

## Gwen Dall

---

**From:** Elizabeth Steffens  
**Sent:** Wednesday, May 12, 2021 12:48 PM  
**To:** Hunyadi - DNR, John  
**Cc:** Gwen Dall  
**Subject:** NOI - Non-Jurisdictional Dam - WWSD Rolling Hills Water Tank Site Water Quality Structure  
**Attachments:** 102.121\_NOI\_NJ Form w attachments.pdf

John,

Attached is the submittal for a proposed non-jurisdictional structure at the Widefield Water and Sanitation District Rolling Hills Water Tank Site (Address: TBD, Located approx. 2,168 ft southwest of Drennan Road and Mockingbird Lane intersection, Colorado Springs, CO 80908) to serve as a water quality structure and in accordance with Section 37-87-125, C.R.S. I have included the drainage basin plans and sand filter basin sizing calculations in the attachments. Let me know if you have any questions or comments.

Thanks,  
Elizabeth

**Elizabeth Steffens, P.E.**  
**JDS-HYDRO CONSULTANTS, INC.**

5540 Tech Center Dr., Suite 100  
Colorado Springs, CO 80919

Office: 719-227-0072 Ext. 113 | Fax: 719-471-3401 |  
Excelling in municipal water and wastewater solutions

[www.jdshydro.com](http://www.jdshydro.com)  
[esteffens@jdshydro.com](mailto:esteffens@jdshydro.com)



**COLORADO**  
Division of Water Resources  
Department of Natural Resources  
[dwr.colorado.gov](http://dwr.colorado.gov) P 303.866.3581

## NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE<sup>1</sup>

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

### OWNER INFORMATION

Name: Widefield Water and Sanitation District Telephone/E-Mail: (719) 390-7111 / [info@wwsdonline.com](mailto:info@wwsdonline.com)  
Address: 8495 Fontaine Blvd. Colorado Springs CO 80925  
Street / P.O. Box/ Rural Route City State Zip Code  
Responsible Person: Robert Bannister, District Engineer Telephone/E-Mail: (719) 390-7111 / [rob@wwsdonline.com](mailto:rob@wwsdonline.com)  
Address: 8495 Fontaine Blvd. Colorado Springs CO 80925  
Street / P.O. Box/ Rural Route City State Zip Code  
Contractor: Preload, LLC / Glacier Construction Telephone/E-Mail: (251) 709-5734 [wcooksey@preload.com](mailto:wcooksey@preload.com)

### STRUCTURE INFORMATION

Name of Dam: WWSD Rolling Hills Tank SFB Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, and GPS Point taken at crest of dam above streamline/outlet)

- Section: 1 Township: 15S Range: 65W 6th P.M.
- Northing 4292157.94 meters, Easting 532889.32 meters (Datum should be UTM, NAD 83)

#### Dam Dimensions:

- Vertical Height<sup>2</sup>: 5 ft., Length: 70 ft., Crest Width: 5 ft., Slopes: U/S: 4 (H:1V), D/S 12.5 (H:1V)

#### Reservoir:

- Surface Area<sup>1</sup>: .072 acres, Capacity<sup>1</sup>: 0.048 acre-feet, Drainage Area\*: 1.76 acres  
(If drainage area is unknown leave blank and a spillway size will be assigned):

#### Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 1 ft., Side Slopes: 4 H:1V, Freeboard<sup>3</sup>: 1.75 ft

Outlet Conduit Type: RCP, Size: 15 inches, Location: Outlet Structure

Stream Name or Water Source<sup>4</sup>: Site drainage only Proposed Water Use: Water Quality / Sand Filter Basin

Water Court Case or WDID : N/A  
(Water District Identification Number)

*Robert K Bannister - 5/12/2021*  
Signature of Owner Date

#### Office Use Only

<b>DIVISION ENGINEER'S REQUIREMENTS:</b> _____ _____ _____		
--	--	--

**Dam I.D.** \_\_\_\_\_ Signature of Division Engineer Date

<sup>1</sup> A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less and a surface area of 20 acres or less and a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.

<sup>2</sup> "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.

<sup>3</sup> "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.

<sup>4</sup> If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at [dwr.colorado.gov](http://dwr.colorado.gov))

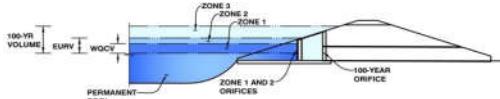
# *SFB Sizing Calculations*

## DETENTION BASIN STAGE-STORAGE TABLE BUILDER

*MHFD-Detention, Version 4.00 (December 2019)*

Project: Widefield Water and Sanitation District - Rolling Hills Tank

Basin ID: Subbasin C



#### **Example Zone Configuration (Retention Pond)**

## Watershed Information

Selected BMP Type =	<b>SF</b>	Note: L / W Ratio > 8
Watershed Area =	1.76	acres L / W Ratio = 13.04

**Note: L / W Ratio > 8**

**L / W Ratio = 13.04**

Selected BMP Type =	<b>SF</b>	<b>Note: L / W Ratio &gt; 8</b>
Watershed Area =	1.76	acres
Watershed Length =	1,000	ft
Watershed Length to Centroid =	650	ft
Watershed Slope =	0.012	ft/ft
Watershed Impermeability =	36.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	100.0%	percent
Target WQCV Drain Time =	12.0	hours
Location for 1-h Rainfall Depth =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

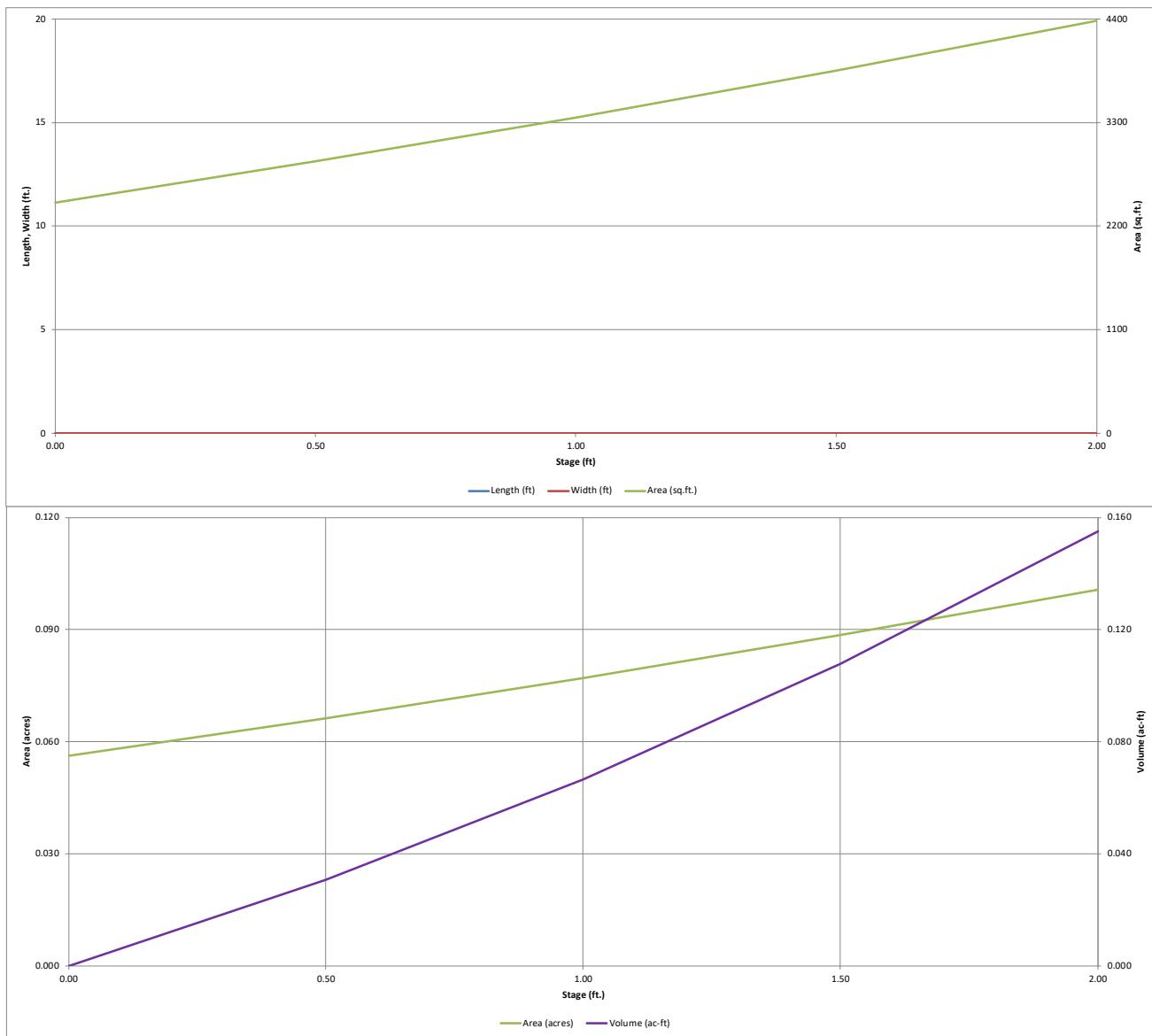
The Unchecked Controls Check (Graphical Procedure)		Optional User Overrides
Water Quality Capture Volume (WQCV) =	0.020	acre-feet
Excess Urban Runoff Volume (EURV) =	0.058	acre-feet
2-yr Runoff Volume ( $P_1 = 1.19 \text{ in.}$ ) =	0.075	acre-feet
5-yr Runoff Volume ( $P_1 = 1.5 \text{ in.}$ ) =	0.116	acre-feet
10-yr Runoff Volume ( $P_1 = 1.75 \text{ in.}$ ) =	0.151	acre-feet
25-yr Runoff Volume ( $P_1 = 2 \text{ in.}$ ) =	0.195	acre-feet
50-yr Runoff Volume ( $P_1 = 2.25 \text{ in.}$ ) =	0.233	acre-feet
100-yr Runoff Volume ( $P_1 = 2.52 \text{ in.}$ ) =	0.280	acre-feet
500-yr Runoff Volume ( $P_1 = 3.14 \text{ in.}$ ) =	0.376	acre-feet
Approximate 2-yr Detention Volume =	0.051	acre-feet
Approximate 5-yr Detention Volume =	0.081	acre-feet
Approximate 10-yr Detention Volume =	0.093	acre-feet
Approximate 25-yr Detention Volume =	0.103	acre-feet
Approximate 50-yr Detention Volume =	0.108	acre-feet
Approximate 100-yr Detention Volume =	0.128	acre-feet

### Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.020	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.039	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.070	acre-feet
Total Detention Basin Volume =	0.128	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft <sup>3</sup>
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H <sub>total</sub> ) =	user	ft
Depth of Trickle Channel (H <sub>Tc</sub> ) =	N/A	ft
Slope of Trickle Channel (S <sub>Tc</sub> ) =	N/A	ft/ft
Slopes of Main Basin Sides (S <sub>main</sub> ) =	user	H:V
Basin Length-to-Width Ratio (R <sub>L/W</sub> ) =	user	
Initial Surcharge Area (A <sub>ISV</sub> ) =	user	ft <sup>2</sup>
Surcharge Volume Length (L <sub>SV</sub> ) =	user	ft
Surcharge Volume Width (W <sub>SV</sub> ) =	user	ft
Depth of Basin Floor (H <sub>FLOOR</sub> ) =	user	ft
Length of Basin Floor (L <sub>FLOOR</sub> ) =	user	ft
Width of Basin Floor (W <sub>FLOOR</sub> ) =	user	ft
Area of Basin Floor (A <sub>FLOOR</sub> ) =	user	ft <sup>2</sup>
Volume of Basin Floor (V <sub>FLOOR</sub> ) =	user	ft <sup>3</sup>
Depth of Main Basin (H <sub>MAIN</sub> ) =	user	ft
Length of Main Basin (L <sub>MAIN</sub> ) =	user	ft
Width of Main Basin (W <sub>MAIN</sub> ) =	user	ft
Area of Main Basin (A <sub>MAIN</sub> ) =	user	ft <sup>2</sup>
Volume of Main Basin (V <sub>MAIN</sub> ) =	user	ft <sup>3</sup>
Calculated Total Basin Volume (V <sub>BASIN</sub> ) =	user	acre-feet

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

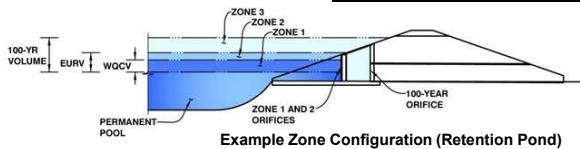
*MHFD-Detention, Version 4.00 (December 2019)*



# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)

**Project:** Widefield Water and Sanitation District - Rolling Hills Tank  
**Basin ID:** Subbasin C



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.34	0.020	Filtration Media
Zone 2 (EURV)	0.90	0.039	Circular Orifice
Zone 3 (100-year)	1.73	0.070	Weir&Pipe (Circular)
Total (all zones)		0.128	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	1.83	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	0.35	inches

Underdrain Orifice Area =	0.0	ft <sup>2</sup>
Underdrain Orifice Centroid =	0.01	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate	
WQ Orifice Area per Row =	N/A
Elliptical Half-Width =	N/A
Elliptical Slot Centroid =	N/A
Elliptical Slot Area =	N/A

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

Stage of Orifice Centroid (ft)	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Stage of Orifice Centroid (ft)	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

Zone 2 Circular	Not Selected
Invert of Vertical Orifice =	0.05
Depth at top of Zone using Vertical Orifice =	0.50
Vertical Orifice Diameter =	1.50

Calculated Parameters for Vertical Orifice	
Zone 2 Circular	Not Selected
Vertical Orifice Area =	0.01
Vertical Orifice Centroid =	0.06

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, Ho =	0.30	N/A
Overflow Weir Front Edge Length =	4.00	N/A
Overflow Weir Grate Slope =	3.00	N/A
Horiz. Length of Weir Sides =	4.00	N/A
Overflow Grate Open Area % =	70%	N/A
Debris Clogging % =	0%	N/A

Calculated Parameters for Overflow Weir	
Zone 3 Weir	Not Selected
Height of Grate Upper Edge, H <sub>t</sub> =	1.63
feet	N/A
Overflow Weir Slope Length =	4.22
feet	N/A
Grate Open Area / 100-yr Orifice Area =	153.92
N/A	ft <sup>2</sup>
Overflow Grate Open Area w/o Debris =	11.81
N/A	ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	11.81
N/A	ft <sup>2</sup>

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Zone 3 Circular	Not Selected
Depth to Invert of Outlet Pipe =	1.92
Circular Orifice Diameter =	3.75

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate	
Zone 3 Circular	Not Selected
Outlet Orifice Area =	0.08
Outlet Orifice Centroid =	0.16
Half-Central Angle of Restrictor Plate on Pipe =	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	0.75	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	1.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway	
Spillway Design Flow Depth=	0.36
Stage at Top of Freeboard =	2.11
Basin Area at Top of Freeboard =	0.10
Basin Volume at Top of Freeboard =	0.16

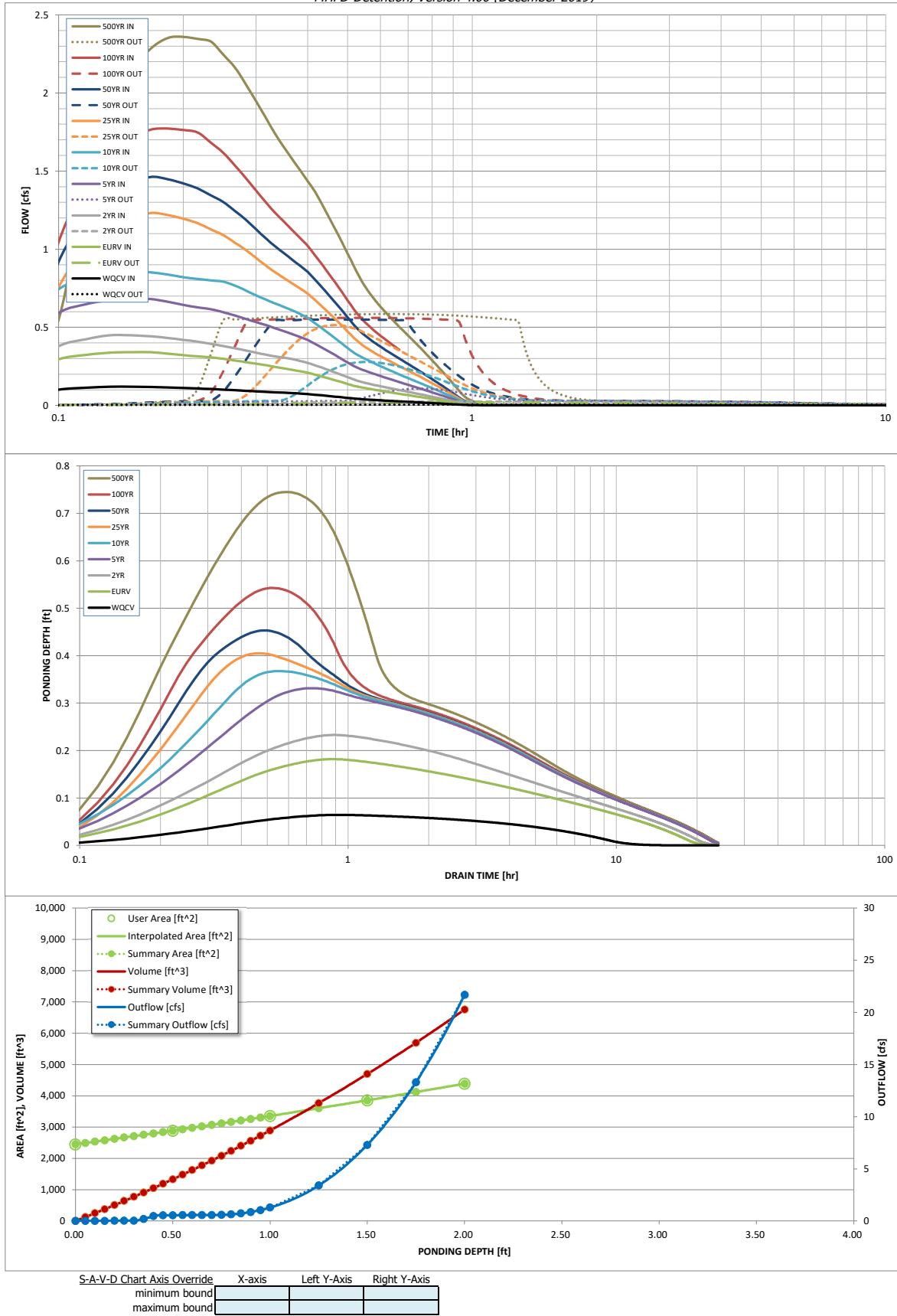
## Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	0.020	0.058	0.075	0.116	0.151	0.195	0.233	0.280	0.376
Inflow Hydrograph Volume (acre-ft) =	0.004	0.012	0.015	0.023	0.030	0.039	0.047	0.056	0.075
CUHP Predevelopment Peak Q (cfs) =	0.0	0.0	0.1	0.3	0.4	0.7	0.8	1.1	1.5
OPTIONAL Override Predevelopment Peak Q (cfs) =	0.0	0.0							
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.08	0.17	0.24	0.38	0.47	0.62	0.86
Peak Inflow Q (cfs) =	0.1	0.3	0.4	0.7	0.9	1.2	1.5	1.8	2.4
Peak Outflow Q (cfs) =	0.005	0.020	0.0	0.1	0.278	0.5	0.5	0.561	0.6
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.4	0.7	0.8	0.7	0.5	0.4
Structure Controlling Flow =	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.0	0.0	0.0	0.0	0.0	0.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	12	19	21	22	22	21	20	20	18
Time to Drain 99% of Inflow Volume (hours) =	14	21	22	24	24	23	23	23	23
Maximum Pending Depth (ft) =	0.06	0.18	0.23	0.33	0.37	0.41	0.45	0.54	0.75
Area at Maximum Pending Depth (acres) =	0.06	0.06	0.06	0.06	0.06	0.06	0.07	0.07	0.07
Maximum Volume Stored (acre-ft) =	0.003	0.010	0.013	0.020	0.022	0.024	0.027	0.033	0.047

## DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.00 (December 2019)*



S-A-V-D Chart Axis Override  
 minimum bound      X-axis      Left Y-Axis      Right Y-Axis  
 maximum bound

# DETENTION BASIN OUTLET STRUCTURE DESIGN

### *Outflow Hydrograph Workbook Filename:*

## Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

# DETENTION BASIN OUTLET STRUCTURE DESIGN

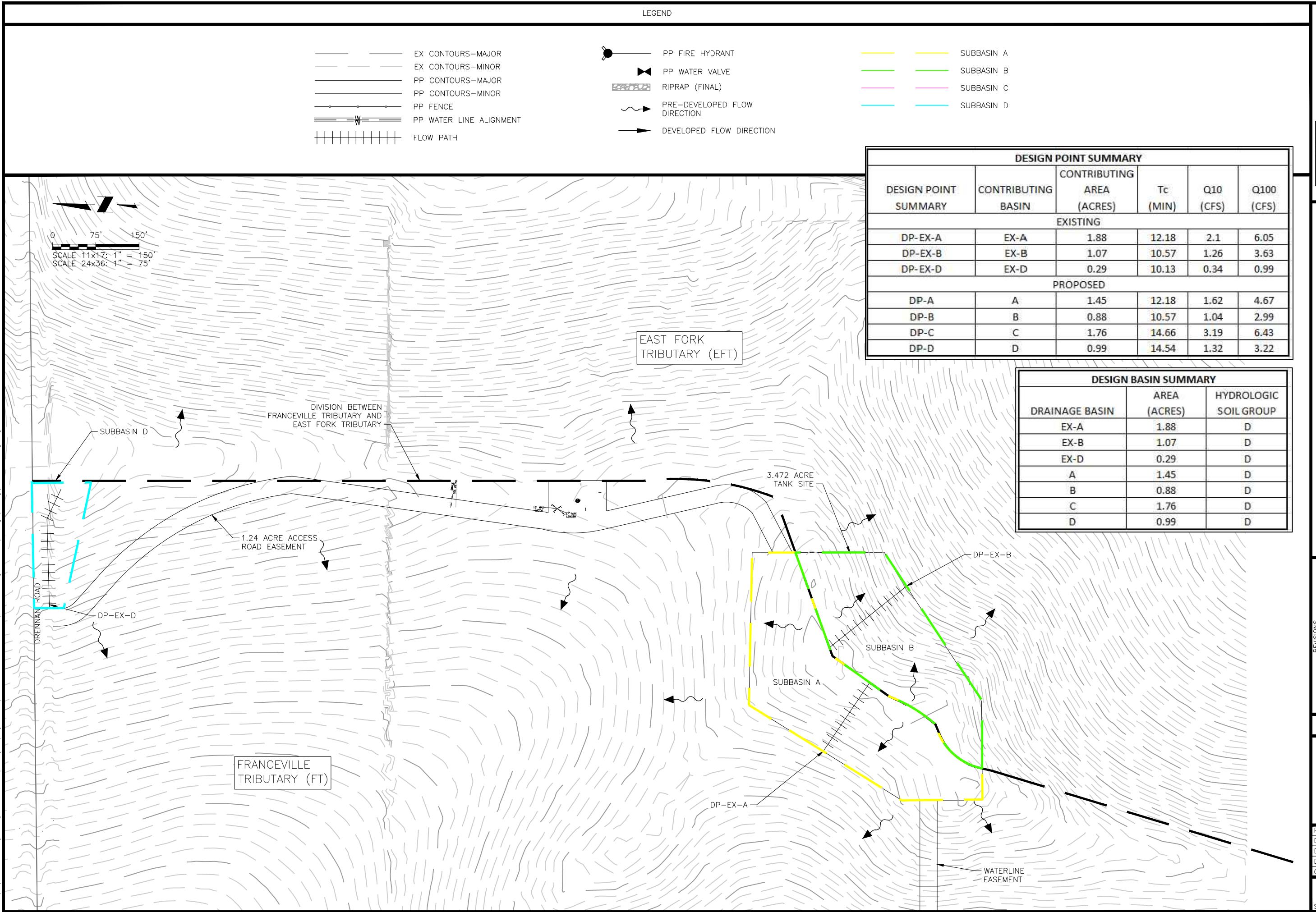
*MHFD-Detention, Version 4.00 (December 2019)*

## Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

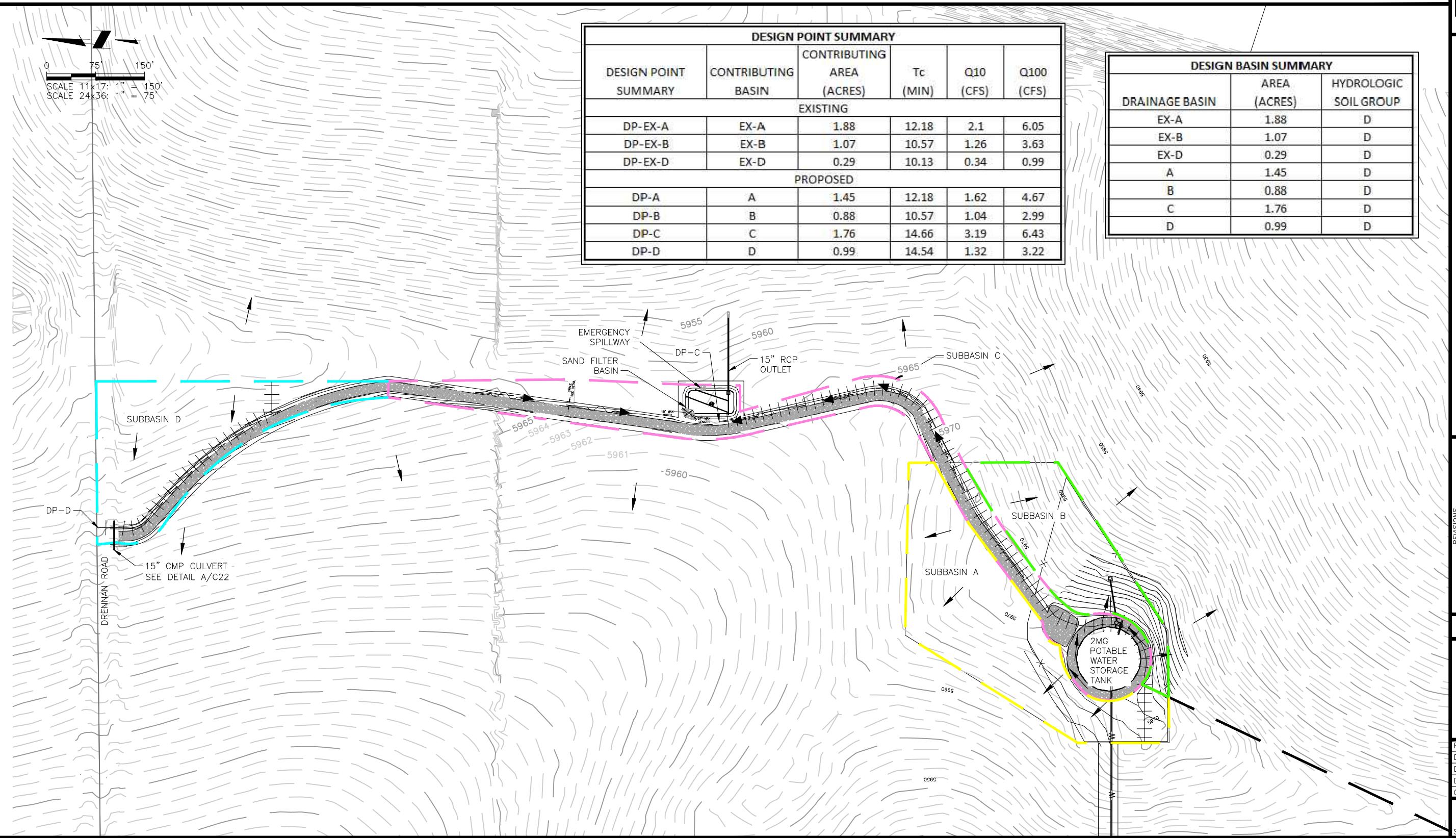
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

# *Drainage Plans*



**JDS-HYDRO CONSULTANTS, INC.**  
5540 TECH CENTER DR, SUITE 100  
COLORADO SPRINGS, COLORADO 80919  
(719) 227-0072

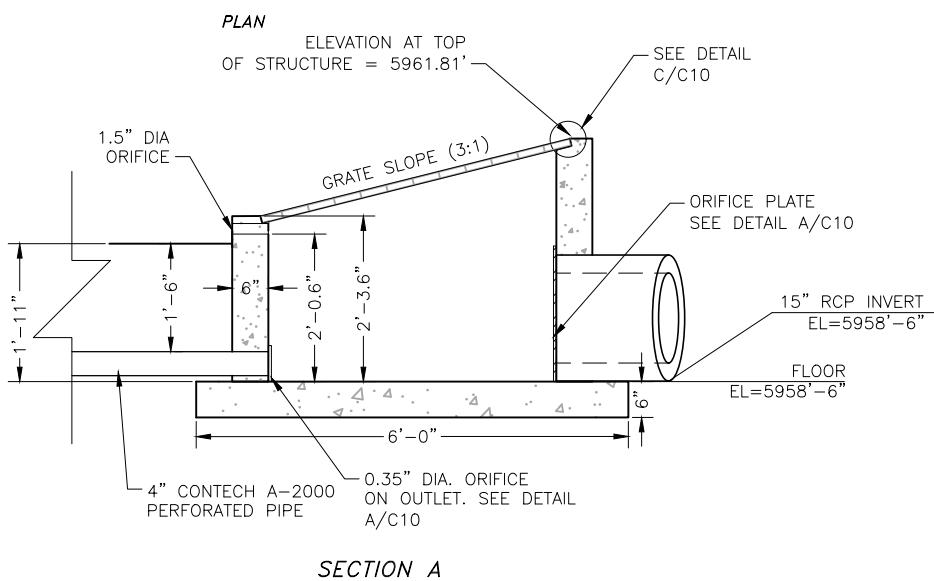
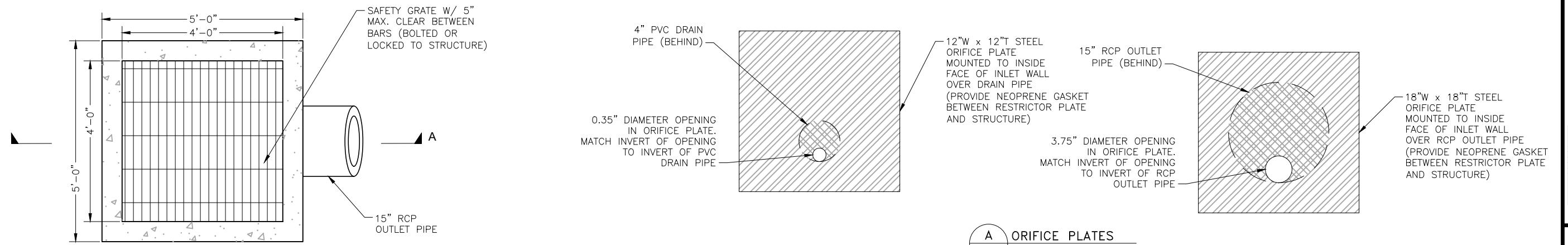
DISCLAIMER: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ANY ERRORS OR OMISSIONS SHALL BE NOTIFIED TO THE OWNER AND CORRECTED AS SOON AS POSSIBLE. NO LIABILITY FOR INACCURATE DRAWINGS OR INFORMATION IS ASSUMED.



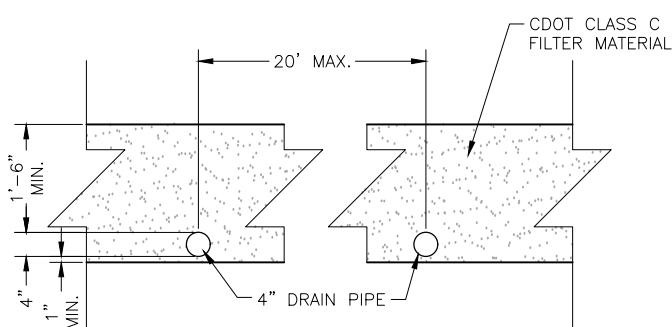
**DS-HYDRO CONSULTANTS, INC.**  
5540 TECH CENTER DR, SUITE 100  
COLORADO SPRINGS, COLORADO 80919  
(719) 227-0072

DISCLAIMER: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ANY ERRORS OR OMISSIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. DS-HYDRO CONSULTANTS, INC. SHALL NOT BE HELD LIABLE FOR ANY DAMAGES, WHETHER DIRECT OR INDIRECT, ARISING OUT OF THE USE OF THIS DRAWING.

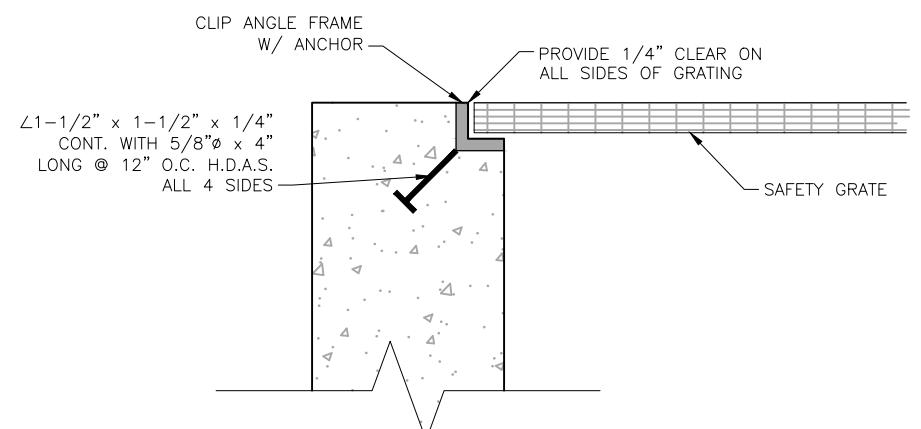




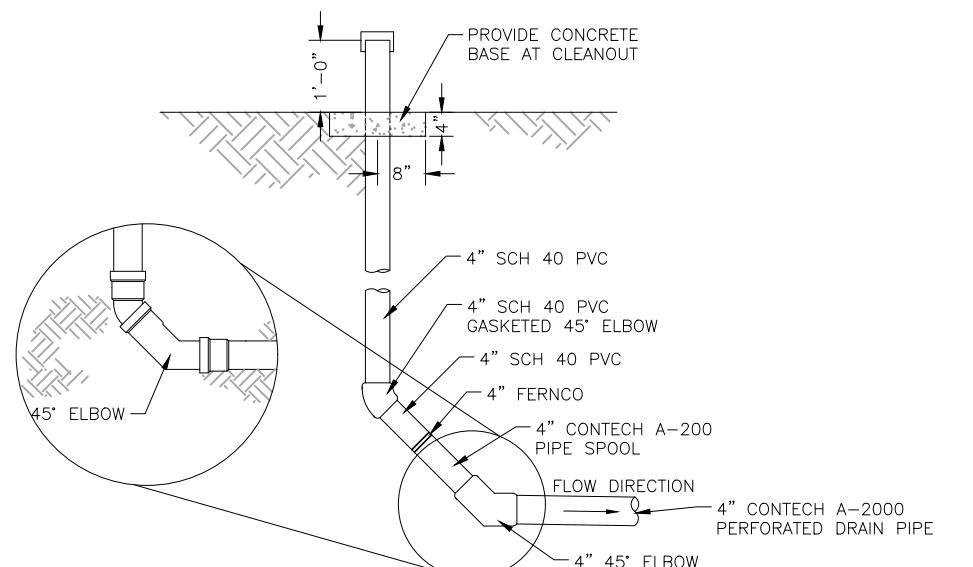
**B OUTLET STRUCTURE**  
C10 SCALE: 3/8" = 1'-0"



**D DRAINAGE BASIN SECTION (SEE SHEET C9)**  
C10 SCALE: 3/8" = 1'-0"



**C GRATING DETAIL**  
C10 SCALE: N.T.S.

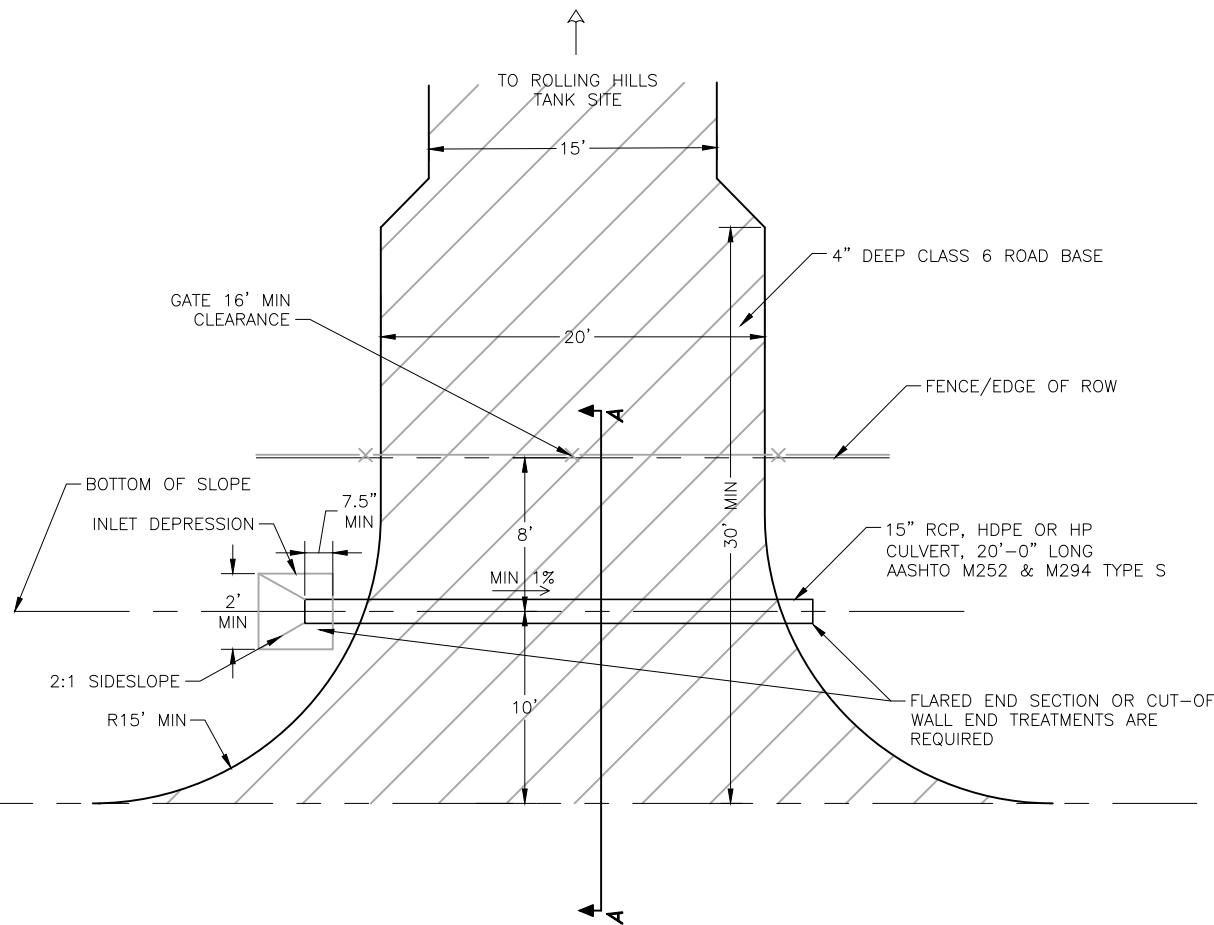


**E CLEANOUT**  
C10 SCALE: N.T.S.

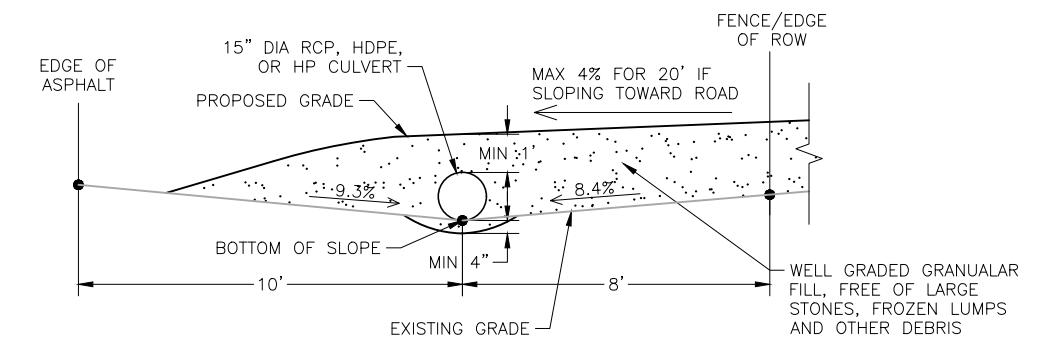
# FOR REFERENCE ONLY

NO.	DESCRIPTION	REVISIONS	BY APP.	DATE
1				
2				
3				
4				
5				
6				
7				

**TO BID**



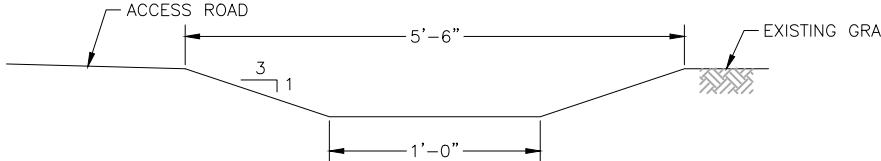
DRENNAN ROAD (CITY OF COLORADO SPRINGS R.O.W.)



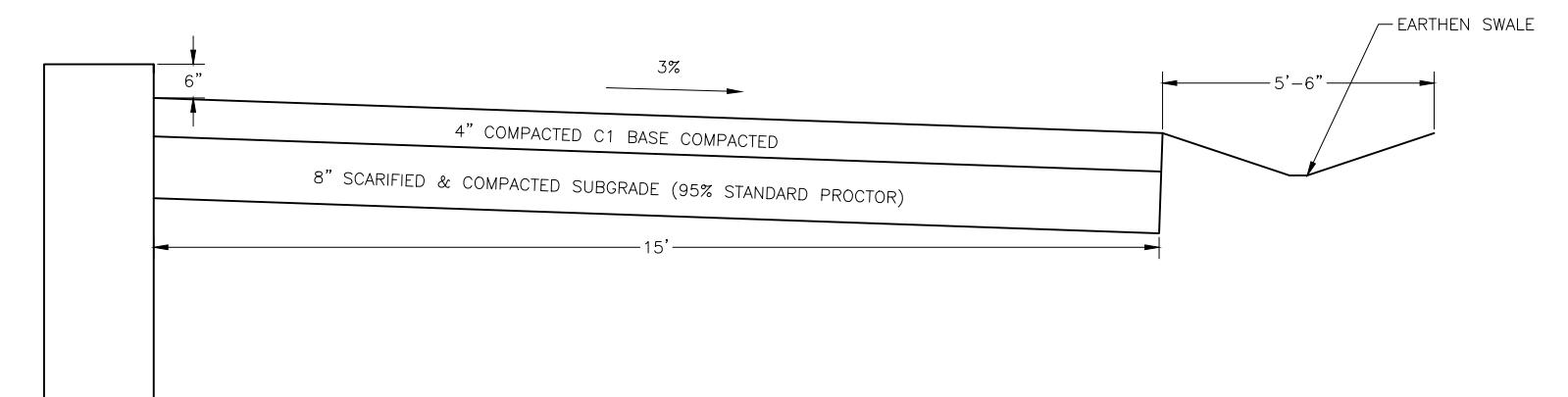
SECTION A-A  
SCALE: N.T.S.

NOTES:

1. SURFACE TREATMENT OF DRIVEWAY TO BE CLASS 6 ROAD BASE AT 4" DEEP.
2. DRAIN PIPE TO BE 15" DIAMETER MINIMUM.
3. PIPE SLOPE TO BE CONSISTANT WITH FLOW LINE OF DITCH, MINIMUM OF 1%.
4. BACKFILL TO BE PLACED IN 6" LAYERS, DEPOSITED AND COMPAKTED ON ALTERNATING SIDES OF THE PIPE.
5. MEASUREMENTS ARE APPROXIMATE AND SHOULD BE FIELD VERIFIED PRIOR TO COMMENCING CONSTRUCTION.



B TYPICAL GRASS SWALE SECTION  
C8 SCALE: N.T.S.



C TANK MAINTENANCE ROAD  
C8 SCALE: N.T.S.

FOR REFERENCE ONLY

NO.	DESCRIPTION	BY APP.	DATE	REVISIONS
1				
2				
3				
4				
5				
6				
7				

TO BID

Project No.: 102.121  
Date: 02/22/21  
Design: GJD  
Drawn: SNW  
Check: JPM