4		8		8		2	6			
Detector Phase	7	4	3	8	8	2	2	1	6		
Switch Phase											
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	10.0	
Total Split (s)	10.0	14.0	10.0	14.0	14.0	86.0	86.0	10.0	82.0	14.0	
Total Split (%)	8.3%	11.7%	8.3%	11.7%	11.7%	71.7%	71.7%	8.3%	68.3%	12%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0		
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5		
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lead	Lag	Lead	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None	Min	Min	None	Min	None	
Act Effct Green (s)	7.5	7.3	7.9	7.2	7.2	31.8	31.8	39.7	39.7		
Actuated g/C Ratio	0.14	0.14	0.15	0.13	0.13	0.59	0.59	0.74	0.74		
v/c Ratio	0.03	0.01	0.33	0.01	0.26	0.28	0.08	0.15	0.60		
Control Delay	23.9	28.7	27.3	29.0	5.0	8.4	1.4	3.6	6.4		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	23.9	28.7	27.3	29.0	5.0	8.4	1.4	3.6	6.4		
LOS	С	С	С	С	А	А	А	А	А		
Approach Delay		24.6		16.3		7.6			6.2		
Approach LOS		С		В		А			А		
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 53.9											
Natural Cycle: 70											
Control Type: Actuated-Uncod	ordinated										
Maximum v/c Ratio: 0.60											
Intersection Signal Delay: 7.4				Ir	ntersectio	n LOS: A					
Intersection Capacity Utilization				10	CU Level	of Service	с				
Analysis Period (min) 15											

Splits and Phases: 2: Meridian Rd & Eastonville Rd

	<b>√</b> Ø3	4 <sub>04</sub>
10 s 86 s	10 s	14 s
<b>↑</b> ø5 <b>∲</b> ø6	∕ Ø7	
14 s 82 s	10 s	14 s

Timings 25: Golden Sage Rd & Woodmen Rd

	٦	-	$\mathbf{F}$	4	-	•	1	Ť	۲	1	ţ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>††</b>	1	ሻ	- <b>††</b>	1	<u>۲</u>	<b>↑</b>	1	<u>۲</u>	4	
Traffic Volume (vph)	51	838	38	7	1719	24	132	7	4	27	6	
Future Volume (vph)	51	838	38	7	1719	24	132	7	4	27	6	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	17.0	80.0	80.0	10.0	73.0	73.0	16.0	15.0	15.0	15.0	14.0	
Total Split (%)	14.2%	66.7%	66.7%	8.3%	60.8%	60.8%	13.3%	12.5%	12.5%	12.5%	11.7%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	78.3	74.5	74.5	72.5	65.9	65.9	20.5	15.1	15.1	17.5	7.9	
Actuated g/C Ratio	0.71	0.68	0.68	0.66	0.60	0.60	0.19	0.14	0.14	0.16	0.07	
v/c Ratio	0.25	0.36	0.04	0.02	0.91	0.03	0.60	0.03	0.01	0.11	0.55	
Control Delay	8.3	8.5	0.1	5.1	28.3	0.0	52.1	48.4	0.0	37.9	21.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	8.3	8.5	0.1	5.1	28.3	0.0	52.1	48.4	0.0	37.9	21.0	
LOS	А	А	А	А	С	А	D	D	А	D	С	
Approach Delay		8.1			27.8			50.5			24.2	
Approach LOS		А			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 109	.6											
Natural Cycle: 80												
Control Type: Semi Act-Und	coord											
Maximum v/c Ratio: 0.91												
Intersection Signal Delay: 2	2.9			lr	ntersectio	n LOS: C						
Intersection Capacity Utiliza		ı		10	CU Level	of Service	ЭC					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

✓ Ø1	<b>↑</b> ø3	
10 s 80 s	16 s	14 s
▶ <sub>Ø5</sub> ♦ ∞6	Ø7	- <b>1</b> ø8
17 s 73 s	15 s 1	.5 s

### Intersection

Int Delay, s/veh

5.7

	SRL SRI								EDT	EDI	N A
		INDR SI	NBT	NBL	WBR	WBT	WBL	EBR	EBT	EBL	Movement
Lane Configurations is a contract of the contr		1	- କ			- सी			ef 👘		Lane Configurations
Traffic Vol, veh/h 0 0 0 138 0 0 0 82 0 0 0	0 0	82	0	0	0	0	138	0	0	0	Traffic Vol, veh/h
Future Vol, veh/h 0 0 0 138 0 0 0 82 0 0 0	0 0	82	0	0	0	0	138	0	0	0	Future Vol, veh/h
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0	0	0	0	0	0	0	0	0	0	Conflicting Peds, #/hr
Sign Control Stop Stop Stop Stop Stop Stop Free Free Free Free Free Free	Free Free	Free Fr	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Sign Control
RT Channelized None None None None		None	-	-	None	-	-	None	-	-	RT Channelized
Storage Length 0		0	-	-	-	-	-	-	-	-	Storage Length
Veh in Median Storage, # - 0 0 0 0 -	- 0	-	0	-	-	0	-	-	0	# -	Veh in Median Storage,
Grade, % - 0 0 0 0 -	- 0	-	0	-	-	0	-	-	0	-	Grade, %
Peak Hour Factor 95 95 95 95 95 95 95 95 95 95 95 95 95	95 95	95	95	95	95	95	95	95	95	95	Peak Hour Factor
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2 2	2	2	2	2	2	2	2	2	2	Heavy Vehicles, %
Mvmt Flow 0 0 0 145 0 0 0 86 0 0 0	0 0	86	0	0	0	0	145	0	0	0	Mvmt Flow

Major/Minor	Minor2			Vinor1			Major1		Ν	/lajor2				
Conflicting Flow All	-	87	1	1	1	-	1	0	0	86	0	0		
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-		
Stage 2	-	86	-	1	1	-	-	-	-	-	-	-		
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-		
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-		
Pot Cap-1 Maneuver	0	803	1084	1022	895	0	1622	-	-	1510	-	-		
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-		
Stage 2	0	824	-	1022	895	0	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	• -	803	1084	1022	895	-	1622	-	-	1510	-	-		
Mov Cap-2 Maneuver	• -	803	-	1022	895	-	-	-	-	-	-	-		
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-		
Stage 2	-	824	-	1022	895	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	9.1	0	0
HCM LOS	А	А		

Minor Lane/Major Mvmt	NBL	NBT	NBR EE	BLn1W	BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1510	-	-	
HCM Lane V/C Ratio	-	-	-	- (	).142	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.1	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.5	0	-	-	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u></u>	1	ካካ	- <b>†</b> †	1	ሻሻ	<u></u>	1	ካካ	<u></u>	1
Traffic Volume (vph)	600	725	200	150	708	92	185	740	175	150	450	342
Future Volume (vph)	600	725	200	150	708	92	185	740	175	150	450	342
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	30.0	55.0		14.0	39.0	39.0	17.0	37.0		14.0	34.0	
Total Split (%)	25.0%	45.8%		11.7%	32.5%	32.5%	14.2%	30.8%		11.7%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	24.1	43.5	111.0	9.7	29.1	29.1	11.7	28.8	111.0	9.7	26.8	111.0
Actuated g/C Ratio	0.22	0.39	1.00	0.09	0.26	0.26	0.11	0.26	1.00	0.09	0.24	1.00
v/c Ratio	0.83	0.53	0.13	0.51	0.78	0.17	0.52	0.82	0.11	0.51	0.54	0.22
Control Delay	53.2	27.6	0.2	57.1	45.0	0.7	54.2	48.0	0.1	57.1	40.5	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.2	27.6	0.2	57.1	45.0	0.7	54.2	48.0	0.1	57.1	40.5	0.3
LOS	D	С	А	Е	D	А	D	D	А	Е	D	A
Approach Delay		34.1			42.6			41.4			28.5	
Approach LOS		С			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 111												
Natural Cycle: 80												
Control Type: Actuated-Uncod	ordinated											
Maximum v/c Ratio: 0.83												
Intersection Signal Delay: 36.	5			lı	ntersectio	n LOS: D						
Intersection Capacity Utilization		ı				of Service	Ð					
Analysis Period (min) 15												

<b>√</b> Ø1	<b>→</b> Ø2		<b>≜</b> Ø3		Ø4
14 s	55 s		17 s	34	s
<b>≯</b> <sub>∅5</sub>		<b>4</b> <sup>♠</sup> _ Ø6	Ø7	_ <b>†</b>	Ø8
30 s		39 s	14 s	37 s	

	٦	-	4	-	•	1	۲	1	ţ		
Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	NBR	SBL	SBT	Ø5	
Lane Configurations	ካካ	<b>↑</b>	ሻ	<b>↑</b>	1	- <b>†</b> †	1	ሻ	- <b>†</b> †		
Traffic Volume (vph)	13	3	40	3	150	1257	175	75	843		
Future Volume (vph)	13	3	40	3	150	1257	175	75	843		
Turn Type	pm+pt	NA	pm+pt	NA	Perm	NA	Perm	pm+pt	NA		
Protected Phases	7	4	3	8		2		1	6	5	
Permitted Phases	4		8		8		2	6			
Detector Phase	7	4	3	8	8	2	2	1	6		
Switch Phase											
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	10.0	
Total Split (s)	15.0	24.0	12.0	21.0	21.0	72.0	72.0	12.0	62.0	22.0	
Total Split (%)	12.5%	20.0%	10.0%	17.5%	17.5%	60.0%	60.0%	10.0%	51.7%	18%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0		
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5		
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lead	Lag	Lead	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None	Min	Min	None	Min	None	
Act Effct Green (s)	8.8	7.7	9.5	8.3	8.3	39.1	39.1	49.8	48.2		
Actuated g/C Ratio	0.13	0.11	0.14	0.12	0.12	0.57	0.57	0.73	0.71		
v/c Ratio	0.03	0.01	0.19	0.01	0.47	0.63	0.19	0.23	0.34		
Control Delay	29.3	35.3	30.6	34.7	10.8	12.5	2.1	4.9	4.6		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	29.3	35.3	30.6	34.7	10.8	12.5	2.1	4.9	4.6		
LOS	С	D	С	С	В	В	А	А	А		
Approach Delay		30.4		15.3		11.2			4.7		
Approach LOS		С		В		В			А		
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 68.1											
Natural Cycle: 60											
Control Type: Actuated-Unco	ordinated										
Maximum v/c Ratio: 0.63											
Intersection Signal Delay: 9.3	3			I	ntersectio	n LOS: A					
Intersection Capacity Utilizat						of Service	B				
Analysis Period (min) 15											

Ø1 102		€ø3	- <b>1</b> 04
12 s 72 s		12 s	24 s
★ ø5	<b>↓</b> ∞ <sub>6</sub>		<b>₹</b> Ø8
22 s	62 s	15 s	21 s

Timings 25: Golden Sage Rd & Woodmen Rd

	۶	-	$\mathbf{F}$	4	←	•	1	Ť	۲	1	ţ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	4	
Traffic Volume (vph)	88	1544	85	15	1111	24	106	16	16	30	6	
Future Volume (vph)	88	1544	85	15	1111	24	106	16	16	30	6	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	18.0	82.0	82.0	10.0	74.0	74.0	15.0	18.0	18.0	10.0	13.0	
Total Split (%)	15.0%	68.3%	68.3%	8.3%	61.7%	61.7%	12.5%	15.0%	15.0%	8.3%	10.8%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	58.4	53.4	53.4	53.5	46.7	46.7	19.0	13.5	13.5	12.5	7.8	
Actuated g/C Ratio	0.68	0.62	0.62	0.62	0.54	0.54	0.22	0.16	0.16	0.15	0.09	
v/c Ratio	0.26	0.72	0.09	0.07	0.59	0.03	0.40	0.06	0.04	0.14	0.48	
Control Delay	7.0	14.7	1.3	5.7	16.0	0.0	36.6	42.1	0.2	34.1	18.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.0	14.7	1.3	5.7	16.0	0.0	36.6	42.1	0.2	34.1	18.3	
LOS	А	В	А	А	В	А	D	D	А	С	В	
Approach Delay		13.6			15.5			33.0			21.6	
Approach LOS		В			В			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 86												
Natural Cycle: 65												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay: 15	.6			lr	ntersection	n LOS: B						
Intersection Capacity Utilizati				10	CU Level	of Service	ЭC					
Analysis Period (min) 15												

✓ø1 ↓ ø2		<b>▲</b> Ø3	₽ Ø4
10 s 82 s		15 s	13 s
▶ Ø5	★ ♥ Ø6	Ø7	- Ø8
18 s	'4s	10 s	18 s

#### Intersection

Int Delay, s/veh

4.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		et			÷			÷	1		\$		
Traffic Vol, veh/h	0	0	0	145	0	0	0	0	128	0	0	0	
Future Vol, veh/h	0	0	0	145	0	0	0	0	128	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	153	0	0	0	0	135	0	0	0	

Major/Minor	Minor2			Minor1			Major1			Major2			
Conflicting Flow All	-	136	1	1	1	-	1	0	0	135	0	0	
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-	
Stage 2	-	135	-	1	1	-	-	-	-	-	-	-	
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	0	755	1084	1022	895	0	1622	-	-	1449	-	-	
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-	
Stage 2	0	785	-	1022	895	0	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	• -	755	1084	1022	895	-	1622	-	-	1449	-	-	
Mov Cap-2 Maneuver	· -	755	-	1022	895	-	-	-	-	-	-	-	
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-	
Stage 2	-	785	-	1022	895	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.1	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EE	BLn1W	/BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1449	-	-	
HCM Lane V/C Ratio	-	-	-	-	0.149	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.1	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.5	0	-	-	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u> </u>	1	ካካ	- <b>†</b> †	1	ሻሻ	<u>^</u>	1	ካካ	- <b>†</b> †	1
Traffic Volume (vph)	293	645	100	100	754	167	150	381	50	241	621	807
Future Volume (vph)	293	645	100	100	754	167	150	381	50	241	621	807
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	21.0	56.0		12.0	47.0	47.0	16.0	34.0		18.0	36.0	
Total Split (%)	17.5%	46.7%		10.0%	39.2%	39.2%	13.3%	28.3%		15.0%	30.0%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	14.6	40.0	99.6	8.0	30.4	30.4	10.6	22.3	99.6	12.8	24.5	99.6
Actuated g/C Ratio	0.15	0.40	1.00	0.08	0.31	0.31	0.11	0.22	1.00	0.13	0.25	1.00
v/c Ratio	0.59	0.46	0.06	0.37	0.71	0.28	0.42	0.49	0.03	0.56	0.73	0.52
Control Delay	47.4	24.4	0.1	52.4	35.4	5.5	48.9	37.3	0.0	49.0	41.2	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.4	24.4	0.1	52.4	35.4	5.5	48.9	37.3	0.0	49.0	41.2	1.2
LOS	D	С	А	D	D	А	D	D	А	D	D	A
Approach Delay		28.5			32.2			37.1			23.0	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 99.	6											
Natural Cycle: 60												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 2	28.4			Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliza		ı		10	CU Level	of Service	ЭC					
Analysis Period (min) 15												
. ,												

<b>√</b> Ø1 →Ø2			<b>1</b> ø3	↓ Ø4
12 s 56 s		16	5s 3	6 s
<b>≸</b> <sub>Ø5</sub>	<b>▲</b> Ø6		Ø7	<b>↑</b> ø8
21 s	47 s	18	3 s	34 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻ	- <b>†</b> †	1	ሻ	- <b>††</b>	1
Traffic Volume (vph)	113	42	89	75	45	75	285	481	75	90	1458	78
Future Volume (vph)	113	42	89	75	45	75	285	481	75	90	1458	78
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	10.0	14.0	14.0	10.0	14.0	14.0	18.0	86.0	86.0	10.0	78.0	78.0
Total Split (%)	8.3%	11.7%	11.7%	8.3%	11.7%	11.7%	15.0%	71.7%	71.7%	8.3%	65.0%	65.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	13.2	8.7	8.7	13.0	8.7	8.7	71.2	62.1	62.1	58.7	50.8	50.8
Actuated g/C Ratio	0.14	0.09	0.09	0.14	0.09	0.09	0.74	0.65	0.65	0.61	0.53	0.53
v/c Ratio	0.31	0.26	0.32	0.37	0.28	0.27	0.86	0.21	0.07	0.16	0.79	0.09
Control Delay	39.3	50.4	2.8	43.9	50.8	2.2	51.8	8.0	0.7	5.1	21.5	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	39.3	50.4	2.8	43.9	50.8	2.2	51.8	8.0	0.7	5.1	21.5	0.2
LOS	D	D	Α	D	D	А	D	А	А	А	С	A
Approach Delay		27.8			29.4			22.4			19.5	
Approach LOS		С			С			С			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 95.7												
Natural Cycle: 80												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 21.				lı	ntersectio	n LOS: C						
Intersection Capacity Utilizati		)		[(	CU Level	of Service	эD					
Analysis Period (min) 15												
,												

Ø1 Ø2	🖌 Ø3 🔶 Ø4
10 s 86 s	10 s 14 s
▲ Ø5	▶ Ø7
18 s 78 s	10 s 14 s

Timings 25: Golden Sage Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	4	
Traffic Volume (vph)	93	826	38	7	1680	24	132	10	4	27	7	
Future Volume (vph)	93	826	38	7	1680	24	132	10	4	27	7	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	17.0	80.0	80.0	10.0	73.0	73.0	16.0	15.0	15.0	15.0	14.0	
Total Split (%)	14.2%	66.7%	66.7%	8.3%	60.8%	60.8%	13.3%	12.5%	12.5%	12.5%	11.7%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	74.1	70.3	70.3	66.7	60.2	60.2	21.1	15.9	15.9	18.2	8.6	
Actuated g/C Ratio	0.70	0.66	0.66	0.63	0.57	0.57	0.20	0.15	0.15	0.17	0.08	
v/c Ratio	0.39	0.36	0.04	0.02	0.86	0.03	0.60	0.04	0.01	0.10	0.70	
Control Delay	16.5	8.8	0.1	5.4	26.0	0.0	52.3	49.5	0.0	38.1	31.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	16.5	8.8	0.1	5.4	26.0	0.0	52.3	49.5	0.0	38.1	31.2	
LOS	В	А	А	А	С	А	D	D	А	D	С	
Approach Delay		9.2			25.6			50.7			32.2	
Approach LOS		А			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 106.4	ł											
Natural Cycle: 75												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 22.	.1			lr	ntersectio	n LOS: C						
Intersection Capacity Utilizati				10	CU Level	of Service	ε					
Analysis Period (min) 15												

✓ Ø1	<b>Ø</b> 3	<b>₽</b> ⁄04
10 s 80 s	16 s	14 s
▶ø5 ♥ø6	Ø7	<b>1</b> 08
17 s 73 s	15 s	15 s

#### Intersection

Int Delay, s/veh

5.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		et			÷			ŧ	1		\$		
Traffic Vol, veh/h	0	0	0	195	0	0	0	0	127	0	0	0	
Future Vol, veh/h	0	0	0	195	0	0	0	0	127	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	205	0	0	0	0	134	0	0	0	

Major/Minor	Minor2			Minor1			Major1		l	Major2			
Conflicting Flow All	-	135	1	1	1	-	1	0	0	134	0	0	
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-	
Stage 2	-	134	-	1	1	-	-	-	-	-	-	-	
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	0	756	1084	1022	895	0	1622	-	-	1451	-	-	
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-	
Stage 2	0	785	-	1022	895	0	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· -	756	1084	1022	895	-	1622	-	-	1451	-	-	
Mov Cap-2 Maneuver	· -	756	-	1022	895	-	-	-	-	-	-	-	
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-	
Stage 2	-	785	-	1022	895	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.4	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EE	3Ln1V	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1451	-	-	
HCM Lane V/C Ratio	-	-	-	-	0.201	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.4	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.7	0	-	-	

### SimTraffic Performance Report

87: Meridian Rd &	Site RIR	O Per	formai	nce by	moven	nent Interval #1 7:00
Movement	EBR	NBT	SBT	SBR	All	
Stop Del/Veh (s)	11.6	0.7	0.1	0.1	1.1	

## 87: Meridian Rd & Site RIRO Performance by movement Interval #2 7:15

Movement EBR NBT SBT SBR All
18.8 0.7 0.1 0.1

# 87: Meridian Rd & Site RIRO Performance by movement Interval #3 7:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	15.3	0.7	0.1	0.1	1.2

#### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 7:45

M		NDT	ODT	000	A 11
Movement	EBK	NBT	SBT	SBR	All
Stop Del/Veh (s)	9.2	0.6	0.1	0.1	0.8

#### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Movement	EBR	NBT	SBT	SBR	All
p Del/Veh (s)	14.1	0.7	0.1	0.1	1.

#### Total Zone Performance By Interval

Interval Start	7:00	7:15	7:30	7:45	All	
Stop Del/Veh (s)	74.9	78.3	52.8	85.8	499.8	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	- <b>†</b> †	1	ካካ	- <b>†</b> †	1	ካካ	<b>^</b>	1	ካካ	- <b>†</b> †	7
Traffic Volume (vph)	622	669	200	150	611	258	175	871	175	225	566	368
Future Volume (vph)	622	669	200	150	611	258	175	871	175	225	566	368
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	30.0	55.0		14.0	39.0	39.0	17.0	37.0		14.0	34.0	
Total Split (%)	25.0%	45.8%		11.7%	32.5%	32.5%	14.2%	30.8%		11.7%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	24.5	42.1	112.1	9.7	27.3	27.3	11.5	31.1	112.1	10.1	29.6	112.1
Actuated g/C Ratio	0.22	0.38	1.00	0.09	0.24	0.24	0.10	0.28	1.00	0.09	0.26	1.00
v/c Ratio	0.85	0.51	0.13	0.52	0.72	0.50	0.51	0.91	0.11	0.75	0.62	0.24
Control Delay	54.3	28.4	0.2	57.1	44.3	14.8	54.0	53.6	0.1	66.9	41.0	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	54.3	28.4	0.2	57.1	44.3	14.8	54.0	53.6	0.1	66.9	41.0	0.4
LOS	D	С	А	E	D	В	D	D	А	E	D	A
Approach Delay		35.4			38.7			46.0			33.1	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	2.1											
Natural Cycle: 80												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.91												
Intersection Signal Delay:	38.2			Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliz	ation 81.0%			(	CU Level	of Service	e D					
Analysis Period (min) 15												
- , ,												

Ø1	<b>→</b> <sub>Ø2</sub>		<b>▲</b> Ø3	↓ Ø4
14 s	55 s		17 s	34 s
<b>≯</b> <sub>∅5</sub>		<b>4</b> <sup>♠</sup> _ Ø6	Ø7	<b>1</b> Ø8
30 s		39 s	14 s 💦 🕺	37 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	ሻ	<b>†</b>	1	٦	- <b>†</b> †	1	۲	- <b>†</b> †	7
Traffic Volume (vph)	262	113	134	40	68	150	442	1133	175	75	881	91
Future Volume (vph)	262	113	134	40	68	150	442	1133	175	75	881	91
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	15.0	24.0	24.0	12.0	21.0	21.0	23.0	72.0	72.0	12.0	61.0	61.0
Total Split (%)	12.5%	20.0%	20.0%	10.0%	17.5%	17.5%	19.2%	60.0%	60.0%	10.0%	50.8%	50.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	24.3	18.2	18.2	17.5	10.0	10.0	55.3	44.6	44.6	39.7	30.5	30.5
Actuated g/C Ratio	0.28	0.21	0.21	0.20	0.11	0.11	0.63	0.51	0.51	0.45	0.35	0.35
v/c Ratio	0.39	0.31	0.31	0.14	0.34	0.48	0.94	0.65	0.21	0.27	0.74	0.15
Control Delay	27.6	36.9	5.8	26.9	43.0	11.1	51.7	19.0	2.7	10.8	29.3	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.6	36.9	5.8	26.9	43.0	11.1	51.7	19.0	2.7	10.8	29.3	1.1
LOS	С	D	А	С	D	В	D	В	А	В	С	A
Approach Delay		23.9			22.0			25.8			25.4	
Approach LOS		С			С			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 88.3	3											
Natural Cycle: 65												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.94												
Intersection Signal Delay: 2	5.1			Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliza	tion 74.2%	)		(	CU Level	of Service	e D					
Analysis Period (min) 15												
- · · /												

Ø1	1 ø2	✓ Ø3
12 s	72 s	12 s 24 s
▲ Ø5	<b>₽</b> Ø6	▶ <sub>Ø7</sub> ♥ <sub>Ø8</sub>
23 s	61s	15 s 21 s

Timings 25: Golden Sage Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	•	1	ሻ	eî 👘	
Traffic Volume (vph)	170	1505	85	15	1115	24	106	20	16	30	10	
Future Volume (vph)	170	1505	85	15	1115	24	106	20	16	30	10	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	18.0	82.0	82.0	10.0	74.0	74.0	15.0	18.0	18.0	10.0	13.0	
Total Split (%)	15.0%	68.3%	68.3%	8.3%	61.7%	61.7%	12.5%	15.0%	15.0%	8.3%	10.8%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	57.5	52.3	52.3	48.8	40.2	40.2	19.0	13.5	13.5	12.5	7.9	
Actuated g/C Ratio	0.68	0.62	0.62	0.57	0.47	0.47	0.22	0.16	0.16	0.15	0.09	
v/c Ratio	0.48	0.70	0.09	0.07	0.68	0.03	0.41	0.07	0.04	0.14	0.57	
Control Delay	10.5	14.6	1.3	5.9	19.4	0.1	36.6	40.7	0.2	33.4	18.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.5	14.6	1.3	5.9	19.4	0.1	36.6	40.7	0.2	33.4	18.5	
LOS	В	В	А	А	В	А	D	D	А	С	В	
Approach Delay		13.5			18.8			33.0			20.9	
Approach LOS		В			В			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 84.9												
Natural Cycle: 65												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 16.	7				ntersectio							
Intersection Capacity Utilization	on 76.2%			10	CU Level	of Service	e D					
Analysis Period (min) 15												

🖌 Ø1 🕹 Ø2		▲ Ø3		₽ <mark>∞</mark> 4	
10 s 82 s		15 s		13 s	
	∲ Ø6	Ø7	-	Ø8	
18 s	74 s	10 s	18 s		

#### Intersection

Int Delay, s/veh

4.4

M		FDT			WDT			NDT			ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		- <b>î</b> >			- सी			- सी	1		- <b>4</b> >		
Traffic Vol, veh/h	0	0	0	187	0	0	0	0	215	0	0	0	
Future Vol, veh/h	0	0	0	187	0	0	0	0	215	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	197	0	0	0	0	226	0	0	0	

Major/Minor	Minor2			Minor1			Major1			Major2			
Conflicting Flow All	-	227	1	1	1	-	1	0	0	226	0	0	
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-	
Stage 2	-	226	-	1	1	-	-	-	-	-	-	-	
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	0	672	1084	1022	895	0	1622	-	-	1342	-	-	
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-	
Stage 2	0	717	-	1022	895	0	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	• -	672	1084	1022	895	-	1622	-	-	1342	-	-	
Mov Cap-2 Maneuver	· -	672	-	1022	895	-	-	-	-	-	-	-	
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-	
Stage 2	-	717	-	1022	895	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.4	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EB	Ln1W	/BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1342	-	-	
HCM Lane V/C Ratio	-	-	-	-	0.193	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.4	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.7	0	-	-	

### SimTraffic Performance Report

87: Meridian Rd & Site RIRO Performance by movement Interval #1 5:00												
Movement	EBR	NBT	SBT	SBR	All							
Stop Del/Veh (s)	8.5	2.0	0.1	0.1	1.8							
87: Meridian Rd & Site RIRO Performance by movement Interval #2 5:15												
Movement	EBR	NBT	SBT	SBR	All							
Stop Del/Veh (s)	7.9	1.8	0.1	0.1	1.7							

### 87: Meridian Rd & Site RIRO Performance by movement Interval #3 5:30

ovement EBR NBT SBT SBR A
9.4 1.9 0.1 0.1

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 5:45

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

viovement

#### Total Zone Performance By Interval

Interval Start	5:00	5:15	5:30	5:45	All
Stop Del/Veh (s)	50.4	72.0	40.5	40.3	82.2

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	- <b>†</b> †	1	ካካ	- <b>†</b> †	1	ሻሻ	- <b>††</b>	1	ካካ	- <b>†</b> †	1
Traffic Volume (vph)	286	645	100	100	852	69	212	319	50	241	621	842
Future Volume (vph)	286	645	100	100	852	69	212	319	50	241	621	842
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	21.0	56.0		12.0	47.0	47.0	16.0	34.0		18.0	36.0	
Total Split (%)	17.5%	46.7%		10.0%	39.2%	39.2%	13.3%	28.3%		15.0%	30.0%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	14.9	40.3	104.4	8.0	33.4	33.4	11.4	23.8	104.4	12.8	25.2	104.4
Actuated g/C Ratio	0.14	0.39	1.00	0.08	0.32	0.32	0.11	0.23	1.00	0.12	0.24	1.00
v/c Ratio	0.62	0.48	0.06	0.39	0.77	0.11	0.57	0.40	0.03	0.58	0.74	0.54
Control Delay	50.1	25.5	0.1	54.8	37.7	0.4	53.6	37.2	0.0	51.8	43.5	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.1	25.5	0.1	54.8	37.7	0.4	53.6	37.2	0.0	51.8	43.5	1.3
LOS	D	С	А	D	D	А	D	D	А	D	D	A
Approach Delay		30.1			36.9			40.0			23.9	
Approach LOS		С			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 104	4.4											
Natural Cycle: 65												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 3	30.6			I	ntersectio	n LOS: C						
Intersection Capacity Utilization	ation 71.1%	I		[(	CU Level	of Service	ЭC					
Analysis Period (min) 15												

<b>√</b> Ø1 →Ø2			<b>1</b> ø3	↓ Ø4
12 s 56 s		16	5s 3	6 s
<b>≸</b> <sub>Ø5</sub>	<b>▲</b> Ø6		Ø7	<b>↑</b> ø8
21 s	47 s	18	3 s	34 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻ	- <b>††</b>	1	ሻ	- <b>††</b>	1
Traffic Volume (vph)	113	42	89	75	45	75	117	482	75	90	1472	64
Future Volume (vph)	113	42	89	75	45	75	117	482	75	90	1472	64
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	10.0	14.0	14.0	10.0	14.0	14.0	14.0	86.0	86.0	10.0	82.0	82.0
Total Split (%)	8.3%	11.7%	11.7%	8.3%	11.7%	11.7%	11.7%	71.7%	71.7%	8.3%	68.3%	68.3%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	13.1	9.0	9.0	13.1	9.0	9.0	64.7	57.9	57.9	56.8	48.5	48.5
Actuated g/C Ratio	0.15	0.10	0.10	0.15	0.10	0.10	0.75	0.67	0.67	0.66	0.57	0.57
v/c Ratio	0.26	0.23	0.34	0.33	0.24	0.29	0.40	0.21	0.07	0.15	0.75	0.07
Control Delay	35.2	45.8	8.0	38.8	46.1	5.2	12.8	8.4	0.8	4.9	17.9	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.2	45.8	8.0	38.8	46.1	5.2	12.8	8.4	0.8	4.9	17.9	0.6
LOS	D	D	Α	D	D	А	В	А	А	А	В	A
Approach Delay		27.1			27.5			8.3			16.5	
Approach LOS		С			С			А			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 85.8												
Natural Cycle: 65												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 16.				Ir	ntersectio	n LOS: B						
Intersection Capacity Utilizati		)		(	CU Level	of Service	ЭC					
Analysis Period (min) 15												
- , , ,												

	<b>√</b> Ø3	4 <sub>04</sub>
10 s 86 s	10 s	14 s
<b>↑</b> ø5 <b>∲</b> ø6	∕ <mark>ø</mark> 7	
14 s 82 s	10 s	14 s

Timings					
25: Golden	Sage	Rd 8	λ,	Woodmen	Rd

	٦	-	$\mathbf{r}$	4	-	•	1	Ť	1	1	Ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	•	1	ሻ	4	
Traffic Volume (vph)	89	826	38	7	1680	24	132	10	4	27	7	
Future Volume (vph)	89	826	38	7	1680	24	132	10	4	27	7	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	17.0	80.0	80.0	10.0	73.0	73.0	16.0	15.0	15.0	15.0	14.0	
Total Split (%)	14.2%	66.7%	66.7%	8.3%	60.8%	60.8%	13.3%	12.5%	12.5%	12.5%	11.7%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	74.0	70.2	70.2	66.7	60.2	60.2	21.1	15.9	15.9	18.2	8.6	
Actuated g/C Ratio	0.70	0.66	0.66	0.63	0.57	0.57	0.20	0.15	0.15	0.17	0.08	
v/c Ratio	0.38	0.36	0.04	0.02	0.86	0.03	0.60	0.04	0.01	0.10	0.70	
Control Delay	15.5	8.8	0.1	5.4	25.9	0.0	52.2	49.4	0.0	38.1	31.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	15.5	8.8	0.1	5.4	25.9	0.0	52.2	49.4	0.0	38.1	31.2	
LOS	В	А	А	А	С	А	D	D	А	D	С	
Approach Delay		9.1			25.5			50.7			32.1	
Approach LOS		А			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 106	.3											
Natural Cycle: 75												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 22	2.0			Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliza	tion 84.1%			10	CU Level	of Service	εE					
Analysis Period (min) 15												

✓ Ø1	<b>Ø</b> 3	<b>₽</b> ⁄04
10 s 80 s	16 s	14 s
▶ø5 ♥ø6	Ø7	<b>1</b> 08
17 s 73 s	15 s	15 s

5.7

#### Intersection

Int Delay, s/veh

					MOT			NET		0.51	0.0	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		- <b>î</b> >			୍ କ			- सी	1		- <b>4</b> >		
Traffic Vol, veh/h	0	0	0	195	0	0	0	0	123	0	0	0	
Future Vol, veh/h	0	0	0	195	0	0	0	0	123	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	205	0	0	0	0	129	0	0	0	

Major/Minor	Minor2			Minor1			Major1			Major2				
Conflicting Flow All	-	130	1	1	1	-	1	0	0	129	0	0		
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-		
Stage 2	-	129	-	1	1	-	-	-	-	-	-	-		
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-		
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-		
Pot Cap-1 Maneuver	0	761	1084	1022	895	0	1622	-	-	1457	-	-		
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-		
Stage 2	0	789	-	1022	895	0	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	· -	761	1084	1022	895	-	1622	-	-	1457	-	-		
Mov Cap-2 Maneuver	· -	761	-	1022	895	-	-	-	-	-	-	-		
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-		
Stage 2	-	789	-	1022	895	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.4	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EB	Ln1W	/BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1457	-	-	
HCM Lane V/C Ratio	-	-	-	-	0.201	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.4	0	-	-	
HCM Lane LOS	А	-	-	А	А	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.7	0	-	-	

### SimTraffic Performance Report

87: Meridian Rd & Site RIRO Performance by movement Interval #1 7:00	
	_

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	10.6	0.5	0.1	0.1	0.9

## 87: Meridian Rd & Site RIRO Performance by movement Interval #2 7:15

Movement
/eh (s)

### 87: Meridian Rd & Site RIRO Performance by movement Interval #3 7:30

Movement
′eh (s)

#### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 7:45

#### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Novement	EBR	NBT	SBT	SBR	All
p Del/Veh (s)	11.3	0.4	0.1	0.1	0.

#### Total Zone Performance By Interval

Interval Start	7:00	7:15	7:30	7:45	All	
Stop Del/Veh (s)	58.7	42.2	61.8	47.8	191.3	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	- <b>†</b> †	1	ካካ	- <b>†</b> †	1	ሻሻ	<u></u>	1	ካካ	- <b>†</b> †	1
Traffic Volume (vph)	612	669	200	150	753	116	279	767	175	225	566	398
Future Volume (vph)	612	669	200	150	753	116	279	767	175	225	566	398
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	30.0	55.0		14.0	39.0	39.0	17.0	37.0		14.0	34.0	
Total Split (%)	25.0%	45.8%		11.7%	32.5%	32.5%	14.2%	30.8%		11.7%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	24.9	45.8	114.3	9.7	30.6	30.6	12.7	29.6	114.3	10.1	27.0	114.3
Actuated g/C Ratio	0.22	0.40	1.00	0.08	0.27	0.27	0.11	0.26	1.00	0.09	0.24	1.00
v/c Ratio	0.86	0.48	0.13	0.53	0.81	0.22	0.75	0.85	0.11	0.76	0.69	0.26
Control Delay	56.8	26.9	0.2	58.6	47.2	2.3	63.2	51.1	0.1	69.2	45.5	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.8	26.9	0.2	58.6	47.2	2.3	63.2	51.1	0.1	69.2	45.5	0.4
LOS	E	С	А	E	D	А	E	D	А	E	D	A
Approach Delay		35.9			43.8			46.5			34.9	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 114	4.3											
Natural Cycle: 75												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 3	39.9			I	ntersectio	n LOS: D						
Intersection Capacity Utilization	ation 82.3%	)		[(	CU Level	of Service	εE					
Analysis Period (min) 15												

Ø1	<b>→</b> <sub>Ø2</sub>		<b>▲</b> Ø3	↓ Ø4
14 s	55 s		17 s	34 s
<b>≯</b> <sub>∅5</sub>		<b>4</b> <sup>♠</sup> _ Ø6	Ø7	<b>1</b> Ø8
30 s		39 s	14 s 💦 🕺	37 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻ	<u>^</u>	1	ሻ	- <b>††</b>	1
Traffic Volume (vph)	262	113	134	40	68	150	187	1133	175	75	895	77
Future Volume (vph)	262	113	134	40	68	150	187	1133	175	75	895	77
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	15.0	24.0	24.0	12.0	21.0	21.0	22.0	72.0	72.0	12.0	62.0	62.0
Total Split (%)	12.5%	20.0%	20.0%	10.0%	17.5%	17.5%	18.3%	60.0%	60.0%	10.0%	51.7%	51.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	24.4	18.4	18.4	17.5	9.9	9.9	49.1	39.1	39.1	41.9	32.6	32.6
Actuated g/C Ratio	0.29	0.22	0.22	0.21	0.12	0.12	0.59	0.47	0.47	0.51	0.39	0.39
v/c Ratio	0.36	0.29	0.29	0.13	0.32	0.47	0.51	0.69	0.22	0.28	0.65	0.11
Control Delay	25.5	35.2	5.6	25.6	41.0	10.7	12.3	20.4	2.8	10.2	22.8	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.5	35.2	5.6	25.6	41.0	10.7	12.3	20.4	2.8	10.2	22.8	0.3
LOS	С	D	А	С	D	В	В	С	А	В	С	A
Approach Delay		22.5			21.0			17.2			20.2	
Approach LOS		С			С			В			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 82.	8											
Natural Cycle: 60												
Control Type: Actuated-Und	coordinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 1	9.3			lı	ntersectio	n LOS: B						
Intersection Capacity Utiliza	ation 60.9%			10	CU Level	of Service	θB					
Analysis Period (min) 15												
. ,												

Ø1	≪† <sub>Ø2</sub>	✓ Ø3
12 s	72 s	12 s 24 s
↑ ø5		▶ <sub>Ø7</sub> ♥ <sub>Ø8</sub>
22 s	62 s	15 s 21 s

Timings					
25: Golden	Sage	Rd 8	ß	Woodmen	Rd

	≯	-	$\mathbf{F}$	¥	-	•	1	1	۲	1	Ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	<u>۲</u>	<b>↑</b>	1	<u>۲</u>	<b>4</b>	
Traffic Volume (vph)	160	1515	85	15	1115	24	106	20	16	30	10	
Future Volume (vph)	160	1515	85	15	1115	24	106	20	16	30	10	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	18.0	82.0	82.0	10.0	74.0	74.0	15.0	18.0	18.0	10.0	13.0	
Total Split (%)	15.0%	68.3%	68.3%	8.3%	61.7%	61.7%	12.5%	15.0%	15.0%	8.3%	10.8%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
_ost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
_ead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
_ead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	57.5	52.3	52.3	49.1	40.4	40.4	19.0	13.5	13.5	12.5	7.9	
Actuated g/C Ratio	0.68	0.62	0.62	0.58	0.48	0.48	0.22	0.16	0.16	0.15	0.09	
v/c Ratio	0.46	0.71	0.09	0.07	0.68	0.03	0.41	0.07	0.04	0.14	0.57	
Control Delay	9.4	14.7	1.3	5.9	19.1	0.1	36.7	40.9	0.2	33.5	18.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	9.4	14.7	1.3	5.9	19.1	0.1	36.7	40.9	0.2	33.5	18.5	
LOS	А	В	А	А	В	А	D	D	А	С	В	
Approach Delay		13.5			18.5			33.1			20.9	
Approach LOS		В			В			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 84.9												
Natural Cycle: 65												
Control Type: Semi Act-Unco	oord											
Maximum v/c Ratio: 0.71												
Intersection Signal Delay: 16	6.6			Ir	ntersectio	n LOS: B						
Intersection Capacity Utilizat		)		10	CU Level	of Service	e D					
Analysis Period (min) 15												

🖌 Ø1 🕹 Ø2		▲ Ø3	Ø4
10 s 82 s		15 s	13 s
	₩ Ø6	Ø7	<b>≪</b> †ø8
18 s	74 s	10 s	18 s

4.5

#### Intersection

Int Delay, s/veh

•													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			÷			÷.	1		\$		
Traffic Vol, veh/h	0	0	0	187	0	0	0	0	204	0	0	0	
Future Vol, veh/h	0	0	0	187	0	0	0	0	204	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	95	95	95	95	92	95	92	95	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	0	0	197	0	0	0	0	215	0	0	0	

Major/Minor	Minor2		1	Vinor1			Major1		l	Major2				
Conflicting Flow All	-	216	1	1	1	-	1	0	0	215	0	0		
Stage 1	-	1	-	0	0	-	-	-	-	-	-	-		
Stage 2	-	215	-	1	1	-	-	-	-	-	-	-		
Critical Hdwy	-	6.52	6.22	7.12	6.52	-	4.12	-	-	4.12	-	-		
Critical Hdwy Stg 1	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	5.52	-	6.12	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	-	4.018	3.318	3.518	4.018	-	2.218	-	-	2.218	-	-		
Pot Cap-1 Maneuver	0	682	1084	1022	895	0	1622	-	-	1355	-	-		
Stage 1	0	895	-	-	-	0	-	-	-	-	-	-		
Stage 2	0	725	-	1022	895	0	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	• -	682	1084	1022	895	-	1622	-	-	1355	-	-		
Mov Cap-2 Maneuver	· -	682	-	1022	895	-	-	-	-	-	-	-		
Stage 1	-	895	-	-	-	-	-	-	-	-	-	-		
Stage 2	-	725	-	1022	895	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	9.4	0	0	
HCM LOS	А	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR EB	SLn1W	BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1622	-	-	-	1022	1355	-	-	
HCM Lane V/C Ratio	-	-	-	- (	0.193	-	-	-	
HCM Control Delay (s)	0	-	-	0	9.4	0	-	-	
HCM Lane LOS	А	-	-	Α	Α	Α	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	0.7	0	-	-	

### SimTraffic Performance Report

87: Meridian Rd & Site RIRO Performance by movement Interval #1 5:00	

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	7.2	0.8	0.1	0.1	1.1

## 87: Meridian Rd & Site RIRO Performance by movement Interval #2 5:15

Movement
op Del/Veh (s)

### 87: Meridian Rd & Site RIRO Performance by movement Interval #3 5:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	8.1	0.9	0.1	0.1	1.2

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 5:45

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Movement
Del/Veh (s)

#### Total Zone Performance By Interval

Interval Start	5:00	5:15	5:30	5:45	All	
Stop Del/Veh (s)	27.5	50.1	31.4	36.3	62.0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	- <b>†</b> †	1	ካካ	- <b>††</b>	1	ካካ	<u></u>	1	ካካ	- <b>††</b>	1
Traffic Volume (vph)	450	550	175	150	850	150	250	350	100	250	900	925
Future Volume (vph)	450	550	175	150	850	150	250	350	100	250	900	925
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	22.0	45.0		17.0	40.0	40.0	17.0	35.0		23.0	41.0	
Total Split (%)	18.3%	37.5%		14.2%	33.3%	33.3%	14.2%	29.2%		19.2%	34.2%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	17.9	39.5	116.4	11.2	32.9	32.9	12.6	31.6	116.4	15.0	34.0	116.4
Actuated g/C Ratio	0.15	0.34	1.00	0.10	0.28	0.28	0.11	0.27	1.00	0.13	0.29	1.00
v/c Ratio	0.88	0.47	0.11	0.46	0.87	0.27	0.69	0.37	0.06	0.58	0.89	0.60
Control Delay	67.9	32.3	0.1	55.1	50.4	5.1	61.2	36.6	0.1	53.5	51.3	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.9	32.3	0.1	55.1	50.4	5.1	61.2	36.6	0.1	53.5	51.3	1.7
LOS	E	С	А	E	D	А	E	D	Α	D	D	A
Approach Delay		41.2			45.1			40.2			29.4	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 116	6.4											
Natural Cycle: 80												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 37.1 Intersection LOS: D												
Intersection Capacity Utilization	ation 84.3%			10	CU Level	of Service	εE					
Analysis Period (min) 15												
o												

<b>√</b> Ø1	→ <sub>Ø2</sub>	<b>↑</b> Ø3	
17 s	45 s	17 s	41 s
<b>⋬</b> <sub>Ø5</sub>	<b>4</b> <sup>⊕</sup> Ø6	Ø7	<b>1</b> Ø8
22 s	40 s	23 s	35 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>†</b>	1	٦	<b>†</b>	1	ሻሻ	<u></u>	1	ሻ	- <b>†</b> †	1
Traffic Volume (vph)	36	25	52	250	55	125	155	645	150	100	1724	73
Future Volume (vph)	36	25	52	250	55	125	155	645	150	100	1724	73
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	15.0	17.0	17.0	18.0	20.0	20.0	20.0	75.0	75.0	10.0	65.0	65.0
Total Split (%)	12.5%	14.2%	14.2%	15.0%	16.7%	16.7%	16.7%	62.5%	62.5%	8.3%	54.2%	54.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	14.1	8.1	8.1	23.8	16.9	16.9	11.5	65.2	65.2	67.3	59.8	59.8
Actuated g/C Ratio	0.13	0.07	0.07	0.22	0.16	0.16	0.11	0.60	0.60	0.62	0.55	0.55
v/c Ratio	0.10	0.19	0.20	0.85	0.20	0.34	0.45	0.31	0.16	0.20	0.90	0.08
Control Delay	34.3	52.1	1.7	64.0	44.9	5.6	50.4	11.6	2.0	7.2	31.0	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.3	52.1	1.7	64.0	44.9	5.6	50.4	11.6	2.0	7.2	31.0	0.2
LOS	С	D	А	E	D	А	D	В	А	Α	С	A
Approach Delay		23.1			44.5			16.5			28.5	
Approach LOS		С			D			В			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 10	8.6											
Natural Cycle: 90												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.90												
Intersection Signal Delay:					ntersectio							
Intersection Capacity Utiliz Analysis Period (min) 15	ation 83.8%			10	CU Level	of Service	εE					

Ø1 Ø2	<b>√</b> Ø3	<b>↓</b> <sub>Ø4</sub>
10 s 75 s	18 s	17 s
▲ ø5 ● ø6	▶ <sub>Ø7</sub>	Ø8
20 s 65 s	15 s 20	)s

Timings	
25: Golden Sage Rd & Woodmen	Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>^</b>	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1
Traffic Volume (vph)	383	876	74	77	1764	184	150	16	48	251	21	299
Future Volume (vph)	383	876	74	77	1764	184	150	16	48	251	21	299
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.5	12.5	10.0	12.5	12.5	10.0	11.0	11.0	10.0	11.0	
Total Split (s)	19.0	74.0	74.0	10.0	65.0	65.0	20.0	13.0	13.0	23.0	16.0	
Total Split (%)	15.8%	61.7%	61.7%	8.3%	54.2%	54.2%	16.7%	10.8%	10.8%	19.2%	13.3%	
Yellow Time (s)	3.0	5.5	5.5	3.0	5.5	5.5	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.5	6.5	4.0	6.5	6.5	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	15.0	70.0	70.0	67.2	58.7	58.7	23.7	7.4	7.4	25.2	9.3	116.2
Actuated g/C Ratio	0.13	0.60	0.60	0.58	0.51	0.51	0.20	0.06	0.06	0.22	0.08	1.00
v/c Ratio	0.91	0.42	0.08	0.20	1.01	0.22	0.46	0.15	0.19	0.73	0.15	0.20
Control Delay	75.9	14.5	0.8	8.9	52.9	3.0	41.2	55.9	1.6	53.6	52.8	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.3	0.0	0.0
Total Delay	75.9	14.5	0.8	8.9	52.9	3.0	41.2	55.9	1.6	63.9	52.8	0.3
LOS	E	В	А	А	D	А	D	E	А	E	D	A
Approach Delay		31.7			46.6			33.4			30.1	
Approach LOS		С			D			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 116	5.2											
Natural Cycle: 90												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 1.01												
Intersection Signal Delay: 3	8.8			Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliza	Intersection Capacity Utilization 93.2% ICU Level of Service F											
Analysis Period (min) 15												

<b>√</b> Ø1	<b>→</b> Ø2	<b>1</b> Ø3	Ø4
10 s	74 s	20 s	16 s
	₩ Ø6	Ø7	- <b>1</b> 08
19 s	65 s	23 s	13 s

	-	4	-	1	1	
Lane Group	EBT	WBL	WBT	NBL	NBR	
Lane Configurations	4		र्स	۰Y	1	
Traffic Volume (vph)	27	320	2	100	483	
Future Volume (vph)	27	320	2	100	483	
Turn Type	NA	pm+pt	NA	Prot	Over	
Protected Phases	4	3	8	2	3	
Permitted Phases		8				
Detector Phase	4	3	8	2	3	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	10.0	30.0	40.0	80.0	30.0	
Total Split (%)	8.3%	25.0%	33.3%	66.7%	25.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0		-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag	Lead			Lead	
Lead-Lag Optimize?	Yes	Yes			Yes	
Recall Mode	None	None	None	Min	None	
Act Effct Green (s)	14.8		27.1	11.6	8.1	
Actuated g/C Ratio	0.31		0.57	0.25	0.17	
v/c Ratio	0.42		0.59	0.60	0.59	
Control Delay	5.1		9.9	14.1	8.9	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	5.1		9.9	14.2	8.9	
LOS	A		А	В	А	
Approach Delay	5.1		9.9	11.6		
Approach LOS	А		А	В		
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 47.2	2					
Natural Cycle: 45						
Control Type: Semi Act-Und	coord					
Maximum v/c Ratio: 0.60						
Intersection Signal Delay: 9	0.6			lr	ntersectio	LOS: A
Intersection Capacity Utiliza						of Service B
Analysis Period (min) 15						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> €Ø3	<b>→</b> Ø4
80 s	30 s	10 s
	<b>₩</b> Ø8	
	40 s	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>ሕ</u> ግ	- <b>†</b> †	1	ካካ	- <b>†</b> †	1	ሻሻ	<u></u>	1	ካካ	- <b>†</b> †	1
Traffic Volume (vph)	750	850	375	225	600	250	350	900	200	400	600	500
Future Volume (vph)	750	850	375	225	600	250	350	900	200	400	600	500
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	31.0	41.0		17.0	27.0	27.0	27.0	40.0		22.0	35.0	
Total Split (%)	25.8%	34.2%		14.2%	22.5%	22.5%	22.5%	33.3%		18.3%	29.2%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	27.0	36.1	118.7	12.4	21.5	21.5	18.5	33.7	118.7	17.5	32.7	118.7
Actuated g/C Ratio	0.23	0.30	1.00	0.10	0.18	0.18	0.16	0.28	1.00	0.15	0.28	1.00
v/c Ratio	0.99	0.81	0.24	0.64	0.95	0.52	0.67	0.91	0.13	0.81	0.63	0.32
Control Delay	76.6	45.4	0.4	59.9	74.6	9.2	53.5	55.6	0.2	62.5	41.7	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.6	45.4	0.4	59.9	74.6	9.2	53.5	55.6	0.2	62.5	41.7	0.5
LOS	E	D	А	E	E	А	D	Е	А	E	D	A
Approach Delay		48.8			56.3			47.4			33.5	
Approach LOS		D			E			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	8.7											
Natural Cycle: 90												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.99												
Intersection Signal Delay:	46.0			Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliz		ı		10	CU Level	of Service	εE					
Analysis Period (min) 15												

<b>√</b> Ø1	<b>→</b> Ø2		<b>▲</b> Ø3	<b>↓</b> Ø4
17 s	41 s		27 s	35 s
<b>⋬</b> <sub>Ø5</sub>		▲ <sup>4</sup>	Ø7	¶ø8
31 s		27 s	22 s	40 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	٦	<b>↑</b>	1	ካካ	- <b>†</b> †	1	ሻ	- <b>†</b> †	7
Traffic Volume (vph)	119	66	103	200	50	225	280	1370	250	100	1128	92
Future Volume (vph)	119	66	103	200	50	225	280	1370	250	100	1128	92
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	20.0	24.0	24.0	18.0	22.0	22.0	27.0	55.0	55.0	23.0	51.0	51.0
Total Split (%)	16.7%	20.0%	20.0%	15.0%	18.3%	18.3%	22.5%	45.8%	45.8%	19.2%	42.5%	42.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	19.5	10.2	10.2	26.9	14.1	14.1	14.9	48.3	48.3	53.3	42.5	42.5
Actuated g/C Ratio	0.20	0.10	0.10	0.27	0.14	0.14	0.15	0.49	0.49	0.54	0.43	0.43
v/c Ratio	0.21	0.36	0.40	0.57	0.20	0.55	0.57	0.81	0.29	0.43	0.75	0.13
Control Delay	29.0	48.9	10.7	37.1	41.6	10.9	44.4	26.3	2.9	17.4	28.2	3.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.0	48.9	10.7	37.1	41.6	10.9	44.4	26.3	2.9	17.4	28.2	3.5
LOS	С	D	В	D	D	В	D	С	А	В	С	A
Approach Delay		27.0			25.2			25.9			25.6	
Approach LOS		С			С			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 98.6												
Natural Cycle: 65												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 25.	8			lı	ntersectio	n LOS: C						
Intersection Capacity Utilization	on 72.4%			10	CU Level	of Service	ЭC					
Analysis Period (min) 15												

Ø1	Ø2	<b>√</b> Ø3	2 <sub>04</sub>
23 s	55 s	18 s	24 s
<b>▲</b> Ø5	<b>↓</b> Ø6		<b>4</b> ▼ Ø8
27 s	51 s	20 s	22 s

Timings	
25: Golden Sage Rd & Woodmen	Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>^</b>	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	<u>۲</u>	<b>↑</b>	1
Traffic Volume (vph)	329	1671	121	99	1030	329	152	35	114	191	22	408
Future Volume (vph)	329	1671	121	99	1030	329	152	35	114	191	22	408
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	25.0	75.0	75.0	10.0	60.0	60.0	20.0	17.0	17.0	18.0	15.0	
Total Split (%)	20.8%	62.5%	62.5%	8.3%	50.0%	50.0%	16.7%	14.2%	14.2%	15.0%	12.5%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	17.0	60.5	60.5	57.7	49.6	49.6	25.4	9.1	9.1	18.5	8.0	107.5
Actuated g/C Ratio	0.16	0.56	0.56	0.54	0.46	0.46	0.24	0.08	0.08	0.17	0.07	1.00
v/c Ratio	0.64	0.86	0.13	0.60	0.64	0.38	0.44	0.24	0.41	0.70	0.17	0.27
Control Delay	49.3	25.1	2.3	32.7	24.8	3.3	39.5	52.9	6.3	52.8	53.3	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.6	0.0	0.0
Total Delay	49.3	25.1	2.3	32.7	24.8	3.3	39.5	52.9	6.3	54.4	53.3	0.4
LOS	D	С	А	С	С	А	D	D	А	D	D	A
Approach Delay		27.6			20.4			28.5			18.9	
Approach LOS		С			С			С			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 107	.5											
Natural Cycle: 70												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 24	4.1			li	ntersectio	n LOS: C						
Intersection Capacity Utiliza	tion 80.6%	)		10	CU Level	of Service	ЭD					
Analysis Period (min) 15												

Ø1 → Ø2	<b>1</b> Ø3	₽ <sup>™</sup> Ø4
10 s 75 s	20 s	15 s
▶ø5 ♥ø6	Ø7	<b>1</b> 08
25 s 60 s	18 s	17 s

Protected Phases         4         3         8         2         3           Permitted Phases         8          3         8         2         3           Switch Phase          3         8         2         3           Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0           Minimum Split (s)         10.0         10.0         10.0         10.0         10.0         10.0           Total Split (s)         10.0         30.0         40.0         80.0         30.0         30.0         Total Split (s)         10.0		<b>→</b>	4	+	1	1	
Traffic Volume (vph)       19       446       8       300       393         Future Volume (vph)       19       446       8       300       393         Turn Type       NA       pm+pt       NA       Prot       Over         Protected Phases       4       3       8       2       3         Permitted Phases       8       0       10.0       10.0       10.0       10.0         Switch Phase       4       3       8       2       3       Switch Phase         Minimum Initial (s)       5.0       5.0       5.0       5.0       5.0       5.0         Switch Phase       40.0       10.0       10.0       10.0       10.0       10.0         Total Split (s)       10.0       30.0       40.0       80.0       30.0       30.0         All-Red Time (s)       2.0	Lane Group	EBT	WBL	WBT	NBL	NBR	
Traffic Volume (vph)       19       446       8       300       393         Future Volume (vph)       19       446       8       300       393         Protected Phases       4       3       8       2       3         Permitted Phases       8         Detector Phase       4       3       8       2       3         Switch Phase        10.0       10.	Lane Configurations	4		र्स	۰Y	1	
Turn Type         NA         pm+pt         NA         Prot         Over           Protected Phases         4         3         8         2         3           Permitted Phases         8         2         3         3         Switch Phase         3         8         2         3           Switch Phase         4         3         8         2         3         3         Switch Phase         3         8         2         3           Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0         Minimum Split (s)         10.0 <td></td> <td></td> <td>446</td> <td></td> <td></td> <td></td> <td></td>			446				
Protected Phases         4         3         8         2         3           Permitted Phases         8         8         2         3           Switch Phase         4         3         8         2         3           Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0           Minimum Split (s)         10.0         10.0         10.0         10.0         10.0         10.0           Total Split (s)         10.0         3.0         3.0         3.0         3.0         3.0           All-Red Time (s)         2.0         2.0         2.0         2.0         2.0         2.0           Lost Time Adjust (s)         -1.0         -1.0         -1.0         -1.0         -1.0           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead-Lag Optimize?         Yes         Yes         Yes         Yes         Yes           Recall Mode         None         None         None         Min         None           Act Effot Green (s)         23.8         36.2         19.3         8.3         Actuated g/C Ratio         0.37         0.57         0.30         0.13         Vic	Future Volume (vph)	-	446	-	300	393	
Permitted Phases         8           Detector Phase         4         3         8         2         3           Switch Phase	Turn Type	NA	pm+pt			Over	
Detector Phase         4         3         8         2         3           Switch Phase         Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0           Minimum Split (s)         10.0         10.0         10.0         10.0         10.0         10.0           Total Split (s)         10.0         30.0         40.0         80.0         30.0         Total Split (s)         3.0         3.0         3.0         3.0         3.0         Total Split (s)         3.0		4		8	2	3	
Switch Phase           Minimum Initial (s)         5.0         5.0         5.0         5.0           Minimum Split (s)         10.0         10.0         10.0         10.0           Total Split (s)         10.0         30.0         40.0         80.0         30.0           Total Split (s)         8.3%         25.0%         33.3%         66.7%         25.0%           Yellow Time (s)         3.0         3.0         3.0         3.0         3.0         3.0           All-Red Time (s)         2.0         2.0         2.0         2.0         2.0         2.0           Lost Time Adjust (s)         -1.0         -1.0         -1.0         -1.0         -1.0           Total Lost Time (s)         4.0         4.0         4.0         Lead/Lag         Lead/Lag         Lead         Lead/Lag         Lead         Lead/Lag         Lead         Lead/Lag         Lag/Lag/Lag/Lag/Lag/Lag/Lag/Lag/Lag/Lag/	Permitted Phases						
Minimum Initial (s)       5.0       5.0       5.0       5.0       5.0         Minimum Split (s)       10.0       10.0       10.0       10.0       10.0         Total Split (s)       10.0       30.0       40.0       80.0       30.0         Total Split (s)       3.0       3.0       3.0       3.0       3.0       3.0         Yellow Time (s)       2.0       2.0       2.0       2.0       2.0       2.0         Lost Time Adjust (s)       -1.0       -1.0       -1.0       -1.0       -1.0         Total Lost Time (s)       4.0       4.0       4.0       4.0       4.0         Lead-Lag       Lag       Lead       Lead       Lead       Lead         Lead-Lag Optimize?       Yes       Yes       Yes       Yes         Recall Mode       None       None       None       Min       None         Act Effct Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6 <td< td=""><td></td><td>4</td><td>3</td><td>8</td><td>2</td><td>3</td><td></td></td<>		4	3	8	2	3	
Minimum Split (s)       10.0       10.0       10.0       10.0       10.0         Total Split (s)       10.0       30.0       40.0       80.0       30.0         Total Split (s)       8.3%       25.0%       33.3%       66.7%       25.0%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0         All-Red Time (s)       2.0       2.0       2.0       2.0       2.0       2.0         Lost Time Adjust (s)       -1.0       -1.0       -1.0       -1.0       -1.0         Total Lost Time (s)       4.0       4.0       4.0       4.0         Lead/Lag       Lag       Lead       Lead         Lead-Lag Optimize?       Yes       Yes       Yes         Recall Mode       None       None       None       Min       None         Act Effet Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13       v/c Ratio       0.00       0.0       0.0         Queue Delay       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0							
Total Split (s)       10.0       30.0       40.0       80.0       30.0         Total Split (%)       8.3%       25.0%       33.3%       66.7%       25.0%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0         All-Red Time (s)       2.0       2.0       2.0       2.0         Lost Time Adjust (s)       -1.0       -1.0       -1.0       -1.0         Total Lost Time (s)       4.0       4.0       4.0       4.0         Lead/Lag       Lag       Lead       Lead         Lead-Lag Optimize?       Yes       Yes       Yes         Recall Mode       None       None       Min       None         Act Effct Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0       1.6         LOS       A       B       B       B       1.6         LOS       A       B       B	Minimum Initial (s)						
Total Split (%)         8.3%         25.0%         33.3%         66.7%         25.0%           Yellow Time (s)         3.0         3.0         3.0         3.0         3.0         3.0           All-Red Time (s)         2.0         2.0         2.0         2.0         2.0         2.0           Lost Time Adjust (s)         -1.0         -1.0         -1.0         -1.0         -1.0           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lead         Lead         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach LOS         A         B         B         B	Minimum Split (s)						
Yellow Time (s)       3.0       3.0       3.0       3.0       3.0         All-Red Time (s)       2.0       2.0       2.0       2.0       2.0         Lost Time Adjust (s)       -1.0       -1.0       -1.0       -1.0         Total Lost Time (s)       4.0       4.0       4.0       4.0         Lead/Lag       Lag       Lead       Lead         Lead-Lag Optimize?       Yes       Yes       Yes         Recall Mode       None       None       None       Min         Act Effct Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       5.5       19.2       26.3       11.6         LOS       A       B       C       B         Approach LOS       A       B       B       Intersection Signal Delay         Cycle Length: 120       Actuated Cycle Length: 63.5       Natural Cycle: 50       Cont							
All-Red Time (s)       2.0       2.0       2.0       2.0         Lost Time Adjust (s)       -1.0       -1.0       -1.0       -1.0         Total Lost Time (s)       4.0       4.0       4.0       4.0         Lead/Lag       Lag       Lead       Lead         Lead-Lag Optimize?       Yes       Yes       Yes         Recall Mode       None       None       None       Min         Act Effct Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       5.5       19.2       26.3       11.6         LOS       A       B       C       B         Approach Delay       5.5       19.2       19.3         Approach LOS       A       B       B         Intersection Summary       Cycle Length: 63.5       19.2       19.3         Cycle Length: 120       Actuated Cycle Length: 63.5       14.5<							
Lost Time Adjust (s)         -1.0         -1.0         -1.0         -1.0           Total Lost Time (s)         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lead         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3         Approach LOS         A           A         B         B         B         B         Intersection Summary         Sintersection Signal Delay: 17.3         Intersection LOS: B           Notesetio	Yellow Time (s)						
Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lead         Lead         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3         Approach LOS         A           Approach LOS         A         B         B         B         Intersection Summary           Cycle Length: 120         Actuated Cycle Length: 63.5         Natural Cycle: 50         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76<	All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lead/Lag         Lag         Lead         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3           Approach LOS         A         B         B           Intersection Summary         Cycle Length: 120         Actuated Cycle Length: 63.5         Natural Cycle: 50           Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection LOS: B         Intersection LOS: B           Intersection Signal Delay: 17.3         Intersection LOS: B         ICU Level of Service C <td>Lost Time Adjust (s)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Lost Time Adjust (s)						
Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3           Approach LOS         A         B         B           Intersection Summary         Cycle Length: 120         Actuated Cycle Length: 63.5         Natural Cycle: 50           Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection LOS: B           Intersection Signal Delay: 17.3         Intersection LOS: B         Intersection LOS: B	Total Lost Time (s)	4.0		4.0	4.0	4.0	
Recall Mode         None         None         None         Min         None           Act Effct Green (s)         23.8         36.2         19.3         8.3           Actuated g/C Ratio         0.37         0.57         0.30         0.13           v/c Ratio         0.28         0.76         0.71         0.70           Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3           Approach LOS         A         B         B           Intersection Summary         Cycle Length: 120         Actuated Cycle Length: 63.5         Natural Cycle: 50           Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection LOS: B         Intersection LOS: B           Intersection Signal Delay: 17.3         Intersection LOS: B         ICU Level of Service C	Lead/Lag		Lead			Lead	
Act Effct Green (s)       23.8       36.2       19.3       8.3         Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       5.5       19.2       26.3       11.6         LOS       A       B       C       B         Approach Delay       5.5       19.2       19.3         Approach Delay       5.5       19.2       19.3         Approach LOS       A       B       B         Intersection Summary       Cycle Length: 120       Actuated Cycle Length: 63.5         Natural Cycle: 50       Control Type: Semi Act-Uncoord       Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3       Intersection LOS: B         Intersection Capacity Utilization 71.6%       ICU Level of Service C	Lead-Lag Optimize?	Yes	Yes			Yes	
Actuated g/C Ratio       0.37       0.57       0.30       0.13         v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       5.5       19.2       26.3       11.6         LOS       A       B       C       B         Approach Delay       5.5       19.2       19.3         Approach LOS       A       B       B         Intersection Summary       Cycle Length: 120         Actuated Cycle Length: 63.5       Natural Cycle: 50         Control Type: Semi Act-Uncoord       Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3       Intersection LOS: B         Intersection Capacity Utilization 71.6%       ICU Level of Service C	Recall Mode	None	None	None	Min		
v/c Ratio       0.28       0.76       0.71       0.70         Control Delay       5.5       19.2       26.3       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       5.5       19.2       26.3       11.6         LOS       A       B       C       B         Approach Delay       5.5       19.2       19.3         Approach LOS       A       B       B         Intersection Summary       Cycle Length: 120         Actuated Cycle Length: 63.5       Natural Cycle: 50         Control Type: Semi Act-Uncoord       Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3       Intersection LOS: B         Intersection Capacity Utilization 71.6%       ICU Level of Service C	Act Effct Green (s)	23.8		36.2	19.3	8.3	
Control Delay         5.5         19.2         26.3         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3           Approach LOS         A         B         B           Intersection Summary         Cycle Length: 120         Actuated Cycle Length: 63.5           Natural Cycle: 50         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76           Intersection Signal Delay: 17.3         Intersection LOS: B           Intersection Capacity Utilization 71.6%         ICU Level of Service C	Actuated g/C Ratio						
Queue Delay         0.0         0.0         0.0         0.0           Total Delay         5.5         19.2         26.3         11.6           LOS         A         B         C         B           Approach Delay         5.5         19.2         19.3           Approach LOS         A         B         B           Intersection Summary         Cycle Length: 120         Actuated Cycle Length: 63.5         Natural Cycle: 50           Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection LOS: B         Intersection LOS: B           Intersection Capacity Utilization 71.6%         ICU Level of Service C         ICU Level of Service C	v/c Ratio			0.76	0.71	0.70	
Total Delay5.519.226.311.6LOSABCBApproach Delay5.519.219.3Approach LOSABBIntersection SummaryCycle Length: 120Actuated Cycle Length: 63.5Actuated Cycle Length: 63.5Natural Cycle: 50Control Type: Semi Act-UncoordMaximum v/c Ratio: 0.76Intersection LOS: BIntersection Capacity Utilization 71.6%ICU Level of Service C	Control Delay			19.2	26.3	11.6	
LOSABCBApproach Delay5.519.219.3Approach LOSABBIntersection SummaryCycle Length: 120Actuated Cycle Length: 63.5Natural Cycle: 50Control Type: Semi Act-UncoordMaximum v/c Ratio: 0.76Intersection Signal Delay: 17.3Intersection Capacity Utilization 71.6%ICU Level of Service C	Queue Delay	0.0					
Approach Delay5.519.219.3Approach LOSABBIntersection SummaryCycle Length: 120Actuated Cycle Length: 63.5Natural Cycle: 50Control Type: Semi Act-UncoordMaximum v/c Ratio: 0.76Intersection Signal Delay: 17.3Intersection Capacity Utilization 71.6%ICU Level of Service C						11.6	
Approach LOS       A       B       B         Intersection Summary	LOS					В	
Intersection Summary Cycle Length: 120 Actuated Cycle Length: 63.5 Natural Cycle: 50 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.76 Intersection Signal Delay: 17.3 Intersection LOS: B Intersection Capacity Utilization 71.6% ICU Level of Service C	Approach Delay			19.2	19.3		
Cycle Length: 120 Actuated Cycle Length: 63.5 Natural Cycle: 50 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.76 Intersection Signal Delay: 17.3 Intersection LOS: B Intersection Capacity Utilization 71.6%	Approach LOS	А		В	В		
Actuated Cycle Length: 63.5 Natural Cycle: 50 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.76 Intersection Signal Delay: 17.3 Intersection Capacity Utilization 71.6% ICU Level of Service C	Intersection Summary						
Natural Cycle: 50         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3         Intersection Capacity Utilization 71.6%         ICU Level of Service C	Cycle Length: 120						
Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3         Intersection Capacity Utilization 71.6%         ICU Level of Service C	Actuated Cycle Length: 63.5	5					
Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 0.76         Intersection Signal Delay: 17.3         Intersection Capacity Utilization 71.6%         ICU Level of Service C							
Maximum v/c Ratio: 0.76Intersection Signal Delay: 17.3Intersection Capacity Utilization 71.6%ICU Level of Service C		coord					
Intersection Capacity Utilization 71.6% ICU Level of Service C							
Intersection Capacity Utilization 71.6% ICU Level of Service C	Intersection Signal Delay: 1	7.3			lr	ntersection	LOS: B
					10	CU Level of	Service C
	Analysis Period (min) 15						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> €Ø3	<b>→</b> Ø4
80 s	30 s	10 s
	<b>₩</b> Ø8	
	40 s	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	አካ	<u></u>	1	ኘ	<u></u>	1	ሻሻ	<u></u>	1	ካካ	<u></u>	1
Traffic Volume (vph)	450	550	175	150	860	140	282	318	100	250	900	1012
Future Volume (vph)	450	550	175	150	860	140	282	318	100	250	900	1012
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	22.0	45.0		17.0	40.0	40.0	17.0	35.0		23.0	41.0	
Total Split (%)	18.3%	37.5%		14.2%	33.3%	33.3%	14.2%	29.2%		19.2%	34.2%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	17.9	39.7	116.9	11.3	33.1	33.1	12.8	31.9	116.9	15.0	34.1	116.9
Actuated g/C Ratio	0.15	0.34	1.00	0.10	0.28	0.28	0.11	0.27	1.00	0.13	0.29	1.00
v/c Ratio	0.88	0.47	0.11	0.46	0.88	0.25	0.77	0.34	0.06	0.58	0.89	0.65
Control Delay	68.6	32.4	0.1	55.3	51.3	4.1	65.7	36.1	0.1	53.7	51.7	2.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	68.6	32.4	0.1	55.3	51.3	4.1	65.7	36.1	0.1	53.7	51.7	2.1
LOS	E	С	А	E	D	А	E	D	А	D	D	A
Approach Delay		41.5			46.1			42.9			28.7	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	6.9											
Natural Cycle: 90												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 3	37.4			lı	ntersectio	n LOS: D						
Intersection Capacity Utiliz				10	CU Level	of Service	εE					
Analysis Period (min) 15												
Onlite and Dhannes 4. Mr												

Ø1	<b>→</b> <sub>Ø2</sub>	<b>1</b> Ø3	<b>↓</b> Ø4
17 s	45 s	17 s	41 s
<b>⋬</b> <sub>Ø5</sub>	Ø6	Ø7	¶ø8
22 s	40 s	23 s	35 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1
Traffic Volume (vph)	36	25	52	250	55	125	112	645	150	100	1798	29
Future Volume (vph)	36	25	52	250	55	125	112	645	150	100	1798	29
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	15.0	17.0	17.0	18.0	20.0	20.0	16.0	75.0	75.0	10.0	69.0	69.0
Total Split (%)	12.5%	14.2%	14.2%	15.0%	16.7%	16.7%	13.3%	62.5%	62.5%	8.3%	57.5%	57.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	14.0	8.1	8.1	23.7	16.9	16.9	10.0	65.7	65.7	69.3	61.7	61.7
Actuated g/C Ratio	0.13	0.07	0.07	0.22	0.15	0.15	0.09	0.60	0.60	0.64	0.57	0.57
v/c Ratio	0.10	0.19	0.20	0.85	0.20	0.34	0.37	0.31	0.16	0.20	0.92	0.03
Control Delay	35.2	53.0	1.7	65.5	45.9	5.6	51.9	11.5	2.0	6.9	31.0	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.2	53.0	1.7	65.5	45.9	5.6	51.9	11.5	2.0	6.9	31.0	0.1
LOS	D	D	А	E	D	А	D	В	А	А	С	A
Approach Delay		23.6			45.6			15.0			29.2	
Approach LOS		С			D			В			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 10	9.1											
Natural Cycle: 90												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.92												
Intersection Signal Delay:				li	ntersectio	n LOS: C						
Intersection Capacity Utiliz	ation 85.6%			10	CU Level	of Service	εE					
Analysis Period (min) 15												

Ø1	¶ø₂	<b>√</b> Ø3	<b>↓</b> <sub>Ø4</sub>
10 s	75 s	18 s	17 s
▲ ø5	<b>€</b> Ø6	▶ <sub>Ø7</sub> ·	Ø8
16 s	69 s	15 s 20	)s

Timings				
25: Golden	Sage Rd	&	Woodmen	Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>^</b>	1	ሻ	- <b>†</b> †	1	<u>۲</u>	<b>↑</b>	1	<u>۲</u>	<b>↑</b>	1
Traffic Volume (vph)	382	877	74	77	1764	116	150	16	48	251	21	299
Future Volume (vph)	382	877	74	77	1764	116	150	16	48	251	21	299
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.5	12.5	10.0	12.5	12.5	10.0	11.0	11.0	10.0	11.0	
Total Split (s)	19.0	74.0	74.0	10.0	65.0	65.0	20.0	13.0	13.0	23.0	16.0	
Total Split (%)	15.8%	61.7%	61.7%	8.3%	54.2%	54.2%	16.7%	10.8%	10.8%	19.2%	13.3%	
Yellow Time (s)	3.0	5.5	5.5	3.0	5.5	5.5	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.5	6.5	4.0	6.5	6.5	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	15.0	70.0	70.0	67.2	58.7	58.7	23.7	7.4	7.4	25.2	9.3	116.2
Actuated g/C Ratio	0.13	0.60	0.60	0.58	0.51	0.51	0.20	0.06	0.06	0.22	0.08	1.00
v/c Ratio	0.91	0.42	0.08	0.20	1.01	0.14	0.46	0.15	0.19	0.73	0.15	0.20
Control Delay	75.5	14.5	0.8	8.9	52.9	1.3	41.2	55.9	1.6	53.6	52.8	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.3	0.0	0.0
Total Delay	75.5	14.5	0.8	8.9	52.9	1.3	41.2	55.9	1.6	63.9	52.8	0.3
LOS	E	В	А	А	D	А	D	E	А	E	D	A
Approach Delay		31.5			48.0			33.4			30.1	
Approach LOS		С			D			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 116	.2											
Natural Cycle: 90												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 1.01												
Intersection Signal Delay: 3				Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliza	tion 93.1%	)		10	CU Level	of Service	θF					
Analysis Period (min) 15												

Ø1 -	02	<b>↑</b> ø3	<b>₽</b> Ø4
10 s 74 s		20 s	16 s
	₩ Ø6	Ø7	<b>1</b> 08
19 s	65 s	23 s	13 s

	-	1	+	1	1	
Lane Group	EBT	WBL	WBT	NBL	NBR	
Lane Configurations	4		र्स	Y	1	
Traffic Volume (vph)	27	320	2	100	414	
Future Volume (vph)	27	320	2	100	414	
Turn Type	NA	pm+pt	NA	Prot	Over	
Protected Phases	4	3	8	2	3	
Permitted Phases		8				
Detector Phase	4	3	8	2	3	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	10.0	30.0	40.0	80.0	30.0	
Total Split (%)	8.3%	25.0%	33.3%	66.7%	25.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0		-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag	Lead			Lead	
Lead-Lag Optimize?	Yes	Yes			Yes	
Recall Mode	None	None	None	Min	None	
Act Effct Green (s)	14.8		27.0	11.2	8.0	
Actuated g/C Ratio	0.32		0.58	0.24	0.17	
v/c Ratio	0.42		0.58	0.55	0.56	
Control Delay	4.9		9.6	14.4	8.7	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	4.9		9.6	14.4	8.7	
LOS	А		А	В	А	
Approach Delay	4.9		9.6	11.6		
Approach LOS	А		А	В		
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 46	.7					
Natural Cycle: 45						
Control Type: Semi Act-Un	ncoord					
Maximum v/c Ratio: 0.58						
Intersection Signal Delay:	9.4			Ir	ntersection	LOS: A
Intersection Capacity Utiliz					CU Level o	
Analysis Period (min) 15						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> Ø2	<b>€</b> ¶Ø3	<b>₩</b> Ø4
80 s	30 s	10 s
	<b>√</b> Ø8	
	40 s	

# Timings 1: Meridian Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	<u>ሕ</u> ግ	<u></u>	1	ካካ	- <b>†</b> †	1	ካካ	<u></u>	1	ካካ	<u></u>	7
Traffic Volume (vph)	750	850	375	225	615	235	403	847	200	400	600	555
Future Volume (vph)	750	850	375	225	615	235	403	847	200	400	600	555
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	32.0	44.0		17.0	29.0	29.0	25.0	37.0		22.0	34.0	
Total Split (%)	26.7%	36.7%		14.2%	24.2%	24.2%	20.8%	30.8%		18.3%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	28.0	39.0	119.2	12.5	23.4	23.4	19.3	31.2	119.2	17.5	29.5	119.2
Actuated g/C Ratio	0.23	0.33	1.00	0.10	0.20	0.20	0.16	0.26	1.00	0.15	0.25	1.00
v/c Ratio	0.97	0.75	0.24	0.64	0.90	0.48	0.74	0.93	0.13	0.81	0.70	0.36
Control Delay	70.2	40.9	0.4	60.1	64.4	8.6	56.3	60.5	0.2	62.8	46.2	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.2	40.9	0.4	60.1	64.4	8.6	56.3	60.5	0.2	62.8	46.2	0.6
LOS	E	D	А	E	E	А	E	E	А	E	D	A
Approach Delay		44.5			51.3			51.0			34.2	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	9.2											
Natural Cycle: 90												
Control Type: Actuated-Ur	ncoordinated											
Maximum v/c Ratio: 0.97												
Intersection Signal Delay:					ntersectio							
Intersection Capacity Utiliz	ation 89.5%	)		10	CU Level	of Service	εE					
Analysis Period (min) 15												

Splits and Phases: 1: Meridian Rd & Woodmen Rd

<b>√</b> Ø1	→ <sub>Ø2</sub>		<b>1</b> Ø3		Ø4
17 s	44 s		25 s	34	s
<b>★</b> <sub>Ø5</sub>			Ø7	_ <b>†</b> ∉	08
32 s		29 s	22 s	37 s	

# Timings 2: Meridian Rd & Eastonville Rd

2040 Background	Traffic WITH	<b>Right-in Access</b>
		PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ካካ		1	ሻ	- <b>†</b> †	1
Traffic Volume (vph)	119	66	103	200	50	225	212	1370	250	100	1172	67
Future Volume (vph)	119	66	103	200	50	225	212	1370	250	100	1172	67
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	21.0	27.0	27.0	16.0	22.0	22.0	24.0	54.0	54.0	23.0	53.0	53.0
Total Split (%)	17.5%	22.5%	22.5%	13.3%	18.3%	18.3%	20.0%	45.0%	45.0%	19.2%	44.2%	44.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	19.5	10.2	10.2	24.4	12.6	12.6	12.8	48.1	48.1	55.1	44.5	44.5
Actuated g/C Ratio	0.20	0.11	0.11	0.25	0.13	0.13	0.13	0.50	0.50	0.57	0.46	0.46
v/c Ratio	0.21	0.35	0.39	0.60	0.22	0.58	0.49	0.79	0.29	0.43	0.74	0.09
Control Delay	29.0	47.1	10.4	38.6	42.3	11.7	44.0	25.0	2.8	16.1	25.3	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.0	47.1	10.4	38.6	42.3	11.7	44.0	25.0	2.8	16.1	25.3	1.4
LOS	С	D	В	D	D	В	D	С	А	В	С	A
Approach Delay		26.5			26.2			24.1			23.4	
Approach LOS		С			С			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 96.8												
Natural Cycle: 65												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 24.	3			lı	ntersectio	n LOS: C						
Intersection Capacity Utilization	on 72.4%			[(	CU Level	of Service	ЭC					
Analysis Period (min) 15												
- , ,												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1	Ø2	<b>√</b> Ø3	<b>₩</b> Ø4
23 s	54 s	16 s	27 s
<b>▲</b> ø5	v Ø6		₹ Ø8
24 s	53 s	21 s	22 s

Timings				
25: Golden	Sage Ro	۱&	Woodmen	Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	•	1	٦	•	1
Traffic Volume (vph)	324	1676	121	99	1030	300	152	35	114	191	26	408
Future Volume (vph)	324	1676	121	99	1030	300	152	35	114	191	26	408
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	25.0	75.0	75.0	10.0	60.0	60.0	20.0	17.0	17.0	18.0	15.0	
Total Split (%)	20.8%	62.5%	62.5%	8.3%	50.0%	50.0%	16.7%	14.2%	14.2%	15.0%	12.5%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	16.9	60.6	60.6	58.0	49.8	49.8	25.6	9.2	9.2	18.6	8.2	107.8
Actuated g/C Ratio	0.16	0.56	0.56	0.54	0.46	0.46	0.24	0.09	0.09	0.17	0.08	1.00
v/c Ratio	0.64	0.86	0.13	0.61	0.64	0.35	0.44	0.23	0.41	0.70	0.19	0.27
Control Delay	49.5	25.4	2.3	33.3	24.8	3.2	39.5	52.7	6.2	52.9	53.7	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.5	0.0	0.0
Total Delay	49.5	25.4	2.3	33.3	24.8	3.2	39.5	52.7	6.2	54.4	53.7	0.4
LOS	D	С	А	С	С	А	D	D	А	D	D	Α
Approach Delay		27.8			20.8			28.4			19.1	
Approach LOS		С			С			С			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 107	.8											
Natural Cycle: 75												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 24	4.4			lı	ntersectio	n LOS: C						
Intersection Capacity Utiliza	tion 80.7%	)		10	CU Level	of Service	эD					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

✓ Ø1 → Ø2	<b>▲</b> Ø3	<b>₽</b> Ø4
10 s 75 s	20 s	15 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>	Ø7	- Ø8
25 s 60 s	18 s	17 s

Lane GroupEBTWBLWBTNBLNBRLane ConfigurationsImage: Additional systemImage: Additional systemImage: Additional systemImage: Additional systemTraffic Volume (vph)194468300359Future Volume (vph)194468300359Turn TypeNApm+ptNAProtOverProtected Phases43823Permitted Phases8Image: Additional systemImage: Additional systemImage: Additional systemDetector Phase43823
Traffic Volume (vph)         19         446         8         300         359           Future Volume (vph)         19         446         8         300         359           Turn Type         NA         pm+pt         NA         Prot         Over           Protected Phases         4         3         8         2         3           Permitted Phases         8         8         2         3
Traffic Volume (vph)         19         446         8         300         359           Future Volume (vph)         19         446         8         300         359           Turn Type         NA         pm+pt         NA         Prot         Over           Protected Phases         4         3         8         2         3           Permitted Phases         8         8         2         3
Turn TypeNA pm+ptNA ProtOverProtected Phases43823Permitted Phases8
Protected Phases 4 3 8 2 3 Permitted Phases 8
Permitted Phases 8
Detector Phase 4 3 8 2 3
Switch Phase
Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0
Minimum Split (s) 10.0 10.0 10.0 10.0 10.0
Total Split (s) 10.0 30.0 40.0 80.0 30.0
Total Split (%) 8.3% 25.0% 33.3% 66.7% 25.0%
Yellow Time (s) 3.0 3.0 3.0 3.0 3.0
All-Red Time (s) 2.0 2.0 2.0 2.0 2.0
Lost Time Adjust (s) -1.0 -1.0 -1.0 -1.0
Total Lost Time (s) 4.0 4.0 4.0 4.0
Lead/Lag Lag Lead Lead
Lead-Lag Optimize? Yes Yes Yes
Recall Mode None None None Min None
Act Effct Green (s) 23.9 36.2 18.6 8.2
Actuated g/C Ratio 0.38 0.58 0.30 0.13
v/c Ratio 0.28 0.75 0.69 0.68
Control Delay 5.3 18.3 26.2 11.5
Queue Delay 0.0 0.0 0.0 0.0
Total Delay 5.3 18.3 26.2 11.5
LOS A B C B
Approach Delay 5.3 18.3 19.2
Approach LOS A B B
Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 62.8
Natural Cycle: 45
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.75
Intersection Signal Delay: 16.8 Intersection LOS: B
Intersection Capacity Utilization 70.9% ICU Level of Service C
Analysis Period (min) 15

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> €Ø3	<b>-</b> Ø4
80 s	30 s	10 s
	<b>₩</b> Ø8	
	40 s	

# Timings 1: Meridian Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u></u>	1	ካካ	- <b>†</b> †	1	ካካ	<u></u>	1	ካካ	<u></u>	1
Traffic Volume (vph)	464	520	175	150	804	248	250	422	100	294	941	929
Future Volume (vph)	464	520	175	150	804	248	250	422	100	294	941	929
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	22.0	45.0		17.0	40.0	40.0	17.0	35.0		23.0	41.0	
Total Split (%)	18.3%	37.5%		14.2%	33.3%	33.3%	14.2%	29.2%		19.2%	34.2%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	18.1	39.3	116.6	11.3	32.5	32.5	12.5	30.9	116.6	16.1	34.5	116.6
Actuated g/C Ratio	0.16	0.34	1.00	0.10	0.28	0.28	0.11	0.27	1.00	0.14	0.30	1.00
v/c Ratio	0.90	0.45	0.11	0.46	0.83	0.41	0.69	0.46	0.06	0.63	0.92	0.60
Control Delay	70.1	32.1	0.1	55.1	48.1	6.1	61.4	38.6	0.1	54.1	54.4	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.1	32.1	0.1	55.1	48.1	6.1	61.4	38.6	0.1	54.1	54.4	1.7
LOS	E	С	А	E	D	А	E	D	А	D	D	A
Approach Delay		42.6			40.3			41.0			31.7	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	6.6											
Natural Cycle: 80												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.92												
Intersection Signal Delay: 3	37.4			li	ntersectio	n LOS: D						
Intersection Capacity Utiliz	ation 84.6%			10	CU Level	of Service	εE					
Analysis Period (min) 15												
Calite and Dhasses 1. M		\ <b>A</b> /= = -  =										

Splits and Phases: 1: Meridian Rd & Woodmen Rd

Ø1	<b>→</b> <sub>Ø2</sub>	<b>1</b> Ø3	▼ Ø4
17 s	45 s	17 s	41 s
<b>⋬</b> <sub>Ø5</sub>	<b>4</b> <sup>♠</sup> Ø6	Ø7	¶ø8
22 s	40 s	23 s	35 s

# Timings 2: Meridian Rd & Eastonville Rd

Lane Group         EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR         SBI           Lane Configurations         1	↑↑↑ 1722 1722 1722 t NA 6 6 6 6 6 0 5.0 0 11.5 65.0 0 54.2% 0 4.5	SBR 141 141 Perm 6 6 6 5.0 11.5 65.0 54.2% 4.5
Traffic Volume (vph)       122       73       138       250       113       125       387       596       150       100         Future Volume (vph)       122       73       138       250       113       125       387       596       150       100         Turn Type       pm+pt       NA       Perm       Perm       PM+pt       NA       Perm       PA       A       B       B       C       C       C       C       C       C       C       <	<ul> <li>1722</li> <li>1722</li> <li>1722</li> <li>1722</li> <li>NA</li> <li>6</li> <li>6</li> <li>5.0</li> <li>11.5</li> <li>65.0</li> <li>54.2%</li> <li>4.5</li> </ul>	141 141 Perm 6 6 6 5.0 11.5 65.0 54.2%
Future Volume (vph)         122         73         138         250         113         125         387         596         150         100           Turn Type         pm+pt         NA         Perm         pm+pt         NA         Perm         Prot         NA         Perm         pm+pt         NA         Perm         Prot         NA         Perm         pm+pt           Protected Phases         7         4         4         8         8         5         2         7           Detector Phase         7         4         4         3         8         5         2         2         7           Switch Phase         7         4         4         3         8         5         2         2         7           Minimum Initial (s)         5.0	<ul> <li>1722</li> <li>NA</li> <li>6</li> <li>6</li> <li>5.0</li> <li>11.5</li> <li>65.0</li> <li>54.2%</li> <li>4.5</li> </ul>	141 Perm 6 6 5.0 11.5 65.0 54.2%
Turn Type         pm+pt         NA         Perm         pm+pt         NA         Perm         Prot         NA         Perm         pm+pt           Protected Phases         7         4         3         8         5         2         7           Permitted Phases         4         4         8         8         5         2         7           Switch Phase         7         4         4         3         8         8         5         2         2         7           Switch Phase         7         4         4         3         8         8         5         2         2         7           Minimum Initial (s)         5.0	t NA 6 6 9 5.0 9 5.0 9 11.5 9 65.0 9 54.2% 9 4.5	Perm 6 5.0 11.5 65.0 54.2%
Protected Phases         7         4         3         8         5         2           Permitted Phases         4         4         8         8         2         6           Detector Phase         7         4         4         3         8         8         5         2         2           Switch Phase         7         4         4         3         8         8         5         2         2           Minimum Initial (s)         5.0	6 5.0 5.0 11.5 65.0 54.2% 54.2%	6 5.0 11.5 65.0 54.2%
Permitted Phases         4         4         8         8         2         6           Detector Phase         7         4         4         3         8         8         5         2         2         5           Switch Phase          5.0	6 5.0 11.5 65.0 54.2% 4.5	6 5.0 11.5 65.0 54.2%
Detector Phase         7         4         4         3         8         8         5         2         2           Switch Phase           Minimum Initial (s)         5.0         6.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0         3.0	6 5.0 11.5 65.0 54.2% 4.5	5.0 11.5 65.0 54.2%
Switch Phase         Minimum Initial (s)         5.0         6.0	5.0 11.5 65.0 54.2% 4.5	5.0 11.5 65.0 54.2%
Minimum Initial (s)         5.0         6.0         0.0	<ul> <li>11.5</li> <li>65.0</li> <li>54.2%</li> <li>4.5</li> </ul>	11.5 65.0 54.2%
Minimum Split (s)         10.0         11.5         11.5         10.0         10.5         11.5         10.0         11.5         11.5         10.0         11.5         11.5         10.0         11.5         11.5         10.0         11.5         11.5         10.0         11.5         11.5         10.0         11.5         11.5         11.5         10.0         11.5         11.5 <td><ul> <li>11.5</li> <li>65.0</li> <li>54.2%</li> <li>4.5</li> </ul></td> <td>11.5 65.0 54.2%</td>	<ul> <li>11.5</li> <li>65.0</li> <li>54.2%</li> <li>4.5</li> </ul>	11.5 65.0 54.2%
Total Split (s)       15.0       17.0       17.0       18.0       20.0       20.0       75.0       75.0       10.0         Total Split (%)       12.5%       14.2%       14.2%       15.0%       16.7%       16.7%       16.7%       62.5%       62.5%       8.3%         Yellow Time (s)       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       3.0       4.5       4.5       3.0         All-Red Time (s)       2.0	65.0 54.2% 4.5	65.0 54.2%
Total Split (%)       12.5%       14.2%       15.0%       16.7%       16.7%       16.7%       62.5%       62.5%       8.3%         Yellow Time (s)       3.0 <t< td=""><td>54.2% 4.5</td><td>54.2%</td></t<>	54.2% 4.5	54.2%
Yellow Time (s)       3.0	4.5	
All-Red Time (s)       2.0 <td></td> <td>4.5</td>		4.5
Lost Time Adjust (s)-1.0 <th< td=""><td>20</td><td></td></th<>	20	
Total Lost Time (s)       4.0       4.0       4.0       4.0       4.0       4.0       4.0       4.0       4.0       5.5       5.5       4.0         Lead/Lag       Lead       Lag       Lag <t< td=""><td></td><td>2.0</td></t<>		2.0
Lead/Lag       Lead       Lag		-1.0
Lead-Lag Optimize?       Yes       Yes </td <td>5.5</td> <td>5.5</td>	5.5	5.5
Recall Mode         None         None         None         None         None         None         Min         Min         Min         None           Act Effct Green (s)         20.4         10.8         10.8         28.4         15.1         15.1         16.0         69.5         69.5         67.0           Actuated g/C Ratio         0.17         0.09         0.09         0.24         0.13         0.13         0.14         0.59         0.59         0.57           v/c Ratio         0.25         0.45         0.49         0.82         0.50         0.38         0.87         0.29         0.16         0.27           Control Delay         36.4         59.4         10.7         61.8         55.9         6.3         70.3         12.6         2.1         8.4           Queue Delay         0.0	l Lag	Lag
Act Effct Green (s)20.410.810.828.415.115.116.069.569.567.0Actuated g/C Ratio0.170.090.090.240.130.130.140.590.590.57v/c Ratio0.250.450.490.820.500.380.870.290.160.27Control Delay36.459.410.761.855.96.370.312.62.18.4Queue Delay0.00.00.00.00.00.00.00.00.0Total Delay36.459.410.761.855.96.370.312.62.18.4LOSDEBEAEBAA		Yes
Actuated g/C Ratio0.170.090.090.240.130.130.140.590.590.57v/c Ratio0.250.450.490.820.500.380.870.290.160.27Control Delay36.459.410.761.855.96.370.312.62.18.4Queue Delay0.00.00.00.00.00.00.00.00.0Total Delay36.459.410.761.855.96.370.312.62.18.4LOSDEBEAEBAA	e Min	Min
v/c Ratio       0.25       0.45       0.49       0.82       0.50       0.38       0.87       0.29       0.16       0.27         Control Delay       36.4       59.4       10.7       61.8       55.9       6.3       70.3       12.6       2.1       8.4         Queue Delay       0.0 <t< td=""><td></td><td>59.5</td></t<>		59.5
Control Delay36.459.410.761.855.96.370.312.62.18.4Queue Delay0.00.00.00.00.00.00.00.00.0Total Delay36.459.410.761.855.96.370.312.62.18.4LOSDEBEEAEBAA		0.51
Queue Delay         0.0 <th< td=""><td></td><td>0.17</td></th<>		0.17
Total Delay         36.4         59.4         10.7         61.8         55.9         6.3         70.3         12.6         2.1         8.4           LOS         D         E         B         E         A         E         B         A         A		2.7
LOS DEBEEAEBA A		0.0
		2.7
Approach Delay 30.8 46.2 31.2		A
	41.6	
Approach LOS C D C	D	
Intersection Summary		
Cycle Length: 120		
Actuated Cycle Length: 117.8		
Natural Cycle: 90		
Control Type: Actuated-Uncoordinated		
Maximum v/c Ratio: 0.98		
Intersection Signal Delay: 38.2 Intersection LOS: D		
Intersection Capacity Utilization 90.4% ICU Level of Service E		
Analysis Period (min) 15		

Splits and Phases: 2: Meridian Rd & Eastonville Rd

	Ø3	<b>₩</b> Ø4
10 s 75 s	18 s	17 s
↑ ø5 <b>♦</b> ø6		<b>4</b> <b>Ø</b> 8
20 s 65 s	15 s	20 s

Timings				
25: Golden	Sage Ro	۱&	Woodmen	Rd

	۶	-	$\mathbf{F}$	4	-	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	- <b>††</b>	1	ሻ	- <b>††</b>	1	<u>۲</u>	<b>↑</b>	1	- ሽ	<b>↑</b>	1
Traffic Volume (vph)	422	860	74	77	1722	184	150	19	48	251	21	354
Future Volume (vph)	422	860	74	77	1722	184	150	19	48	251	21	354
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.5	12.5	10.0	12.5	12.5	10.0	11.0	11.0	10.0	11.0	
Total Split (s)	20.0	74.0	74.0	10.0	64.0	64.0	20.0	13.0	13.0	23.0	16.0	
Total Split (%)	16.7%	61.7%	61.7%	8.3%	53.3%	53.3%	16.7%	10.8%	10.8%	19.2%	13.3%	
Yellow Time (s)	3.0	5.5	5.5	3.0	5.5	5.5	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.5	6.5	4.0	6.5	6.5	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	16.1	70.0	70.0	66.2	57.7	57.7	23.7	7.4	7.4	25.2	9.4	116.3
Actuated g/C Ratio	0.14	0.60	0.60	0.57	0.50	0.50	0.20	0.06	0.06	0.22	0.08	1.00
v/c Ratio	0.94	0.41	0.08	0.20	1.00	0.22	0.46	0.17	0.19	0.73	0.15	0.24
Control Delay	79.2	14.4	0.8	9.0	52.1	3.1	41.2	56.4	1.6	53.6	52.7	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.3	0.0	0.0
Total Delay	79.2	14.4	0.8	9.0	52.1	3.1	41.2	56.4	1.6	63.9	52.7	0.4
LOS	E	В	А	А	D	А	D	Е	А	Е	D	A
Approach Delay		34.2			45.7			33.7			27.6	
Approach LOS		С			D			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 116	5.3											
Natural Cycle: 90												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 1.00												
Intersection Signal Delay: 3	8.6			Ir	ntersectio	n LOS: D						
Intersection Capacity Utiliza				10	CU Level	of Service	e F					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

<b>√</b> Ø1 -	<b>∞</b> •Ø2	<b>1</b> Ø3	₩ø4
10 s 74	ŧs	20 s	16 s
	<b>◆</b> Ø6	▶ <sub>Ø7</sub>	- <b>1</b> 08
20 s	64 s	23 s	13 s

	-	4	-	1	1	
Lane Group	EBT	WBL	WBT	NBL	NBR	
Lane Configurations	4		र्भ	Y	1	
Traffic Volume (vph)	30	377	4	100	525	
Future Volume (vph)	30	377	4	100	525	
Turn Type	NA	pm+pt	NA	Prot	Over	
Protected Phases	4	3	8	2	3	
Permitted Phases		8				
Detector Phase	4	3	8	2	3	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	10.0	30.0	40.0	80.0	30.0	
Total Split (%)	8.3%	25.0%	33.3%	66.7%	25.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0		-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag	Lead			Lead	
Lead-Lag Optimize?	Yes	Yes			Yes	
Recall Mode	None	None	None	Min	None	
Act Effct Green (s)	21.9		34.0	12.1	8.0	
Actuated g/C Ratio	0.40		0.63	0.22	0.15	
v/c Ratio	0.36		0.64	0.67	0.65	
Control Delay	4.7		10.8	16.4	10.1	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	4.7		10.8	16.4	10.1	
LOS	А		В	В	В	
Approach Delay	4.7		10.8	13.3		
Approach LOS	А		В	В		
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 54.	.2					
Natural Cycle: 50						
Control Type: Semi Act-Un	coord					
Maximum v/c Ratio: 0.67						
Intersection Signal Delay: 1	10.7			lr	ntersectior	ILOS: B
Intersection Capacity Utilization				10	CU Level o	of Service C
Analysis Period (min) 15						
-						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> Ø3	<b>→</b> Ø4
80 s	30 s	10 s
	₹Ø8	
	40 s	

# SimTraffic Performance Report

|--|

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	21.7	1.0	0.1	0.1	1.4

## 87: Meridian Rd & Site RIRO Performance by movement Interval #2 7:15

Movement
p Del/Veh (s)

# 87: Meridian Rd & Site RIRO Performance by movement Interval #3 7:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	17.2	1.2	0.1	0.1	1.1

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 7:45

Movement
/Veh (s)

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Movement
Del/Veh (s)

### Total Zone Performance By Interval

Interval Start	7:00	7:15	7:30	7:45	All	
Stop Del/Veh (s)	34.1	70.8	38.1	31.2	70.5	

Intersection				
Intersection Delay, s/veh	3.6			
Intersection LOS	A.			
Approach	EB	WB	SB	
Entry Lanes	1	1	1	
Conflicting Circle Lanes	1	1	1	
Adj Approach Flow, veh/h	162	168	40	
Demand Flow Rate, veh/h	165	171	40	
Vehicles Circulating, veh/h	17	0	171	
Vehicles Exiting, veh/h	194	182	0	
Ped Vol Crossing Leg, #/h	0	0	0	
Ped Cap Adj	1.000	1.000	1.000	
Approach Delay, s/veh	3.7	3.7	3.4	
Approach LOS	А	А	А	
Lane	Left	Left	Left	
Designated Moves	Т	Т	LR	
Assumed Moves	Т	Т	LR	
RT Channelized				
Lane Util	1.000	1.000	1.000	
Follow-Up Headway, s	2.609	2.609	2.609	
Critical Headway, s	4.976	4.976	4.976	
Entry Flow, veh/h	165	171	40	
Cap Entry Lane, veh/h	1356	1380	1159	
Entry HV Adj Factor	0.980	0.980	1.000	
Flow Entry, veh/h	162	168	40	
Cap Entry, veh/h	1330	1353	1159	
V/C Ratio	0.122	0.124	0.035	
		<u> </u>	2.4	
Control Delay, s/veh	3.7	3.7	3.4	
Control Delay, s/veh LOS	3.7 A	3.7 A	3.4 A	

Intersection	_						
Intersection Delay, s/veh 3							
Intersection LOS	A						
Approach	WB			NB		SB	
Entry Lanes	1			1		1	
Conflicting Circle Lanes	1			1		1	
Adj Approach Flow, veh/h	533			278		82	
Demand Flow Rate, veh/h	544			283		84	
Vehicles Circulating, veh/h	3			79		355	
Vehicles Exiting, veh/h	359			360		3	
Ped Vol Crossing Leg, #/h	0			0		0	
Ped Cap Adj	1.000			1.000		1.000	
Approach Delay, s/veh	3.2			4.8		4.6	
Approach LOS	А			А		А	
Lane L	eft E	Bypass	Left		Left		
Designated Moves	L	R	TR		LT		
Assumed Moves	L	R	TR		LT		
RT Channelized					LI		
		Free			LI		
Lane Util 1.0	00	Free	1.000		1.000		
		Free					
Lane Util 1.0	)9		1.000		1.000		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3	)9 76 55		1.000 2.609 4.976 283		1.000 2.609		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9	)9 76 55	189 1938 0.980	1.000 2.609 4.976 283 1273		1.000 2.609 4.976		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3	)9 76 55 76	189 1938 0.980	1.000 2.609 4.976 283		1.000 2.609 4.976 84		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3Cap Entry Lane, veh/h13Entry HV Adj Factor0.9Flow Entry, veh/h3	09 76 55 76 80 48	189 1938 0.980	1.000 2.609 4.976 283 1273		1.000 2.609 4.976 84 961 0.975 82		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3Cap Entry Lane, veh/h13Entry HV Adj Factor0.9Flow Entry, veh/h3Cap Entry, veh/h3Cap Entry, veh/h3	09 76 55 76 30 18 18	189 1938 0.980 185	1.000 2.609 4.976 283 1273 0.982 278 1250		1.000 2.609 4.976 84 961 0.975		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3Cap Entry Lane, veh/h13Entry HV Adj Factor0.9Flow Entry, veh/h3	09 76 55 76 30 18 18	189 1938 0.980 185 1900	1.000 2.609 4.976 283 1273 0.982 278		1.000 2.609 4.976 84 961 0.975 82		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3Cap Entry Lane, veh/h13Entry HV Adj Factor0.9Flow Entry, veh/h3Cap Entry, veh/h3V/C Ratio0.2Control Delay, s/veh4	09 76 55 76 30 18 18	189 1938 0.980 185 1900 0.097	1.000 2.609 4.976 283 1273 0.982 278 1250		1.000 2.609 4.976 84 961 0.975 82 937		
Lane Util1.0Follow-Up Headway, s2.6Critical Headway, s4.9Entry Flow, veh/h3Cap Entry Lane, veh/h13Entry HV Adj Factor0.9Flow Entry, veh/h3Cap Entry, veh/h13V/C Ratio0.2	09 76 55 76 30 18 18 18 58	189 1938 0.980 185 1900 0.097 0.0	1.000 2.609 4.976 283 1273 0.982 278 1250 0.222		1.000 2.609 4.976 84 961 0.975 82 937 0.087		

Int Delay, s/veh	1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et 👘		Y	
Traffic Vol, veh/h	13	157	160	53	27	0
Future Vol, veh/h	13	157	160	53	27	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	14	165	168	56	28	0

Major/Minor	Major1	Maj	jor2	I	Minor2	
Conflicting Flow All	224	0	-	0	389	196
Stage 1	-	-	-	-	196	-
Stage 2	-	-	-	-	193	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1345	-	-	-	615	845
Stage 1	-	-	-	-	837	-
Stage 2	-	-	-	-	840	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1345	-	-	-	609	845
Mov Cap-2 Maneuver	-	-	-	-	656	-
Stage 1	-	-	-	-	829	-
Stage 2	-	-	-	-	840	-

Approach	EB	WB	SB	
HCM Control Delay, s	0.6	0	10.7	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1345	-	-	- 656
HCM Lane V/C Ratio	0.01	-	-	- 0.043
HCM Control Delay (s)	7.7	-	-	- 10.7
HCM Lane LOS	А	-	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.1

1.5

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	3	4		3	¢,			4		000	4	0011	
Traffic Vol, veh/h	11	170	3	8	195	34	5	6	4	19	2	13	
Future Vol, veh/h	11	170	3	8	195	34	5	6	4	19	2	13	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None										
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	12	179	3	8	205	36	5	6	4	20	2	14	

Major/Minor	Major1		Ν	/lajor2			Minor1			Minor2			
Conflicting Flow All	241	0	0	182	0	0	452	462	181	449	445	223	
Stage 1	-	-	-	-	-	-	205	205	-	239	239	-	
Stage 2	-	-	-	-	-	-	247	257	-	210	206	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1326	-	-	1393	-	-	518	497	862	520	508	817	
Stage 1	-	-	-	-	-	-	797	732	-	764	708	-	
Stage 2	-	-	-	-	-	-	757	695	-	792	731	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1326	-	-	1393	-	-	502	490	862	506	500	817	
Mov Cap-2 Maneuver	-	-	-	-	-	-	502	490	-	506	500	-	
Stage 1	-	-	-	-	-	-	790	725	-	757	704	-	
Stage 2	-	-	-	-	-	-	738	691	-	774	724	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0.5	0.3	11.6	11.5	
HCM LOS			В	В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1
Capacity (veh/h)	559	1326	-	-	1393	-	-	592
HCM Lane V/C Ratio	0.028	0.009	-	-	0.006	-	-	0.06
HCM Control Delay (s)	11.6	7.7	-	-	7.6	-	-	11.5
HCM Lane LOS	В	А	-	-	А	-	-	В
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.2

1.2

Intersection

Movement E	BL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	۳.	<b>f</b>		- ሽ	4			4			- 🗘		
Traffic Vol, veh/h	7	183	3	9	228	35	0	0	9	24	0	9	
Future Vol, veh/h	7	183	3	9	228	35	0	0	9	24	0	9	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control Fr	ree	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length 1	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	7	193	3	9	240	37	0	0	9	25	0	9	

Major/Minor	Major1		Ν	/lajor2			Minor1			Minor2			
Conflicting Flow All	277	0	0	196	0	0	490	504	195	490	487	259	
Stage 1	-	-	-	-	-	-	209	209	-	277	277	-	
Stage 2	-	-	-	-	-	-	281	295	-	213	210	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1286	-	-	1377	-	-	489	470	846	489	481	780	
Stage 1	-	-	-	-	-	-	793	729	-	729	681	-	
Stage 2	-	-	-	-	-	-	726	669	-	789	728	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1286	-	-	1377	-	-	479	464	846	479	475	780	
Mov Cap-2 Maneuver	-	-	-	-	-	-	479	464	-	479	475	-	
Stage 1	-	-	-	-	-	-	789	725	-	725	676	-	
Stage 2	-	-	-	-	-	-	712	664	-	776	724	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0.3	0.3	9.3	12.2	
HCM LOS			А	В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	846	1286	-	-	1377	-	-	535
HCM Lane V/C Ratio	0.011	0.006	-	-	0.007	-	-	0.065
HCM Control Delay (s)	9.3	7.8	-	-	7.6	-	-	12.2
HCM Lane LOS	А	А	-	-	А	-	-	В
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.2

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Int Delay, s/veh	1.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	et P		٦	1	Y	
Traffic Vol, veh/h	188	50	32	249	23	53
Future Vol, veh/h	188	50	32	249	23	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage	,#0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	204	54	35	271	25	58

Major/Minor	Major1	Major2		Minor1	
Conflicting Flow All	0	0 258	(	) 572	231
Stage 1	-			- 231	-
Stage 2	-			- 341	-
Critical Hdwy	-	- 4.12		- 6.42	6.22
Critical Hdwy Stg 1	-			- 5.42	-
Critical Hdwy Stg 2	-			- 5.42	-
Follow-up Hdwy	-	- 2.218		- 3.518	3.318
Pot Cap-1 Maneuver	-	- 1307		- 482	808
Stage 1	-			- 807	-
Stage 2	-			- 720	-
Platoon blocked, %	-	-		-	
Mov Cap-1 Maneuve	r -	- 1307		- 469	808
Mov Cap-2 Maneuve	r -			- 545	-
Stage 1	-			- 785	-
Stage 2	-			- 720	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.9	10.8
HCM LOS			В

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	705	-	-	1307	-
HCM Lane V/C Ratio	0.117	-	-	0.027	-
HCM Control Delay (s)	10.8	-	-	7.8	-
HCM Lane LOS	В	-	-	А	-
HCM 95th %tile Q(veh)	0.4	-	-	0.1	-

Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et -		٦	1
Traffic Vol, veh/h	1	9	238	4	14	302
Future Vol, veh/h	1	9	238	4	14	302
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	9	251	4	15	318

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	601	253	0	0	255	0
Stage 1	253	-	-	-	-	-
Stage 2	348	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	463	786	-	-	1310	-
Stage 1	789	-	-	-	-	-
Stage 2	715	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		786	-	-	1310	-
Mov Cap-2 Maneuver	544	-	-	-	-	-
Stage 1	780	-	-	-	-	-
Stage 2	715	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	9.8	0	0.3
HCM LOS	А		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	753	1310	-
HCM Lane V/C Ratio	-	-	0.014	0.011	-
HCM Control Delay (s)	-	-	9.8	7.8	-
HCM Lane LOS	-	-	А	А	-
HCM 95th %tile Q(veh)	-	-	0	0	-

Intersection Delay, s/veh Intersection LOS

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		٦	ef 🔰		۳.	ef 🔰	
Traffic Vol, veh/h	52	23	1	52	26	0	5	121	124	1	253	65
Future Vol, veh/h	52	23	1	52	26	0	5	121	124	1	253	65
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	55	24	1	55	27	0	5	127	131	1	266	68
Number of Lanes	0	1	0	0	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			1			1		
HCM Control Delay	9.5			9.6			10.4			12.4		
HCM LOS	А			А			В			В		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	100%	0%	68%	67%	100%	0%
Vol Thru, %	0%	49%	30%	33%	0%	80%
Vol Right, %	0%	51%	1%	0%	0%	20%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	5	245	76	78	1	318
LT Vol	5	0	52	52	1	0
Through Vol	0	121	23	26	0	253
RT Vol	0	124	1	0	0	65
Lane Flow Rate	5	258	80	82	1	335
Geometry Grp	7	7	2	2	7	7
Degree of Util (X)	0.008	0.354	0.125	0.128	0.002	0.472
Departure Headway (Hd)	5.803	4.941	5.634	5.634	5.727	5.079
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Сар	613	722	630	630	622	706
Service Time	3.57	2.708	3.728	3.728	3.49	2.841
HCM Lane V/C Ratio	0.008	0.357	0.127	0.13	0.002	0.475
HCM Control Delay	8.6	10.4	9.5	9.6	8.5	12.4
HCM Lane LOS	А	В	А	А	А	В
HCM 95th-tile Q	0	1.6	0.4	0.4	0	2.5

Peak Hour Factor

Heavy Vehicles, %

Mvmt Flow

Intersection						
Int Delay, s/veh	2.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		- ሽ	<b>↑</b>
Traffic Vol, veh/h	8	101	160	13	100	363
Future Vol, veh/h	8	101	160	13	100	363
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	25	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	767	175	0	0	182	0
Stage 1	175	-	-	-	-	-
Stage 2	592	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	370	868	-	-	1393	-
Stage 1	855	-	-	-	-	-
Stage 2	553	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	342	868	-	-	1393	-
Mov Cap-2 Maneuver	391	-	-	-	-	-
Stage 1	791	-	-	-	-	-
Stage 2	553	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	10.3	0	1.7
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT	
Capacity (veh/h)	-	- 797	1393	-	
HCM Lane V/C Ratio	-	- 0.144	0.076	-	
HCM Control Delay (s)	-	- 10.3	7.8	-	
HCM Lane LOS	-	- B	A	-	
HCM 95th %tile Q(veh)	-	- 0.5	0.2	-	

Int Delay, s/veh	0.1						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	Ł
Lane Configurations	Y		٦	1	et 👘		
Traffic Vol, veh/h	4	2	4	257	461	5	;
Future Vol, veh/h	4	2	4	257	461	5	j
Conflicting Peds, #/hr	0	0	0	0	0	0	J
Sign Control	Stop	Stop	Free	Free	Free	Free	,
RT Channelized	-	None	-	None	-	None	<b>,</b>
Storage Length	0	-	25	-	-	-	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	95	95	95	95	95	95	5
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	4	2	4	271	485	5	,

Major/Minor	Minor2		Major1	Majo	or2	
Conflicting Flow All	767	488	490	0	-	0
Stage 1	488	-	-	-	-	-
Stage 2	279	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	370	580	1073	-	-	-
Stage 1	617	-	-	-	-	-
Stage 2	768	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	369	580	1073	-	-	-
Mov Cap-2 Maneuver	369	-	-	-	-	-
Stage 1	615	-	-	-	-	-
Stage 2	768	-	-	-	-	-

Minor Lane/Major Mvmt	NBL	NBT E	EBLn1	SBT	SBR
Capacity (veh/h)	1073	-	420	-	-
HCM Lane V/C Ratio	0.004	-	0.015	-	-
HCM Control Delay (s)	8.4	-	13.7	-	-
HCM Lane LOS	А	-	В	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Int Delay, s/veh	1.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<del>ب</del>	et -		Y	
Traffic Vol, veh/h	50	95	186	19	6	14
Future Vol, veh/h	50	95	186	19	6	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	2	2	5	5	5
Mvmt Flow	53	100	196	20	6	15

Major/Minor	Major1	Maj	jor2	I	Minor2	
Conflicting Flow All	216	0	-	0	412	206
Stage 1	-	-	-	-	206	-
Stage 2	-	-	-	-	206	-
Critical Hdwy	4.15	-	-	-	6.45	6.25
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	2.245	-	-	-	3.545	3.345
Pot Cap-1 Maneuver	1336	-	-	-	591	827
Stage 1	-	-	-	-	821	-
Stage 2	-	-	-	-	821	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1336	-	-	-	566	827
Mov Cap-2 Maneuver	-	-	-	-	566	-
Stage 1	-	-	-	-	787	-
Stage 2	-	-	-	-	821	-
Approach	EB		WB		SB	

Approach	EB	WB	SB
HCM Control Delay, s	2.7	0	10.1
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1336	-	-	- 726
HCM Lane V/C Ratio	0.039	-	-	- 0.029
HCM Control Delay (s)	7.8	0	-	- 10.1
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.1

Int Delay, s/veh 1.8 Movement EBL EBT WBT WBR SBL SBR ¥ 27 Lane Configurations đ ₽ 177 92 Traffic Vol, veh/h 9 9 28 Future Vol, veh/h 9 92 177 9 27 28 0 0 Conflicting Peds, #/hr 0 0 0 0 Sign Control Free Free Free Stop Stop Free RT Channelized -None None -None -Storage Length 0 \_ ----Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 95 95 95 95 95 95 2 2 2 Heavy Vehicles, % 2 2 2 Mvmt Flow 9 97 186 9 28 29

Major/Minor	Major1	Ма	ajor2		Minor2	
Conflicting Flow All	195	0	-	0	306	191
Stage 1	-	-	-	-	191	-
Stage 2	-	-	-	-	115	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1378	-	-	-	686	851
Stage 1	-	-	-	-	841	-
Stage 2	-	-	-	-	910	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve		-	-	-	681	851
Mov Cap-2 Maneuver	-	-	-	-	681	-
Stage 1	-	-	-	-	835	-
Stage 2	-	-	-	-	910	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.7		0		10.1	
HCM LOS					В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1378	-	-	- 758
HCM Lane V/C Ratio	0.007	-	-	- 0.076
HCM Control Delay (s)	7.6	0	-	- 10.1
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.2

Int Delay, s/veh	1.8						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ŧ	el el		Y		
Traffic Vol, veh/h	6	113	168	14	41	18	)
Future Vol, veh/h	6	113	168	14	41	18	)
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	)
RT Channelized	-	None	-	None	-	None	)
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	)
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	6	119	177	15	43	19	

Major/Minor	Major1	Maj	jor2	I	Minor2	
Conflicting Flow All	192	0	-	0	316	185
Stage 1	-	-	-	-	185	-
Stage 2	-	-	-	-	131	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1381	-	-	-	677	857
Stage 1	-	-	-	-	847	-
Stage 2	-	-	-	-	895	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1381	-	-	-	674	857
Mov Cap-2 Maneuver	-	-	-	-	702	-
Stage 1	-	-	-	-	843	-
Stage 2	-	-	-	-	895	-
Approach	EB		\//R		SB	

Approach	EB	WB	SB
HCM Control Delay, s	0.4	0	10.3
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1381	-	-	- 743
HCM Lane V/C Ratio	0.005	-	-	- 0.084
HCM Control Delay (s)	7.6	0	-	- 10.3
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.3

# Timings 1: Meridian Rd & Woodmen Rd

2040 Total Traffic WITHOUT Right-in Access
PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>ሕ</u> ካ	- <b>††</b>	1	ካካ	- <b>†</b> †	1	ካካ	- <b>††</b>	1	ካካ	- <b>†</b> †	1
Traffic Volume (vph)	765	794	375	225	561	367	350	1008	200	483	704	535
Future Volume (vph)	765	794	375	225	561	367	350	1008	200	483	704	535
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	32.0	44.0		17.0	29.0	29.0	25.0	37.0		22.0	34.0	
Total Split (%)	26.7%	36.7%		14.2%	24.2%	24.2%	20.8%	30.8%		18.3%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	28.0	38.5	119.5	12.5	23.0	23.0	18.2	31.5	119.5	18.0	31.4	119.5
Actuated g/C Ratio	0.23	0.32	1.00	0.10	0.19	0.19	0.15	0.26	1.00	0.15	0.26	1.00
v/c Ratio	0.98	0.71	0.24	0.64	0.84	0.70	0.69	1.10	0.13	0.96	0.77	0.34
Control Delay	73.7	39.8	0.4	60.2	58.8	19.3	54.9	103.4	0.2	80.5	48.1	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.7	39.8	0.4	60.2	58.8	19.3	54.9	103.4	0.2	80.5	48.1	0.6
LOS	E	D	Α	E	E	В	D	F	А	F	D	А
Approach Delay		45.7			46.5			79.2			42.4	
Approach LOS		D			D			E			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 119	.5											
Natural Cycle: 110												
Control Type: Actuated-Unc	coordinated											
Maximum v/c Ratio: 1.10												
Intersection Signal Delay: 5	3.2			li	ntersectio	n LOS: D						
Intersection Capacity Utiliza				[(	CU Level	of Service	e F					
Analysis Period (min) 15												
- , , ,												

Splits and Phases: 1: Meridian Rd & Woodmen Rd

Ø1	<b>→</b> Ø2		<b>▲</b> ø3		Ø4
17 s	44 s		25 s		34 s
<b>⋬</b> <sub>Ø5</sub>		<b>▲</b> Ø6	Ø7	· ·	Ø8
32 s		29 s	22 s	37 s	

# Timings 2: Meridian Rd & Eastonville Rd

	۶	-	$\mathbf{F}$	4	-	•	1	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	- ሻ	<b>↑</b>	1	ካካ	- <b>††</b>	1	<u>۲</u>	- <b>††</b>	1
Traffic Volume (vph)	339	197	228	200	136	225	644	1246	250	100	1154	165
Future Volume (vph)	339	197	228	200	136	225	644	1246	250	100	1154	165
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	21.0	27.0	27.0	16.0	22.0	22.0	24.0	54.0	54.0	23.0	53.0	53.0
Total Split (%)	17.5%	22.5%	22.5%	13.3%	18.3%	18.3%	20.0%	45.0%	45.0%	19.2%	44.2%	44.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	32.9	18.3	18.3	26.9	15.1	15.1	20.2	53.7	53.7	54.6	43.3	43.3
Actuated g/C Ratio	0.30	0.16	0.16	0.24	0.14	0.14	0.18	0.48	0.48	0.49	0.39	0.39
v/c Ratio	0.52	0.68	0.52	0.72	0.57	0.57	1.09	0.74	0.29	0.42	0.86	0.25
Control Delay	33.1	56.1	9.7	47.0	55.7	11.5	106.7	27.5	3.2	16.0	38.7	6.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.1	56.1	9.7	47.0	55.7	11.5	106.7	27.5	3.2	16.0	38.7	6.1
LOS	С	Е	A	D	E	В	F	С	А	В	D	A
Approach Delay		32.0			34.9			48.9			33.1	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 111.3	3											
Natural Cycle: 90												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 1.09												
Intersection Signal Delay: 40	.0			Ir	ntersectio	n LOS: D						
Intersection Capacity Utilizati					CU Level		εE					
Analysis Period (min) 15												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1	₽ø2		ø3 🕹	04
23 s	54 s	16 s	27 s	
<b>▲</b> Ø5		الح ا	Ø7	<b>◆</b> Ø8
24 s	53 s	21 s		22 s

Timings				
25: Golden	Sage Ro	۱&	Woodmen	Rd

	٦	-	$\mathbf{r}$	1	←	•	1	Ť	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	- <b>†</b> †	1	٦	- <b>†</b> †	1	۲	•	1	۲.	•	1
Traffic Volume (vph)	404	1630	121	99	1030	329	152	39	114	191	26	446
Future Volume (vph)	404	1630	121	99	1030	329	152	39	114	191	26	446
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	25.0	75.0	75.0	10.0	60.0	60.0	20.0	17.0	17.0	18.0	15.0	
Total Split (%)	20.8%	62.5%	62.5%	8.3%	50.0%	50.0%	16.7%	14.2%	14.2%	15.0%	12.5%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	18.4	59.2	59.2	55.0	46.9	46.9	25.5	9.2	9.2	18.6	8.2	106.4
Actuated g/C Ratio	0.17	0.56	0.56	0.52	0.44	0.44	0.24	0.09	0.09	0.17	0.08	1.00
v/c Ratio	0.72	0.84	0.14	0.60	0.67	0.39	0.43	0.26	0.41	0.69	0.19	0.30
Control Delay	50.3	24.6	2.3	32.1	26.5	3.4	38.9	52.8	6.1	51.8	53.1	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0
Total Delay	50.3	24.6	2.3	32.1	26.5	3.4	38.9	52.8	6.1	53.2	53.1	0.5
LOS	D	С	А	С	С	А	D	D	А	D	D	А
Approach Delay		28.2			21.6			28.4			17.7	
Approach LOS		С			С			С			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 106.	.4											
Natural Cycle: 70												
Control Type: Semi Act-Unc	oord											
Maximum v/c Ratio: 0.84												
Intersection Signal Delay: 24	4.6			I	ntersectio	n LOS: C						
Intersection Capacity Utilizat	tion 79.5%	ı		[(	CU Level	of Service	e D					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

Ø1 → Ø2	<b>1</b> Ø3	₽ <sup>™</sup> Ø4
10 s 75 s	20 s	15 s
▶ø5 ♥ø6	Ø7	<b>1</b> 08
25 s 60 s	18 s	17 s

	-	4	-	1	1	
Lane Group	EBT	WBL	WBT	NBL	NBR	
Lane Configurations	4		र्स	۰Y	1	
Traffic Volume (vph)	23	488	12	300	473	
Future Volume (vph)	23	488	12	300	473	
Turn Type	NA	pm+pt	NA	Prot	Over	
Protected Phases	4	3	8	2	3	
Permitted Phases		8				
Detector Phase	4	3	8	2	3	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	10.0	30.0	40.0	80.0	30.0	
Total Split (%)	8.3%	25.0%	33.3%	66.7%	25.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0		-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag	Lead			Lead	
Lead-Lag Optimize?	Yes	Yes			Yes	
Recall Mode	None	None	None	Min	None	
Act Effct Green (s)	23.6		36.2	21.1	8.6	
Actuated g/C Ratio	0.36		0.55	0.32	0.13	
v/c Ratio	0.29		0.87	0.74	0.73	
Control Delay	6.1		29.5	26.6	12.0	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	6.1		29.5	26.6	12.0	
LOS	А		С	С	В	
Approach Delay	6.1		29.5	19.7		
Approach LOS	А		С	В		
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 65.	4					
Natural Cycle: 60						
Control Type: Semi Act-Und	coord					
Maximum v/c Ratio: 0.87						
Intersection Signal Delay: 2	21.2			Ir	ntersectior	LOS: C
Intersection Capacity Utiliza				10	CU Level o	of Service D
Analysis Period (min) 15						
,						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> €Ø3	<b>-</b> Ø4
80 s	30 s	10 s
	<b>₩</b> Ø8	
	40 s	

# SimTraffic Performance Report

87: Meridian Rd & Site RIRO	Performance by movement Interval #1 5:00

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	14.7	2.6	0.2	0.1	2.2

# 87: Meridian Rd & Site RIRO Performance by movement Interval #2 5:15

Movement
Veh (s)

# 87: Meridian Rd & Site RIRO Performance by movement Interval #3 5:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	19.3	2.4	0.1	0.2	2.6

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 5:45

BT SBT SBR /
2.6 0.1 0.1

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

nent EBR NBT SBT SBR All
(s) 19.7 2.6 0.1 0.1

### Total Zone Performance By Interval

Interval Start	5:00	5:15	5:30	5:45	All
Stop Del/Veh (s)	76.0	124.7	82.1	130.8	395.4

Intersection				
Intersection Delay, s/veh	4.1			
Intersection LOS	4.1 A			
Approach	EB	WB	SB	
Entry Lanes	1	1	1	
Conflicting Circle Lanes	1	1	1	
Adj Approach Flow, veh/h	269	177	9	
Demand Flow Rate, veh/h	274	181	9	
Vehicles Circulating, veh/h	5	0	181	
Vehicles Exiting, veh/h	185	279	0	
Ped Vol Crossing Leg, #/h	0	0	0	
Ped Cap Adj	1.000	1.000	1.000	
Approach Delay, s/veh	4.3	3.7	3.2	
Approach LOS	А	А	А	
Lane	Left	Left	Left	
Designated Moves	Т	Т	LR	
Assumed Moves	Т	Т	LR	
RT Channelized				
Lane Util	1.000	1.000	1.000	
Follow-Up Headway, s	2.609	2.609	2.609	
Critical Headway, s	4.976	4.976	4.976	
Entry Flow, veh/h	274	181	9	
Cap Entry Lane, veh/h	1373	1380	1147	
Entry HV Adj Factor	0.980	0.980	1.000	
Flow Entry, veh/h	269	177	9	
Cap Entry, veh/h	1346	1353	1147	
V/C Ratio	0.200	0.131	0.008	
V/C Ratio Control Delay, s/veh		0.131 3.7	0.008 3.2	
V/C Ratio	0.200			

Intersection							
Intersection Delay, s/veh							
Intersection LOS	А						
Approach		WB		NB		SB	
Entry Lanes		1		1		1	
Conflicting Circle Lanes		1		1		1	
Adj Approach Flow, veh/l		992		599	:	226	
Demand Flow Rate, veh/	h 10	012		611	:	230	
Vehicles Circulating, veh		8		221		674	
Vehicles Exiting, veh/h	8	824		683		8	
Ped Vol Crossing Leg, #/		0		0		0	
Ped Cap Adj		000	1	1.000	1.	000	
Approach Delay, s/veh		5.1		10.2		9.5	
Approach LOS		А		В		А	
Lane	Left	Bypass	Left		Left		
Designated Moves	L	R	TR		LT		
Assumed Moves	L	R	TR		LT		
			113		LI		
RT Channelized		Free			LI		
	.000		1.000		1.000		
Lane Util 1 Follow-Up Headway, s 2		Free	1.000		1.000		
Lane Util 1 Follow-Up Headway, s 2	.609	Free	1.000 2.609		1.000 2.609		
Lane Util 1 Follow-Up Headway, s 2 Critical Headway, s 4 Entry Flow, veh/h	.609 .976	Free 338	1.000 2.609 4.976		1.000 2.609 4.976		
Lane Util1Follow-Up Headway, s2Critical Headway, s4Entry Flow, veh/h7Cap Entry Lane, veh/h7	.609 .976 674	Free 338 1938 0.980	1.000 2.609 4.976 611		1.000 2.609 4.976 230		
Lane Util1Follow-Up Headway, s2Critical Headway, s4Entry Flow, veh/h7Cap Entry Lane, veh/h7	.609 .976 674 1369	Free 338 1938 0.980	1.000 2.609 4.976 611 1101		1.000 2.609 4.976 230 694		
Lane Util 1 Follow-Up Headway, s 2 Critical Headway, s 4 Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor 0 Flow Entry, veh/h	.609 .976 674 1369 .981	Free 338 1938 0.980 331	1.000 2.609 4.976 611 1101 0.980		1.000 2.609 4.976 230 694 0.982		
Lane Util1Follow-Up Headway, s2Critical Headway, s4Entry Flow, veh/h4Cap Entry Lane, veh/h6Entry HV Adj Factor0Flow Entry, veh/h6Cap Entry, veh/h6	.609 .976 674 1369 .981 661	Free 338 1938 0.980 331 1900	1.000 2.609 4.976 611 1101 0.980 599 1079		1.000 2.609 4.976 230 694 0.982 226		
Lane Util1Follow-Up Headway, s2Critical Headway, s4Entry Flow, veh/h7Cap Entry Lane, veh/h7Entry HV Adj Factor0Flow Entry, veh/h7Cap Entry, veh/h7V/C Ratio0	.609 .976 674 1369 .981 661 1342	Free 338 1938 0.980 331 1900 0.174	1.000 2.609 4.976 611 1101 0.980 599 1079		1.000 2.609 4.976 230 694 0.982 226 681		
Lane Util1Follow-Up Headway, s2Critical Headway, s4Entry Flow, veh/h4Cap Entry Lane, veh/h6Entry HV Adj Factor0Flow Entry, veh/h6Cap Entry, veh/h6	.609 .976 674 1369 .981 661 1342 .492	Free 338 1938 0.980 331 1900 0.174 0.0	1.000 2.609 4.976 611 1101 0.980 599 1079 0.555		1.000 2.609 4.976 230 694 0.982 226 681 0.331		

Int Delay, s/veh	1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et –		Y	
Traffic Vol, veh/h	8	253	167	39	39	1
Future Vol, veh/h	8	253	167	39	39	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	266	176	41	41	1

Major/Minor	Major1	Ма	jor2	ľ	Minor2	
Conflicting Flow All	217	0	-	0	479	197
Stage 1	-	-	-	-	197	-
Stage 2	-	-	-	-	282	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1353	-	-	-	545	844
Stage 1	-	-	-	-	836	-
Stage 2	-	-	-	-	766	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1353	-	-	-	542	844
Mov Cap-2 Maneuver	-	-	-	-	608	-
Stage 1	-	-	-	-	831	-
Stage 2	-	-	-	-	766	-
Approach	EB		WB		SB	

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	11.3
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1353	-	-	- 612
HCM Lane V/C Ratio	0.006	-	-	- 0.069
HCM Control Delay (s)	7.7	-	-	- 11.3
HCM Lane LOS	А	-	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.2

3

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	1	4		۲.	et			\$			\$		
Traffic Vol, veh/h	31	258	3	19	181	54	5	4	21	57	4	20	
Future Vol, veh/h	31	258	3	19	181	54	5	4	21	57	4	20	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None										
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	33	272	3	20	191	57	5	4	22	60	4	21	

Major/Minor	Major1		Ν	/lajor2		Ν	/linor1			Minor2			
Conflicting Flow All	248	0	0	275	0	0	612	628	274	613	601	220	
Stage 1	-	-	-	-	-	-	340	340	-	260	260	-	
Stage 2	-	-	-	-	-	-	272	288	-	353	341	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1318	-	-	1288	-	-	405	400	765	405	414	820	
Stage 1	-	-	-	-	-	-	675	639	-	745	693	-	
Stage 2	-	-	-	-	-	-	734	674	-	664	639	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1318	-	-	1288	-	-	379	384	765	378	397	820	
Mov Cap-2 Maneuver	-	-	-	-	-	-	379	384	-	378	397	-	
Stage 1	-	-	-	-	-	-	658	623	-	726	682	-	
Stage 2	-	-	-	-	-	-	700	663	-	624	623	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0.8	0.6	11.5	15.2	
HCM LOS			В	С	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	588	1318	-	-	1288	-	-	437
HCM Lane V/C Ratio	0.054	0.025	-	-	0.016	-	-	0.195
HCM Control Delay (s)	11.5	7.8	-	-	7.8	-	-	15.2
HCM Lane LOS	В	А	-	-	А	-	-	С
HCM 95th %tile Q(veh)	0.2	0.1	-	-	0	-	-	0.7

2.7

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
			LDIX		-	WDIX	NDL		NDIN	ODL	-	ODIX	
Lane Configurations		િં		ግ	ef 👘			- <del>4</del> >			- <del>4</del> >		
Traffic Vol, veh/h	20	312	4	14	239	58	3	9	21	60	4	12	
Future Vol, veh/h	20	312	4	14	239	58	3	9	21	60	4	12	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	21	328	4	15	252	61	3	9	22	63	4	13	

Major/Minor	Major1		N	Major2		Ν	/linor1			Minor2			
Conflicting Flow All	313	0	0	332	0	0	693	715	330	701	687	283	
Stage 1	-	-	-	-	-	-	372	372	-	313	313	-	
Stage 2	-	-	-	-	-	-	321	343	-	388	374	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1247	-	-	1227	-	-	358	356	712	353	370	756	
Stage 1	-	-	-	-	-	-	648	619	-	698	657	-	
Stage 2	-	-	-	-	-	-	691	637	-	636	618	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1247	-	-	1227	-	-	341	346	712	328	359	756	
Mov Cap-2 Maneuver	-	-	-	-	-	-	341	346	-	328	359	-	
Stage 1	-	-	-	-	-	-	637	608	-	686	649	-	
Stage 2	-	-	-	-	-	-	667	629	-	596	607	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0.5	0.4	12.5	17.7	
HCM LOS			В	С	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	513	1247	-	-	1227	-	-	362
HCM Lane V/C Ratio	0.068	0.017	-	-	0.012	-	-	0.221
HCM Control Delay (s)	12.5	7.9	-	-	8	-	-	17.7
HCM Lane LOS	В	А	-	-	А	-	-	С
HCM 95th %tile Q(veh)	0.2	0.1	-	-	0	-	-	0.8

Int Delay, s/veh	1.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	el el		۲	1	Y	
Traffic Vol, veh/h	377	16	62	294	17	53
Future Vol, veh/h	377	16	62	294	17	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	410	17	67	320	18	58

Major/Minor	Major1	Major2		Minor1	
Conflicting Flow All	0	0 427	(	) 873	419
Stage 1	-			- 419	-
Stage 2	-			- 454	-
Critical Hdwy	-	- 4.12		- 6.42	6.22
Critical Hdwy Stg 1	-			- 5.42	-
Critical Hdwy Stg 2	-			- 5.42	-
Follow-up Hdwy	-	- 2.218		- 3.518	3.318
Pot Cap-1 Maneuver	-	- 1132		- 321	634
Stage 1	-			- 664	-
Stage 2	-			- 640	-
Platoon blocked, %	-	-		-	
Mov Cap-1 Maneuve	r -	- 1132		- 302	634
Mov Cap-2 Maneuve	r -			- 410	-
Stage 1	-			- 625	-
Stage 2	-			- 640	-

Approach	EB	WB	NB
HCM Control Delay, s	0	1.5	12.4
HCM LOS			В

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	560	-	-	1132	-
HCM Lane V/C Ratio	0.136	-	-	0.06	-
HCM Control Delay (s)	12.4	-	-	8.4	-
HCM Lane LOS	В	-	-	А	-
HCM 95th %tile Q(veh)	0.5	-	-	0.2	-

Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et –		1	•
Traffic Vol, veh/h	2	29	424	6	20	354
Future Vol, veh/h	2	29	424	6	20	354
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	31	446	6	21	373

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	864	449	0	0	452	0
Stage 1	449	-	-	-	-	-
Stage 2	415	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	325	610	-	-	1109	-
Stage 1	643	-	-	-	-	-
Stage 2	666	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		610	-	-	1109	-
Mov Cap-2 Maneuver	437	-	-	-	-	-
Stage 1	631	-	-	-	-	-
Stage 2	666	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.4	0	0.4
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT	
Capacity (veh/h)	-	- 595	1109	-	
HCM Lane V/C Ratio	-	- 0.055	0.019	-	
HCM Control Delay (s)	-	- 11.4	8.3	-	
HCM Lane LOS	-	- B	А	-	
HCM 95th %tile Q(veh)	-	- 0.2	0.1	-	

Intersection Delay, s/veh Intersection LOS

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eh 45.7
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		٦	ef 🔰		٦	ef 🔰	
Traffic Vol, veh/h	174	71	2	60	44	0	4	278	171	1	312	200
Future Vol, veh/h	174	71	2	60	44	0	4	278	171	1	312	200
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	183	75	2	63	46	0	4	293	180	1	328	211
Number of Lanes	0	1	0	0	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			1			1		
HCM Control Delay	20.1			14.5			43.9			66		
HCM LOS	С			В			Е			F		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	100%	0%	70%	58%	100%	0%
Vol Thru, %	0%	62%	29%	42%	0%	61%
Vol Right, %	0%	38%	1%	0%	0%	39%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	4	449	247	104	1	512
LT Vol	4	0	174	60	1	0
Through Vol	0	278	71	44	0	312
RT Vol	0	171	2	0	0	200
Lane Flow Rate	4	473	260	109	1	539
Geometry Grp	7	7	2	2	7	7
Degree of Util (X)	0.009	0.897	0.555	0.255	0.002	1.005
Departure Headway (Hd)	7.725	6.936	7.815	8.562	7.504	6.71
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Сар	466	525	465	423	474	539
Service Time	5.425	4.636	5.815	6.562	5.303	4.508
HCM Lane V/C Ratio	0.009	0.901	0.559	0.258	0.002	1
HCM Control Delay	10.5	44.2	20.1	14.5	10.3	66.1
HCM Lane LOS	В	E	С	В	В	F
HCM 95th-tile Q	0	10.3	3.3	1	0	14.3

Intersection						
Int Delay, s/veh	2.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		eî 👘		۲.	•
Traffic Vol, veh/h	15	149	421	31	132	497
Future Vol, veh/h	15	149	421	31	132	497
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	25	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	157	443	33	139	523

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	1261	460	0	0	476	0
Stage 1	460	-	-	-	-	-
Stage 2	801	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	188	601	-	-	1086	-
Stage 1	636	-	-	-	-	-
Stage 2	442	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	164	601	-	-	1086	-
Mov Cap-2 Maneuver	243	-	-	-	-	-
Stage 1	555	-	-	-	-	-
Stage 2	442	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	15	0	1.8
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT	
Capacity (veh/h)	-	-	530	1086	-	
HCM Lane V/C Ratio	-	-	0.326	0.128	-	
HCM Control Delay (s)	-	-	15	8.8	-	
HCM Lane LOS	-	-	С	А	-	
HCM 95th %tile Q(veh)	-	-	1.4	0.4	-	

Int Delay, s/veh	0.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		٦	1	et –	
Traffic Vol, veh/h	14	9	20	550	620	14
Future Vol, veh/h	14	9	20	550	620	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	25	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	15	9	21	579	653	15

Major/Minor	Minor2		Major1	Мај	or2	
Conflicting Flow All	1282	661	668	0	-	0
Stage 1	661	-	-	-	-	-
Stage 2	621	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	182	462	922	-	-	-
Stage 1	514	-	-	-	-	-
Stage 2	536	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	· 178	462	922	-	-	-
Mov Cap-2 Maneuver	· 178	-	-	-	-	-
Stage 1	502	-	-	-	-	-
Stage 2	536	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	22.2	0.3	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBL	NBT EE	3Ln1	SBT	SBR
Capacity (veh/h)	922	-	234	-	-
HCM Lane V/C Ratio	0.023	- 0	.103	-	-
HCM Control Delay (s)	9	-	22.2	-	-
HCM Lane LOS	А	-	С	-	-
HCM 95th %tile Q(veh)	0.1	-	0.3	-	-

#### Intersection Int Delay, s/veh 1.8 EBL EBT WBT WBR SBL SBR Movement Y Lane Configurations र्न Þ Traffic Vol, veh/h 48 244 270 7 18 46 Future Vol, veh/h 48 244 270 7 18 46 0 Conflicting Peds, #/hr 0 0 0 0 0 Sign Control Free Free Free Stop Stop Free RT Channelized -None None -None -Storage Length 0 \_ ----Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 95 95 95 95 95 95 Heavy Vehicles, % 5 2 2 5 5 5 Mvmt Flow 51 257 284 7 19 48

Major/Minor	Major1	Ma	ior2	P	dinar?	
	Major1		jor2		Minor2	
Conflicting Flow All	291	0	-	0	647	288
Stage 1	-	-	-	-	288	-
Stage 2	-	-	-	-	359	-
Critical Hdwy	4.15	-	-	-	6.45	6.25
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	2.245	-	-	-	3.545	3.345
Pot Cap-1 Maneuver	1254	-	-	-	431	744
Stage 1	-	-	-	-	754	-
Stage 2	-	-	-	-	700	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1254	-	-	-	411	744
Mov Cap-2 Maneuver	-	-	-	-	411	-
Stage 1	-	-	-	-	719	-
Stage 2	-	-	-	-	700	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.3		0		11.7	
HCM LOS					В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1254	-	-	- 606
HCM Lane V/C Ratio	0.04	-	-	- 0.111
HCM Control Delay (s)	8	0	-	- 11.7
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.4

Int Delay, s/veh	1.1						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		<del>ب</del>	et 👘		Y		
Traffic Vol, veh/h	31	231	259	31	18	18	
Future Vol, veh/h	31	231	259	31	18	18	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	33	243	273	33	19	19	

Major/Minor	Major1	Maj	or2		Minor2	
Conflicting Flow All	306	0	-	0	599	290
Stage 1	-	-	-	-	290	-
Stage 2	-	-	-	-	309	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1255	-	-	-	465	749
Stage 1	-	-	-	-	759	-
Stage 2	-	-	-	-	745	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1255	-	-	-	451	749
Mov Cap-2 Maneuver	-	-	-	-	451	-
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	745	-
Approach	FB		WB	_	SB	

Approach	EB	WB	SB
HCM Control Delay, s	0.9	0	11.9
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1255	-	-	- 563
HCM Lane V/C Ratio	0.026	-	-	- 0.067
HCM Control Delay (s)	7.9	0	-	- 11.9
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.2

Int Delay, s/veh	1									
Movement	EBL	EBT	WBT	WBR	SBL	SBR	l			
Lane Configurations		÷	et –		Y					
Traffic Vol, veh/h	21	228	278	46	28	12	!			
Future Vol, veh/h	21	228	278	46	28	12				
Conflicting Peds, #/hr	0	0	0	0	0	0	1			
Sign Control	Free	Free	Free	Free	Stop	Stop	,			
RT Channelized	-	None	-	None	-	None	;			
Storage Length	-	-	-	-	0	-	•			
Veh in Median Storage,	# -	0	0	-	0	-				
Grade, %	-	0	0	-	0	-	•			
Peak Hour Factor	95	95	95	95	95	95	,			
Heavy Vehicles, %	2	2	2	2	2	2				
Mvmt Flow	22	240	293	48	29	13				

Major/Minor	Major1	Ma	ajor2	ľ	Minor2	
Conflicting Flow All	341	0	-	0	601	317
Stage 1	-	-	-	-	317	-
Stage 2	-	-	-	-	284	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1218	-	-	-	463	724
Stage 1	-	-	-	-	738	-
Stage 2	-	-	-	-	764	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1218	-	-	-	453	724
Mov Cap-2 Maneuver	· _	-	-	-	541	-
Stage 1	-	-	-	-	723	-
Stage 2	-	-	-	-	764	-
Approach	EB		WB		SB	
HCM Control Delay, s			0		11.6	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1218	-	-	- 585
HCM Lane V/C Ratio	0.018	-	-	- 0.072
HCM Control Delay (s)	8	0	-	- 11.6
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.2

# Timings 1: Meridian Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	<u>ሕ</u> ግ	<u></u>	1	ሻሻ	<u></u>	1	ኘኘ	<u></u>	1	ኘኘ		7
Traffic Volume (vph)	457	520	175	150	884	168	328	344	100	294	941	1041
Future Volume (vph)	457	520	175	150	884	168	328	344	100	294	941	1041
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	22.0	45.0		17.0	40.0	40.0	17.0	35.0		23.0	41.0	
Total Split (%)	18.3%	37.5%		14.2%	33.3%	33.3%	14.2%	29.2%		19.2%	34.2%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	18.0	40.4	118.4	11.3	33.6	33.6	13.0	31.5	118.4	16.2	34.7	118.4
Actuated g/C Ratio	0.15	0.34	1.00	0.10	0.28	0.28	0.11	0.27	1.00	0.14	0.29	1.00
v/c Ratio	0.92	0.44	0.11	0.47	0.90	0.30	0.89	0.37	0.06	0.64	0.93	0.67
Control Delay	74.5	32.1	0.1	55.7	53.5	6.6	78.3	37.5	0.1	54.9	55.9	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.5	32.1	0.1	55.7	53.5	6.6	78.3	37.5	0.1	54.9	55.9	2.3
LOS	E	С	А	E	D	А	E	D	А	D	E	A
Approach Delay		44.4			47.2			50.0			31.3	
Approach LOS		D			D			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	8.4											
Natural Cycle: 90												
Control Type: Actuated-Un	ncoordinated											
Maximum v/c Ratio: 0.93												
Intersection Signal Delay:				lr	ntersectio	n LOS: D						
Intersection Capacity Utiliz	ation 89.1%	)		10	CU Level	of Service	εE					
Analysis Period (min) 15												

Splits and Phases: 1: Meridian Rd & Woodmen Rd

Ø1	<b>→</b> <sub>Ø2</sub>	<b>1</b> Ø3	▼ Ø4
17 s	45 s	17 s	41 s
<b>⋬</b> <sub>Ø5</sub>	<b>4</b> <sup>≜</sup> Ø6	Ø7	¶ø8
22 s	40 s	23 s	35 s

# Timings 2: Meridian Rd & Eastonville Rd

	٦	-	$\mathbf{F}$	4	+	*	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ካካ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1
Traffic Volume (vph)	122	73	138	250	113	125	222	596	150	100	1807	86
Future Volume (vph)	122	73	138	250	113	125	222	596	150	100	1807	86
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	15.0	17.0	17.0	18.0	20.0	20.0	16.0	75.0	75.0	10.0	69.0	69.0
Total Split (%)	12.5%	14.2%	14.2%	15.0%	16.7%	16.7%	13.3%	62.5%	62.5%	8.3%	57.5%	57.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	20.4	10.8	10.8	28.4	15.1	15.1	11.7	69.2	69.2	71.0	63.5	63.5
Actuated g/C Ratio	0.17	0.09	0.09	0.24	0.13	0.13	0.10	0.59	0.59	0.60	0.54	0.54
v/c Ratio	0.25	0.45	0.49	0.82	0.50	0.38	0.69	0.29	0.16	0.20	0.96	0.10
Control Delay	36.3	59.3	10.7	61.4	55.8	6.3	62.5	12.6	2.1	7.8	40.3	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.3	59.3	10.7	61.4	55.8	6.3	62.5	12.6	2.1	7.8	40.3	0.3
LOS	D	E	В	E	Е	А	E	В	А	А	D	A
Approach Delay		30.8			46.0			22.6			36.9	
Approach LOS		С			D			С			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	7.5											
Natural Cycle: 90												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.96												
Intersection Signal Delay: 3					ntersectio							
Intersection Capacity Utilization	ation 88.1%			10	CU Level	of Service	ε					
Analysis Period (min) 15												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1	¶ø₂	<b>√</b> Ø3	<b>₽</b> Ø4
10 s	75 s	18 s	17 s
▲ Ø5	<b>♦</b> Ø6	<u>∕</u> ø7	<b>₽</b> Ø8
16 s	69 s	15 s 2	20 s

# Timings 25: Golden Sage Rd & Woodmen Rd

	≯	-	$\mathbf{F}$	4	-	•	1	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>††</b>	1	ሻ	- <b>††</b>	1	ሻ	<b>↑</b>	1	٦	<b>↑</b>	1
Traffic Volume (vph)	418	864	74	77	1730	116	150	19	48	251	21	354
Future Volume (vph)	418	864	74	77	1730	116	150	19	48	251	21	354
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.5	12.5	10.0	12.5	12.5	10.0	11.0	11.0	10.0	11.0	
Total Split (s)	20.0	74.0	74.0	10.0	64.0	64.0	20.0	13.0	13.0	23.0	16.0	
Total Split (%)	16.7%	61.7%	61.7%	8.3%	53.3%	53.3%	16.7%	10.8%	10.8%	19.2%	13.3%	
Yellow Time (s)	3.0	5.5	5.5	3.0	5.5	5.5	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.5	6.5	4.0	6.5	6.5	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	16.1	70.0	70.0	66.2	57.7	57.7	23.7	7.4	7.4	25.2	9.4	116.3
Actuated g/C Ratio	0.14	0.60	0.60	0.57	0.50	0.50	0.20	0.06	0.06	0.22	0.08	1.00
v/c Ratio	0.93	0.41	0.08	0.20	1.01	0.14	0.46	0.17	0.19	0.73	0.15	0.24
Control Delay	77.7	14.4	0.8	9.0	53.2	1.3	41.2	56.4	1.6	53.6	52.7	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.3	0.0	0.0
Total Delay	77.7	14.4	0.8	9.0	53.2	1.3	41.2	56.4	1.6	63.9	52.7	0.4
LOS	E	В	А	А	D	А	D	E	А	Е	D	A
Approach Delay		33.6			48.2			33.7			27.6	
Approach LOS		С			D			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 116	5.3											
Natural Cycle: 90												
Control Type: Semi Act-Un	coord											
Maximum v/c Ratio: 1.01												
Intersection Signal Delay: 3	39.4			Ir	ntersectio	n LOS: D						
Intersection Capacity Utilization	ation 93.2%	)		10	CU Level	of Service	϶F					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

✓ Ø1 → Ø2	▲ ø3	<b>₽</b> Ø4
10 s 74 s	20 s	16 s
▶ø₅ ♥ø6	Ø7	<b>→</b> Ø8
20 s 64 s	23 s	13 s

	-	4	-	1	1	
Lane Group	EBT	WBL	WBT	NBL	NBR	
Lane Configurations	4		र्भ	۰Y	1	
Traffic Volume (vph)	30	377	4	100	453	
Future Volume (vph)	30	377	4	100	453	
Turn Type	NA	pm+pt	NA	Prot	Over	
Protected Phases	4	3	8	2	3	
Permitted Phases		8				
Detector Phase	4	3	8	2	3	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	10.0	30.0	40.0	80.0	30.0	
Total Split (%)	8.3%	25.0%	33.3%	66.7%	25.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0		-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag	Lead			Lead	
Lead-Lag Optimize?	Yes	Yes			Yes	
Recall Mode	None	None	None	Min	None	
Act Effct Green (s)	22.0		34.0	11.6	7.9	
Actuated g/C Ratio	0.41		0.63	0.22	0.15	
v/c Ratio	0.36		0.64	0.63	0.62	
Control Delay	4.5		10.3	16.5	9.9	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	4.5		10.3	16.5	9.9	
LOS	А		В	В	А	
Approach Delay	4.5		10.3	13.2		
Approach LOS	А		В	В		
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 53	.7					
Natural Cycle: 45						
Control Type: Semi Act-Un	ncoord					
Maximum v/c Ratio: 0.64						
Intersection Signal Delay:	10.3			Ir	ntersection	LOS: B
Intersection Capacity Utiliz				10	CU Level o	f Service B
Analysis Period (min) 15						
-						

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> Ø3	<b>→</b> Ø4
80 s	30 s	10 s
	₹Ø8	
	40 s	

# SimTraffic Performance Report

87: Meridian Rd & Site RIRO Performance by movement Interva	l #1 7:00
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Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	28.2	0.8	0.1	0.1	1.5

# 87: Meridian Rd & Site RIRO Performance by movement Interval #2 7:15

### 87: Meridian Rd & Site RIRO Performance by movement Interval #3 7:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	22.8	0.8	0.1	0.1	1.3

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 7:45

Vovement
l/Veh (s)

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Movement
Del/Veh (s)

### Total Zone Performance By Interval

Interval Start	7:00	7:15	7:30	7:45	All
Stop Del/Veh (s)	87.3	39.9	68.5	61.0	219.4

HCM 6th Roundabout	2040 Total Traffic With Right-in Access
58: Woodmen Right-In Only/Lot 11 Access & Internal	reet AM Peak Hour

Intersection						
Intersection Delay, s/veh	5.3					
Intersection LOS	А					
Approach	EB	۷	VB	NB		SB
Entry Lanes	1		1	1		1
Conflicting Circle Lanes	1		1	1		1
Adj Approach Flow, veh/h	159		63	368		40
Demand Flow Rate, veh/h	162		64	375		40
Vehicles Circulating, veh/h	17	2	13	179		277
Vehicles Exiting, veh/h	300	3	41	0		0
Ped Vol Crossing Leg, #/h	0		0	0		0
Ped Cap Adj	1.000	1.0	00	1.000		1.000
Approach Delay, s/veh	3.7	3	3.8	6.4		3.8
Approach LOS	А		A	А		А
Lane	Left	Left	Left		Left	
Designated Moves	Т	Т	LR		LR	
Assumed Moves	Т	Т	LR		LR	
RT Channelized						
Lane Util	1.000	1.000	1.000		1.000	
Follow-Up Headway, s	2.609	2.609	2.609		2.609	
Critical Headway, s	4.976	4.976	4.976		4.976	
Entry Flow, veh/h	162	64	375		40	
Cap Entry Lane, veh/h	1356	1110	1150		1040	
Entry HV Adj Factor	0.980	0.980	0.981		1.000	
Flow Entry, veh/h	159	63	368		40	
Cap Entry, veh/h	1330	1089	1128		1040	
V/C Ratio	0.119	0.058	0.326		0.038	
Control Delay, s/veh	3.7	3.8	6.4		3.8	
LOS	А	А	А		А	
95th %tile Queue, veh	0	0	1		0	

last a second base							l
Intersection	25						
Intersection Delay, s/vel Intersection LOS	13.5 A						
Intersection LOS	A						
Approach		WB		NB		SB	
Entry Lanes		1		1		1	
Conflicting Circle Lanes		1		1		1	
Adj Approach Flow, veh	/h	444		278		83	
Demand Flow Rate, veh		453		283		85	
Vehicles Circulating, veh	n/h	3		79		264	
Vehicles Exiting, veh/h		359		270		3	
Ped Vol Crossing Leg, #		0		0		0	
Ped Cap Adj		1.000		1.000	1	.000	
Approach Delay, s/veh		2.5		4.8		4.2	
Approach LOS		А		А		А	
Lane	Left	Bypass	Left		Left		
Designated Moves	L	R	TR		LT		
			117		LI		
Assumed Moves	L	R	TR		LT		
Assumed Moves RT Channelized	L						
RT Channelized Lane Util	L 1.000	R	TR 1.000		LT 1.000		
RT Channelized Lane Util Follow-Up Headway, s 2	2.609	R Free	TR 1.000 2.609		LT		
RT Channelized Lane Util Follow-Up Headway, s 2	2.609 4.976	R Free	TR 1.000		LT 1.000		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h	2.609 4.976 264	R Free 189 1938	TR 1.000 2.609 4.976 283		LT 1.000 2.609 4.976 85		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h	2.609 4.976 264 1376	R Free 189 1938 0.980	TR 1.000 2.609 4.976 283 1273		LT 1.000 2.609 4.976 85 1054		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h	2.609 4.976 264 1376 0.981	R Free 189 1938 0.980	TR 1.000 2.609 4.976 283		LT 1.000 2.609 4.976 85 1054 0.975		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	2.609 4.976 264 1376 0.981 259	R Free 189 1938 0.980	TR 1.000 2.609 4.976 283 1273 0.982 278		LT 1.000 2.609 4.976 85 1054 0.975 83		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h	2.609 4.976 264 1376 0.981 259 1350	R Free 189 1938 0.980 185 1900 0.097	TR 1.000 2.609 4.976 283 1273 0.982 278 1250		LT 1.000 2.609 4.976 85 1054 0.975 83 1028		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	2.609 4.976 264 1376 0.981 259	R Free 189 1938 0.980 185 1900 0.097	TR 1.000 2.609 4.976 283 1273 0.982 278		LT 1.000 2.609 4.976 85 1054 0.975 83		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh	2.609 4.976 264 1376 0.981 259 1350 0.192 4.3	R Free 189 1938 0.980 185 1900 0.097 0.0 A	TR 1.000 2.609 4.976 283 1273 0.982 278 1250 0.222 4.8		LT 1.000 2.609 4.976 85 1054 0.975 83 1028 0.081 4.2		
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	2.609 4.976 264 1376 0.981 259 1350 0.192	R Free 189 1938 0.980 185 1900 0.097 0.0	TR 1.000 2.609 4.976 283 1273 0.982 278 1250 0.222		LT 1.000 2.609 4.976 85 1054 0.975 83 1028 0.081		

Int Delay, s/veh	1.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	٦	1	et 👘		Y		
Traffic Vol, veh/h	49	262	60	17	27	0	
Future Vol, veh/h	49	262	60	17	27	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	)
RT Channelized	-	None	-	None	-	None	
Storage Length	150	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	52	276	63	18	28	0	

Major/Minor	Major1	Ма	jor2	1	Minor2	
Conflicting Flow All	81	0	-	0	452	72
Stage 1	-	-	-	-	72	-
Stage 2	-	-	-	-	380	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1517	-	-	-	565	990
Stage 1	-	-	-	-	951	-
Stage 2	-	-	-	-	691	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1517	-	-	-	546	990
Mov Cap-2 Maneuver	-	-	-	-	566	-
Stage 1	-	-	-	-	919	-
Stage 2	-	-	-	-	691	-
Approach	EB		WB		SB	

Approach	EB	WB	SB
HCM Control Delay, s	1.2	0	11.7
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR S	BLn1
Capacity (veh/h)	1517	-	-	-	566
HCM Lane V/C Ratio	0.034	-	-	-	0.05
HCM Control Delay (s)	7.5	-	-	-	11.7
HCM Lane LOS	А	-	-	-	В
HCM 95th %tile Q(veh)	0.1	-	-	-	0.2

2.4

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ľ	et		۲.	et P			\$			\$		
Traffic Vol, veh/h	60	227	9	4	59	23	5	6	4	19	2	13	
Future Vol, veh/h	60	227	9	4	59	23	5	6	4	19	2	13	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	63	239	9	4	62	24	5	6	4	20	2	14	

Major/Minor	Major1		Ν	/lajor2		N	1inor1			Minor2			
Conflicting Flow All	86	0	0	248	0	0	460	464	244	457	456	74	
Stage 1	-	-	-	-	-	-	370	370	-	82	82	-	
Stage 2	-	-	-	-	-	-	90	94	-	375	374	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1510	-	-	1318	-	-	512	495	795	514	501	988	
Stage 1	-	-	-	-	-	-	650	620	-	926	827	-	
Stage 2	-	-	-	-	-	-	917	817	-	646	618	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1510	-	-	1318	-	-	486	473	795	489	478	988	
Mov Cap-2 Maneuver	-	-	-	-	-	-	486	473	-	489	478	-	
Stage 1	-	-	-	-	-	-	623	594	-	887	825	-	
Stage 2	-	-	-	-	-	-	899	815	-	609	592	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	1.5	0.4	11.9	11.3	
HCM LOS			В	В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	536	1510	-	-	1318	-	-	605
HCM Lane V/C Ratio	0.029	0.042	-	-	0.003	-	-	0.059
HCM Control Delay (s)	11.9	7.5	-	-	7.7	-	-	11.3
HCM Lane LOS	В	Α	-	-	А	-	-	В
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0	-	-	0.2

1.9

Intersection

	EDI	EDT			MOT		NIDI	NDT	NDD	0.01	ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	<u>۲</u>	4		- ሽ	4			- 44			- <b>4</b> >		
Traffic Vol, veh/h	36	211	3	5	77	24	0	0	9	24	0	9	
Future Vol, veh/h	36	211	3	5	77	24	0	0	9	24	0	9	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	38	222	3	5	81	25	0	0	9	25	0	9	

Major/Minor	Major1		Ν	/lajor2			Minor1			Minor2			
Conflicting Flow All	106	0	0	225	0	0	408	416	224	408	405	94	
Stage 1	-	-	-	-	-	-	300	300	-	104	104	-	
Stage 2	-	-	-	-	-	-	108	116	-	304	301	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1485	-	-	1344	-	-	554	527	815	554	535	963	
Stage 1	-	-	-	-	-	-	709	666	-	902	809	-	
Stage 2	-	-	-	-	-	-	897	800	-	705	665	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1485	-	-	1344	-	-	536	511	815	535	519	963	
Mov Cap-2 Maneuver	-	-	-	-	-	-	536	511	-	535	519	-	
Stage 1	-	-	-	-	-	-	691	649	-	879	806	-	
Stage 2	-	-	-	-	-	-	885	797	-	679	648	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	1.1	0.4	9.5	11.3	
HCM LOS			А	В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR \$	SBLn1
Capacity (veh/h)	815	1485	-	-	1344	-	-	609
HCM Lane V/C Ratio	0.012	0.026	-	-	0.004	-	-	0.057
HCM Control Delay (s)	9.5	7.5	-	-	7.7	-	-	11.3
HCM Lane LOS	А	А	-	-	А	-	-	В
HCM 95th %tile Q(veh)	0	0.1	-	-	0	-	-	0.2

Int Delay, s/veh	2.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	et		٦	1	Y	
Traffic Vol, veh/h	194	50	32	83	23	53
Future Vol, veh/h	194	50	32	83	23	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	211	54	35	90	25	58

Major/Minor	Major1	Major2		Minor1	
Conflicting Flow All	0	0 265	(	) 398	238
Stage 1	-			- 238	-
Stage 2	-			- 160	-
Critical Hdwy	-	- 4.12		- 6.42	6.22
Critical Hdwy Stg 1	-			- 5.42	-
Critical Hdwy Stg 2	-			- 5.42	-
Follow-up Hdwy	-	- 2.218		- 3.518	3.318
Pot Cap-1 Maneuver	-	- 1299		- 607	801
Stage 1	-			- 802	-
Stage 2	-			- 869	-
Platoon blocked, %	-	-		-	
Mov Cap-1 Maneuve		- 1299		- 591	801
Mov Cap-2 Maneuve	r -			- 637	-
Stage 1	-			- 780	-
Stage 2	-			- 869	-

Approach	EB	WB	NB
HCM Control Delay, s	0	2.2	10.5
HCM LOS			В

/linor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	743	-	-	1299	-
HCM Lane V/C Ratio	0.111	-	-	0.027	-
HCM Control Delay (s)	10.5	-	-	7.8	-
HCM Lane LOS	В	-	-	А	-
HCM 95th %tile Q(veh)	0.4	-	-	0.1	-

Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et P		٦	•
Traffic Vol, veh/h	1	9	243	4	14	114
Future Vol, veh/h	1	9	243	4	14	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	9	256	4	15	120

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	408	258	0	0	260	0
Stage 1	258	-	-	-	-	-
Stage 2	150	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	599	781	-	-	1304	-
Stage 1	785	-	-	-	-	-
Stage 2	878	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		781	-	-	1304	-
Mov Cap-2 Maneuver	641	-	-	-	-	-
Stage 1	776	-	-	-	-	-
Stage 2	878	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	9.8	0	0.9
HCM LOS	А		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT	
Capacity (veh/h)	-	-	764	1304	-	
HCM Lane V/C Ratio	-	-	0.014	0.011	-	
HCM Control Delay (s)	-	-	9.8	7.8	-	
HCM Lane LOS	-	-	А	А	-	
HCM 95th %tile Q(veh)	-	-	0	0	-	

Intersection Delay, s/veh Intersection LOS

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h 9.4
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		۳.	ef 🔰		٦	el 🗧	
Traffic Vol, veh/h	52	23	1	39	26	0	5	123	124	1	88	67
Future Vol, veh/h	52	23	1	39	26	0	5	123	124	1	88	67
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	55	24	1	41	27	0	5	129	131	1	93	71
Number of Lanes	0	1	0	0	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			1			1		
HCM Control Delay	8.9			8.8			9.9			9		
HCM LOS	А			А			А			А		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	100%	0%	68%	60%	100%	0%
Vol Thru, %	0%	50%	30%	40%	0%	57%
Vol Right, %	0%	50%	1%	0%	0%	43%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	5	247	76	65	1	155
LT Vol	5	0	52	39	1	0
Through Vol	0	123	23	26	0	88
RT Vol	0	124	1	0	0	67
Lane Flow Rate	5	260	80	68	1	163
Geometry Grp	7	7	2	2	7	7
Degree of Util (X)	0.008	0.34	0.115	0.098	0.002	0.22
Departure Headway (Hd)	5.568	4.712	5.166	5.175	5.653	4.845
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Сар	642	762	691	690	632	739
Service Time	3.305	2.448	3.215	3.226	3.392	2.584
HCM Lane V/C Ratio	0.008	0.341	0.116	0.099	0.002	0.221
HCM Control Delay	8.4	9.9	8.9	8.8	8.4	9
HCM Lane LOS	А	А	А	А	А	А
HCM 95th-tile Q	0	1.5	0.4	0.3	0	0.8

Intersection						
Int Delay, s/veh	3.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et e		ľ	•
Traffic Vol, veh/h	8	101	162	13	100	147
Future Vol, veh/h	8	101	162	13	100	147
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	25	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	106	171	14	105	155

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	543	178	0	0	185	0
Stage 1	178	-	-	-	-	-
Stage 2	365	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	501	865	-	-	1390	-
Stage 1	853	-	-	-	-	-
Stage 2	702	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	463	865	-	-	1390	-
Mov Cap-2 Maneuver	509	-	-	-	-	-
Stage 1	788	-	-	-	-	-
Stage 2	702	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	10.1	0	3.2
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRWBLn	SBL	SBT	
Capacity (veh/h)	-	- 823	3 1390	-	
HCM Lane V/C Ratio	-	- 0.13	0.076	-	
HCM Control Delay (s)	-	- 10.1	7.8	-	
HCM Lane LOS	-	- 6	8 A	-	
HCM 95th %tile Q(veh)	-	- 0.	5 0.2	-	

Int Delay, s/veh	0.2						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	l
Lane Configurations	Y		٦	1	et 👘		
Traffic Vol, veh/h	4	2	4	259	245	5	
Future Vol, veh/h	4	2	4	259	245	5	,
Conflicting Peds, #/hr	0	0	0	0	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free	;
RT Channelized	-	None	-	None	-	None	,
Storage Length	0	-	25	-	-	-	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	4	2	4	273	258	5	

Major/Minor	Minor2		Major1	Мај	or2	
Conflicting Flow All	542	261	263	0	-	0
Stage 1	261	-	-	-	-	-
Stage 2	281	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318		-	-	-
Pot Cap-1 Maneuver	501	778	1301	-	-	-
Stage 1	783	-	-	-	-	-
Stage 2	767	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver		778	1301	-	-	-
Mov Cap-2 Maneuver	499	-	-	-	-	-
Stage 1	781	-	-	-	-	-
Stage 2	767	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	11.4	0.1	0
HCM LOS	В		

Minor Lane/Major Mvmt	NBL	NBT E	EBLn1	SBT	SBR
Capacity (veh/h)	1301	-	567	-	-
HCM Lane V/C Ratio	0.003	-	0.011	-	-
HCM Control Delay (s)	7.8	-	11.4	-	-
HCM Lane LOS	А	-	В	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Int Delay, s/veh	1.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<del>ب</del>	et 👘		Y	
Traffic Vol, veh/h	50	91	284	19	6	14
Future Vol, veh/h	50	91	284	19	6	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	2	2	5	5	5
Mvmt Flow	53	96	299	20	6	15

Major/Minor	Major1	Majo	or2		Minor2			_				
Conflicting Flow All	319	0	-	0	511	309				 		
Stage 1	-	-	-	-	309	-						
Stage 2	-	-	-	-	202	-						
Critical Hdwy	4.15	-	-	-	6.45	6.25						
Critical Hdwy Stg 1	-	-	-	-	5.45	-						
Critical Hdwy Stg 2	-	-	-	-	5.45	-						
Follow-up Hdwy	2.245	-	-	-	3.545	3.345						
Pot Cap-1 Maneuver	1224	-	-	-	517	724						
Stage 1	-	-	-	-	738	-						
Stage 2	-	-	-	-	825	-						
Platoon blocked, %		-	-	-								
Mov Cap-1 Maneuver	1224	-	-	-	493	724						
Mov Cap-2 Maneuver	-	-	-	-	493	-						
Stage 1	-	-	-	-	704	-						
Stage 2	-	-	-	-	825	-						

Approach	EB	WB	SB	
HCM Control Delay, s	2.9	0	10.9	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1224	-	-	- 635
HCM Lane V/C Ratio	0.043	-	-	- 0.033
HCM Control Delay (s)	8.1	0	-	- 10.9
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.1

Int Delay, s/veh	1.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		÷	et –		Y	
Traffic Vol, veh/h	9	88	275	9	27	28
Future Vol, veh/h	9	88	275	9	27	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	93	289	9	28	29

Major/Minor	Major1	Majo	or2		Minor2	
Conflicting Flow All	298	0	-	0	405	294
Stage 1	-	-	-	-	294	-
Stage 2	-	-	-	-	111	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1263	-	-	-	602	745
Stage 1	-	-	-	-	756	-
Stage 2	-	-	-	-	914	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1263	-	-	-	597	745
Mov Cap-2 Maneuver	-	-	-	-	597	-
Stage 1	-	-	-	-	750	-
Stage 2	-	-	-	-	914	-

Approach	EB	WB	SB	
HCM Control Delay, s	0.7	0	10.9	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1263	-	-	- 664
HCM Lane V/C Ratio	0.008	-	-	- 0.087
HCM Control Delay (s)	7.9	0	-	- 10.9
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.3

Int Delay, s/veh	1.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		÷	et –		Y	
Traffic Vol, veh/h	6	109	266	14	41	18
Future Vol, veh/h	6	109	266	14	41	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	115	280	15	43	19

Major/Minor	Major1	Maj	or2	1	Minor2	
Conflicting Flow All	295	0	-	0	415	288
Stage 1	-	-	-	-	288	-
Stage 2	-	-	-	-	127	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1266	-	-	-	594	751
Stage 1	-	-	-	-	761	-
Stage 2	-	-	-	-	899	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1266	-	-	-	591	751
Mov Cap-2 Maneuver	-	-	-	-	637	-
Stage 1	-	-	-	-	757	-
Stage 2	-	-	-	-	899	-
Approach	FB	1	WB		SB	

Approach	EB	WB	SB
HCM Control Delay, s	0.4	0	10.9
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1266	-	-	- 668
HCM Lane V/C Ratio	0.005	-	-	- 0.093
HCM Control Delay (s)	7.9	0	-	- 10.9
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0	-	-	- 0.3

# Timings 1: Meridian Rd & Woodmen Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ልካ	<u>^</u>	1	ካካ	<u></u>	1	ሻሻ	- <b>†</b> †	1	ካካ	<u></u>	1
Traffic Volume (vph)	755	794	375	225	665	262	465	893	200	483	704	602
Future Volume (vph)	755	794	375	225	665	262	465	893	200	483	704	602
Turn Type	Prot	NA	Free	Prot	NA	Perm	Prot	NA	Free	Prot	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			Free			6			Free			Free
Detector Phase	5	2		1	6	6	3	8		7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	11.5		10.0	11.5	11.5	10.0	11.5		10.0	11.5	
Total Split (s)	32.0	44.0		17.0	29.0	29.0	25.0	37.0		22.0	34.0	
Total Split (%)	26.7%	36.7%		14.2%	24.2%	24.2%	20.8%	30.8%		18.3%	28.3%	
Yellow Time (s)	3.0	4.5		3.0	4.5	4.5	3.0	4.5		3.0	4.5	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0		-1.0	-1.0	
Total Lost Time (s)	4.0	5.5		4.0	5.5	5.5	4.0	5.5		4.0	5.5	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	Min		None	Min	Min	None	None		None	None	
Act Effct Green (s)	28.0	39.0	120.0	12.5	23.5	23.5	20.3	31.5	120.0	18.0	29.2	120.0
Actuated g/C Ratio	0.23	0.32	1.00	0.10	0.20	0.20	0.17	0.26	1.00	0.15	0.24	1.00
v/c Ratio	1.00	0.70	0.24	0.64	0.98	0.51	0.82	0.98	0.13	0.96	0.83	0.39
Control Delay	78.0	39.6	0.4	60.4	77.9	8.6	60.4	69.7	0.2	81.7	53.0	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.0	39.6	0.4	60.4	77.9	8.6	60.4	69.7	0.2	81.7	53.0	0.7
LOS	E	D	А	E	Е	А	E	E	А	F	D	A
Approach Delay		47.5			58.7			58.0			43.2	
Approach LOS		D			E			Е			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120												
Natural Cycle: 110												
Control Type: Actuated-Uncoo	ordinated											
Maximum v/c Ratio: 1.00												
Intersection Signal Delay: 50.8				lr	ntersection	n LOS: D						
Intersection Capacity Utilizatio	n 95.1%			10	CU Level	of Service	e F					
Analysis Period (min) 15												

Splits and Phases: 1: Meridian Rd & Woodmen Rd

Ø1	<b>→</b> Ø2		<b>A</b> Ø3		Ø4	
17 s	44 s		25 s		34 s	
<b>⋬</b> <sub>Ø5</sub>		<b>▲</b> Ø6	Ø7		¶ø8	
32 s		29 s	22 s	37 :	S	

# Timings 2: Meridian Rd & Eastonville Rd

	٦	-	$\mathbf{r}$	•	-	*	1	1	1	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1
Traffic Volume (vph)	339	197	228	200	136	225	415	1246	250	100	1203	134
Future Volume (vph)	339	197	228	200	136	225	415	1246	250	100	1203	134
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8			2	6		6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	11.5	11.5	10.0	11.5	11.5
Total Split (s)	21.0	27.0	27.0	16.0	22.0	22.0	24.0	54.0	54.0	23.0	53.0	53.0
Total Split (%)	17.5%	22.5%	22.5%	13.3%	18.3%	18.3%	20.0%	45.0%	45.0%	19.2%	44.2%	44.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.5	5.5	4.0	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	33.0	18.3	18.3	26.8	15.1	15.1	18.5	52.7	52.7	55.4	44.0	44.0
Actuated g/C Ratio	0.30	0.17	0.17	0.24	0.14	0.14	0.17	0.48	0.48	0.50	0.40	0.40
v/c Ratio	0.51	0.67	0.52	0.71	0.56	0.56	0.76	0.75	0.29	0.42	0.87	0.20
Control Delay	32.8	55.6	9.6	46.3	55.6	11.5	54.5	27.7	3.2	16.2	39.1	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.8	55.6	9.6	46.3	55.6	11.5	54.5	27.7	3.2	16.2	39.1	6.7
LOS	С	E	А	D	E	В	D	С	А	В	D	A
Approach Delay		31.8			34.6			30.4			34.4	
Approach LOS		С			С			С			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 11	0.4											
Natural Cycle: 70												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.87												
Intersection Signal Delay: 3				Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliz	ation 81.1%	1		10	CU Level	of Service	эD					
Analysis Period (min) 15												

Splits and Phases: 2: Meridian Rd & Eastonville Rd

Ø1	¶ø₂	<b>√</b> Ø3	<b>₩</b> 04
23 s	54 s	16 s	27 s
★ ø5	<b>↓</b> <sub>Ø6</sub>		<b>₩</b> Ø8
24 s	53 s	21 s	22 s

# Timings 25: Golden Sage Rd & Woodmen Rd

	۶	-	$\mathbf{r}$	*	+	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	- <b>†</b> †	1	ሻ	- <b>†</b> †	1	ሻ	<b>↑</b>	1	<u>۲</u>	<b>↑</b>	1
Traffic Volume (vph)	394	1640	121	99	1030	300	152	39	114	191	26	446
Future Volume (vph)	394	1640	121	99	1030	300	152	39	114	191	26	446
Turn Type	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Free
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2	6		6	8		8	4		Free
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	10.0	12.0	12.0	10.0	12.0	12.0	10.0	10.0	10.0	10.0	10.0	
Total Split (s)	25.0	75.0	75.0	10.0	60.0	60.0	20.0	17.0	17.0	18.0	15.0	
Total Split (%)	20.8%	62.5%	62.5%	8.3%	50.0%	50.0%	16.7%	14.2%	14.2%	15.0%	12.5%	
Yellow Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.0	6.0	6.0	4.0	6.0	6.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	
Act Effct Green (s)	18.3	59.4	59.4	55.4	47.3	47.3	25.5	9.2	9.2	18.6	8.2	106.6
Actuated g/C Ratio	0.17	0.56	0.56	0.52	0.44	0.44	0.24	0.09	0.09	0.17	0.08	1.00
v/c Ratio	0.71	0.85	0.14	0.60	0.67	0.36	0.43	0.26	0.41	0.70	0.19	0.30
Control Delay	50.1	24.7	2.3	32.1	26.3	3.4	39.0	52.9	6.2	52.0	53.2	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.5	0.0	0.0
Total Delay	50.1	24.7	2.3	32.1	26.3	3.4	39.0	52.9	6.2	53.5	53.2	0.5
LOS	D	С	А	С	С	А	D	D	А	D	D	A
Approach Delay		28.2			21.8			28.5			17.8	
Approach LOS		С			С			С			В	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 106	.6											
Natural Cycle: 70												
Control Type: Semi Act-Und	coord											
Maximum v/c Ratio: 0.85												
Intersection Signal Delay: 2	4.7			Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliza		I		(	CU Level	of Service	e D					
Analysis Period (min) 15												

Splits and Phases: 25: Golden Sage Rd & Woodmen Rd

Ø1 <b>3</b> 02	<b>1</b> Ø3	
10 s 75 s	20 s	15 s
▶ø5 ♥ø6	Ø7	<b>1</b> 08
25 s 60 s	18 s	17 s

Lane GroupEBTWBLWBTNBLNBRLane ConfigurationsImage: Additional system of the system
Traffic Volume (vph)         23         488         12         300         434           Future Volume (vph)         23         488         12         300         434           Turn Type         NA         pm+pt         NA         Prot         Over           Protected Phases         4         3         8         2         3           Permitted Phases         8         2         3         3           Detector Phase         4         3         8         2         3           Switch Phase         4         3         8         2         3
Traffic Volume (vph)         23         488         12         300         434           Future Volume (vph)         23         488         12         300         434           Turn Type         NA         pm+pt         NA         Prot         Over           Protected Phases         4         3         8         2         3           Permitted Phases         8         2         3         3           Detector Phase         4         3         8         2         3           Switch Phase         4         3         8         2         3
Turn TypeNApm+ptNAProtOverProtected Phases43823Permitted Phases823Detector Phase43823Switch Phase3323
Protected Phases43823Permitted Phases8Detector Phase43823Switch Phase
Permitted Phases8Detector Phase43823Switch Phase55555
Detector Phase 4 3 8 2 3 Switch Phase
Switch Phase
Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0
Minimum Split (s) 10.0 10.0 10.0 10.0 10.0
Total Split (s) 10.0 30.0 40.0 80.0 30.0
Total Split (%) 8.3% 25.0% 33.3% 66.7% 25.0%
Yellow Time (s) 3.0 3.0 3.0 3.0 3.0
All-Red Time (s) 2.0 2.0 2.0 2.0 2.0
Lost Time Adjust (s) -1.0 -1.0 -1.0 -1.0
Total Lost Time (s) 4.0 4.0 4.0 4.0
Lead/Lag Lag Lead Lead
Lead-Lag Optimize? Yes Yes Yes
Recall Mode None None Min None
Act Effct Green (s) 23.7 36.1 20.1 8.4
Actuated g/C Ratio 0.37 0.56 0.31 0.13
v/c Ratio 0.29 0.86 0.72 0.71
Control Delay 5.8 27.2 26.3 11.8
Queue Delay 0.0 0.0 0.0 0.0
Total Delay 5.8 27.2 26.4 11.8
LOS A C C B
Approach Delay 5.8 27.2 19.4
Approach LOS A C B
Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 64.3
Natural Cycle: 60
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.86
Intersection Signal Delay: 20.3 Intersection LOS: C
Intersection Capacity Utilization 75.1% ICU Level of Service D
Analysis Period (min) 15

Splits and Phases: 26: Golden Sage Rd & Woodmen Frontage Rd

<b>↑</b> ø2	<b>€</b> €Ø3	<b>-</b> Ø4
80 s	30 s	10 s
	<b>₩</b> Ø8	
	40 s	

### SimTraffic Performance Report

87: Meridian Rd & Site RIRO Performance by movement Interval #1 5:00	

Movement
h (s)

### 87: Meridian Rd & Site RIRO Performance by movement Interval #2 5:15

Movement
Del/Veh (s)

# 87: Meridian Rd & Site RIRO Performance by movement Interval #3 5:30

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	16.8	2.0	0.1	0.2	2.1

### 87: Meridian Rd & Site RIRO Performance by movement Interval #4 5:45

### 87: Meridian Rd & Site RIRO Performance by movement Entire Run

Movement	EBR	NBT	SBT	SBR	All
Stop Del/Veh (s)	19.3	2.1	0.1	0.1	2.3

### Total Zone Performance By Interval

			F.00		
Interval Start	5:00	5:15	5:30	5:45	All
Stop Del/Veh (s)	49.6	103.7	52.7	77.5	135.1

HCM 6th Roundabout	2040 Total Traffic	With Right-in Access
58: Woodmen Right-In Only/Lot 11 Access & Internal	Street	PM Peak Hour

Intersection				
Intersection Delay, s/veh	5.6			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	264	68	353	9
Demand Flow Rate, veh/h	269	69	360	9
Vehicles Circulating, veh/h	5	168	274	237
Vehicles Exiting, veh/h	241	466	0	0
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	4.3	3.7	7.1	3.4
Approach LOS	А	А	А	А
Lane	Left	Left	Left	Left
Designated Moves	Т	Т	LR	LR
Assumed Moves	-	_		
	Т	Т	LR	LR
RT Channelized	I		LR	LR
	I 1.000	T 1.000	1.000	1.000
RT Channelized	1.000 2.609	1.000 2.609		1.000 2.609
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s	1.000 2.609 4.976	1.000 2.609 4.976	1.000 2.609 4.976	1.000 2.609 4.976
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h	1.000 2.609 4.976 269	1.000 2.609 4.976 69	1.000 2.609 4.976 360	1.000 2.609 4.976 9
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h	1.000 2.609 4.976 269 1373	1.000 2.609 4.976 69 1163	1.000 2.609 4.976 360 1043	1.000 2.609 4.976 9 1084
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor	1.000 2.609 4.976 269 1373 0.980	1.000 2.609 4.976 69 1163 0.980	1.000 2.609 4.976 360 1043 0.981	1.000 2.609 4.976 9
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h	1.000 2.609 4.976 269 1373 0.980 264	1.000 2.609 4.976 69 1163 0.980 68	1.000 2.609 4.976 360 1043 0.981 353	1.000 2.609 4.976 9 1084 1.000 9
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h	1.000 2.609 4.976 269 1373 0.980 264 1346	1.000 2.609 4.976 69 1163 0.980 68 1140	1.000 2.609 4.976 360 1043 0.981 353 1023	1.000 2.609 4.976 9 1084 1.000 9 1084
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	1.000 2.609 4.976 269 1373 0.980 264 1346 0.196	1.000 2.609 4.976 69 1163 0.980 68 1140 0.059	1.000 2.609 4.976 360 1043 0.981 353 1023 0.345	1.000 2.609 4.976 9 1084 1.000 9 1084 0.008
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio Control Delay, s/veh	1.000 2.609 4.976 269 1373 0.980 264 1346 0.196 4.3	1.000 2.609 4.976 69 1163 0.980 68 1140 0.059 3.7	1.000 2.609 4.976 360 1043 0.981 353 1023 0.345 7.1	1.000 2.609 4.976 9 1084 1.000 9 1084 0.008 3.4
RT Channelized Lane Util Follow-Up Headway, s Critical Headway, s Entry Flow, veh/h Cap Entry Lane, veh/h Entry HV Adj Factor Flow Entry, veh/h Cap Entry, veh/h V/C Ratio	1.000 2.609 4.976 269 1373 0.980 264 1346 0.196	1.000 2.609 4.976 69 1163 0.980 68 1140 0.059	1.000 2.609 4.976 360 1043 0.981 353 1023 0.345	1.000 2.609 4.976 9 1084 1.000 9 1084 0.008

Intersection					
Intersection Delay, s/veh 6.2					
Intersection LOS					
				05	
Approach	WB		NB	SB	
Entry Lanes	1		1	1	
Conflicting Circle Lanes	1		1	1	
Adj Approach Flow, veh/h	722		595	226	
Demand Flow Rate, veh/h	737		507	230	
Vehicles Circulating, veh/h	8		221	399	
Vehicles Exiting, veh/h	820	2	108	8	
Ped Vol Crossing Leg, #/h	0		0	0	
Ped Cap Adj	1.000		000	1.000	
Approach Delay, s/veh	2.8	1	0.1	6.6	
Approach LOS	А		В	A	
Lane Lef	Bypas	s Left	Left		
Designated Moves L		r tr	LT		
Assumed Moves L		r tr	LT		
RT Channelized	Fre	е			
Lane Util 1.000		1.000	1.000		
Follow-Up Headway, s 2.609		2.609	2.609		
Critical Headway, s 4.976	33	8 4.976	4.976		
Entry Flow, veh/h 399		8 607	230		
Cap Entry Lane, veh/h 1369	0.98	0 1101	919		
Entry HV Adj Factor 0.980		1 0.980	0.982		
Flow Entry, veh/h 391	190	0 595	226		
Cap Entry, veh/h 1341	0.17	4 1079	902		
	0.	0 0.551	0.250		
V/C Ratio 0.292	0.	0.001			
V/C Ratio 0.292 Control Delay, s/veh 5.2		A 10.1	6.6		
			6.6 A		

Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	•	el 👘		Y	
Traffic Vol, veh/h	26	409	64	21	39	1
Future Vol, veh/h	26	409	64	21	39	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage,	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	27	431	67	22	41	1

Major/Minor	Major1	Majo	or2	I	Minor2							
Conflicting Flow All	89	0	-	0	563	78						
Stage 1	-	-	-	-	78	-						
Stage 2	-	-	-	-	485	-						
Critical Hdwy	4.12	-	-	-	6.42	6.22						
Critical Hdwy Stg 1	-	-	-	-	5.42	-						
Critical Hdwy Stg 2	-	-	-	-	5.42	-						
Follow-up Hdwy	2.218	-	-	-	3.518	3.318						
Pot Cap-1 Maneuver	1506	-	-	-	487	983						
Stage 1	-	-	-	-	945	-						
Stage 2	-	-	-	-	619	-						
Platoon blocked, %		-	-	-								
Mov Cap-1 Maneuver		-	-	-	478	983						
Mov Cap-2 Maneuver	-	-	-	-	519	-						
Stage 1	-	-	-	-	928	-						
Stage 2	-	-	-	-	619	-						

Approach	EB	WB	SB
HCM Control Delay, s	0.4	0	12.5
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)	1506	-	-	-	525
HCM Lane V/C Ratio	0.018	-	-	-	0.08
HCM Control Delay (s)	7.4	-	-	-	12.5
HCM Lane LOS	А	-	-	-	В
HCM 95th %tile Q(veh)	0.1	-	-	-	0.3

3.8

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	1	et 👘		<u>ک</u>	et			\$			\$		
Traffic Vol, veh/h	96	340	12	11	60	36	5	4	21	57	4	20	
Future Vol, veh/h	96	340	12	11	60	36	5	4	21	57	4	20	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	101	358	13	12	63	38	5	4	22	60	4	21	

Major/Minor	Major1		Ν	/lajor2			Minor1			Minor2			
Conflicting Flow All	101	0	0	371	0	0	686	692	365	686	679	82	
Stage 1	-	-	-	-	-	-	567	567	-	106	106	-	
Stage 2	-	-	-	-	-	-	119	125	-	580	573	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1491	-	-	1188	-	-	362	367	680	362	374	978	
Stage 1	-	-	-	-	-	-	508	507	-	900	807	-	
Stage 2	-	-	-	-	-	-	885	792	-	500	504	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1491	-	-	1188	-	-	330	339	680	327	345	978	
Mov Cap-2 Maneuver	-	-	-	-	-	-	330	339	-	327	345	-	
Stage 1	-	-	-	-	-	-	473	473	-	839	799	-	
Stage 2	-	-	-	-	-	-	853	784	-	447	470	-	

Approach	EB	WB	NB	SB
HCM Control Delay, s	1.6	0.8	12.4	16.7
HCM LOS			В	С

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	519	1491	-	-	1188	-	-	393
HCM Lane V/C Ratio	0.061	0.068	-	-	0.01	-	-	0.217
HCM Control Delay (s)	12.4	7.6	-	-	8.1	-	-	16.7
HCM Lane LOS	В	А	-	-	А	-	-	С
HCM 95th %tile Q(veh)	0.2	0.2	-	-	0	-	-	0.8

3.3

Intersection

		FDT					NDI	NDT		001	007	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	- ሽ	4		ኘ	ef 👘			- <b>4</b> >			- <b>4</b> >		
Traffic Vol, veh/h	60	344	14	8	92	40	3	9	21	60	4	12	
Future Vol, veh/h	60	344	14	8	92	40	3	9	21	60	4	12	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	63	362	15	8	97	42	3	9	22	63	4	13	

Major/Minor	Major1		Ν	lajor2			Minor1			Minor2			
Conflicting Flow All	139	0	0	377	0	0	639	651	370	645	637	118	
Stage 1	-	-	-	-	-	-	496	496	-	134	134	-	
Stage 2	-	-	-	-	-	-	143	155	-	511	503	-	
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1445	-	-	1181	-	-	389	388	676	385	395	934	
Stage 1	-	-	-	-	-	-	556	545	-	869	785	-	
Stage 2	-	-	-	-	-	-	860	769	-	545	541	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1445	-	-	1181	-	-	366	368	676	351	375	934	
Mov Cap-2 Maneuver	-	-	-	-	-	-	366	368	-	351	375	-	
Stage 1	-	-	-	-	-	-	532	521	-	831	780	-	
Stage 2	-	-	-	-	-	-	838	764	-	495	517	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	1.1	0.5	12.4	16.6	
HCM LOS			В	С	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	518	1445	-	-	1181	-	-	391
HCM Lane V/C Ratio	0.067	0.044	-	-	0.007	-	-	0.205
HCM Control Delay (s)	12.4	7.6	-	-	8.1	-	-	16.6
HCM Lane LOS	В	А	-	-	Α	-	-	С
HCM 95th %tile Q(veh)	0.2	0.1	-	-	0	-	-	0.8

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Int	Delay.	s/veh

Int Delay, s/veh	1.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	et		٦	1	Y	
Traffic Vol, veh/h	388	37	36	123	17	53
Future Vol, veh/h	388	37	36	123	17	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	422	40	39	134	18	58

Major/Minor	Major1	Major2	Minor1	
Conflicting Flow All	0	0 462	0 654	442
Stage 1	-		- 442	-
Stage 2	-		- 212	-
Critical Hdwy	-	- 4.12	- 6.42	6.22
Critical Hdwy Stg 1	-		- 5.42	-
Critical Hdwy Stg 2	-		- 5.42	-
Follow-up Hdwy	-	- 2.218	- 3.518	3.318
Pot Cap-1 Maneuver	-	- 1099	- 431	615
Stage 1	-		- 648	-
Stage 2	-		- 823	-
Platoon blocked, %	-	-	-	
Mov Cap-1 Maneuve		- 1099	- 416	615
Mov Cap-2 Maneuve	r -		- 502	-
Stage 1	-		- 625	-
Stage 2	-		- 823	-

Approach	EB	WB	NB
HCM Control Delay, s	0	1.9	12.1
HCM LOS			В

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	583	-	-	1099	-
HCM Lane V/C Ratio	0.131	-	-	0.036	-
HCM Control Delay (s)	12.1	-	-	8.4	-
HCM Lane LOS	В	-	-	А	-
HCM 95th %tile Q(veh)	0.4	-	-	0.1	-

Int Delay, s/veh	0.8						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		el el		٦	1	
Traffic Vol, veh/h	2	29	432	9	17	157	'
Future Vol, veh/h	2	29	432	9	17	157	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	100	-	
Veh in Median Storage	,# 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	95	95	95	95	95	95	,
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	2	31	455	9	18	165	,

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	661	460	0	0	464	0
Stage 1	460	-	-	-	-	-
Stage 2	201	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	427	601	-	-	1097	-
Stage 1	636	-	-	-	-	-
Stage 2	833	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	420	601	-	-	1097	-
Mov Cap-2 Maneuver	507	-	-	-	-	-
Stage 1	626	-	-	-	-	-
Stage 2	833	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.4	0	0.8
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT	
Capacity (veh/h)	-	- 594	1097	-	
HCM Lane V/C Ratio	-	- 0.055	0.016	-	
HCM Control Delay (s)	-	- 11.4	8.3	-	
HCM Lane LOS	-	- B	А	-	
HCM 95th %tile Q(veh)	-	- 0.2	0.1	-	

Intersection Delay, s/veh Intersection LOS

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19.8
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С

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$		۳.	ef 🔰		٦.	ef 🔰	
Traffic Vol, veh/h	174	71	2	46	40	0	10	280	171	1	126	138
Future Vol, veh/h	174	71	2	46	40	0	10	280	171	1	126	138
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	183	75	2	48	42	0	11	295	180	1	133	145
Number of Lanes	0	1	0	0	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			1			1		
HCM Control Delay	15.3			11.6			26.9			14.5		
HCM LOS	С			В			D			В		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	100%	0%	70%	53%	100%	0%
Vol Thru, %	0%	62%	29%	47%	0%	48%
Vol Right, %	0%	38%	1%	0%	0%	52%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	10	451	247	86	1	264
LT Vol	10	0	174	46	1	0
Through Vol	0	280	71	40	0	126
RT Vol	0	171	2	0	0	138
Lane Flow Rate	11	475	260	91	1	278
Geometry Grp	7	7	2	2	7	7
Degree of Util (X)	0.02	0.788	0.472	0.177	0.002	0.479
Departure Headway (Hd)	6.758	5.978	6.541	7.026	7.086	6.201
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Сар	532	608	551	510	507	584
Service Time	4.468	3.689	4.588	5.084	4.8	3.915
HCM Lane V/C Ratio	0.021	0.781	0.472	0.178	0.002	0.476
HCM Control Delay	9.6	27.3	15.3	11.6	9.8	14.5
HCM Lane LOS	А	D	С	В	А	В
HCM 95th-tile Q	0.1	7.6	2.5	0.6	0	2.6

Intersection						
Int Delay, s/veh	3.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		eî 👘		<u>ار ا</u>	•
Traffic Vol, veh/h	15	149	423	31	132	249
Future Vol, veh/h	15	149	423	31	132	249
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	25	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	157	445	33	139	262

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	1002	462	0	0	478	0
Stage 1	462	-	-	-	-	-
Stage 2	540	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	269	600	-	-	1084	-
Stage 1	634	-	-	-	-	-
Stage 2	584	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	235	600	-	-	1084	-
Mov Cap-2 Maneuver	330	-	-	-	-	-
Stage 1	553	-	-	-	-	-
Stage 2	584	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	14.3	0	3.1
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRV	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	558	1084	-
HCM Lane V/C Ratio	-	-	0.309	0.128	-
HCM Control Delay (s)	-	-	14.3	8.8	-
HCM Lane LOS	-	-	В	А	-
HCM 95th %tile Q(veh)	-	-	1.3	0.4	-

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Int Delay, s/veh	0.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		٦	1	el el	
Traffic Vol, veh/h	14	9	20	552	372	8
Future Vol, veh/h	14	9	20	552	372	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	25	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	15	9	21	581	392	8

Major/Minor	Minor2		Major1	Maj	or2		
Conflicting Flow All	1019	396	400	0	-	0	
Stage 1	396	-	-	-	-	-	
Stage 2	623	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	263	653	1159	-	-	-	
Stage 1	680	-	-	-	-	-	
Stage 2	535	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	258	653	1159	-	-	-	
Mov Cap-2 Maneuver	258	-	-	-	-	-	
Stage 1	668	-	-	-	-	-	
Stage 2	535	-	-	-	-	-	

Approach	EB	NB	SB
HCM Control Delay, s	16.5	0.3	0
HCMLOS	С		

Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT	SBR
Capacity (veh/h)	1159	- 338	-	-
HCM Lane V/C Ratio	0.018	- 0.072	-	-
HCM Control Delay (s)	8.2	- 16.5	-	-
HCM Lane LOS	А	- C	-	-
HCM 95th %tile Q(veh)	0.1	- 0.2	-	-

#### Intersection

Int Delay, s/veh	2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<del>ب</del>	et -		Y	
Traffic Vol, veh/h	48	234	167	7	18	46
Future Vol, veh/h	48	234	167	7	18	46
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	2	2	5	5	5
Mvmt Flow	51	246	176	7	19	48

Major/Minor	Major1	Ма	ajor2	ľ	Minor2	
Conflicting Flow All	183	0	-	0	528	180
Stage 1	-	-	-	-	180	-
Stage 2	-	-	-	-	348	-
Critical Hdwy	4.15	-	-	-	6.45	6.25
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	2.245	-	-	-	3.545	3.345
Pot Cap-1 Maneuver	1374	-	-	-	506	855
Stage 1	-	-	-	-	844	-
Stage 2	-	-	-	-	708	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1374	-	-	-	484	855
Mov Cap-2 Maneuver	-	-	-	-	484	-
Stage 1	-	-	-	-	808	-
Stage 2	-	-	-	-	708	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.3		0		10.7	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1374	-	-	- 703
HCM Lane V/C Ratio	0.037	-	-	- 0.096
HCM Control Delay (s)	7.7	0	-	- 10.7
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.3

Intersection							
Int Delay, s/veh	1.3						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		- <del>स</del> ी	4		۰¥		
Traffic Vol, veh/h	31	221	156	31	18	18	
Future Vol, veh/h	31	221	156	31	18	18	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	,
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage	, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	33	233	164	33	19	19	

Major/Minor	Major1	Ma	ajor2	ſ	Minor2	
Conflicting Flow All	197	0	-	0	480	181
Stage 1	-	-	-	-	181	-
Stage 2	-	-	-	-	299	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1376	-	-	-	545	862
Stage 1	-	-	-	-	850	-
Stage 2	-	-	-	-	752	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1376	-	-	-	530	862
Mov Cap-2 Maneuver	-	-	-	-	530	-
Stage 1	-	-	-	-	826	-
Stage 2	-	-	-	-	752	-
Approach	EB		WB		SB	
Approach	ĽD		110		00	

HCM Control Delay, s 0.9 0 10.8
HCM LOS B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1376	-	-	- 656
HCM Lane V/C Ratio	0.024	-	-	- 0.058
HCM Control Delay (s)	7.7	0	-	- 10.8
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.2

#### Intersection Int Delay, s/veh 1.2 Movement EBL EBT WBT WBR SBL SBR Y Lane Configurations đ Þ 175 28 Traffic Vol, veh/h 21 218 46 12 Future Vol, veh/h 21 218 175 46 28 12 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized -None -None -None Storage Length 0 -----Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 95 95 95 95 95 95 Heavy Vehicles, % 2 2 2 2 2 2 Mvmt Flow 22 229 184 48 29 13

Major/Minor	Major1	Ν	/lajor2	I	Minor2	
Conflicting Flow All	232	0	-	0	481	208
Stage 1	-	-	-	-	208	-
Stage 2	-	-	-	-	273	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	0.12	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1336	-	-	-	544	832
Stage 1	-	-	-	-	827	-
Stage 2	-	-	-	-	773	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	534	832
Mov Cap-2 Maneuver	r -	-	-	-	599	-
Stage 1	-	-	-	-	811	-
Stage 2	-	-	-	-	773	-
Approach	EB		WB		SB	
HCM Control Delay, s			0		10.9	
HCM LOS	5 0.7		0		10.9 B	
					D	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR \$	SBLn1

winor Lane/wajor www.	EBL	ERI	<b>VVBI</b>	WBR SBLUI
Capacity (veh/h)	1336	-	-	- 654
HCM Lane V/C Ratio	0.017	-	-	- 0.064
HCM Control Delay (s)	7.7	0	-	- 10.9
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 0.2



		F	REEWAY	WEAV	ING WOF	RKSHEE	Т		
Genera	I Informatio	on			Site Info	rmation			
Analyst Agency/Cor Date Perfor Analysis Tir Project Des	med	KDF LSC 7/21/20 AM Pea			Freeway/Dir Weaving Seg Analysis Yea	gment Locati		men Rd an Rd & Rigl	nt-in Only
Inputs		viarkeipiace							
Weaving se Freeway fre	umber of lanes, N gment length, L <sub>s</sub> ee-flow speed, FF	S		One-Sided 3 885ft 50 mph	Segment typ Freeway min Freeway maa Terrain type	imum speed			Freeway 15 1900 Leve
Conver	sions to po	:/h Unde	r Base Co	ndition	1		1		<u> </u>
	V (veh/h)	PHF	Truck (%)	RV (%)	Ε <sub>Τ</sub>	E <sub>R</sub>	f <sub>HV</sub>	fp	v (pc/h)
V <sub>FF</sub>	958	0.94	2	0	1.5	1.2	0.990	1.00	1029
V <sub>RF</sub>	956	0.94	2	0	1.5	1.2	0.990	1.00	1027
V <sub>FR</sub>	265	0.94	2	0	1.5	1.2	0.990	1.00	285
V <sub>RR</sub>	85	0.94	2	0	1.5	1.2	0.990	1.00	91
V <sub>NW</sub>	1120							V =	2432
V <sub>w</sub>	1312								
VR	0.539								
Configu	iration Cha	aracterist	tics						
Minimum m	aneuver lanes, N	N <sub>WL</sub>		2 lc	Minimum we	aving lane c	hanges, LC <sub>MIN</sub>		1312 lc/h
Interchange	e density, ID			1.0 int/mi	Weaving lan	e changes, L	-C <sub>w</sub>		1460 lc/h
Minimum R	F lane changes,	LC <sub>RF</sub>		1 lc/pc	Non-weaving	g lane chang	es, LC <sub>NW</sub>		133 lc/h
Minimum F	R lane changes,	LC <sub>FR</sub>		1 lc/pc	Total lane ch	nanges, LC <sub>AL</sub>	L		1593 lc/h
Minimum R	R lane changes,	LC <sub>RR</sub>		lc/pc	Non-weaving	g vehicle inde	ex, I <sub>NW</sub>		99
Weavin	g Segment	Speed,	Density, I	_evel of	Service,	and Cap	pacity		
Weaving se	egment flow rate,	V		2409 veh/h	Weaving inte	ensity factor,	W		0.359
U U	egment capacity,			3959 veh/h	Weaving seg				38.8 mph
Weaving se	egment v/c ratio			0.608	Average wea	aving speed,	S <sub>w</sub>		40.7 mph
-	egment density, [	)	20	).9 pc/mi/ln	Average nor	n-weaving sp	eed, S <sub>NW</sub>		36.7 mph
Level of Se	rvice, LOS			С	Maximum we	eaving length	n, L <sub>MAX</sub>		8291 ft
Notes									
Chapter 13,	egments longer th "Freeway Merge a es that exceed the	nd Diverge Se	gments".	-		olated merge	and diverge are	eas using the	procedures of

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			REEWAY	' WEAV					
General	Information	on			Site Info	rmation			
Analyst Agency/Con Date Perforr Analysis Tin	med ne Period		)17 ak Hour		Freeway/Dir Weaving Seg Analysis Yea	gment Locat		men Rd ian Rd & Rigł	nt-in Only
Project Desc Inputs	cription Falcon	Marketplace							
Weaving co Weaving nu Weaving se Freeway fre	mber of lanes, N gment length, L e-flow speed, Fl	S FS		One-Sided 3 885ft 50 mph	Segment type Freeway min Freeway max Terrain type	imum speec			Freeway 1 1900 Leve
Conver	sions to po	<u>c/h Unde</u>	r Base Co	ndition	S			-	
	V (veh/h)	PHF	Truck (%)	RV (%)	Ε <sub>Τ</sub>	E <sub>R</sub>	f <sub>HV</sub>	fp	v (pc/h)
V <sub>FF</sub>	878	0.94	2	0	1.5	1.2	0.990	1.00	943
V <sub>RF</sub>	538	0.94	2	0	1.5	1.2	0.990	1.00	578
V <sub>FR</sub>	272	0.94	2	0	1.5	1.2	0.990	1.00	292
V <sub>RR</sub>	64	0.94	2	0	1.5	1.2	0.990	1.00	69
V <sub>NW</sub>	1012							V =	1882
V <sub>W</sub>	870							-	•
VR	0.462								
Configu	ration Cha	aracteris	tics						
Minimum m	aneuver lanes,	N <sub>WI</sub>		2 lc	Minimum we	aving lane c	hanges, LC <sub>MIN</sub>		870 lc/ł
Interchange	e density, ID			1.0 int/mi	Weaving lan	e changes, l	_C <sub>w</sub>		1018 lc/ł
Minimum R	F lane changes,	LC <sub>RF</sub>		1 lc/pc	Non-weaving	g lane chang	jes, LC <sub>NW</sub>		110 lc/ł
Minimum Fl	R lane changes,	LC <sub>FR</sub>		1 lc/pc	Total lane ch	nanges, LC <sub>AI</sub>	L		1128 lc/ł
Minimum R	R lane changes	, LC <sub>RR</sub>		lc/pc	Non-weaving	g vehicle ind	ex, I <sub>NW</sub>		90
Weaving	g Segmen	t Speed,	Density, I		<u>I</u>				
Weaving se	gment flow rate	, V	,	1864 veh/h	Weaving inte	ensity factor,	W		0.274
Weaving se	gment capacity	, C <sub>w</sub>		4164 veh/h	Weaving seg				41.5 mpł
Weaving se	gment v/c ratio			0.448	Average wea	aving speed,	S <sub>w</sub>		42.5 mpl
•	gment density,	D	15	5.1 pc/mi/ln	Average non				40.7 mpl
Level of Sei	rvice, LOS			В	Maximum we	eaving lengt	h, L <sub>MAX</sub>		73891
Notes									
Chapter 13, "	egments longer t Freeway Merge a es that exceed the	and Diverge Se	egments".	U U		olated merge	and diverge are	eas using the	procedures of

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# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	160	168	237	265	62	108	282	286	18	142	161	187
Average Queue (ft)	81	98	132	147	23	59	188	181	1	37	90	104
95th Queue (ft)	139	152	212	231	54	101	265	263	13	102	148	169
Link Distance (ft)			1012	1012			2167	2167	2167		1181	1181
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500	500			435	435				315		
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	157	132	147	263	270
Average Queue (ft)	65	68	86	142	148
95th Queue (ft)	133	121	132	224	231
Link Distance (ft)	1181			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	Т	L	Т	R	L	Т	Т	R	L	Т	Т
Maximum Queue (ft)	44	29	123	64	62	58	100	118	34	58	150	242
Average Queue (ft)	12	4	48	3	25	19	25	32	6	27	55	95
95th Queue (ft)	37	20	100	37	45	44	69	84	23	52	112	196
Link Distance (ft)	261	261		796			444	444			1387	1387
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)			100		200	425			525	375		
Storage Blk Time (%)			3									
Queuing Penalty (veh)			2									

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB
Directions Served	R
Maximum Queue (ft)	8
Average Queue (ft)	0
95th Queue (ft)	4
Link Distance (ft)	1387
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	90	140	152	31	27	348	334	30	187	85	30	65
Average Queue (ft)	34	57	67	6	5	146	163	5	84	10	4	23
95th Queue (ft)	70	111	121	24	22	287	293	21	152	54	22	56
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)							0		5			
Queuing Penalty (veh)							0		1			

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	115
Average Queue (ft)	43
95th Queue (ft)	83
Link Distance (ft)	161
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	59
Average Queue (ft)	30
95th Queue (ft)	51
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	
<b>Z</b> ana Ourrene and	

#### Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	B88	B88	B88	WB	WB	WB	WB	WB
Directions Served	UL	L	Т	Т	Т	Т	Т	L	L	Т	Т	R
Maximum Queue (ft)	356	361	282	289	128	537	290	135	160	282	280	121
Average Queue (ft)	199	206	145	161	5	24	10	42	83	186	182	24
95th Queue (ft)	300	302	231	242	94	228	152	96	130	264	264	84
Link Distance (ft)			1012	1012	715	715	715			2167	2167	2167
Upstream Blk Time (%)							0					
Queuing Penalty (veh)							0					
Storage Bay Dist (ft)	500	500						435	435			
Storage Blk Time (%)	0	0										
Queuing Penalty (veh)	0	0										

Movement	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	L	Т	Т	L	L	Т	Т
Maximum Queue (ft)	140	176	381	364	129	137	190	208
Average Queue (ft)	42	98	243	215	51	71	120	126
95th Queue (ft)	113	162	357	331	102	113	177	185
Link Distance (ft)		1181	1181	1181			635	635
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	315				475	475		
Storage Blk Time (%)								
Queuing Penalty (veh)								

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB	SB
Directions Served	L	L	Т	L	Т	R	L	Т	Т	R	L	Т
Maximum Queue (ft)	44	6	17	70	33	66	59	194	217	46	89	151
Average Queue (ft)	12	0	2	24	4	33	21	65	82	11	36	52
95th Queue (ft)	36	4	13	58	19	54	45	147	172	32	73	110
Link Distance (ft)	261	261	261		796			444	444			1387
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)				100		200	425			525	375	
Storage Blk Time (%)				0								
Queuing Penalty (veh)				0								

#### Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB
Directions Served	Т	R
Maximum Queue (ft)	182	16
Average Queue (ft)	54	2
95th Queue (ft)	125	11
Link Distance (ft)	1387	1387
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	107	206	224	52	46	264	280	34	139	63	36	61
Average Queue (ft)	43	107	117	16	11	105	123	5	69	16	14	23
95th Queue (ft)	82	189	197	42	35	218	237	23	118	46	39	52
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)									1	0		
Queuing Penalty (veh)									0	0		

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	92
Average Queue (ft)	45
95th Queue (ft)	82
Link Distance (ft)	161
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	61
Average Queue (ft)	33
95th Queue (ft)	54
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	193	158	246	249	61	112	289	276	60	139	167	242
Average Queue (ft)	100	93	130	142	24	57	183	176	10	34	90	147
95th Queue (ft)	165	147	211	221	59	104	257	255	46	99	149	218
Link Distance (ft)			1012	1012			2167	2167	2167		1181	1181
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500	500			435	435				315		
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	198	142	151	267	284
Average Queue (ft)	88	76	94	149	159
95th Queue (ft)	174	128	139	243	251
Link Distance (ft)	1181			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	L	Т	R	L	Т	R	L	Т	Т	R	L
Maximum Queue (ft)	154	125	111	100	117	108	57	357	147	153	37	76
Average Queue (ft)	92	23	41	41	56	40	29	196	33	48	7	33
95th Queue (ft)	149	86	84	76	105	84	50	335	104	119	24	66
Link Distance (ft)	261	261	261	261		796			444	444		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					100		200	425			525	375
Storage Blk Time (%)					3	0		0				
Queuing Penalty (veh)					4	1		0				

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB
Directions Served	Т	Т	R
Maximum Queue (ft)	432	552	59
Average Queue (ft)	210	315	18
95th Queue (ft)	371	502	44
Link Distance (ft)	1387	1387	1387
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)	0		
Queuing Penalty (veh)	0		

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	122	137	154	42	27	368	371	33	158	88	24	81
Average Queue (ft)	52	60	69	7	5	182	197	4	84	9	3	23
95th Queue (ft)	98	118	134	27	21	344	352	21	142	40	16	60
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)							0		3	0		
Queuing Penalty (veh)							0		0	0		

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	135
Average Queue (ft)	56
95th Queue (ft)	108
Link Distance (ft)	161
Upstream Blk Time (%)	1
Queuing Penalty (veh)	1
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	68
Average Queue (ft)	39
95th Queue (ft)	62
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	
7 0	

#### Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	B88	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	L	Т	Т	Т	L	L	Т	Т	R	L	L
Maximum Queue (ft)	354	368	270	268	136	123	150	293	294	218	163	176
Average Queue (ft)	229	222	133	151	5	43	83	190	186	78	49	104
95th Queue (ft)	331	334	216	227	99	93	131	267	269	181	128	166
Link Distance (ft)			1012	1012	715			2167	2167	2167		1181
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500	500				435	435				315	
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	NB	NB	SB	SB	SB	SB
Directions Served	Т	Т	L	L	Т	Т
Maximum Queue (ft)	505	463	146	152	256	266
Average Queue (ft)	328	283	83	99	160	180
95th Queue (ft)	463	424	134	145	245	265
Link Distance (ft)	1181	1181			635	635
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			475	475		
Storage Blk Time (%)						
Queuing Penalty (veh)						

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	L	Т	R	L	Т	R	L	Т	Т	R	L
Maximum Queue (ft)	215	162	190	96	77	118	83	415	396	364	76	93
Average Queue (ft)	130	55	77	43	29	53	35	262	170	161	22	40
95th Queue (ft)	200	146	140	80	67	104	61	431	377	322	57	78
Link Distance (ft)	261	261	261	261		796			444	444		
Upstream Blk Time (%)								1	1	0		
Queuing Penalty (veh)								0	8	0		
Storage Bay Dist (ft)					100		200	425			525	375
Storage Blk Time (%)					0	1		2	1	0		
Queuing Penalty (veh)					0	3		11	4	0		

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB
Directions Served	Т	Т	R
Maximum Queue (ft)	292	317	89
Average Queue (ft)	176	191	27
95th Queue (ft)	268	285	63
Link Distance (ft)	1387	1387	1387
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)	0		
Queuing Penalty (veh)	0		

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	149	242	255	52	55	331	328	40	138	53	41	68
Average Queue (ft)	64	110	119	14	11	120	135	6	68	18	10	21
95th Queue (ft)	118	199	204	41	35	247	259	24	117	48	34	55
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)							0		1			
Queuing Penalty (veh)							0		0			

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	121
Average Queue (ft)	53
95th Queue (ft)	102
Link Distance (ft)	161
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	78
Average Queue (ft)	38
95th Queue (ft)	62
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

# Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	173	164	254	258	77	120	341	339	22	162	196	218
Average Queue (ft)	92	104	124	139	22	56	216	217	1	70	124	115
95th Queue (ft)	151	158	206	222	59	97	309	315	12	157	184	184
Link Distance (ft)			1024	1024			2167	2167	2167		1181	1181
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500	500			435	435				315		
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	181	168	184	244	258
Average Queue (ft)	68	84	102	147	158
95th Queue (ft)	147	149	160	227	232
Link Distance (ft)	1181			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	L	Т	R	L	Т	R	L	Т	Т	R	L
Maximum Queue (ft)	165	125	107	90	114	86	61	128	121	138	41	75
Average Queue (ft)	79	17	42	36	51	37	28	57	38	54	7	33
95th Queue (ft)	140	74	85	65	96	78	49	110	98	120	24	65
Link Distance (ft)	261	261	261	261		796			444	444		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					100		200	425			525	375
Storage Blk Time (%)					2	0						
Queuing Penalty (veh)					2	0						

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB
Directions Served	Т	Т	R
Maximum Queue (ft)	536	609	308
Average Queue (ft)	147	227	22
95th Queue (ft)	357	445	214
Link Distance (ft)	1387	1387	1387
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	0
Storage Bay Dist (ft)			
Storage Blk Time (%)	0		
Queuing Penalty (veh)	0		

# Intersection: 18: Woodmen frontage rd & Right-In Only/Site Access

Movement	NB	SB	NE
Directions Served	LR	LR	Т
Maximum Queue (ft)	54	50	28
Average Queue (ft)	8	23	2
95th Queue (ft)	34	47	17
Link Distance (ft)	80	154	368
Upstream Blk Time (%)	0		
Queuing Penalty (veh)	0		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	108	135	139	36	32	349	367	37	184	130	31	73
Average Queue (ft)	53	60	69	9	5	166	180	7	84	13	5	24
95th Queue (ft)	98	114	126	30	22	311	324	26	148	64	24	60
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)							0		3	0		
Queuing Penalty (veh)							0		0	0		

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	151
Average Queue (ft)	59
95th Queue (ft)	115
Link Distance (ft)	161
Upstream Blk Time (%)	2
Queuing Penalty (veh)	2
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	86
Average Queue (ft)	41
95th Queue (ft)	71
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	
7 0	

#### Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	424	443	238	261	122	146	324	342	114	220	275	404
Average Queue (ft)	242	251	134	151	43	78	219	231	16	132	179	276
95th Queue (ft)	362	377	218	228	101	132	311	317	73	230	252	387
Link Distance (ft)			1024	1024			2167	2167	2167		1181	1181
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500	500			435	435				315		
Storage Blk Time (%)	0	0									0	
Queuing Penalty (veh)	0	0									0	

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	372	187	202	261	295
Average Queue (ft)	245	101	115	164	187
95th Queue (ft)	361	172	190	246	269
Link Distance (ft)	1181			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	L	Т	R	L	Т	R	L	Т	Т	R	L
Maximum Queue (ft)	216	181	161	84	61	112	73	174	329	320	72	100
Average Queue (ft)	122	45	69	41	23	49	37	83	136	147	18	39
95th Queue (ft)	194	131	129	70	55	93	62	150	293	300	49	77
Link Distance (ft)	261	261	261	261		796			444	444		
Upstream Blk Time (%)	0											
Queuing Penalty (veh)	0											
Storage Bay Dist (ft)					100		200	425			525	375
Storage Blk Time (%)						0						
Queuing Penalty (veh)						1						

#### Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB
Directions Served	Т	Т	R
Maximum Queue (ft)	233	312	60
Average Queue (ft)	137	156	18
95th Queue (ft)	221	262	42
Link Distance (ft)	1387	1387	1387
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

# Intersection: 18: Woodmen frontage rd & Right-In Only/Site Access

Movement	NB	SB	NE	SW
Directions Served	LR	LR	Т	Т
Maximum Queue (ft)	111	29	16	18
Average Queue (ft)	21	6	1	1
95th Queue (ft)	66	24	10	11
Link Distance (ft)	80	154	368	203
Upstream Blk Time (%)	0			
Queuing Penalty (veh)	1			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB
Directions Served	L	Т	Т	R	L	Т	Т	R	L	Т	R	L
Maximum Queue (ft)	145	189	225	48	46	311	304	31	157	88	36	73
Average Queue (ft)	62	101	113	16	10	130	147	7	68	18	12	26
95th Queue (ft)	111	179	196	41	33	256	267	26	128	51	37	59
Link Distance (ft)		957	957			969	969			531		161
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	465			400	475			400	125		200	
Storage Blk Time (%)									2	0		
Queuing Penalty (veh)									1	0		

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	150
Average Queue (ft)	56
95th Queue (ft)	105
Link Distance (ft)	161
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

## Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

Movement	WB
Directions Served	LT
Maximum Queue (ft)	79
Average Queue (ft)	39
95th Queue (ft)	62
Link Distance (ft)	1964
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

# Intersection: 88: Woodmen Rd & Right-In Only

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)
Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	т
Maximum Queue (ft)	382	362	196	207	109	129	368	361	120	217	243	250
Average Queue (ft)	223	212	113	128	38	76	246	241	30	107	148	156
95th Queue (ft)	361	354	179	192	85	122	339	333	95	201	213	234
Link Distance (ft)	1011	1011	1011	1011			2167	2167	2167		1180	1180
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					535	535				400		
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	203	160	171	312	326
Average Queue (ft)	99	92	109	207	233
95th Queue (ft)	184	144	158	301	320
Link Distance (ft)	1180			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	NB
Directions Served	L	L	Т	R	L	Т	R	L	L	Т	Т	R
Maximum Queue (ft)	154	112	133	153	200	563	300	254	259	219	201	62
Average Queue (ft)	81	12	64	52	157	226	78	137	148	80	97	21
95th Queue (ft)	138	57	120	103	233	585	241	231	241	166	165	49
Link Distance (ft)	261	261	261	261		796				444	444	
Upstream Blk Time (%)						4						
Queuing Penalty (veh)						0						
Storage Bay Dist (ft)					100		200	425	425			525
Storage Blk Time (%)					44	8						
Queuing Penalty (veh)					105	29						

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB	SB
	50	50	50	50
Directions Served	L	Т	Т	R
Maximum Queue (ft)	475	1417	1418	1415
Average Queue (ft)	211	1090	1116	757
95th Queue (ft)	551	1693	1693	1839
Link Distance (ft)		1380	1380	1380
Upstream Blk Time (%)		16	37	20
Queuing Penalty (veh)		0	0	0
Storage Bay Dist (ft)	375			
Storage Blk Time (%)		41		
Queuing Penalty (veh)		41		

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	B16	B16	NB
Directions Served	L	L	Т	Т	R	L	Т	Т	R	Т	Т	L
Maximum Queue (ft)	400	421	594	580	40	575	990	988	500	383	407	181
Average Queue (ft)	291	311	299	294	9	193	665	676	298	65	73	91
95th Queue (ft)	528	575	941	895	26	592	1154	1164	677	348	374	154
Link Distance (ft)			1455	1455			969	969		3861	3861	
Upstream Blk Time (%)							11	14				
Queuing Penalty (veh)							106	133				
Storage Bay Dist (ft)	450	450			400	475			400			125
Storage Blk Time (%)	16	23	0				28	36				6
Queuing Penalty (veh)	68	101	0				22	66				4

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	NB	NB	SB	SB
Directions Served	Т	R	L	Т
Maximum Queue (ft)	117	48	171	59
Average Queue (ft)	17	16	144	23
95th Queue (ft)	67	35	188	54
Link Distance (ft)	526		154	154
Upstream Blk Time (%)			26	
Queuing Penalty (veh)			54	
Storage Bay Dist (ft)		200		
Storage Blk Time (%)	0			
Queuing Penalty (veh)	0			

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

EB	WB	NB	NB
TR	LT	LR	R
199	566	146	135
81	230	68	13
156	532	139	83
1503	1983	154	154
		0	0
		0	0
	TR 199 81 156	TR LT 199 566 81 230 156 532	TR         LT         LR           199         566         146           81         230         68           156         532         139           1503         1983         154           0         0

# Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	B88	B88	B88	B88	WB	WB	WB	WB
Directions Served	UL	L	Т	Т	Т	Т	Т	Т	L	L	Т	Т
Maximum Queue (ft)	860	865	422	340	30	35	146	152	153	177	517	524
Average Queue (ft)	556	552	212	224	4	5	5	5	78	110	316	315
95th Queue (ft)	1012	1021	334	310	35	34	107	111	142	163	554	548
Link Distance (ft)	1011	1011	1011	1011	716	716	716	716			2167	2167
Upstream Blk Time (%)	5	6					0	0				
Queuing Penalty (veh)	20	22					0	0				
Storage Bay Dist (ft)									1000	1000		
Storage Blk Time (%)												
Queuing Penalty (veh)												

Movement	WB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	R	L	L	Т	Т	L	L	Т	Т
Maximum Queue (ft)	328	392	929	1203	1153	391	407	368	372
Average Queue (ft)	167	134	332	730	697	276	292	235	227
95th Queue (ft)	290	273	814	1321	1281	479	512	521	410
Link Distance (ft)	2167		1851	1851	1851			635	635
Upstream Blk Time (%)								3	0
Queuing Penalty (veh)								20	1
Storage Bay Dist (ft)		400				475	475		
Storage Blk Time (%)			0			6	9	0	
Queuing Penalty (veh)			0			19	33	0	

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB
Directions Served	L	L	Т	Т	R	L	Т	Т	R	L	Т	R
Maximum Queue (ft)	287	297	390	387	49	130	349	367	165	187	170	131
Average Queue (ft)	158	141	221	237	14	63	186	196	74	91	39	47
95th Queue (ft)	273	252	353	365	34	114	323	330	139	158	112	101
Link Distance (ft)			944	944			969	969			525	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	450	450			400	475			400	125		200
Storage Blk Time (%)	0	0	0	0				0		5	0	
Queuing Penalty (veh)	1	0	0	0				0		8	0	

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB	SB
Directions Served	L	Т
Maximum Queue (ft)	166	84
Average Queue (ft)	129	25
95th Queue (ft)	185	65
Link Distance (ft)	154	154
Upstream Blk Time (%)	12	
Queuing Penalty (veh)	28	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

	WB	NB	NB
TR	LT	LR	R
183	494	164	183
55	209	127	19
119	477	191	106
1503	1983	154	154
		6	0
		24	1
	183 55 119	183 494 55 209 119 477	183         494         164           55         209         127           119         477         191           1503         1983         154           6         6

# Zone Summary

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	NB
Directions Served	L	L	Т	R	L	Т	R	L	L	Т	Т	R
Maximum Queue (ft)	258	194	260	194	199	536	300	425	430	500	452	231
Average Queue (ft)	158	91	127	77	144	232	103	330	338	330	248	35
95th Queue (ft)	227	190	216	147	228	614	277	504	507	603	422	124
Link Distance (ft)	261	261	261	261		796				444	444	
Upstream Blk Time (%)	0		1	0		6		3	12	19	0	0
Queuing Penalty (veh)	0		0	0		0		0	0	208	4	0
Storage Bay Dist (ft)					100		200	425	425			525
Storage Blk Time (%)					44	11		11	21	12	0	0
Queuing Penalty (veh)					159	48		68	131	79	1	0

## Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB	SB
	30	30	১০	১চ
Directions Served	L	Т	Т	R
Maximum Queue (ft)	274	510	506	114
Average Queue (ft)	63	311	315	50
95th Queue (ft)	177	442	457	91
Link Distance (ft)		1380	1380	1380
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	375			
Storage Blk Time (%)		3		
Queuing Penalty (veh)		3		

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	350	358	208	205	122	217	511	498	131	281	307	228
Average Queue (ft)	219	219	116	128	49	81	337	341	10	169	207	121
95th Queue (ft)	377	383	185	197	98	155	506	504	69	269	296	192
Link Distance (ft)	1022	1022	1022	1022			2167	2167	2167		1180	1180
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					535	535				400		
Storage Blk Time (%)							1				0	
Queuing Penalty (veh)							1				0	

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	T	T
Maximum Queue (ft)	178	164	170	413	436
Average Queue (ft)	69	97	113	285	301
95th Queue (ft)	143	151	168	400	415
Link Distance (ft)	1180			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)				0	
Queuing Penalty (veh)				0	

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	NB
Directions Served	L	L	Т	R	L	Т	R	L	L	Т	Т	R
Maximum Queue (ft)	168	105	131	152	200	720	244	137	148	135	148	68
Average Queue (ft)	79	14	61	50	173	395	138	73	87	61	74	18
95th Queue (ft)	140	61	118	103	237	909	352	127	135	141	156	49
Link Distance (ft)	261	261	261	261		796				444	444	
Upstream Blk Time (%)						11						
Queuing Penalty (veh)						0						
Storage Bay Dist (ft)					100		200	425	425			525
Storage Blk Time (%)					63	11						
Queuing Penalty (veh)					151	40						

#### Intersection: 2: Meridian Rd & Eastonville Rd

Movement	SB	SB	SB	SB
Directions Served	L			R
Maximum Queue (ft)	475	1135	1279	886
Average Queue (ft)	175	855	909	356
95th Queue (ft)	501	1481	1488	1300
Link Distance (ft)		1380	1380	1380
Upstream Blk Time (%)		7	15	9
Queuing Penalty (veh)		0	0	0
Storage Bay Dist (ft)	375			
Storage Blk Time (%)		30		
Queuing Penalty (veh)		30		

## Intersection: 18: Woodmen frontage rd & Right-In Only/Site Access

Movement	NB	SB	NE	SW
Directions Served	LR	LR	Т	Т
Maximum Queue (ft)	102	41	29	36
Average Queue (ft)	26	25	1	6
95th Queue (ft)	72	46	13	27
Link Distance (ft)	80	154	368	203
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	2			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	B16	B16	NB
Directions Served	L	L	Т	Т	R	L	Т	Т	R	Т	Т	L
Maximum Queue (ft)	426	437	280	293	42	571	900	923	500	117	120	203
Average Queue (ft)	276	287	102	116	9	123	558	573	176	20	21	91
95th Queue (ft)	433	442	214	210	29	448	996	1006	535	147	153	164
Link Distance (ft)			1455	1455			969	969		3861	3861	
Upstream Blk Time (%)							5	5				
Queuing Penalty (veh)							45	51				
Storage Bay Dist (ft)	450	450			400	475			400			125
Storage Blk Time (%)	0	0	0				20	28				5
Queuing Penalty (veh)	1	1	0				16	32				4

#### Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	NB	NB	SB	SB
Directions Served	Т	R	L	Т
Maximum Queue (ft)	157	45	165	70
Average Queue (ft)	18	14	146	20
95th Queue (ft)	74	34	188	53
Link Distance (ft)	526		154	154
Upstream Blk Time (%)			31	
Queuing Penalty (veh)			64	
Storage Bay Dist (ft)		200		
Storage Blk Time (%)	0			
Queuing Penalty (veh)	0			

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

EB	WB	NB	NB
TR	LT	LR	R
220	903	154	170
87	422	74	11
180	1115	141	79
1503	1983	154	154
		0	0
		1	0
	TR 220 87 180	TR         LT           220         903           87         422           180         1115	TR         LT         LR           220         903         154           87         422         74           180         1115         141           1503         1983         154

# Zone Summary

# Intersection: 1: Meridian Rd & Woodmen Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	L	Т	Т	L	L	Т	Т	R	L	L	Т
Maximum Queue (ft)	805	840	326	334	173	318	511	516	230	346	391	725
Average Queue (ft)	532	534	210	217	90	124	344	349	98	167	215	463
95th Queue (ft)	804	810	285	287	164	209	516	523	198	282	305	769
Link Distance (ft)	1022	1022	1022	1022			2167	2167	2167		1180	1180
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					1000	1000				400		
Storage Blk Time (%)											0	
Queuing Penalty (veh)											0	

Movement	NB	SB	SB	SB	SB
Directions Served	Т	L	L	Т	Т
Maximum Queue (ft)	708	440	434	350	372
Average Queue (ft)	421	292	305	223	239
95th Queue (ft)	719	416	429	335	350
Link Distance (ft)	1180			635	635
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		475	475		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 2: Meridian Rd & Eastonville Rd

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	NB
Directions Served	L	L	Т	R	L	Т	R	L	L	Т	Т	R
Maximum Queue (ft)	276	219	258	178	194	244	73	222	236	330	352	112
Average Queue (ft)	163	102	143	80	121	106	42	126	141	176	184	38
95th Queue (ft)	241	202	225	151	195	181	67	203	211	285	295	84
Link Distance (ft)	261	261	261	261		796				444	444	
Upstream Blk Time (%)	1		0									
Queuing Penalty (veh)	0		0									
Storage Bay Dist (ft)					100		200	425	425			525
Storage Blk Time (%)					22	10						
Queuing Penalty (veh)					80	43						

## Intersection: 2: Meridian Rd & Eastonville Rd

	0.0	0.5	0.5	0.5
Movement	SB	SB	SB	SB
Directions Served	L	Т	Т	R
Maximum Queue (ft)	284	559	534	95
Average Queue (ft)	70	325	331	35
95th Queue (ft)	204	496	510	70
Link Distance (ft)		1380	1380	1380
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	375			
Storage Blk Time (%)		6		
Queuing Penalty (veh)		5		

## Intersection: 18: Woodmen frontage rd & Right-In Only/Site Access

Movement	NB	SB	NE	SW
Directions Served	LR	LR	Т	Т
Maximum Queue (ft)	119	29	16	30
Average Queue (ft)	39	9	1	7
95th Queue (ft)	92	31	7	27
Link Distance (ft)	80	154	368	203
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	5			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB
Directions Served	L	L	Т	Т	R	L	Т	Т	R	L	Т	R
Maximum Queue (ft)	238	221	348	384	49	156	378	360	206	192	164	157
Average Queue (ft)	149	120	205	218	16	61	188	197	72	94	43	50
95th Queue (ft)	220	186	316	323	37	117	328	334	155	168	111	105
Link Distance (ft)			944	944			969	969			525	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	450	450			400	475			400	125		200
Storage Blk Time (%)				0						4	0	
Queuing Penalty (veh)				0						6	0	

## Intersection: 25: Golden Sage Rd & Woodmen Rd

Movement	SB	SB
Directions Served	L	Т
Maximum Queue (ft)	167	90
Average Queue (ft)	130	29
95th Queue (ft)	187	66
Link Distance (ft)	154	154
Upstream Blk Time (%)	16	
Queuing Penalty (veh)	36	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

#### Intersection: 26: Golden Sage Rd & Woodmen Frontage Rd

LT 641 258 527	LR 170 115	R 173 17
258	115	
		17
527		
021	194	99
1983	154	154
	5	0
	20	1
	1983	5

# Zone Summary