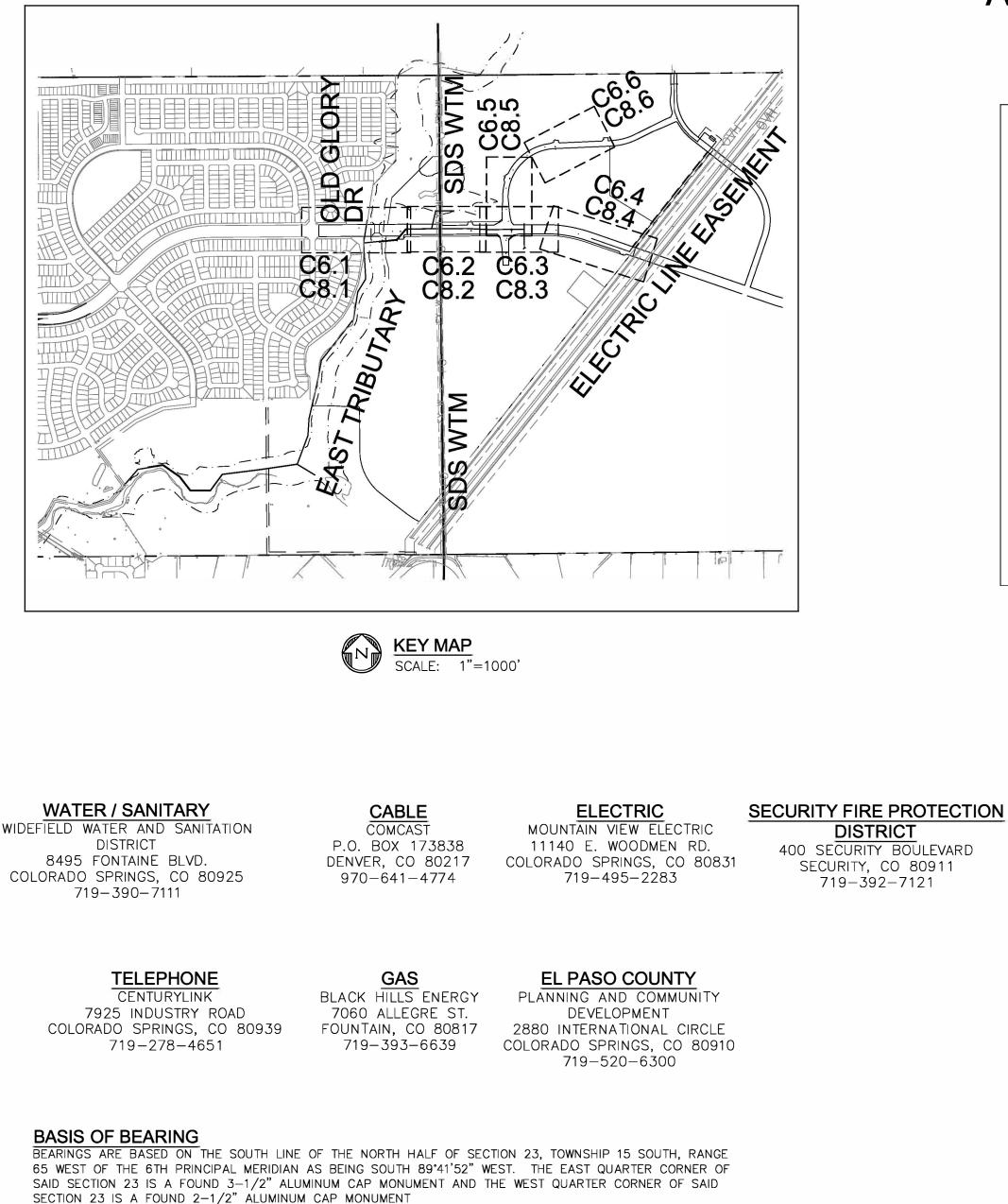
CONSTRUCTION PLANS FOR **FONTAINE BOULEVARD & LAMPREY DRIVE** WITHIN LORSON RANCH EAST STREET, STORM SEWER, AND WATERMAIN CONSTRUCTION PLANS

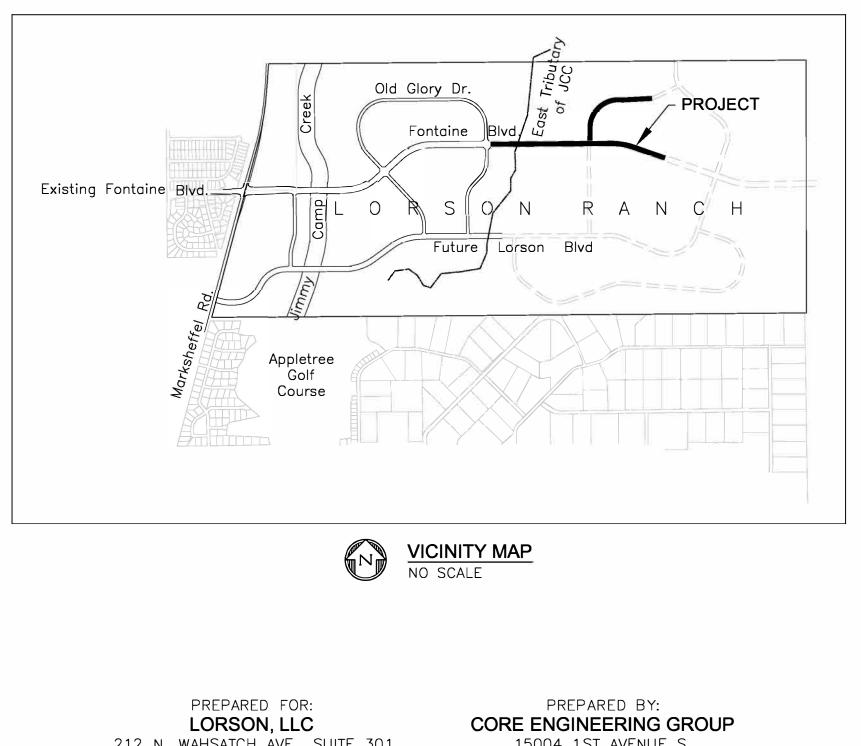


BENCHMARK

FIMS MONUMENT F204 LOCATED AT THE NORTHWEST CORNER OF FONTAINE BLVD AND COTTONWOOD GROVE DR. ELEVATION 5724.072 (N.G.V.D. 29)

TRAFFIC CONTROL NOTE

THE CONTRACTOR SHALL PROVIDE ALL TRAFFIC CONTROL DEVICES AND MONITORING NECESSARY TO SAFELY COMPLETE THE WORK SHOWN IN THESE CONSTRUCTION DOCUMENTS IN CONFORMANCE WITH M.U.T.C.D. GUIDELINES. THE CONTRACTOR SHALL COMPLETE ALL NECESSARY WORK FOR PLAN REVIEW, PERMITS AND PROCESSING. TRAFFIC CONTROL WILL NOT BE PAID SEPARATELY BUT IS INCLUDED IN THE COST OF THE PROJECT.



212 N. WAHSATCH AVE., SUITE 301 COLORADO SPRINGS, CO 80903 719-635-3200 CONTACT: JEFF MARK

15004 1ST AVENUE S BURNSVILLE, MN 55306 719-570-1100 CONTACT: RICHARD L. SCHINDLER P.E.

DISTRICT APPROVAL (WATER)

THE WIDEFIELD WATER AND SANITATION DISTRICT RECOGNIZES THE DESIGN ENGINEER AS HAVING RESPONSIBILITY FOR THE DESIGN. THE WIDEFIELD WATER AND SANITATION DISTRICT HAS LIMITED ITS SCOPE OF REVIEW ACCORDINGLY.

> WIDEFIELD WATER AND SANITATION DISTRICT WATER DESIGN APPROVAL

DATE ______ BY _____

PROJECT NO. _____

IN CASE OF ERRORS OR OMISSIONS WITH THE WATER DESIGN AS SHOWN ON THIS DOCUMENT THE STANDARDS AS DEFINED IN THE "RULES AND REGULATIONS FOR INSTALLATION OF WATER MAINS AND SERVICES" SHALL RULE.

APPROVAL EXPIRES 180 DAYS FROM DESIGN APPROVAL

DISTRICT APPROVAL (WASTEWATER)

THE WIDEFIELD WATER AND SANITATION DISTRICT RECOGNIZES THE DESIGN ENGINEER AS HAVING RESPONSIBILITY FOR THE DESIGN. THE WIDEFIELD WATER AND SANITATION DISTRICT HAS LIMITED ITS SCOPE OF REVIEW ACCORDINGLY.

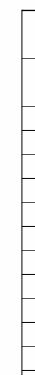
> WIDEFIELD WATER AND SANITATION DISTRICT WASTEWATER DESIGN APPROVAL

DATE ______ BY _____

PROJECT NO. _____

IN CASE OF ERRORS OR OMISSIONS WITH THE WATER DESIGN AS SHOWN ON THIS DOCUMENT THE STANDARDS AS DEFINED IN THE "RULES AND REGULATIONS FOR INSTALLATION OF WATER MAINS AND SERVICES" SHALL RULE.

APPROVAL EXPIRES 180 DAYS FROM DESIGN APPROVAL



DEVELOPER'S STATEMENT

THE UNDERSIGNED OWNER/DEVELOPER HAS READ AND WILL COMPLY WITH ALL THE REQUIREMENTS SPECIFIED IN THESE CONSTRUCTION PLANS AND THE ACCOMPANYING DRAINAGE REPORT

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TITLE					
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COLC WATE
APPROVED BY:
DATE:
PROJECT NUMBER: 2017-
WORK ORDER NUMBER:
CSU SHEET OF
APPROVAL EXPIRES ONE (RESUBMITTAL OF THESE PL CONSTRUCTION DOES NOT

CONSTRUCTION APPROVAL

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

FILED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, DRAINAGE CRITERIA MANUALS VOLUME 1 AND 2, AND ENGINEERING CRITERIA MANUAL AS AMENDED. CONSTRUCTION DOCUMENTS WILL BE VALID FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO COUNTY ENGINEER.

CONDITIONS:

ENGINEER'S APPROVAL

THESE DETAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECTION AND SUPERVISION. SAID PLANS AND SPECIFICATIONS HAVE BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR DETAILED ROADWAY, DRAINAGE, GRADING AND EROSION CONTROL PLANS AND SPECIFICATIONS, AND SAID PLANS AND SPECIFICATIONS ARE IN CONFORMITY WITH APPLICABLE MASTER DRAINAGE PLANS AND MASTER TRANSPORTATION PLANS. SAID PLANS AND SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR ROADWAY AND DRAINAGE FACILITIES ARE DESIGNED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARATION OF THESE DETAILED PLANS AND SPECIFICATIONS.

RICHARD L. SCHINDLER, P.E. # 33997 FOR AND ON BEHALF OF CORE ENGINEERING GROUP



CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES

SHEET INDEX								
SHEET NO.	SHEET DESCRIPTION							
C1.1	COVER SHEET							
C1.2	NOTES							
C1.3	TYPICAL STREET SECTIONS							
C2.1	HORIZONTAL CONTROL PLAN							
C5.1-C5.3	SIGNING AND STRIPING PLAN							
C6.1-C6.9	STREET/STORM PLAN AND PROFILE							
C7.1	STORM SEWER LATERAL CONSTRUCTION							
C8.1-C8.9	WATERMAIN CONSTRUCTION							
C9.1-C9.3	ROUNDABOUT CONSTRUCTION							
C10.1-C10.3	STREET/STORM DETAILS							
S1	STRUCTURAL DETAILS							
C12.1	WATERMAIN DETAILS							

DATE

212 N. WAHSATCH AVE. SUITE 301 COLORADO SPRINGS, CO 80903

> **ORADO SPRINGS UTILITIES** ER PLAN DESIGN APPROVAL

(1) YEAR FROM THE DATE ABOVE AND LÁNS FOR REVIEW AND APPROVAL IS REQUIRED IF BEGIN DURING THIS PERIOD.

JENNIFER IRVINE, COUNTY ENGINEER/ECM ADMINISTRATOR

DATE

FONTAINE BLVD STREET/STORM AND No. DESCRIPTION FONTAINE BLVD STREET/STORM AND No. DESCRIPTION WATER/SEWER CONSTRUCTION 27 Street DESCRIPTION VATER/SEWER CONSTRUCTION 28 Street DESCRIPTION LORSON RANCH EAST CONTAINE BLVD IN DESCRIPTION CORSON RANCH EAST 212 N. WHENCH AND DESCRIPTION COLORADO SPRINGS, COLORADO BODO3 COLORADO SPRINGS, COLORADO BODO3 DORSON, LLC	CORE	ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
TORM AND RUCTION BUCTION AST AST FONTAINE BLVD IN FONTAINE BLVD IN FONTAINE BLVD IN FONTAINE BLVD. COLORADO SPRINGS, COLORADO		PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK
DRAWN: RLS DESIGNED: RLS CHECKED: RLS NOLONE NOLONE LSE		DJECT: FONTAINE BLVD IN ORSON RANCH EAST FONTAINE BLVDOLD GLORY DR COLORADO SPRINGS, COLORADO
	DRAWN: DESIGNED	RLS D: RLS
		ER/SEWER CONSTRUCTION LORSON RANCH EAST

TOTAL SHEETS: 34

CONSTRUCTION NOTES

- ALL WORK SHALL COMPLY WITH THE CODES AND POLICIES FOR EL PASO COUNTY.
- 2. EXISTING TOPOGRAPHIC INFORMATION SHOWN ON THIS GRADING PLAN WAS OBTAINED FROM DREXEL, BARRELL & CO., JULY, 2005. THE CONTRACTOR SHALL BE RESPONSIBLE TO EXAMINE THE SITE AND BE FAMILIAR WITH THE EXISTING CONDITIONS.
- 3. DEPTH OF MOISTURE-DENSITY CONTROL FOR THIS PROJECT SHALL BE AS FOLLOWS: BASE OF ALL CUTS AND FILLS - 12 INCHES. FULL DEPTH OF ALL EMBANKMENTS
- 4. THE CONTRACTOR IS RESPONSIBLE FOR THE RE-ESTABLISHMENT OF ALL SURVEY MONUMENTS DISTURBED WITHIN THE PROJECT LIMITS.
- 5. THE CONTRACTOR SHALL PROTECT ALL WORK AREAS AND FACILITIES FROM FLOODING AT ALL TIMES. AREAS AND FACILITIES SUBJECTED TO FLOODING, REGARDLESS OF THE SOURCE OF WATER, SHALL BE PROMPTLY DEWATERED AND RESTORED.
- 6. PRIOR TO PAVING OPERATIONS, THE ENTIRE SUBGRADE SHALL BE PROOF-ROLLED WITH A LOADED 988 FRONT-END LOADER OR SIMILAR HEAVY RUBBER TIRED VEHICLE (GVW OF 50,000 POUNDS WITH 18 KIP PER AXLE AT TIRE PRESSURES OF 90 PSI) TO DETECT ANY SOFT OR LOOSE AREAS. IN AREAS WHERE SOFT OR LOOSE SOILS, PUMPING OR EXCESSIVE MOVEMENT IS OBSERVED, THE EXPOSED MATERIALS SHALL BE OVER-EXCAVATED TO A MINIMUM DEPTH OF TWO FEET BELOW PROPOSED FINAL GRADE OR TO A DEPTH AT WHICH SOILS ARE STABLE. AFTER THIS HAS BEEN COMPLETED, THE EXPOSED MATERIALS SHALL BE SCARIFIED TO A DEPTH OF 12 INCHES AND MOISTURE CONDITIONED. THE SUBGRADE SHALL THEN BE UNIFORMLY COMPACTED TO A MINIMUM OF 95% OF STANDARD PROCTOR DENSITY (ASTMM D-698) AT 0 TO +4.0% OF OPTIMUM MOISTURE CONTENT FOR A-6 AND A-7-6 SOILS ENCOUNTERED. OTHER SUBGRADE TYPES SHALL BE UNIFORMLY COMPACTED TO A MINIMUM OF 95% OF MODIFIED PROCTOR DENSITY (ASTM D-1557) AT PLUS OR MINUS 2.0% OF OPTIMUM MOISTURE CONTENT. AREAS WHERE STABLE NATURAL SOILS ARE ENCOUNTERED AT PROPOSED SUBGRADE DEEVATION SHALL ALSO BE SCARIFIED (18 INCHES FOR A-7-6 SOILS BELOW FULL-DEPTH ASPHALT CONCRETE) AND COMPACTED AS OUTLINED ABOVE PRIOR TO PAVING OPERATIONS SUBGRADE FULL SHALL BE PLACED IN SIX-INCH LIFTS AND UNIFORMLY COMPACTED, MEETING THE REQUIREMENTS AS PREVIOUSLY DESCRIBED.
- SUBGRADE MATERIALS DEEMED UNSUITABLE BY THE ENGINEER SHALL BE EXCAVATED, DISPOSED OF AND REPLACED WITH APPROVED MATERIALS.
- 8. FILL SHALL BE PLACED IN 8-INCH MAXIMUM LOOSE LIFTS AND SHALL BE COMPACTED PRIOR TO SUCCESSIVE LIFTS.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR PREVENTING AND CONTROLLING EROSION DURING CONSTRUCTION ACTIVITIES AT ALL TIMES DURING GRADING AND CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE THE FOLLOWING EROSION AND SEDIMENT CONTROL MEASURES:
 - HAY BALE BARRIERS WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
 - SILT FENCE WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
 - TEMPORARY SEDIMENTATION BASINS WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
 - MULCHING AND SEEDING OF EXCESSIVE SLOPED AREAS AS NEEDED OR AS DIRECTED BY THE ENGINEER.
 - TEMPORARY VEHICLE TRACKING CONTROL AS NEEDED AND/OR DIRECTED BY THE ENGINEER.
 - CONCRETE WASH AREAS. - INLET PROTECTION.

THESE AND ALL EROSION CONTROL BEST MANAGEMENT PRACTICES AS SHOWN IN THE GRADING AND EROSION CONTROL PLANS SHALL BE STRICTLY ADHERED TO.

10. FINISHED CONTOURS/SPOT ELEVATIONS SHOWN HEREON REPRESENT FINISHED GRADES. ALL GRADING SHALL CONFORM TO THE GEOTECHICAL RECOMMENDATIONS FOR LORSON RANCH EAST PREPARED BY RMG, "PRELIMINARY SOILS AND GEOLOGY FOR LORSON RANCH EAST", DATED SEPTEMBER 7, 2016.

EL PASO COUNTY STANDARD CONSTRUCTION NOTES:

- 1. ALL DRAINAGE AND ROADWAY CONSTRUCTION SHALL MEET THE STANDARDS AND SPECIFICATIONS OF THE CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL, VOLUMES 1 AND 2, AND THE EL PASO COUNTY ENGINEERING CRITERIA MANUAL.
- 2. CONTRACTOR SHALL BE RESPONSIBLE FOR THE NOTIFICATION AND FIELD NOTIFICATION OF ALL EXISTING UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, BEFORE BEGINNING CONSTRUCTION. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CALL 811 TO CONTACT THE UTILITY NOTIFICATION CENTER OF COLORADO (UNCC).
- CONTRACTOR SHALL KEEP A COPY OF THESE APPROVED PLANS, THE GRADING AND EROSION CONTROL PLAN, THE STORMWATER MANAGEMENT PLAN (SWMP), THE SOILS AND GEOTECHNICAL REPORT, AND THE APPROPRIATE DESIGN AND CONSTRUCTION STANDARDS AND SPECIFICATIONS AT THE JOB SITE AT ALL TIMES, INCLUDING THE FOLLOWING: a. EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM)
- b. CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL, VOLUMES 1 AND 2
- c. COLORADO DEPARTMENT OF TRANSPORTATION (CDOT) STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION
- d. CDOT M & S STANDARDS
- NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION. ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE AND EROSION CONTROL SHALL CONFORM TO THE 4. STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL, AND THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS FROM REGULATIONS AND STANDARDS MUST BE REQUESTED, AND APPROVED, IN WRITING. ANY MODIFICATIONS NECESSARY TO MEET CRITERIA AFTER-THE-FACT WILL BE ENTIRELY THE DEVELOPER'S RESPONSIBILITY TO RECTIFY.
- 5. IT IS THE DESIGN ENGINEER'S RESPONSIBILITY TO ACCURATELY SHOW EXISTING CONDITIONS, BOTH ONSITE AND OFFSITE, ON THE CONSTRUCTION PLANS. ANY MODIFICATIONS NECESSARY DUE TO CONFLICTS, OMISSIONS, OR CHANGED CONDITIONS WILL BE ENTIRELY THE DEVELOPER'S RESPONSIBILITY TO RECTIFY.
- 6. CONTRACTOR SHALL SCHEDULE A PRE-CONSTRUCTION MEETING WITH EL PASO COUNTY PLANNING AND COMMUNITY DEVELOPMENT (PCD) INSPECTIONS, PRIOR TO STARTING CONSTRUCTION.
- 7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO UNDERSTAND THE REQUIREMENTS OF ALL JURISDICTIONAL AGENCIES AND TO OBTAIN ALL REQUIRED PERMITS, INCLUDING BUT NOT LIMITED TO EL PASO COUNTY EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP), REGIONAL BUILDING FLOODPLAIN DEVELOPMENT PERMIT, U.S. ARMY CORPS OF ENGINEERS-ISSUED 401 AND/OR 404 PERMITS, AND COUNTY AND STATE FUGITIVE DUST PERMITS.
- CONTRACTOR SHALL NOT DEVIATE FROM THE PLANS WITHOUT FIRST OBTAINING WRITTEN APPROVAL FROM THE DESIGN ENGINEER AND PCD. CONTRACTOR SHALL NOTIFY THE DESIGN ENGINEER IMMEDIATELY UPON DISCOVERY OF ANY ERRORS OR INCONSISTENCIES.
- 9. ALL STORM DRAIN PIPE SHALL BE CLASS III RCP UNLESS OTHERWISE NOTED AND APPROVED BY PCD.
- 10. CONTRACTOR SHALL COORDINATE GEOTECHNICAL TESTING PER ECM STANDARDS. PAVEMENT DESIGN SHALL BE APPROVED BY EL PASO COUNTY PCD PRIOR TO PLACEMENT OF CURB AND GUTTER AND PAVEMENT.
- 11. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE AT APPROVED CONSTRUCTION ACCESS POINTS.
- 12. SIGHT VISIBILITY TRIANGLES AS IDENTIFIED IN THE PLANS SHALL BE PROVIDED AT ALL INTERSECTIONS. OBSTRUCTIONS GREATER THAN 18 INCHES ABOVE FLOWLINE ARE NOT ALLOWED WITHIN SIGHT TRIANGLES. 13. SIGNING AND STRIPING SHALL COMPLY WITH EL PASO COUNTY PUBLIC WORK DEPARTMENT AND MUTCD CRITERIA.
- 14. CONTRACTOR SHALL OBTAIN ANY PERMITS REQUIRED BY EL PASO COUNTY PWD, INCLUDING WORK WITHIN THE RIGHT-OF-WAY AND SPECIAL TRANSPORT PERMITS.
- 15. THE LIMITS OF CONSTRUCTION SHALL REMAIN WITHIN THE PROPERTY LINE UNLESS OTHERWISE NOTED. THE OWNER/DEVELOPER SHALL OBTAIN WRITTEN PERMISSION AND EASEMENTS, WHERE REQUIRED, FROM ADJOINING PROPERTY OWNER(S) PRIOR TO ANY OFF-SITE DISTURBANCE, GRADING, OR CONSTRUCTION.

3. THE DEVELOPER OR HIS ENGINEER HAS LOCATED ALL FIRE HYDRANTS AND FUTURE SERVICE STUBS. ANY REQUIRED REALIGNMENT, EITHER HORIZONTAL OR VERTICAL, SHALL BE AT THE EXPENSE OF THE DEVELOPER. 4. ALL DUCTILE IRON PIPE, TO INCLUDE FITTINGS, VALVES AND FIRE HYDRANTS WILL BE WRAPPED WITH POLYETHEYLENE TUBING, BONDED AT EACH JOINT AND ELECTRICALLY ISOLATED. 5. ALL DUCTILE IRON PIPE SHALL BE DOUBLE BONDED. DIP SHALL HAVE CATHODIC PROTECTION USING NO. 6 WIRE WITH 17 LB. MAGNESIUM ANODES EVERY 400 FEET. 6. PVC MAIN LINES SHALL BE INSTALLED WITH COATED NO. 12 TRACER WIRE. 7. ALL FITTINGS SHALL BE DUCTILE IRON - MECHANICAL JOINT AND HAVE 1 LB. MAGNESIUM ANODES AT EVERY FITTING. 8. THE CONTRACTOR IS REQUIRED TO NOTIFY THE WIDEFIELD WATER AND SANITATION DISTRICT (390-7111) A MINIMUM OF 48 HOURS AND A MAXIMUM OF 96 HOURS PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL ALSO NOTIFY AFFECTED UTILITY COMPANIES 48 HOURS PRIOR TO CONSTRUCTION ADJACENT TO THE KNOWN UTILITY LINES. 9. THE LOCATION OF ALL UTILITIES AS SHOWN ON THESE DRAWINGS ARE APPROXIMATE ONLY. THE LOCATION OF ALL UTILITIES SHALL BE VERIFIED PRIOR TO CONSTRUCTION BY THE CONTRACTOR. 10. THE CONTRACTOR SHALL FIELD EXCAVATE AND VERIFY THE VERTICAL AND HORIZONTAL LOCATION OF ALL TIE-INS. CONTRACTOR SHALL NOTIFY THE WIDEFIELD WATER AND SANITATION DISTRICT AND THE ENGINEER OF THE FIELD VERIFIED INFORMATION PRIOR TO CONSTRUCTION. 11. ALL BENDS SHALL BE FIELD STAKED PRIOR TO CONSTRUCTION. 12. ANY WATER UTILITY MATERIAL REMOVED AND NOT REUSED SHALL BE RETURNED TO THE WIDEFIELD WATER AND SANITATION DISTRICT IF THE DISTRICT SO REQUESTS.

13. THE CONTRACTOR SHALL AT HIS EXPENSE SUPPORT AND PROTECT ALL UTILITY MAINS SO THAT THEY WILL FUNCTION CONTINUOUSLY DURING CONSTRUCTION. SHOULD A UTILITY MAIN FAIL AS A RESULT OF THE CONTRACTOR'S OPERATION, IT WILL BE REPLACED IMMEDIATELY BY EITHER THE CONTRACTOR OR THE WIDEFIELD WATER AND SANITATION DISTRICT AT FULL COST OF LABOR AND MATERIALS TO THE CONTRACTOR.

14. ANY PUMPING OR BYPASS OPERATIONS MUST BE REVIEWED AND APPROVED PRIOR TO EXECUTION BY BOTH THE WIDEFIELD WATER AND SANITATION DISTRICT AND THE ENGINEER.

16. CONTRACTOR MUST REPLACE OR REPAIR ANY DAMAGE TO ALL SURFACE IMPROVEMENTS, INCLUDING BUT NOT LIMITED TO FENCES, CURB AND GUTTER AND/OR ASPHALT THAT MAY BE CAUSED DURING CONSTRUCTION.

17. ALL WATER LINES 6" AND LARGER, AND ALL SEWER LINES 8" AND LARGER, SHALL HAVE AS "AS-BUILT" PLANS PREPARED AND APPROVED PRIOR TO FINAL ACCEPTANCE BY THE WIDEFIELD WATER AND SANITATION DISTRICT.

18. PRIOR TO CONSTRUCTION, A PRE-CONSTRUCTION CONFERENCE IS REQUIRED A MINIMUM OF 72 HOURS IN ADVANCE OF COMMENCEMENT OF WORK. TO SET THE PRE-CONSTRUCTION CONFERENCE, CONTACT BRANDON BERNARD-WATER SUPERINTENDENT (464-2051) AND/OR MARK MCCORMICK, WASTEWATER SUPERINTENDENT OF THE WIDEFIELD WATER AND SANITATION DISTRICT FOR A TIME. NO PRE-CONSTRUCTION CONFERENCE TIMES WILL BE SET UNTIL 4 SETS OF SIGNED DRAWINGS ARE RECEIVED BY THE WIDEFIELD W & S DISTRICT. PRE-CONSTRUCTION DATE /INITIALS____

WIDEFIELD WATER AND SANITATION DISTRICT UTILITY CONSTRUCTION NOTES ALL DUCTILE IRON PIPE AND FITTINGS SHALL HAVE CATHODIC PROTECTION AND 1 LB MAGNESIUM ANODES AT EVERY FITTING.

2. ALL FIRE HYDRANTS SHALL BE MEULLER SUPER CENTURION 200 OR AMERICAN AVK SERIES 2700, (MODERN)

WIDEFIELD WATER AND SANITATION DISTRICT GENERAL NOTES

1. ALL UTILITY CONSTRUCTION TO BE CONDUCTED IN CONFORMANCE WITH THE CURRENT WIDEFIELD WATER AND SANITATION DISTRICT SPECIFICATIONS. COMPACTION REQUIREMENTS SHALL BE 95% STANDARD PROCTOR AS DETERMINED BY ASTM D698, UNLESS OTHERWISE APPROVED BY THE WIDEFIELD WATER AND SANITATION DISTRICT OR A HIGHER STANDARD IS IMPOSED BY ANOTHER AGENCY HAVING RIGHT-OF-WAY JURISDICTION.

2. ALL MATERIALS AND WORKMANSHIP SHALL BE SUBJECT TO INSPECTION BY THE WIDEFIELD WATER AND SANITATION DISTRICT. THE WIDEFIELD WATER AND SANITATION DISTRICT RESERVES THE RIGHT TO ACCEPT OR REJECT ANY SUCH MATERIALS AND WORKMANSHIP THAT DOES NOT CONFORM TO ITS STANDARDS AND SPECIFICATIONS.

15. DISINFECTION SHALL BE ACCOMPLISHED BY GLUING TABLETS TO THE TOP OF THE LINE. POWDER OR GRANULER HTH SHALL NOT BE USED. SEE WIDEFIELD SPECS FOR FURTHER DEFINITION OF DISINFECTION TECHNIQUES.

Full-depth asphalt is not allowed

WORK WITHIN CSU SOUTHERN DELIVERY SYSTEM EASEMENT CONSTRUCTION NOTES

1. CONTRACTOR SHALL COMPLY WITH CSU LESS 2.6.H.8 "CROSSING RAW WATER TRANSMISSION MAINS" FOR ALL WORK WITHIN THE CSU WATERMAIN EASEMENT

2. UTILITIES CROSSING OVER THE SDS WATERMAIN MUST BE POTHOLED WITH HYDRO-VAC AT EVERY CROSSING TO OBTAIN VISUAL VERIFICATIN OF THE WATERMAIN ELEVATION.

3. A COLORADO SPRINGS UTILITIES WATER INSPECTOR SHALL BE NOTIFIED, 719-668-4658, AND PRESENT BEFORE AND DURING CONSTRUCTION ACTIVITIES WITHIN THE SDS EASEMENT

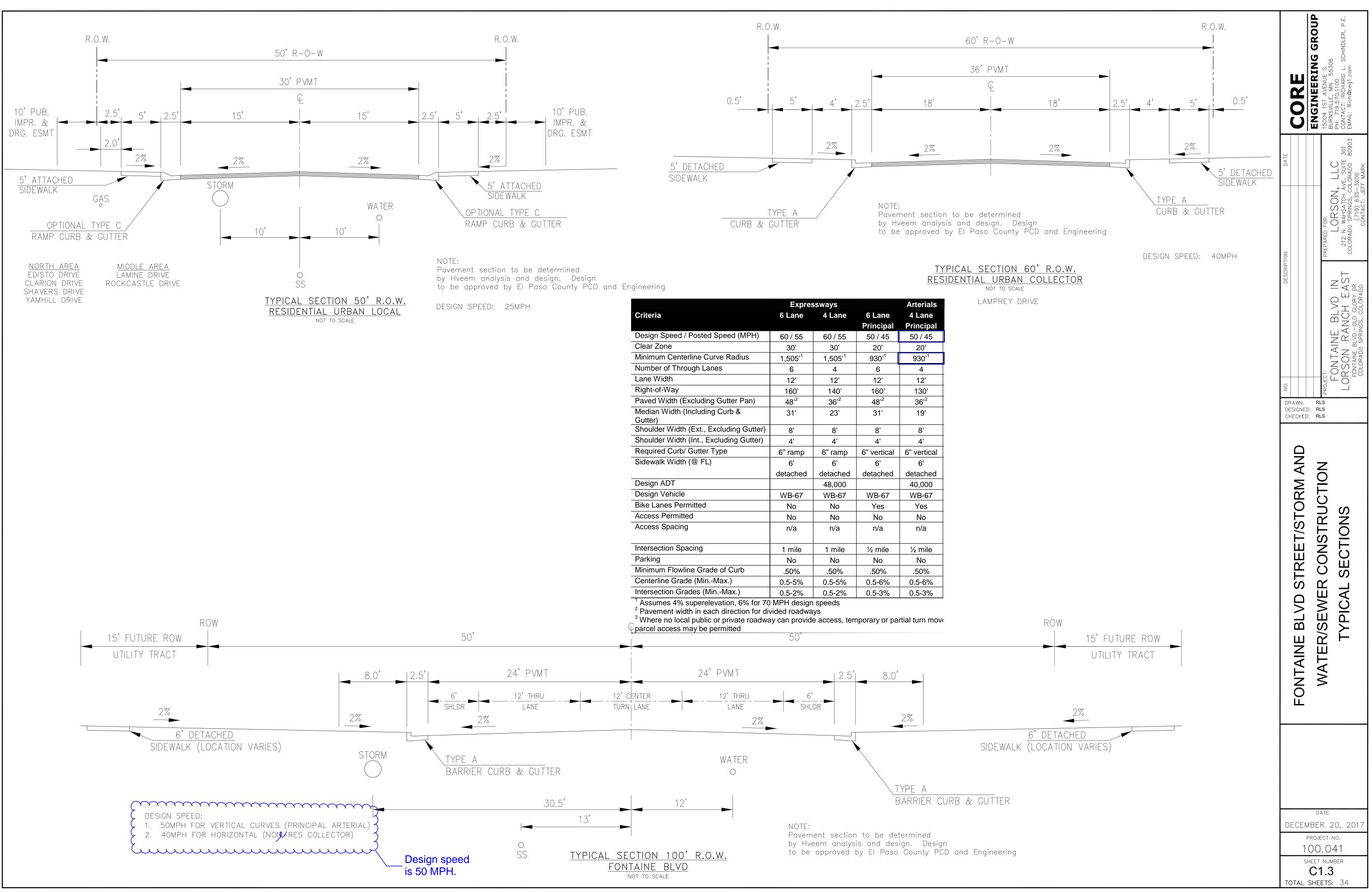
4. CONTACT WAYNE RUST, 719-668-3996, COLORADO SPRINGS UTILITIES, FOR ADDITIONAL INFORMATION REGARDING THE SDS FIBER LINE.

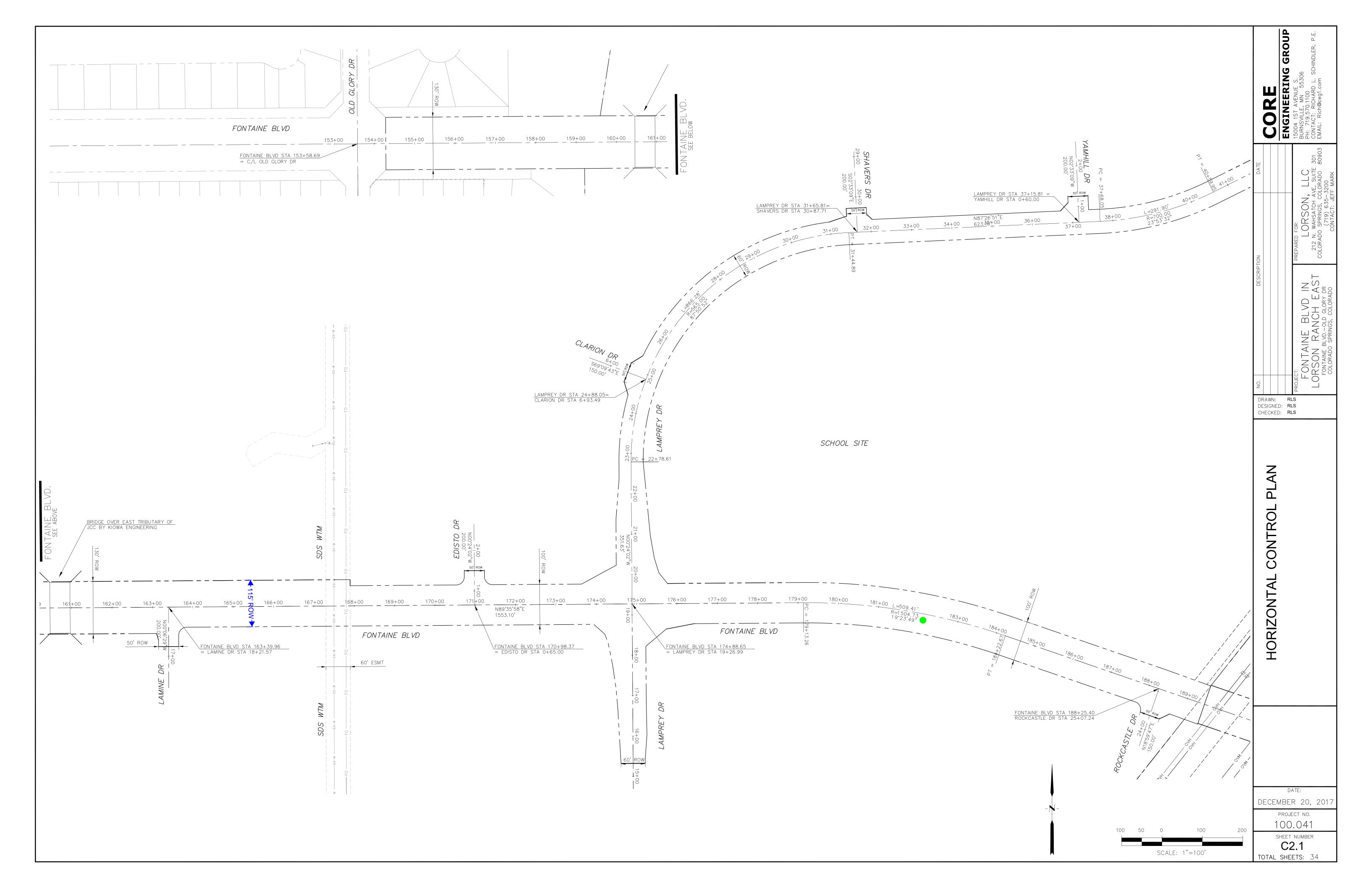
5. CONTRACTOR SHALL MAINTAIN A MINIMUM OF 5' OF COVER OVER THE SDS WATERMAIN.

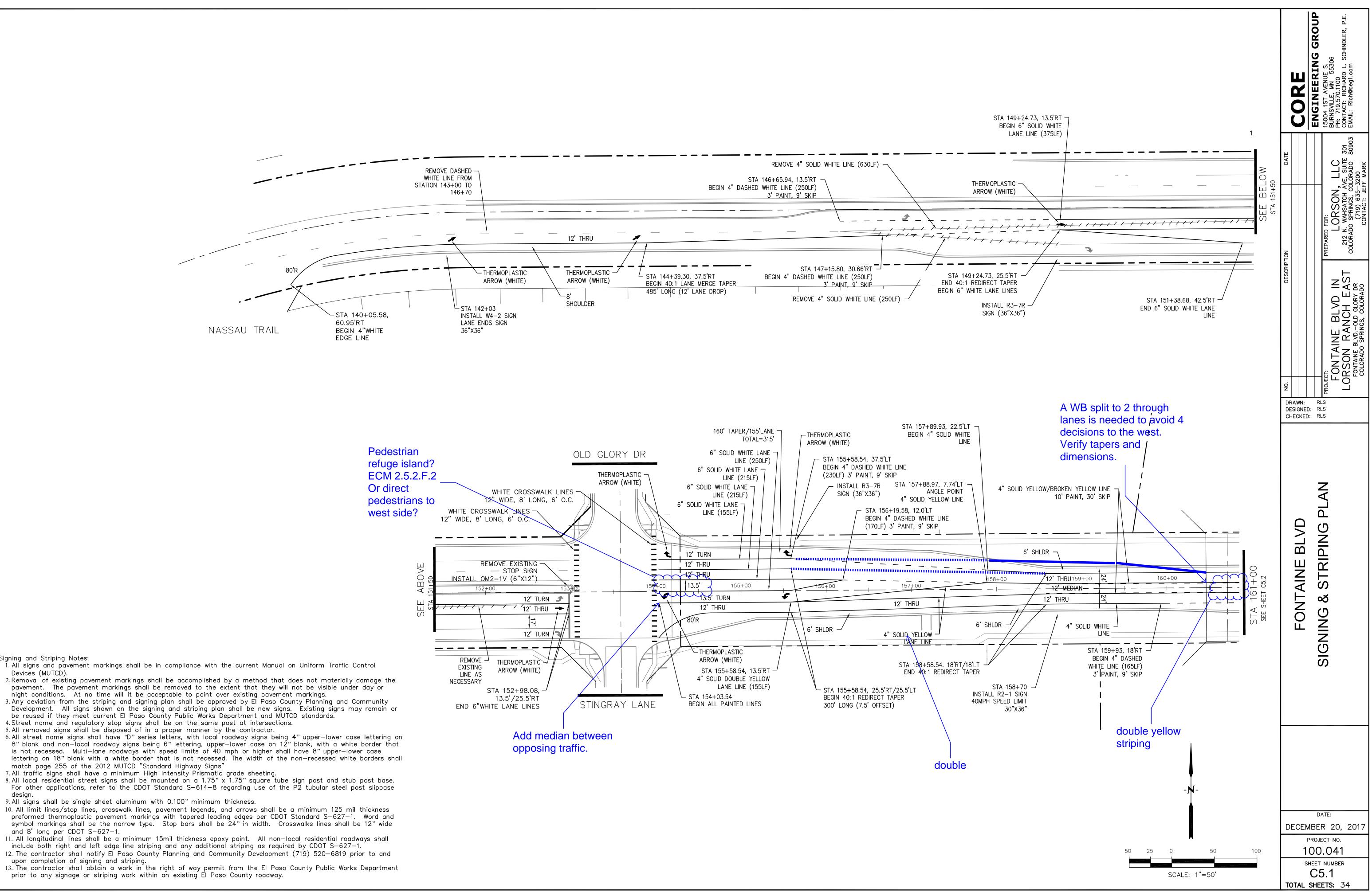
6. CONTRACTOR SHALL SALVAGE AND REPLACE ALL CARSONITE WATER MARKERS OVER THE WATERMAIN AFTER CONSTRUCTION.

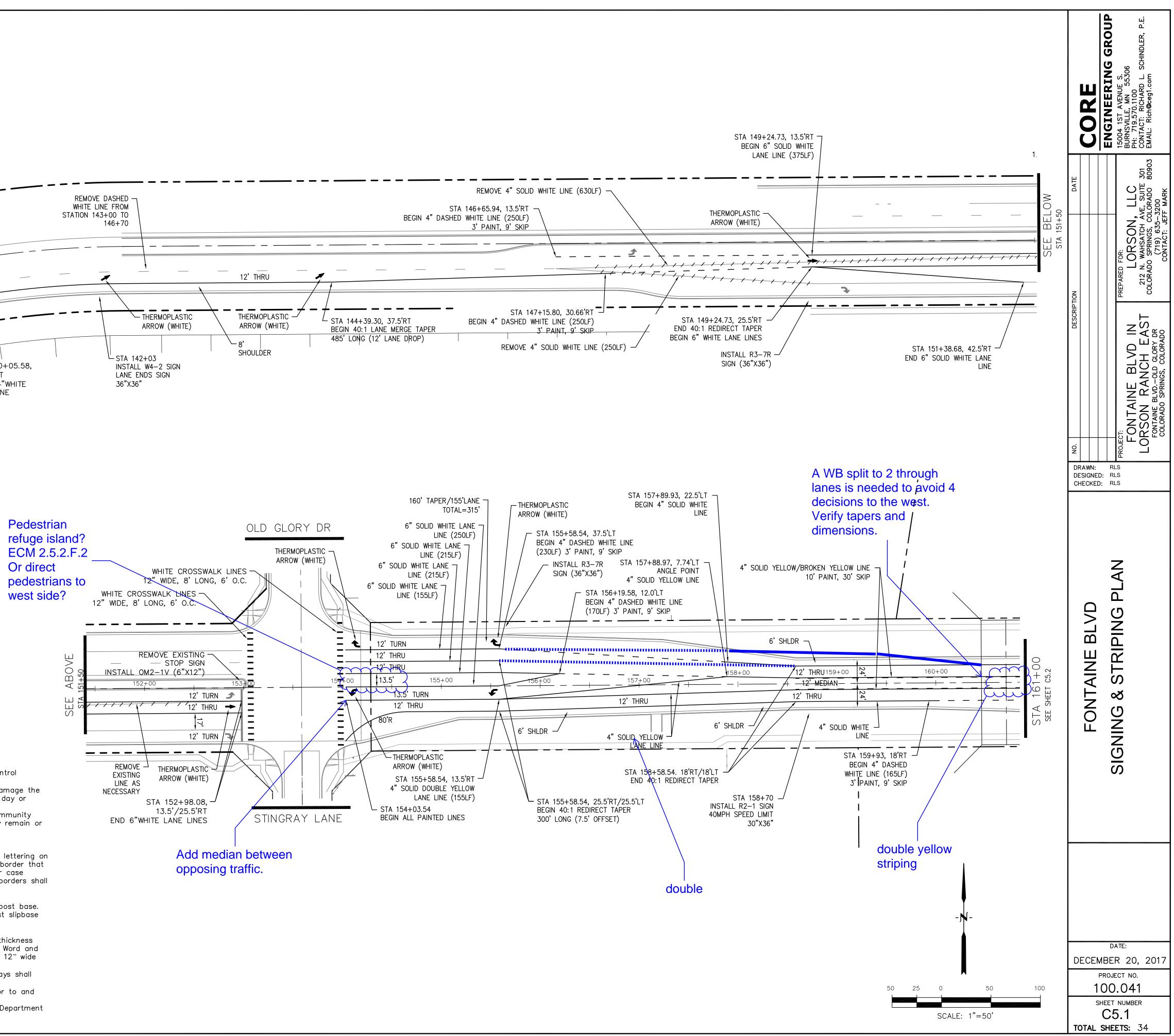
FONTAINE BLVD STREET/STORM AND WATER/SEWER CONSTRUCTION NOTES	DESCRIPTIO	SON, LLC HSATCH AVE, SUITE 301 RINGS, COLORADO 80903 19) 635-3200 TACT: JEFF MARK	CORRE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: RICH@Ceg1.com
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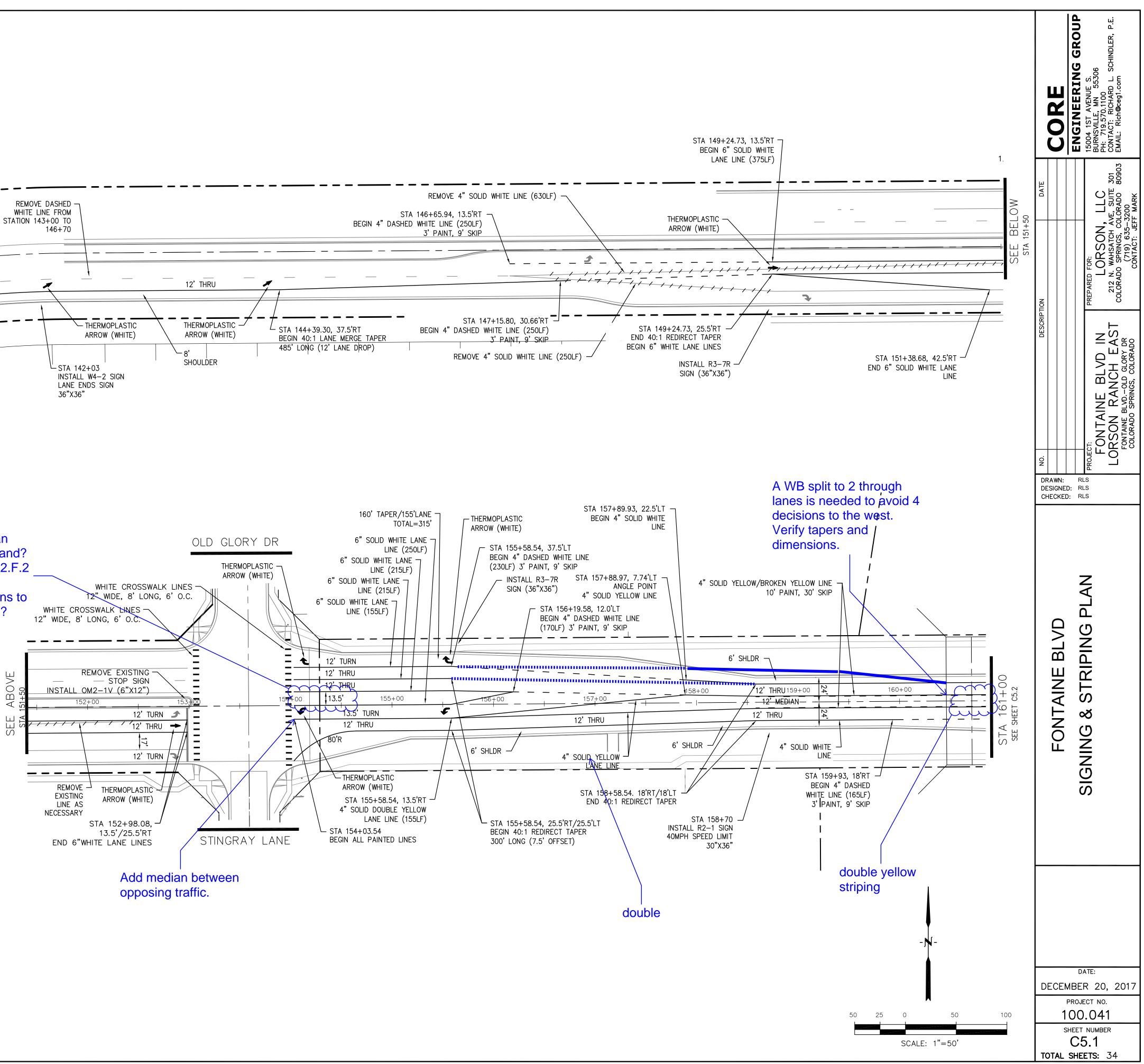
TOTAL SHEETS: 34











Signing and Striping Notes:

1. All signs and pavement markings shall be in compliance with the current Manual on Uniform Traffic Control Devices (MUTCD)

2. Removal of existing pavement markings shall be accomplished by a method that does not materially damage the pavement. The pavement markings shall be removed to the extent that they will not be visible under day or night conditions. At no time will it be acceptable to paint over existing pavement markings.

6. All street name signs shall have "D" series letters, with local roadway signs being 4" upper-lower case lettering on 8" blank and non-local roadway signs being 6" lettering, upper-lower case on 12" blank, with a white border that is not recessed. Multi-lane roadways with speed limits of 40 mph or higher shall have 8" upper-lower case lettering on 18" blank with a white border that is not recessed. The width of the non-recessed white borders shall match page 255 of the 2012 MUTCD "Standard Highway Signs"

7. All traffic signs shall have a minimum High Intensity Prismatic grade sheeting.

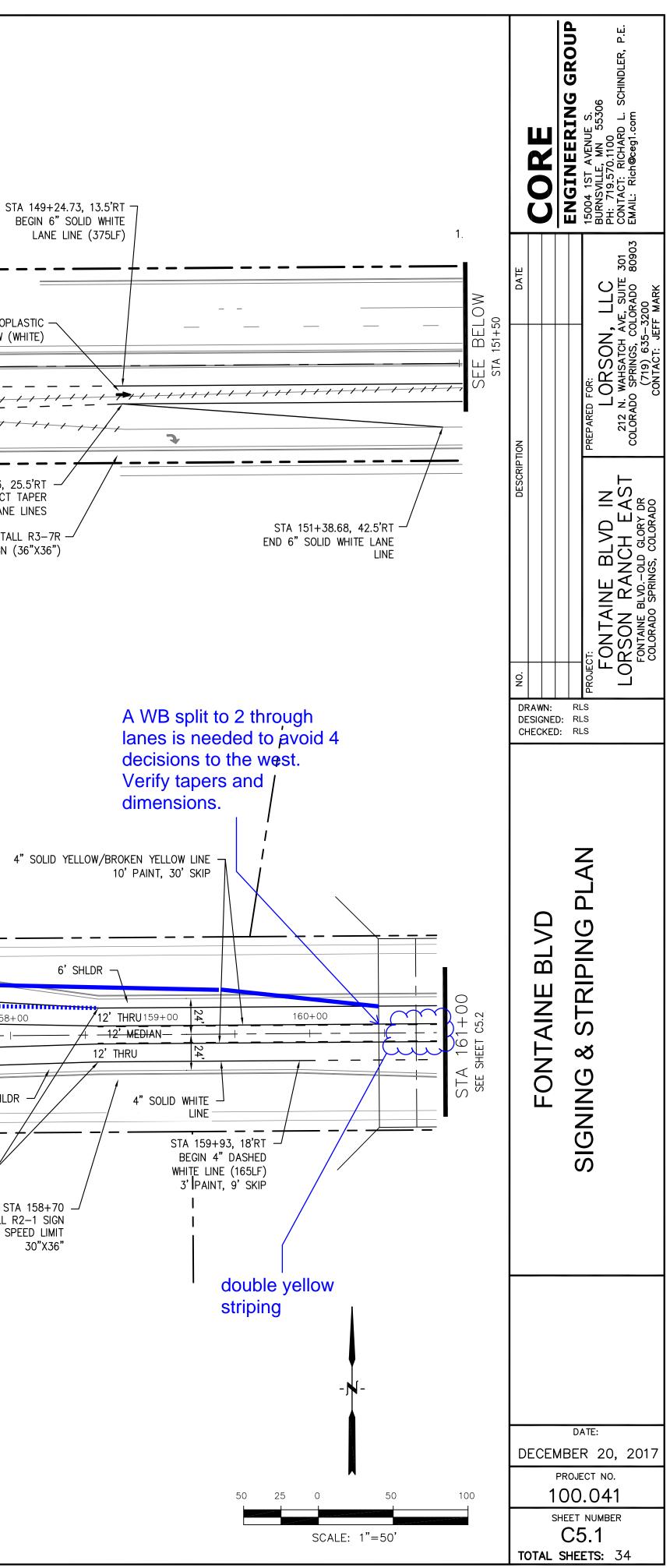
8. All local residential street signs shall be mounted on a 1.75" x 1.75" square tube sign post and stub post base. For other applications, refer to the CDOT Standard S-614-8 regarding use of the P2 tubular steel post slipbase design.

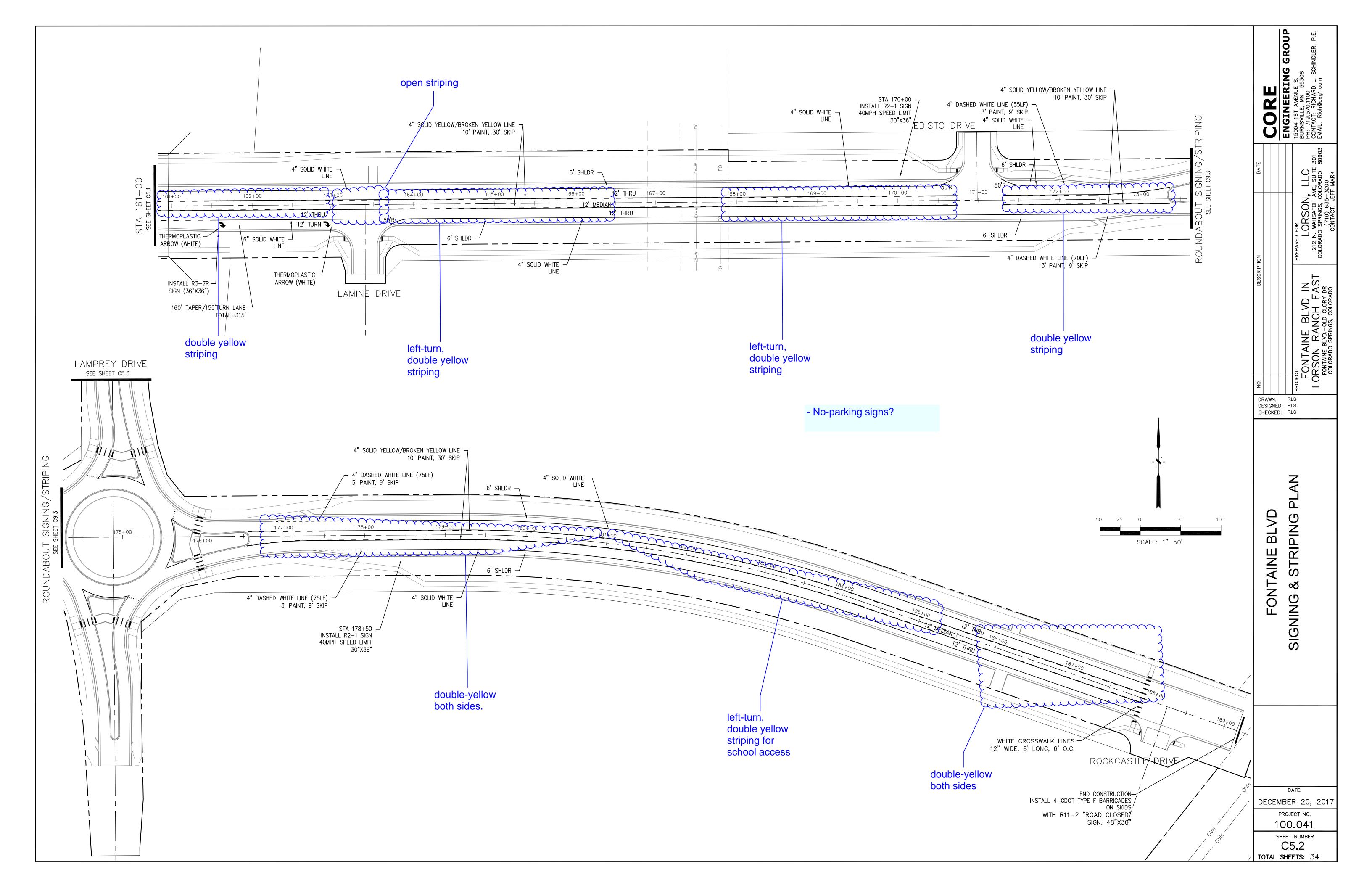
9. All signs shall be single sheet aluminum with 0.100" minimum thickness.

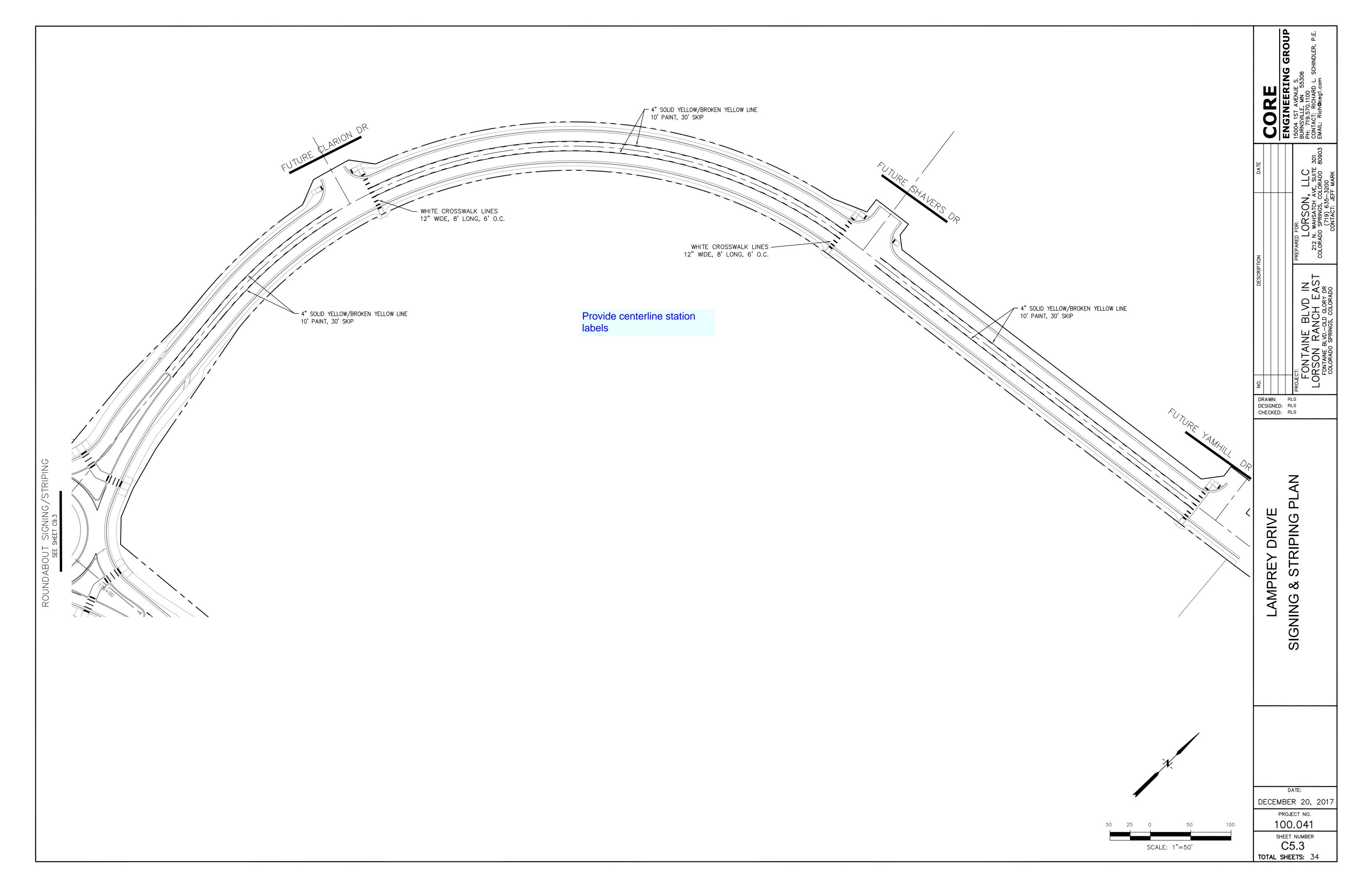
10. All limit lines/stop lines, crosswalk lines, pavement legends, and arrows shall be a minimum 125 mil thickness preformed thermoplastic pavement markings with tapered leading edges per CDOT Standard S-627-1. Word and symbol markings shall be the narrow type. Stop bars shall be 24" in width. Crosswalks lines shall be 12" wide and 8' long per CDOT S-627-1.

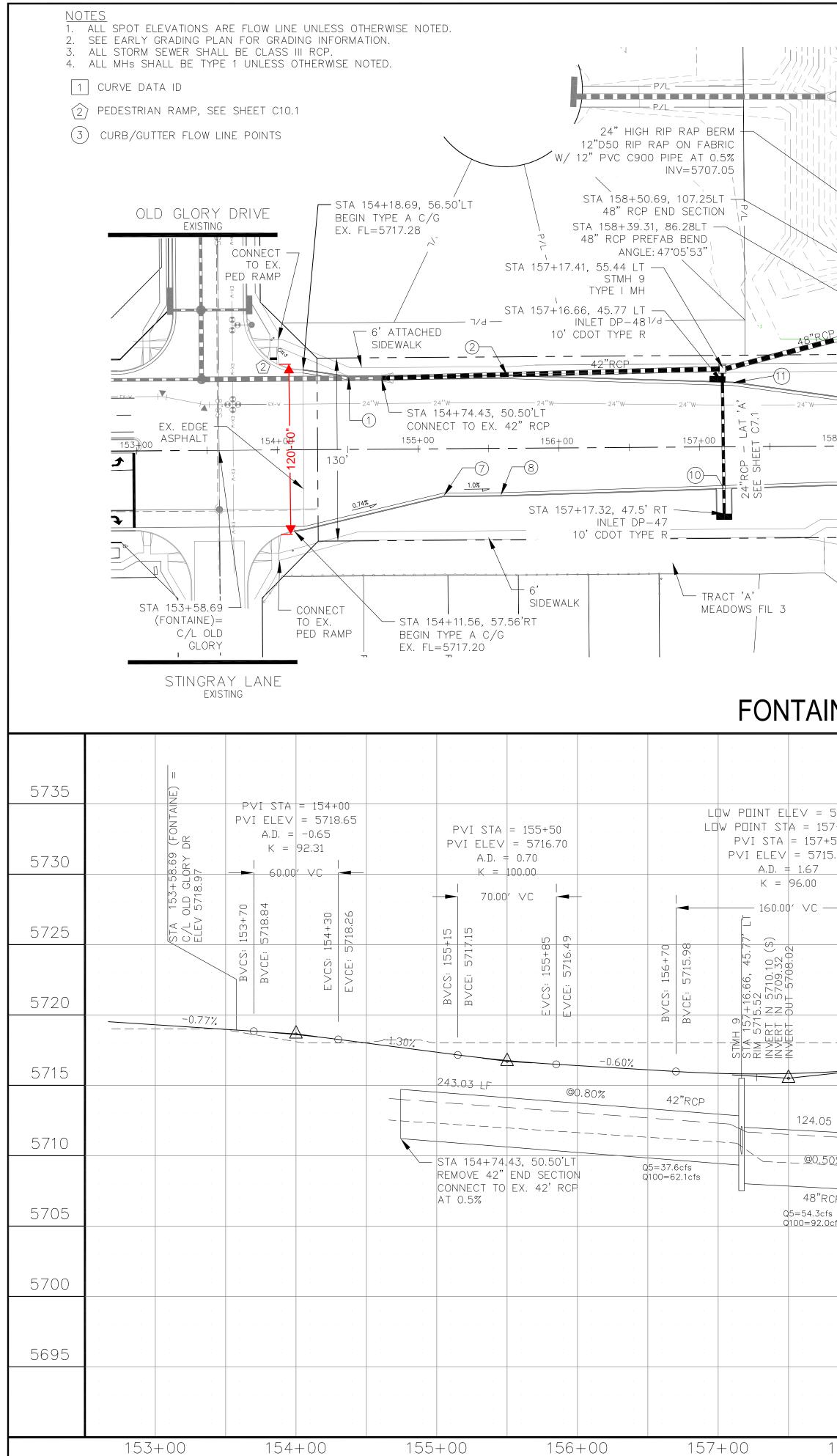
11. All longitudinal lines shall be a minimum 15mil thickness epoxy paint. All non-local residential roadways shall include both right and left edge line striping and any additional striping as required by CDOT S-627-1. 12. The contractor shall notify El Paso County Planning and Community Development (719) 520-6819 prior to and upon completion of signing and striping.

13. The contractor shall obtain a work in the right of way permit from the El Paso County Public Works Department prior to any signage or striping work within an existing El Paso County roadway.

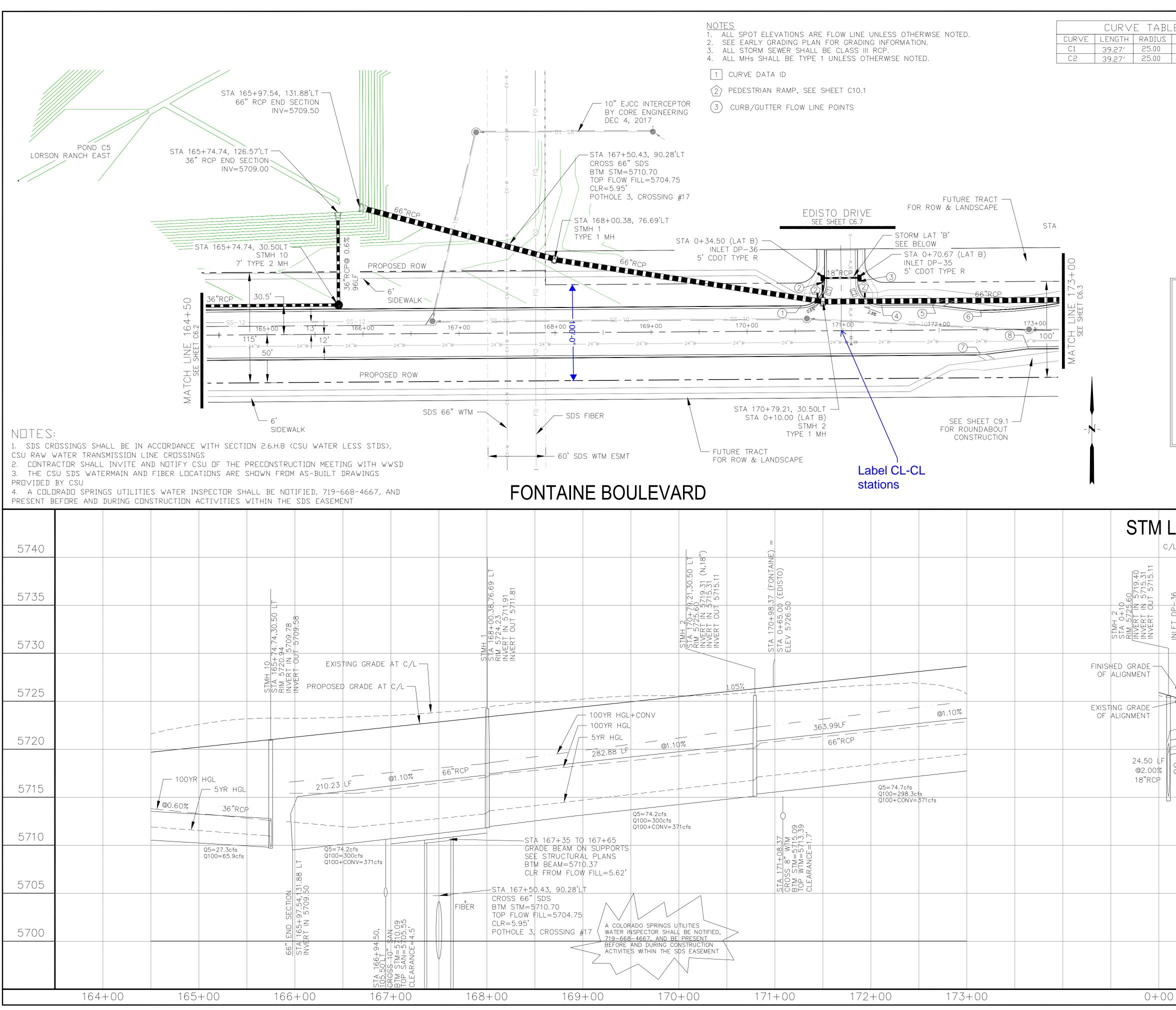






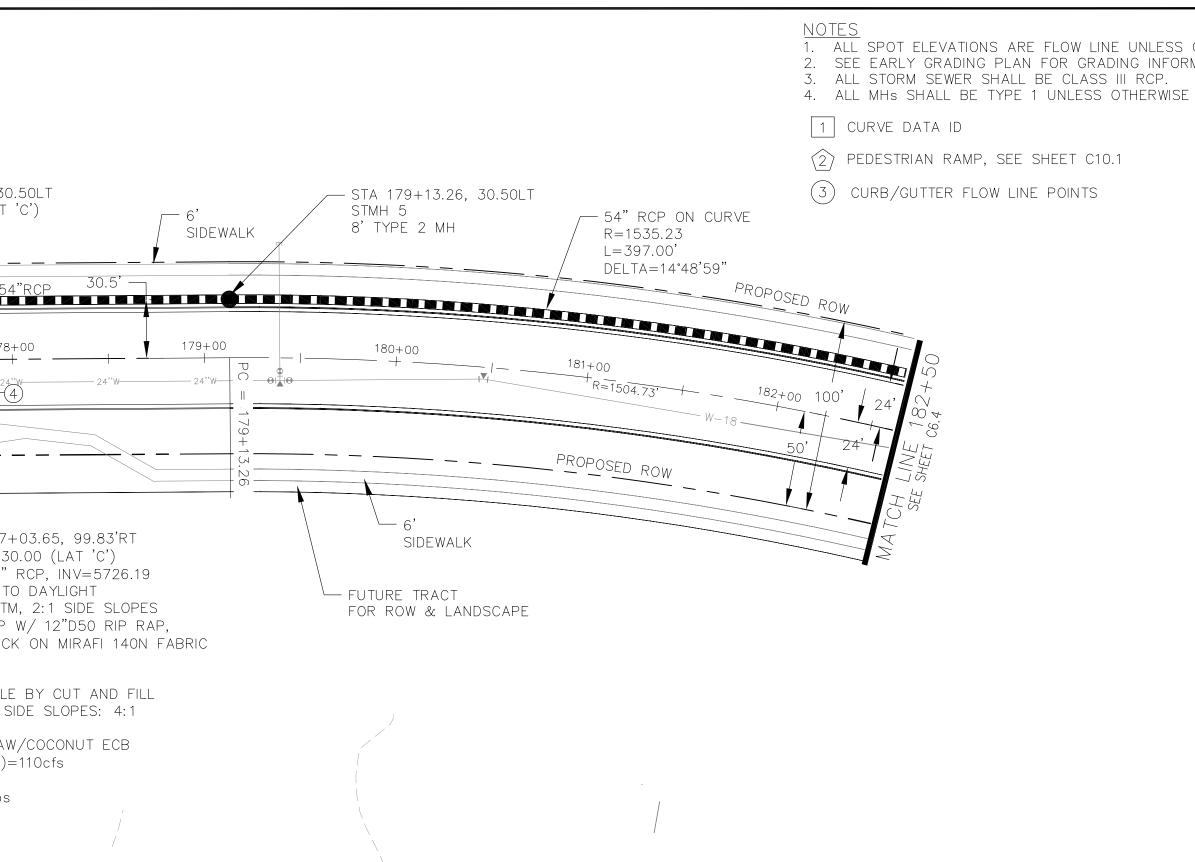


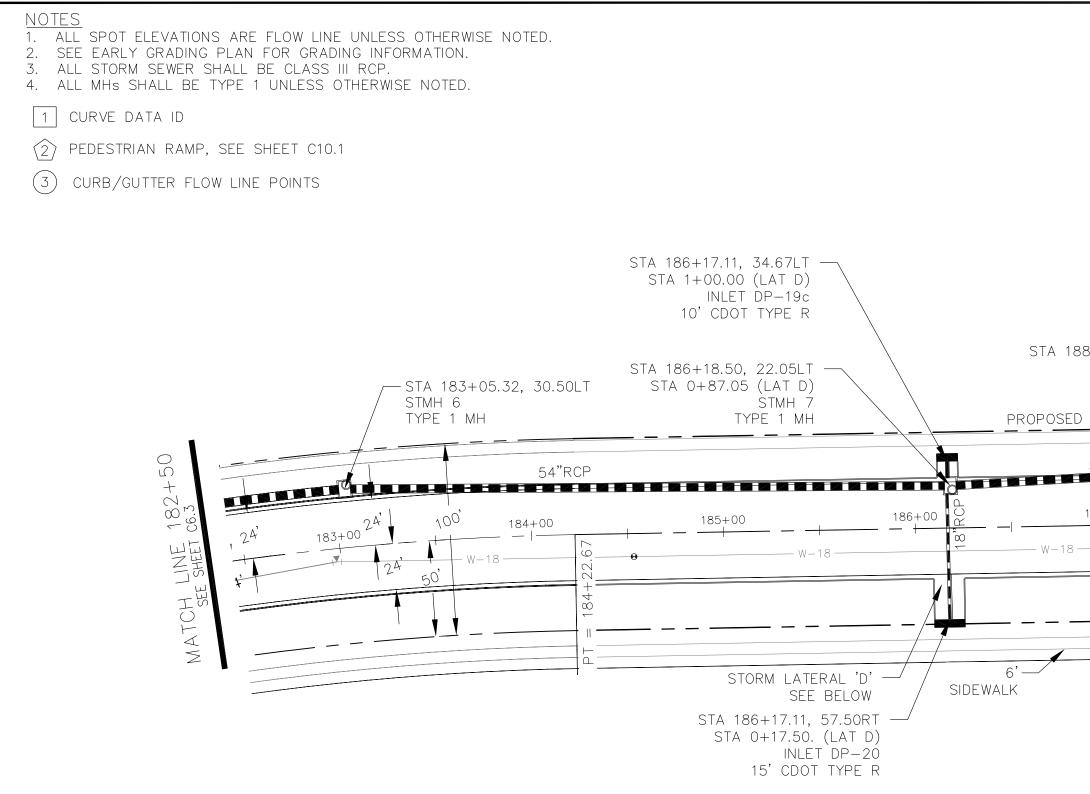
EXISTING POND BR BY PIONEER LANDING PL 2 4' CONC. LOW FLOW CHANNEL MATCH EXISTING TYPE 5706.7 5	CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
ID CUOIT TYPE R SEE SHEET C6.7 STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STM TYPE 2 MH STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STA 163+50.12, 30.21LT STA 160+71.10 STA 160+71.10 STA 160+71.10 C/L FONTAINE BRIDGE SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 C/L FONTAINE BRIDGE SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 C/L FONTAINE BRIDGE SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE KIOWA ENCINEERING PLANS STA 160+71.10 STA 160+71.10 STA 160+71.10 SEE SEE SEE SEE SEE	DESCRIPTION DATE DATE
BROPOSED ROW PROPOSED ROW PROPOSED ROW PROPOSED 24" WIM PROPOSED 25 C 50 100	NO. NO. NO. NO. NO. NO. NO. NO.
AINE BOULEVARD INTERCEPTOR SCALE: 1"=50' SCALES: HORIZ. 1"=50' VERT. 1"=5'	
HIGH PDINT ELEV = 5719.70 LOW PDINT ELEV = \$719.10 5735 HIGH PDINT STA = 161+71.38 LOW PDINT STA = 163+49.35 PVI STA = 163+68.45 5735 = 5715.81 PVI ELEV = 572<0.01	-AN/PROFIL ARD +50
	LEV/ 164-
- - <td>A SEWE NE BOUI 1+00 TO</td>	A SEWE NE BOUI 1+00 TO
Image: Signal with the second seco	TORM TAINI A 1534
1.07%	FON STA
05 LF 100YR HGL 05 LF STA 158+39.31, 86.28' LT 48" RCP END SECTION INV=5707.28 5710 5710	STREET
2.50% 23.86 LF PROPOSED CONSPAN Label CL-CL 100=65.9cfs "RCP 00.50% BRIDGE. SEE KIOWA 5YR HGL 5YR HGL 2.0cfs 48"RCP ENGINEERING PLANS 1000 - 65.9cfs 5YR HGL 0.50% INCLUDING 1000 - 65.9cfs 5YR HGL 5705	ی ا
BRIDGE. SEE KIOWA SEE KIOWA 5705 2.0cfs 48"RCP ENGINEERING PLANS 5705 10 INCLUDING END SECTION INCLUDING END SECTION INCLUDING END SECTION 1000000000000000000000000000000000000	
INCLUDING END SECTION UNEQUIVE END SECTION UNEQUIVE	DATE:
	DECEMBER 20, 2017 project no.
	100.041 Sheet Number



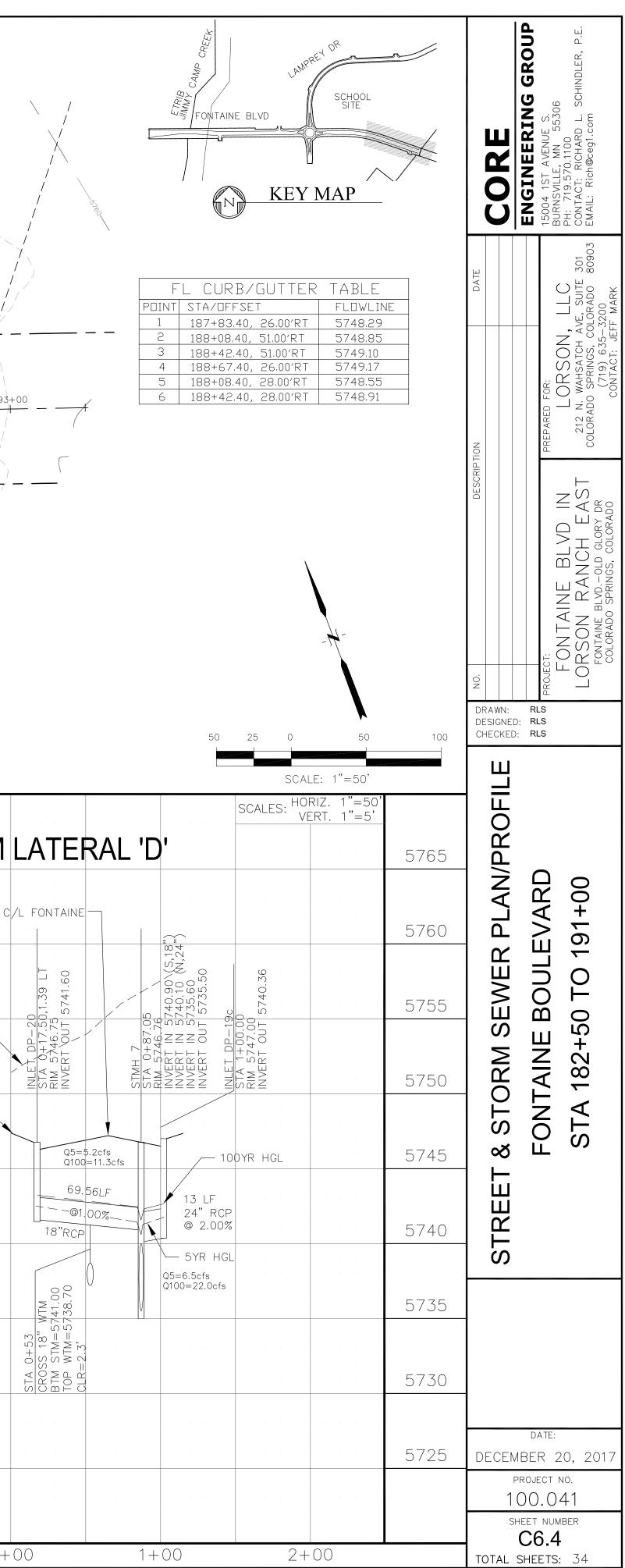
00 90°00′00″ 00 90°00′00″	AMP CREEK	BLVD KEYN	SCHOOL SITE		CORRE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
	PDINT STA/DFI 1 170+56. 2 170+81.3 3 171+15.3 4 171+40.3 5 171+93. 6 172+43 7 172+43	37, 26.00'LT 37, 51.50'LT 37, 51.50'LT 37, 26.00'LT 25, 26.00'LT .25, 20.00'LT	TABLE FLDWLINE 5725.42 5725.20 5725.20 5726.30 5726.85 5727.50 5727.38 5728.02		PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK
APPROVED BY: DATE: PROJECT NUMBER WORK ORDER NUI CSU SHEET APPROVAL EXPIRI RESUBMITTAL OF REQUIRED IF CON	R: 2017– MBER: OF ES ONE (1) YEAF THESE PLANS F	R FROM THE D. OR REVIEW ANI	PROVAL	IS 🛛	NO. DESCRIPTIO
	50	25 0	50	100	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
/ LATERAL	'B'	CALES. HORIZ	Z. 1"=50' Z. 1"=50' T. 1"=5'		OFIL
C/L EDISTO				5740	AN/PROFII RD -00
P 35 25.70 0JT 572 IN 572 0JT 572 70.67	27 21.61		· · · · ·	5735	
INLET I STA 0- RIM 57 INVERT INVERT INVERT INVERT INLET D STA 0+ RIM 572					
				5730	50 .
	100YR HGL			5725	ORM SEV TAINE BC 164+50
	5.17 LF 52.00% 8"RCP 5YR HGL				T & STORM SEWER PI FONTAINE BOULEV STA 164+50 TO 173
LF Q5=3.1cfs Q100=6.7cfs NN NU	/ 5.17 LF 22.00% 8"RCP 5YR HGL =2.8cfs 00=6.1cfs			5725	
LF Q5=3.1cfs Q100=6.7cfs NN NU	/ 5.17 LF 22.00% 8"RCP 5YR HGL =2.8cfs 00=6.1cfs			5725 5720 5715	STREET & STORM SEV FONTAINE BC STA 164+50
LF Q5=3.1cfs Q10 Q100=6.7cfs Q10	/ 5.17 LF 22.00% 8"RCP 5YR HGL =2.8cfs 00=6.1cfs		Image: second	5725 5720	
LF Q5=3.1cfs Q100=6.7cfs NN NU	/ 5.17 LF 52.00% 8"RCP 5YR HGL =2.8cfs 50=6.1cfs			5725 5720 5715	
LF Q5=3.1cfs Q100=6.7cfs NN NU	/ 5.17 LF 52.00% 8"RCP 5YR HGL =2.8cfs 50=6.1cfs			5725 5720 5715 5710	

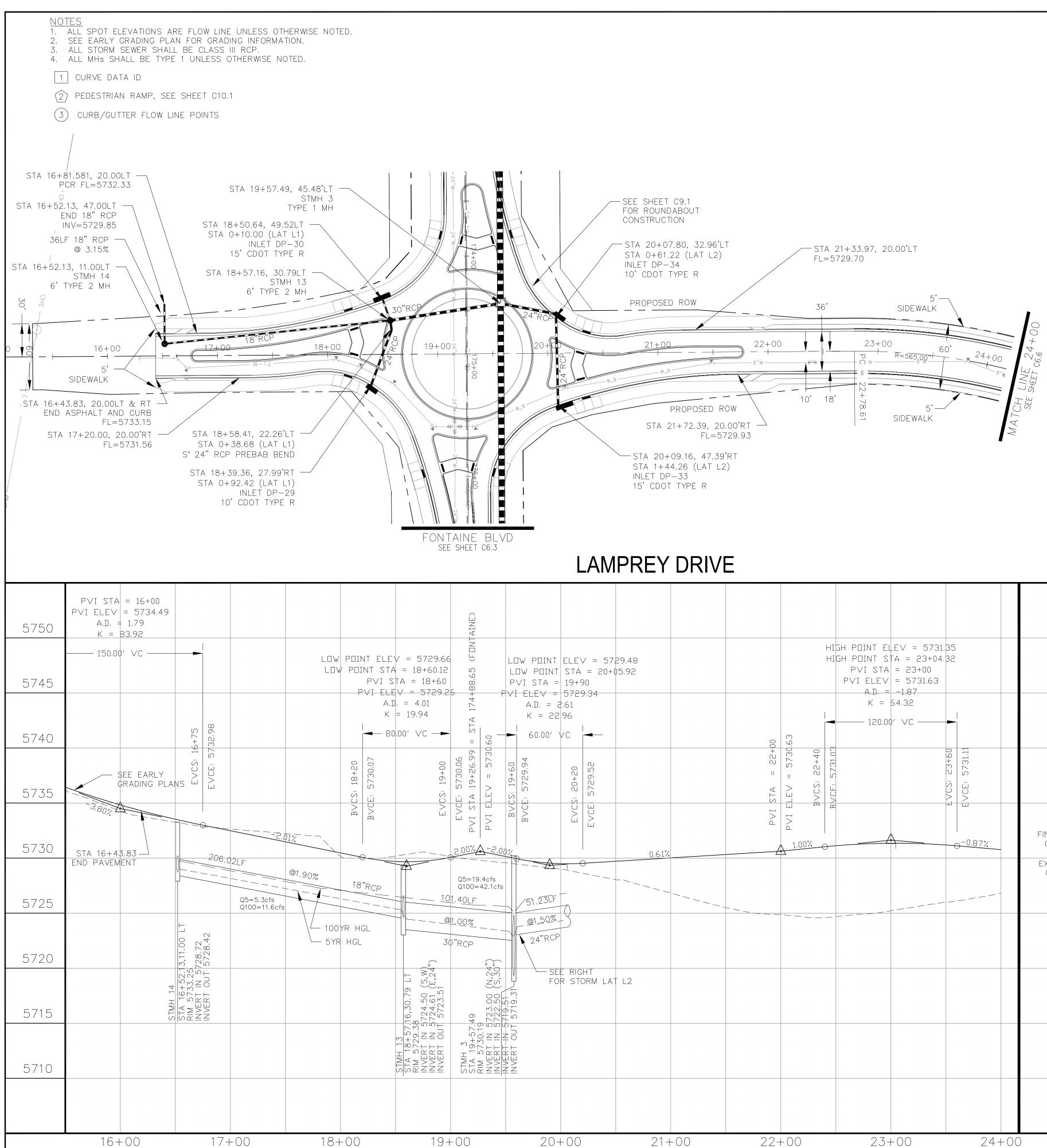
	STORY LAT 1.2 STORY LAT 1.2 STORY LAT 1.2 STA 1724-43.17, 30.50LT STA 1724-43.17, 30.50LT S		CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. CONTACT: RICHARD L. SCHINDLER, P.E.
		NE 2	NO. DESCRIPTION DESCRIPTION DATE DATE DESCRIPTION DATE DESCRIPTION DATE DESCRIPTION DATE DESCRIPTION DATE DESCRIPTION DATE DESCRIPTION DATE DATE DESCRIPTION DATE DATE DATE DATE DATE DATE DATE DATE
		100	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
	FONTAINE BOULEVARD SCALE: 1"=50' PVI STA = 180+76.87 PVI STA = 180+76.87 STM LATERAL 'C' SCALES: HORIZ. 1"=50' VERT. 1"=5')'	
5755	PVI STA = 180+76.87 PVI STA = 180+76.87 PVI ELEV = 5741.51 A.D. = -1.45 K = 96.55	5755	D D
5750	PVI STA = 177+50 PVI ELEV = 5733.34	5750	PLAN EVARD 82+50
5745	A.D. = 1.45 $K = 96.58$ $140.00' VC$ $H = 1.40$ $140.00' VC$	5745	WER OULE TO 1
5740	1000 1000	5740	DRM SE TAINE B 173+00
5735	Sign of the second se	5735	T & ST(FON STA
5730	PROPOSED GRADE AT ROUNDABOUT SEE SHEET C9.1 01.70% 54"RCP 01.40% 01.40\%	5730	TREE
5725	1.05/ 209,000 01.10% 66"RCP 260,48LF 66"RCP 05=23.6cfs 0100+CONV=233cfs	5725	S S
5720	©1.10%	5720	
5715		5715	DATE: DECEMBER 20, 2017 PROJECT NO. 100.041
	CLEP ROLL COUNT COUN		SHEET NUMBER



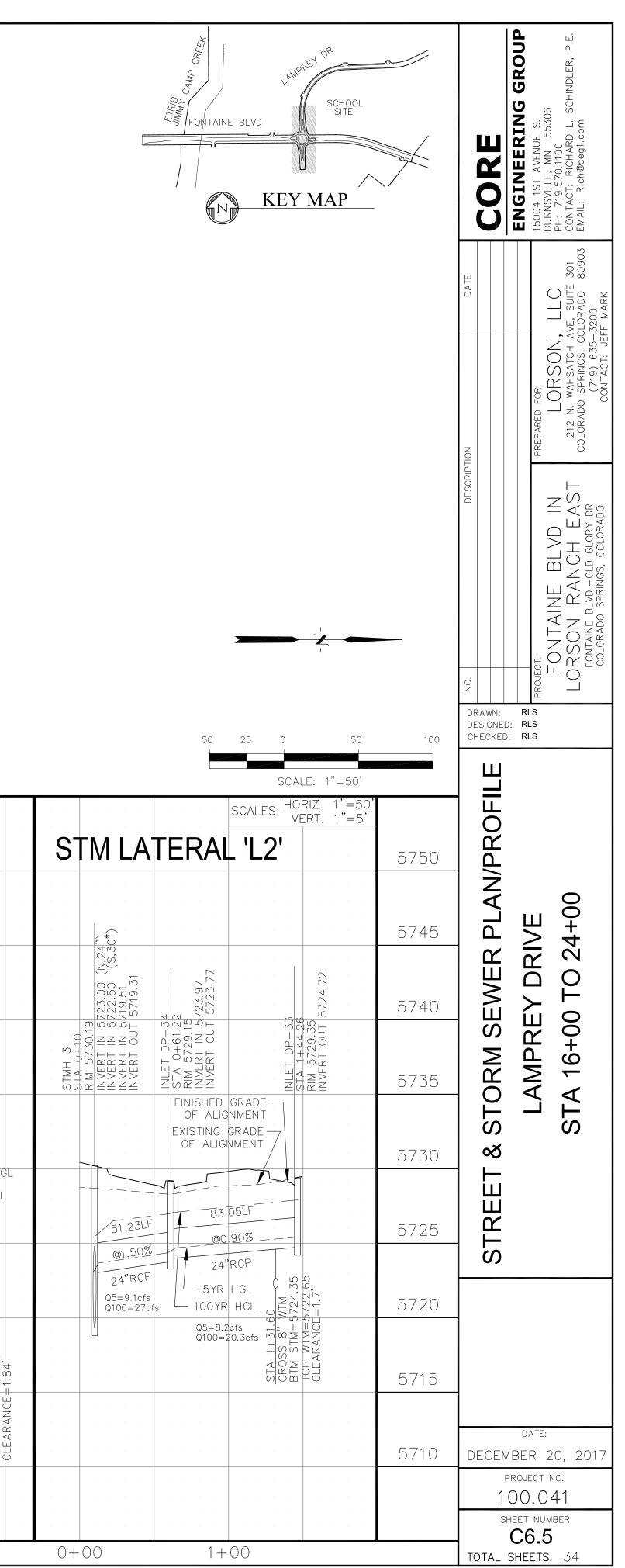


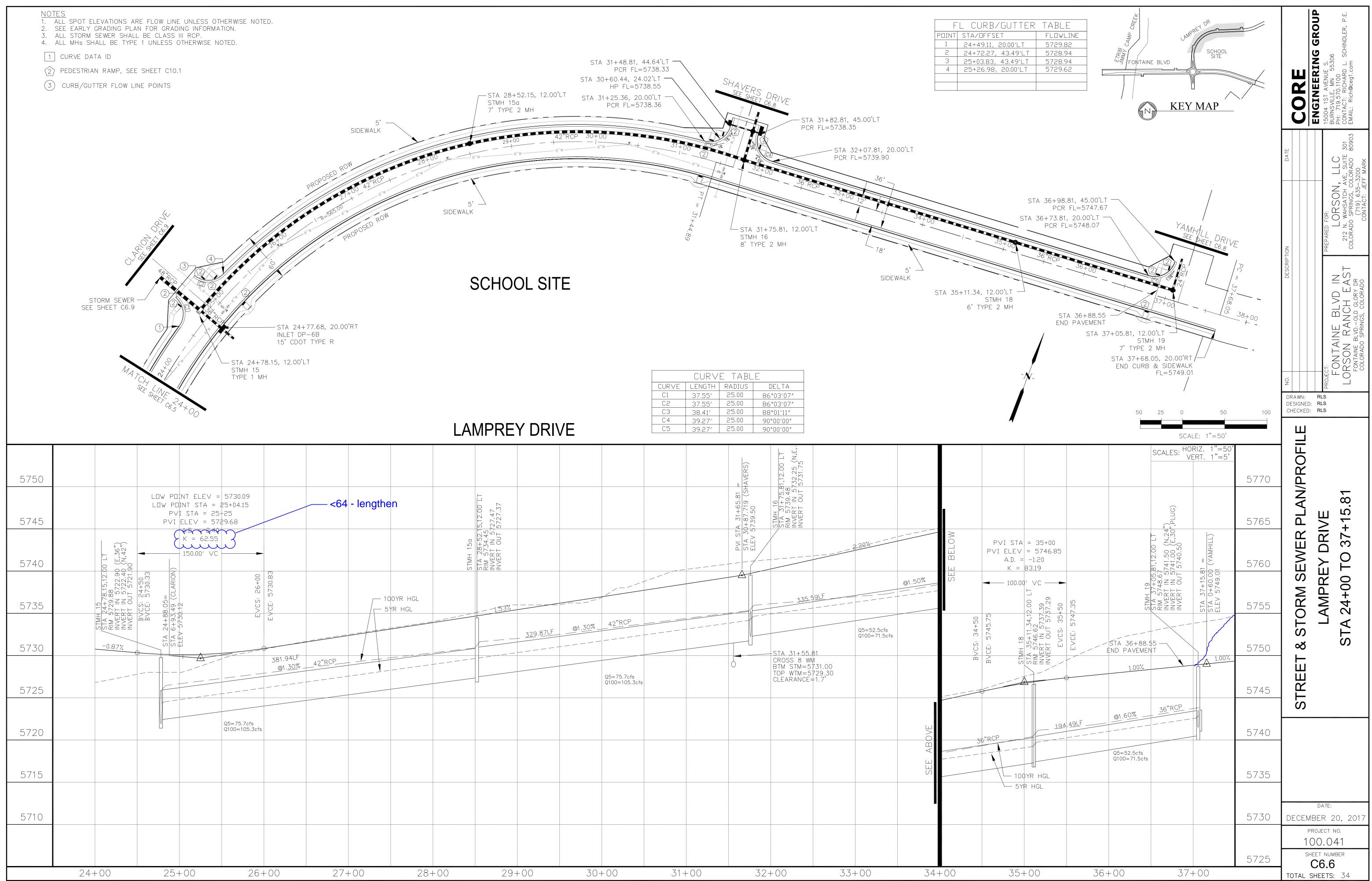
4. 1 (2)	ALL MHS SHALL CURVE DATA II PEDESTRIAN R/ CURB/GUTTER	AMP, SEE SHEET C10.1	THERWISE NOTED.	54"RCP	STA 1+00.(INL 10' CD STA 186+18.5 STA 0+87.	11, 34.67LT 00 (LAT D) ET DP-19c 00T TYPE R 50, 22.05LT .05 (LAT D) STMH 7 TYPE 1 MH -00 -00 -00 -00 -00 W-18 STORM LATERAL SEE BEL 186+17.11, 57.5 TA 0+17.50. (LA INLET DP 15' CDOT TYP	186+00 	PROPOSED	+15.40, 30.50L STMH TYPE 1 M 64"RCP 64"RCP 87+00 - +- PROPOSED RC STOR SEE SHEET C6.	H 2 188+00 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188+00 2 188 FUTURE SEE EARL FOR LORSON RANCE	Y GRADING PLAN H EAST PHASE	LE DRIVE	END 54" RCP,	TO DAYLIGHT SIDE SLOPES 50 RIP RAP, 140N FABRIC	VERSION SWALE	Ho Ho Ho Ho Ho Ho Ho Ho Ho Ho Ho Ho Ho H	
				F	ONTAIN	NE BOU	LEVAF	RD	CURV C1 C2	'E LENGTH RADIU: 39.27' 25.00 39.27' 25.00	90°00′00″					,	
5765 5760 5755 5750 5745			STA 183+05.32,30.50 LT RIM 5743.85 INVERT IN 5732.35 INVERT OUT 5732.25		- 100YR HGL - 100YR HGL - 5YR HGL	+ CONV		STA 186+18.60,22.05 LT STA 186+18.60,22.05 LT RIM 5746.76 INVERT IN INVERT IN 5740.90 (S,18") INVERT IN 5740.10 (N,24") INVERT IN 5735.60 INVERT OUT 5735.50		STMH 8 STMH 8 STA 188+15.40,30.50 LT RIM 5749.21 INVERT IN 5749.57 INVERT IN 5737.77 INVERT IN 5737.57 STA 25+07.24 (ROCKCASTLE) STA 188+25.40 (FONTAINE)	lt a lin lov	END 01.33% 018"WTM 05=10.0cfs	M 52 B C Si 190+22 B C C Si 190+22 B C C Si 190+22 B C C Si 190+22 B C Si 190+22 C S			EXISTING GR EXISTING GR OF ALIGNMI FINISHED GR OF ALIGNMI	IENT
5740 5735 5730 5725				BLF Q5=23.6cfs Q100=163.4cfs Q100+C0NV=233c	©1.00%		"RCP		@1.00%	54"RCP		Q100=87.0c Q100+CONV	fs =200.0cfs 	RCP 1 1 1	. .		
	182+0	0 183+	- 0 0	166+00	167-	+00	168+0		7+00	188+00	189	9+00 19	a a a a a b a b b b b b b b b	191+00			0+





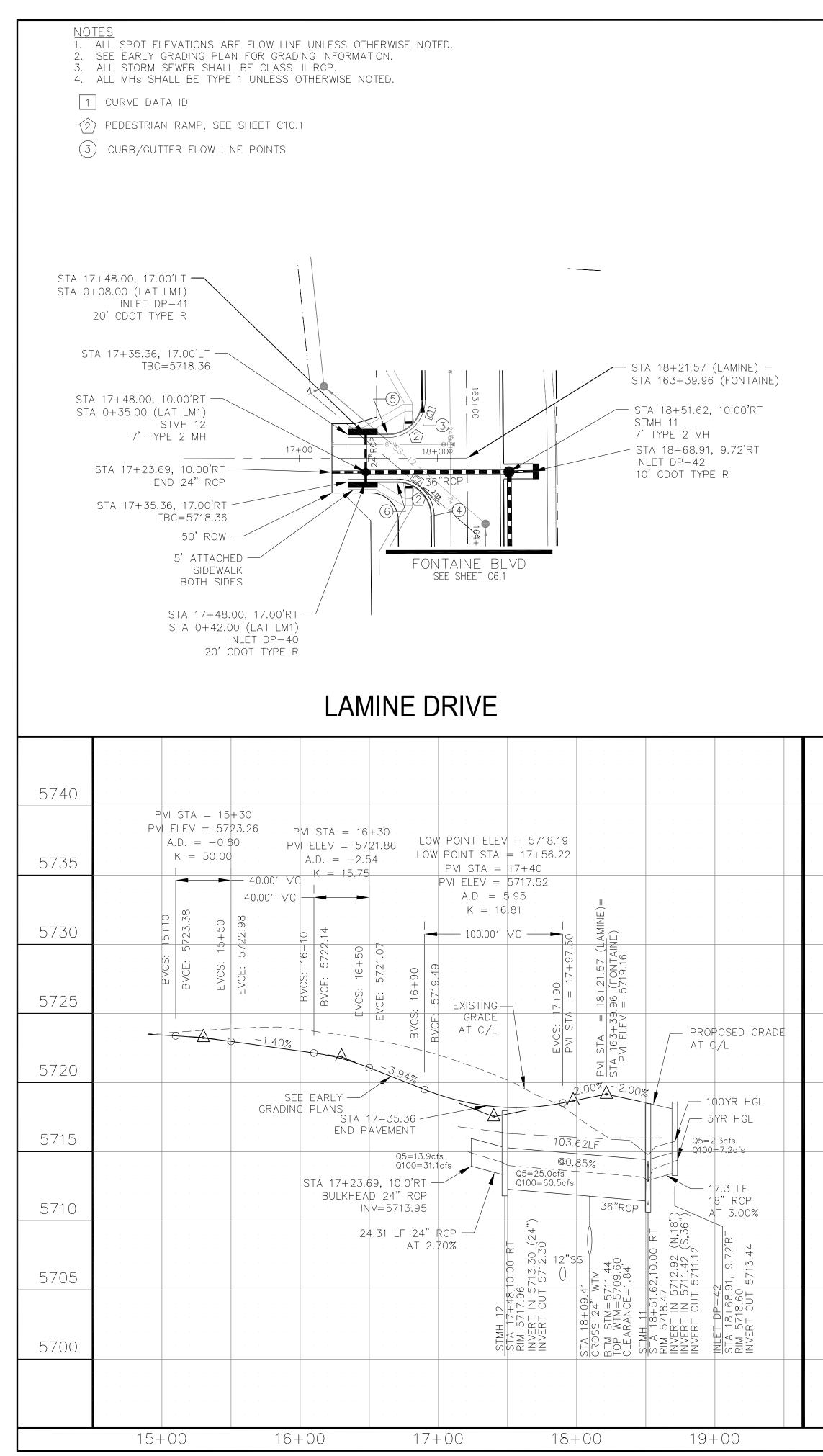
									1		1
· ·						· ·		STM L	ATERA	AL 'L1'	
· ·			HIGH POINT EL HIGH POINT S PVI STA PVI ELEV	FA = 23+04,2 = 23+00		· ·	· · · · ·				
			A.D. =	-1.87						24,)	
· ·										51 51 51	
)′VC —		· ·		24.80	30.06 9.38 10.5724 50	5723.	725.
· ·		= 55 55	/ = 5730,63 S: 22+40		s: 23+60 : 5731.11	· ·		INLET DP-30 STA 0+10 RIM 5729.27 INVERT OUT 57	$ $ $ $ $ $ $ $		5729.68 ERT OUT
			ELE C B C C B C C		E V CE:			NKER 0	STMH 1 STA 04 RIM 57	INLE STA	
0 (1 %					-0.87%		HED GRADE Alignment				
0.61%						EXIS OF	TING GRADE ALIGNMENT			53.75LF	F 100YR HGL /- 5YR HGL
								20.00LF @1.50%		@1.50%	
						· ·		24"RCP Q! Q'	5=7.2cfs 00=20.1cfs	STA 0+38.66	=8.6cfs 00=16.3cfs
· · ·				· · · · ·						24" RCP PREFAB BEN INV=5724.14	D
										.62LF 24" RC 1.50%	86 75725.1 1.84 1.84
 				· · · ·		· · ·					TA 0+81.85 CROSS 12" WTM CROSS 12" WTM BTM STM=5725.14 TOP WTM=5723.30 CLEARANCE=1.84
				· · · ·	· · · · · · · ·						
						· ·					
21+	-00	22-	+00 23-	-00	24+00			0+	-00	1+	00



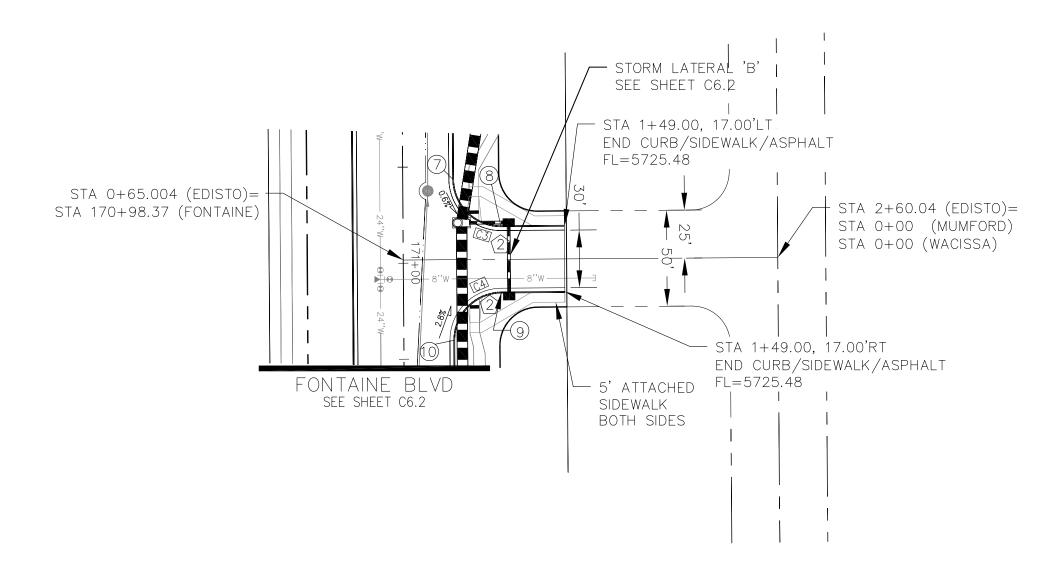


CURVE TABLE									
CURVE	LENGTH	RADIUS	DELTA						
C1	37.55′	25.00	86°03′07″						
C2	37.55′	25.00	86°03′07″						
C3	38.41′	25.00	88°01′11″						
C4	39,27′	25.00	90°00′00″						
C5	39,27′	25.00	90°00′00″						

				PVI STA 31+65.81 = STA 30+87.719 (SHAVERS) ELEV 5739.50	STMH 16 STA 31+75.81,12.00 LT RIM 5739.48 INVERT IN 5732.25 (N,E,C) INVERT OUT 5731.75			
				(SH 100 - 10	75.8 9.48 001			
,37				+65. 50	1 10 31+1 55739 11 11 12 14 14 14 14 14 14 14 14 14 14		· · · · · · · · ·	
5727				739.	NVEI NVEI			
				VI STA VI STA LEV 5		220%		PVI STA =
INVERT OUT 5727.37								$P \lor I E L E \lor = A D = -$
<u> </u>								<u> </u>
							@1.50%	→ 100.00′
·					<u>335.59LF</u>			
1.537.								
	329.87LF	@1.30%42"RCP				Q5=52.5cfs Q100=71.5cfs		BVCS: 34+50 BVCE: 5745.75 STMH 18 STMH 18 STA 35+11 34 12 00
	<u>329.071</u>					Q100=71.5cfs		21 21 21 21 21 21 21 21 21 21 21 21 21 2
					-STA 31+55.81			BVCS: BVCE: STMH 18 STM 18
					CROSS 8 WM BTM STM=5731.00 TOP WTM=5729.30 CLEARANCE=1.7'			
· ·		Q5=75.7cfs Q100=105.3cfs	5		CLEARANCE=1.7'			
							ABOVE	36"RCP
· ·							A B	
								- 5YR
29-	+00	30+00	31+00	32+	-00 3	33+00	34+00	35+(



	CURV	Ε ΤΑ
CURVE	LENGTH	RADIL
C1	39,40′	25.00
С2	39,14′	25.00
СЗ	39.27′	25.00
C4	39,27′	25.00



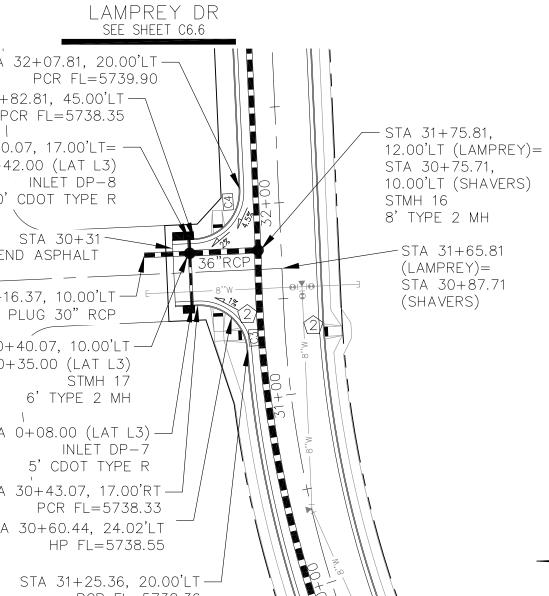
EDISTO DRIVE

				19,19			 		S	STM L	ATER/	AL 'LM	1' · · · ·
		(FONTA DISTO)	D. = 3.31 = 12.09	40.00′ ∨C	2+45.04 572720		· · · · ·			 			
PR	OPOSED		BVCE: 5725,90 EVCS: 1+35 EVCE: 5725.76		STA STA	2+60 0+00 572	· · · · ·			 	 		
C	STING RADE T_C/L	-2,00%		1.31%		0%-2.00% EARLY				5713.57		IN 5713.30 OUT 5712.30 -40 20	5713.51
· · ·		66" STM	18" STM		GRA — STA 1+49 -END PAVE	DING PLANS	· · · ·			INLET DP-41 STA 0+08 RIM 5718.20 INVERT OUT	STMH 12 STA 0+3 RIM 5717	INVERT IN INVERT OU INLET DP-40 STA 0+42 RIM 5718.20	
· · ·	24"	WTM					· · · ·	EXISTING OF ALIGI FINISHED OF ALIGI	NMEN GRAD			— 100YR — 5YR Ho	
· · ·		10"SS					· · · ·			27.00 LF @1.00% 24"RCP Q5=2.0cfs Q100=19.3ct		7.00 LF @3.00% 24"RCP Q5=12.9cfs Q100=26.0cfs	
· · ·							· · · ·				TM 13.25 711.55 1.7		
· · ·							· · · · ·			STA 0+15.00	CROSS 8" WTM BTM STM=5713.25 TOP WTM=5711.55 CLEARANCE=1.7'		
· · · ·													
0+	-00	1+	00	2+	00	3+()0	1		0+	00	1+0	

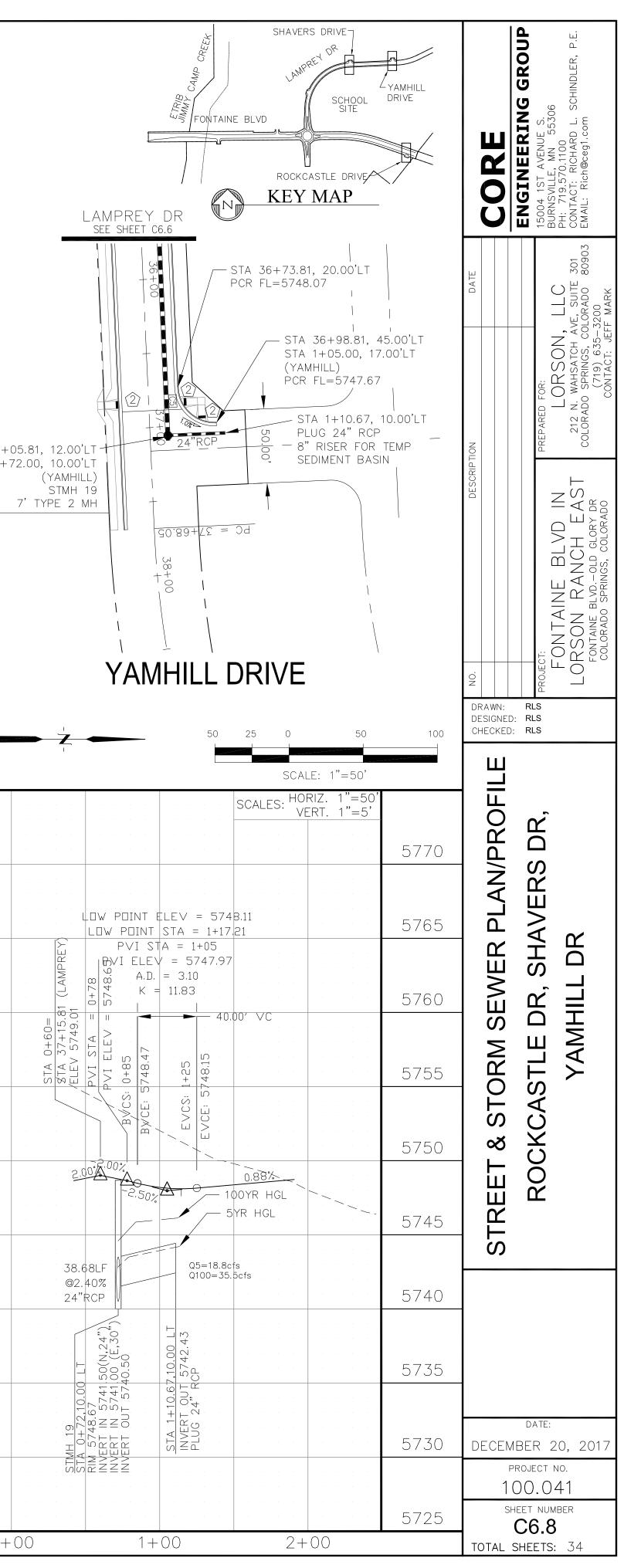
Image: 1 Image: 1 <td< th=""><th>3LE 5 DELTA 90°17′33″ 89°42′27″ 90°00′00″ 90°00′00″</th><th>EDISTO DRIVE FONTAINE BLVD LAMINE DRIVE KEY MAP</th><th>CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: RICH@Ceg1.com</th></td<>	3LE 5 DELTA 90°17′33″ 89°42′27″ 90°00′00″ 90°00′00″	EDISTO DRIVE FONTAINE BLVD LAMINE DRIVE KEY MAP	CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: RICH@Ceg1.com
10 10 <td< th=""><th></th><th>PDINT STA/DFFSET FLDWLINE 3 163+01.50, 32.16'RT 5718.51 4 163+81.70, 26.0'RT 5718.57 5 17+65.87, 17.00'LT 5717.74 6 17+70.57, 17.00'RT 5717.78 7 170+56.37, 26.00'LT 5725.42 8 1+16.00, 17.00'RT 5725.20 9 1+16.00, 17.00'RT 5725.20</th><th>DESCRIPTION DAT PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE COLORADO SPRINGS, COLORADO (719) 635–3200 (719) 635–3200 CONTACT: JEFF MARK</th></td<>		PDINT STA/DFFSET FLDWLINE 3 163+01.50, 32.16'RT 5718.51 4 163+81.70, 26.0'RT 5718.57 5 17+65.87, 17.00'LT 5717.74 6 17+70.57, 17.00'RT 5717.78 7 170+56.37, 26.00'LT 5725.42 8 1+16.00, 17.00'RT 5725.20 9 1+16.00, 17.00'RT 5725.20	DESCRIPTION DAT PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE COLORADO SPRINGS, COLORADO (719) 635–3200 (719) 635–3200 CONTACT: JEFF MARK
SCALES: HORIZ. 1"=50" IIIO 5740 5740 5735 5736 5736 5737 5737 5738 5738 5739 5739 5739 5730 5739 5730 5739 5725 5729 5710 5710 5700 5705 5700 5700 5700 5700 5700 100.041			ORAWN: RLS FONTAINE BLVDOLD COLORADO SPRINGS, COLORADO SPRINGS,
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SHEET NUMBER		5700	DECEMBER 20, 2017 project no. 100.041 sheet number

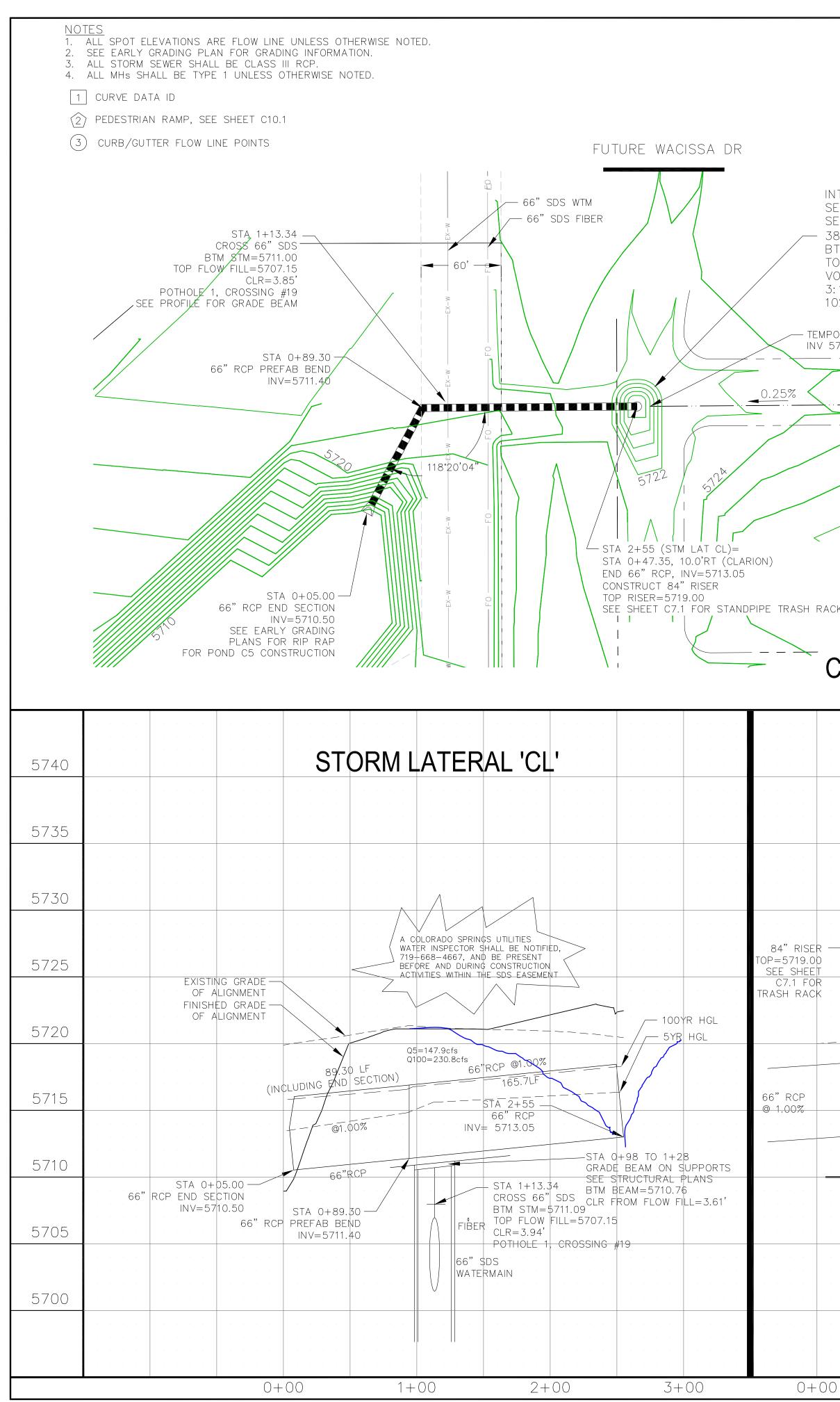
2. SE 3. Al 4. Al 1 2	<u>S</u> L SPOT ELEVATIONS ARE FLOW LINE UNLESS OTHERWISE NOTED. EE EARLY GRADING PLAN FOR GRADING INFORMATION. LL STORM SEWER SHALL BE CLASS III RCP. LL MHS SHALL BE TYPE 1 UNLESS OTHERWISE NOTED. CURVE DATA ID PEDESTRIAN RAMP, SEE SHEET C10.1 CURB/GUTTER FLOW LINE POINTS		FLCURB/GUTTERTABLEPDINTSTA/DFFSETFLDWLINE1187+83.40, 26.00'RT5748.29224+56.24, 17.00'LT5748.85324+56.24, 17.00'RT5749.104188+67.40, 26.00'RT5749.175188+08.40, 28.00'RT5748.556188+42.40, 28.00'RT5748.91	
	$\begin{array}{c} +6 \\ -7 \\ -7 \\ -7 \\ -7 \\ -7 \\ -7 \\ -7 \\ -$	A 188+15.40, 30:50LT A 188+15.40, 30:50LT A 25+37.74, 10.50°LT WH 8 PE 1 Mil	CURVE LENGTH RADIUS DELTA C1 39.27' 25.00 90°00'00" C2 39.27' 25.00 90°00'00" C3 38.41' 25.00 88°01'11" C4 39.27' 25.00 90°00'00" C5 39.27' 25.00 90°00'00"	A 37+05 TA 0+72 7
	ROCKCASTLE DRIVE	SHAVERS DRIVE		
	IGH PUINT ELEV = 5751.09 IIGH PUINT STA = 23+12.17 PVI STA = 23+13.16 PVI ELEV = 5751.23 A.D = -2.74 K = 14.62 91766+225 9176	LOW POINT ELEV = 5738.76 LOW POINT STA = 30+34.47 PVI STA = 30+34.47 PVI STA = 30+43 PVI ELEV = 5738.60 A.D. = 304 K = 16.45 5750 PROPOSED PROPOSED PROPOSED	STM LATERAL 'L3' (% 5750 (% 5750 (% 5750 (% 5750 (% 5750 (% 5750 (% 5750 (% 5750 (% 5745 (% 5745 (% 5745 (% 5745 (% 5740 (% 5740 (% 5740 (% 5740 (% 5735 (% 5735 (% 5735 (% 5735 (% 5735 (% 5735 (% 5730 (% 5730 (% 5725 (% 5725	
5740 5735 5730	STA 24+36.24, 10.00'LT USER END 18" RCP PLUG END PLUG END 05=4.0cfs 100=18.0cfs 100=18.0cfs WLM S 0 65 100=18.0cfs 100=18.0cfs 1100 100=18.0cfs 1100 100=18.0cfs 1100 100=18.0cfs 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100 1110 100	2722 577 577 577 577 577 577 577		

F	L CURB/GUTTER	TABLE
POINT	STA/OFFSET	FLOWLINE
1	187+83.40, 26.00'RT	5748.29
2	24+56.24, 17.00'LT	5748.85
3	24+56.24, 17.00'RT	5749.10
4	188+67.40, 26.00'RT	5749,17
5	188+08.40, 28.00'RT	5748.55
6	188+42.40, 28.00'RT	5748,91



	CURV	e tabl	E
CURVE	LENGTH	RADIUS	DELTA
C1	39,27′	25.00	90°00′00″
С2	39.27′	25.00	90°00′00″
СЗ	38,41′	25.00	88°01′11″
C 4	39.27′	25.00	90°00′00″
С5	39,27′	25.00	90°00′00″





	FL CURB/ PDINT STA/DFFSE 1 24+49.11, 2 6+49.93, 2 3 6+49.93, 2 4 25+26.98,	20.00'LT5729.8217.00'RT5728.9417.00'LT5728.94	E	CURVE LE C1 3	CURVE TAE ENGTH RADIUS 7.55' 25.00 87.55' 25.00			
SEE EARL 38'X65' A	IATE ATION BASIN 1 LY GRADING PLANS							Ī
BTM=5717 TOP=5722 VOLUME= 3:1 SIDE 10% ACCE TEMPORARY SV INV 5718.89	2.00 4,600 CF SLOPES ESS RD.		5	FUT.	URE MUMFOR	•		
5%		0.25		5121			AMPREY SEE SHEET C6.6	
RASH RACK	ON C BTM= Q100 VELO	ORARY SWALE FROM 48 CLARION TO 66" RCP AT 6' WIDE, 3:1 SIDE SLOF =140cfs, DEPTH=2.5', CITY=4.1fps W/COCONUT ECB ON SV	T 0.5% SLOPE PES /		48" RCP =5721.10 RAP PAD HICK, ON	24+00 8"1"	STA 7+3 PLUG 30 STA 7+13. INLET DP- 15' CDOT STA 24+78 STA 7+13.5 STMH 15TYF)"RCI 58,1(6b TYPE .15,1 58,10
CLAF	RION DRIVE		-	/	/			
						= (MUMFORD)	· · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·
	57.40 = 12.50 (WACISSA) $722.72 (WACISSA)$ $A = 0+72.40$ $A = 0+72.42$ $A = 0+75.40$ $A = 0+75.40$ $A = 0+75.40$ $A = 0+78.40$		ELEV = 5725	DSED GRADE AT RLY CRADING P EXISTIN GRAE AT C		STA 5+28.49 STA 6+30.13 ELEV 5727.67		
" RISER	STA 0+57.40 :: STA 6+12.50 (ELEV 5722.72 PVI ELEV = PVI ELEV = PVI ELEV = PVI ELEV =				0.88%		STA 6+49.93 END PAVEMEN	<u>1,42</u> 3
RCP 00%	TEMP SED. BA PER EARLY GI	ASIN RADING PLANS				TRAPEZOID SWALE 6' STRA	© 0.25% BTM, 3:1 SIDES W/COCONUT ECB ON SLOPES	
	<u>— STA 2+55 (LAT CL)=</u> STA 0+47.35 (CLARION) INV=5713.05					ST	A 6+20.00, 10.73'R NV 48" RCP=5721.10 INV SWALE=5720.20	0
							

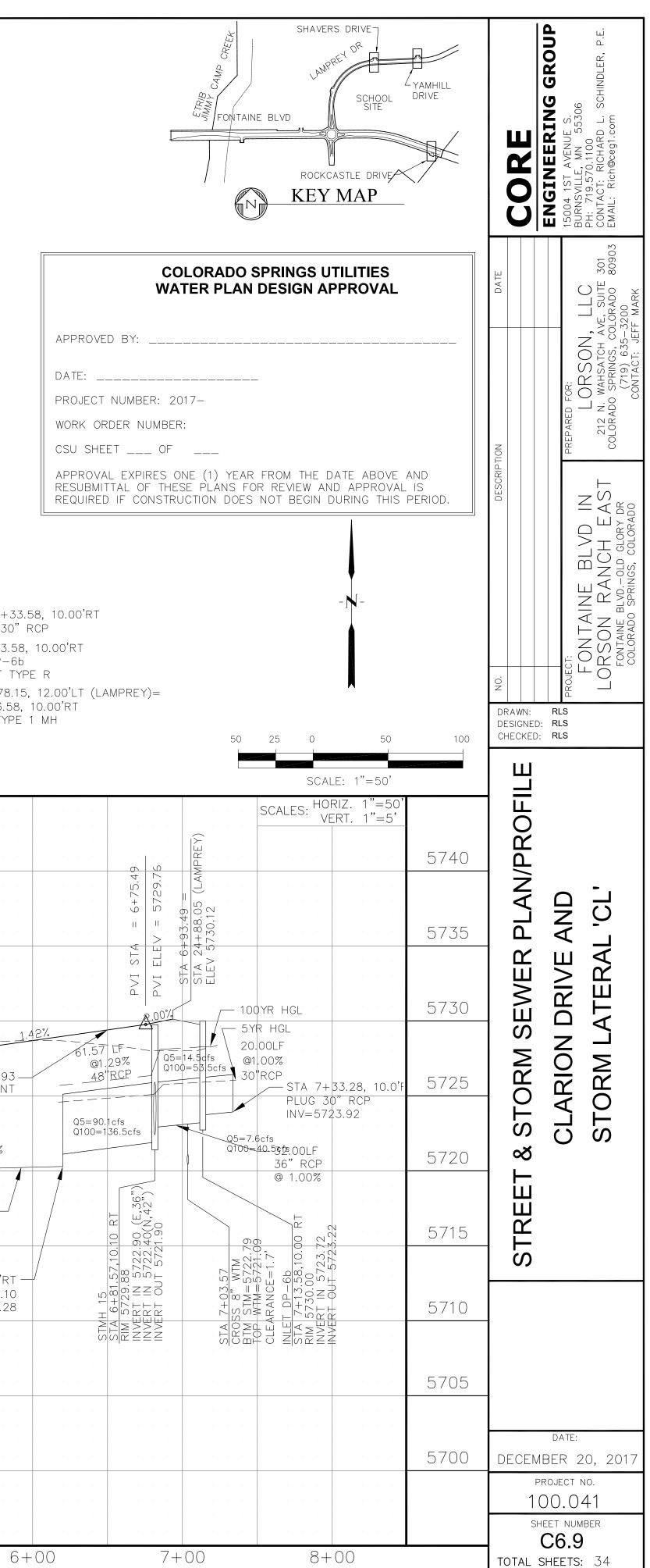
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2+00

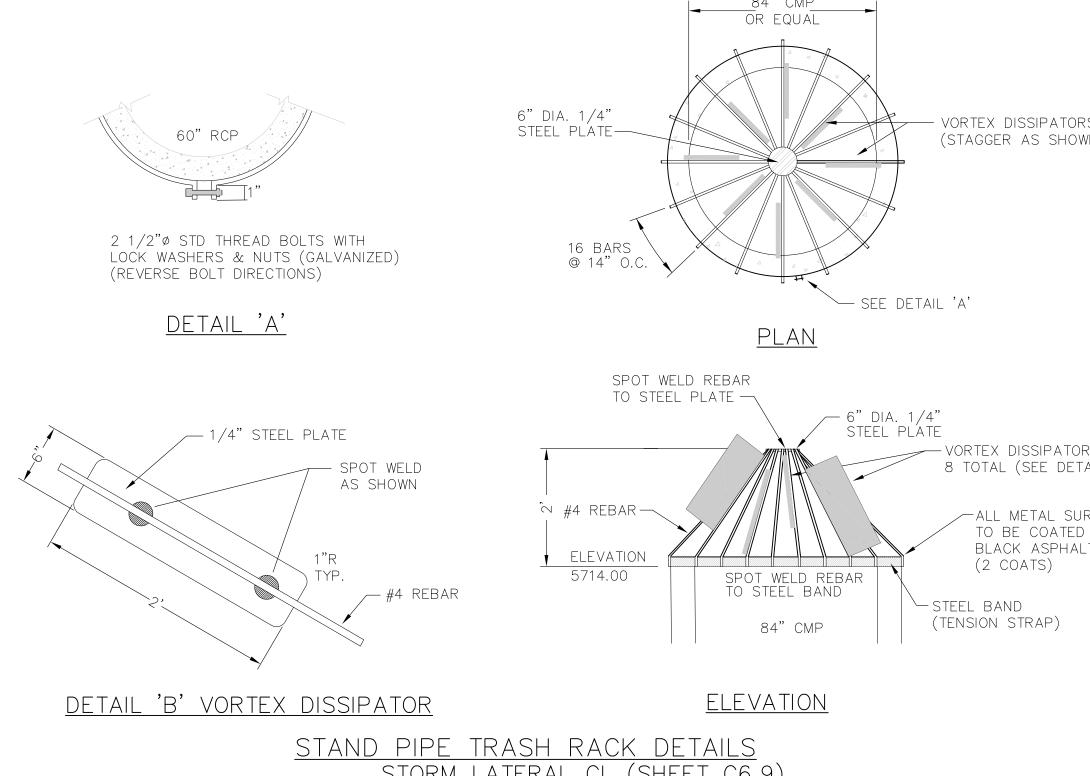
3+00

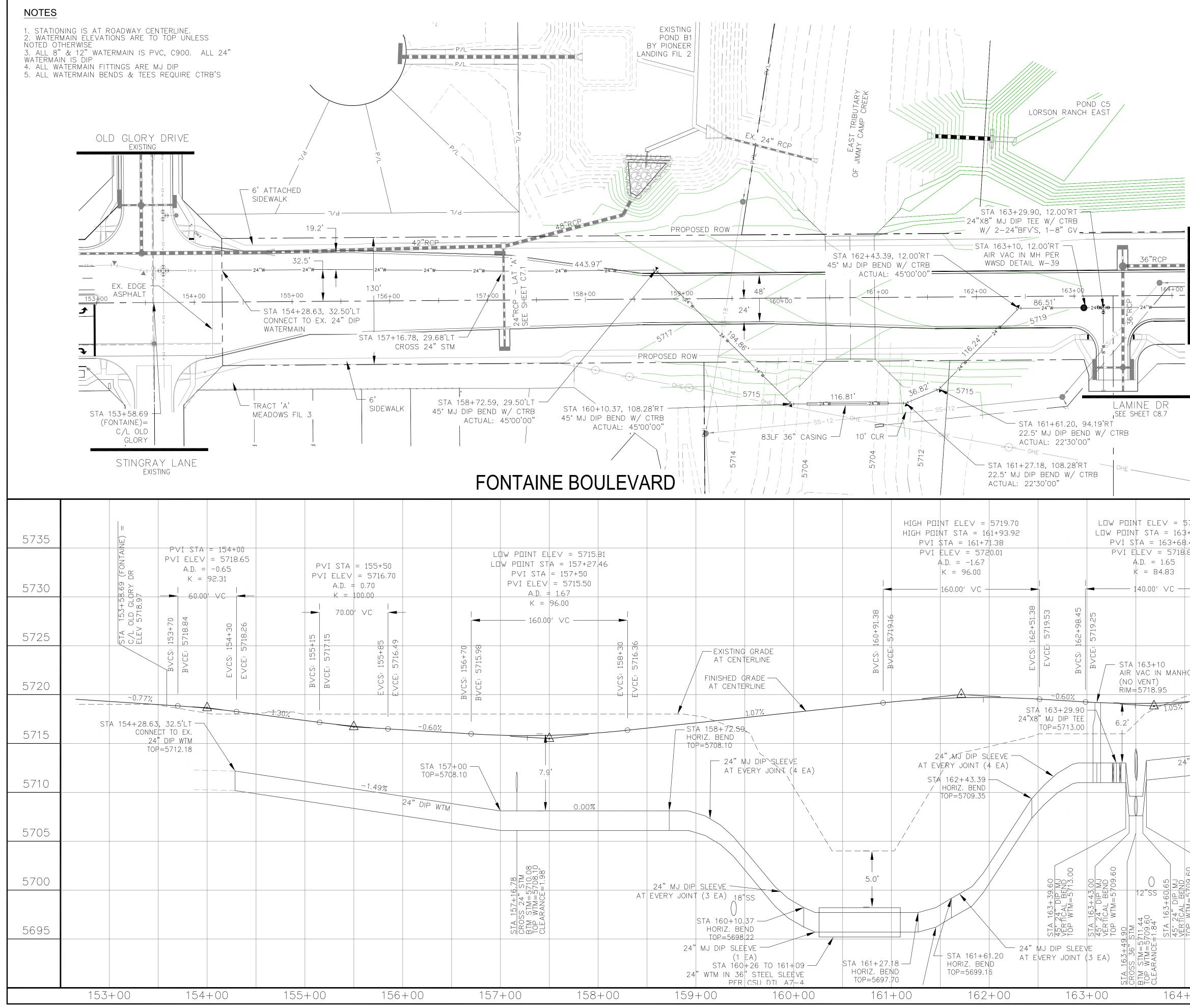
4+00

5+00



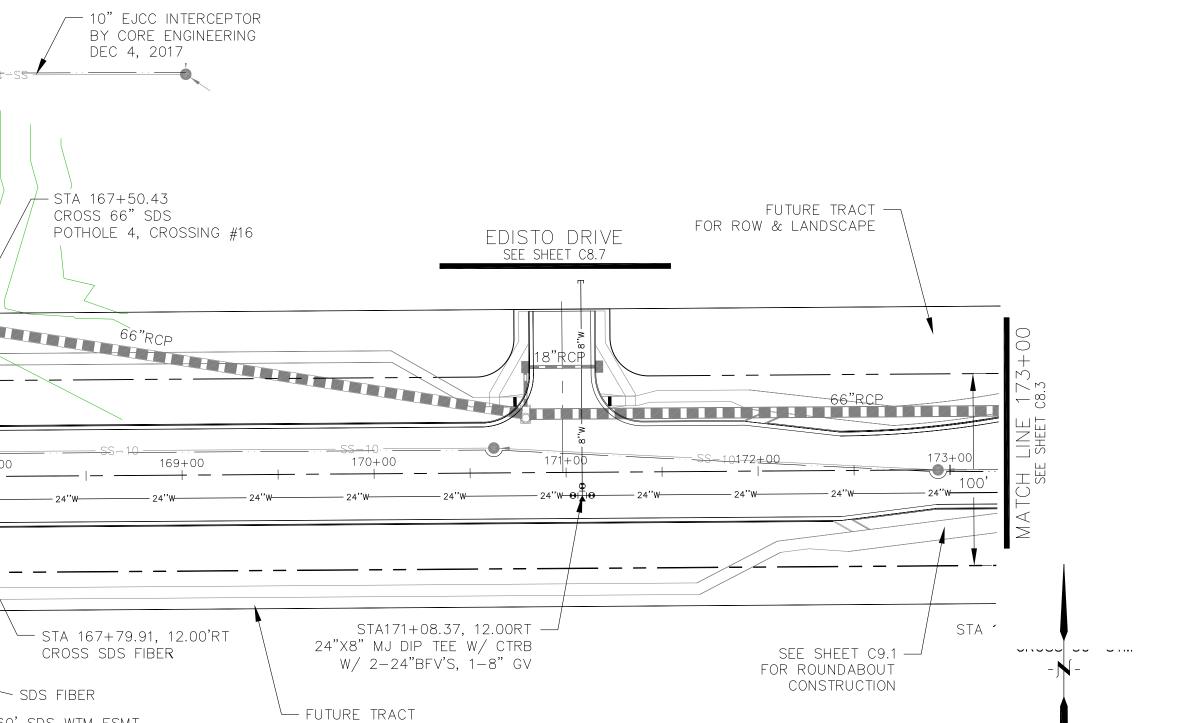
2. SEE EARLY GRADI 3. ALL STORM SEWE 4. ALL MHS SHALL E 1 CURVE DATA ID	NG PLAN FOR GRADING R SHALL BE CLASS III F BE TYPE 1 UNLESS OTH MP, SEE SHEET C10.1 FLOW LINE POINTS	RCP. ERWISE NOTED. STA 157+23.49, STA 157+23.49, STA 1+20.43 TN STA 157+19.86, d STA 1+10.78 INLE 10' CDO	A5.73 LT 3 (LAT A) STMH 9 YPE I MH 3.6.39 LT 8 (LAT A) 1/d ET DP-48 DT TYPE R 24''W R=600.00' B 157+00 CDOT TYPE R- 09.35,56.30 RT -17.50 (LAT A) INLET DP-47 CDOT TYPE R- TRAC	Z4"W- 24"W- PC=157+52.99	158+(158+(R=8 HONTAINE Second			2 1/2"¢ LOCK W (REVERS	60" RCP 50 THREAD BOL ASHERS & NUTS (SE BOLT DIRECTION DETAIL 'A' 1/4" STEEL 6' VORTEX I	PLATE SPOT WE AS SHOW 1"R 1"R TYP. #4 DISSIPATO	LD 'N REBAR <u>OR</u> ORM LAT	6" DIA. 1/4" STEEL PLATE 16 BARS @ 14" C SF TC N #4 REBA ELEVATI 5714.00 SH RACH ERAL CL TO SCALE	POT WELD RE D.C.	PLAN PLAN BAR BAR BAR BAR BAR BAR BAR BAR BAR BAR	- 6" DIA. 1/4" STEEL PLATE	- 'A' Vortex dis: 8 total (Si	SIPATORS EE DETAIL "B") TAL SURFACES COATED W/ ASPHALTIC PAIN TS)		KEY M	SCHOOL SITE	NO. DESCRIPTION DESCRIPTION DATE DATE COLORADO SPRINGS, COLORADO SPRINGS, COLORADO SOGOI EMAIL: RICHOGOGI.COM
		STORM	I LATERAL	'A'														50 25	5 0 SCALE:	50 10 	DRAWN: RLS DES IGNE D: RLS CHE CKE D: RLS
5735						 	· · · · · · ·					 				· · · · · ·			ALES: HORIZ. VERT.		
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5725			-47 .50 20 17 5710.71 -48 0.78 .81	5710.25 JT 5710.15 5.43 5710.10 5709.32 JT 5708.02			 					· · · · · · · · · · · · · · · · · · ·				· · · · · · · · · · · · · · · · · · ·				5725	
5720			ET DP- A 0+17 1 5715. ER 00 A 1+10 A 1+10 A 5714									· · · · · · · ·								5720	
5715		EXISTING GRADE	93.28 LF -		OYR HGL 'R HGL														5715	
5710			Q5=7.8cfs Q100=13.9cfs	9.66 9.66 9.66 24'RC 24'RC Q5=16.7c Q100=29.)% }P										· · · · · · · · · · · · · · · · · · ·				5710	
5700																				5700	
5695 · · · · · · · · · · ·			STA 0 +94	CROSS 24"-44.00 CROSS 24"-14 TOP WTM=51 CLEARANCE																5695	PROJECT NO.
		0+	00	+00			· · · · · ·													· · ·	100.041 Sheet Number C7.1 TOTAL SHEETS: 34



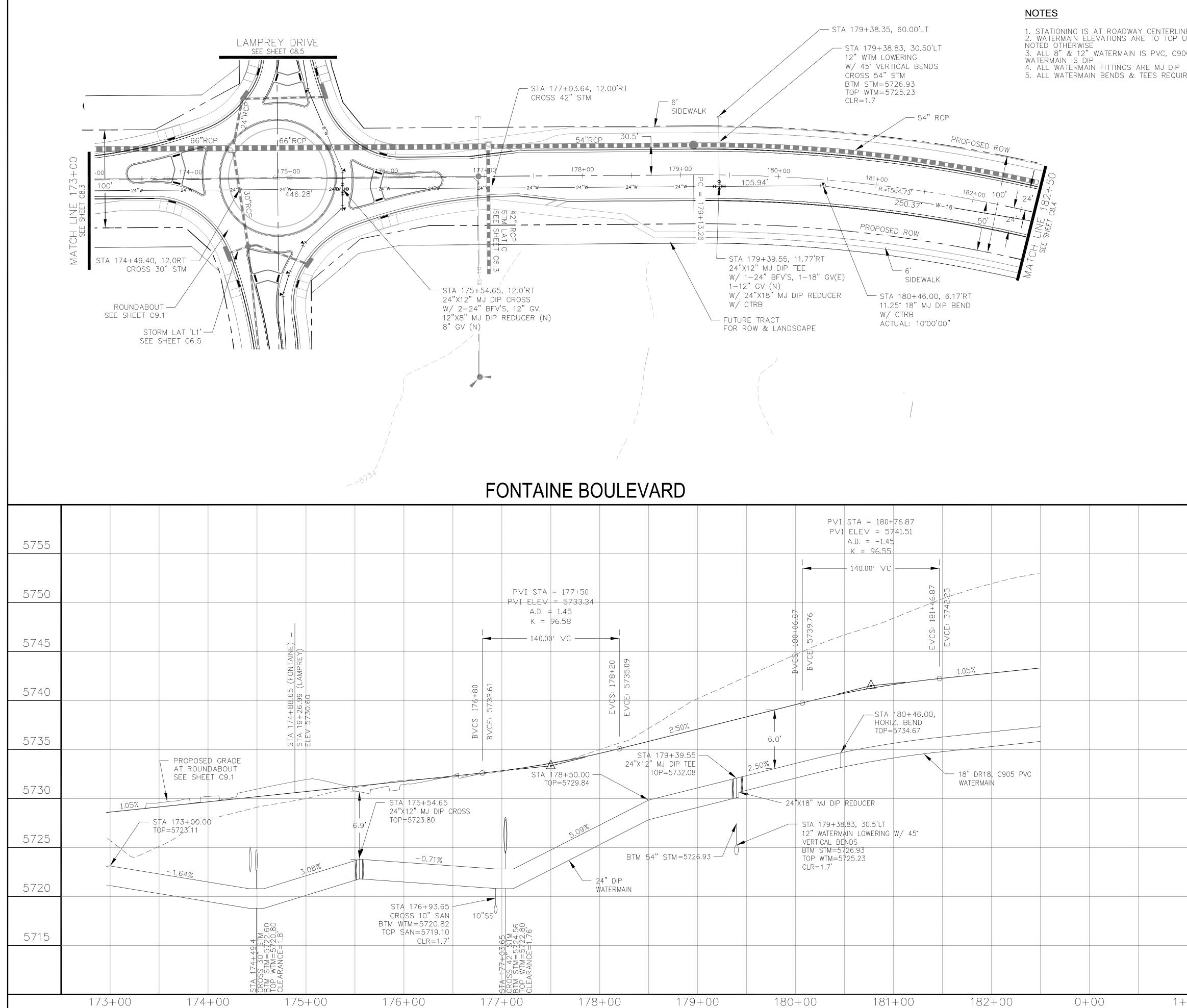


	FONTAINE BLVD KEY MAP	CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. 60NTACT: RICHARD L. SCHINDLER, P.E.
164+00 c8.2		DN DATE DATE DATE DATE PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK
MATCH LINE C SEE SHEET C		NO. DESCRIPTION NO. DESCRIPTION PROJECT: FONTAINE BLVD IN LORSON RANCH EAST FONTAINE BLVDOLD GLORY DR COLORADO SPRINGS, COLORADO
OHE	SCALE: 1"=50'	
5719.10	SCALES: HORIZ. 1"=50' VERT. 1"=5'	
3+49.35 3.45 .83	••••• ••••• ••••• 5735	
	1 1	PROFI EVARD 64+00
: 164+38,45 E: 5719,56	5725	WATERMAIN PLAN/PROFILE FONTAINE BOULEVARD STA 153+00 TO 164+00
E C C S E C C S E C C S E A C C C C C S E A C C C C C C C C C C C C C C C C C C		AIN INE 33+0
	• •	ERM 7 1; 7 1;
	5715	ATE FO ST
1.05% 4" DIP WTM	· ·	
	· · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · ·	-
=5709.60 +57.00 DIP MJ BEND =5713.25		
TOP WTM=57 STA 163+57. 45°24" DIP VERTICAL BE TOP WTM=57	5695	DATE: DECEMBER 20, 2017 project no.
+00	165+00	100.041 sheet number C8.1 total sheets: 34

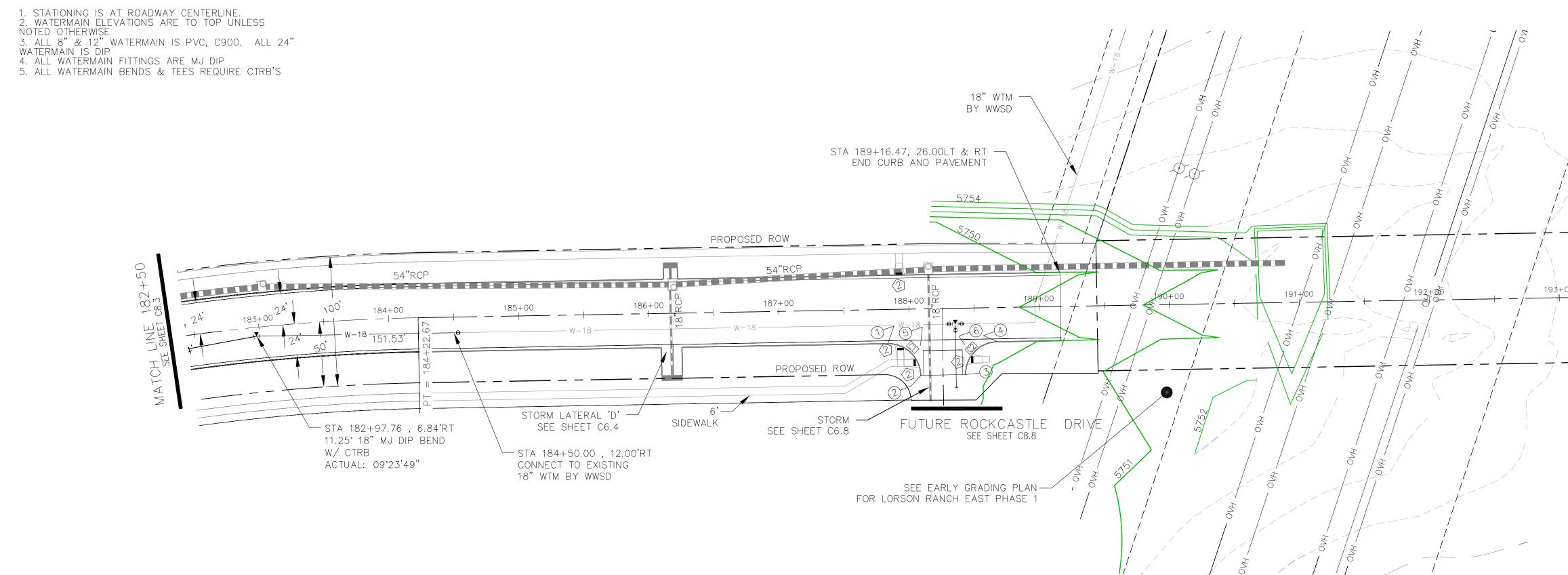
	POND C5							EX-WEX-W -	C C 	BY CORE DEC 4,	C INTERCEP E ENGINEER 2017	2 TOR 2ING						NOTES 1. STATIONII 2. WATERMA NOTED OTH 3. ALL 8" & WATERMAIN 4. ALL WAT 5. ALL WAT	NG IS AT RO IN ELEVATIO ERWISE & 12" WATEF IS DIP ERMAIN FITT ERMAIN BEN	DADWAY CEN DNS ARE TO RMAIN IS PVO INGS ARE MU DS & TEES F	ERLINE. Top unless 2, C900. All 2 Dip Require ctrb's	:4"	E CAMP CAMP CAMP
LORSON	RANCH EAST				PROF	66"RCP		EX-W	EO	- STA 167+50. CROSS 66" S POTHOLE 4, 66"F	.43 SDS CROSSING # RCP	#16		EDIS	STO DRIVE SHEET C8.7	- F(-	Futur Dr Row & La	E TRACT — NDSCAPE		- 00+			
	MATCH LINE 164+00 See Sheet C8.2	:P 3 164+00 164+00 ₩ 24''₩	6"RCP 30 	.5' 5+00 13' 50' 24"W 12' 50'	24"W	6' SIDEWALK 	+	24"W ×	24"W		169+00 +				171+00 24"W-0110		24"W	66"RCP	173+00 24"W	MATCH LINE 173 See Sheet C8.3		APPROVED BY: DATE: PROJECT NUMBER: WORK ORDER NUM	 : 2017— ИВЕR:
CSU RAW WA 2. CONTRAC 3. THE CSU PROVIDED B 4. A COLOR	SSINGS SHALL BE IN ATER TRANSMISSION L CTOR SHALL INVITE A J SDS WATERMAIN ANI Y CSU RADO SPRINGS UTILITI FORE AND DURING COM	INE CROSSINGS ND NOTIFY CSU D FIBER LOCATIO ES WATER INSPE	OF THE PREC NS ARE SHOV ICTOR SHALL	CONSTRUCTION MEE VN FROM AS-BUILT BE NOTIFIED, 719	LESS STDS Fing with w Drawings -668-4667,	/wsd		FC	SDS 60' SE	STA 167+79.9 CROSS SDS FIE S FIBER DS WTM ESMT	BER	FUTI FOR	STA171+08 "X8" MJ DIP T W/ 2-24"BF\ JRE TRACT ROW & LAND	√'S, 1−8" G\	3		FOR R	SHEET C9.1 DUNDABOUT NSTRUCTION	STA			CSU SHEET (APPROVAL EXPIRES RESUBMITTAL OF T REQUIRED IF CONS	ES ONE (1) These pl
5740 5735	.												3.7 (FONTAINE) =									
5730 5725					STING GRADE ED GRADE AT								1.05%	ELEV 5726.50 (EUISI									
5720									TO 167+80 WTM JOINT EMENT 1.05% 24" DIP W						24"X8" MJ DI TOP=5721.10		5.5'						
5715 5710			1.05% DIP WTM		167+49.91, CROSS 6	12.0'RT																	
5705 5700					CROSS 6 BTM WTM= FLOW FILL= OLE 4, CROSS	5706.44 CLR=8.8'	FIBER			WATER INS	ADO SPRINGS L SPECTOR SHAL -4667, AND BE ND DURING CC WITHIN THE S	UTILITIES L BE NOTIFIED, E PRESENT DNSTRUCTION SDS EASEMENT											
	164+00	165+C		166+00	167	7+00	168 1	3+00	169	2 + 00)+00	171+	- 00	172	+00	173	+00			0+0		+00



End End <th>LURSUN, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK</th>	LURSUN, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK
APPROVED BY:	
APPROVED BY:	
WORK ORDER NUMBER: CSU SHEET OF APPROVAL EXPIRES ONE (1) YEAR FROM THE DATE ABOVE AND RESUBMITTAL OF THESE PLANS FOR REVIEW AND APPROVAL IS REQUIRED IF CONSTRUCTION DOES NOT BEGIN DURING THIS PERIOD.	F UN TAINE BLVU IN LORSON RANCH EAS Fontaine blvdold glory dr colorado springs, colorado
DRAWN: RLS DESIGNED: RLS CHECKED: RLS	
SCALE: 1"=50' SCALES: HORIZ. 1"=50' VERT. 1"=5'	
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	ST
DATE:	20, 2017
0+00 1+00 0+00 1+00	



03.64, 12.00'RT ."STM 54"RCP 30.5' —	6' SIDEWALK		A 179+38.35, 60.00'LT - STA 179+38.83, 30.50'LT 12" WTM LOWERING W/ 45° VERTICAL BENDS CROSS 54" STM BTM STM=5726.93 TOP WTM=5725.23 CLR=1.7 54" RC	1. 2. N 3. W 4. 5.	OTES STATIONING IS WATERMAIN EL OTED OTHERWIS ALL 8" & 12" ATERMAIN IS DI ALL WATERMA ALL WATERMA	AT ROADWAY C LEVATIONS ARE WATERMAIN IS I P IN FITTINGS ARE IN BENDS & TEE	ENTERLINE. O TOP UNLESS PVC, C900. ALL MJ DIP S REQUIRE CTRE	L 24" 3'S	AND	KEY MAP		CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
178+00 - 24"W 24"W	24 W II I	180+00 05.94' 179+39.55, 11.77'RT (12" MJ DIP TEE 1-24" BFV'S, 1-18" GV(E) 2" GV (N) 24"X18" MJ DIP REDUCER CTRB RE TRACT ROW & LANDSCAPE	$ \begin{array}{c} 181+00 \\ +R=1504.73' + $	6.17'RT ? BEND	SEE SHEET C8.4 2450							NO. DESCRIPTION DESCRIPTION DATE DATE DATE DECOLORADO RANCH EAST PREPARED FOR: LORSON RANCH EAST PREPARED FOR: LORSON RANCH EAST PREPARED FOR: COLORADO SPRINGS, SOLOSI SPRINGS, SOLOSI SPRINGS, SOLOSI SPRINGS, SOLOSI SPRINGS, SOLOSI SPRINGS, COLORADO SPRINGS, COLOR
E BOULEV	ARD								50 25	- N - 0 50 SCALE: 1"=50'	100	URAWN: RLS DESIGNED: RLS CHECKED: RLS
			I STA = 180+76.87 I ELEV = 5741.51				· · · · · ·	· · · · · · ·	SCAL	ES: HORIZ. 1"=50' VERT. 1"=5'		
· · · · · · · · ·	· · · · · · · · · · · ·		A.D. $x = -1.45$ K = 96.55 140.00' VC				· · · · · · ·				5755	AN/PROFILE ULEVARD 0 182+50
77+50 733.34			46.87								5750	/PROFI EVARD 182+50
		3, 180+06.87 EI, 5739.76	CS: 181+								5745	
178+20			0	1.05%				· · · · · · · ·				MAIN F AINE B 173+00
E SS E C C E C C E C											5740	
	2,50%	6.0' · · · · ·	HORIZ. BEND TOP=5734.67								5735	WATE FON ST/
24"X12"	179+39.55 MJ DIP TEE TOP=5732.08	2.50% 24"X18" MJ DIP	REDUCER	— 18" DR18, C905 PVC WATERMAIN							5730	
5.09%		STA 179+38 12" WATERM	8.83, 30.5'LT /AIN LOWERING W/ 45°								トフクト	
	4" STM=5726.93 —	VERTICAL BE BTM STM=5 TOP WTM=5 CLR=1.7'	ENDS 726.93								5725	
24" DIP WATERMAIN							· · · · · · ·				5720	
											5715	DATE: DECEMBER 20, 2017
												project no. 100.041 Sheet number
178+00	179+00	180+00	181+00	182+00	0+	00	1+00	2	+00	3+00		C8.3 TOTAL SHEETS: 34

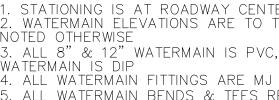


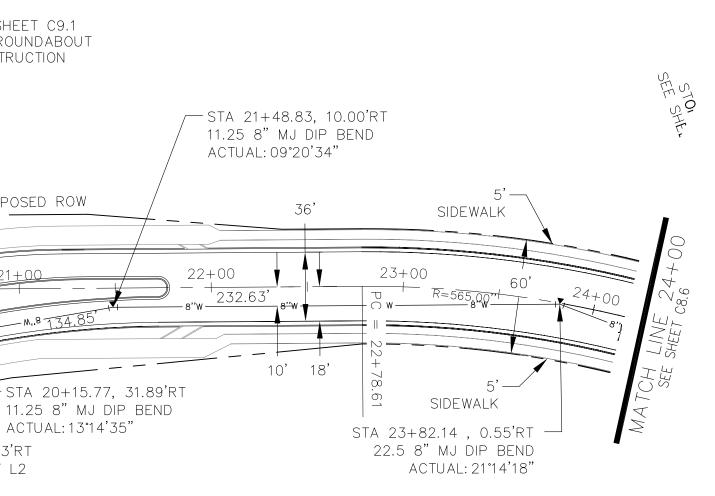
FONTAINE BOULEVARD

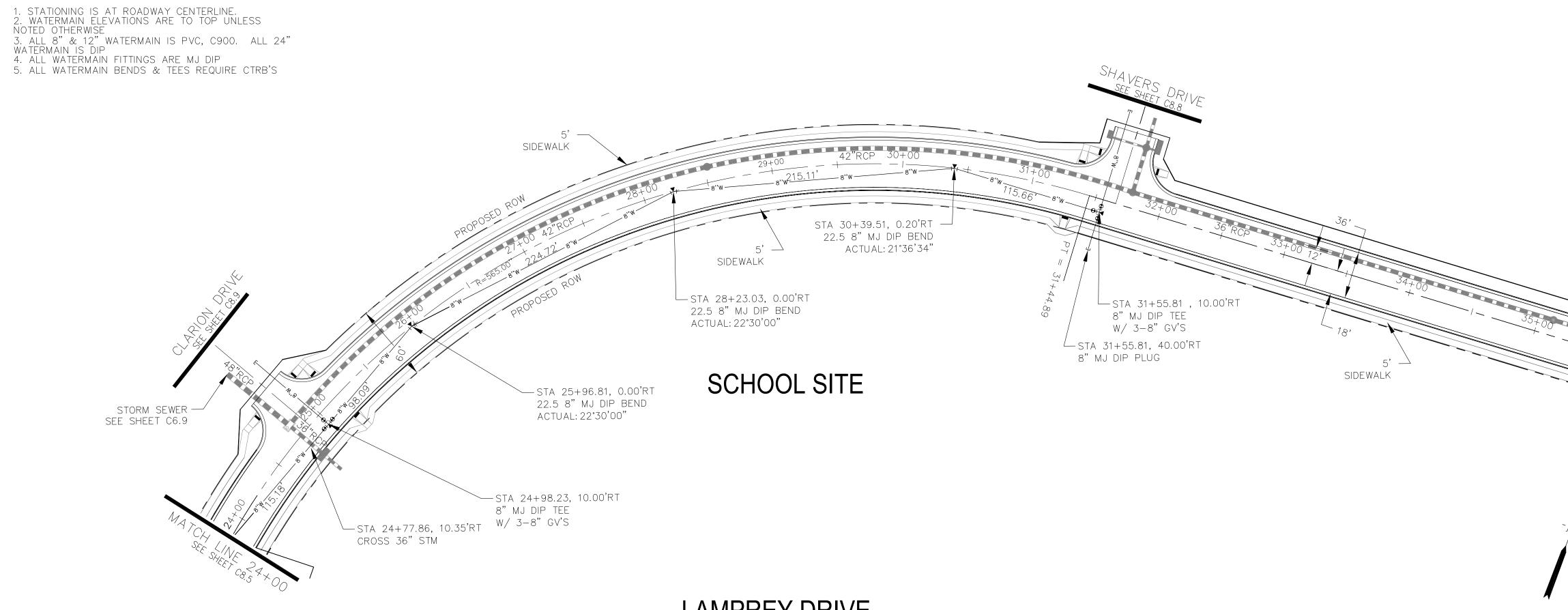
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	ANNIN FONTAIN BILLIS BI	NE BLVD		CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
				N, LLC NAVE, SUITE 301 COLORADO 80903 55-3200 JEFF MARK
+00	— — F			DESCRIPTION N PREPARED FOR: LORSON, LL(212 N. WAHSATCH AVE, SU COLORADO SPRINGS, COLORAD (719) 635-3200 COLORADO CONTACT: JEFF MAR
				NE BLVD IN RANCH EA Ervides, colorado
	50	25 0 50	100	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
		SCALE: 1"=50' SCALES: HORIZ. 1"=50' VERT. 1"=5'		-
		VERT. 1"=5'	5765	Ш
			5760	PROFILE EVARD 91+00
			5755	PLAN/I BOULE 0 TO 1
			5750	ATERMAIN PI FONTAINE B STA 182+50
			5745	WATERMAIN FONTAINE STA 182+5
			5740	5
			5735	
			5730	DATE:
			5725	DATE: DECEMBER 20, 2017 PROJECT NO. 100.041
		2+00		SHEET NUMBER

		- 24"W				4. ALL WAIERN	IS AT ROADWAY CENTERLINE. ELEVATIONS ARE TO TOP UNLE 1'SE 2" WATERMAIN IS PVC, C900. DIP 1AIN FITTINGS ARE MJ DIP 1AIN BENDS & TEES REQUIRE C	STRB'S	SCHOOL SITE	CORE CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. BORNESTICE
STA 16+62.60 45° 12" MJ I ACTUAL: STA 16+28.83, 2.00RT 8" MJ DIP PLUG 6 6 6 6 7 7 8 16 10 10 10 10 10 10 10 10 10 10	DIP BEND -3'04'45"	24"RCP	SEE SHEET C9.1 FOR ROUNDABOUT CONSTRUCTION PROPOSED ROW							PREPARED FOR: PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE 3C COLORADO SPRINGS, COLORADO 80 (719) 635-3200 COLORADO SPRINGS, COLORADO 80 (719) 635-3200 CONTACT: JEFF MARK
/ 22.5° 12" ACTUA	17+00 18+00 135.48 135.48 6'RT 135.48 3END 135.48 4'41" 135.48 Y.71, 4.47'RT STA 18+95.95, 66.00'R AJ DIP BEND STA 18+95.95, 66.00'R L: 20'03'58" 22.5' 12" MJ DIP BEN 9.41, 11.19'RT ACTUAL: 20'34'58 STA 18+95.95, 66.00'R STA 18+95.95, 66.00'R		21+00 N ⁸ 134.85 STA 20+15.77, 31.89'R 11.25 8" MJ DIP BEND ACTUAL: 13°14'35" A 20+08.95, 34.73'RT OSS 24" STM LAT L2 E SHEET C6.5 STA 19+33.78 , 66.00'RT 2.5 8" MJ DIP BEND ACTUAL: 22°35'10"	T 10' 18' T S STA 23+82. 22.5 8"	60' 24+00 5' 24+00 6'' 24+00 10 10 10 10 10 10 10 10 10					10. DESCRIF OLECT: FONTAINE BLVD IN ORSON RANCH EAST FONTAINE BLVDOLD GLORY DR COLORADO SPRINGS, COLORADO
ACTU STA 18+4.	AL: 09°33'32" 3.10, 18.11'RT ' STM LAT L1 SHEET C6.5 W/ 2-24" BFV'S, 12" GV, 12"X8" MJ DIP REDUCER (N) 8" GV (N)		LAMPREY DRIVE					50 25	0 50 1 SCALE: 1"=50' S: HORIZ. 1"=50' VERT. 1"=5'	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
$5750 \qquad \begin{array}{c} P \lor I \ E L E \lor \\ A.D. = 1.7 \\ K = 83.9 \end{array}$	5734.49 '9								575	
		STA = 18+60.12 00 LOW POINT S /I STA = 18+60 00 PVI STA = 1 ELEV = 5729.26 + PVI ELEV = 5	EV = 5729.48 TA = 20+05.92 $\overline{729.34}$	HIGH POINT ELE HIGH POINT STA PVI STA = PVI ELEV = A.D. = -	A = 23+04,32 = 23+00 = 5731.63 -1.87			.	574	Ц С С Ш О С П О С П О О П
5740		A.D. = 4.01 7 A.D. = 2.6 K = 19.94 7 K = 22.9 - 80.00' VC 5 60.00' VC		$K = 5^{2}$	4.32 · · · · · · · · · · · · · · · · · · ·				574	TO 2
	EARLY CS: 18+20 E: 5730.07	CS: 19+00 E: 5730,06 E: 5730,06 ILEV = 5730,6 CS: 19+60 E: 5729,94 E: 5729,94	S: 20+20 S	STA = 22+00 LEV = 5730.6 VCS: 22+40 VCF: 5731.03					574	AMPRE 22
5730 STA 16+43.83 END PAVEMENT	5.5' <u>STA 18+07.71</u> 12" HORIZ BEND TOP=5724.81	$\begin{array}{c c} & & & & \\ & & & \\ & & & \\ & & & \\$			$\frac{1}{2} = \frac{1}{2} = \frac{1}$		<td< th=""><th> <td< th=""><th>573</th><th>ST/ ST/ ST/</th></td<></th></td<>	<td< th=""><th>573</th><th>ST/ ST/ ST/</th></td<>	573	ST/ ST/ ST/
5725	-2.01% STA 16+28.83, 2.0'RT 12" MJ DIP PLUG	STA 19+14.99 24"X12" CROSS TOP=5723.80 8" 0.00% 0.00%		0.32%					572	<u>25</u>
5720	AND 2" TEMP BOV TOP=5728.41			0.32% 8" PVC WTM					572 	<u>20</u>
5715 · · · · · · · · · · · · · · · · · · ·	STA 18+43.10 CROSS 24" STM BTM STM=5725.1 TOP WTM=5725.1 CLEARANCE=1.84	0 8.38 8.38 8.38 1.7 7 8.38 8.38 1.2 1.2 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.3	VD 18.38 VD 22.65 77,65 77,65 77,65 77,65 72.65 70 77,65 70 77,65 70 70 70 70 70 70 70 70 70 70 70 70 70		<td></td> <td></td> <td>. .<td>571 571 571 571 571</td><td>DATE:</td></td>			. . <td>571 571 571 571 571</td> <td>DATE:</td>	571 571 571 571 571	DATE:
		CLEARANCE 19+44.73 STA 19+44.73 VERTICAL BEND VERTICAL VERTIC	C 45° 8″ DIP MJ VERTICAL BEND TOP WTM=5718.38 45° 8″ DIP MJ VERTICAL BEND TOP WTM=5722.65 CROSS 24″ STM ETM STM=5724.35 TOP WTM=5722.65 CLEARANCE=1.7'	22+00 23+0		· · · · · · · · · · · · · · · · · · ·	1+00	0+00 1+00		PROJECT NO. 100.041 SHEET NUMBER C8.5 TOTAL SHEETS: 34

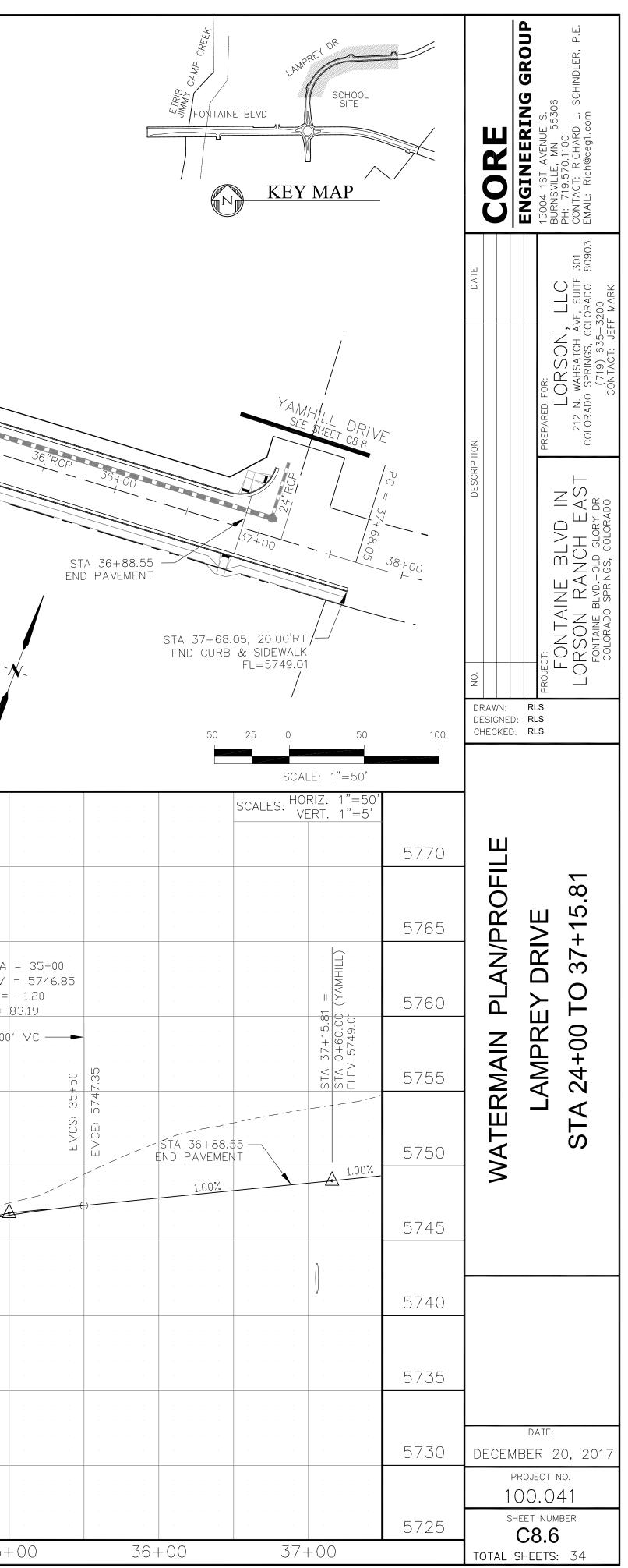


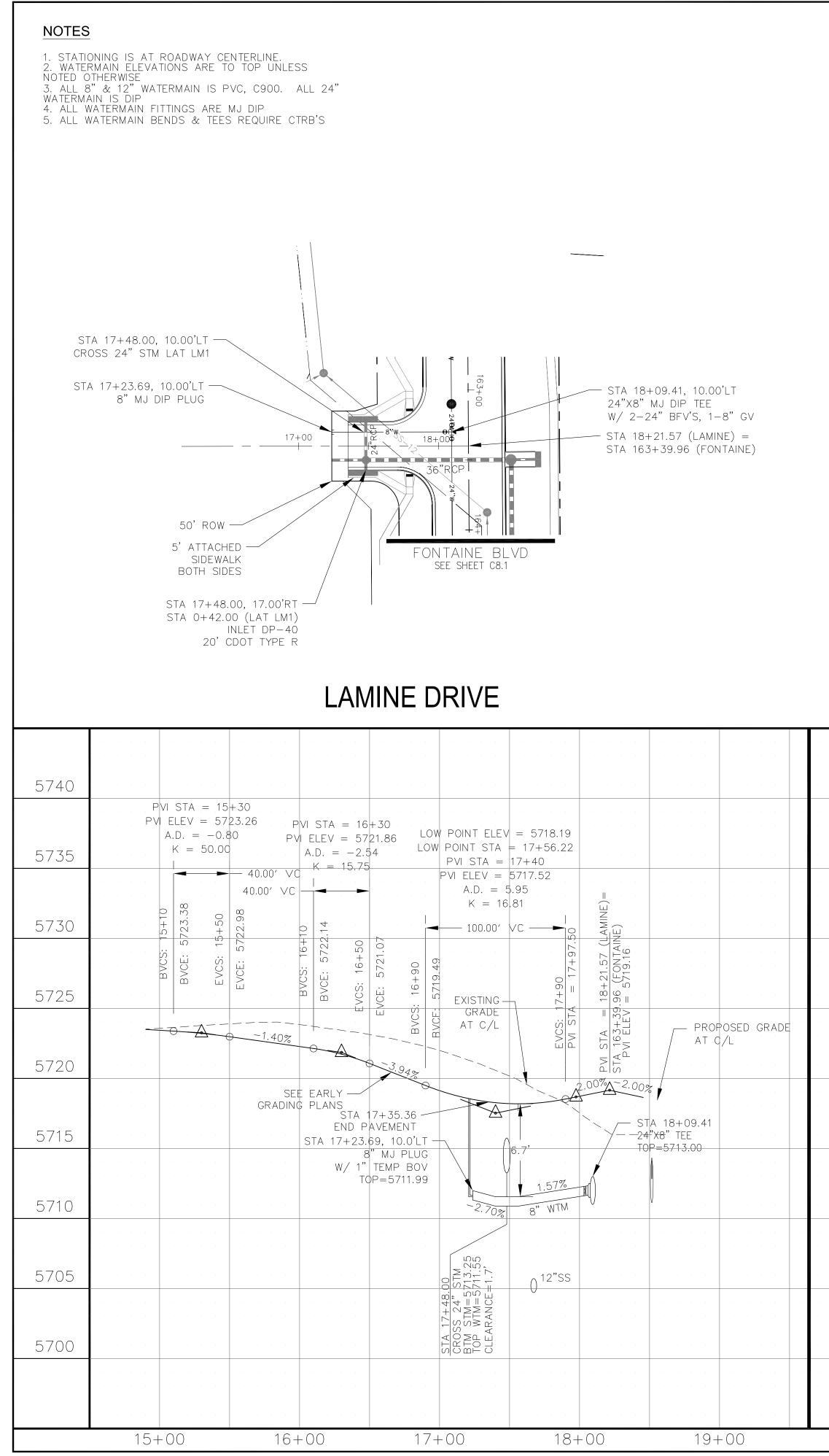




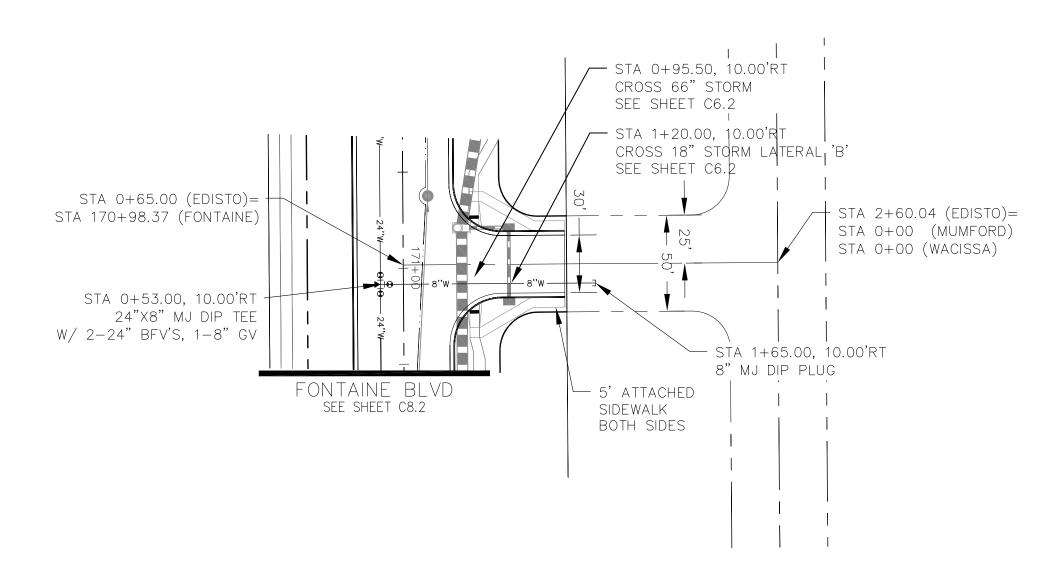
5750										VERS)				
		L'OW POINT ELEV" = 5730.09		· · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·				81 =		· · · · · · · · · · · · · · · · · · ·		
5745		LOW POINT STA = $25+04.15$ PVI STA = $25-25$ PVI ELEV = 5729.68				· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · ·		PVI STA 31+65.81 = STA 30+87.719 (SHAVERS) ELEV 5739.50				· · · · · · · ·
		$\begin{array}{c} \text{A.D.} = 2.40 \\ \text{K} = 62.55 \end{array}$		· · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·				STA V 57,1				
5740		150.00′ VC				· · · · · · · · · · · · · · · · · · ·				ELE STA				$\begin{array}{c c} P \lor I & STA = \\ P \lor I & ELE \lor = \\ A.D. = -1 \end{array}$
5740			90,83									· · · · ·		K = 83
	24+50 2000		2 <u>2</u>						STA 31+55.81 8" MJ DIP TEE 5 c, TOP=5733.65				· · · · · · · ·	→ 100.00′ N
5735		+ 88.05= 93.49 (Cl							5.6, TOP=5733.65					50
5730	-0,87%	STA 24 STA 6+ ELEV 5-											· · · · · · · · · ·	CS: 34+ CE: 5745
		STA 24+98.2 8" MJ DIP TEE	23	5.4'		1.46%								B
5725	6.2' ·	TOP=5724.07			8" PVC WTM									=
						· · · · · · · · · · · ·		· · · · · · · · · · ·				· · · · · · · · ·		· · · · · · ·
5720														· · · · · ·
5715														
				· · · · · · · · ·								· · · · · · · · · · · ·		
5710	66.88 66.88 P MJ 1=5724.07 69.86 P MJ BEND BEND BEND	36 2721.09 36 1.7' 21.09 721.09 721.09 721.09 724.07 724.07												
	STA 24+66.88 45° 8″ DIP MJ VERTICAL BEND TOP WTM=5724.07 STA 24+69.86 45° 8″ DIP MJ VERTICAL BEND 109 WTM=5721.09	STA 24+77.86 CROSS 36" STM BTM STM=5722.79 TOP WTM=5722.09 CLEARANCE = 1.7' STA 24+85.86 45° 8" DIP MJ VERTICAL BEND TOP WTM=5721.09 STA 24+88.84 45° 8" DIP MJ VERTICAL BEND TOP WTM=5724.07												
	24+00	25+00 2	26+00	27+00	28+0	29)+00	30+00	31+00	32+	- 00	33+00	34+00	35+0

LAMPREY DRIVE





	CURV	Ε ΤΑ
CURVE	LENGTH	RADIL
C1	37,46′	25.00
С2	39,14′	25.00
СЗ	39,27′	25.00
C4	39,27′	25.00



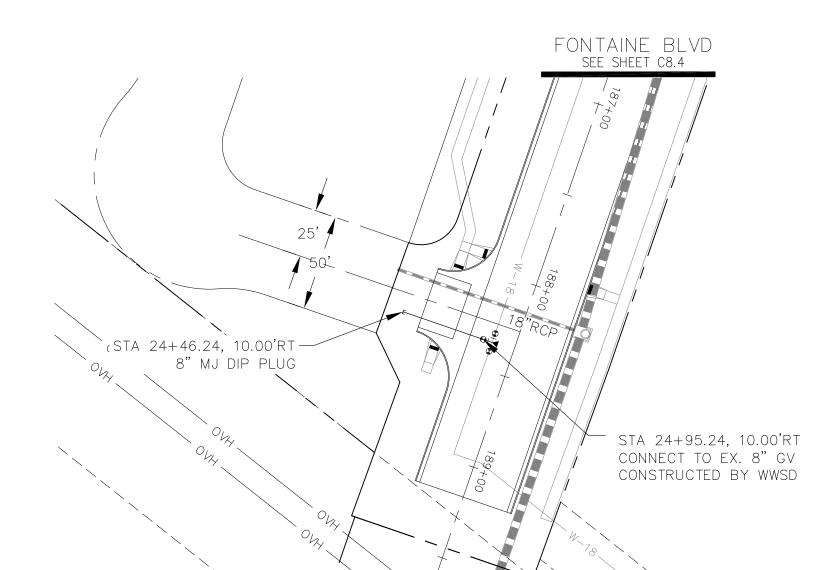
EDISTO DRIVE

$L \Box W P \Box I NT E L E V = 5$ $L \Box W P \Box I NT STA = 1+$ $H P V I STA = 1+15$ $\Psi P V I E L E V = 5725$	19.19		 		
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BVCS: 0+65.0 BVCS: 0+65.0 BVCS: 0+65.0 BVCS: 0+95	I STA	STA 2+60.0 STA 0+00 ELEV 5727.			
EXISTING GRADE AT C/L -2.00%	1.31% SEE E	¹ <u>−2,00%</u> ARLY			
STA 0+53.00 24"X8" MJ DIP TEE TOP=5721.10 0.00% 1.31%	1' STA 1+49 -END PAVEN	ING PLANS	· · · · ·		
	STA 1+65.00 8" MJ PLUG W/ 1" TEMP TOP=5720.11		 		
STA 0+80.46 45°8 DIP MJ VERTICAL BEND TOP WTM=5720.43 0+87.50 0+87.50 0+87.50 70P WTM=5713.39 70P WTM=5713.39	VER FICAL BEND TOP WTM= 5713.39 STA 1+09.49 VER FICAL BEND TOP WTM=5719.38 STA 1+20.00 CROSS 18" STM CROSS 18" STM ETM STM=5721.22 TOP WTM=5719.52 CLEARANCE=1.7'				
STA STA STA STA STA CER STA STA STA STA STA STA STA STA STA STA	CLEAX SOLUTION CONTRACTOR TO CROMING CROME				
0+00 1+00	2+00	3+00	0+0)0	1+00

BLE S DELTA 85°50'39" 89°42'27" 90°00'00" 90°00'00"	CHARD CHARD			CORE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
	PDINT STA/DF 3 163+01. 4 163+81. 5 17+65.8 6 17+70.5 7 170+56 8 1+16.00, 9 1+16.00,	B/GUTTER TABL FSET FLOWL 50, 32.16'RT 5718.5 70, 26.0'RT 5718.5 37, 17.00'LT 5717.7 57, 17.00'RT 5717.7 .37, 26.00'LT 5725.4 , 17.00'RT 5725.4 37, 26.00'LT 5726.3	_INE 51 57 74 78 42 20 20	RIPTION DATE DATE PREPARED FOR: LORSON, LLC 212 N. WAHSATCH AVE, SUITE 301 COLORADO SPRINGS, COLORADO 80903 (719) 635-3200 CONTACT: JEFF MARK
				NO. NO. PESCRIP PROJECT: FONTAINE BLVD IN LORSON RANCH EAST FONTAINE BLVD-OLD GLORY DR COLORADO SPRINGS, COLORADO
	50	25 0 50 SCALE: 1"=50'	100	DESIGNED: RLS Checked: RLS
	· · · · · ·	SCALES: HORIZ. 1"=50 VERT. 1"=5"	-	
	· · · · · ·		5740	ROFILE VE
			5735	
			5730	PLAN E DR STO I
			5725	WATERMAIN PLAN/PF LAMINE DRIVE AND EDISTO DRI
			5720	VATE
			5715	
				-
			5710	
			5705	
			5700	date: DECEMBER 20, 2017
				PROJECT NO.

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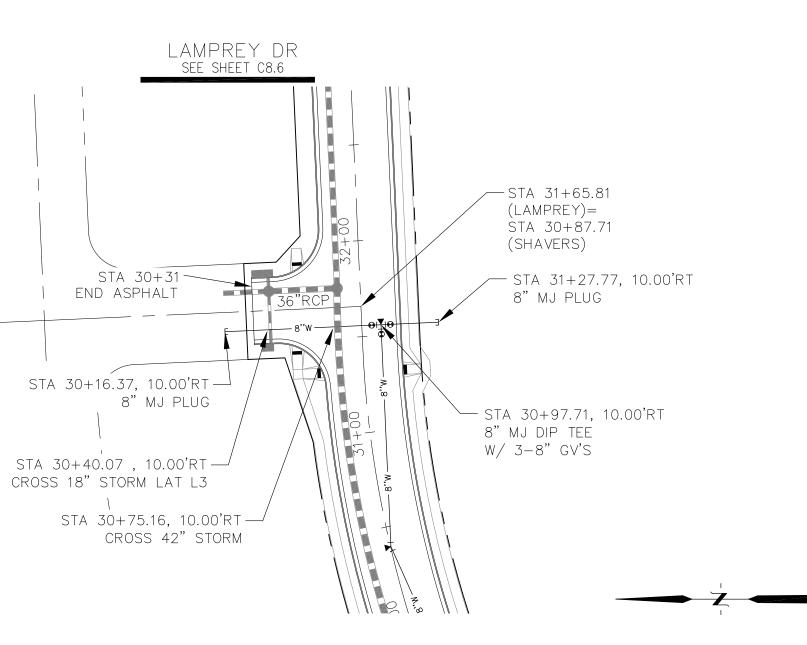
- 1. STATIONING IS AT ROADWAY CENTERLINE. 2. WATERMAIN ELEVATIONS ARE TO TOP UNLESS NOTED OTHERWISE 3. ALL 8"& 12" WATERMAIN IS PVC, C900. ALL 24" WATERMAIN IS DIP 4. ALL WATERMAIN FITTINGS ARE MJ DIP 5. ALL WATERMAIN BENDS & TEES REQUIRE CTRB'S



ROCKCASTLE DRIVE

		RUUM	CASILE DRIVE	=		517												
5770							ELEV = 5738.76 T STA = 30+34.47											
5770						PVI (STA = 30+43											-
	HIGH POINT ELEV = HIGH POINT STA =						_EV = 5738,60 ° D, = 3,04 ° ° °			· · · · ·	· · · · · · ·	· · · · · · ·						
	A = A = A = A = A = A = A = A = A = A =	-13.16			builden builden builden builden	К	(16,45)											
5765	PVI ELEV = 57 A.D. = -2.74			5750			50,00′ VC -							5750	· · · ·	· · · ·	· · · · ·	
	K = 14.62		I ELEV = 5748.90 $I STA = 24+79.24$ $I ELEV = 5748.74$ $STA = 24+83.24$ $ELEV = 5748.90$ $ELEV = 5748.90$ $I 88+25.40 (FONTAINE$ $V=5749.38$		(M			STA 30+87.71 = STA 31+65.81 (LAMPF ELEV 5739.50										
			5748,90 5748,90 5748,74 5748,74 5748,74 5748,90 0 (FONTA		PROPOSED + 6000		30+68 5739.11	0 0 []										
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	22+93.16 5750.97 23+33.16	600 □	A A A A F C 33+255 749.5			EXISTING S	E C C S	30+31+1										
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						-1.00%	3.00,42.0	2%										
		PROPOSED			SEE EARLY				- STA 30+97.71 8" MJ DIP TEE									
5750		AT C/L -1,44%		5735	GRADING PLANS STA	30+31.37 PAVEMENT	6.3' 5.8'		TOP=5733.65					5735				
		1,44/	2.00%															
	SEE EARLY GRADING PLANS				STA 30+16.3 8" MJ PLUG W/ 1" TC	7, 10.00 RT			- STA 31+07 77	10.00'PT								
5745	GRADING PLANS			5730	TC)P=5732.97			- STA 31+27.77 8" MJ PLUG TOP=5733.65					5730				
		STA 24+56.24/		5750					TOP=5/33.65					5750				
5740							++4 											
5740				5725			BTM STM=5734.43 TOP WTM=5732.73 CLEARANCE=1.7' 3.97 5732.49 5732.49 7.16							5725				
			STA 24+ 18"X8" N AND 8" BY WWSE TOP=575	-95.24 MJ TEE		+01	AN CE											
			AND 8" By wwsi	GV		A 3(0)	M E A R S S S S S S S S S S S S S S S S S S											
5735			TOP=573	<u> </u>		CR.T.	101 101 101 101 101 101 101 101	WTM=5729.0 30+75.16 555.42" STM 5723.0	1 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1			· · · · · · · · ·		5720	· · · ·		· · · ·	
	STA 24+46	.24, 10.00'RT						$ \begin{bmatrix} \mathbb{N} \\ \mathbb{N}$										
	8" MJ PLUG W/	1" TEMP BOV TOP=5741.59					STA 30+6 45°8° DII VERTICAL TOP WTM- STA 30+6 45°8° DII		EAR/ A 3C 8" A 3C									
5730							455 455 455 705 705 705 705 705 705 705 705 705 7	STA CR05 DD	CLEARANCE=1.7' STA 30+83.16 45°8″ DIP MJ VERTICAL BEND TOP WTM=5729.1 STA 30+87.51	45 19 19 19 19 19 19 19 19 19 19 19 19 19						· · · ·		
	23+00	24+00	25+00	-	29+00	30+00	31+	00				0+00	1+0	0	0+	00	1+	- () (
L																		

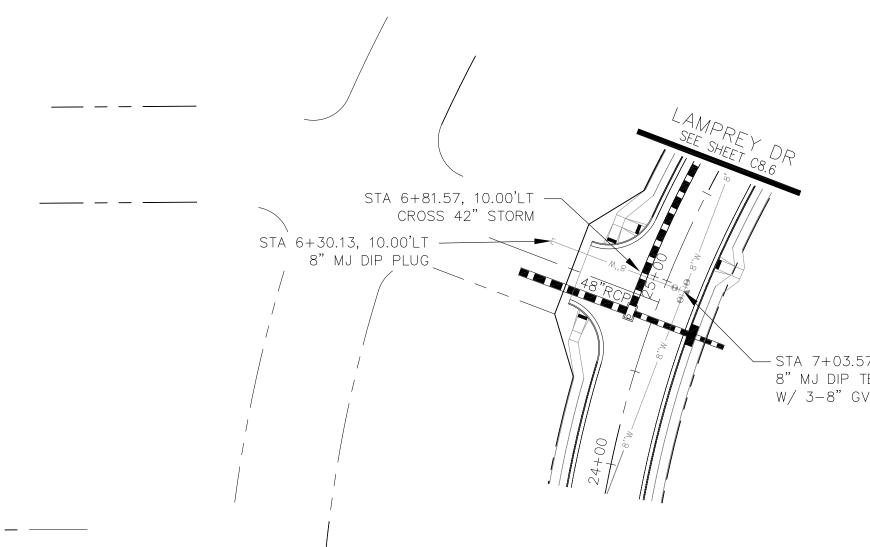
SHAVERS DRIVE



		AMP CAMP CAMP	SHAVERS DRIV	CHOOL DRIVE	CORERING GROUP ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
					on date date date date date date date date
					NO. DESCRIPTI PROJECT: FONTAINE BLVD IN LORSON RANCH EAST FONTAINE BLVDOLD GLORY DR COLORADO SPRINGS, COLORADO
		50	25 0	50 100	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
			SCALE: 1'	'=50' 1"=50' 1"=5'	
		· · · · · · · · ·	VERT.	<u>1"=5'</u> 5770	AN/PROFII ERS DR,
		. .			LAN/PRO
				5765	
				5760	
					RM SEV TLE DR YAMHIL
		· · · · · · · ·		5755	AST /
				5750	EET & STOF ROCKCAST
				5745	STREE RO
					\ \ \ \
				5740	-
				5735	
			5730	_{Date:} DECEMBER 20, 2017
	· · · ·				project no. 100.041
0+00		1+00	2+00	5725	SHEET NUMBER C8.8 TOTAL SHEETS: 34

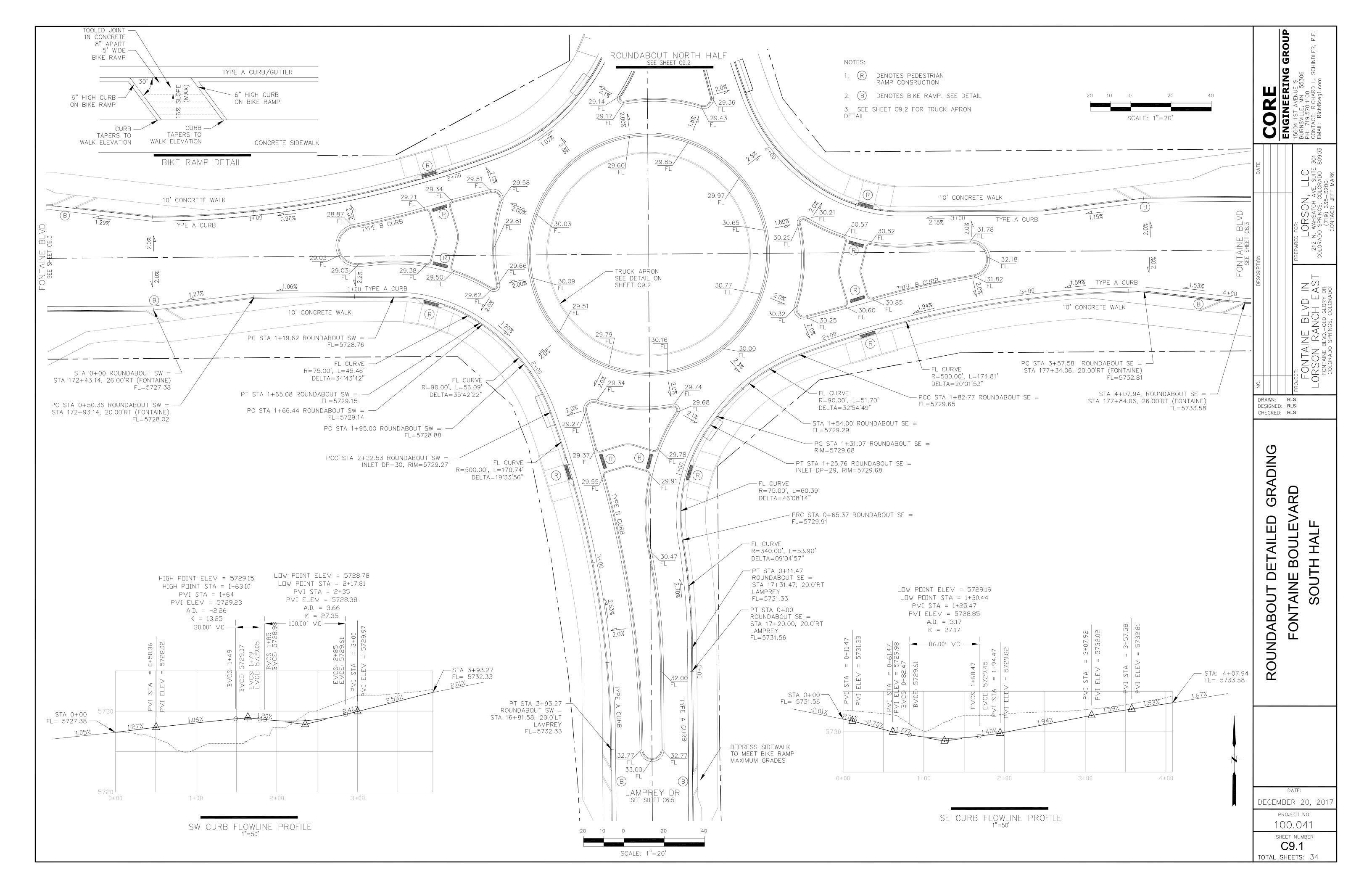
1. STATIONING IS AT ROADWAY CENTERLINE. 2. WATERMAIN ELEVATIONS ARE TO TOP UNLESS NOTED OTHERWISE 3. ALL 8" & 12" WATERMAIN IS PVC, C900. ALL 24" WATERMAIN IS DIP 4. ALL WATERMAIN FITTINGS ARE MJ DIP 5. ALL WATERMAIN BENDS & TEES REQUIRE CTRB'S

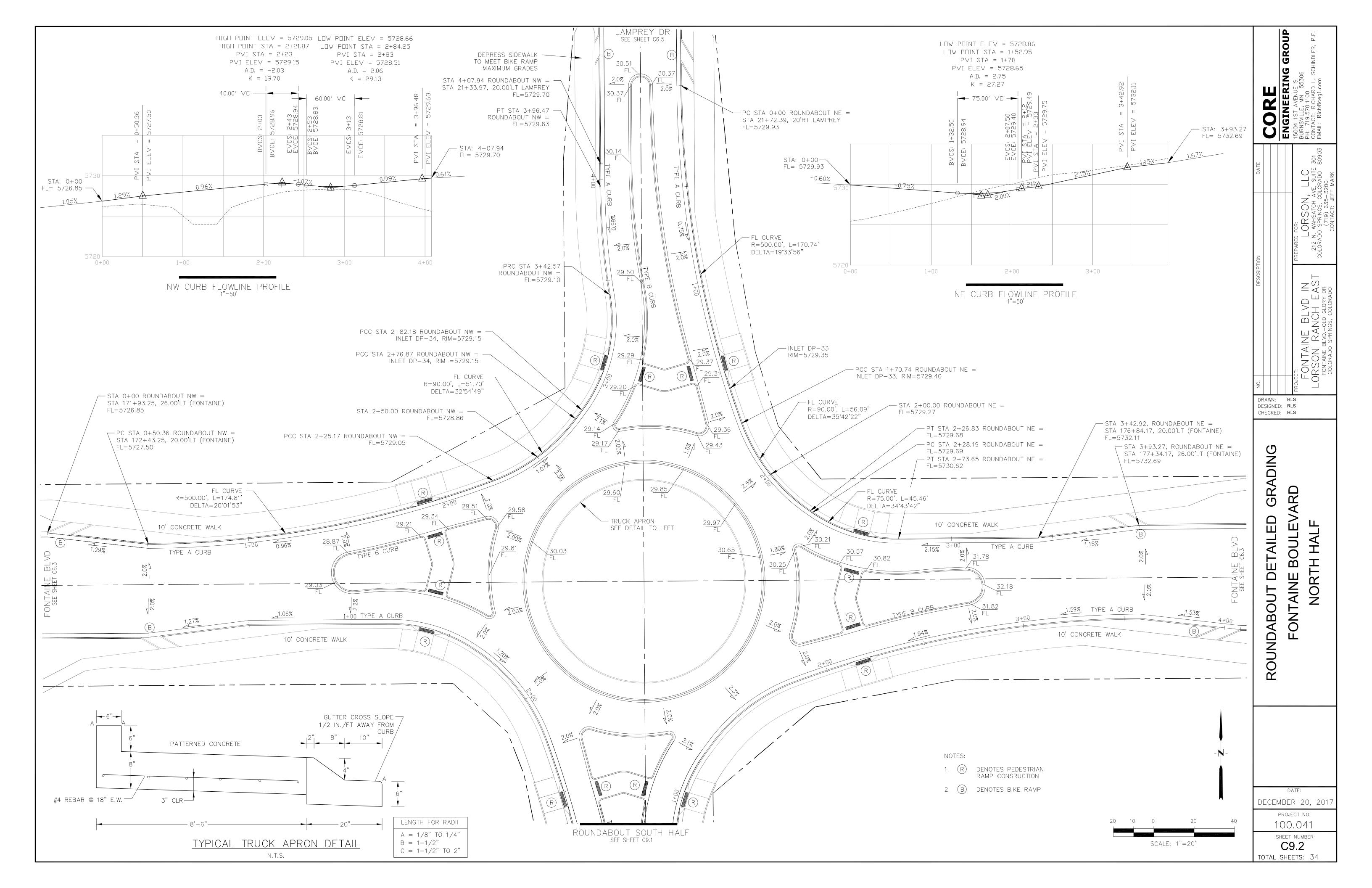
		0+	- 0 0	1	+00	2+0	00	3	+00	0 -	+00	1 -	-00	2	2+00	3+	00	4-	+00	5+	00	6+00
5700	 												· · · ·									
5705													· · · · ·									
J/IU	 																					
5710																						=5/20.46
	 																				TOP=5	721.79 × 6+73.57 — =5720.46
5715							 													8" MJ	STA 6+30.13, 10 PLUG W/ 1" TEMF TOP=5	0.00'LT
5720	 																					
5725	 												· · · ·								STA 6+ END PAV	+49.93 — EMENT
																		0,8 8			10	1,42%
5730													· · · · ·			<pre></pre>	AT	C/L · · · ·				· · · ·
																	EXIS	TING RADE C/L		STA STA		
5735		· · · · ·															SED GRADE RLY GRADING	PLANS		STA 5+28.49 = STA 6+30.13 (MUMFORD)		
6736														· · · · ·	· · · · · ·					= 64. = 13 (N		
	 																			IOWEO		
5740																				RD)		

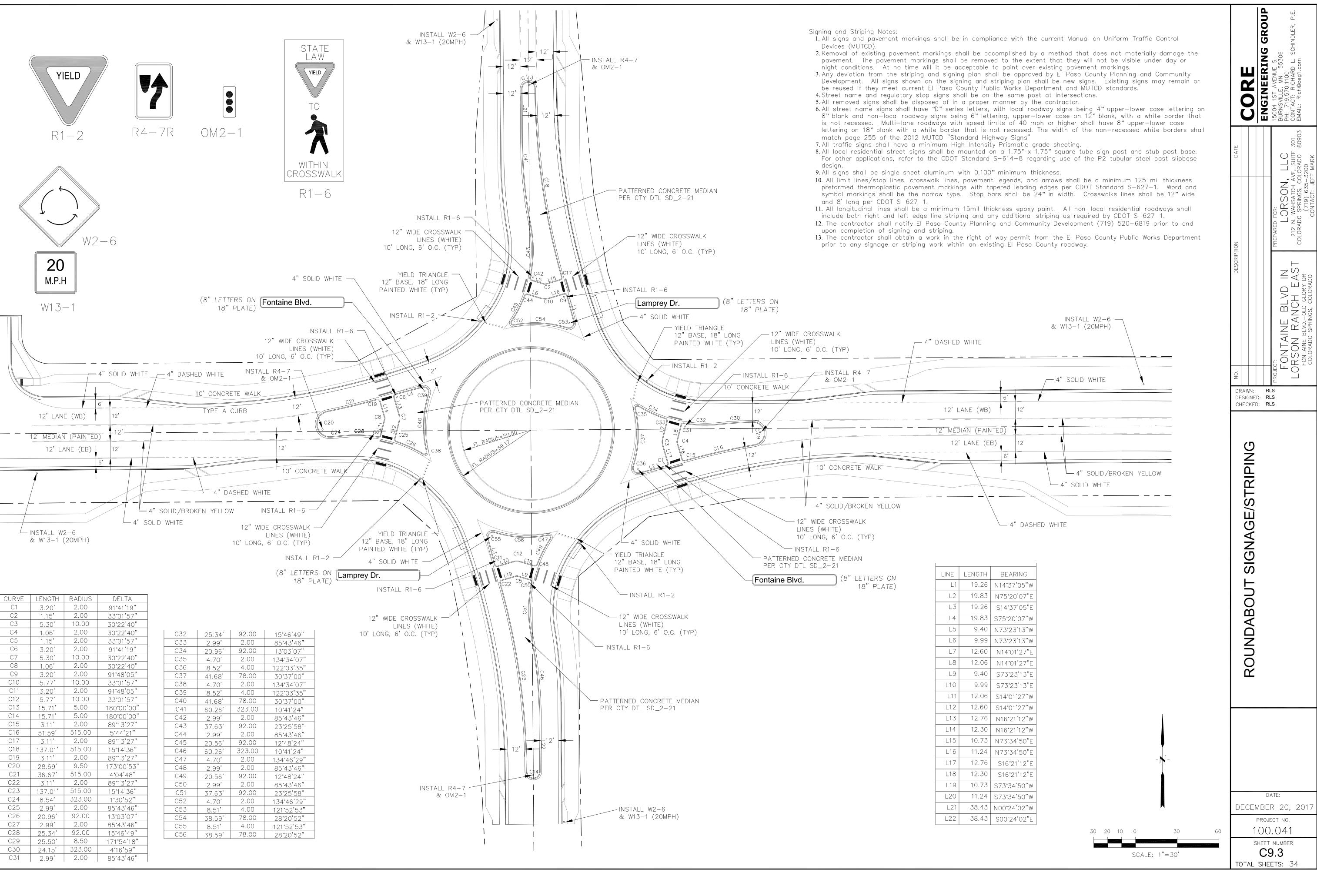


CLARION DRIVE

CLARION DRIVE SCHOOL SITE FONTAINE BLVD KEY MAP	CORRE ENGINEERING GROUP 15004 1ST AVENUE S. BURNSVILLE, MN 55306 PH: 719.570.1100 CONTACT: RICHARD L. SCHINDLER, P.E. EMAIL: Rich@ceg1.com
	N, LLC AVE, SUITE 301 COLORADO 80903 55-3200 JEFF MARK
.57, 10.00'LT	NO. DESCRIPTION PROJECT: PROJECT: FONTAINE BLVD IN FONTAINE BLVD IN LORSON RANCH EAST LORSON RANCH EAST COLORADO SPRINGS, COLORA FONTAINE BLVDOLD GLORY DR COLORADO SPRINGS, COLORA (719) 635-3200 CONTACT: JEFF MA
	DRAWN: RLS DESIGNED: RLS
SCALE: $1"=50'$ SCALES: HORIZ. $1"=50'$ VERT. $1"=5'$	
6+75.49 6+75.49 05 (LAMPREY 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
ELEC STA ELEC STA STA ELEC STA STA STA STA STA STA STA STA	RIVE
STA 7+03.57 8" MJ TEE	CLAR
TOP=5724.07	WATERMAIN PLAN/P
81.57 81	
9 2 2 2 2 2 2 2 2 2 2 2 2 2	
5705	
	DATE:
	project no. 100.041
-00 7+00 8+00	SHEET NUMBER C8.9 TOTAL SHEETS: 34

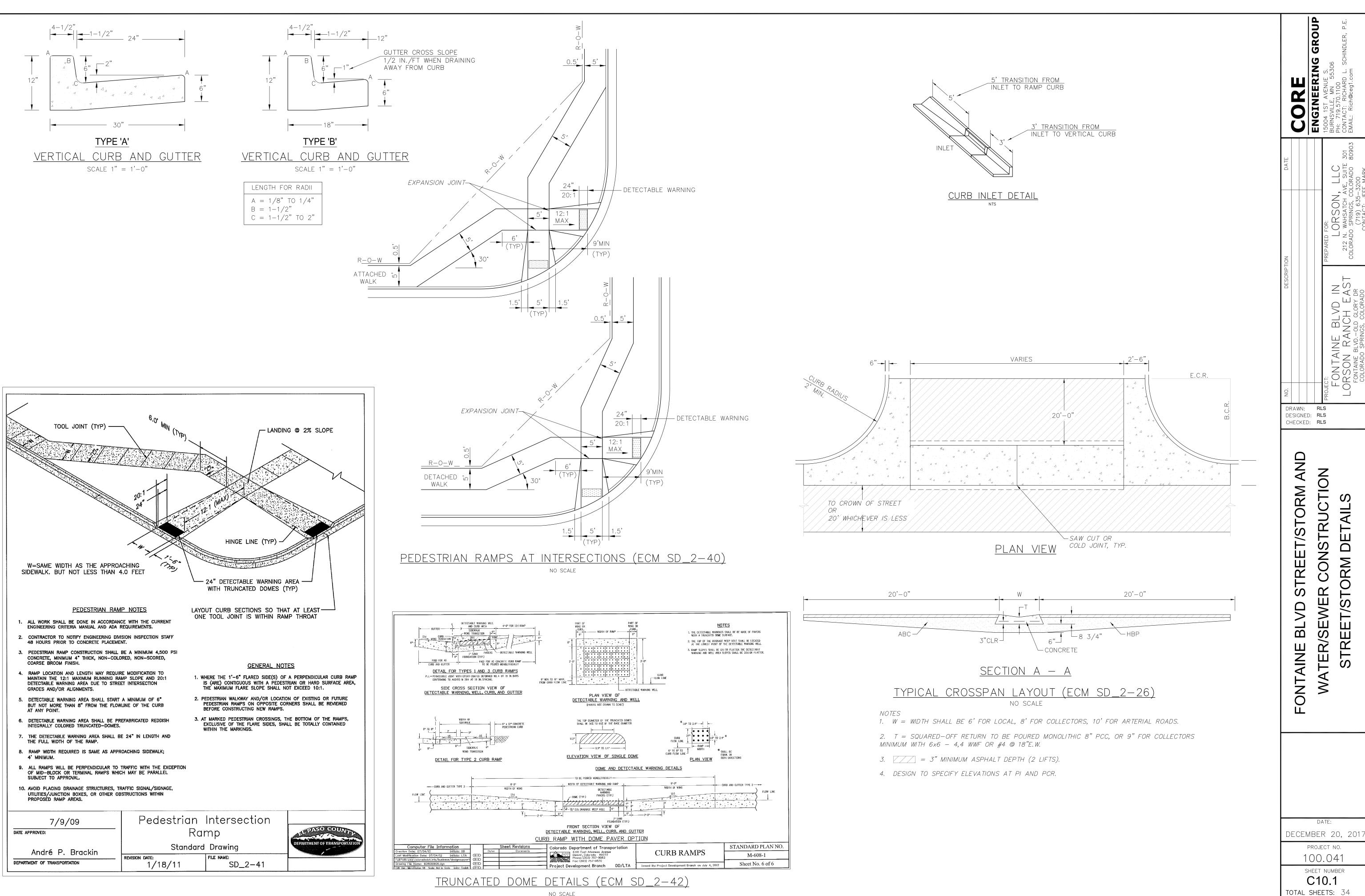


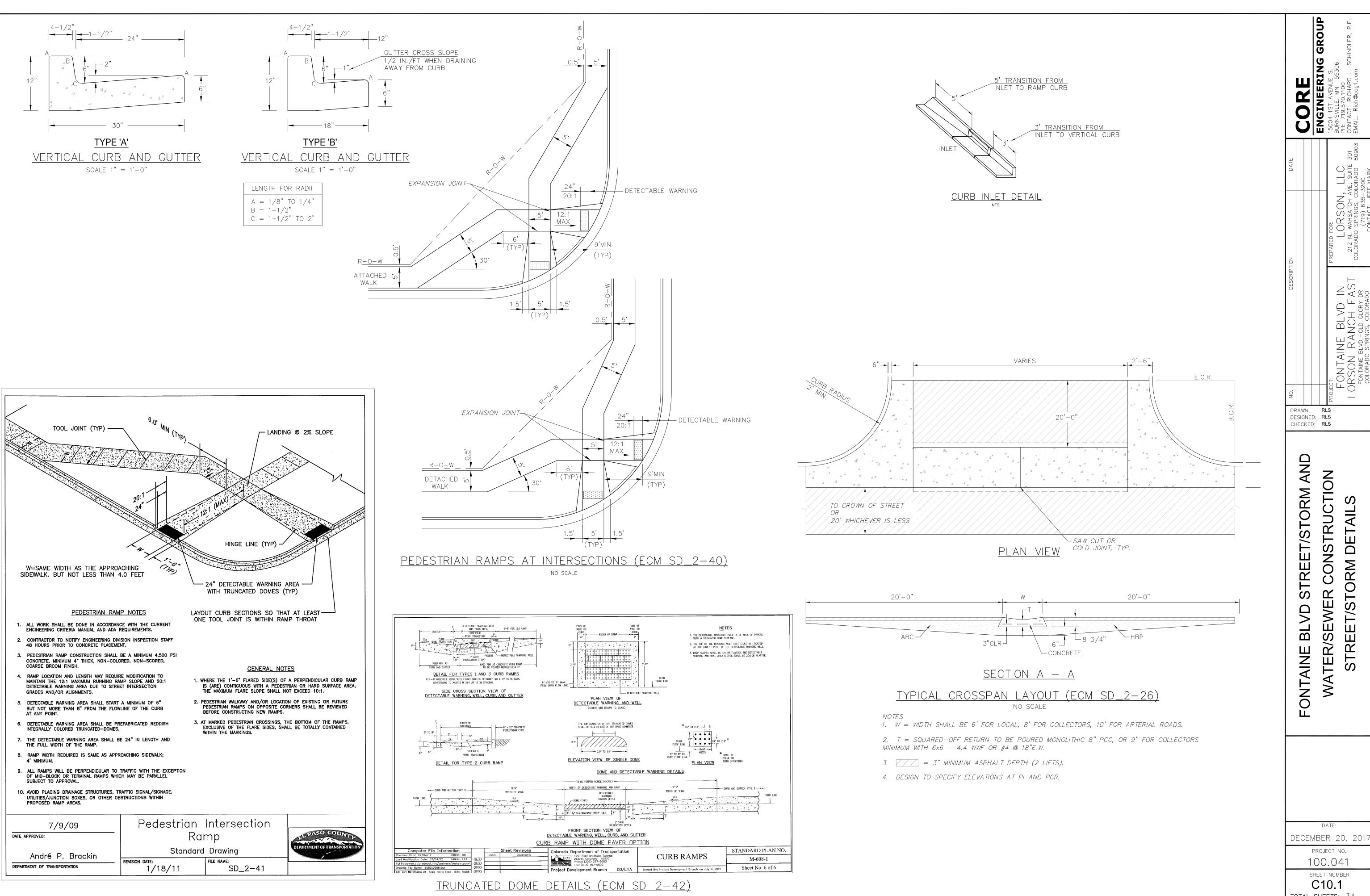


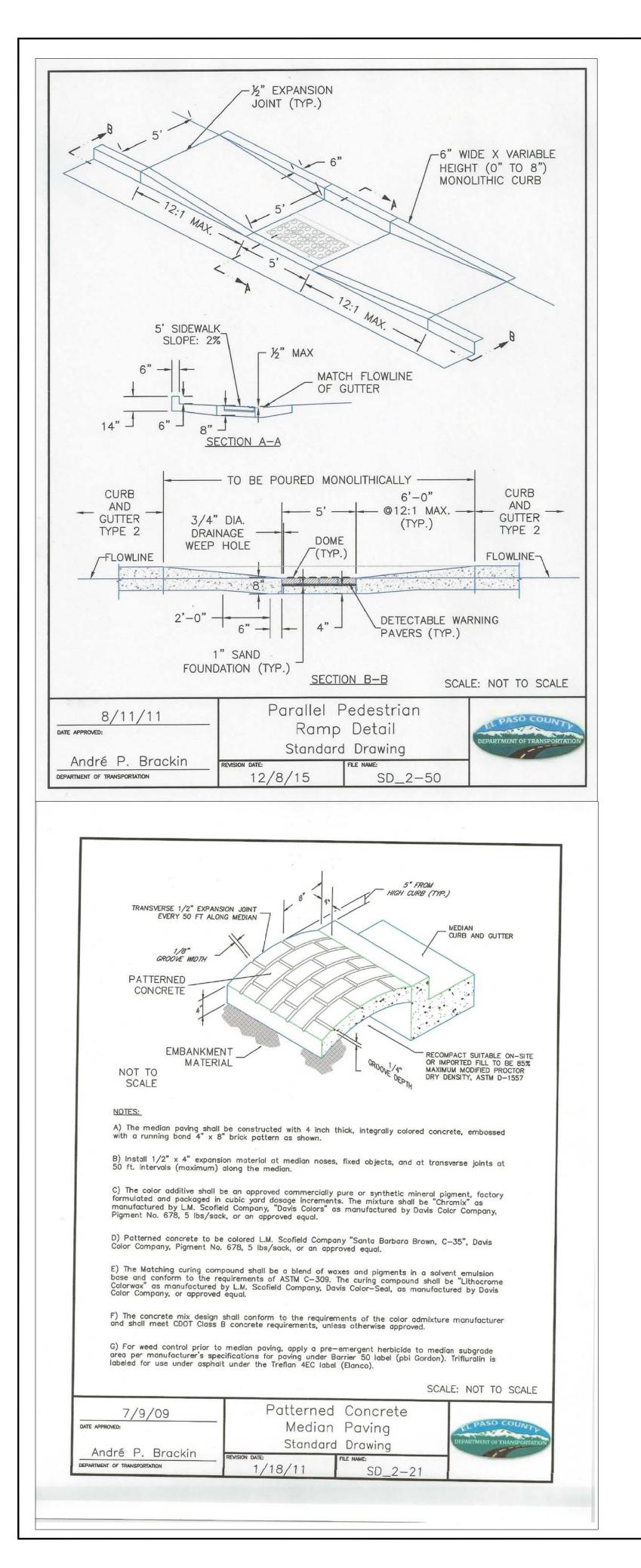


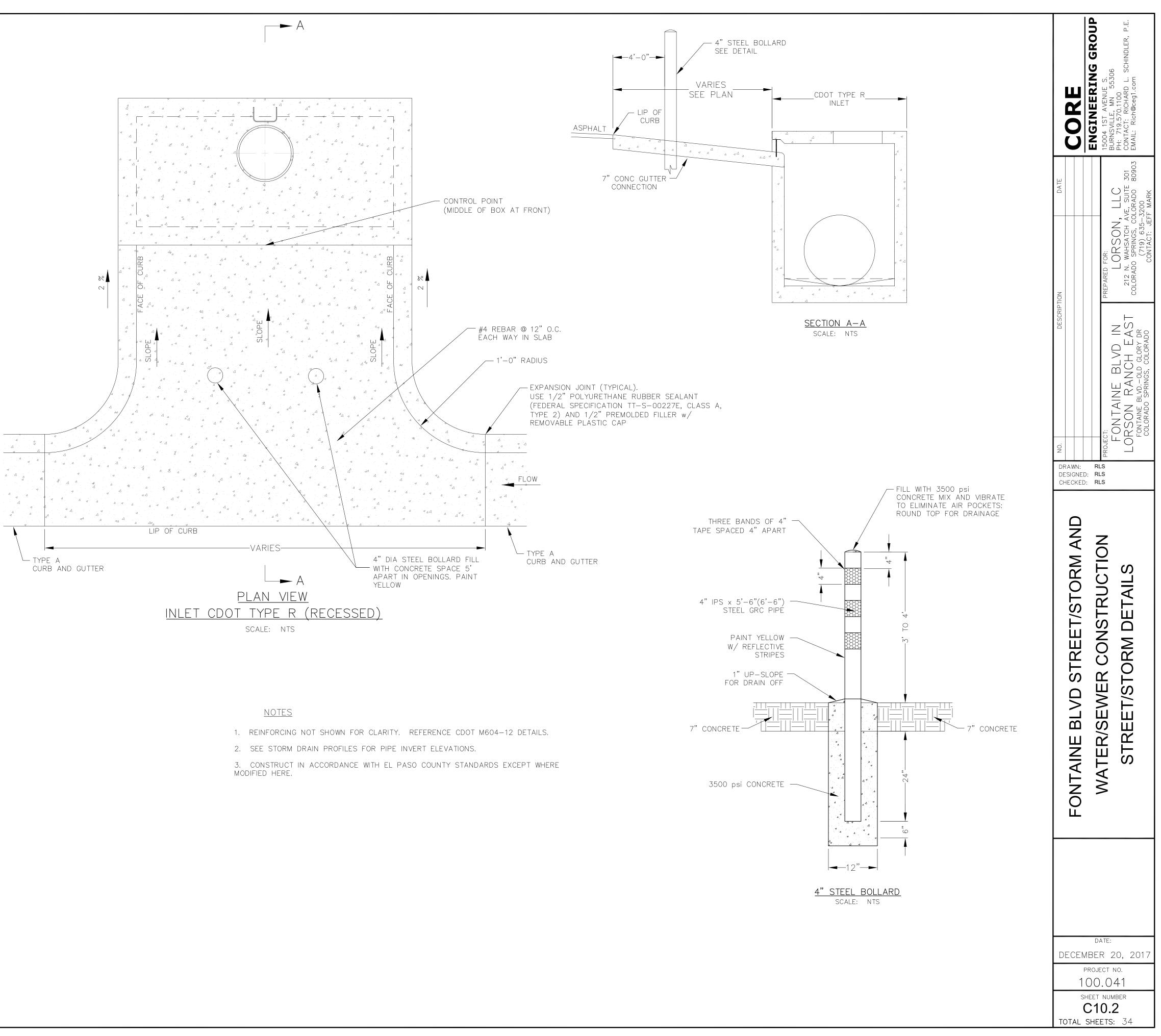
C10 5.77' 10.00 33°01'57" C11 3.20' 2.00 91°48'05" C12 5.77' 10.00 33°01'57" C13 15.71' 5.00 180°00'00" C14 15.71' 5.00 180°00'00" C15 3.11' 2.00 89°13'27" C16 51.59' 515.00 5°44'21" C17 3.11' 2.00 89'13'27" C18 137.01' 515.00 15°14'36" C20 28.69' 9.50 173°00'53" C21 36.67' 515.00 4°04'48" C22 3.11' 2.00 89°13'27" C23 137.01' 515.00 15°14'36" C24 8.54' 323.00 1°30'52" C25 2.99' 2.00 85°43'46" C26 20.96' 92.00 13°03'07" C27 2.99' 2.00 85°43'46" C28 25.34' 92.00 15°46'49" C29 25.50' 8.50 171°54'18" C30 24.15' 323.00 4°16'59"

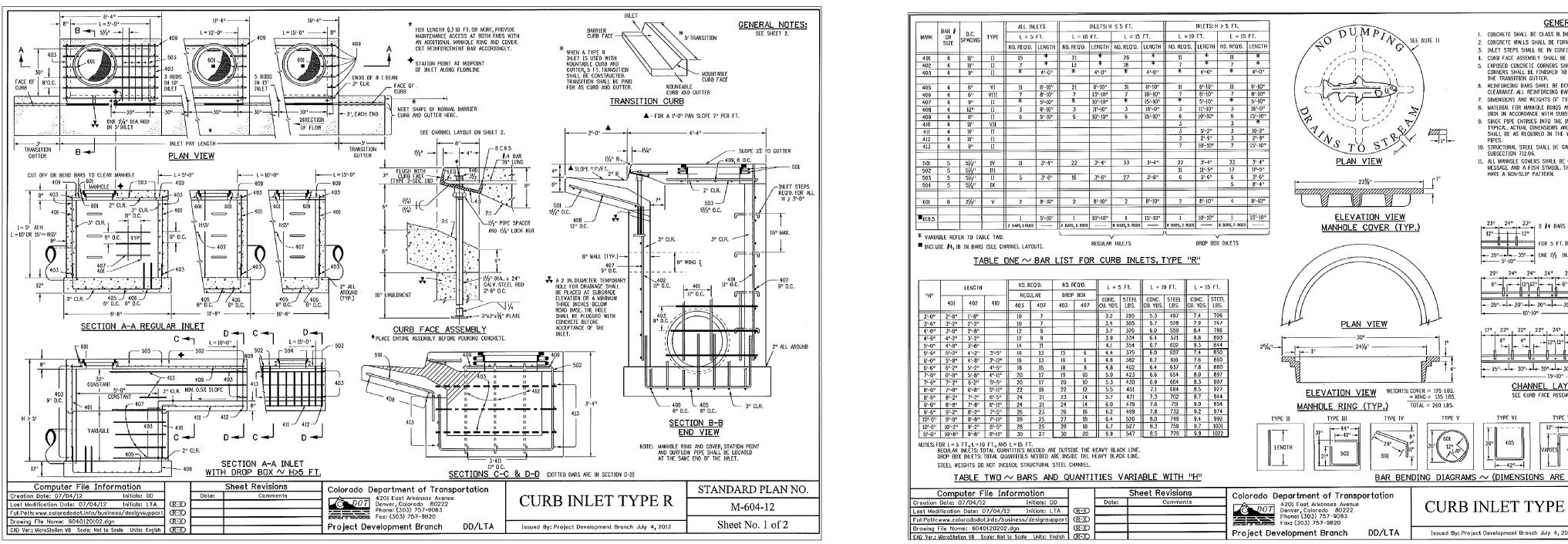
GTH	BEARING
.26	N14°37'05"W
.83	N75°20'07"E
.26	S14°37'05"E
.83	S75°20'07"W
.40	N73°23'13"W
.99	N73°23'13"W
.60	N14°01'27"E
.06	N14°01'27"E
.40	S73°23'13"E
.99	S73°23'13"E
2.06	S14°01'27"W
.60	S14°01'27"W
.76	N16°21'12"W
.30	N16°21'12"W
.73	N73°34'50"E
.24	N73°34'50"E
2.76	S16°21'12"E
.30	S16°21'12"E
.73	S73°34'50"W
.24	S73°34'50"W
.43	N00°24'02"W
.43	S00°24'02"E

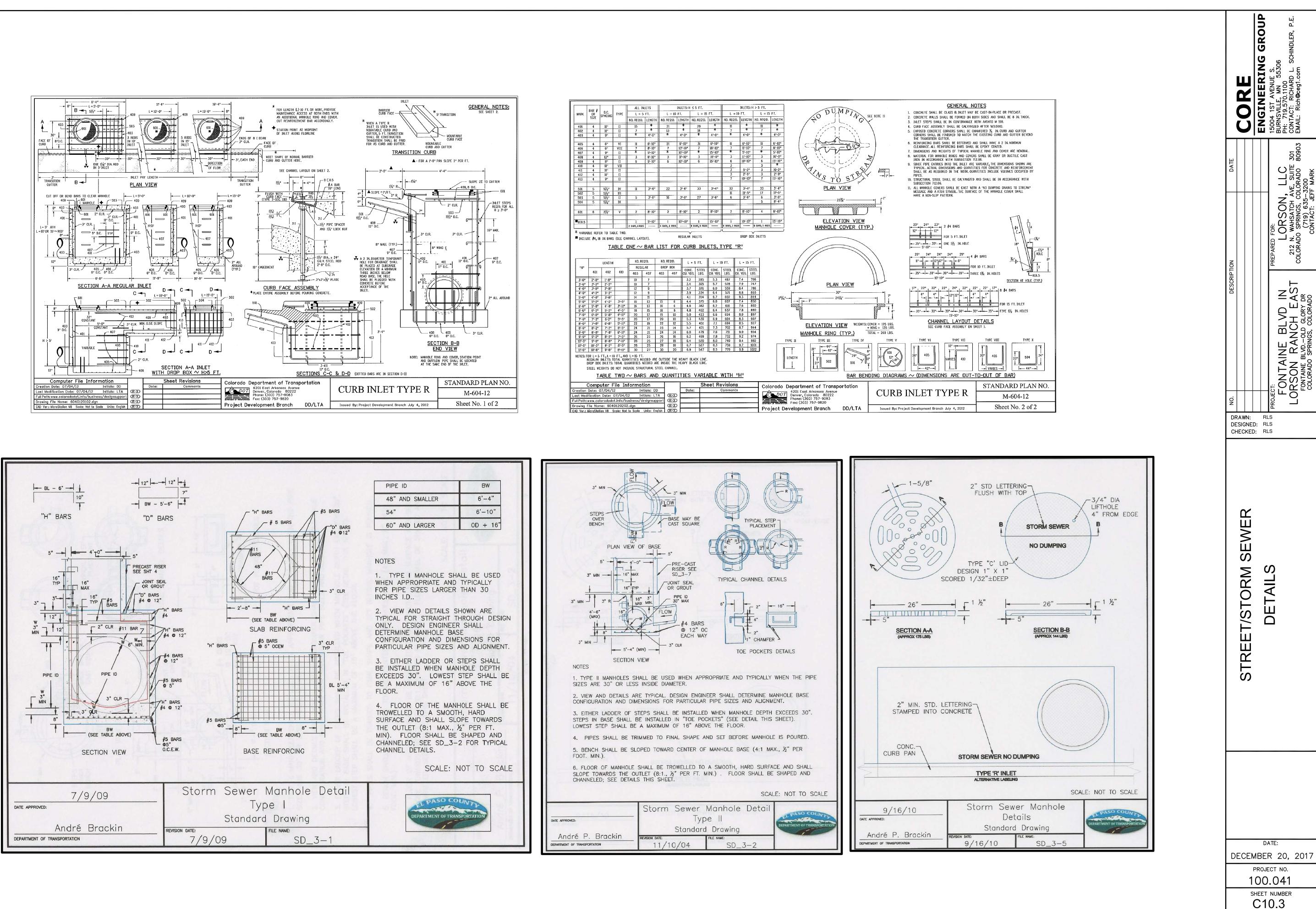












TOTAL SHEETS: 34

GENERAL STRUCTURAL NOTES

I. APPLICABLE CODES:

A. These general notes apply to all structural drawings. This project is designed in accordance with the International Building Code (IBC), 2009 Edition, and the 'Minimum Design Loads for Buildings and Other Structures' (ASCE 7-05) and The Pikes Peak Regional Building Code. (2011 Edition).

B. All material and workmanship shall be in accordance with applicable provisions of the codes specified above.

2. CONCRETE:

A. Concrete has been designed and shall be constructed in accordance with the American Concrete Institute 'Building Code Requirement Reinforced Concrete' and 'Specifications for Structural Concrete for Buildings" (ACI 318 and ACI 301) latest editions. Section 1.3 "Inspection" of ACI 318 is deleted in its entirety, see "Field Observations" paragraph. All concrete shall be of stone aggregate, unless noted otherwise.

- B. <u>Concrete mixes</u>: See specifications for any additional durability requirements.
- Mix 'A' For drilled piers
- 3,000 psi minimum compressive strength at age of 28 days. Type I/II Cement, minimum of 470 pounds per cubic yard.
- Fly ash not allowed.
- 3/4" maximum aggregate size. 6" minimum - 8" maximum slump.

Mix 'B' For footings, grade beams, and miscellaneous concrete:

- 4,000 psi minimum compressive strength at age of 28 days.
- Type I/II Cement, minimum of 564 pounds per cubic yard. 3/4" maximum aggregate size.
- 6%± 11/2% Entrained air.
- 4" (8" with superplasticizer) maximum slump.

C. Reinforcing is to be new billet steel ASTM A615, Grade-60, except ties and bars to be welded shall be Grade-40. Provide not less than (2) #5 around all sides of all openings in concrete and extend 2'-0' past edges of openings. No splices of reinforcement are permitted except as detailed or authorized by structural engineer. Where permitted, use contact lap splices (36) bar diameters minimum. Welded Wire Fabric (WWF,) shall be in accordance with ASTM A185. Lap (1) full mesh minimum at splices. No welding of reinforcement permitted unless detailed.

D. Placing of Reinforcement: Provide chairs, bolsters, additional reinforcement, and accessories necessary to support reinforcement at position shown on drawings. Support of reinforcement on form ties, wood, brick, brickbat or other unacceptable material, will not be permitted.

E. Reinforcement shall be placed so that the following minimum concrete protection is provided, unless noted otherwise:

- 1) Concrete surfaces poured against ground 3' Clear 2) Formed surfaces exposed to ground or weather:
- a) Bars *6 and larger 2" Clear
- **b)** Bars # 5 and smaller 1 1/2" Clear
- 3) Slabsat center (u.n.o.) 4) Concrete not exposed to earth or weather3/4"
- 5) Beams, Columns, Ties, Stirrups or spirals around
- primary reinforcement, or primary reinforcement

F. The contractor is responsible for determining when it is safe to remove forms and/or shoring. Forms and shoring must not be removed until the walls are strong enough to carry their own weight and any anticipated superimposed loads. For foundation walls, this typically requires at least 12 hours of cumulative curing time at a temperature of 50°F or more. Concrete must be adequately covered during cold periods to maintain this surface temperature. Due to varying weather conditions, alternative curing processes, and the use of Type I/II cement, Rocky Mountain Group suggests forms remain in place a minimum of 3 days to assure this performance specification has been met. When forms are stripped there must be no excessive deflection or distortion or discoloration and no evidence of damage to the concrete. Adequate thermal protection of the concrete shall be continued after stripping for a cumulative period of 48 hours at 50°F, or more, after the initial pour. See applicable notes for specifications on when to backfill foundation walls. G. Field quality control:

1) Reference standard: ACI 301 Chapters 16 and 17, latest edition.

2) Slump tests: The general contractor shall provide necessary equipment and shall make test in conformity with ASTM CI43. The contractor shall make slump tests on the first truck of each pour and as often as deemed necessary by the contractor to maintain the required slump tests when directed by the Architect or Engineer. 3) Control tests:

a. Control tests of concrete work shall be made on every 50 cubic yards or fraction thereof of concrete placed and, in any case, minimum of once during each day's pour.

b. Each test shall consist of four standard 6' test cylinders cast and cured in accordance with ASTM C31 and ASTM C172. c. Sample concrete at point of placement.

d. One cylinder shall be broken at end of seven days after placing, two cylinders shall be broken at end of 28 days after placing, and remaining cylinder shall be stored until its disposition is determined by Architect.

e. In general, remaining cylinder will be broken only when previous test reports indicated unsatisfactory results. f. Tests on remaining cylinder shall be at expense of the contractor.

g. Architect and/or Engineer reserves right to stop future concrete work when seven or 28 day tests indicate unsatisfactory results until, in the opinion of the Architect and/or the Engineer of Record, proper corrective measures have been taken to insure quality concrete in future work and corrections deemed necessary have been made. h. Tests shall be made at time control tests are taken and so stated in reports to determine slump, air content, unit weight and temperature of concrete. i. All tests shall be made in accordance with ASTM C138 or ASTM C231.

4) Slab tolerance: Maintain surface flatness with maximum variation of 1/8 inch in 20 feet.

3. DRILLED PIER FOUNDATIONS:

A. The foundation design has been completed in accordance with pertinent standards, recommended design soil parameters, accepted engineering design procedures, and is based on the best information available at the time of completion. The design is intended to minimize differential movement as described in the reference Geotechnical Report. It must be recognized that foundation components will undergo movement. It shall be the responsibility of the contractor and/or present owner to inform any subsequent owners of the soil condition and advised to maintain good practices in the future with regard to surface and subsurface drainage, framing of partitions above floor slabs, and finish work above the floor slabs, etc. B. Foundation design is based on soil report No. 159665 prepared by RMG Engineers dated 11/1/17. The Contractor shall thoroughly review and understand all pertinent construction aspects of this report before beginning any work C. The structure is to be founded on concrete grade beams bearing on drilled piers. Design of drilled piers is based on the following criteria:

Maximum allowable end bearing pressure. . . . 35,000 psf

at least ??'-??" below bottom of grade beam)

D. The maximum variation of the center of any drilled pier at its top from the required location shall not be more than 5% of its diameter, and no pier shall be out of plumb more than 1% of its overall length.

E. A representative of the Geotechnical engineer shall provide full time observation of the drilling operation and reinforcement / concrete placement to verify that the soil type and conditions are consistent with design criteria of the soil report. If the soil properties are found to be different from this criteria, the foundation engineer shall be promptly notified so that the foundation design may be reviewed.

F. The bottom of all piers shall be thoroughly cleaned and dewatered prior to concrete placement.

G. Continuous horizontal bars and corner bars in grade beams supported on piers shall be spliced only where necessary for purposes of handling and bar length. Bar splices shall be placed in accordance with the followings requirements:

<u>Bar Location</u>	Splice Location	<u>Minimum Lap</u>
Bottom bars	. at support	(36) bar diameters
Top bars	at mid-span	(42) bar diameters
Corner top bars	at corner	(42) bar diameters
Other bar's		(36) bar diameters

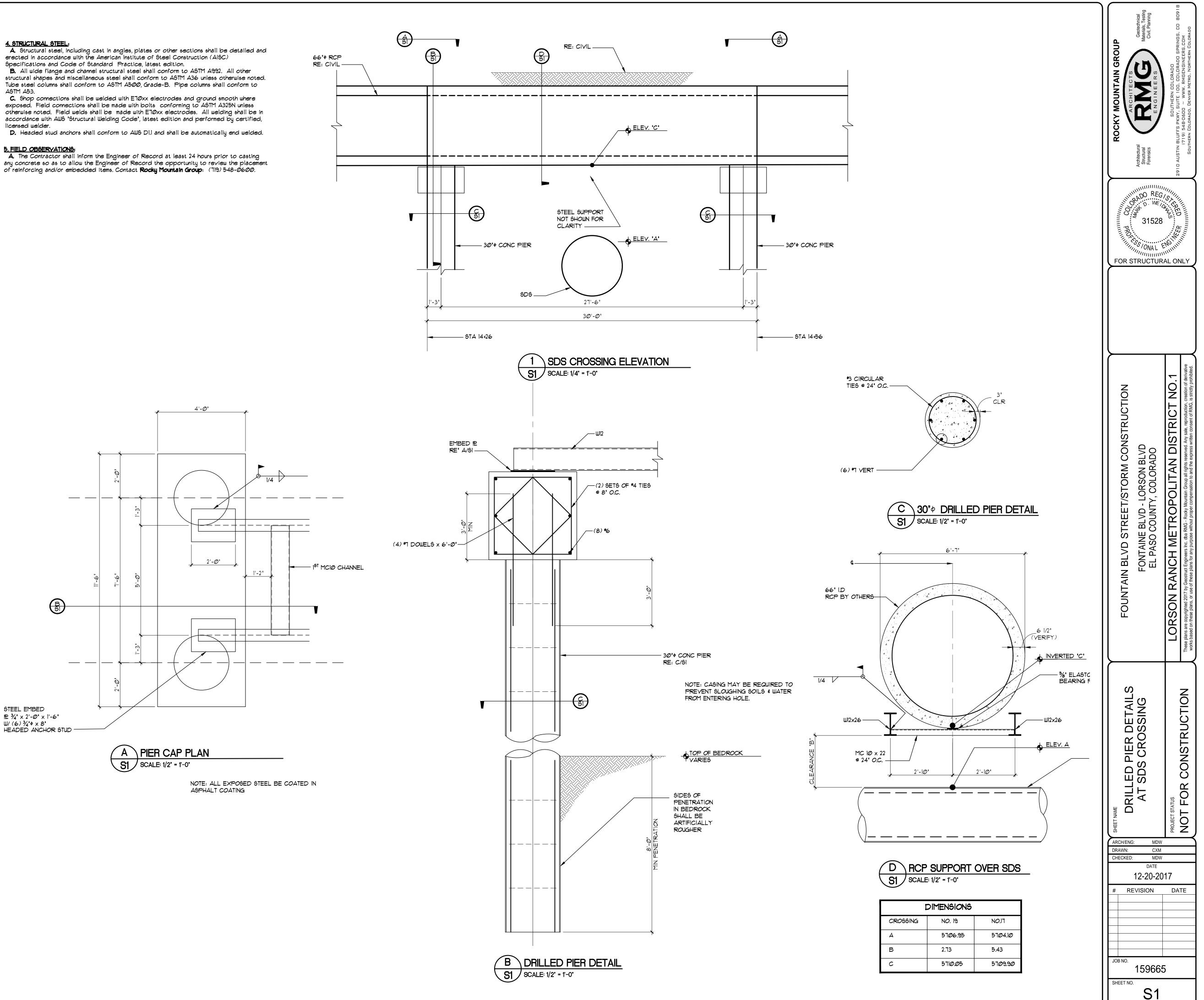
4. STRUCTURAL STEEL:

Specifications and Code of Standard Practice, latest edition.

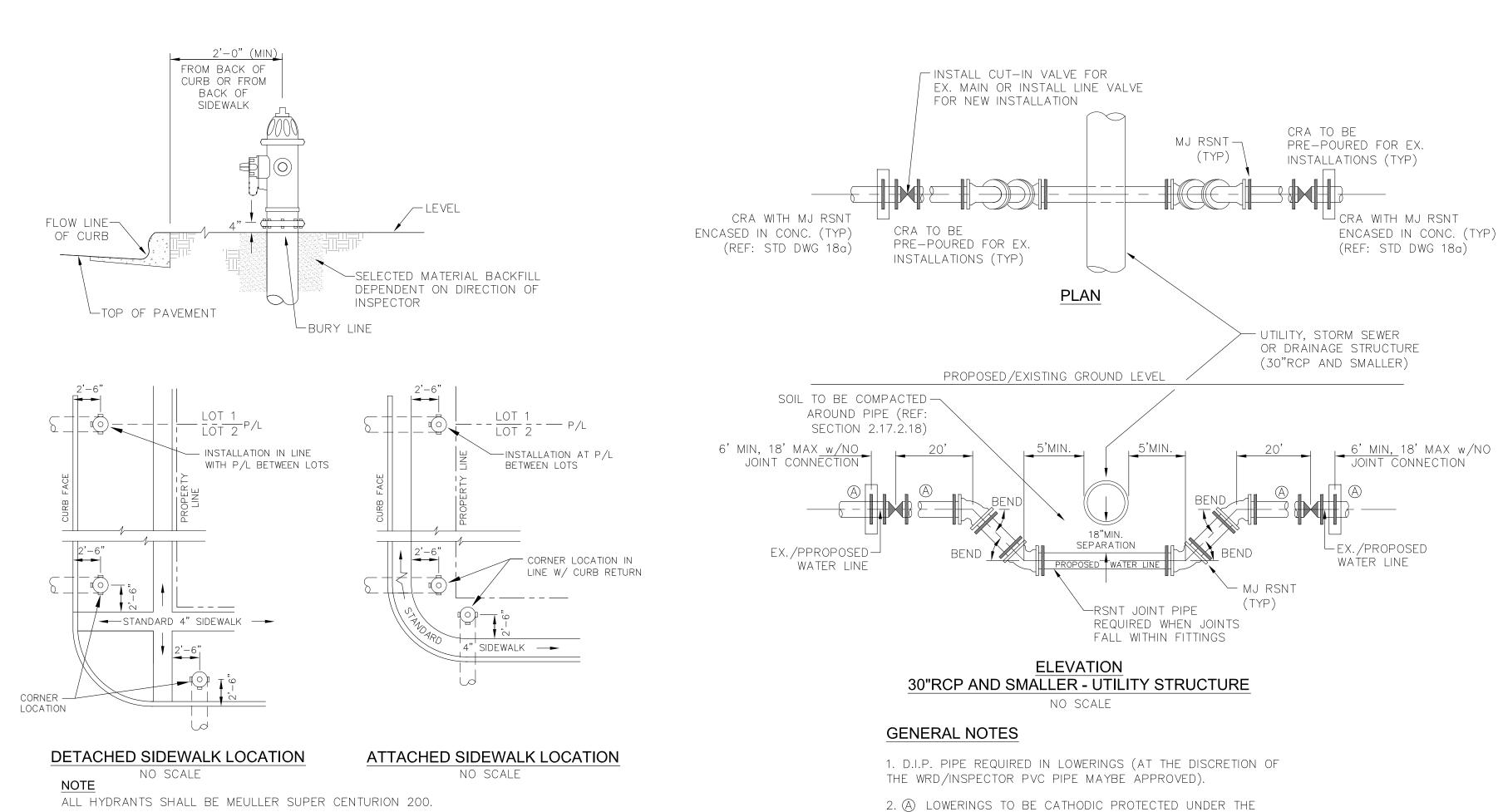
ASTM A53.

licensed welder.

5. FIELD OBSERVATIONS:



33 of 14 SITE SPEC / 2017 / LORSON RANCH METROPOLITAN



1. HYDRANT NOZZLE SHALL BE POSITIONED AT RIGHT ANGLES TO CURB. IF NO CURB OR SIDEWALK EXIST, NOZZLE SHALL BE PLACED AT RIGHT ANGLE TO STREET OR ALLEY.

2. HYDRANTS WILL BE PLACED A MINIMUM OF 5.0 FEET FROM ANY UTILITY OR DRAINAGE STRUCTURE (TO BE CO-ORDINATED WITH JOINT TRENCH INSTALLATION)

3. ANY HYDRANT BEING INSTALLED WITH CONDITIONS OTHER THAN THOSE MENTIONED AND/OR DETAILED BELOW WILL REQUIRE SIGNED APPROVAL FROM SECURITY FIRE PROTECTION DISTRICT.

FIRE HYDRANT LOCATIONS

NO SCALE

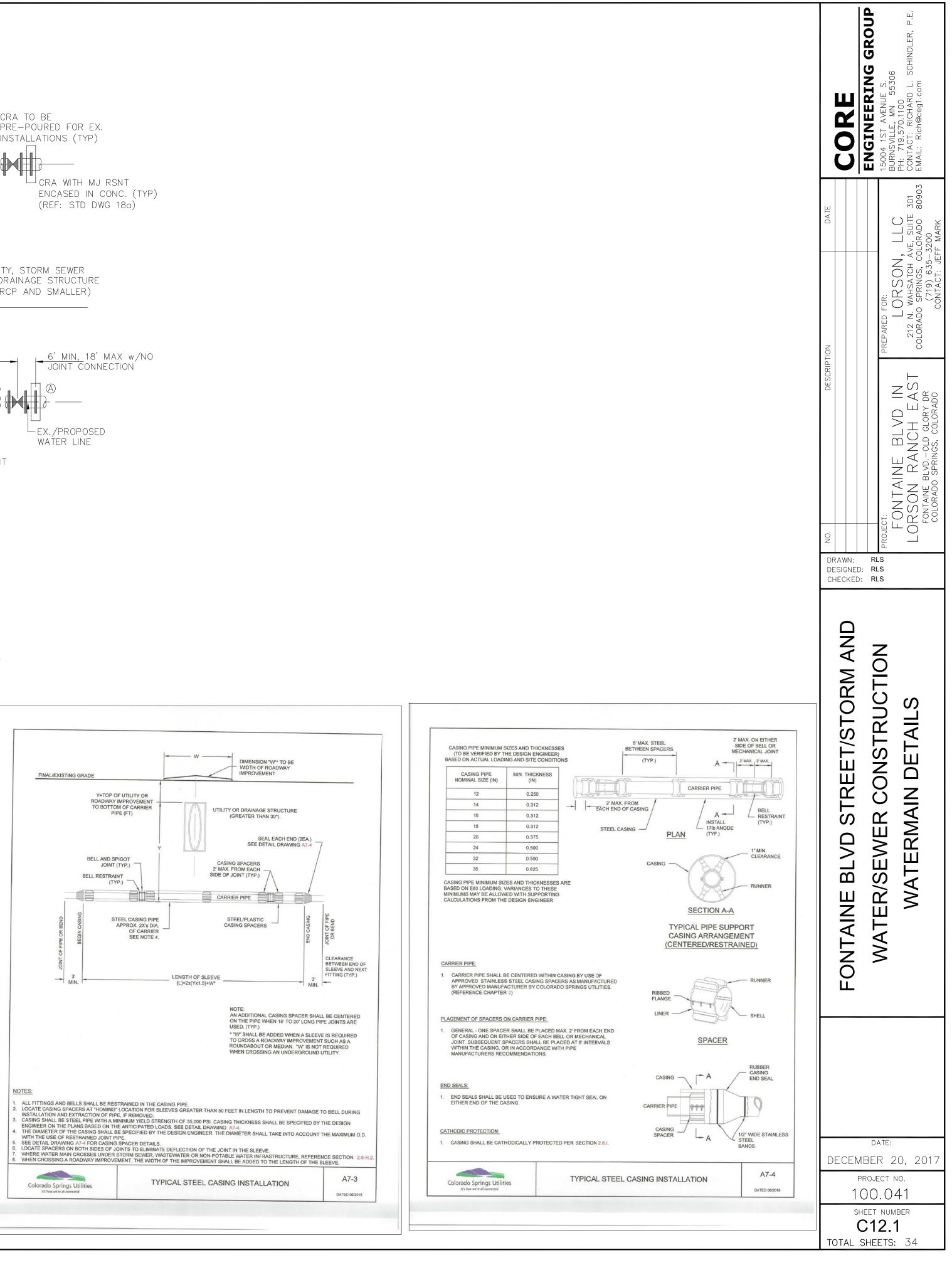
DIRECTION OF THE WRD INSPECTOR. (17 Ib).

3. ALL FITTINGS SHALL HAVE MJ RSNTS. SEE DRAWINGS NO. 27, 28 & SECTION 5.14 OF THE WRD SPECIFICATIONS.

4. EXAMPLE CAN VARY DUE TO SITE CONDITIONS AND INSPECTOR'S DIRECTION'S.



NO SCALE



2 (1) Subject: Cloud+ Full-depth asphalt is not allowed. Page Label: 2 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/2/2018 3:42:53 PM Color: 3 (5) Subject: Delete VLJ (FI) Delete Page Label: 3 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 11:58:30 AM Color: Subject: Cloud+ Design speed is 50 MPH. Page Label: 3 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 11:57:38 AM Color: Subject: Snapshot This is just for review reference. Page Label: 3 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 12:01:05 PM Color: Subject: Rectangle Page Label: 3 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 12:01:45 PM Color: Subject: Rectangle Page Label: 3 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 12:01:55 PM Color:

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Add median between opposing traffic.

A WB split to 2 through lanes is needed to avoid 4 decisions to the west. Verify tapers and dimensions.



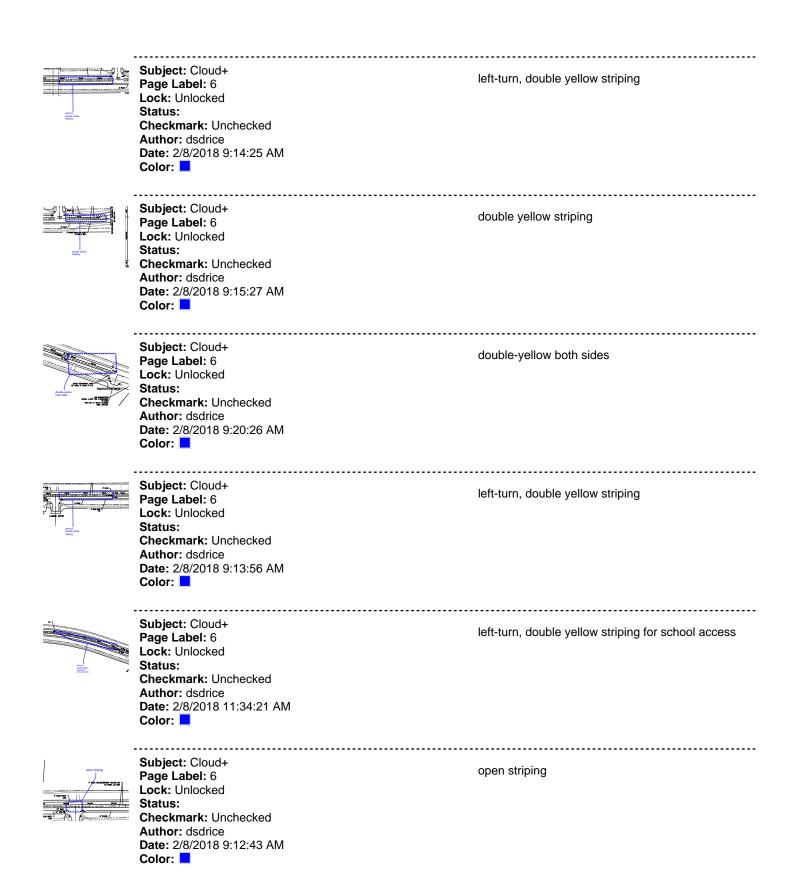
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6 (9)



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double yellow striping



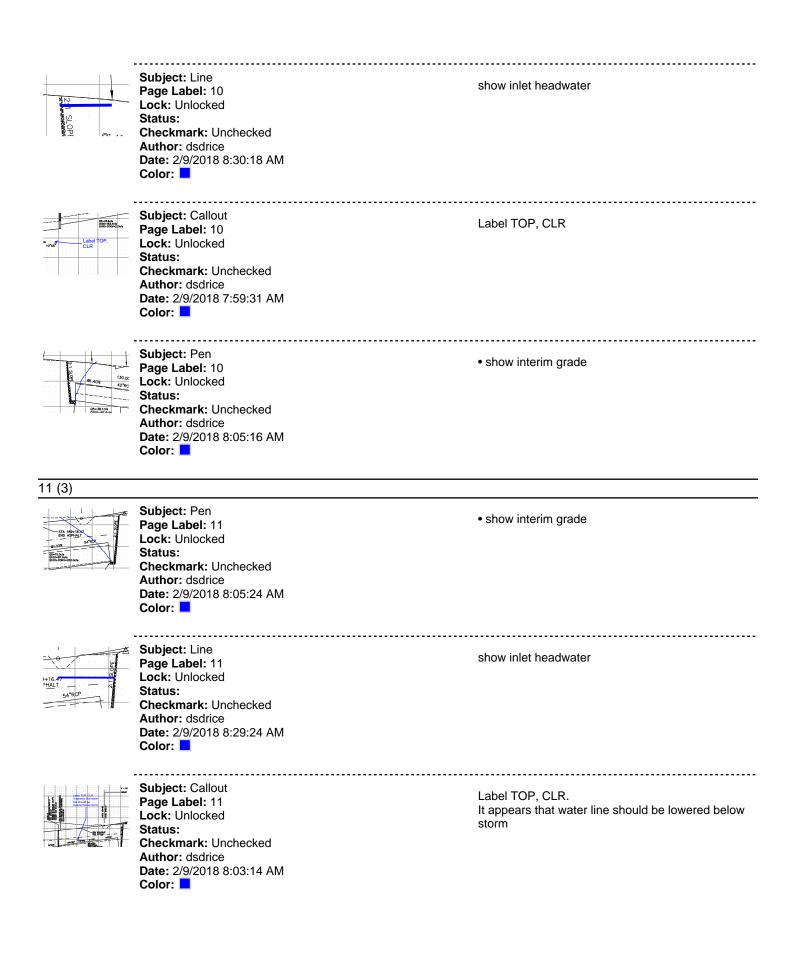
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Provide centerline station labels	Subject: Text Box Page Label: 7 Lock: Unlocked Status: Checkmark: Unchecked Author: dsdrice Date: 2/8/2018 12:48:25 PM Color:	Provide centerline station labels
8 (6)		
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Author: dsdrice

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Date: 2/9/2018 7:59:18 AM



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4. STRUCTURAL STEEL: : A. Structural steel, including cast in angles, plates or other sections shall be detailed and A. Structural steel, including cast in angles, plates or other sections shall be detailed and Structural steel, including cast in angles, plates or other sections shall be detailed and erected in accordance with the American Institute of Steel Construction (AISC) Specifications and Code of Standard Practice, latest edition. B. All wide flange and channel structural steel shall conform to ASTM A992. All other B. All wide flange and channel structural steel shall conform to ASTM A992. All other All wide flange and channel structural steel shall conform to ASTM A992. All other structural shapes and miscellaneous steel shall conform to ASTM A36 unless otherwise noted. Tube steel columns shall conform to ASTM A500, Grade-B. Pipe columns shall conform to ASTM A53. C. Shop connections shall be welded with E70xx electrodes and ground smooth where C. Shop connections shall be welded with E70xx electrodes and ground smooth where Shop connections shall be welded with E70xx electrodes and ground smooth where exposed. Field connections shall be made with bolts conforming to ASTM A325N unless otherwise noted. Field welds shall be made with E70xx electrodes. All welding shall be in accordance with AWS "Structural Welding Code", latest edition and performed by certified, licensed welder. D. Headed stud anchors shall conform to AWS D1.1 and shall be automatically end welded. D. Headed stud anchors shall conform to AWS D1.1 and shall be automatically end welded. Headed stud anchors shall conform to AWS D1.1 and shall be automatically end welded.

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SIDES OF PENETRATION IN BEDROCK SHALL BE ARTIFICIALLY ROUGHER	Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color:	SIDES OF PENETRATION IN BEDROCK SHALL BE ARTIFICIALLY ROUGHER
S1	Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color:	S1

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MC 1Ø x 22 @ 24 ' O.C. —	Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color:	MC 10 x 22 @ 24" O.C.
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	Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color:	5. FIELD OBSERVATIONS: : A. The Contractor shall inform the Engineer of Record at least 24 hours prior to casting A. The Contractor shall inform the Engineer of Record at least 24 hours prior to casting The Contractor shall inform the Engineer of Record at least 24 hours prior to casting any concrete so as to allow the Engineer of Record the opportunity to review the placement of reinforcing and/or embedded items. Contact Rocky Mountain Group: (719) 548-0600.Rocky Mountain Group: (719) 548-0600.

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2. CONCRETE: : A. Concrete has been designed and shall be constructed in accordance with the American A. Concrete has been designed and shall be constructed in accordance with the American Concrete has been designed and shall be constructed in accordance with the American Concrete Institute "Building Code Requirement Reinforced Concrete" and "Specifications for Structural Concrete for Buildings" (ACI 318 and ACI 301) latest editions. Section 1.3 "Inspection" of ACI 318 is deleted in its entirety, see "Field Observations" paragraph. All concrete shall be of stone aggregate, unless noted otherwise. B. Concrete mixes: B. Concrete mixes: Concrete mixes: Concrete mixes: : See specifications for any additional durability requirements. Mix 'A' For drilled piers Mix 'A' For drilled piers For drilled piers For drilled piers 3,000 psi minimum compressive strength at age of 28 days. Type I/II Cement, minimum of 470 pounds per cubic yard. Fly ash not allowed. 3/4" maximum aggregate size. 6" minimum- 8" maximum slump. Mix 'B' For footings, grade beams, and miscellaneous concrete: Mix 'B' For footings, grade beams, and miscellaneous concrete: Mix 'B' For footings, grade beams, and miscellaneous concrete: For footings, grade beams, and miscellaneous concrete: For footings, grade beams, and miscellaneous concrete: : 4,000 psi minimum compressive strength at age of 28 days. Type I/II Cement, minimum of 564 pounds per cubic yard. 3/4" maximum aggregate size. 6%; 1 % Entrained air. 12% Entrained air. 4" (8" with superplasticizer) maximum slump. C. Reinforcing is to be new billet steel ASTM A615, Grade-60, except ties and bars to C. Reinforcing is to be new billet steel ASTM A615, Grade-60, except ties and bars to Reinforcing is to be new billet steel ASTM A615, Grade-60, except ties and bars to be welded shall be Grade-40. Provide not less than (2) #5 around all sides of all openings in concrete and extend 2'-0" past edges of openings. No splices of reinforcement are permitted except as detailed or authorized by structural engineer. Where permitted, use contact lap splices, (36) bar diameters minimum. Welded Wire Fabric (W.W.F.) shall be in accordance with ASTM A185. Lap (1) full mesh minimum at splices. No welding of reinforcement permitted unless detailed. D. Placing of Reinforcement: Provide chairs, bolsters, additional reinforcement, and D. Placing of Reinforcement: Provide chairs, bolsters, additional reinforcement, and Placing of Reinforcement: Provide chairs, bolsters, additional reinforcement, and accessories necessary to support reinforcement at position shown on drawings. Support of reinforcement on form ties, wood, brick, brickbat or other unacceptable material, will not be permitted. E. Reinforcement shall be placed so that the following minimum concrete protection is E. Reinforcement shall be placed so that the following minimum concrete protection is Reinforcement shall be placed so that the following minimum concrete protection is provided, unless 1) Concrete surfaces noted otherwise: poured against ground 3" Clear 1) Concrete surfaces poured against ground 3" Clear Concrete surfaces poured against ground3" Clear 2) Formed surfaces exposed to 2) Formed surfaces ground or weather: exposed to around or weather: Formed

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And a second sec	Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color:	1. APPLICABLE CODES: A. These general notes apply to all structural drawings. This project is designed in A. These general notes apply to all structural drawings. This project is designed in These general notes apply to all structural drawings. This project is designed in accordance with the International Building Code (IBC), 2009 Edition, and the 'Minimum Design Loads for Buildings and Other Structures" (ASCE 7-05) and The Pikes Peak Regional Building Code. (2011 Edition). B. All material and workmanship shall be in accordance with applicable provisions of the B. All material and workmanship shall be in accordance with applicable provisions of the All material and workmanship shall be in accordance with applicable provisions of the codes specified above.
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Subject: Page Label: 33 Lock: Unlocked Status: Checkmark: Unchecked Author: AutoCAD SHX Text Date: Color: 3. DRILLED PIER FOUNDATIONS: : A The foundation design has been completed in accordance with pertinent standards. A. The foundation design has been completed in accordance with pertinent standards, The foundation design has been completed in accordance with pertinent standards, recommended design soil parameters, accepted engineering design procedures, and is based on the best information available at the time of completion. The design is intended to minimize differential movement as described in the reference Geotechnical Report. It . It must be recognized that foundation components will undergo movement. It shall be the responsibility of the contractor and/or present owner to inform any subsequent owners of the soil condition and advised to maintain good practices in the future with regard to surface and subsurface drainage, framing of partitions above floor slabs, and finish work above the floor slabs, etc. B. Foundation design is based on soil report No. 159665 prepared by RMG B. Foundation design is based on soil report No. 159665 prepared by RMG Foundation design is based on soil report No. 159665 prepared by RMG 159665 prepared by RMG prepared by RMG RMG Engineers dated 11/7/17. The Contractor shall thoroughly review and understand dated 11/7/17. The Contractor shall thoroughly review and understand 11/7/17. The Contractor shall thoroughly review and understand . The Contractor shall thoroughly review and understand all pertinent construction aspects of this report before beginning any work. C. The structure is to be founded on concrete grade beams bearing on C. The structure is to be founded on concrete grade beams bearing on The structure is to be founded on concrete grade beams bearing on drilled piers. Design of drilled piers is based on the following criteria: Maximum allowable end bearing pressure. . . . 35,000 psf 35,000 psf psf Maximum allowable side shear 3,200 psf 3,200 psf psf (For the portion of the pier in bedrock and at least ??'-??" below bottom of grade beam) ??'-??" below bottom of grade beam) below bottom of grade beam) Maximum penetration into bedrock. 8'-0" 8'-0" Minimum total length 40'-0" 40'-0" Minimum spacing 3 PIER DIA. 3 PIER DIA. D. The maximum variation of the center of any drilled pier at its top from the The maximum variation of the center of any drilled pier at its top from the required location shall not be more than 5% of its diameter, and no pier shall be out of plumb more than 1% of its overall length. E. A representative of the Geotechnical engineer shall provide full time E. A representative of the Geotechnical engineer shall provide full time A representative of the Geotechnical engineer shall provide full time observation of the drilling operation and reinforcement / concrete placement to verify that the soil type and conditions are consistent with design criteria of the soil report. If the soil properties are found to be different from this criteria, the foundation engineer shall be promptly notified so that the foundation design may be reviewed. F. The bottom of all piers shall be thoroughly cleaned and dewatered prior F. The

bottom of all piers shall be thoroughly cleaned and

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