PRELIMINARY DRAINAGE REPORT
FOR
STERLING RANCH PHASE 2 PRELIMINARY PLAN

Prepared For:

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September, 2021
Project No. 25188.02
SP-20-003

Prepared By:

JR Engineering, LLC
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J·R ENGINEERING

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

COLORADO LICENSED MIKE BRAMLETT MB 32314 9/1/21 PROFESSIONAL ENGINEER

Mike Bramlett, Colorado P.E. 32314
For and On Behalf of JR Engineering, LLC

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: SR Land, LLC

By:

Title: MANAGER

Address: 20 Boulder Crescent, Suite 200
Colorado Springs, CO 80903

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

APPROVED
Engineering Department
12/16/2021 12:33:39 PM
dsdnijkamp
EPC Planning & Community Development Department

Jennifer Irvine, P.E.
County Engineer/ ECM Administrator

Conditions:

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APPENDIX

PURPOSE

This document is the Preliminary Drainage Report for Sterling Ranch Phase 2. The purpose of this report is to identify on-site and off-site drainage patterns, storm sewer, culvert and inlet locations, areas tributary to the site, and to safely route developed storm water to adequate outfall facilities.

GENERAL SITE DESCRIPTION

GENERAL LOCATION

Sterling Ranch Phase 2 (hereby referred to as the "site") is a proposed development within the Sterling Ranch master planned community with a total area of approximately 75 acres that are presently undeveloped.

The site is located in portions of Section 4, 5 & 33, Township 12 & 13 South, Range 65 West of the Sixth Principal Meridian in El Paso County, State of Colorado. The site is bounded by Un-platted land to the southwest, the Barbarick Subdivision to the north, Sterling Ranch Road cuts through the site, and Sand Creek borders the site to east. The parcels are planned to be platted after approval of the Preliminary Plan. Refer to the vicinity map in Appendix A for additional information.

DESCRIPTION OF PROPERTY

The property will be primarily be single-family residential development (approximately 42 acres), Open space and drainage tracts (approximately 28 acres, and an approximate 5 acre tract in the southwest corner where the Sterling Ranch Lift Station is located. The site is comprised of variable sloping grasslands that generally slope(s) downward to the southeast at 3 to 8% towards the Sand Creek tributary basin.

Soil characteristics are comprised of Type A and B hydrologic Soil groups. Refer to the soil survey map in Appendix A for additional information.

There are no major drainage ways running through the site, although a tributary to the Sand Creek basin is immediately to the east of the site. Currently, Kiowa Engineering Corp. is performing studies and plans to address Sand Creek stabilization.

There are no known irrigation facilities located on the project site.

FLOODPLAIN STATEMENT

Based on the FEMA FIRM Maps number 08041C0533G, dated December 7, 2018, the far eastern portion of the project site that is adjacent to the existing drainage way lies within Zone AE. Zone AE is defined as area subject to inundation by the 1-percent-annual-chance flood event. The majority of

the proposed development lies within Zone X. Zone X is defined as area outside the Special Flood Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. No grading operations are proposed within the Zone AE at this time. FIRM Maps have been presented in Appendix A.

EXISTING DRAINAGE CONDITIONS

MAJOR BASIN DESCRIPTIONS

The site lies within the Sand Creek Drainage Basin based **o**n the "Sand Creek Drainage Basin Planning Study" (DBPS) completed by Kiowa Engineering Corporation in January 1993, revised March 1996. The Sand Creek Drainage Basin covers approximately 54 square miles and is divided into major sub-basins. The site is within the respective sub-basin is shown in Appendix E.

The Sand Creek DBPS assumed the Sterling Ranch Phase 2 property to have a "large lot residential" use for the majority of the site. The Sterling Ranch MDDP assumed a mix of commercial and single family residential lots ranging in size from 0.2 to 0.3 acres for the Sterling Ranch Phase 2 site. The proposed Sterling Ranch master plan is a mix of; school, multi-family, single-family, and commercial land uses, resulting in higher runoff. Any additional runoff will be provided for with the extended detention basin located at the southern edge of the site. The site generally drains from north to south consisting of rolling hills. Currently, the site is used as pasture land for cattle. Sand Creek is located east of the site running north to south. This reach of drainage conveyance is not currently improved. There are a few stock ponds within the creek channel used for cattle watering. Currently, Kiowa is performing studies and plans to address Sand Creek stabilization adjacent to the site.

The proposed drainage on the site closely follows the approved "Master Development Drainage Plan for Sterling Ranch", (MDDP) prepared by M&S Civil Consultants, Inc., dated October 24, 2018. The site is tributary to Pond W5 and full-spectrum detention for the site was previously analyzed and can be found in the Final Drainage Report for Sterling Ranch Filing 2.

EXISTING SUB-BASIN DRAINAGE

The existing / predeveloped condition of the site was broken into two major basins: Basin A (western portion) and Basin B (Eastern Portion), as well as several offsite basins. The basin and sub-basin delineation is shown in the existing drainage map in Appendix E and is described as follows:

Sub-basin A1(Q_5= 1.1cfs, Q_{100}=8.0cfs) is 5.17 acres and 0 percent impervious consists of the eastern portion of Sterling Ranch phase 2. Runoff from this basin drains to the south west into the assumed existing storm sewer built with Filing 2 just east of Marksheffel Road located at design point 1.

Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin A2 (Q_5= 4.6cfs, Q_{100}=33.6cfs) is 27.48 acres and 0 percent impervious and consists the central portion of Sterling Ranch Phase 2. Runoff from this basin drains south onsite into the assumed existing storm sewer built with Filing 2 located at design point 2. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin A3 (Q_5= 2.9cfs, Q_{100}=21.5cfs) is 11.68 acres and 0 percent impervious and is located onsite in the northern part of Sterling Ranch Phase 2. Runoff from this basin drains to the assumed existing storm sewer built with Filing 2 just north of Sterling Ranch Road located at design point 5. Design Point 5.1 is a confluence of flows from basins A3, OS6 and OS7. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin B1 (Q_5= 2.6cfs, Q_{100}=19.0cfs) is 11.78 and is 0 percent impervious and is located on the eastern portion of the site portion of the site. Runoff from this basin drains to the southeast into Sand Creek at design point 6.

Sub-basin OS1(Q_5= 13.4cfs, Q_{100}=29.8cfs) is 9.27 acres is 30.7 percent impervious and is located to the east of the site. Runoff from this basin drains into the Sterling Ranch Filing 2 detention Pond in confluence with upstream flows from the eastern portion of Sub-basin A2. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin OS2 (Q_5= 6.3cfs, Q_{100}=11.2cfs) is 5.00 acres and 100 percent impervious and is comprised of the southern half street of Sterling Ranch Road. Runoff from this basin drains into the assumed existing storm sewer built with Filing 2 located at design point 7. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin OS3 (Q_5= 8.1cfs, Q_{100}=14.6cfs) is 2.36 acres and 100 percent impervious and is comprised of the northern half street of Sterling Ranch Road. Runoff from this basin drains into the assumed existing storm sewer built with Filing 2 located at design point 8. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin OS4 (Q_5= 2.8cfs, Q_{100}=16.9cfs) is 11.71 acres and 3.6 percent impervious and is located immediately north of Sterling Ranch Road and the eastern portion of the site. Runoff from this basin drains south into assumed existing storm sewer built with Filing 2 located at design point 9. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Sub-basin OS5 (Q_5= 0.7cfs, Q_{100}=5.0cfs) is 3.46 acres and 0 percent impervious and is located to the east of the northern portion of the site. Runoff from this basin drains to a low point just north of

Sterling Ranch Road located at Design Point 4 and will be collected in the assumed existing storm sewer built with Filing 2 and piped to the Filing 2 detention pond located south of the site and outfalls to Sand Creek.

Sub-basin OS6 (Q_5= 35.4cfs, Q_{100}=72.2cfs) is 18.38 acres and 11.3 percent impervious as is located northwest of the site in the Barbarick subdivision. Historic runoff from this basins drains south onto the site at design point 10. Detained flow from this basin will be piped through the site to the detention pond and will outfall to Sand Creek.

Sub-basin OS7(Q_5= 20.6cfs, Q_{100}=60.4cfs) is 33.07 Acres and 19.1 percent impervious and is located directly north of the site in the Barbarick subdivision. Historic runoff from this site drains south onto the site at design point 11. Detained flow from this basin will be piped through the site to the detention pond and will outfall to Sand Creek.

PROPOSED DRAINAGE CONDITIONS

PROPOSED SUB-BASIN DRAINAGE

The proposed site was broken into three major basins: Basin A (lower-portion), Basin B (mid and eastern –portion), and Basin C (upper-portion) of the site. The proposed basin (and sub-basin) delineation is shown on the drainage basin map within Appendix E and is described as follows.

Basin A1 (Q_5= 8.1cfs, Q_{100}=17.4cfs) is 4.31 acres and 63 percent impervious and is comprised of single-family residential lots and a local road. Runoff from this basin drains to design point 15, a 15' type R on grade inlet at the southwest corner of the basin.

Basin A2 (Q_5= 1.4cfs, Q_{100}=4.0cfs) is 1.41 acres and 31 percent impervious is comprised of single-family residential lots, open space, several trails, and a local road. Runoff from this basin drains to design point 17, a 15' type R on grade inlet on the southwest corner of the basin, in confluence with upstream by-pass flows from basin A1.

Basin A3 (Q_5= 6.8cfs, Q_{100}=14.6cfs) is 3.68 acres and 65 percent impervious is comprised of single-family residential lots and a local road. Runoff from this basin drains to a 15' on grade type R inlet located at design point 20.

Basin A4 (Q_5= 5.7cfs, Q_{100}=13.4cfs) is 3.94 acres and 52 percent impervious is comprised of single-family residential lots, open space a local road and two urban knuckles. Runoff from this basin drains to a sump 15' type R inlet located at design point 22 in confluence with upstream by-pass flows from basins A1, A2, and A3. The emergency overflow for this basins drains directly to pond W-5 south of the inlet. The runoff from this basin is piped to DP 23 where the runoff confluence with the entire

southern portion of the Sterling Ranch Phase 2 site. From here on, the runoff is then piped into an existing 42" RCP and Structure associated with design point 23.

Basin A5 (Q_5= 1.4cfs, Q_{100}=2.9cfs) is 0.45 acres and 78 percent impervious is comprised of single-family residential lots and a local road. Runoff from this basin drains to a 10' type R on grade inlet at design point 16.

Basin A6 (Q_5= 15.3cfs, Q_{100}=31.4cfs) is 7.60 acres and 73 percent impervious is comprised of single-family residential lots, local roads. Runoff from this basin drains to an on grade 15' type R inlet at design point 19.

Basin A7 (Q_5= 3.0cfs, Q_{100}=6.1cfs) is 1.43 acres and 75 percent impervious is comprised of single family residential lots and local roads. The runoff from this basin drains to a 15' sump type R inlet located at design point 21.

Basin A8 (Q_5= 2.2cfs, Q_{100}=9.1cfs) 4.22 acres and 13 percent impervious is comprised of a single family residential lots and open space. The runoff from this basin drains to a swale on western side of the site and into a type C inlet located at design point 24.

Basin A9 (Q_5= 0.7cfs, Q_{100}=3.5cfs) 2.02 acres and 8 percent impervious is comprised of a single family residential lots and open space. The runoff from this basin drains to a swale on the western side of the site and into a flared end section and pipe located at design point 25. From there on, the flow enters and existing structure at design point 26.

Basin A10 (Q_5= 2.4cfs, Q_{100}=8.0cfs) 3.23 acres and 24 percent impervious is comprised of a single family residential lots and open space. The runoff from this basin sheet flows to the south and into existing pond W5 at design point 27.

Basin B1 (Q_5= 6.2cfs, Q_{100}=12.0cfs) is 2.44 acres and 80 percent impervious is comprised of single-family residential lots, local roads, two urban knuckles, and a cul-de-sac. The runoff from basin B1 drains to a 15' type R sump inlet located at design point 13. From here-on the runoff is piped in a 24" RCP to DP 14.1.

Basin B2 (Q_5= 9.1cfs, Q_{100}=18.7cfs) is 4.33 acres and 73 percent impervious is comprised of single family residential lots. Runoff from basin B2 drains to a 15' type R sump inlet located at design point 12. The runoff is piped in an 18" RCP and drains to an inlet associated with design point 13.1 on the south side of Polson drive.

Basin B3 (Q_5= 3.5cfs, Q_{100}=7.3cfs) is 2.34 acres and 61 percent impervious is comprised of open space, Sterling Ranch road and sidewalk. Runoff from basin B3 drains to a 15' type R on grade inlet

located at design point 9 in existing Sterling Ranch Road. All of the runoff is captured in the 100 year event. Runoff from this sump inlet is piped and outfalls into pond W-5.

Basin B4 (Q_5= 2.3cfs, Q_{100}=5.6cfs) is 1.80 acres and 51.3 percent impervious is comprised of single family residential lots and open space. Runoff from basin B4 drains to a rear lot type C area inlet at DP 10.

Basin B5 (Q_5=0.7cfs, Q_{100}=1.7cfs) is 0.45 acres and 51 percent impervious is comprised of single family residential lots and open space. Runoff from basin B4 drains to a rear lot area type C inlet at DP 11.

Basin B6 (Q_5=0.8cfs, Q_{100}=2.2cfs) is 0.78 acres and 44 percent impervious is comprised of single family residential lots and open space. Runoff from basin B4 drains to a rear lot area type C inlet at DP 14. The total runoff at design point 14.1 is confluences from upstream basins B1, B2, B4, B5, and B6 from here-on the runoff is piped in a 30" RCP. Runoff in the interim condition will outfall directly into a temporary swale at design point 16.1 and will drain into a temporary 42" FES at design point 11 to pond W-5 as shown in the Interim Drainage map in Appendix E. When the site is entirely developed, the runoff will be pipe to pond W-5 as shown in the Proposed Drainage Map within Appendix E.

Basin C1 (Q_5= 5.5cfs, Q_{100}=11.4cfs) is 2.62 acres and 68.7 percent impervious is comprised of single family residential lots, local roads, and an urban knuckle Runoff from basin C1 drains to 15' a sump type R inlet located at design point 6. The combined runoff at DP 6.1 drains to the existing drainage structure DP 7.2.

Basin C2 (Q_5= 12.0cfs, Q_{100}=25.9cfs) is 6.74 acres and 63 percent impervious is comprised of local roads, single-family residential lots, an urban knuckle, open space, and paved walks. Runoff from basin C2 drains to a 15' type R sump inlet located at design point 5.

Basin C3 (Q_5= 2.2cfs, Q_{100}=9.9cfs) is 3.77 acres and 10 percent impervious is comprised of single family residential lots, open space, and paved walks. Runoff from basin C3 drains to a swale on the western side of the site and into a type C area inlet located at design point 7.

Basin C4 (Q_5= 4.6cfs, Q_{100}=10.2 cfs) is 3.79 acres and 54.7 percent impervious is comprised of open space, roads and rear yards of single family residential lots. Runoff from basin B3 drains to an on-grade 15' type R inlet located at design point 8 in existing Sterling Ranch Road. In the 100 year event, 0.8 cfs is by-passed to a sump inlet adjacent to the intersection of Sterling Ranch Road and Marksheffel Road. From there on the runoff is piped out falls into pond W-5.

Basin D1(Q_5= 0.3cfs, Q_{100}=1.3cfs) is 0.42 acres and 11.5 percent impervious is comprised of open space area. Runoff from basin D1 sheet flow to the southeast and adjacent properties into Sandcreek as per the historic condition. Flows generated from this basin have been attributed to design point 28.

Basin D2 (Q_5= 1.9cfs, Q_{100}=10.5 cfs) is 3.67 acres and 4.6 percent impervious is comprised of open space area. Runoff from basin D1 sheet flow to the southeast into Sandcreek as per the historic condition. Flows generated from this basin have been attributed to design point 29.

Basin OS4 (Q_5= 2.8cfs, Q_{100}=16.9cfs) is 11.71 acres and 3.6 percent impervious and is located immediately north of Sterling Ranch Road and the eastern portion of the site. Runoff from this basin drains south into assumed existing storm sewer built with Filing 2 located at design point 2. Collected runoff is piped south to the existing detention pond built with Filing 2 and outfalls to Sand Creek.

Basin OS6 (Q_5= 35.4cfs, Q_{100}=72.2cfs) (Q5= 35.4cfs, Q100=72.2cfs) is 18.38 acres, and 54 percent impervious is located near the northwest border of the site in the Barbarick subdivision. Runoff from the Barbarick, a portion of lots 3 and 4 for 3.13 acres site, is treated in this area with a sand filter. The other portion of the site is piped with two existing 24" HDPE. In the event, the sand filter clogs in the 100-year event, the emergency overflow from the sand filter will sheet flow across an open area of land i.e. tract B at 11.6 CFS, to sheet flow onto Ennis Drive. The total runoff from basin OS6 will be piped to throughout the Phase 2 site at design point 4 and will outfall in detention pond W5 and will ultimately outfall to Sand Creek. Assumed pipe sizes will be confirmed with the FDR during final platting.

Basin OS7 (Q_5= 20.6cfs, Q_{100}=60.4cfs) is 33.07 Acres and 23 percent impervious and is located directly north of the site in the Barbarick subdivision. Runoff from the eastern portion of the basin travels overland towards design point 1. Historic runoff from this site drains south onto the site at design point 1. Detained flow from this basin will be piped through the site to the detention pond and will outfall to Sand Creek. Emergency overflow from this basin will be routed around the lots and into the open space directly North of Ennis Drive. Assumed pipe and channel sizes will be confirmed with the FDR during final platting.

INTERIM CONDITION PROPOSED SUB-BASIN DRAINAGE

In the interim site condition, all the basins stay the same except basins A2, A3, A4, A6, A7, A8, A9 and A10 will remain undeveloped. The undeveloped basins are summarized below. An interim condition map can be found in Appendix E. The total runoff for the interim site confluences at design point 16.1 and outfalls to a 30" FES within an interim swale and will drain into a temporary 42" FES at design point 11 then the runoff will be piped to pond W-5. For an in-depth discussion on the inlets, storm pipes and design points within the interim condition refer to the basin descriptions for basins B1-B6, and D1-D2 and basins A1 and A5 in the Proposed Drainage Conditions section of this report.

Basin I1 (Q_5= 4.4 cfs, Q_{100}=31.2cfs) 21.99 acres and 1 percent impervious is comprised of open space and a proposed interim channel to convey runoff from the interim developed area as shown on the interim condition map drainage map in within Appendix E. The runoff from the interim development outfalls at design point 16.1 within a 30" RCP. The runoff from basin I1 sheet flows generally to the south and east into a temporary drainage channel where it is conveyed to an existing 42" storm stub with a temporary 42" FES at design point I1 in confluency with the upstream runoff from the interim development. The stormwater for the site will then be treated for water quality and detained for the in pond W-5.

Basin I2 (Q_5= 0.7cfs, Q_{100}=4.9cfs) 3.47 acres and 0 percent impervious is comprised of open space. The runoff from this basin sheet flows to the south and east into an existing drainage swale, where it eventually enters an existing 18" storm stub provided from the Sterling Ranch Filing No 2. Project at design point I1. The stormwater for the site will then be treated for water quality and detained for the in pond W-5.

DRAINAGE DESIGN CRITERIA

DEVELOPMENT CRITERIA REFERENCE

Storm drainage analysis and design criteria for this project were taken from the "*City of Colorado Springs/El Paso County Drainage Criteria Manual*" Volumes 1 and 2 (EPCDCM), dated October 12, 1994, the "*Urban Storm Drainage Criteria Manual*" Volumes 1 to 3 (USDCM) and Chapter 6 and Section 3.2.1 of Chapter 13 of the "*Colorado Springs Drainage Criteria Manual*" (CSDCM), dated May 2014, as adopted by El Paso County.

HYDROLOGIC CRITERIA

All hydrologic data was obtained from the "*El Paso Drainage Criteria Manual*" Volumes 1 and 2, and the "*Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual*" Volumes 1, 2, and 3. Onsite drainage improvements were designed based on the 5 year (minor) storm event and the 100-year (major) storm event. Runoff was calculated using the Rational Method, and rainfall intensities for the 5-year and the 100-year storm return frequencies were obtained from Table 6-2 of the CSDCM. One hour point rainfall data for the storm events is identified in the chart below. Runoff coefficients were determined based on proposed land use and from data in Table 6-6 from the CSDCM. Time of concentrations were developed using equations from CSDCM. All runoff calculations and applicable charts and graphs are included in the Appendices.

Table 2 - 1-hr Point Rainfall Data

Storm	Rainfall (in.)
5-year	1.50
100-year	2.52

HYDRAULIC CRITERIA

The Rational Method and USDCM's SF-2 and SF-3 forms were used to determine the runoff from the minor and major storms on the site. Sump and on-grade inlets were sized using UDFCD UD-Inlet v4.05. StormCAD was used to model the proposed storm sewer system within the interim area and to analyze the proposed HGL calculations for the Construction Drawings. Autodesk Hydraflow express was used to size the overflow channel and an interim swale.

DRAINAGE FACILITY DESIGN

GENERAL CONCEPT

The proposed stormwater conveyance system was designed to convey the developed Sterling Ranch Phase 2 runoff to an existing (Filing 2) full spectrum water quality and detention pond via storm

sewer. The proposed pond was designed to release at less than historic rates to minimize adverse impacts downstream. Treated water will outfall directly into the Sand Creek Drainage way, where it will eventually outfall into Fountain Creek. A proposed drainage map is presented in Appendix E showing locations of the pond. JR Engineering is working on a separate plan to stabilize Sand Creek directly adjacent to the site.

FOUR STEP PROCESS TO MINIMIZE ADVERSE IMPACTS OF URBANIZATION

In accordance with the El Paso County Drainage Criteria Manual Volume 2, this site has implemented the four step process to minimize adverse impacts of urbanization. The four step process includes reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls.

Step 1 – Reducing Runoff Volumes: The Sterling Ranch Phase 2 development project consists of single-family homes with open spaces and lawn areas interspersed within the development which helps disconnect impervious areas and reduce runoff volumes. Roof drains from the structures will discharge to lawn areas, where feasible, to allow for infiltration and runoff volume reduction.

Step 2 – Stabilize Drainageways: The site lies within the Sand Creek Drainage Basin. Basin and bridge fees will be due at time of platting. These funds will be used for the channel stabilization being designed by JR Engineering adjacent to the site and on future projects within the basin to stabilize drainageways. The site does not discharge directly into the open drainageway of Sand Creek, therefore no downstream stabilization will be accomplished with this project.

Step 3 – Treat the WQCV: Water Quality treatment for this site is provided in an existing full spectrum water quality detention pond (W5). The runoff from this site will be collected within inlets and conveyed to the proposed ponds via storm sewer. Upon entrance to the ponds, flows will be captured in a forebay designed to promote settlement of suspended solids. A trickle channel is also incorporated into the ponds to minimize the amount of standing water. The outlet structure has been designed to detain the water quality capture volume (WQCV) for 40 hours, and the extended urban runoff volume (EURV) for 72 hours. All flows released from the ponds will be reduced to less than historic rates.

Step 4 –BMPs will be utilized to minimize off-site contaminants and to protect the downstream receiving waters. The Phase 2 site is residential. There is no proposed commercial or industrial use for the site. The permanent erosion control BMPs include asphalt drives, storm inlets and storm pipe, the full spectrum detention pond W-5 and permanent vegetation. Maintenance responsibilities and plans will be defined at the time of final platting.

WATER QUALITY

In accordance with Section 13.3.2.1 of the CCS/EPCDCM, full spectrum water quality and detention are provided for all developed basins. This site will drain into an existing Full Spectrum Drainage Pond W5 developed during the Sterling Ranch Filing No. 2 Project. Further details as well as all pond volume, water quality, and outfall calculations are included in the Sterling Ranch Filing 2 Final Drainage Report. Pond W5 corresponds to pond FSD6 from the Master Development Drainage Plan for Sterling Ranch", (MMDP) prepared by M&S Civil Consultants, Inc., dated October 24, 2018. (Q_5=7.6 cfs, Q_{100}=149.7 cfs) and is releasing less than the MDDP values in the proposed design. A summary of Pond W-5 has been included below for reference.

Table 3. Pond Volumes & Release Rates

	REQUIRED VOLUME (AC-FT)	VOLUME PROVIDED (AC-FT)	WQCV (AC-FT)	EURV (AC-FT)	5-YEAR RELEASE (CFS)	100-YEAR RELEASE (CFS)
POND W5	18.217	18.441	3.29	11.71	2.7	137.1

EROSION CONTROL PLAN

We respectfully request that the Erosion Control Plan and Cost Estimate be submitted in conjunction with the grading and erosion control plan and construction assurances posted prior to obtaining a grading permit.

OPERATION & MAINTENANCE

In order to ensure the function and effectiveness of the stormwater infrastructure, maintenance activities such as inspection, routine maintenance, restorative maintenance, rehabilitation and repair, are required. The district shall be responsible for the inspection, maintenance, rehabilitation and repair of stormwater and erosion control facilities located on the property unless another party accepts such responsibility in writing and responsibility is properly assigned through legal documentation. Access is provided from onsite facilities and easements for proposed infrastructure located offsite. We respectfully request that the Operation & Maintenance Manual be submitted in conjunction with the construction documents, prior to obtaining a grading permit. A maintenance road was provided for the existing pond W5 and information on the road can be found in the Final Drainage Report for Sterling Ranch Filing No. 2. The maintenance road access is off of Marksheffel Road and wraps around the top of the pond providing access to the inflow pipe wingwalls and outlet structure for the pond.

DRAINAGE AND BRIDGE FEES

Note: This if for informational purposes only and will be confirmed by specific Filing FDR reports.

The site lies within the Sand Creek Drainage Basin. Anticipated drainage and bridge fees are presented below and will be due at time of platting (depending on date of plat submittal):.

2021 DRAINAGE AND BRIDGE FEES – STERLING RANCH PHASE 2				
Impervious Acres (ac)	Drainage Fee (Per Imp. Acre)	Bridge Fee (Per Imp. Acre)	Sterling Ranch Drainage Fee	Sterling Ranch Bridge Fee
37	$20,387	$8,339	$754,319	$308,543

Construction Cost Opinion

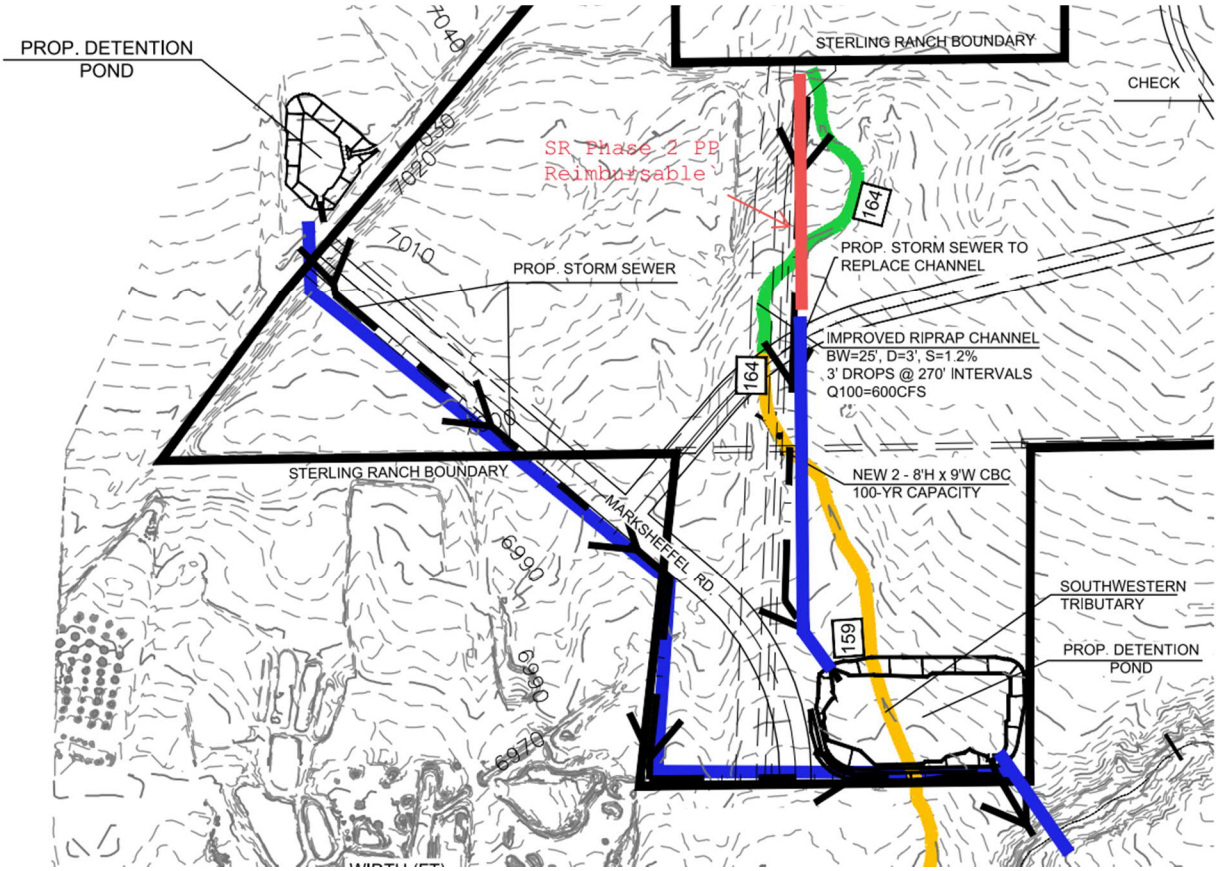
While this is a Preliminary Drainage Report, the Drainage Criteria Manual specifies a Cost Estimate of proposed drainage facility improvements be submitted with each Final Drainage Report. It is anticipated the Sterling Ranch Phase 2 Preliminary Plan will be developed as three (3) filings and a FDR will be prepared for each one. A preliminary construction cost opinion of the reimbursable improvements has been provided below. Swapping of DBPS improvements for proposed improvements is being proposed for this project and is consistent with the methodology used in the Sterling Ranch Filing 2 Final Drainage Report. A map demonstrating the DBPS improvements being swapped is shown below.

It is also anticipated that Per LDC section 8.5.5.C.3.b(ii) Fee Reductions, Credits or Reimbursement for Facilities, this development will request that no cash drainage fees are due at platting as the value of reimbursable DBPS improvements for the Sand Creek Tributary segment 159 and 164 shown in the below table and completed as part of Sterling Ranch Filing 2 and extended in Sterling Ranch Phase 2 Preliminary Plan will exceed the drainage fee estimate shown above.

Sterling Ranch Phase 2 Preliminary Plan (Public - Reimbursable) (Agreed to at Drainage Board meeting of 6/3/21)

Item	Description	Quantity	Unit	Unit Price	Cost	Reimbursable Cost
1	48" RCP	760	L.F.	$ 202	$ 153,520.00	$ 153,520.00
2	Storm Sewer MH, box base	3	Ea.	$ 12,034	$ 36,102.00	$ 36,102.00
				Total	$ 189,622.00	$ 189,622.00

Est. of Sterling Ranch F2 Excess Improvements Cost	$ 1,500,000.00



SUMMARY

The proposed Sterling Ranch Phase 2 drainage improvements were designed to meet or exceed the El Paso County Drainage Criteria. The proposed development will not adversely affect the offsite drainageways or surrounding development. This report is in conformance and meets the latest El Paso County Storm Drainage Criteria requirements for this site.

REFERENCES

1. "El Paso County and City of Colorado Springs Drainage Criteria Manual, Vol I & II".

2. Sand Creek Channel Design Report, prepared by JR Engineering, May 19, 2021 (not yet approved)

3. "Master Development Drainage Plan for Sterling Ranch", (MMDP) prepared by M&S Civil Consultants, Inc., dated October 24, 2018.

4. Sand Creek Drainage Basin Planning Study, prepared Kiowa Engineering Corporation, January 1993, revised March 1996.

5. "Sterling Ranch Filing 2 Final Drainage Report", prepared by JR Engineering, dated May 2020 (not yet approved)

6. Urban Storm Drainage Criteria Manual (Volumes 1, 2, and 3), Urban Drainage and Flood Control District, June 2001.

7. Sand Creek Stabilization at Aspen Meadows Subdivision Filing No. 1 – 100% Design Plans, April 2020

8. Final Drainage Report For Barbarick Subdivision Portion Of Lots 1,2 And Lots 3 and 4, Prepared by Matrix Design Group, June 2016

Appendix A
Vicinity Map, Soil Descriptions, FEMA Floodplain Map

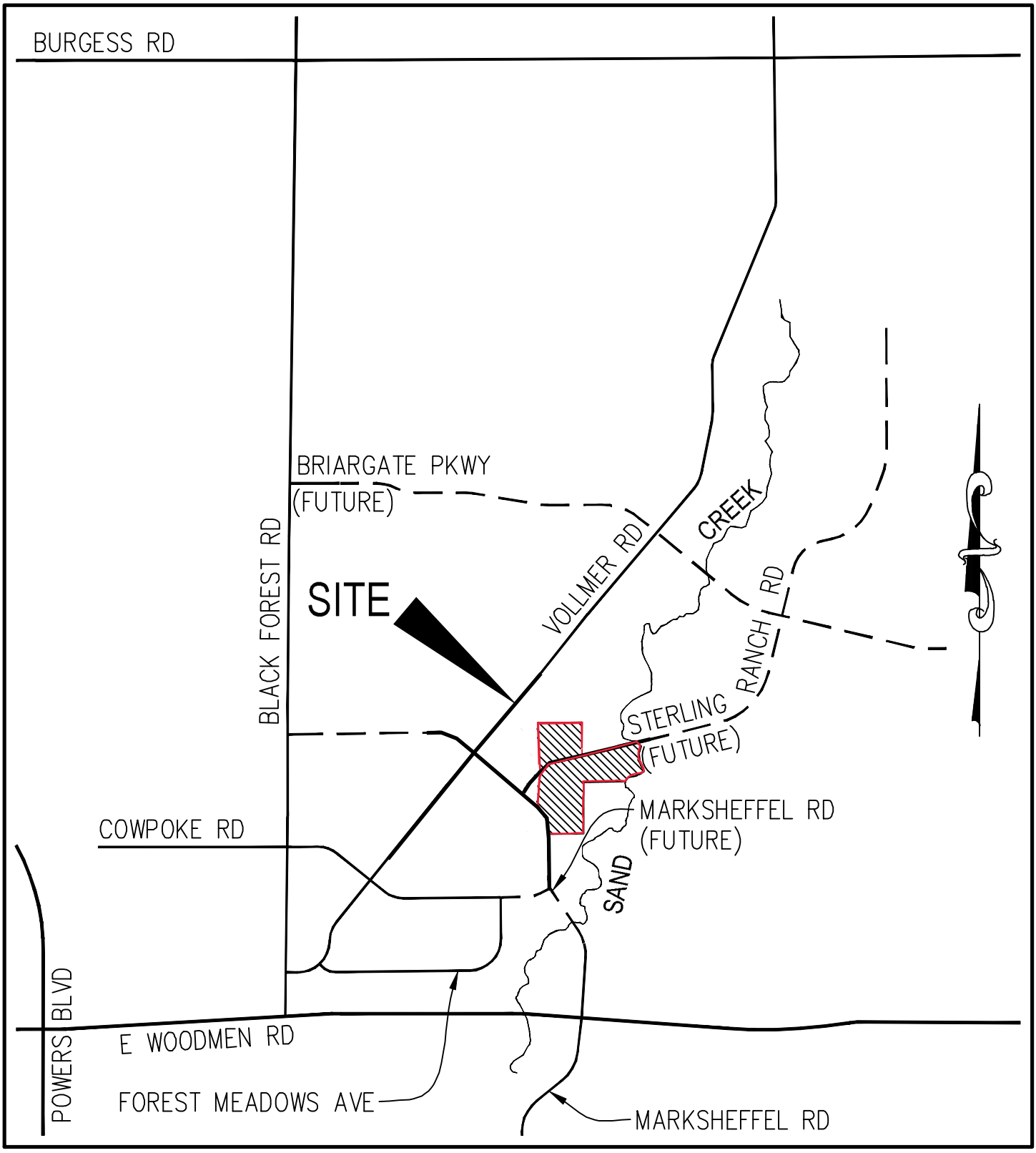
Appendix B
Hydrologic Calculations

Appendix C
Hydraulic Calculations

Appendix D
Reference Material

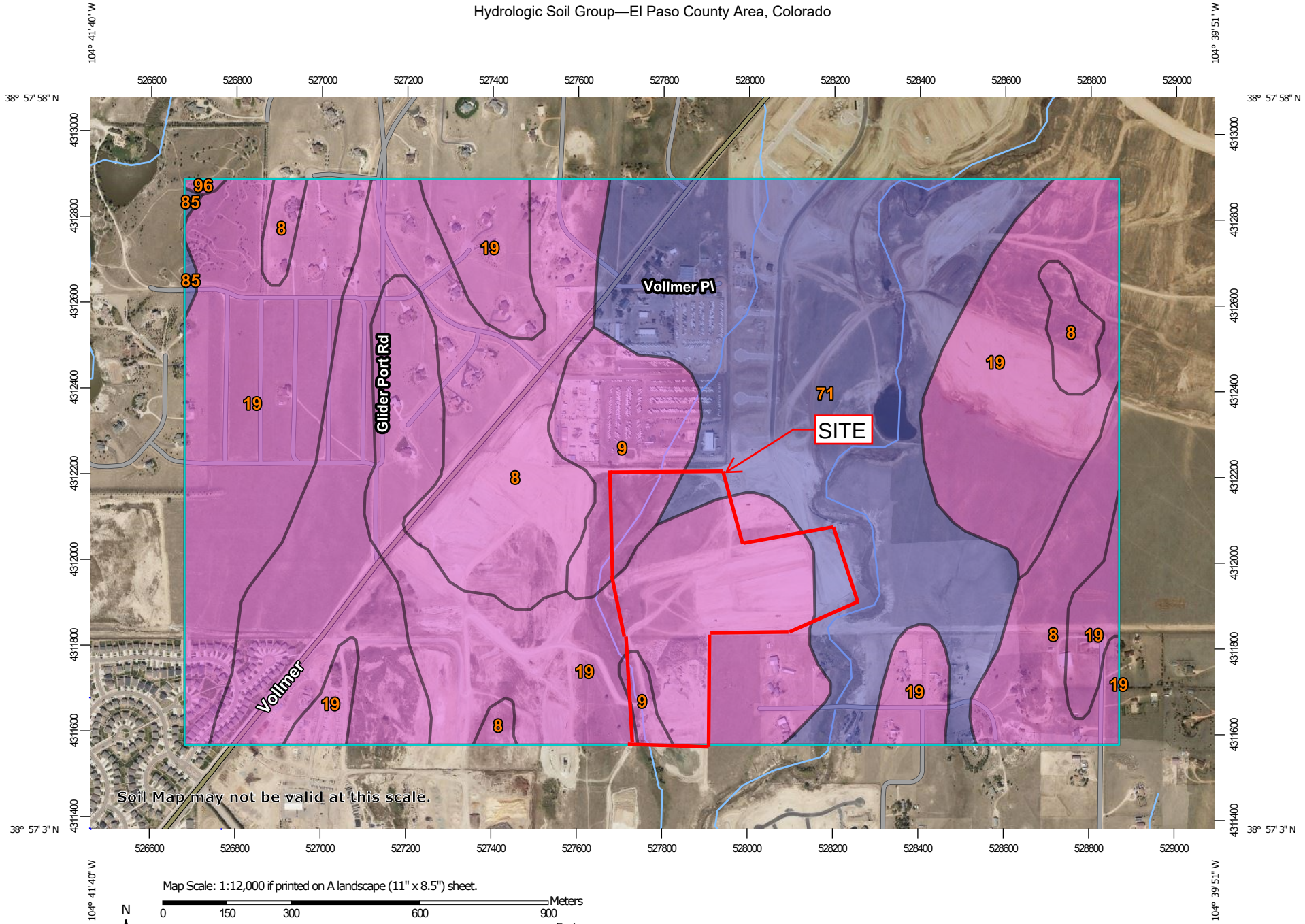
Appendix E
Drainage Maps

Appendix A
Vicinity Map, Soil Descriptions, FEMA Floodplain Map



VICINITY MAP

N.T.S.



Map Scale: 1:12,000 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 150 300 600 900

Feet: 0 500 1000 2000 3000

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 17, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	182.3	25.4%
9	Blakeland-Fluvaquentic Haplaquolls	A	36.8	5.1%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	307.5	42.9%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	188.4	26.3%
85	Stapleton-Bernal sandy loams, 3 to 20 percent slopes	B	1.2	0.2%
96	Truckton sandy loam, 0 to 3 percent slopes	A	0.6	0.1%
Totals for Area of Interest			**716.9**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

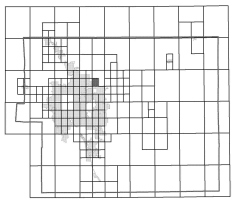
If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

Flooding Source	Vertical Datum Offset (ft)

REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.

NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 13 SOUTH, RANGE 65 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Elevations determined.
ZONE AE Base Flood Elevations determined.
ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99 Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

Floodplain boundary
Floodway boundary
Zone D Boundary
CBRS and OPA boundary
Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513 — Base Flood Elevation line and value; elevation in feet*
(EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A — A — Cross section line
23 — 23 — Transect line
97° 07' 30.00" / 32° 22' 30.00" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4275000mN — 1000-meter Universal Transverse Mercator grid ticks, zone 13
6000000 FT — 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
DX5510 — Bench mark (see explanation in Notes to Users section of this FIRM panel)
M1.5 — River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'
250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

PANEL 0533G

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 533 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0533	G
EL PASO COUNTY	080059	0533	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER 08041C0533G

MAP REVISED DECEMBER 7, 2018

Federal Emergency Management Agency

Appendix B
Hydrologic Calculations

COMPOSITE % IMPERVIOUS & COMPOSITE EXISTING RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/21

Basin ID	Total Area (ac)	Streets (100% Impervious)				Residential (65% Impervious) Neighborhood Area (70% Impervious)				1 Acre lot Rersidential (20% Impervious) Light Commercial (80% Impervious)				Lawns (0% Impervious) School (55% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
A1	5.17	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	5.17	2.0%	0.08	0.35	2.0%
A2	27.48	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	27.48	0.0%	0.08	0.35	0.0%
A3	11.68	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	11.68	0.0%	0.08	0.35	0.0%
B1	11.78	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	11.78	0.0%	0.08	0.35	0.0%
OS1	9.27	0.90	0.96	2.85	30.7%	0.45	0.59	0.00	0.0%	0.30	0.40	2.85	6.1%	0.08	0.35	3.57	0.0%	0.40	0.55	36.9%
OS2	1.94	0.90	0.96	1.94	100.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.90	0.96	100.0%
OS3	2.36	0.90	0.96	2.36	100.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.90	0.96	100.0%
OS4	11.71	0.90	0.96	0.00	0.0%	0.45	0.59	0.65	3.6%	0.59	0.70	0.00	0.0%	0.08	0.35	11.06	0.0%	0.10	0.36	3.6%
OS5	3.46	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	3.46	0.0%	0.08	0.35	0.0%
OS6	18.38	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	10.40	11.3%	0.08	0.35	7.98	0.0%	0.54	0.66	11.3%
OS7	33.07	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	7.91	19.1%	0.08	0.35	25.16	0.0%	0.28	0.48	19.1%
TOTAL (A1-B1)	56.11																			0.2%
TOTAL (OS1-OS7)	80.19																			20.6%
TOTAL	136.30																			12.2%

EXISTING
STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/21

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A1	5.17	A	2%	0.08	0.35	212	2.0%	21.4	517	2.1%	10.0	1.4	6.0	27.4	729.0	32.1	27.4
A2	27.48	A	0%	0.08	0.35	297	2.5%	23.4	1475	2.4%	10.0	1.6	15.7	39.1	1772.0	43.5	39.1
A3	11.68	A	0%	0.08	0.35	121	5.4%	11.6	784	2.7%	10.0	1.7	7.9	19.5	905.0	34.8	19.5
B1	11.78	A	0%	0.08	0.35	297	2.9%	22.4	380	5.2%	10.0	2.3	2.8	25.2	677.0	29.1	25.2
OS1	9.27	A	37%	0.40	0.55	298	2.7%	15.7	737	2.4%	10.0	1.5	8.0	23.7	1035.0	25.4	23.7
OS2	1.94	A	100%	0.90	0.96	117	3.1%	2.7	1745	1.6%	20.0	2.5	11.5	14.2	1862.0	19.0	14.2
OS3	2.36	A	100%	0.90	0.96	41	2.5%	1.7	1681	1.8%	20.0	2.7	10.5	12.2	1722.0	18.1	12.2
OS4	11.71	A	4%	0.10	0.36	491	1.4%	36.0	940	5.6%	10.0	2.4	6.6	42.6	1431.0	32.4	32.4
OS5	3.46	A	0%	0.08	0.35	298	3.0%	22.1	784	2.4%	10.0	1.6	8.4	30.4	1082.0	35.3	30.4
OS6	18.38	A	11%	0.54	0.66	165	3.4%	8.6	612	2.7%	10.0	1.6	6.2	14.8	777.0	30.0	14.8
OS7	33.07	A	19%	0.28	0.48	298	3.0%	17.9	1664	2.7%	10.0	1.6	16.9	34.7	1962.0	37.2	34.7

NOTES:

$$t_c = t_i + t_t$$ Equation 6-2

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$ Equation 6-3

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$ Equation 6-4

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_o = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_o}$
K = NRCS conveyance factor (see Table 6-2).

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$ Equation 6-5

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

STANDARD FORM SF-3 - EXISTING

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	A1	5.17	0.08	27.4	0.41	2.62	1.1															
	2	A2	27.48	0.08	39.1	2.20	2.08	4.6															Basin A2
	3	OS1	9.27	0.40	23.7	3.71	2.83	10.5															Basin A1
	4	OS5	3.46	0.08	30.4	0.28	2.46	0.7															Basin A4
	6	B1	11.78	0.08	25.2	0.94	2.74	2.6															Basin OS1
	7	OS2	1.94	0.90	14.2	1.75	3.60	6.3															Basin OS2
	8	OS3	2.36	0.90	12.2	2.12	3.83	8.1															Basin OS3
	9	OS4	11.71	0.10	32.4	1.18	2.37	2.8															Basin OS4
	10	OS6	18.38	0.54	14.8	10.00	3.54	35.4						10.0	3.4					998	1.8	9.1	Basin OS6 travel to design point 5.1
	11	OS7	33.07	0.28	34.7	9.13	2.26	20.6						9.13	3.2					936	1.8	8.7	Basin OS7 travel to design point 5.1
	5	A3	11.68	0.08	19.5	0.93	3.13	2.9															Basin A3
	5.1								19.5	20.06	3.13	62.7											Design point 5.1 fed by basins A3, OS6, and OS7

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

STANDARD FORM SF-3 - EXISTING

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/21

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	A1	5.17	0.35	27.4	1.81	4.39	8.0															
	2	A2	27.48	0.35	39.1	9.62	3.49	33.6															Basin A2
	3	OS1	9.27	0.55	23.7	5.13	4.76	24.4															Basin A1
	4	OS5	3.46	0.35	30.4	1.21	4.13	5.0															Basin A4
	6	B1	11.78	0.35	25.2	4.12	4.60	19.0															Basin OS1
	7	OS2	1.94	0.96	14.2	1.86	6.04	11.2															Basin OS2
	8	OS3	2.36	0.96	12.2	2.27	6.44	14.6															Basin OS3
	9	OS4	11.71	0.36	32.4	4.25	3.97	16.9															Basin OS4
	10	OS6	18.38	0.66	14.8	12.15	5.94	72.2						12.2	3.4					998	1.8	9.1	Basin OS6 travel to design point 5.1
	11	OS7	33.07	0.48	34.7	15.93	3.79	60.4						15.93	3.2					936	1.8	8.7	Basin OS7 travel to design point 5.1
	5	A3	11.68	0.35	19.5	4.09	5.25	21.5															Basin A3
	5.1								19.5	32.17	5.25	168.9											Design point 5.1 fed by basins A3, OS6, and OS7

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

COMPOSITE % IMPERVIOUS & COMPOSITE PROPOSED RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

Basin ID	Total Area (ac)	Streets (100% Impervious)				Residential (65% Impervious)				Light Industrial (80% Impervious) Commercial (95% Impervious)				Lawns (0% Impervious) School (55% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
A1	4.31	0.90	0.96	0.92	21.3%	0.45	0.59	2.79	42.1%	0.59	0.70	0.00	0.0%	0.08	0.35	0.60	0.0%	0.49	0.64	63.4%
A5	0.45	0.90	0.96	0.17	37.8%	0.45	0.59	0.28	40.4%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.62	0.73	78.2%
I1	21.99	0.90	0.96	0.12	0.5%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	21.87	0.0%	0.08	0.35	0.5%
I2	3.47	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	3.47	0.0%	0.08	0.35	0.0%
B1	2.44	0.90	0.96	1.04	42.6%	0.45	0.59	1.40	37.3%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.64	0.75	79.9%
B2	4.33	0.90	0.96	0.94	21.7%	0.45	0.59	3.39	50.9%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.55	0.67	72.6%
C1	3.29	0.90	0.96	0.72	21.9%	0.45	0.59	1.66	32.8%	0.59	0.70	0.00	0.0%	0.08	0.35	0.91	0.0%	0.45	0.60	54.7%
C2	6.74	0.90	0.96	1.49	22.1%	0.45	0.59	4.21	40.6%	0.59	0.70	0.00	0.0%	0.08	0.35	1.04	0.0%	0.49	0.63	62.7%
C3	3.11	0.90	0.96	0.10	3.2%	0.45	0.59	0.37	7.7%	0.59	0.70	0.00	0.0%	0.08	0.35	2.64	0.0%	0.15	0.40	10.9%
B6	0.78	0.90	0.96	0.00	0.0%	0.45	0.59	0.53	44.2%	0.59	0.70	0.00	0.0%	0.08	0.35	0.25	0.0%	0.33	0.51	44.2%
B5	0.45	0.90	0.96	0.00	0.0%	0.45	0.59	0.35	50.6%	0.59	0.70	0.00	0.0%	0.08	0.35	0.10	0.0%	0.37	0.54	50.6%
B4	1.55	0.90	0.96	0.00	0.0%	0.45	0.59	1.35	56.6%	0.59	0.70	0.00	0.0%	0.08	0.35	0.20	0.0%	0.40	0.56	56.6%
B3	0.66	0.90	0.96	0.34	51.5%	0.45	0.59	0.12	11.8%	0.59	0.70	0.00	0.0%	0.08	0.35	0.20	0.0%	0.57	0.71	63.3%
C4	1.34	0.90	0.96	0.19	14.2%	0.45	0.59	0.80	38.8%	0.59	0.70	0.00	0.0%	0.08	0.35	0.35	0.0%	0.42	0.58	53.0%
D1	0.77	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	0.77	0.0%	0.08	0.35	0.0%
D2	3.92	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	3.92	0.0%	0.08	0.35	0.0%
OS6	18.38	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	10.40	53.8%	0.08	0.35	7.98	0.0%	0.54	0.66	53.8%
OS4	11.71	0.90	0.96	0.00	0.0%	0.45	0.59	0.65	3.6%	0.59	0.70	0.00	0.0%	0.58	0.68	11.06	51.9%	0.57	0.68	55.6%
OS7	33.07	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	7.91	22.7%	0.08	0.35	25.16	0.0%	0.28	0.48	22.7%
TOTAL (A1-C4)(I1-I2)	59.60																			28.9%
TOTAL (OS4 -OS7)	63.16																			37.8%
TOTAL	122.76																			33.5%

PROPOSED
STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A1	4.31	A	63%	0.49	0.64	79	1.7%	8.2	1007	3.7%	20.0	3.8	4.4	12.5	1086.0	20.1	12.5
A5	0.45	A	78%	0.62	0.73	54	3.7%	4.1	217	3.9%	20.0	4.0	0.9	5.0	271.0	13.6	5.0
I1	21.99	A	1%	0.08	0.35	793	3.1%	35.5	627	3.7%	10.0	1.9	5.4	41.0	1420.0	31.9	31.9
I2	3.47	A	0%	0.08	0.35	383	3.2%	24.6	394	1.0%	10.0	1.0	6.6	31.1	777.0	33.3	31.1
B1	2.44	A	80%	0.64	0.75	50	2.5%	4.3	1066	1.6%	20.0	2.5	7.1	11.4	1116.0	19.4	11.4
B2	4.33	A	73%	0.55	0.67	226	4.9%	8.8	346	0.7%	20.0	1.7	3.4	12.2	572.0	17.2	12.2
C1	3.29	A	55%	0.45	0.60	228	4.3%	11.0	393	1.8%	20.0	2.7	2.5	13.5	621.0	19.7	13.5
C2	6.74	A	63%	0.49	0.63	99	1.8%	9.0	796	1.7%	20.0	2.6	5.1	14.1	895.0	21.1	14.1
C3	3.11	A	11%	0.15	0.40	144	9.6%	9.8	255	3.5%	15.0	2.8	1.5	11.3	399.0	26.3	11.3
B6	0.78	A	44%	0.33	0.51	246	1.5%	19.1	0	1.0%	20.0	2.0	0.0	19.1	246.0	18.5	18.5
B5	0.45	A	51%	0.37	0.54	129	5.0%	8.8	0	1.0%	20.0	2.0	0.0	8.8	129.0	17.4	8.8
B4	1.55	B	57%	0.40	0.56	222	11.0%	8.5	914	1.1%	20.0	2.1	7.4	15.9	1136.0	25.1	15.9
B3	0.66	A	63%	0.57	0.71	165	3.4%	8.2	612	2.7%	10.0	1.6	6.2	14.4	777.0	18.7	14.4
C4	1.34	A	53%	0.42	0.58	298	3.0%	14.8	1664	2.7%	10.0	1.6	16.9	31.7	1962.0	27.3	27.3
D1	0.77	A	0%	0.08	0.35	16	2.0%	5.9	570	6.0%	10.0	2.4	3.9	9.7	586.0	30.3	9.7
D2	3.92	A	0%	0.08	0.35	105	25.0%	6.5	975	50.0%	15.0	10.6	1.5	8.1	1080.0	28.6	8.1
OS6	18.38	A	54%	0.54	0.66	165	3.4%	8.6	612	2.7%	10.0	1.6	6.2	14.8	777.0	20.6	14.8
OS4	11.71	A	56%	0.57	0.68	491	1.4%	19.0	940	5.6%	10.0	2.4	6.6	25.6	1431.0	20.5	20.5
OS7	33.07	A	23%	0.28	0.48	298	3.0%	17.9	1664	2.7%	10.0	1.6	16.9	34.7	1962.0	36.0	34.7

NOTES:

$$t_c = t_i + t_t$$ Equation 6-2

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_w}} = \frac{L_t}{60V_t}$$ Equation 6-4

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_w = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_w}$
K = NRCS conveyance factor (see Table 6-2).

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$ Equation 6-3

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$ Equation 6-5

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	1	OS7	33.07	0.28	34.7	9.13	2.26	20.6								20.6	9.13	1.0	42	725	8.2	1.5	Offsite Barbarick Subdivision pond release Piped to DP 3
	2	OS4	11.71	0.57	20.5	6.71	3.05	20.5								20.5	6.71	1.0	36	112	8.3	0.2	Offsite future school Piped to DP 3
	3								36.2	15.84	2.20	34.8											Piped to existing storm sewer in Sterling Ranch Road
	4	OS6	18.38	0.54	14.8	10.00	3.54	35.4								35.4	10.00	1.0	48	800	9.4	1.4	Offsite subdivision pond release Piped to DP 7.1
	5	C2	6.74	0.49	14.1	3.32	3.61	12.0								12.0	3.32	1.0	24	63	7.3	0.1	Sump Inlet Piped to DP 6.1
	6	C1	3.29	0.45	13.5	1.47	3.68	5.4															Sump Inlet Piped to DP 6.1
	6.1								14.3	4.79	3.59	17.2				17.2	4.79	1.0	36	245	7.9	0.5	Piped to DP 7.1
	7	C3	3.11	0.15	11.3	0.47	3.95	1.9															Area Inlet Piped to DP 7.1
	7.1								16.2	15.26	3.40	51.9											Piped to existing storm sewer in Sterling Ranch Road
	8	C4	1.34	0.42	27.3	0.56	2.62	1.5															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	9	B3	0.66	0.57	14.4	0.38	3.58	1.4															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	10	B4	1.55	0.40	15.9	0.62	3.43	2.1								2.1	0.62	1.0	12	380	4.7	1.3	Rear lot and area inlets Piped to DP 11.1
	11	B5	0.45	0.37	8.8	0.17	4.31	0.7															Area Inlet Piped to DP 14.1
	11.1								17.3	0.79	3.31	2.6				2.6	0.79	1.0	18	357	4.9	1.2	Piped to DP 14.1
	12	B2	4.33	0.55	12.2	2.37	3.83	9.1								9.1	2.37	1.0	18	38	6.7	0.1	Sump Inlet Piped to DP 13.1
	13	B1	2.44	0.64	11.4	1.57	3.93	6.2															Sump Inlet Piped to DP 13.1
	13.1								12.3	3.94	3.82	15.0				15.0	3.94	1.0	24	125	7.7	0.3	Piped to DP 14.1
	14	B6	0.78	0.33	18.5	0.26	3.21	0.8															Area Inlet Piped to DP 14.1
	14.1								18.5	4.99	3.21	16.0				16.0	4.99	1.0	24	415	7.8	0.9	Piped to DP 15.1

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	15	A1	4.31	0.49	12.5	2.13	3.79	8.1					0.7	0.18	1.6	7.4				230	2.5	1.5	On-grade Inlet Captured Flows piped to DP 15.1, Bypass flow to DP 17
	15.1								19.4	7.12	3.14	22.3				22.3	7.12	1.0	24	45	8.2	0.1	On-grade Inlet Captured Flows piped to DP 16.1
	16	A5	0.45	0.62	5.0	0.28	5.16	1.4															On-grade Inlet Captured Flows piped to DP 16.1
	16.1								19.5	7.40	3.13	23.2				23.2	7.40	1.0	24	125	8.2	0.3	FES release to drainage channel
	I1	I1	21.99	0.08	31.9	1.86	2.39	4.4															FES
	I1.1								31.9	9.26	2.39	22.1				22.1	9.26	0.4	42	62	6.1	0.2	Combined flow from DPI1 & DP16.1 Piped to Existing 84" RCP
	I2	I2	3.47	0.08	31.1	0.28	2.43	0.7															Piped to Existing 84" RCP

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	OS7	33.07	0.48	34.7	15.93	3.79	60.4								60.4	15.93	1.0	42	725	10.9	1.1	Offsite Barbarick Subdivision pond release Piped to DP 3
	2	OS4	11.71	0.68	20.5	7.90	5.12	40.5								40.5	7.90	1.0	36	112	9.9	0.2	Offsite future school Piped to DP 3
	3								35.9	23.83	3.71	88.5											Piped to existing storm sewer in Sterling Ranch Road
	4	OS6	18.38	0.66	14.8	12.15	5.94	72.2								72.2	12.15	1.0	48	800	11.4	1.2	Offsite subdivision pond release Piped to DP 7.1
	5	C2	6.74	0.63	14.1	4.28	6.06	25.9								25.9	4.28	1.0	24	63	8.3	0.1	Sump Inlet Piped to DP 6.1
	6	C1	3.29	0.60	13.5	1.99	6.18	12.3															Sump Inlet Piped to DP 6.1
	6.1								14.3	6.27	6.04	37.8				37.8	6.27	1.0	36	245	9.7	0.4	Piped to DP 7.1
	7	C3	3.11	0.40	11.3	1.24	6.63	8.2															Area Inlet Piped to DP 7.1
	7.1								16.0	19.66	5.75	113.0											Piped to existing storm sewer in Sterling Ranch Road
	8	C4	1.34	0.58	27.3	0.78	4.40	3.4															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	9	B3	0.66	0.71	14.4	0.47	6.01	2.8															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	10	B4	1.55	0.56	15.9	0.87	5.76	5.0								5.0	0.87	1.0	12	380	6.4	1.0	Rear lot and area inlets Piped to DP 11.1
	11	B5	0.45	0.54	8.8	0.24	7.24	1.7															Area Inlet Piped to DP 14.1
	11.1								16.9	1.11	5.61	6.2				6.2	1.11	1.0	18	357	6.2	1.0	Piped to DP 14.1
	12	B2	4.33	0.67	12.2	2.90	6.43	18.7								18.7	2.90	1.0	18	38	10.6	0.1	Sump Inlet Piped to DP 13.1
	13	B1	2.44	0.75	11.4	1.82	6.60	12.0															Sump Inlet Piped to DP 13.1
	13.1								12.3	4.72	6.42	30.3				30.3	4.72	1.0	24	125	9.7	0.2	Piped to DP 14.1
	14	B6	0.78	0.51	18.5	0.40	5.38	2.2															Area Inlet Piped to DP 14.1
	14.1								18.5	6.23	5.38	33.5				33.5	6.23	1.0	24	415	10.7	0.6	Piped to DP 15.1
	15	A1	4.31	0.64	12.5	2.74	6.37	17.4					10.0	1.5777	1.6	7.4				230	2.5	1.5	On-grade Inlet Captured Flows piped to DP 15.1, Bypass flow to DP 17
	15.1								19.1	8.97	5.30	47.5				47.5	8.97	1.0	24	45	15.1	0.0	On-grade Inlet Captured Flows piped to DP 16.1

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Interim
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 5/4/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	16	A5	0.45	0.73	5.0	0.33	8.66	2.9															On-grade Inlet Captured Flows piped to DP 16.1
	16.1								19.2	9.30	5.29	49.2				49.2	9.30	1.0	24	125	15.7	0.1	FES release to drainage channel
	I1	I1	21.99	0.35	31.9	7.77	4.01	31.2															FES
	I1.1								31.9	17.07	4.01	68.4				68.4	17.07	0.4	42	62	7.7	0.1	Combined flow from DPI1 & DP16.1 Piped to Existing 84" RCP
	I2	I2	3.47	0.35	31.1	1.21	4.07	4.9															Piped to Existing 84" RCP

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

COMPOSITE % IMPERVIOUS & COMPOSITE PROPOSED RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

Basin ID	Total Area (ac)	Paved/Streets (100% Impervious)				Residential (65% Impervious)				Light Industrial (80% Impervious) Commercial (95% Impervious)				Lawns (0% Impervious) School (55% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
A1	4.31	0.90	0.96	0.92	21.3%	0.45	0.59	2.79	42.1%	0.59	0.70	0.00	0.0%	0.08	0.35	0.60	0.3%	0.49	0.64	63.7%
A2	1.41	0.90	0.96	0.22	15.6%	0.45	0.59	0.34	15.7%	0.59	0.70	0.00	0.0%	0.08	0.35	0.85	0.0%	0.30	0.50	31.3%
A3	3.68	0.90	0.96	0.71	19.3%	0.45	0.59	2.59	45.7%	0.59	0.70	0.00	0.0%	0.08	0.35	0.38	0.0%	0.50	0.64	65.1%
A4	3.94	0.90	0.96	0.67	17.0%	0.45	0.59	2.13	35.1%	0.59	0.70	0.00	0.0%	0.08	0.35	1.14	0.0%	0.42	0.58	52.1%
A5	0.45	0.90	0.96	0.17	37.8%	0.45	0.59	0.28	40.4%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.62	0.73	78.2%
A6	7.60	0.90	0.96	1.76	23.2%	0.45	0.59	5.84	49.9%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.55	0.68	73.1%
A7	1.43	0.90	0.96	0.43	29.8%	0.45	0.59	1.00	45.5%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.58	0.70	75.3%
A8	4.22	0.90	0.96	0.12	2.8%	0.45	0.59	0.68	10.5%	0.59	0.70	0.00	0.0%	0.08	0.35	3.42	0.0%	0.16	0.41	13.3%
B1	2.44	0.90	0.96	1.04	42.6%	0.45	0.59	1.40	37.3%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.64	0.75	79.9%
B2	4.33	0.90	0.96	0.94	21.7%	0.45	0.59	3.39	50.9%	0.59	0.70	0.00	0.0%	0.08	0.35	0.00	0.0%	0.55	0.67	72.6%
C1	2.62	0.90	0.96	0.72	27.5%	0.45	0.59	1.66	41.2%	0.59	0.70	0.00	0.0%	0.08	0.35	0.24	0.0%	0.54	0.67	68.7%
C2	6.74	0.90	0.96	1.49	22.1%	0.45	0.59	4.21	40.6%	0.59	0.70	0.00	0.0%	0.08	0.35	1.04	0.0%	0.49	0.63	62.7%
C3	3.77	0.90	0.96	0.13	3.4%	0.45	0.59	0.37	6.4%	0.59	0.70	0.00	0.0%	0.08	0.35	3.27	0.0%	0.14	0.39	9.8%
A9	2.02	0.90	0.96	0.06	3.0%	0.45	0.59	0.15	4.8%	0.59	0.70	0.00	0.0%	0.08	0.35	1.81	0.0%	0.13	0.39	7.8%
A10	3.23	0.90	0.96	0.14	4.3%	0.45	0.59	0.98	19.7%	0.59	0.70	0.00	0.0%	0.08	0.35	2.11	0.0%	0.23	0.45	24.1%
B6	0.78	0.90	0.96	0.00	0.0%	0.45	0.59	0.53	44.2%	0.59	0.70	0.00	0.0%	0.08	0.35	0.25	0.0%	0.33	0.51	44.2%
B5	0.45	0.90	0.96	0.00	0.0%	0.45	0.59	0.35	50.6%	0.59	0.70	0.00	0.0%	0.08	0.35	0.10	0.0%	0.37	0.54	50.6%
B4	1.80	0.90	0.96	0.05	2.6%	0.45	0.59	1.35	48.8%	0.59	0.70	0.00	0.0%	0.08	0.35	0.40	0.0%	0.38	0.55	51.3%
B3	2.36	0.90	0.96	1.37	57.9%	0.45	0.59	0.12	3.3%	0.59	0.70	0.00	0.0%	0.08	0.35	0.87	0.0%	0.57	0.72	61.2%
C4	3.79	0.90	0.96	1.55	41.0%	0.45	0.59	0.80	13.7%	0.59	0.70	0.00	0.0%	0.08	0.35	1.44	0.0%	0.49	0.65	54.7%
D1	0.42	0.90	0.96	0.05	11.5%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	0.37	0.0%	0.17	0.42	11.5%
D2	3.67	0.90	0.96	0.17	4.6%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	3.50	0.0%	0.12	0.38	4.6%
OS6	18.38	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	10.40	53.8%	0.08	0.35	7.98	0.0%	0.54	0.66	53.8%
OS4	11.71	0.90	0.96	0.00	0.0%	0.45	0.59	0.65	3.6%	0.59	0.70	0.00	0.0%	0.58	0.68	11.06	51.9%	0.57	0.68	55.6%
OS7	33.07	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.90	0.90	7.91	22.7%	0.08	0.35	25.16	0.0%	0.28	0.48	22.7%
TOTAL (A1-C4)	61.37																			53.2%
TOTAL (OS4 -OS7)	63.16																			37.8%
TOTAL	128.62																			44.1%

PROPOSED
STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A1	4.31	A	64%	0.49	0.64	79	1.7%	8.2	1007	3.7%	20.0	3.8	4.4	12.5	1086.0	20.0	12.5
A2	1.41	A	31%	0.30	0.50	266	3.7%	15.4	141	1.5%	20.0	2.4	1.0	16.3	407.0	22.1	16.3
A3	3.68	A	65%	0.50	0.64	120	3.7%	7.7	1008	2.4%	20.0	3.1	5.5	13.2	1128.2	21.0	13.2
A4	3.94	A	52%	0.42	0.58	118	2.1%	10.5	814	1.9%	20.0	2.8	4.9	15.4	932.0	23.2	15.4
A5	0.45	A	78%	0.62	0.73	54	3.7%	4.1	217	3.9%	20.0	4.0	0.9	5.0	271.0	13.6	5.0
A6	7.60	A	73%	0.55	0.68	212	4.3%	8.9	723	1.4%	20.0	2.4	5.0	13.9	934.9	18.8	13.9
A7	1.43	A	75%	0.58	0.70	303	3.4%	10.9	367	1.2%	20.0	2.2	2.8	13.7	670.0	16.1	13.7
A8	4.22	A	13%	0.16	0.41	233	4.9%	15.3	307	0.9%	15.0	1.4	3.6	18.9	540.0	28.7	18.9
B1	2.44	A	80%	0.64	0.75	50	2.5%	4.3	1066	1.6%	20.0	2.5	7.1	11.4	1116.0	19.4	11.4
B2	4.33	A	73%	0.55	0.67	226	4.9%	8.8	346	0.7%	20.0	1.7	3.4	12.2	572.0	17.2	12.2
C1	2.62	A	69%	0.54	0.67	228	4.3%	9.5	393	1.8%	20.0	2.7	2.5	11.9	621.0	17.0	11.9
C2	6.74	A	63%	0.49	0.63	99	1.8%	9.0	796	1.7%	20.0	2.6	5.1	14.1	895.0	21.1	14.1
C3	3.77	A	10%	0.14	0.39	144	9.6%	9.8	255	3.5%	15.0	2.8	1.5	11.3	399.0	26.5	11.3
A9	2.02	A	8%	0.13	0.39	452	2.4%	27.8	108	2.6%	20.0	3.2	0.6	28.4	560.0	25.8	25.8
A10	3.23	A	24%	0.23	0.45	248	2.8%	17.6	0	1.0%	20.0	2.0	0.0	17.6	248.0	21.9	17.6
B6	0.78	A	44%	0.33	0.51	246	1.5%	19.1	0	1.0%	20.0	2.0	0.0	19.1	246.0	18.5	18.5
B5	0.45	A	51%	0.37	0.54	129	5.0%	8.8	0	1.0%	20.0	2.0	0.0	8.8	129.0	17.4	8.8
B4	1.80	B	51%	0.38	0.55	222	11.0%	8.8	914	1.1%	20.0	2.1	7.4	16.2	1136.0	26.4	16.2
B3	2.36	A	61%	0.57	0.72	165	3.4%	8.1	1595	1.5%	10.0	1.2	21.7	29.8	1760.0	27.9	27.9
C4	3.79	A	55%	0.49	0.65	298	3.0%	13.1	1664	1.5%	10.0	1.2	22.6	35.8	1962.0	30.3	30.3
D1	0.42	A	12%	0.17	0.42	16	2.0%	5.3	570	6.0%	10.0	2.4	3.9	9.2	586.0	27.7	9.2
D2	3.67	A	5%	0.12	0.38	105	25.0%	6.3	975	50.0%	15.0	10.6	1.5	7.8	1080.0	27.6	7.8
OS6	18.38	A	54%	0.54	0.66	165	3.4%	8.6	612	2.7%	10.0	1.6	6.2	14.8	777.0	20.6	14.8
OS4	11.71	A	56%	0.57	0.68	491	1.4%	19.0	940	5.6%	10.0	2.4	6.6	25.6	1431.0	20.5	20.5
OS7	33.07	A	23%	0.28	0.48	298	3.0%	17.9	1664	2.7%	10.0	1.6	16.9	34.7	1962.0	36.0	34.7

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	1	OS7	33.07	0.28	34.7	9.13	2.26	20.6								20.6	9.13	1.0	36	430	8.3	0.9	Offsite Barbarick Subdivision pond release Piped to DP 4.1
	2	OS4	11.71	0.57	20.5	6.71	3.05	20.5								20.5	6.71	1.0	36	112	8.3	0.2	Offsite future school Piped to DP 3
	3								20.7	6.71	3.04	20.4											Piped to existing storm sewer in Sterling Ranch Road
	4	OS6	18.38	0.54	14.8	10.00	3.54	35.4															Offsite subdivision pond release Confluenced at DP 4.1
	4.1								35.6	19.13	2.22	42.5				35.6	19.13	1.0	48	775	9.5	1.4	Offsite flow confluenced from basins OS7 and OS4 Piped to DP 7.1
	5	C2	6.74	0.49	14.1	3.32	3.61	12.0								12.0	3.32	1.0	24	63	7.3	0.1	Sump Inlet Piped to DP 6.1
	6	C1	2.62	0.54	11.9	1.41	3.87	5.5															Sump Inlet Piped to DP 6.1
	6.1								14.3	4.73	3.59	17.0				17.0	4.73	1.0	36	245	7.8	0.5	Piped to DP 7.2
	7	C3	3.77	0.14	11.3	0.55	3.94	2.2															Area Inlet Piped to DP 7.1
	7.1								37.0	19.68	2.17	42.7				42.7	19.68	1.0	36	40	10.0	0.1	Structure piped to 7.2
	7.2								37.0	24.41	2.16	52.8											Piped to existing storm sewer in Sterling Ranch Road
	8	C4	3.79	0.49	30.3	1.87	2.47	4.6															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	9	B3	2.36	0.57	27.9	1.35	2.59	3.5															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	10	B4	1.80	0.38	16.2	0.68	3.41	2.3								2.3	0.68	1.0	12	380	4.8	1.3	Rear lot and area inlets Piped to DP 11.1
	11	B5	0.45	0.37	8.8	0.17	4.31	0.7															Area Inlet Piped to DP 14.1
	11.1								17.5	0.85	3.29	2.8				2.8	0.85	1.0	18	357	5.0	1.2	Piped to DP 14.1
	12	B2	4.33	0.55	12.2	2.37	3.83	9.1								9.1	2.37	1.0	18	38	6.7	0.1	Sump Inlet Piped to DP 13.1
	13	B1	2.44	0.64	11.4	1.57	3.93	6.2															Sump Inlet Piped to DP 13.1
	13.1								12.3	3.94	3.82	15.0				15.0	3.94	1.0	24	125	7.7	0.3	Piped to DP 14.1
																							Area Inlet

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	14	B6	0.78	0.33	18.5	0.26	3.21	0.8															Piped to DP 14.1
	14.1								18.7	5.05	3.19	16.1				16.1	5.05	1.0	24	415	7.8	0.9	Piped to DP 15.1

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	15	A1	4.31	0.49	12.5	2.13	3.79	8.1					0.7	0.18	1.6	7.4	1.95			230	2.5	1.5	On-grade Inlet Captured Flows piped to DP 15.1, Bypass flow to DP 17
	15.1								19.6	7.00	3.12	21.8				21.8	7.00	1.0	24	48	8.2	0.1	Captured Flows piped to DP 16.1
	16	A5	0.45	0.62	5.0	0.28	5.16	1.4					0.0	0	2.9	1.4							On-grade Inlet Captured Flows piped to DP 16.1
	16.1								19.7	7.28	3.11	22.7				22.7	7.28	1.0	24	280	8.2	0.6	Piped to DP 18.1
	17	A2	1.41	0.30	16.3	0.42	3.40	1.4	16.3	0.60	3.40	2.0	0.0	0		2.0	0.42	1.0	24	27	4.4	0.1	On-grade Inlet Piped to DP 18.1
	18.1								20.3	7.88	3.07	24.2				24.2	0.00	1.0	30	600	8.7	1.1	Piped to DP20.1
	19	A6	7.60	0.55	13.9	4.21	3.64	15.3					4.5	1.24	1.0	10.8	2.97	1.0	18	30	6.8	0.1	On-grade Inlet Captured Flows piped to DP 20.1, Bypass flow to DP 21
	20	A3	3.68	0.50	13.2	1.84	3.72	6.8					0.0	0	1.0	6.8	1.84	1.0	18	4	6.3	0.0	On-grade Inlet Captured Flows piped to DP 20.1
	20.1								21.4	12.69	2.99	37.9				37.9	12.69	1.0	36	220	9.7	0.4	Piped to DP23
	21	A7	1.43	0.58	13.7	0.83	3.66	3.0	14.0	2.07	3.63	7.5				7.5	2.07	1.0	18	60	6.4	0.2	Sump Inlet Piped to DP21.1
	21.1								21.4	14.76	2.99	44.1				44.1	14.76	1.0	42	90	10.1	0.1	MH Piped to DP23
	22	A4	3.94	0.42	15.4	1.65	3.48	5.7	15.4	1.65	3.48	5.7											Sump Inlet Piped to DP22.1
	22.1								15.4	1.65	3.48	5.7				5.7	1.65	1.0	24	10	6.0	0.0	Piped to DP23
	23								21.8	16.41	2.96	48.6				48.6	16.41	1.0	42	145	10.3	0.2	Piped to DP26
	24	A8	4.22	0.16	18.9	0.69	3.17	2.2															Area Inlet Piped to EX 84" Storm Line Built w/ SR Filing 2 First Phase
	25	A9	2.02	0.13	25.8	0.27	2.71	0.7								0.7	0.27	1.0	18	30	3.4	0.1	EX FES Piped to EX 84" Storm Line Built w/ SR Filing 2 First Phase
	27	A10	3.23	0.23	17.6	0.74	3.28	2.4															Pervious area sheet flows into EX Pond W5
	28	D1	0.42	0.17	9.2	0.07	4.25	0.3															Pervious area sheet flows into Sand Creek
	29	D2	3.67	0.12	7.8	0.43	4.50	1.9															Pervious area sheet flows into Sand Creek

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	OS7	33.07	0.48	34.7	15.93	3.79	60.4								60.4	15.93	1.0	36	725	10.7	1.1	Offsite Barbarick Subdivision pond release Piped to DP 4.1
	2	OS4	11.71	0.68	20.5	7.90	5.12	40.5								40.5	7.90	1.0	36	112	9.9	0.2	Offsite future school Piped to DP 3
	3								20.7	7.90	5.10	40.3											Piped to existing storm sewer in Sterling Ranch Road
	4	OS6	18.38	0.66	14.8	12.15	5.94	72.2								72.2	12.15	1.0	48	800	11.4	1.2	Offsite subdivision pond release Piped to DP 7.1
	4.1								35.9	28.08	3.71	104.3											Offsite subdivision pond release Confluenced at DP 4.1
	5	C2	6.74	0.63	14.1	4.28	6.06	25.9								25.9	4.28	1.0	24	63	8.3	0.1	Sump Inlet Piped to DP 6.1
	6	C1	2.62	0.67	11.9	1.75	6.49	11.4															Sump Inlet Piped to DP 6.1
	6.1								14.3	6.03	6.04	36.4				36.4	6.03	1.0	36	245	9.6	0.4	Piped to DP 7.1
	7	C3	3.77	0.39	11.3	1.49	6.61	9.9															Area Inlet Piped to DP 7.1
	7.1								35.9	29.57	3.71	109.8				109.8	29.57	1.0	36	40	15.5	0.0	Structure piped to 7.2
	7.2								35.9	35.60	3.71	132.1											Piped to existing storm sewer in Sterling Ranch Road
	8	C4	3.79	0.65	30.3	2.47	4.14	10.2															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	9	B3	2.36	0.72	27.9	1.69	4.34	7.3															Offsite flow to existing inlet in Sterling Ranch Road Piped to existing storm sewer in Sterling Ranch Road
	10	B4	1.80	0.55	16.2	0.98	5.72	5.6								5.6	0.98	1.0	12	380	7.2	0.9	Rear lot and area inlets Piped to DP 11.1
	11	B5	0.45	0.54	8.8	0.24	7.24	1.7															Area Inlet Piped to DP 14.1
	11.1								17.1	1.22	5.58	6.8				6.8	1.22	1.0	18	357	6.3	0.9	Piped to DP 14.1
	12	B2	4.33	0.67	12.2	2.90	6.43	18.7								18.7	2.90	1.0	18	38	10.6	0.1	Sump Inlet Piped to DP 13.1
	13	B1	2.44	0.75	11.4	1.82	6.60	12.0															Sump Inlet Piped to DP 13.1
	13.1								12.3	4.72	6.42	30.3				30.3	4.72	1.0	24	125	9.7	0.2	Piped to DP 14.1
	14	B6	0.78	0.51	18.5	0.40	5.38	2.2															Area Inlet Piped to DP 14.1
	14.1								18.5	6.34	5.38	34.1				34.1	6.34	1.0	24	415	10.9	0.6	Piped to DP 15.1
	15	A1	4.31	0.64	12.5	2.74	6.37	17.4					5.0	0.7854	1.6	12.4	1.95			230	2.5	1.5	On-grade Inlet Captured Flows piped to DP 15.1, Bypass flow to DP 17

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	15.1								19.1	8.29	5.30	43.9				43.9	8.29	1.0	24	48	14.0	0.1	Captured Flows piped to DP 16.1

STANDARD FORM SF-3 - PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision -Proposed
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 2
Project No.: 25188.02
Calculated By: CJD
Checked By:
Date: 4/27/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	16	A5	0.45	0.73	5.0	0.33	8.66	2.9					0.0	0	2.9	2.9							On-grade Inlet Captured Flows piped to DP 16.1
	16.1								19.2	8.62	5.29	45.6				45.6	8.62	1.0	24	280	14.5	0.3	Piped to DP 18.1
	17	A2	1.41	0.50	16.3	0.71	5.70	4.0	16.3	1.50	5.70	8.5	0.9	0.1579	1.5	8.5	0.71	1.0	24	27	6.7	0.1	On-grade Inlet Piped to DP 18.1
	18.1								19.5	10.12	5.25	53.1				53.1	10.12	1.0	30	600	10.8	0.9	Piped to DP20.1
	19	A6	7.60	0.68	13.9	5.14	6.11	31.4					15.6	2.5552	1.0	15.8	2.58	1.0	18	30	8.9	0.1	On-grade Inlet Captured Flows piped to DP 20.1, Bypass flow to DP 21
	20	A3	3.68	0.64	13.2	2.34	6.24	14.6					3.0	0.4809	1.0	11.6	1.86	1.0	18	4	6.6	0.0	On-grade Inlet Captured Flows piped to DP 20.1, Bypass flow to DP 22
	20.1								20.4	14.56	5.13	74.7				74.7	14.56	1.0	36	220	10.6	0.3	Piped to DP23
	21	A7	1.43	0.70	13.7	1.00	6.14	6.1	13.9	3.56	6.10	21.7				21.7	3.56	1.0	18	60	12.3	0.1	Sump Inlet Piped to DP21.1
	21.1								20.4	18.12	5.13	93.0				93.0	18.12	1.0	42	90	11.9	0.1	MH Piped to DP23
	22	A4	3.94	0.58	15.4	2.30	5.84	13.4	15.4	2.94	5.84	17.2											Piped to Piped to DP22.1
	22.1								15.4	2.94	5.84	17.2				17.2	2.94	1.0	24	10	7.9	0.0	Piped to DP23
	23								20.8	21.06	5.09	107.2				107.2	21.06	1.0	42	145	11.8	0.2	Piped to DP26
	24	A8	4.22	0.41	18.9	1.71	5.32	9.1															Area Inlet Piped to EX 84" Storm Line Built w/ SR Filing 2 First Phase
	25	A9	2.02	0.39	25.8	0.78	4.55	3.5								3.5	0.78	1.0	18	30	5.4	0.1	EX FES Piped to EX 84" Storm Line Built w/ SR Filing 2 First Phase
	27	A10	3.23	0.45	17.6	1.45	5.50	8.0															Pervious area sheet flows into EX Pond W5
	28	D1	0.42	0.42	9.2	0.18	7.14	1.3															Pervious area sheet flows into Sand Creek
	29	D2	3.67	0.38	7.8	1.39	7.55	10.5															Pervious area sheet flows into Sand Creek

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

Appendix C
Hydraulic Calculations

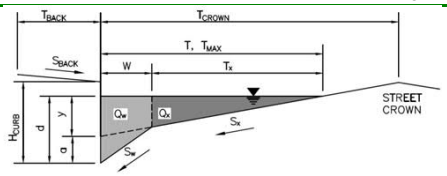
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A1 - DP15



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.033	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	7.8	inches
Allow Flow Depth at Street Crown (leave blank for no)				check = yes

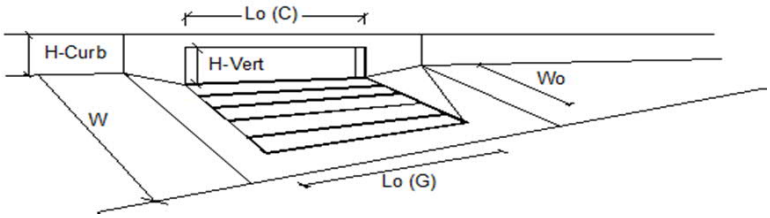
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**21.2**	**24.3**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.8**	**12.4**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.3**	**5.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**96**	**71**	**%**

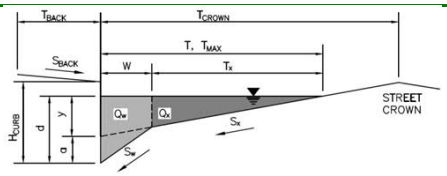
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A2 - DP17



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.8	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.042	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.026	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.8	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.6	7.8	inches
Allow Flow Depth at Street Crown (leave blank for no)				check = yes

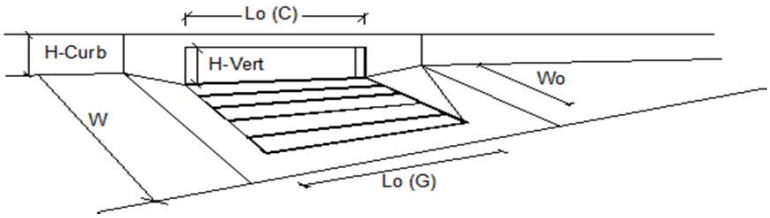
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**13.3**	**49.1**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**2.0**	**7.6**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**90**	**%**

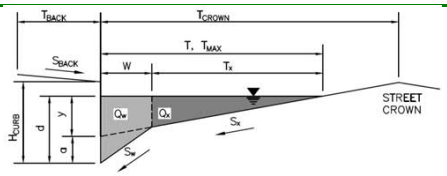
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A4 - DP22



Gutter Geometry (Enter data in the blue cells)

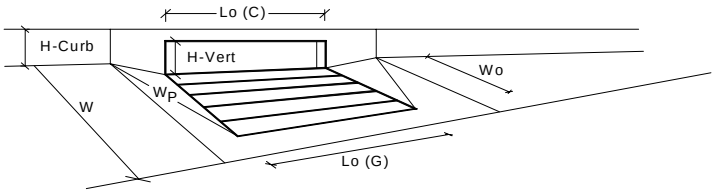
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.8	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.6	7.8	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.6	8.0	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	1.17	1.17	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.29	0.57	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.43	0.75	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.69	0.89	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.2**	**22.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.5	15.9	cfs

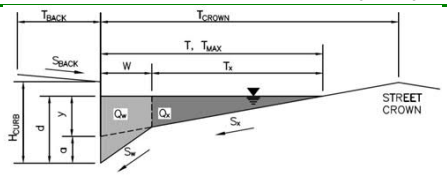
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A6 - DP19



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.8	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.2	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.042	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.010	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.8	16.2	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.6	7.8	inches
Allow Flow Depth at Street Crown (leave blank for no)				check = yes

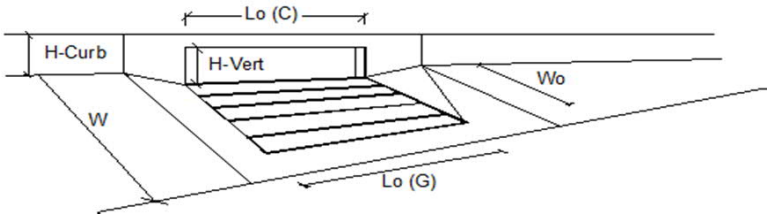
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**8.3**	**8.7**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than the design flow given on sheet 'Inlet Management'

WARNING: MAJOR STORM max. allowable capacity is less than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR & MAJOR STORM		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**10.8**	**15.8**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**4.5**	**15.6**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**71**	**50**	**%**

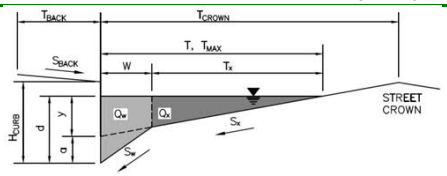
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A5 - DP16



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.029	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.8	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.6	7.8	inches
Allow Flow Depth at Street Crown (leave blank for no)				check = yes

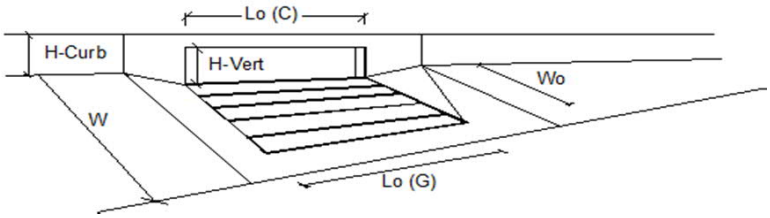
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**13.6**	**40.2**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**1.4**	**2.9**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**100**	**%**

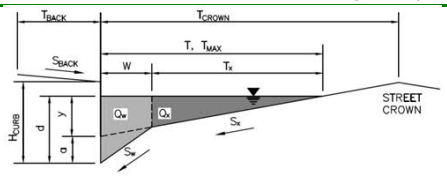
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A3 - DP 20



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.007	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	19.3	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	7.7	inches
Allow Flow Depth at Street Crown (leave blank for no)				check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

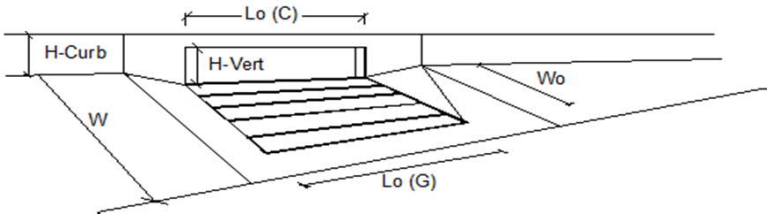
	Minor Storm	Major Storm	
Q_{allow} =	**11.5**	**26.7**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**6.8**	**11.6**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**3.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**79**	**%**

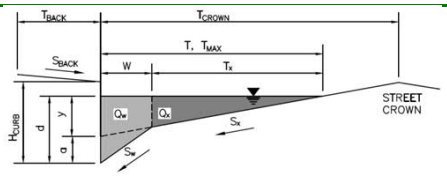
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: A7 - DP21



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	15.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

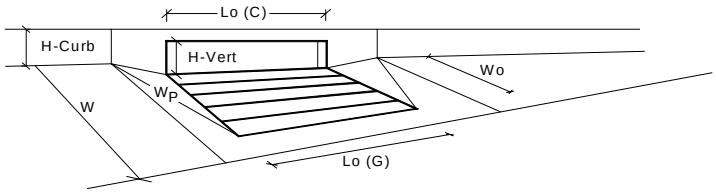
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.8	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.6	12.0	inches

Check boxes are not applicable in SUMP conditions

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	1.17	1.17	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.40	0.90	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.57	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.79	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**11.9**	**39.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	7.5	21.7	cfs

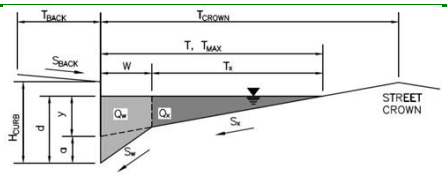
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: B1- DP12



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

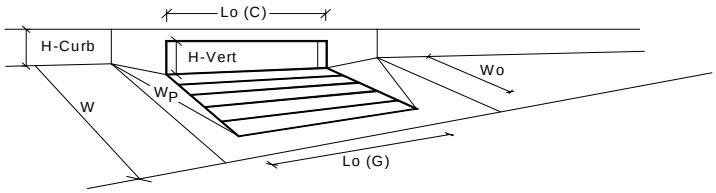
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	12.0	inches

Check boxes are not applicable in SUMP conditions

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.0	12.0	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	1.17	1.17	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.32	0.90	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.47	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.72	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**7.5**	**39.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	6.2	12.0	cfs

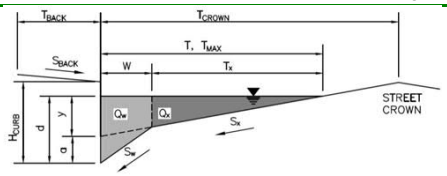
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: B2 - DP13



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	1.17	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

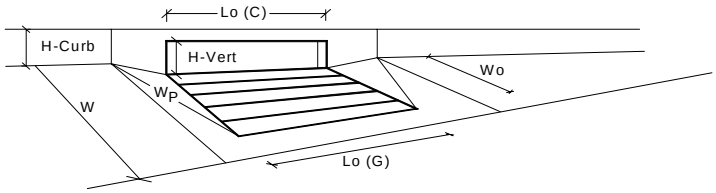
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	12.0	inches

Check boxes are not applicable in SUMP conditions

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.6	12.0	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	20.00	20.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	1.17	1.17	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.37	0.90	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.53	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.76	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**13.1**	**52.7**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	9.1	18.7	cfs

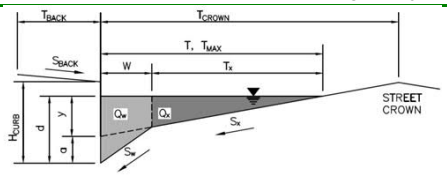
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: C1 - DP 6



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

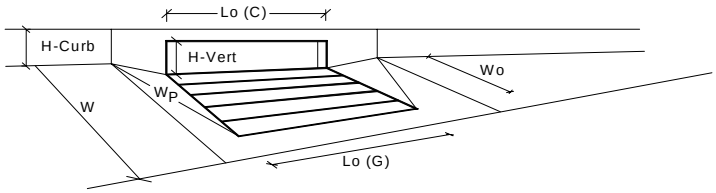
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	19.3	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	7.7	inches

Check boxes are not applicable in SUMP conditions

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	7.7	inches
Grate Information		MINOR	MAJOR	Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.33	0.48	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.57	0.73	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.79	0.88	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**18.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.4	12.3	cfs

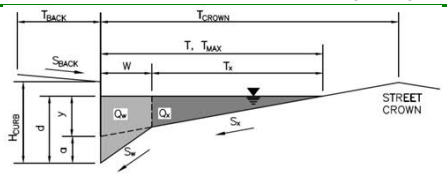
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: C2 - DP5



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	9.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

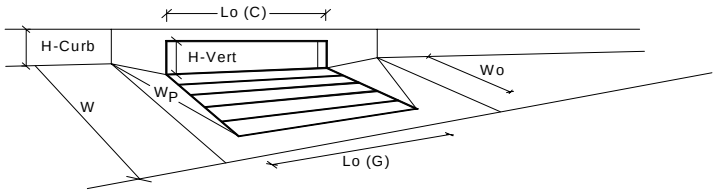
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	19.3	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	7.7	inches

Check boxes are not applicable in SUMP conditions

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	3	3	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	8.0	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.33	0.50	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.57	0.75	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.79	0.89	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**13.5**	**27.9**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	12.0	25.9	cfs

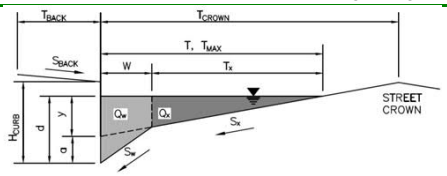
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: C4 - DP8



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	30.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	30.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☐	check = yes

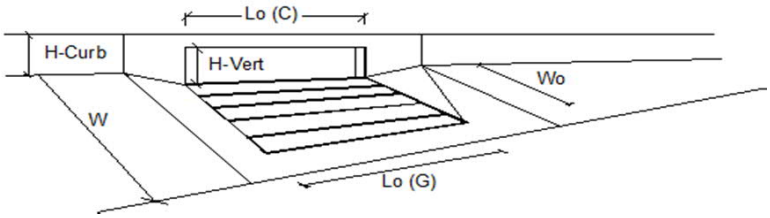
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**9.8**	**16.9**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**4.6**	**9.4**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.8**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**92**	**%**

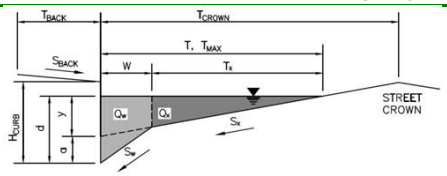
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Phase 2

Inlet ID: B3 - DP9



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.016	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	30.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	30.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☐	check = yes

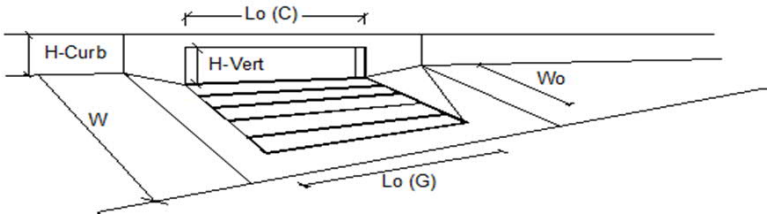
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**9.8**	**16.9**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**3.5**	**7.3**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**100**	**%**

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Apr 27 2021

Barbarick FSD Overflow Channel

Trapezoidal

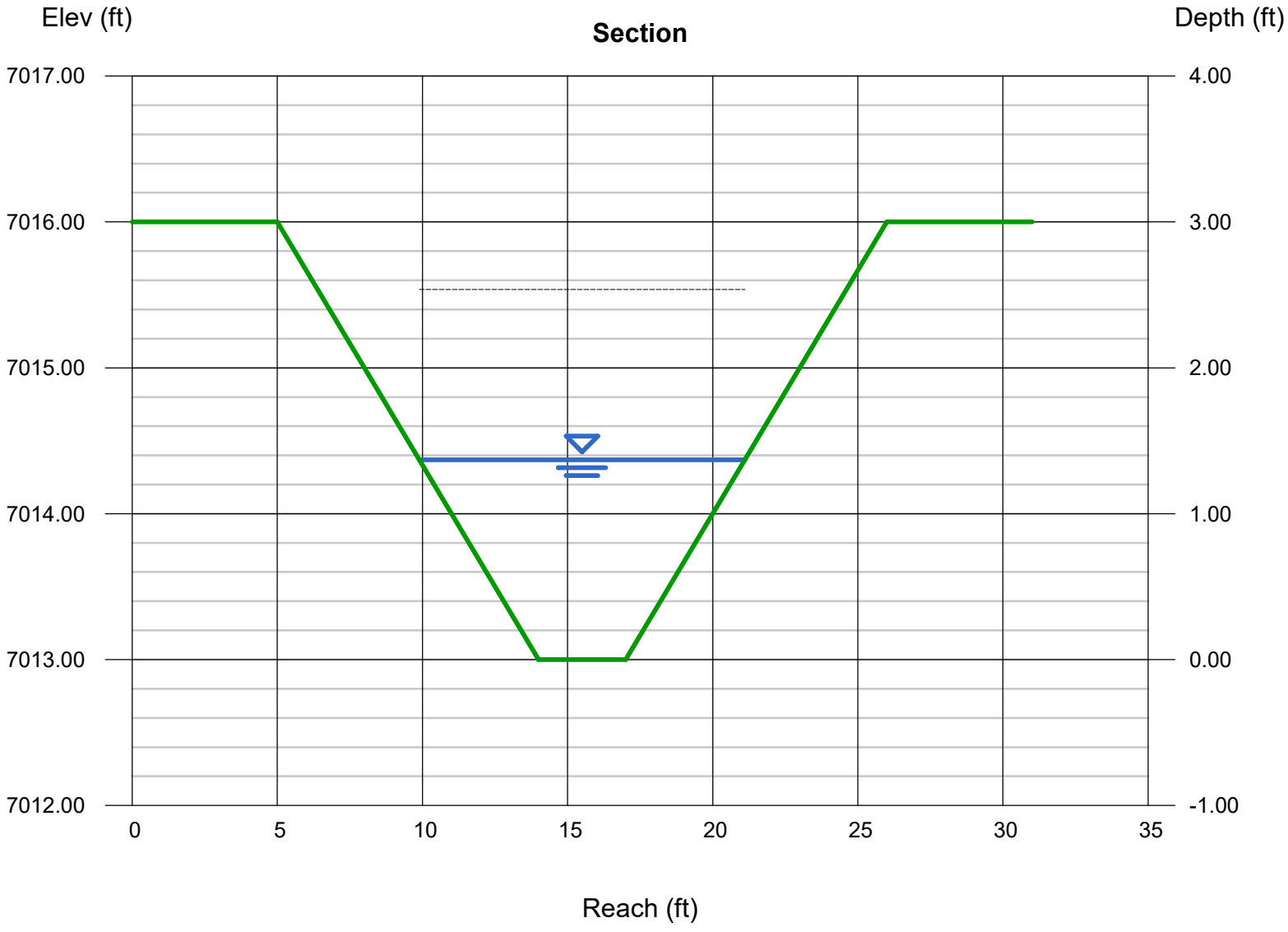
Bottom Width (ft)	= 3.00
Side Slopes (z:1)	= 3.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 7013.00
Slope (%)	= 0.75
N-Value	= 0.013

Calculations

Compute by:	Known Q
Known Q (cfs)	= 84.40

Highlighted

Depth (ft)	= 1.37
Q (cfs)	= 84.40
Area (sqft)	= 9.74
Velocity (ft/s)	= 8.66
Wetted Perim (ft)	= 11.66
Crit Depth, Yc (ft)	= 1.75
Top Width (ft)	= 11.22
EGL (ft)	= 2.54



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Apr 27 2021

Interim Channel - DP I1

Triangular

Side Slopes (z:1)	= 4.00, 4.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 6970.00
Slope (%)	= 0.88
N-Value	= 0.025

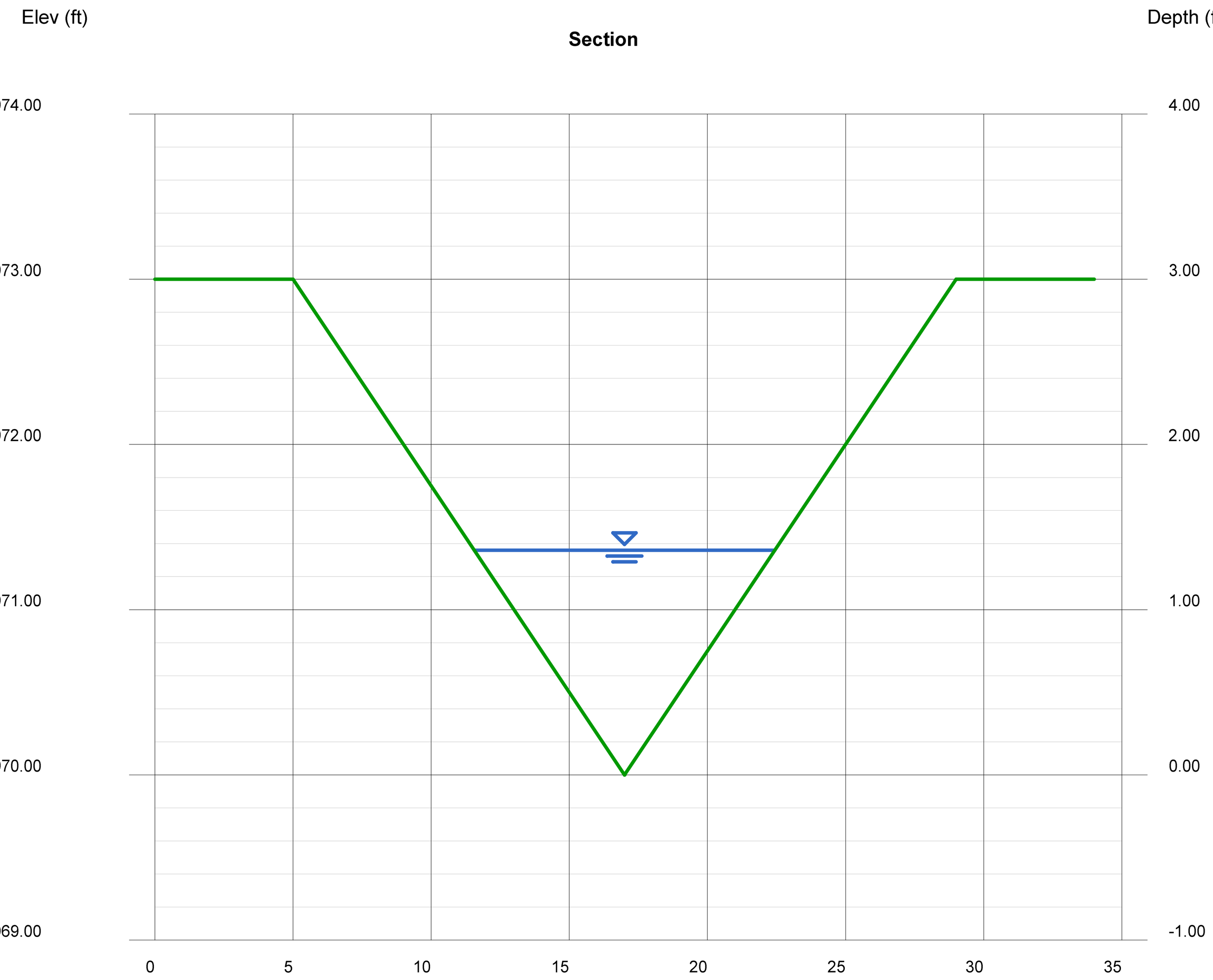
Calculations

Compute by:	Known Q
Known Q (cfs)	= 31.20

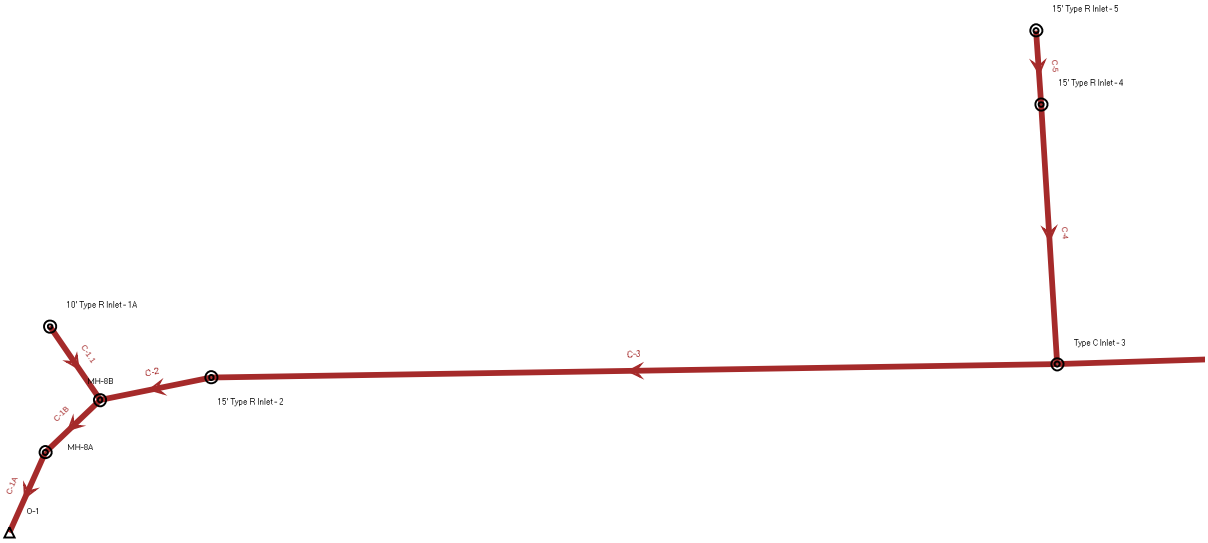
Highlighted

Depth (ft)	= 1.36
Q (cfs)	= 31.20
Area (sqft)	= 7.40
Velocity (ft/s)	= 4.22
Wetted Perim (ft)	= 11.21
Crit Depth, Yc (ft)	= 1.31
Top Width (ft)	= 10.88
EGL (ft)	= 1.64

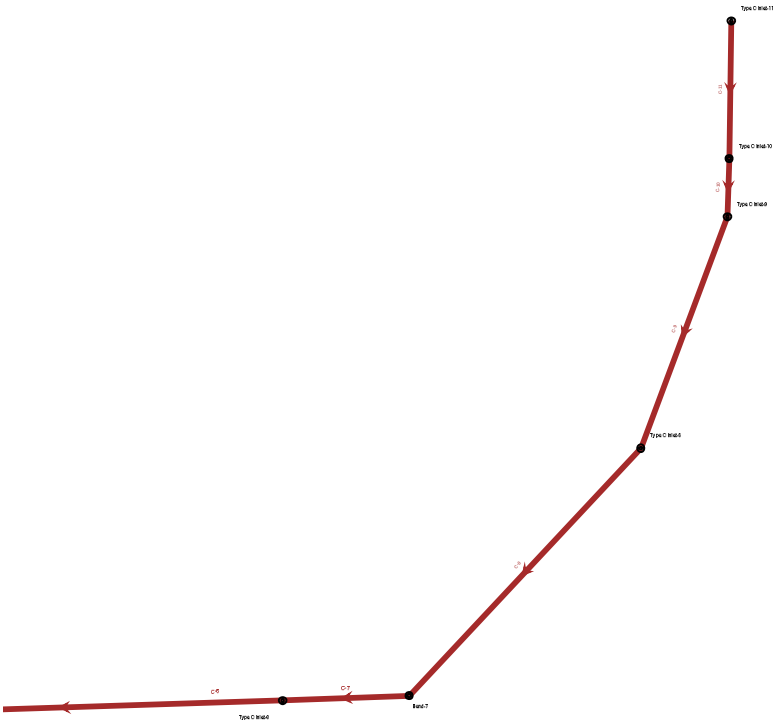
Section



Scenario: **Interim**



Scenario: **Interim**



Scenario: 100 YR
Current Time Step: 0.000 h
FlexTable: Conduit Table

Label	Flow (cfs)	Diameter (in)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
C-4	30.30	24.0	126.0	0.019	0.013	11.21	30.89	7,002.18	6,999.58
C-5	18.70	18.0	35.5	0.010	0.013	10.58	10.47	7,004.07	7,002.95
C-1B	45.60	30.0	52.3	0.003	0.013	9.29	23.38	6,991.86	6,991.22
C-1A	45.60	30.0	33.9	0.003	0.013	9.29	22.29	6,990.54	6,989.98
C-2	43.90	30.0	-	0.002	0.013	8.94	20.47	6,993.18	6,992.53
C-11	5.60	18.0	109.8	0.026	0.013	8.55	16.81	7,014.07	7,011.35
C-10	5.60	18.0	45.0	0.016	0.013	7.23	13.38	7,011.16	7,010.20
C-9	5.60	18.0	199.1	0.015	0.013	7.01	12.82	7,005.39	7,002.21
C-3	34.10	30.0	416.6	0.003	0.013	6.95	22.46	6,996.68	6,993.80
C-6	6.80	18.0	355.0	0.003	0.013	3.85	5.77	6,998.84	6,997.35
C-8	5.60	18.0	275.2	0.010	0.013	3.17	10.65	7,000.12	6,999.34
C-7	5.60	18.0	101.9	0.010	0.013	3.17	10.65	6,999.25	6,998.96
C-1.1	2.90	18.0	17.6	0.306	0.013	1.64	58.12	6,995.21	6,994.99

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Scenario: 5 YR
Current Time Step: 0.000 h
FlexTable: Conduit Table

Label	Flow (cfs)	Diameter (in)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
C-4	15.00	24.0	126.0	0.019	0.013	9.76	30.89	7,001.71	6,998.95
C-5	9.10	18.0	35.5	0.010	0.013	6.67	10.47	7,002.43	7,001.99
C-11	2.30	18.0	109.8	0.026	0.013	6.66	16.81	7,013.73	7,010.92
C-10	2.30	18.0	45.0	0.016	0.013	5.67	13.38	7,010.82	7,009.93
C-9	2.30	18.0	199.1	0.015	0.013	5.49	12.82	7,005.05	7,001.94
C-1B	23.20	30.0	52.3	0.003	0.013	5.43	23.38	6,990.17	6,990.03
C-1A	23.20	30.0	33.9	0.003	0.013	5.16	22.29	6,989.77	6,989.62
C-3	16.10	30.0	416.6	0.003	0.013	4.98	22.46	6,992.04	6,991.21
C-7	2.30	18.0	101.9	0.010	0.013	4.81	10.65	6,994.26	6,993.35
C-8	2.30	18.0	275.2	0.010	0.013	4.81	10.65	6,997.09	6,994.38
C-2	21.80	30.0	-	0.002	0.013	4.69	20.47	6,990.98	6,990.55
C-6	2.80	18.0	355.0	0.003	0.013	3.24	5.77	6,993.27	6,992.38
C-1.1	1.40	18.0	17.6	0.306	0.013	0.79	58.12	6,994.94	6,994.78

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Scenario: 5 YR
Current Time Step: 0.000 h
FlexTable: Manhole Table

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Headloss Coefficient (Standard)
10' Type R Inlet - 1A	6,994.34	6,987.63	1.40	6,994.35	6,994.34	6,994.36	6,994.35	0.600
15' Type R Inlet - 2	6,995.37	6,988.86	21.80	6,991.21	6,990.98	6,991.44	6,991.45	0.500
15' Type R Inlet - 4	7,005.49	7,000.06	15.00	7,002.03	7,001.71	7,002.72	7,002.35	0.500
15' Type R Inlet - 5	7,005.49	7,001.05	9.10	7,002.73	7,002.43	7,003.32	7,003.02	0.500
BEND - 7	7,003.48	6,993.68	2.30	6,994.38	6,994.26	6,994.51	6,994.47	0.600
MH-8A	6,992.67	6,987.74	23.20	6,990.03	6,989.77	6,990.43	6,990.28	0.500
MH-8B	6,994.06	6,988.01	23.20	6,990.38	6,990.17	6,991.06	6,990.58	0.500
Type C Inlet - 3	7,001.33	6,990.21	16.10	6,992.38	6,992.04	6,992.47	6,992.42	0.900
Type C Inlet - 6	7,000.51	6,992.53	2.80	6,993.35	6,993.27	6,993.47	6,993.43	0.500
Type C Inlet - 8	7,004.33	6,996.51	2.30	6,997.22	6,997.09	6,997.68	6,997.30	0.600
Type C Inlet - 9	7,013.09	7,004.48	2.30	7,005.18	7,005.05	7,005.68	7,005.27	0.600
Type C Inlet - 10	7,013.34	7,010.24	2.30	7,010.92	7,010.82	7,011.14	7,011.03	0.500
Type C Inlet - 11	7,015.87	7,013.16	2.30	7,013.84	7,013.73	7,014.05	7,013.95	0.500

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Scenario: 100 YR
Current Time Step: 0.000 h
FlexTable: Manhole Table

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Headloss Coefficient (Standard)
10' Type R Inlet - 1A	6,994.34	6,987.63	2.90	6,994.37	6,994.34	6,994.41	6,994.38	0.600
15' Type R Inlet - 2	6,995.37	6,988.86	43.90	6,993.80	6,993.18	6,994.55	6,994.42	0.500
15' Type R Inlet - 4	7,005.49	7,000.06	30.30	7,002.95	7,002.18	7,004.69	7,003.71	0.500
15' Type R Inlet - 5	7,005.49	7,001.05	18.70	7,004.94	7,004.07	7,006.68	7,005.81	0.500
BEND - 7	7,003.48	6,993.68	5.60	6,999.34	6,999.25	6,999.49	6,999.40	0.600
MH-8A	6,992.67	6,987.74	45.60	6,991.22	6,990.54	6,992.56	6,991.89	0.500
MH-8B	6,994.06	6,988.01	45.60	6,992.53	6,991.86	6,993.78	6,993.20	0.500
Type C Inlet - 3	7,001.33	6,990.21	34.10	6,997.35	6,996.68	6,997.58	6,997.43	0.900
Type C Inlet - 6	7,000.51	6,992.53	6.80	6,998.96	6,998.84	6,999.11	6,999.07	0.500
Type C Inlet - 8	7,004.33	6,996.51	5.60	7,000.21	7,000.12	7,000.98	7,000.28	0.600
Type C Inlet - 9	7,013.09	7,004.48	5.60	7,005.62	7,005.39	7,006.40	7,005.78	0.600
Type C Inlet - 10	7,013.34	7,010.24	5.60	7,011.35	7,011.16	7,011.66	7,011.54	0.500
Type C Inlet - 11	7,015.87	7,013.16	5.60	7,014.26	7,014.07	7,014.65	7,014.46	0.500

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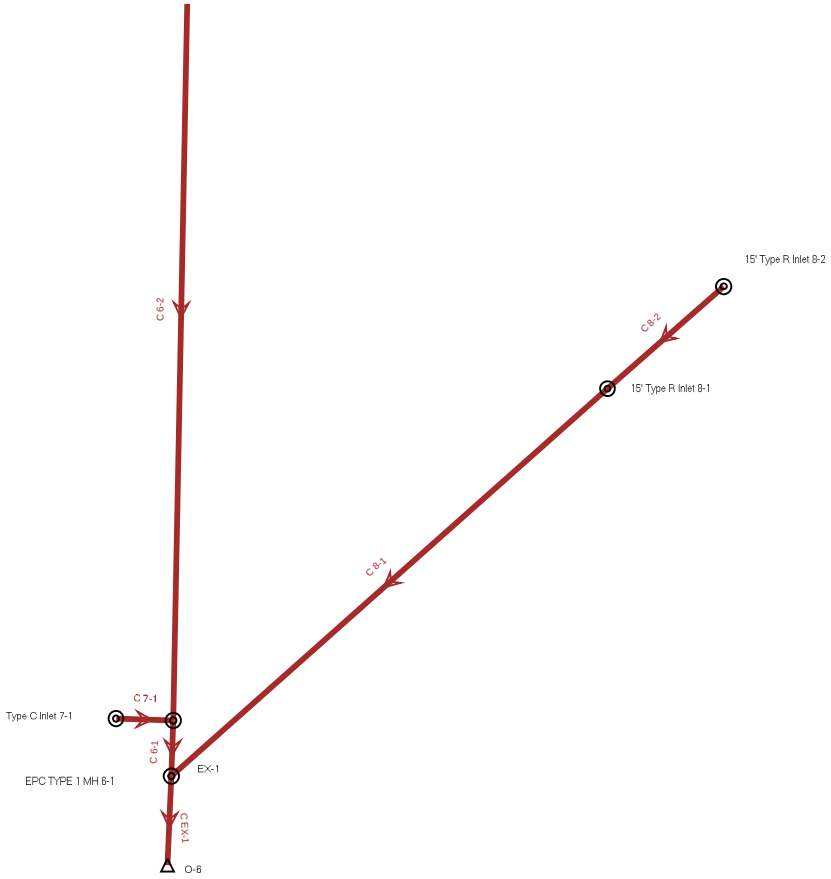
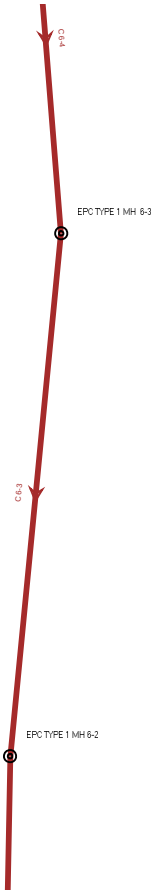
Scenario: **Final**



Scenario: **Final**



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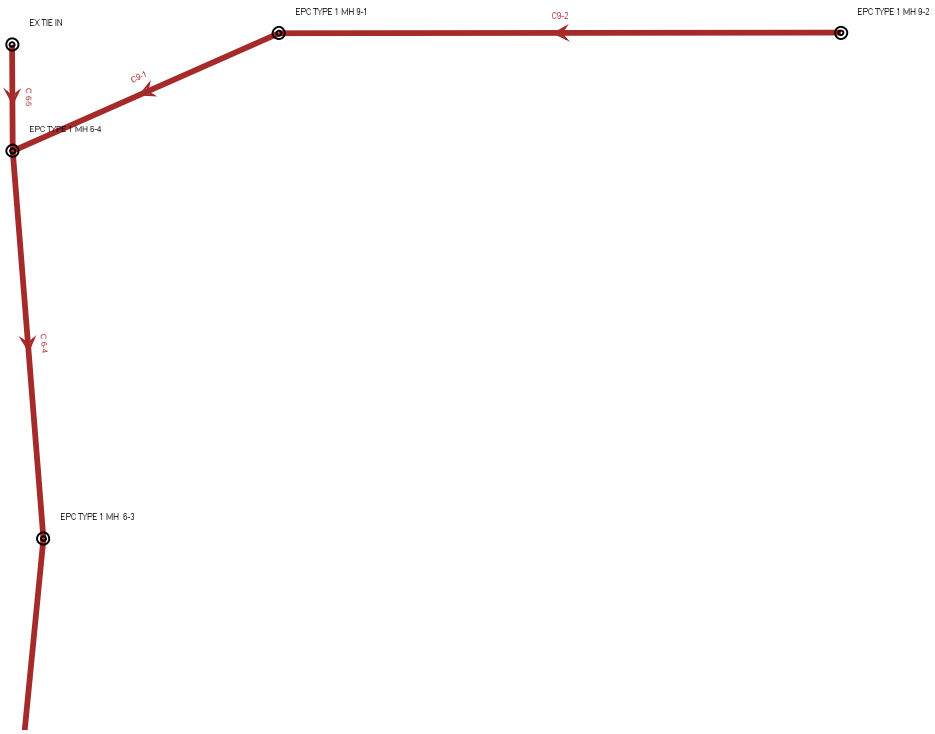

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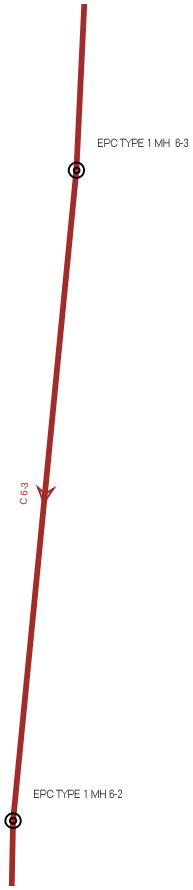

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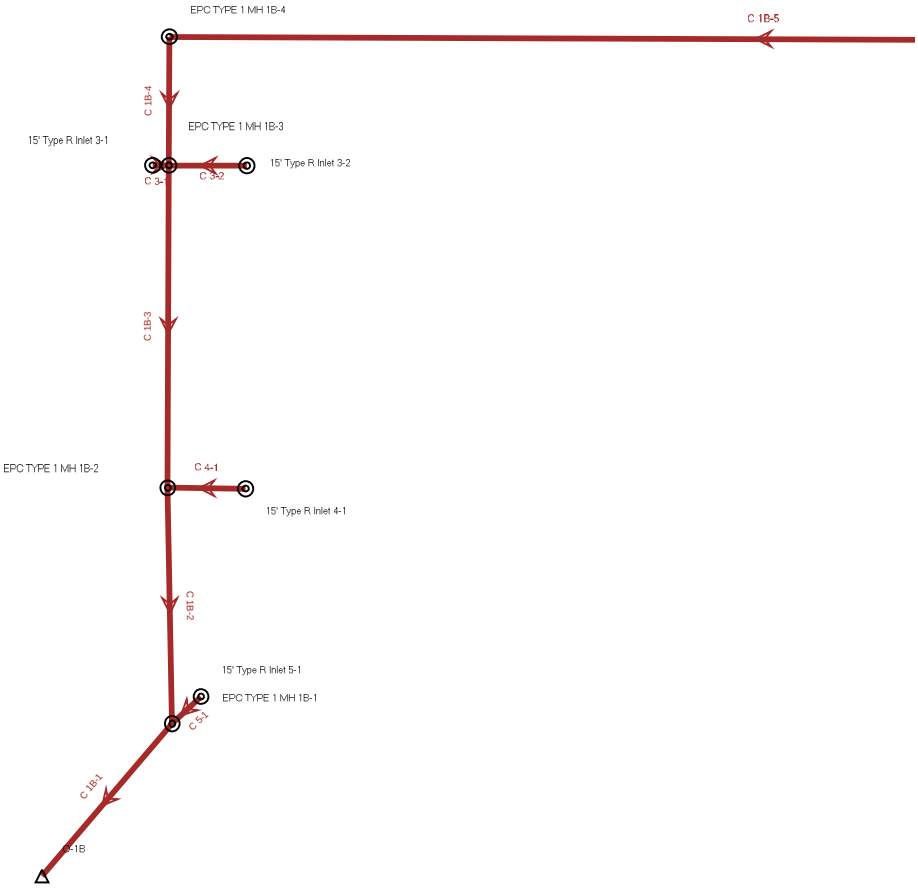

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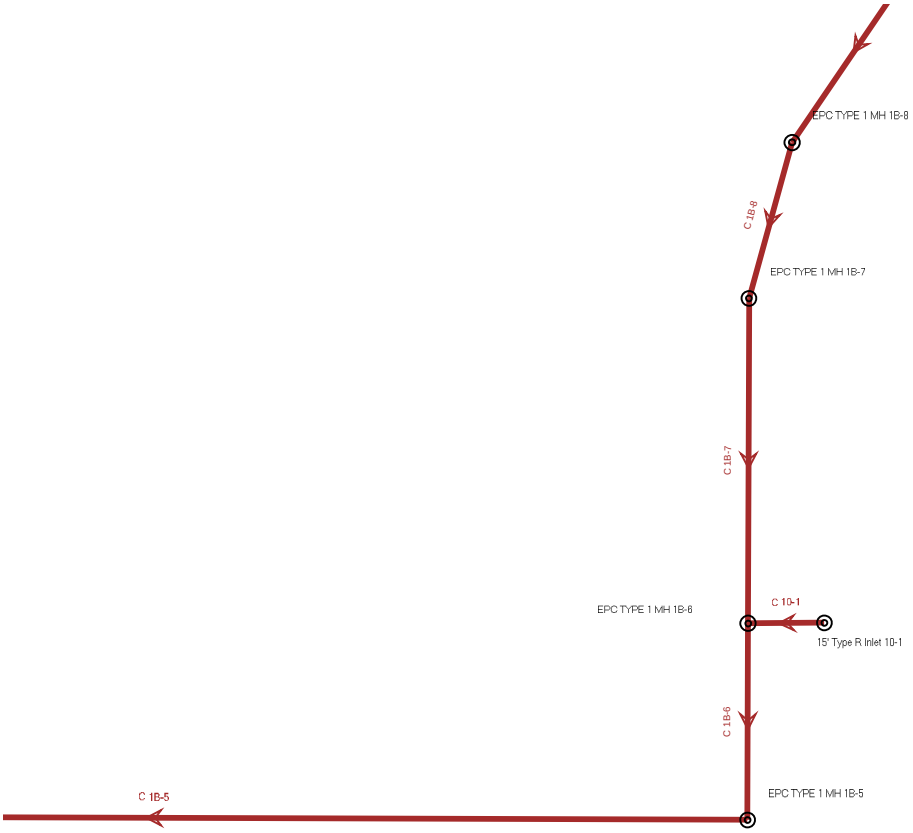

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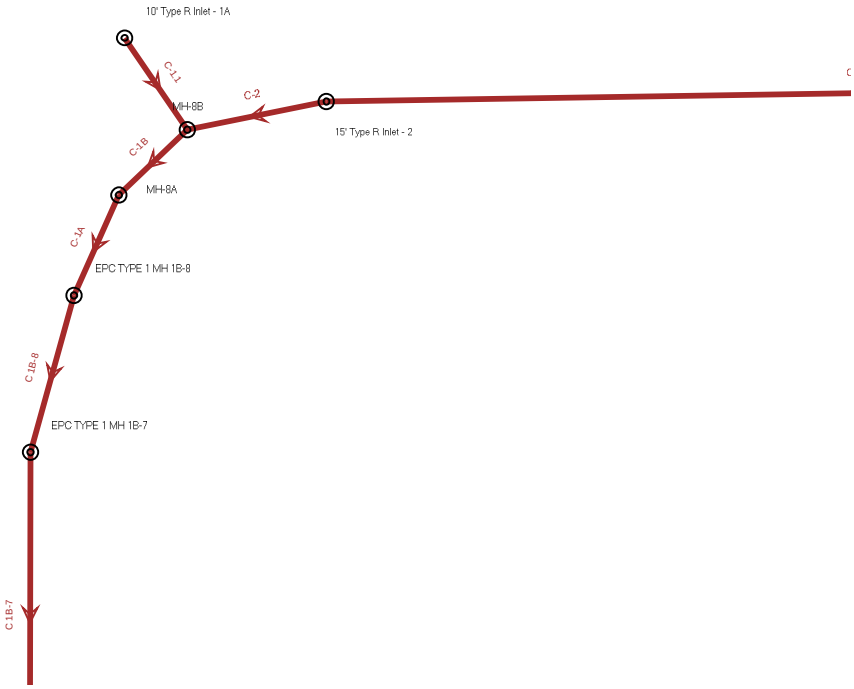

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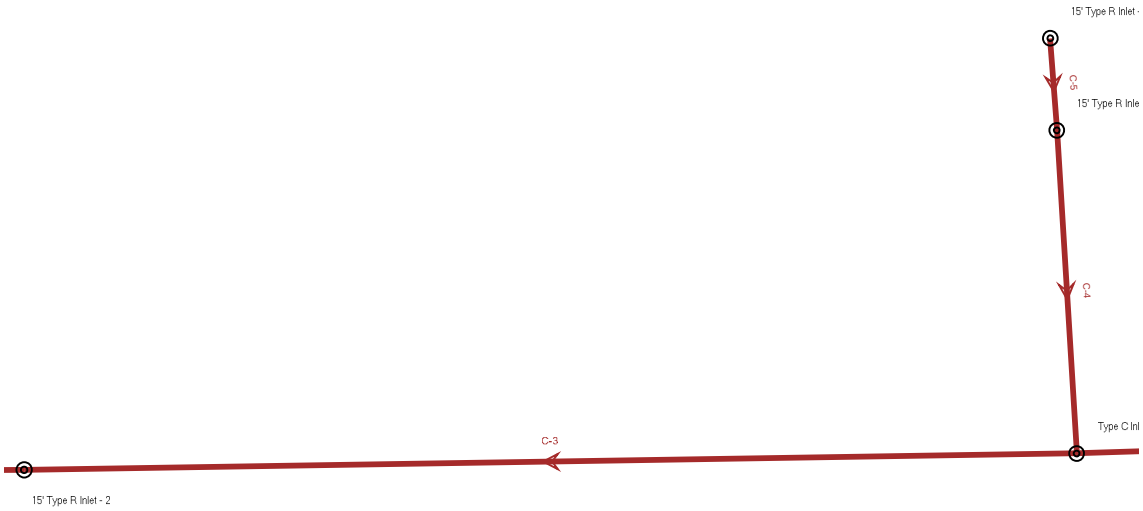

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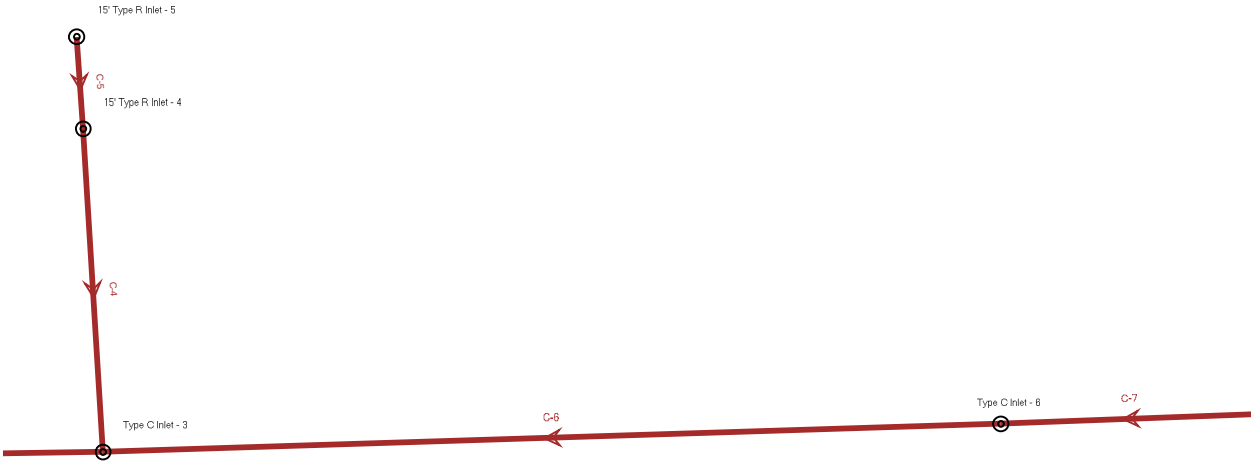

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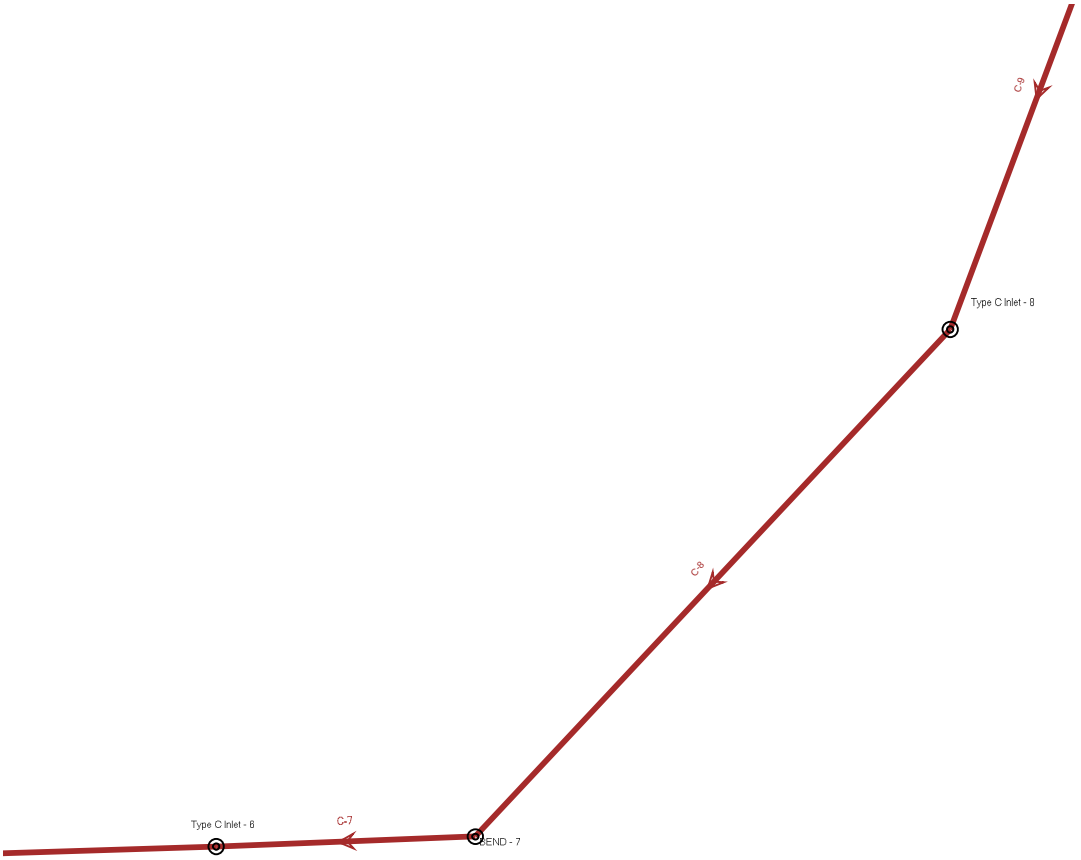

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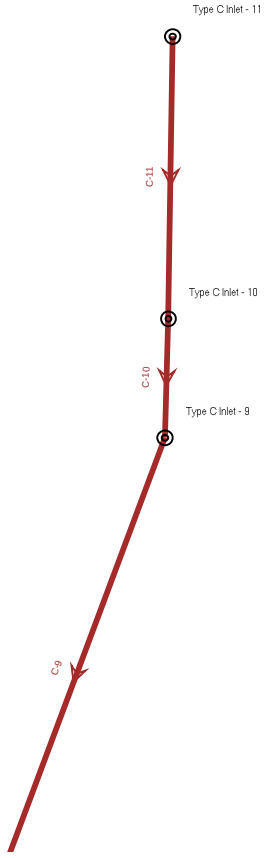

Scenario: 100 YR
Current Time Step: 0.000 h
FlexTable: Conduit Table

Label	Flow (cfs)	Diameter (in)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
C EX-1	132.10	48.0	12.0	0.084	0.013	29.46	417.45	6,990.90	6,989.01
C 6-2	104.30	48.0	321.4	0.027	0.013	18.19	235.90	7,001.85	6,993.49
C 3-2	31.40	18.0	27.0	0.026	0.013	17.77	17.07	6,980.64	6,978.23
C 6-3	104.30	48.0	254.8	0.023	0.013	17.14	217.68	7,009.11	7,002.15
C 6-5	72.20	24.0	50.1	0.043	0.013	16.54	94.29	7,020.58	7,017.93
C 6-4	104.30	48.0	187.2	0.019	0.013	15.86	196.39	7,014.30	7,009.86
C 5-1	21.70	30.0	9.2	0.044	0.013	14.63	86.21	6,975.76	6,974.95
C 1B-6	53.10	36.0	76.1	0.023	0.013	14.57	101.94	6,986.60	6,985.69
C 1B-5	53.10	36.0	472.8	0.018	0.013	13.27	90.16	6,984.71	6,978.74
C-4	30.30	24.0	126.0	0.019	0.013	11.21	30.89	7,002.18	6,999.58
C 1B-1	107.20	42.0	74.8	0.004	0.013	11.14	64.87	6,974.16	6,973.11
C 8-1	36.40	36.0	-	0.014	0.013	10.92	78.78	6,995.05	6,992.55
C 4-1	21.70	24.0	27.0	0.019	0.013	10.62	30.80	6,977.60	6,976.80
C-5	18.70	18.0	35.5	0.010	0.013	10.58	10.47	7,004.07	7,002.95
C 10-1	8.50	24.0	26.3	0.029	0.013	9.83	38.44	6,987.25	6,987.46
C 1B-2	93.00	42.0	92.3	0.005	0.013	9.67	71.02	6,976.11	6,975.32
C 1B-8	45.60	30.0	64.4	0.013	0.013	9.29	47.57	6,990.50	6,989.70
C 1B-7	45.60	30.0	127.2	0.011	0.013	9.29	43.22	6,989.03	6,987.46
C-1B	45.60	30.0	52.3	0.003	0.013	9.29	23.38	6,992.90	6,992.26
C-1A	45.60	30.0	33.9	0.003	0.013	9.29	22.29	6,991.59	6,991.17
C-2	43.90	30.0	-	0.002	0.013	8.94	20.47	6,994.22	6,993.57
C 8-2	25.90	24.0	60.8	0.012	0.013	8.82	24.52	7,000.30	6,999.57
C 6-1	109.80	48.0	39.7	0.022	0.013	8.74	212.95	6,992.78	6,992.55
C-11	5.60	18.0	109.8	0.026	0.013	8.55	16.81	7,014.07	7,011.35
C9-1	60.40	36.0	109.4	0.006	0.013	8.54	51.40	7,016.13	7,015.24
C9-2	60.40	36.0	307.3	0.005	0.013	8.54	47.06	7,019.22	7,016.70
C 3-1	14.60	18.0	2.5	0.112	0.013	8.26	35.12	6,978.40	6,978.28
C 1B-3	74.70	42.0	124.3	0.006	0.013	7.76	76.58	6,977.66	6,976.98
C-10	5.60	18.0	45.0	0.016	0.013	7.23	13.38	7,011.16	7,010.20
C-9	5.60	18.0	199.1	0.015	0.013	7.01	12.82	7,005.39	7,002.21
C-3	34.10	30.0	416.6	0.003	0.013	6.95	22.46	6,997.72	6,994.84
C-8	5.60	18.0	275.2	0.010	0.013	6.10	10.65	6,997.43	6,995.39
C 7-1	9.90	18.0	18.9	0.034	0.013	5.60	19.29	6,993.66	6,993.49
C 1B-4	53.10	42.0	46.8	0.005	0.013	5.52	70.50	6,978.36	6,978.23
C-6	6.80	18.0	355.0	0.003	0.013	3.85	5.77	6,999.88	6,998.39
C-7	5.60	18.0	101.9	0.010	0.013	3.17	10.65	7,000.29	7,000.00
C-1.1	2.90	18.0	17.6	0.306	0.013	1.64	58.12	6,995.21	6,994.99

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Scenario: 100 YR
Current Time Step: 0.000 h
FlexTable: Manhole Table

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Headloss Coefficient (Standard)
10' Type R Inlet - 1A	6,994.34	6,987.63	2.90	6,994.37	6,994.34	6,994.41	6,994.38	0.600
15' Type R Inlet - 2	6,995.37	6,988.86	43.90	6,994.84	6,994.22	6,995.59	6,995.46	0.500
15' Type R Inlet - 4	7,005.49	7,000.06	30.30	7,002.95	7,002.18	7,004.69	7,003.71	0.500
15' Type R Inlet - 5	7,005.49	7,001.05	18.70	7,004.94	7,004.07	7,006.68	7,005.81	0.500
15' Type R Inlet 10-1	6,990.17	6,986.21	8.50	6,987.46	6,987.25	6,987.87	6,987.66	0.500
15' Type R Inlet 3-1	6,980.90	6,976.89	14.60	6,978.93	6,978.40	6,979.99	6,979.46	0.500
15' Type R Inlet 3-2	6,980.84	6,977.18	31.40	6,983.10	6,980.64	6,988.00	6,985.55	0.500
15' Type R Inlet 4-1	6,979.81	6,975.94	21.70	6,978.07	6,977.60	6,979.01	6,978.54	0.500
15' Type R Inlet 5-1	6,979.58	6,974.17	21.70	6,976.10	6,975.76	6,976.78	6,976.44	0.500
15' Type R Inlet 8-1	7,003.00	6,993.10	36.40	6,995.48	6,995.05	6,996.69	6,995.91	0.500
15' Type R Inlet 8-2	7,001.78	6,998.51	25.90	7,000.89	7,000.30	7,002.08	7,001.49	0.500
BEND - 7	7,003.48	6,993.68	5.60	6,995.49	6,995.39	6,995.64	6,995.55	0.600
EPC TYPE 1 MH 6-3	7,015.59	7,006.01	104.30	7,009.88	7,009.11	7,013.44	7,010.66	0.500
EPC TYPE 1 MH 1B-1	6,979.57	6,970.28	107.20	6,975.32	6,974.16	6,976.71	6,976.09	0.600
EPC TYPE 1 MH 1B-2	6,979.46	6,970.76	93.00	6,976.98	6,976.11	6,978.39	6,977.56	0.600
EPC TYPE 1 MH 1B-3	6,980.54	6,971.48	74.70	6,978.23	6,977.66	6,978.70	6,978.60	0.600
EPC TYPE 1 MH 1B-4	6,981.10	6,973.20	53.10	6,978.74	6,978.36	6,979.61	6,978.83	0.800
EPC TYPE 1 MH 1B-5	6,988.71	6,982.35	53.10	6,985.69	6,984.71	6,986.56	6,985.93	0.800
EPC TYPE 1 MH 1B-6	6,989.81	6,984.23	53.10	6,987.46	6,986.60	6,987.57	6,987.83	0.700
EPC TYPE 1 MH 1B-7	6,991.56	6,986.14	45.60	6,989.70	6,989.03	6,991.04	6,990.37	0.500
EPC TYPE 1 MH 1B-8	6,992.27	6,987.64	45.60	6,991.17	6,990.50	6,992.51	6,991.84	0.500
EPC TYPE 1 MH 6-1	6,996.65	6,988.33	109.80	6,993.49	6,992.78	6,994.79	6,993.97	0.600
EPC TYPE 1 MH 6-2	7,008.21	6,998.76	104.30	7,002.63	7,001.85	7,006.98	7,003.41	0.500
EPC TYPE 1 MH 6-4	7,021.96	7,011.27	104.30	7,015.24	7,014.30	7,016.37	7,015.86	0.600
EPC TYPE 1 MH 9-1	7,021.10	7,011.92	60.40	7,016.70	7,016.13	7,017.84	7,017.27	0.500
EPC TYPE 1 MH 9-2	7,020.22	7,013.55	60.40	7,020.13	7,019.22	7,021.26	7,020.36	0.800
EX TIE IN	7,022.00	7,018.65	72.20	7,021.63	7,020.58	7,023.73	7,022.68	0.500
EX-1	6,997.75	6,987.50	132.10	6,992.55	6,990.90	6,993.01	6,992.96	0.800
MH-8A	6,992.67	6,987.74	45.60	6,992.26	6,991.59	6,993.60	6,992.93	0.500
MH-8B	6,994.06	6,988.01	45.60	6,993.57	6,992.90	6,994.82	6,994.24	0.500
Type C Inlet - 3	7,001.33	6,990.21	34.10	6,998.39	6,997.72	6,998.62	6,998.47	0.900
Type C Inlet - 6	7,000.51	6,992.53	6.80	7,000.00	6,999.88	7,000.15	7,000.11	0.500
Type C Inlet - 8	7,004.33	6,996.51	5.60	6,997.66	6,997.43	6,998.42	6,997.81	0.600
Type C Inlet - 9	7,013.09	7,004.48	5.60	7,005.62	7,005.39	7,006.40	7,005.78	0.600
Type C Inlet - 10	7,013.34	7,010.24	5.60	7,011.35	7,011.16	7,011.66	7,011.54	0.500
Type C Inlet - 11	7,015.87	7,013.16	5.60	7,014.26	7,014.07	7,014.65	7,014.46	0.500
Type C Inlet 7-1	6,993.40	6,990.65	9.90	6,993.64	6,993.40	6,994.13	6,993.89	0.500

Scenario: 5 YR
Current Time Step: 0.000 h
FlexTable: Conduit Table

Label	Flow (cfs)	Diameter (in)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
C EX-1	69.00	48.0	12.0	0.084	0.013	24.57	417.45	6,989.98	6,988.19
C 6-1	52.00	48.0	39.7	0.022	0.013	13.99	212.95	6,990.51	6,990.84
C 6-5	35.40	24.0	50.1	0.043	0.013	13.94	94.29	7,020.17	7,017.40
C 6-2	35.40	48.0	321.4	0.027	0.013	13.51	235.90	7,000.53	6,991.14
C 6-3	35.40	48.0	254.8	0.023	0.013	12.75	217.68	7,007.78	7,001.25
C 6-4	35.40	48.0	187.2	0.019	0.013	11.85	196.39	7,012.98	7,008.86
C 1B-6	24.20	36.0	76.1	0.023	0.013	11.81	101.94	6,985.82	6,984.43
C 3-2	15.30	18.0	27.0	0.026	0.013	10.93	17.07	6,978.59	6,977.66
C 1B-5	24.20	36.0	472.8	0.018	0.013	10.81	90.16	6,983.93	6,974.76
C 5-1	5.70	30.0	9.2	0.044	0.013	9.95	86.21	6,974.96	6,974.31
C-4	15.00	24.0	126.0	0.019	0.013	9.76	30.89	7,001.71	6,998.95
C 1B-8	22.70	30.0	64.4	0.013	0.013	9.58	47.57	6,989.26	6,988.04
C 1B-7	22.70	30.0	127.2	0.011	0.013	8.91	43.22	6,987.76	6,986.02
C 8-1	17.00	36.0	-	0.014	0.013	8.89	78.78	6,994.41	6,990.80
C 4-1	7.50	24.0	27.0	0.019	0.013	8.09	30.80	6,976.91	6,976.16
C 1B-3	37.90	42.0	124.3	0.006	0.013	7.94	76.58	6,973.39	6,973.48
C 1B-2	44.10	42.0	92.3	0.005	0.013	7.78	71.02	6,973.09	6,973.05
C 8-2	12.00	24.0	60.8	0.012	0.013	7.76	24.52	6,999.76	6,998.80
C 1B-1	48.60	42.0	74.8	0.004	0.013	7.40	64.87	6,972.54	6,972.15
C 7-1	2.20	18.0	18.9	0.034	0.013	7.25	19.29	6,991.21	6,991.03
C9-1	20.60	36.0	109.4	0.006	0.013	6.87	51.40	7,013.38	7,013.39
C-5	9.10	18.0	35.5	0.010	0.013	6.67	10.47	7,002.43	7,001.99
C-11	2.30	18.0	109.8	0.026	0.013	6.66	16.81	7,013.73	7,010.92
C 1B-4	24.20	42.0	46.8	0.005	0.013	6.64	70.50	6,973.85	6,973.86
C 10-1	2.00	24.0	26.3	0.029	0.013	6.45	38.44	6,986.70	6,986.26
C9-2	20.60	36.0	307.3	0.005	0.013	6.44	47.06	7,015.01	7,013.66
C-10	2.30	18.0	45.0	0.016	0.013	5.67	13.38	7,010.82	7,009.93
C-9	2.30	18.0	199.1	0.015	0.013	5.49	12.82	7,005.05	7,001.94
C-1B	23.20	30.0	52.3	0.003	0.013	5.43	23.38	6,990.17	6,990.03
C-1A	23.20	30.0	33.9	0.003	0.013	5.16	22.29	6,989.77	6,989.62
C-3	16.10	30.0	416.6	0.003	0.013	4.98	22.46	6,992.04	6,991.21
C-7	2.30	18.0	101.9	0.010	0.013	4.81	10.65	6,994.26	6,993.35
C-8	2.30	18.0	275.2	0.010	0.013	4.81	10.65	6,997.09	6,994.38
C-2	21.80	30.0	-	0.002	0.013	4.69	20.47	6,990.98	6,990.55
C 3-1	6.80	18.0	2.5	0.112	0.013	3.85	35.12	6,978.13	6,977.90
C-6	2.80	18.0	355.0	0.003	0.013	3.24	5.77	6,993.27	6,992.38
C-1.1	1.40	18.0	17.6	0.306	0.013	0.79	58.12	6,994.94	6,994.78

X:\2510000.all\2518800\StormCAD\Sterling Ranch PH 2 - Final\Phase 2 - Final.stsw

Scenario: 5 YR
Current Time Step: 0.000 h
FlexTable: Manhole Table

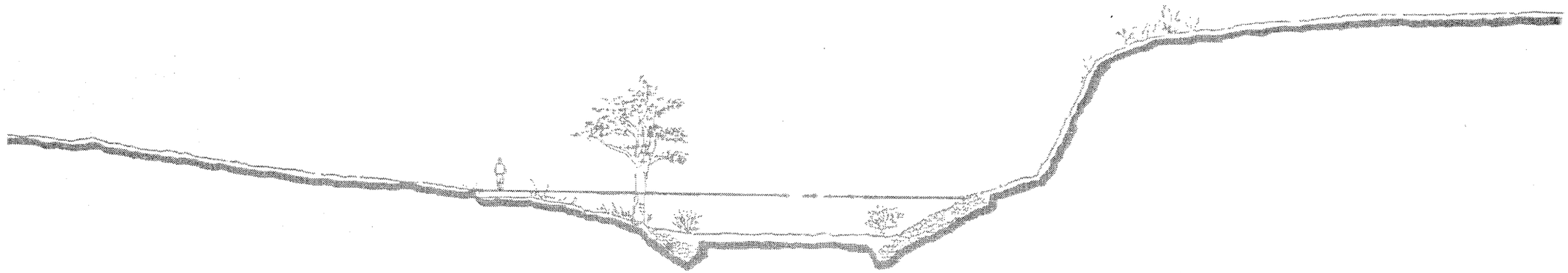
Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Headloss Coefficient (Standard)
10' Type R Inlet - 1A	6,994.34	6,987.63	1.40	6,994.35	6,994.34	6,994.36	6,994.35	0.600
15' Type R Inlet - 2	6,995.37	6,988.86	21.80	6,991.21	6,990.98	6,991.44	6,991.45	0.500
15' Type R Inlet - 4	7,005.49	7,000.06	15.00	7,002.03	7,001.71	7,002.72	7,002.35	0.500
15' Type R Inlet - 5	7,005.49	7,001.05	9.10	7,002.73	7,002.43	7,003.32	7,003.02	0.500
15' Type R Inlet 10-1	6,990.17	6,986.21	2.00	6,986.79	6,986.70	6,986.96	6,986.88	0.500
15' Type R Inlet 3-1	6,980.90	6,976.89	6.80	6,978.24	6,978.13	6,978.47	6,978.36	0.500
15' Type R Inlet 3-2	6,980.84	6,977.18	15.30	6,979.20	6,978.59	6,980.42	6,979.81	0.500
15' Type R Inlet 4-1	6,979.81	6,975.94	7.50	6,977.10	6,976.91	6,977.48	6,977.29	0.500
15' Type R Inlet 5-1	6,979.58	6,974.17	5.70	6,975.11	6,974.96	6,975.39	6,975.25	0.500
15' Type R Inlet 8-1	7,003.00	6,993.10	17.00	6,994.66	6,994.41	6,995.56	6,994.91	0.500
15' Type R Inlet 8-2	7,001.78	6,998.51	12.00	7,000.02	6,999.76	7,000.55	7,000.29	0.500
BEND - 7	7,003.48	6,993.68	2.30	6,994.38	6,994.26	6,994.51	6,994.47	0.600
EPC TYPE 1 MH 6-3	7,015.59	7,006.01	35.40	7,008.12	7,007.78	7,010.30	7,008.46	0.500
EPC TYPE 1 MH 1B-1	6,979.57	6,970.28	48.60	6,973.05	6,972.54	6,973.51	6,973.39	0.600
EPC TYPE 1 MH 1B-2	6,979.46	6,970.76	44.10	6,973.48	6,973.09	6,973.83	6,973.74	0.600
EPC TYPE 1 MH 1B-3	6,980.54	6,971.48	37.90	6,973.86	6,973.39	6,974.06	6,974.16	0.600
EPC TYPE 1 MH 1B-4	6,981.10	6,973.20	24.20	6,974.07	6,973.85	6,975.88	6,974.12	0.800
EPC TYPE 1 MH 1B-5	6,988.71	6,982.35	24.20	6,984.43	6,983.93	6,984.81	6,984.56	0.800
EPC TYPE 1 MH 1B-6	6,989.81	6,984.23	24.20	6,986.26	6,985.82	6,987.49	6,986.45	0.700
EPC TYPE 1 MH 1B-7	6,991.56	6,986.14	22.70	6,988.11	6,987.76	6,989.41	6,988.47	0.500
EPC TYPE 1 MH 1B-8	6,992.27	6,987.64	22.70	6,989.62	6,989.26	6,990.15	6,989.97	0.500
EPC TYPE 1 MH 6-1	6,996.65	6,988.33	52.00	6,991.03	6,990.51	6,991.08	6,991.38	0.600
EPC TYPE 1 MH 6-2	7,008.21	6,998.76	35.40	7,000.87	7,000.53	7,003.39	7,001.20	0.500
EPC TYPE 1 MH 6-4	7,021.96	7,011.27	35.40	7,013.39	7,012.98	7,013.62	7,013.66	0.600
EPC TYPE 1 MH 9-1	7,021.10	7,011.92	20.60	7,013.66	7,013.38	7,014.08	7,013.95	0.500
EPC TYPE 1 MH 9-2	7,020.22	7,013.55	20.60	7,015.46	7,015.01	7,016.03	7,015.58	0.800
EX TIE IN	7,022.00	7,018.65	35.40	7,020.54	7,020.17	7,021.28	7,020.91	0.500
EX-1	6,997.75	6,987.50	69.00	6,990.84	6,989.98	6,992.07	6,991.05	0.800
MH-8A	6,992.67	6,987.74	23.20	6,990.03	6,989.77	6,990.43	6,990.28	0.500
MH-8B	6,994.06	6,988.01	23.20	6,990.38	6,990.17	6,991.06	6,990.58	0.500
Type C Inlet - 3	7,001.33	6,990.21	16.10	6,992.38	6,992.04	6,992.47	6,992.42	0.900
Type C Inlet - 6	7,000.51	6,992.53	2.80	6,993.35	6,993.27	6,993.47	6,993.43	0.500
Type C Inlet - 8	7,004.33	6,996.51	2.30	6,997.22	6,997.09	6,997.68	6,997.30	0.600
Type C Inlet - 9	7,013.09	7,004.48	2.30	7,005.18	7,005.05	7,005.68	7,005.27	0.600
Type C Inlet - 10	7,013.34	7,010.24	2.30	7,010.92	7,010.82	7,011.14	7,011.03	0.500
Type C Inlet - 11	7,015.87	7,013.16	2.30	7,013.84	7,013.73	7,014.05	7,013.95	0.500
Type C Inlet 7-1	6,993.40	6,990.65	2.20	6,991.31	6,991.21	6,991.52	6,991.42	0.500

Appendix D
Reference Material

SAND CREEK DRAINAGE BASIN PLANNING STUDY

PRELIMINARY DESIGN REPORT

CITY OF COLORADO SPRINGS, EL PASO COUNTY, COLORADO



PREPARED FOR:

City of Colorado Springs
Department of Comprehensive Planning, Development and Finance
Engineering Division
30 S. Nevada
Colorado Springs, Colorado 80903

PREPARED BY:

Kiowa Engineering Corporation
1011 North Weber
Colorado Springs, CO 80903

II. STUDY AREA DESCRIPTION

The Sand Creek drainage basin is a left-bank tributary to the Fountain Creek lying in the west-central portions of El Paso County. Sand Creek's drainage area at Fountain Creek is approximately 54 square miles of which approximately 18.8 square miles are inside the City of Colorado Springs corporate limits. The basin is divided into five major sub-basins, the Sand Creek mainstem, the East Fork Sand Creek, the Central Tributary to East Fork, the West Fork, and the East Fork Subtributary. Figure II-1 shows the location of the Sand Creek basin.

Basin Description

The Sand Creek basin covers a total of 54 square miles in unincorporated El Paso County and Colorado Springs, Colorado. Of this total, approximately 28 square miles is encompassed by the Sand Creek basin, and 26 square miles for the East Fork Sand Creek basin. The basin trends in generally a south to southwesterly direction, entering the Fountain Creek approximately two miles upstream of the Academy Boulevard bridge over Fountain Creek. Two main tributaries drain the basin, those being the mainstem of Sand Creek and East Fork Sand Creek. Development presence in most evident along the mainstream. At this time, approximately 25 percent of the basin is developed. This alternative evaluation focuses upon the Sand Creek basin only.

The maximum basin elevation is approximately 7,620 feet above mean sea level, and falls to approximately 5,790 feet at the confluence with Fountain Creek. The headwaters of the basin originate in the conifer covered areas of The Black Forest. The middle eastern portions of the basin are typified by rolling range land with fair to good vegetative cover associated with semi-arid climates.

Climate

This area of El Paso County can be described, in general as high plains, with total precipitation amounts typical of a semi-arid region. Winters are generally cold and dry. Precipitation ranges from 14 to 16 inches per year, with the majority of this precipitation occurring in spring and summer in the form of rainfall. Thunderstorms are common during the summer months, and are typified by quick-moving low pressure cells which draw moisture from the Gulf of Mexico into the region. Average temperatures range from about 30^{O}F in the winter to 75^{O} in the summer. The relative humidity ranges from about 25 percent in the summer to 45 percent in the winter.

Soils and Geology

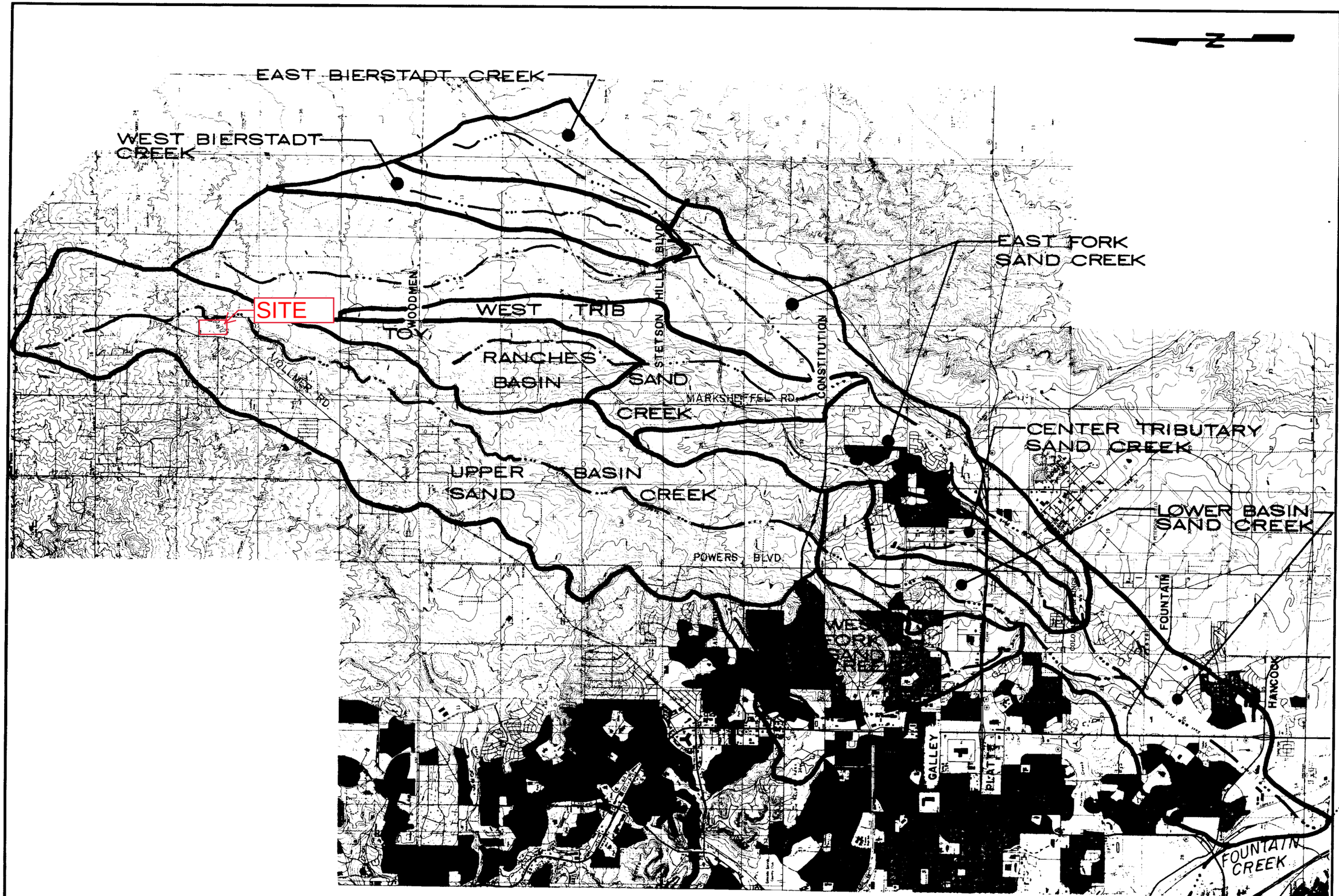
Soils within the Sand Creek basin vary between soil types A through D, as identified by the U. S. Department of Agriculture, Soil Conservation Service. The predominant soil groupings are in the Truckton and Bresser soil associations. The soils consist of deep, well drained soils that formed in alluvium and residium, derived from sedimentary rock. The soils have high to moderate infiltration rates, and are extremely susceptible to wind and water erosion where poor vegetation cover exists. In undeveloped areas, the predominance of Type A and B soils give this basin a lower runoff per unit area as compared to basins with soils dominated by Types C and D. Presented on Figure II-2 is the Hydrologic Soil distribution map for the Sand Creek basin.

Property Ownership and Impervious Land Densities

Property ownership along the major drainageway within the Sand Creek basin vary from public to private. Along the developed reaches, drainage right-of-ways and greenbelts have been dedicated during the development of the adjacent residential and commercial land. Where development has not occurred, the drainageways remain under private ownership with no delineated drainage right-of-way or easements. There are several public parks which abut the mainstem of Sand Creek. Roadway and utility easements abutting or crossing the major drainageways occur most frequently in the developed portions of the basin.

Land use information for the existing and future conditions were reviewed as part of the planning effort. This information is used in the hydrologic analysis to predict runoff rates and volumes for the purposes of facility evaluation. The identification of land uses abutting the drainageways is also useful in the identification of feasible plans for stabilization and aesthetic treatment of the creek. Presented on Figure II-3 is the proposed land use map used in the evaluation of impervious land densities discussed in the hydrologic section of this report. Figure II-3 is not intended to reflect the future zoning or land use policies of the City or the County.

The land use information within the Banning-Lewis Ranch property was obtained from Aries Properties during the time the draft East Fork Sand Creek Drainage Basin Planning Study was being prepared. The land use information was again reviewed with the City of Colorado Springs Department of Planning and was found to be appropriate for use in the estimation of hydrology for the East Fork Basin. The location of future arterial streets and roadways within



Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

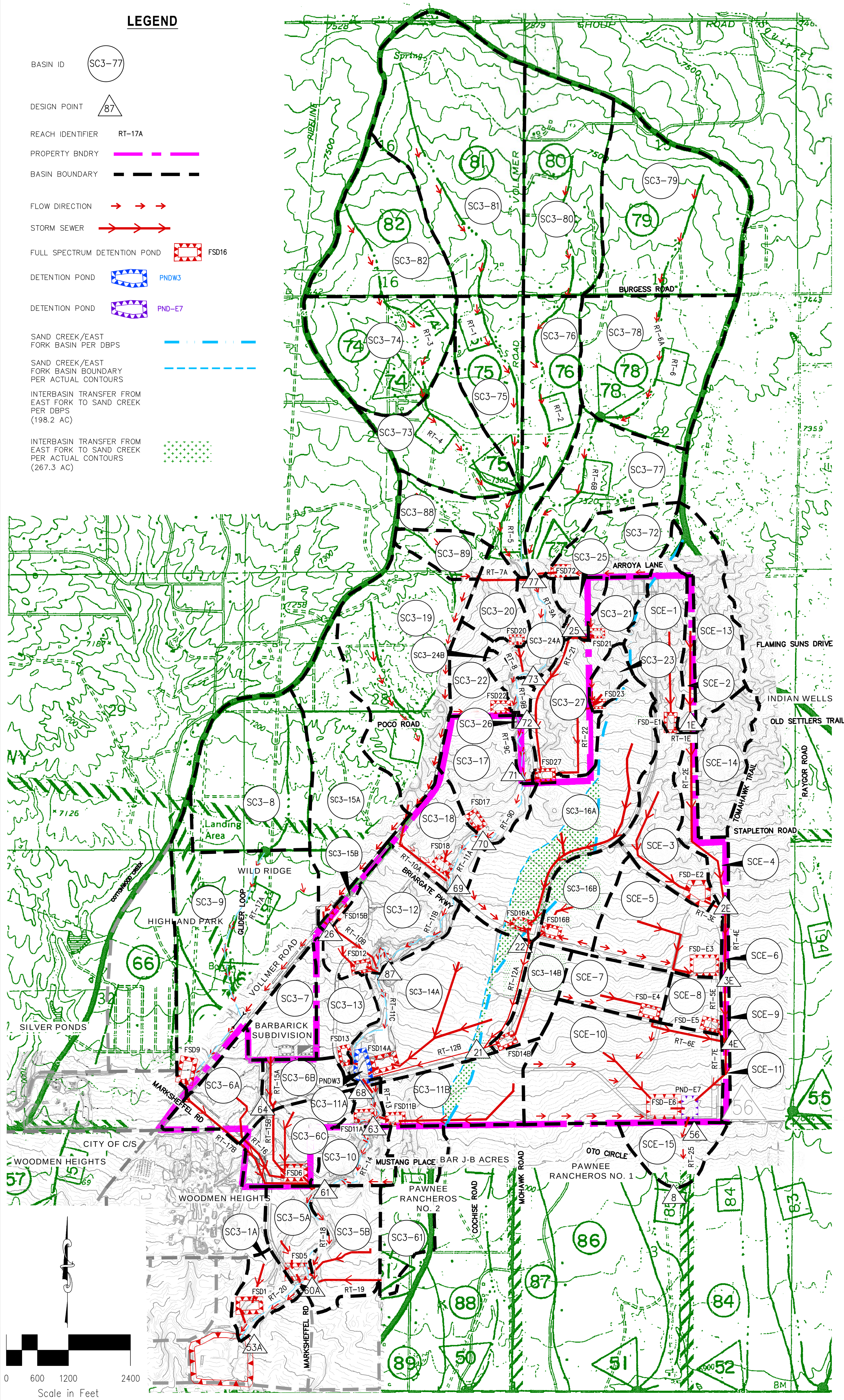
SAND CREEK DRAINAGE BASIN PLANNING STUDY

REGIONAL SUB-BASINS

Project No	90-04-09
Date:	11/90
Design:	
Drawn:	EAK
Check:	
Revisions:	

11

FIG. III-1



BASIN SUMMARY

BASIN	CN	AREA (ACRES)	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)
SC3-1A	73	27.8	0.044	16.3	23.3	33.0	45.8	57.1	68.9
SC3-5A	84	39.1	0.061	40.6	53.7	71.0	92.4	110.6	129.1
SC3-5B	81	63.0	0.098	53.8	73.0	98.5	130.8	158.6	187.0
SC3-6A	88	49.3	0.077	61.4	79.3	102.2	130.1	153.6	177.1
SC3-6B	85	30.9	0.048	32.9	43.4	57.0	73.9	88.2	102.7
SC3-6C	82	58.0	0.091	53.9	72.5	97.1	128.0	154.5	181.5
SC3-7	88	45.7	0.071	54.0	69.9	90.3	115.2	136.2	157.2
SC3-8	62	143.4	0.224	25.4	42.1	66.7	100.7	132.3	166.2
SC3-9	66	217.4	0.340	45.8	71.5	108.6	158.9	204.9	254.0
SC3-10	63	36.0	0.056	7.6	12.3	19.4	29.1	38.0	47.7
SC3-11A	70	10.7	0.017	5.3	7.8	11.3	15.9	20.0	24.3
SC3-11B	80	76.6	0.120	59.4	81.3	110.8	148.1	180.5	213.7
SC3-12	81	88.2	0.138	77.8	105.6	142.5	189.1	229.1	270.0
SC3-13	85	41.0	0.064	43.9	57.8	76.0	98.5	117.6	136.9
SC3-14A	79	164.9	0.258	127.6	175.4	239.8	321.9	393.2	466.3
SC3-14B	77	34.7	0.054	24.6	34.3	47.4	64.2	79.0	94.1
SC3-15A	62	139.7	0.218	21.3	35.5	56.3	85.3	112.1	141.0
SC3-15B	87	7.9	0.012	10.8	14.0	18.2	23.3	27.6	31.9
SC3-16A	74	168.1	0.263	84.4	120.4	170.0	234.8	292.2	351.8
SC3-16B	78	50.7	0.079	39.0	53.7	73.6	99.0	121.1	143.8
SC3-17	73	70.6	0.110	41.8	59.6	85.2	119.0	149.1	180.6
SC3-18	81	53.8	0.084	49.3	67.1	91.0	121.2	147.3	174.0
SC3-19	62	184.0	0.287	28.8	47.7	75.7	114.4	150.2	188.8
SC3-20	65	34.2	0.053	9.9	15.5	23.8	35.1	45.5	56.6
SC3-21	66	23.3	0.036	7.0	10.8	16.3	23.7	30.4	37.5
SC3-22	65	33.9	0.053	9.4	14.8	22.5	32.9	42.5	52.6
SC3-23	67	14.5	0.023	5.5	8.3	12.4	18.0	23.0	28.4
SC3-24A	65	35.7	0.056	13.0	20.4	31.1	45.7	59.0	73.2
SC3-24B	65	12.2	0.019	3.4	5.3	8.1	11.8	15.2	18.9
SC3-25	66	19.0	0.030	5.8	8.9	13.4	19.5	25.1	31.0
SC3-26	63	10.0	0.016	2.5	4.0	6.2	9.2	12.1	15.1
SC3-27	71	70.0	0.109	35.1	51.2	73.8	103.7	130.3	158.3
SC3-61	63	65.5	0.102	13.7	22.0	34.4	51.6	67.6	84.8
SC3-72	64	56.2	0.088	12.8	20.2	31.4	46.7	60.9	76.0
SC3-73	63	90.0	0.141	16.4	26.4	41.3	62.1	81.3	102.0
SC3-74	63	119.7	0.187	22.3	36.5	57.3	85.9	112.3	140.7
SC3-75	63	79.3	0.124	13.1	21.5	33.7	50.5	66.1	82.8
SC3-76	63	86.4	0.135	14.2	23.1	36.4	54.6	71.4	89.6
SC3-77	62	106.9	0.167	16.6	27.6	43.8	66.2	87.0	109.4
SC3-78	63	155.6	0.243	28.1	45.3	70.6	106.2	139.1	174.5
SC3-79	63	189.0	0.295	34.9	57.0	89.5	134.3	175.6	220.1
SC3-80	63	147.7	0.231	27.3	44.3	69.6	104.5	136.8	171.4
SC3-81	62	262.9	0.411	42.6	70.2	111.0	167.4	219.6	275.7
SC3-82	62	117.8	0.184	20.0	33.2	52.8	80.0	105.1	132.3
SC3-88	62	60.2	0.094	10.5	17.4	27.6	41.8	54.9	69.0
SC3-89	62	27.5	0.043	6.1	10	15.7	23.6	30.8	38.6
SCE-1	65	64.4	0.101	23.3	35.9	53.8	79.1	102.4	127.4
SCE-2	64	15.0	0.023	4.4	7.0	10.8	15.9	20.7	25.7
SCE-3	70	67.5	0.105	30.6	45.2	65.9	93.3	118.0	143.9
SCE-4	70	29.5	0.046	13.3	19.6	28.6	40.6	257.8	62.6
SCE-5	87	85.5	0.134	100.4	130.6	169.6	217.4	257.8	298.4
SCE-6	64	3.8	0.006	1.6	2.5	3.7	5.4	7.0	8.6
SCE-7	89	44.9	0.070	58.9	75.5	96.6	122.2	143.7	165.2
SCE-8	92	25.5	0.040	38.6	48.4	60.7	75.4	87.7	99.9
SCE-9	64	4.0	0.006	1.5	2.4	3.6	5.3	6.8	8.5
SCE-10	83	174.3	0.272	7.6	189.4	19.4	29.1	398.9	467.5
SCE-11	64	5.8	0.009	2.3	3.6	5.5	8.0	10.3	12.8
SCE-13	63	78.6	0.123	19.6	31.3	48.7	73.1	95.7	120.0
SCE-14	63	52.5	0.082	13.2	21.2	33.3	49.9	65.2	81.7
SCE-15	51	39.7	0.062	2.2	5.1	10.1	17.7	25.1	33.4

DESIGN POINT SUMMARY

DESIGN POINT	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)	LOCATION
DP-74	0.371	39.3	65.3	104.8	158.9	209.1	262.8	
DP-75	1.413	141.2	235.1	376.6	566.6	750.9	950.5	
DP-77	2.343	209.9	351.9	580.6	886.6	1168.4	1467.7	ARROYA LANE X-ING
DP-78	0.538	59.7	98.4	154.0	232.6	306.2	385.3	
DP-73	2.471	207.5	354.3	588.5	897.1	1187.2	1506.7	
DP-72	2.543	206.2	352.5	586.7	897.2	1195.3	1518.6	POCO ROAD X-ING
DP-71	2.757	205.9	349.3	610.5	932.4	1226.9	1612.2	STERLING RANCH NORTHERN BNDRY
DP-70	2.867	205.3	349.8	614.0	940.1	1260.6	1636.7	
DP-69	3.238	212.7	366.6	653.7	1010.6	1364.1	1775.7	BRIARGATE PARKWAY X-ING
DP-87	3.594	216.9	374.6	681.9	1072.1	1471.5	1905.9	
DP-68	4.312	214.6	374.5	714.9	1187.6	1674.9	2204.1	UPSTREAM OF POND W3
DP-64	0.119	85.9	112.1	145.9	187.5	222.6	258.0	
DP-63	4.449	154.4	201.0	375.7	815.9	1112.1	1385.1	STERLING RANCH SOUTHERN BNDRY
DP-61	5.356	156.6	223.9	428.0	928.2	1287.3	1620.1	COLORADO SPRINGS/EL PASO BNDRY
DP-60A	5.617	161.6	224.8	439.1	950.4	1320.5	1661.8	MARKSHEFFEL X-ING
DP-53A	5.661	161.6	225.7	441.8	951.1	1326.0	1668.9	SAND CREEK AND POND 3
DP-1E	0.247	23.9	38.3	70.1	132.8	173.0	220.9	
DP-2E	0.486	48.9	76.8	123.0	228.7	319.7	419.4	
DP-3E	0.626	48.5	75.7	122.2	271.1	387.1	500.1	
DP-4E	0.745	48.1	76.2	122.4	286.9	407.3	534.8	
DP-56	1.017	23.1	35.3	71.5	108.3	152.1	196.4	NEAR SE PROP CORNER
DP-8	1.079	24.1	37.2	73.5	111.3	155.4	200.7	BELOW SE PROP CORNER
DP-21	0.396	0.6	8.8	17.8	57.1	116.8	174.9	
DP-22	0.342	0.6	8.8	17.6	56.8	105.1	156.4	
DP-25	0.066	5.9	9.1	16.3	35.1	46.4	58.2	
DP-26	0.012	0.1	1.1	3.2	7.3	9.5	12.0	

DESIGN POINT SUMMARY (VOLUME)

DESIGN POINT	AREA (SQ MI)	V_2 (AC-FT)	V_5 (AC-FT)	V_{10} (AC-FT)	V_{25} (AC-FT)	V_{50} (AC-FT)	V_{100} (AC-FT)	LOCATION
DP-74	0.371	5.9	9.0	13.6	19.8	25.5	31.6	
DP-75	1.413	22.7	34.5	51.7	75.4	97.1	120.5	
DP-77	2.343	37.7	57.4	85.9	125.1	161.1	199.9	ARROYA LANE X-ING
DP-78	0.538	8.9	13.5	20.1	29.3	37.7	46.7	
DP-73	2.471	40.0	60.8	91.0	132.5	170.7	211.7	
DP-72	2.543	41.3	62.9	94.0	136.8	176.2	218.5	POCO ROAD X-ING
DP-71	2.757	46.3	70.0	104.3	151.3	194.5	240.8	STERLING RANCH NORTHERN BNDRY
DP-70	2.867	49.5	74.5	110.6	160.1	205.4	254.0	
DP-69	3.238	57.5	86.1	127.4	183.8	235.3	290.6	BRIARGATE PARKWAY X-ING
DP-87	3.594	66.5	98.9	145.6	209.1	267.1	329.1	
DP-68	4.312	81.8	123.7	183.9	264.9	338.0	415.8	UPSTREAM OF POND W3
DP-64	0.119	7.0	9.1	11.8	15.2	18.1	21.1	
DP-63	4.449	85.6	129.5	192.3	276.7	352.8	433.5	STERLING RANCH SOUTHERN BNDRY
DP-61	5.356	103.7	157.8	235.1	338.4	431.3	529.8	COLORADO SPRINGS/EL PASO BNDRY
DP-60A	5.617	111.0	168.6	250.4	359.5	457.7	561.5	MARKSHEFFEL X-ING
DP-53A	5.661	112.0	170.0	252.6	362.6	461.7	566.5	SAND CREEK AND POND 3
DP-1E	0.247	3.1	5.2	8.4	12.7	16.6	20.9	
DP-2E	0.480	6.1	10.4	16.9	25.7	33.7	42.2	
DP-3E	0.620	7.0	13.7	23.4	36.1	47.4	59.3	
DP-4E	0.736	7.6	15.6	27.2	43.0	57.2	72.0	
DP-56	1.017	7.7	16.1	28.6	51.3	71.7	92.9	NEAR SE PROP CORNER
DP-8	1.079	8.0	16.7	26.6	53.0	74.0	95.9	BELOW SE PROP CORNER
DP-21	0.396	6.3	11.3	18.3	27.5	35.6	44.0	
DP-22	0.736	6.3	10.7	16.7	24.6	31.5	38.7	
DP-25	1.017	1.3	1.9	2.8	4.1	5.2	6.4	
DP-26	1.079	0.7	0.9	1.2	1.5	1.8	2.1	

WATER QUALITY & DETENTION POND SUMMARY

FSD1						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	16.3	23.3	33.0	45.8	57.1	68.9
ALLOWABLE RELEASE (CFS)	0.1	1.7	3.3	10.9	17.5	25.5
MODELED RELEASE (CFS)	0.1	1.6	3.2	10.9	17.4	25.4
STORED VOLUME (AC-FT)	2.4	2.6	3.0	3.6	1.9	2.2

FSD5						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	40.6	53.7	71.0	92.4	110.6	129.1
ALLOWABLE RELEASE (CFS)	0.1	1.4	2.6	11.3	19.8	30.2
MODELED RELEASE (CFS)	0.1	1.4	2.6	11.2	19.7	30.1
STORED VOLUME (AC-FT)	3.0	3.2	3.8	4.1	4.7	5.2

FSD6						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	196.5	258.5	339.1	438.7	523.3	608.6
ALLOWABLE RELEASE (CFS)	0.5	7.6	14.6	58.4	99.6	149.7
MODELED RELEASE (CFS)	0.5	7.5	14.5	58.2	99.6	149.6
STORED VOLUME (AC-FT)	15.5	16.4	18.7	20.8	23.3	26.0

FSD9						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	64.6	105.6	169.5	252.3	327.1	410.1
ALLOWABLE RELEASE (CFS)	1.7	24.9	49.8	141.1	207.2	290.0
MODELED RELEASE (CFS)	1.7	24.9	49.8	141.1	207.0	289.9
STORED VOLUME (AC-FT)	8.7	8.7	9.6	10.8	12.3	13.8

FSD11A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	5.3	7.8	11.3	15.9	20.0	24.3
ALLOWABLE RELEASE (CFS)	0.1	1.6	3.2	7.5	9.7	12.4
MODELED RELEASE (CFS)	0.2	0.9	3.0	7.5	9.7	12.3
STORED VOLUME (AC-FT)	0.3	0.3	0.4	0.4	0.5	0.6

FSD11B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	59.4	81.3	110.8	148.1	180.5	213.7
ALLOWABLE RELEASE (CFS)	0.3	4.5	8.7	29.6	47.7	69.6
MODELED RELEASE (CFS)	0.3	4.5	8.6	29.5	47.7	69.5
STORED VOLUME (AC-FT)	4.8	4.9	5.5	6.4	7.3	8.2

FSD12						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	77.8	105.6	142.5	189.1	229.1	270.0
ALLOWABLE RELEASE (CFS)	0.9	13.2	26.7	62.0	80.2	103.2
MODELED RELEASE (CFS)	0.9	9.0	26.7	61.9	80.1	103.1
STORED VOLUME (AC-FT)	5.2	5.5	5.8	6.7	7.8	8.9

FSD13						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	43.9	57.8	76.0	98.5	117.6	136.9
ALLOWABLE RELEASE (CFS)	0.4	6.1	12.3	28.6	37.0	47.6
MODELED RELEASE (CFS)	0.4	4.2	12.3	28.6	36.9	47.2
STORED VOLUME (AC-FT)	3.1	3.1	3.3	3.8	4.4	5.0

FSD14A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	127.6	175.4	239.8	321.9	393.2	466.3
ALLOWABLE RELEASE (CFS)	0.5	7.5	14.4	56.2	95.2	142.4
MODELED RELEASE (CFS)	0.5	7.5	14.4	56.2	95.1	142.2
STORED VOLUME (AC-FT)	9.9	10.6	11.9	13.5	15.3	17.3

FSD14B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	24.6	34.3	47.4	64.2	79.0	94.1
ALLOWABLE RELEASE (CFS)	0.0	0.3	0.5	5.7	11.8	19.3
MODELED RELEASE (CFS)	0.0	0.3	0.5	4.5	11.8	19.3
STORED VOLUME (AC-FT)	1.9	2.5	3.3	3.5	3.5	3.8

FSD15B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	10.8	14.0	18.2	23.3	27.6	31.9
ALLOWABLE RELEASE (CFS)	0.1	1.6	3.2	7.3	9.5	12.0
MODELED RELEASE (CFS)	0.1	1.1	3.2	7.3	9.5	12.0
STORED VOLUME (AC-FT)	0.6	0.6	0.7	0.8	0.9	1.0

FSD16A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	84.4	120.4	170.0	234.8	292.2	351.8
ALLOWABLE RELEASE (CFS)	0.6	8.8	17.3	56.2	88.4	128.3
MODELED RELEASE (CFS)	0.6	8.8	17.3	56.2	88.3	128.3
STORED VOLUME (AC-FT)	7.6	7.7	8.9	10.4	12.1	13.8

WATER QUALITY & DETENTION POND SUMMARY

FSD16B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	39.0	53.7	73.6	99.0	121.1	143.8
ALLOWABLE RELEASE (CFS)	0.0	0.4	0.7	8.3	17.2	28.2
MODELED RELEASE (CFS)	0.0	0.4	0.7	7.9	17.2	28.1
STORED VOLUME (AC-FT)	3.0	3.9	5.1	5.1	5.3	5.8

FSD17						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	41.8	59.6	85.2	119.0	149.1	180.6
ALLOWABLE RELEASE (CFS)	0.7	11.1	22.5	52.0	67.2	86.3
MODELED RELEASE (CFS)	0.7	8.4	22.4	52.0	67.2	86.1
STORED VOLUME (AC-FT)	2.6	2.6	2.8	3.4	4.0	4.7

FSD18						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	49.3	67.1	91.0	121.2	147.3	174.0
ALLOWABLE RELEASE (CFS)	0.6	9.2	18.4	42.2	54.6	69.9
MODELED RELEASE (CFS)	0.6	6.3	18.4	42.2	54.6	69.6
STORED VOLUME (AC-FT)	3.2	3.2	3.4	4.0	4.7	5.3

FSD20						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	9.9	15.5	23.8	35.1	45.5	56.6
ALLOWABLE RELEASE (CFS)	0.4	5.5	11.1	25.7	33.2	42.5
MODELED RELEASE (CFS)	0.4	2.8	10.9	25.7	33.0	42.4
STORED VOLUME (AC-FT)	0.7	0.8	0.8	0.9	1.0	1.2

FSD21						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	7.0	10.8	16.3	23.7	30.4	37.5
ALLOWABLE RELEASE (CFS)	0.3	4.0	8.0	18.3	23.7	30.3
MODELED RELEASE (CFS)	0.3	3.3	8.0	18.3	23.7	30.1
STORED VOLUME (AC-FT)	0.5	0.5	0.5	0.6	0.7	0.8

FSD22						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	9.4	14.8	22.5	32.9	42.5	52.6
ALLOWABLE RELEASE (CFS)	0.4	5.8	11.5	26.5	34.3	43.9
MODELED RELEASE (CFS)	0.4	5.8	11.4	26.5	34.3	43.8
STORED VOLUME (AC-FT)	0.6	0.6	0.7	0.8	0.9	1.0

FSD23						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	5.5	8.3	12.4	18.0	23.0	28.4
ALLOWABLE RELEASE (CFS)	0.2	2.4	4.9	11.2	14.5	18.6
MODELED RELEASE (CFS)	0.2	2.0	4.9	11.2	14.5	18.6
STORED VOLUME (AC-FT)	0.3	0.3	0.4	0.4	0.5	0.6

FSD27						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	38.8	57.6	84.1	119.7	159.2	206.3
ALLOWABLE RELEASE (CFS)	1.4	21.1	42.4	97.8	126.4	161.9
MODELED RELEASE (CFS)	1.4	18.4	42.3	97.7	126.2	161.9
STORED VOLUME (AC-FT)	2.7	2.8	2.9	3.2	3.7	4.2

FSD72						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	12.8	20.2	31.4	46.7	60.9	76.0
ALLOWABLE RELEASE (CFS)	0.6	9.6	19.3	44.4	57.4	73.4
MODELED RELEASE (CFS)	0.6	9.3	19.2	44.4	57.4	73.4
STORED VOLUME (AC-FT)	1.0	1.0	1.1	1.1	1.2	1.3

PNDW3						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	214.6	374.5	714.9	1187.6	1674.9	2204.1
MODELED RELEASE (CFS)	154.3	200.3	366.8	799.9	1085.6	1350.6
STORED VOLUME (AC-FT)	2.8	9.5	26.3	41.2	57.2	78.2

FSD-E1						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	23.3	35.9	53.8	79.1	102.4	127.4
ALLOWABLE RELEASE (CFS)	0.7	11.0	22.1	50.9	65.7	84.1
MODELED RELEASE (CFS)	0.7	5.4	19.9	48.9	62.8	84.0
STORED VOLUME (AC-FT)	1.3	1.3	1.5	1.8	2.1	2.5

FSD-E2						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	30.6	45.2	65.9	93.3	118.0	143.9
ALLOWABLE RELEASE (CFS)	0.6	9.5	19.2	45.5	59.8	77.6
MODELED RELEASE (CFS)	0.6	3.2	18.5	41.3	58.5	74.7
STORED VOLUME (AC-FT)	2.1	2.3	2.4	2.8	3.3	3.8

FSD-E3						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	100.4	130.6	169.6	217.4	257.8	298.4
ALLOWABLE RELEASE (CFS)	0.9	13.2	26.5	61.6	79.8	102.6
MODELED RELEASE (CFS)	1.0	6.8	25.7	56.0	79.8	101.3
STORED VOLUME (AC-FT)	7.0	7.2	7.7	8.9	10.1	11.4

FSD-E4						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	58.9	75.5	96.6	122.2	143.7	165.2
ALLOWABLE RELEASE (CFS)	0.3	4.4	8.8	23.0	32.2	43.7
MODELED RELEASE (CFS)	0.9	2.8	8.7	21.9	32.2	43.6
STORED VOLUME (AC-FT)	4.2	4.3	4.7	5.4	6.2	6.9

FSD-E5						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	38.6	48.4	60.7	75.4	87.7	99.9
ALLOWABLE RELEASE (CFS)	0.0	0.2	0.4	4.2	8.7	14.3
MODELED RELEASE (CFS)	0.0	0.2	0.5	2.2	5.1	10.0
STORED VOLUME (AC-FT)	3.0	3.7	4.4	4.8	5.0	5.3

FSD-E6						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	141.6	189.4	252.5	331.4	398.9	467.5
ALLOWABLE RELEASE (CFS)	0.2	1.9	3.2	37.4	77.3	125.6
MODELED RELEASE (CFS)	0.2	0.9	3.2	18.3	64.1	123.3
STORED VOLUME (AC-FT)	13.0	17.0	21.9	22.2	22.6	23.7

PND-E7						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	46.5	75.4	121.2	285.2	402.4	548.0
MODELED RELEASE (CFS)	23.1	35.3	71.5	108.3	152.1	196.4
STORED VOLUME (AC-FT)	1.0	1.8	4.6	10.5	17.9	28.0

SAND CREEK FLOW COMPARISON CHART

DESIGN POINT	AREA (SQ MI)	Q_{100} (CFS)	DESCRIPTION
DP-77	2.343	1468	PROPOSED CONDITION
	2.91	2262	SAND CREEK DBPS
		2600	FEMA
DP-71	2.757	1612	PROPOSED CONDITION
		2260	SAND CREEK DBPS
DP-63	4.449	1385	PROPOSED CONDITION
	4.33	2630	SAND CREEK DBPS
		2600	FEMA
DP-60A	5.661	1662	PROPOSED CONDITION
	5.38	3295	SAND CREEK DBPS

EFSC DBPS DESIGN POINT SUMMARY (PEAK FLOW)

DBPS DESIGN POINT	AREA (SQ MI)	Q_{10} (CFS) (EXIST)	Q_{100} (CFS) (EXIST)	AREA (SQ MI)	Q_{10} (CFS) (PROP)	Q_{100} (CFS) (PROP)
DP-50	0.32	47.0	195.7	0.32	146.7	370.3
DP-51 (BASIN 86)	0.33	17.7	74.1	0.33	110.0	233.5
DP-52	1.67	80.5	456.5	1.67	1207.9	2123.0
DP-56	0.79	63.6	265.0	0.79	513.0	908.2

Values reported from SCDBPS, (DP 50, 51, 52 Not analyzed as a part of this study)
DBPS Reach 85(Basin91)=Q10=28.8cfs, Q100=115.2cfs (EXISTING) / Q10=345.7cfs, Q100=588.9cfs (PROPOSED)

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2018 STERLING RANCH MDDP

DEVELOPED HYDROLOGIC CONDITIONS MAP

PROJECT NO. 09-002 | FILE: \dwg\Eng Exhibits\2018-MDDP-PROPCOND.dwg

DESIGNED BY: JD
DRAWN BY: JD
CHECKED BY: VAS

SCALE
HORIZ: 1"=2400'
VERT: 1"=2400'

DATE: 10-21-2018

DM2

Pond FDS - Barbarack Subdivision Restrictor plate

Worksheet for FSD Outlet Orifice Plate

Project Description

Solve For	Diameter

Input Data

Discharge	45.90	ft³/s (16.5 Ha + 29.4 Pac)
Headwater Elevation	4.70	ft
Centroid Elevation	0.00	ft
Tailwater Elevation	0.00	ft
Discharge Coefficient	0.60	

Results

Diameter	2.37	ft
Headwater Height Above Centroid	4.70	ft
Tailwater Height Above Centroid	0.00	ft
Flow Area	4.40	ft²
Velocity	10.43	ft/s

Pond FDS - Barbarack Subdivision Restrictor plate

Worksheet for FSD Overflow - Pass

Project Description

Solve For	Discharge

Input Data

Headwater Elevation	0.90	ft
Crest Elevation	0.00	ft
Tailwater Elevation	0.00	ft
Crest Surface Type	Gravel	
Crest Breadth	12.00	ft
Crest Length	36.00	ft

Results

Discharge	86.22	ft³/s
Headwater Height Above Crest	0.90	ft
Tailwater Height Above Crest	0.00	ft
Weir Coefficient	2.80	US
Submergence Factor	1.00	
Adjusted Weir Coefficient	2.80	US
Flow Area	32.40	ft²
Velocity	2.66	ft/s
Wetted Perimeter	37.80	ft
Top Width	36.00	ft

(55 DU + 29.4 pass = 84.4 cfs)

Pond FDS Barbarack Subdivision Overflow Weir

Worksheet for FSD Overflow - Pass

Project Description

Solve For	Discharge		

Input Data

Headwater Elevation		0.90	ft
Crest Elevation		0.00	ft
Tailwater Elevation		0.00	ft
Crest Surface Type	Gravel		
Crest Breadth		12.00	ft
Crest Length		36.00	ft

Results

Discharge	86.22	ft³/s	(55 Dul + 29.4 pass = 84.4 cfs)
Headwater Height Above Crest	0.90	ft	
Tailwater Height Above Crest	0.00	ft	
Weir Coefficient	2.80	US	
Submergence Factor	1.00		
Adjusted Weir Coefficient	2.80	US	
Flow Area	32.40	ft²	
Velocity	2.66	ft/s	
Wetted Perimeter	37.80	ft	
Top Width	36.00	ft	

Pond SFB Barbarack Subdivision Overflow Weir

Worksheet for SFB Overflow Developed

Project Description

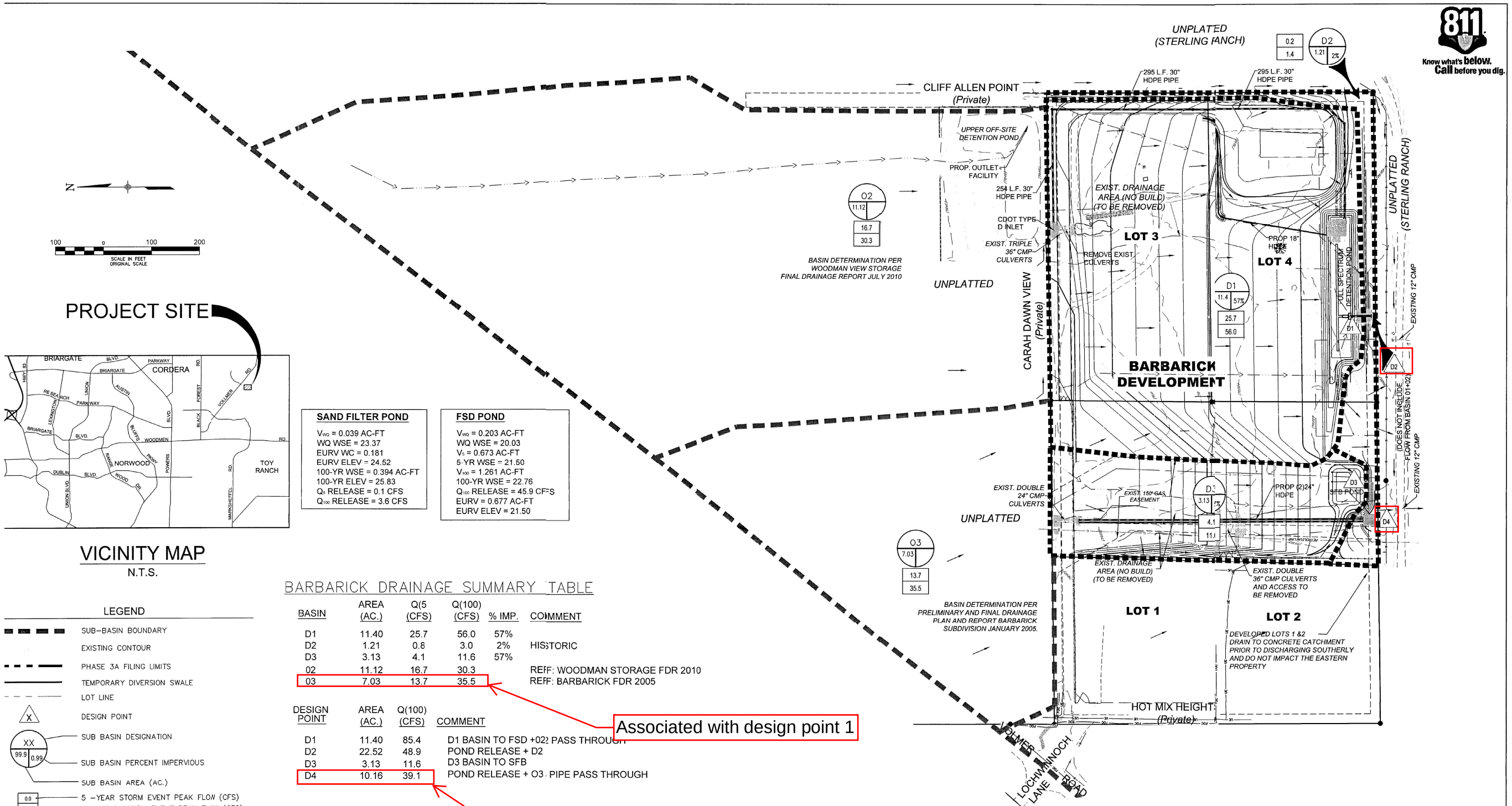
Solve For	Discharge

Input Data

Headwater Elevation		0.45	ft
Crest Elevation		0.00	ft
Tailwater Elevation		0.00	ft
Crest Surface Type	Gravel		
Crest Breadth		6.00	ft
Crest Length		10.00	ft

Results

Discharge	8.08	ft^3/s
Headwater Height Above Crest	0.45	ft
Tailwater Height Above Crest	0.00	ft
Weir Coefficient	2.68	US
Submergence Factor	1.00	
Adjusted Weir Coefficient	2.68	US
Flow Area	4.50	ft^2
Velocity	1.80	ft/s
Wetted Perimeter	10.90	ft
Top Width	10.00	ft



PROJECT SITE

VICINITY MAP
N.T.S.

SAND FILTER POND

V_{WQ} = 0.039 AC-FT
WQ WSE = 23.37
EURV WC = 0.181
EURV ELEV = 24.52
100-YR WSE = 0.394 AC-FT
100-YR ELEV = 25.83
Q_5 RELEASE = 0.1 CFS
Q_{100} RELEASE = 3.6 CFS

FSD POND

V_{WQ} = 0.203 AC-FT
WQ WSE = 20.03
V_5 = 0.673 AC-FT
5-YR WSE = 21.50
V_{100} = 1.261 AC-FT
100-YR WSE = 22.76
Q_{100} RELEASE = 45.9 CFS
EURV = 0.677 AC-FT
EURV ELEV = 21.50

LEGEND

- SUB-BASIN BOUNDARY
- EXISTING CONTOUR
- PHASE 3A FILING LIMITS
- TEMPORARY DIVERSION SWALE
- LOT LINE
- DESIGN POINT
- SUB BASIN DESIGNATION (XX)
- SUB BASIN PERCENT IMPERVIOUS (99.9 / 0.99)
- SUB BASIN AREA (AC.)
- 5 -YEAR STORM EVENT PEAK FLOW (CFS) (0.0)
- 100-YEAR STORM EVENT PEAK FLOW (CFS) (0.0)
- PROPOSED FLOW DIRECTION
- EXISTING FLOW DIRECTION

BARBARICK DRAINAGE SUMMARY TABLE

BASIN	AREA (AC.)	Q(5 (CFS)	Q(100) (CFS)	% IMP.	COMMENT
D1	11.40	25.7	56.0	57%	
D2	1.21	0.8	3.0	2%	HISTORIC
D3	3.13	4.1	11.6	57%	
02	11.12	16.7	30.3		REFF: WOODMAN STORAGE FDR 2010
03	7.03	13.7	35.5		REFF: BARBARICK FDR 2005

DESIGN POINT	AREA (AC.)	Q(100) (CFS)	COMMENT
D1	11.40	85.4	D1 BASIN TO FSD +022 PASS THROUGH
D2	22.52	48.9	POND RELEASE + D2
D3	3.13	11.6	D3 BASIN TO SFB
D4	10.16	39.1	POND RELEASE + O3 PIPE PASS THROUGH

Associated with design point 1

Associated with design point 4

NO.	DATE	DESCRIPTION	BY

REVISIONS

NAME: S:\15.789.001 Tri Lakes\Dwg\CD\Drainage\20160605-DP01.dwg
PCP: Matrix.ctb
PLOT DATE: Tue Jun 07, 2016 12:49pm

BENCHMARK DATA(ELEV.)
(DATUM)
(DESCRIPTION/LOCATION)

VERTICAL BENCHMARK:

THE VERTICAL INFORMATION ON THIS MAP IS BASED ON THE NATIONAL GEODETIC VERTICAL DATUM OF 1929 AND THE 1960 SUPPLEMENTARY ADJUSTMENT BEING A FOUND 3.25" ALUMINUM CAP IN A ROMAD BOX DESIGNATED AS FACILITIES INFORMATION MANAGEMENT SYSTEM (FIMS) MONUMENT "F_69" AND HAVING PUBLISHED ELEVATION OF 6975.62 FEET WAS USED TO REFERENCE THIS VERTICAL DATUM. THE BENCHMARK IS LOCATED ON THE WEST SIDE OF BLACK FOREST ROMOD, ABOUT 1.95 MILES SOUTH OF OLD RANCH ROAD, JUST SOUTH OF THE SCHMIDT CONSTRUCTION COMPANY DRIVEWAY. A CORNER FENCE POST IS 28.1 FEET TO THE SOUTHWEST, AND THE MOST SOUTHERLY GUARD RAIL POST IS 25.7 FEET TO THE NORTH.

BASIS OF BEARING:

THE BASIS OF BEARINGS FOR THIS MAP IS THE NORTH LINE OF BARBARICK SUBDIVISION ACCORDING TO THE OFFICIAL MAP THEREOF RECORDED FEBRUARY 12, 2008 IN THE OFFICE OF THE EL PASO COUNTY CLERK AND RECORDER UNDER RECEPTION NUMBER 208712754,SAID LINE MONUMENTED ON THE WEST END BY A FOUND 5/8" REBAR AND ON THE EAST BY A FOUND 4/8" REBAR WITH 1" ALUMINUM CAP STAMPED "LS 2154" BEING A POINT ON THE NORTH LINE BEARING NORTH 89°12'41 EAST 1287.35 FEET FROM THE WEST END THEREOF

PREPARED UNDER MY DIRECT SUPERVISION, FOR AND ON BEHALF OF MATRIX DESIGN GROUP, INC.

Matrix DESIGN GROUP
2435 Research Parkway, Suite
300 Colorado Springs, CO 80920
Phone 719-575-0100
Fax 719-575-0208

BARBARICK SUBDIVISION LOTS 1-4

PROPOSED DRAINAGE PLAN

DESIGNED B'	BJH	SCALE		DATE ISSUED:	April 2016
DRAWN BY:	BJH	HORIZ:	1"=100'		
CHECKED BY	ES	VERT:	N/A	SHEET NO. 1 OF 2 SHEETS	DP02

DESIGN POINT		
DP	Q5 Total	Q100 Total
1	4.4	9.4
2	1.9	3.9
3	11.1	24.7
4	3.7	7.4
5	4.1	19.6
6	3.3	6.7
6A	2.2	4.1
7	27.5	60.6
8	3.0	12.5
9	1.9	4.8
10	9.2	17.3
11	9.5	19.9
12	1.9	9.5
13	15.7	34.6
14	16.0	37.9
15	5.4	11.7
16	4.4	9.6
17	1.4	4.7
18	4.3	14.0
19	38.8	85.4
20	7.1	13.4
21	7.4	15.2
22	2.7	15.4
23	8.8	15.8
24	11.5	20.6
25	61.0	310.0
26	4.3	22.4
27	6.3	11.7
28	6.9	14.4
29	3.1	16.3
30	0.9	6.4
31	2.0	15.0
32	1.4	10.0
1.0	6.0	10.3
1.1	12.6	19.7
1.2	17.6	28.2
1.3	25.9	46.9
1.3A	5.0	8.7
1.4	52.5	105.9
1.5	55.1	103.9
1.6	56.4	107.7
1.7	17.3	25.3
1.8	68.8	125.0
2.0	23.2	74.5
2.1	38.1	106.6
2.2	56.9	138.7
2.3	9.6	17.2
2.4	63.7	151.9
2.5	96.6	250.7
2.6	97.8	250.4
2.7	162.0	336.8
2.8	189.8	424.4
2.9	14.2	22.5
3.0	189.8	424.4
3.1	14.2	22.5
3.2	187.5	428.2
4.0	18.4	26.1
4.1	56.2	264.7
4.2	12.7	26.0
4.3	49.1	291.2
4.4	3.1	3.1
4.5	51.1	51.1
4.6	56.5	245.8
4.7	58.4	248.6
4.8	59.8	320.3
OS2	13.8	39.1
OS3	17.6	48.9
OS4	2.6	8.5
D1	3.31	8.20
D2	1.63	2.97

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	2.06	66%	0.51	0.65	9.7	4.4	9.4
A2	0.82	69%	0.53	0.66	9.1	1.9	3.9
A3	6.76	60%	0.47	0.62	15.0	11.1	24.7
A4	1.51	77%	0.60	0.71	10.2	3.7	7.4
A5	1.70	76%	0.59	0.70	9.9	4.1	8.3
A6	1.37	75%	0.58	0.70	10.0	3.3	6.6
A6A	0.53	95%	0.81	0.88	5.0	2.2	4.1
A7	19.00	65%	0.45	0.59	18.3	27.5	60.6
A8	1.48	63%	0.56	0.70	13.9	3.0	6.3
A9	0.61	79%	0.73	0.83	8.7	1.9	3.7
A10	2.61	86%	0.79	0.88	7.9	9.2	17.3
A11	2.89	83%	0.76	0.86	8.7	9.5	18.1
A12	3.87	8%	0.13	0.38	11.9	1.9	9.5
A13	9.65	65%	0.45	0.59	14.0	15.7	34.6
A14	11.76	55%	0.39	0.55	15.3	16.0	37.9
A15	2.91	54%	0.52	0.68	14.9	5.4	11.7
A16	2.34	56%	0.54	0.69	14.7	4.4	9.6
A17	1.76	24%	0.21	0.44	13.7	1.4	4.7
A18	5.27	21%	0.24	0.47	16.4	4.3	14.0
A19	31.85	67%	0.45	0.59	25.8	38.8	85.4
A20	1.83	89%	0.81	0.89	8.0	6.6	12.2
A21	1.93	90%	0.82	0.90	8.7	6.8	12.6
A22	8.68	5%	0.11	0.37	23.3	2.7	15.4
B1	2.98	100%	0.90	0.96	17.6	8.8	15.8
B2	3.89	100%	0.90	0.96	17.6	11.5	20.6
B3	1.53	100%	0.90	0.96	9.4	5.8	10.4
B4	1.50	100%	0.90	0.96	9.4	5.7	10.2
B5	2.91	0%	0.08	0.35	13.1	0.9	6.4
C1	8.01	95%	0.81	0.88	9.9	2.0	15.0
C2	5.06	95%	0.81	0.88	7.9	1.4	10.0
OS20	308.00	6%	0.13	0.40	68.9	61.0	310.0
OS21A	20.26	14%	0.13	0.40	52.3	4.3	22.4
OS21B	8.71	9%	0.13	0.40	24.5	3.1	16.3
OS2	17.00	70%	0.49	0.62	36.0	13.8	39.1
OS3	28.70	70%	0.49	0.62	52.6	17.6	48.9
OS4	5.08	15%	0.20	0.40	29.5	2.6	8.5
D1	0.45	95%	0.81	0.88	7.0	1.7	3.1
D2	0.43	95%	0.81	0.88	7.0	1.6	3.0

NOTE

SEDIMENT CONTROL TO BE PROVIDED AT THE STUBS UNTIL THE TIME THOSE PARCELS DEVELOP

LEGEND:

- PROPOSED STORM SEWER
- 5000 FUTURE RD MAJOR CONTOUR
- FUTURE RD MINOR CONTOUR
- 5000 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5000 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN
- A = BASIN DESIGNATION; B = AREA IN ACRES; C = 5-YR RUNOFF COEFFICIENT; D = 100-YR RUNOFF COEFFICIENT
- DESIGN POINT
- HP HIGH POINT
- LP LOW POINT
- DRAINAGE ARROW
- EXISTING DRAINAGE ARROW
- PROPOSED DRAINAGE SWALE

DRAINAGE MAP
STERLING RANCH FILING 2
JOB NO. 25188.01
8/18/21
SHEET 1 OF 7

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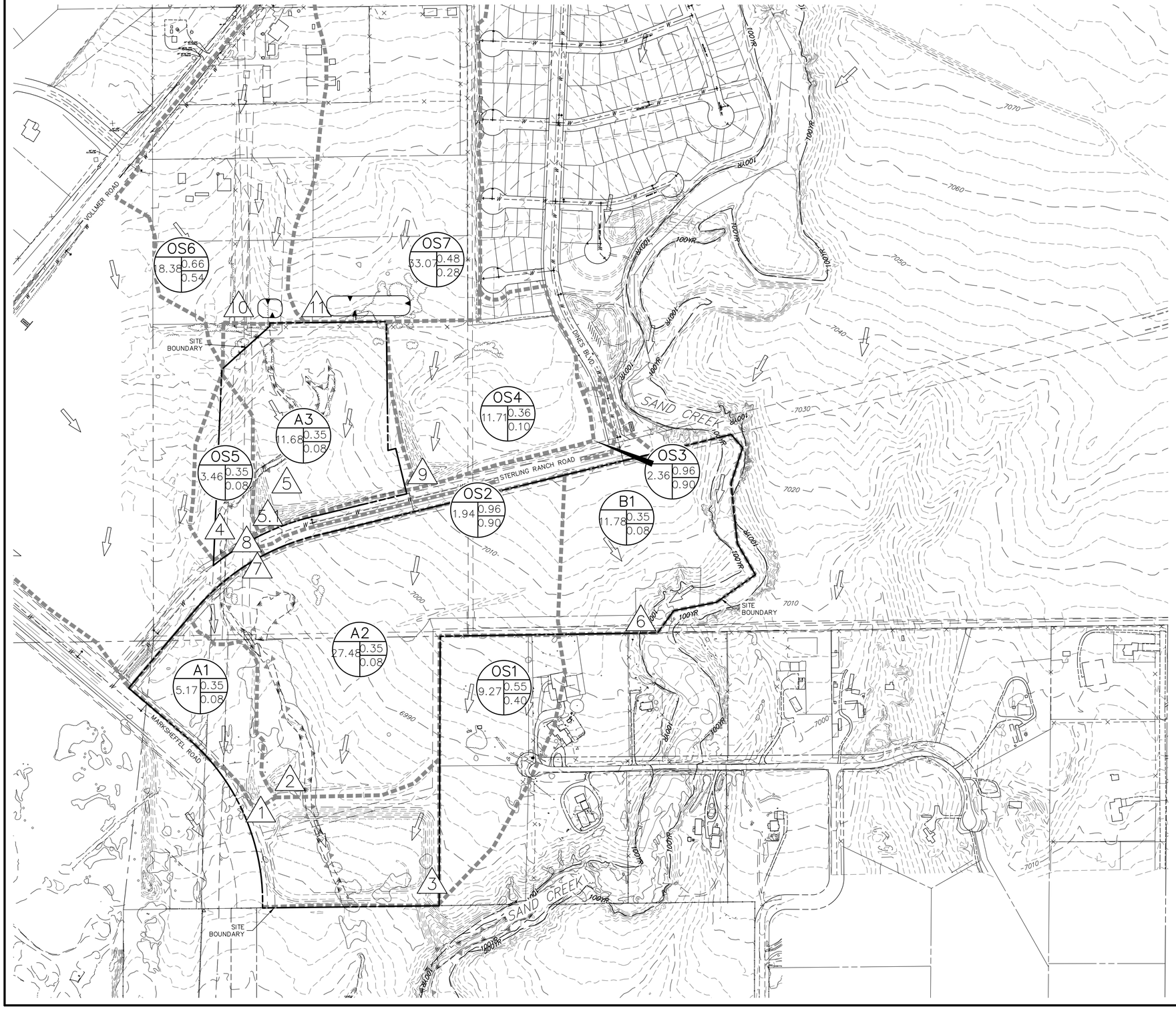
Appendix E
Drainage Maps

STERLING RANCH PHASE 2

EXISTING DRAINAGE MAP



LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C –100 YR
D: C–5 YR

DESIGN POINT

EXISTING FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

SITE BOUNDARY
EXISTING PROPERTY LINE
ROW EXISTING
FL EXISTING
SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING

6100

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	5.17	2%	0.08	0.35	27.4	1.1	8.0
A2	27.48	0%	0.08	0.35	39.1	4.6	33.6
A3	11.68	0%	0.08	0.35	19.5	2.9	21.5
B1	11.78	0%	0.08	0.35	25.2	2.6	19.0
OS1	9.27	37%	0.40	0.55	23.7	10.5	24.4
OS2	5.00	100%	0.90	0.96	14.2	6.3	11.2
OS3	2.36	100%	0.90	0.96	12.2	8.1	14.6
OS4	11.71	4%	0.10	0.36	32.4	2.8	16.9
OS5	3.46	0%	0.08	0.35	30.4	0.7	5.0
OS6	18.38	11%	0.54	0.66	14.8	35.4	72.2
OS7	33.07	19%	0.28	0.48	34.7	20.6	60.4

DESIGN POINT

DP	Q5 Total	Q100 Total
1	1.1	8.0
2	4.6	33.6
3	10.5	24.4
4	0.7	5.0
6	2.6	19.0
7	6.3	11.2
8	8.1	14.6
9	2.8	16.9
10	35.4	72.2
11	20.6	60.4
5	2.9	21.5
5.1	62.7	168.9

TITLE

EXISTING GRADING ASSUMES FILING 2, STERLING RANCH ROAD, & MARKSHEFFEL ROAD ARE BUILT.

N

300 150 0 300 600

ORIGINAL SCALE: 1" = 300'

STERLING RANCH PHASE 2
EXISTING DRAINAGE MAP
JOB NO. 25188.02
04/26/21
SHEET 1 OF 1

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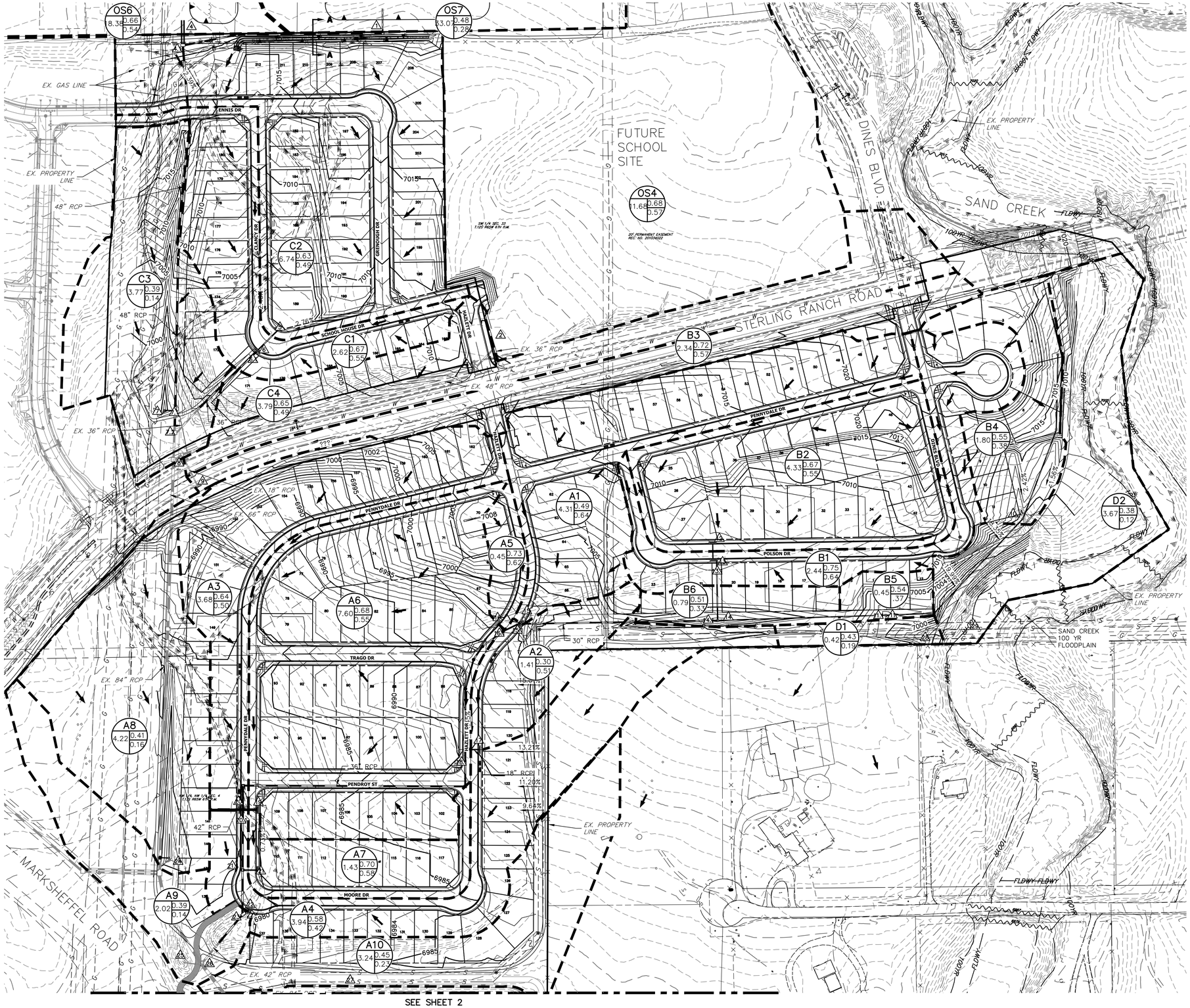
STERLING RANCH PHASE 2
PROPOSED DRAINAGE MAP



100 50 0 100 200

ORIGINAL SCALE: 1" = 100'

LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C –100 YR
D: C–5 YR

DESIGN POINT

PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

FL EXISTING

SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

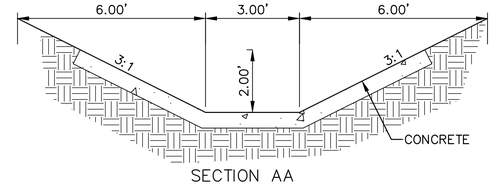
EXISTING 6100

PROPOSED 6100

DESIGN POINT		
DP	Q5 Total	Q100 Total
1	20.6	60.4
2	20.5	40.5
3	20.4	40.3
4	35.4	72.2
4.1	42.5	104.3
5	12.0	25.9
6	5.5	11.4
6.1	17.0	36.4
7	2.2	9.9
7.1	42.7	109.8
7.2	52.8	132.1
8	4.6	10.2
9	3.5	7.3
10	2.3	5.6
11	0.7	1.7
11.1	2.8	6.8
12	9.1	18.7
13	6.2	12.0
13.1	15.0	30.3
14	0.8	2.2
14.1	16.1	34.1
15	8.1	17.4
15.1	21.8	43.9
16	1.4	2.9
16.1	22.7	45.6
17	2.0	8.5
18.1	24.2	53.1
19	15.3	31.4
20	6.8	14.6
20.1	37.9	74.7
21	7.5	21.7
21.1	44.1	93.0
22	5.7	17.2
22.1	5.7	17.2
23	48.6	107.2
24	2.2	9.1
25	0.7	3.5
27	2.4	8.0
28	0.3	1.3
29	1.9	10.5

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	4.31	64%	0.49	0.64	12.5	8.1	17.4
A2	1.41	31%	0.30	0.50	16.3	1.4	4.0
A3	3.68	65%	0.50	0.64	13.2	6.8	14.6
A4	3.94	52%	0.42	0.58	15.4	5.7	13.4
A5	0.45	78%	0.62	0.73	5.0	1.4	2.9
A6	7.60	73%	0.55	0.68	13.9	15.3	31.4
A7	1.43	75%	0.58	0.70	13.7	3.0	6.1
A8	4.22	13%	0.16	0.41	18.9	2.2	9.1
B1	2.44	80%	0.64	0.75	11.4	6.2	12.0
B2	4.33	73%	0.55	0.67	12.2	9.1	18.7
C1	2.62	69%	0.54	0.67	11.9	5.5	11.4
C2	6.74	63%	0.49	0.63	14.1	12.0	25.9
C3	3.77	10%	0.14	0.39	11.3	2.2	9.9
A9	2.02	8%	0.13	0.39	25.8	0.7	3.5
A10	3.23	24%	0.23	0.45	17.6	2.4	8.0
B6	0.78	44%	0.33	0.51	18.5	0.8	2.2
B5	0.45	51%	0.37	0.54	8.8	0.7	1.7
B4	1.80	51%	0.38	0.55	16.2	2.3	5.6
B3	2.36	61%	0.57	0.72	27.9	3.5	7.3
C4	3.79	55%	0.49	0.65	30.3	4.6	10.2
D1	0.42	12%	0.17	0.42	9.2	0.3	1.3
D2	3.67	5%	0.12	0.38	7.8	1.9	10.5
OS6	18.38	54%	0.54	0.66	14.8	35.4	72.2
OS4	11.71	56%	0.57	0.68	20.5	20.5	40.5
OS7	33.07	23%	0.28	0.48	34.7	20.6	60.4



SECTION AA
OVERFLOW DITCH CROSS SECTION

OVERFLOW = 84.4 CFS
VELOCITY = 8.66 FT/S
SHEAR STRESS =4.05 LB/FT^2

STERLING RANCH PHASE 2
PROPOSED DRAINAGE MAP
JOB NO. 25188.00
09/10/21
SHEET 1 OF 2

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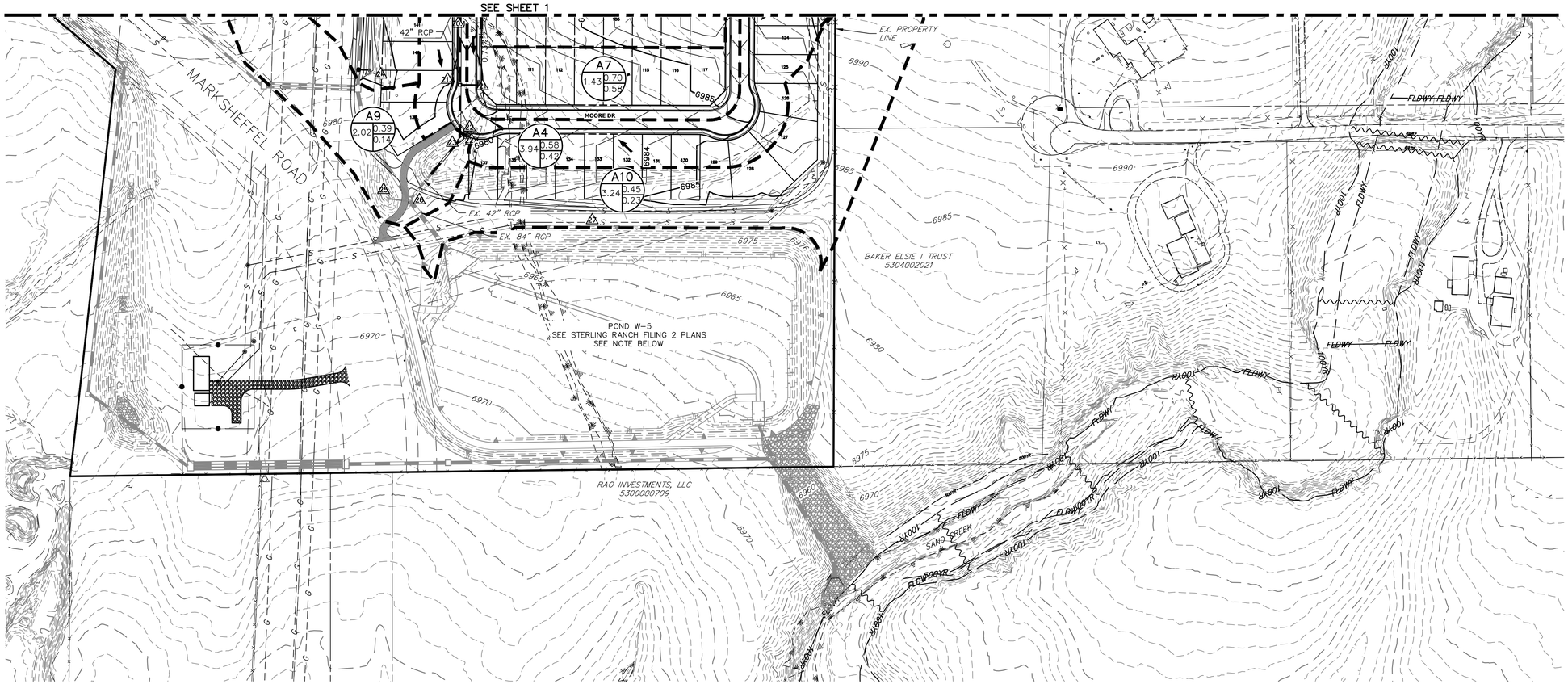
STERLING RANCH PHASE 2
PROPOSED DRAINAGE MAP



NOTE:

FOR ADDITIONAL INFORMATION REGARDING DESIGN POINTS, ROUTING, AND RUNOFF VALUES ASSOCIATED WITH POND W-5. REFER TO THE FILING 2 DRAINAGE MAP, AS SHOWN IN APPENDIX D OF THIS REPORT.

LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C -100 YR
D: C-5 YR

DESIGN POINT

PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

FL EXISTING

SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100 PROPOSED 6100

100 50 0 100 200

ORIGINAL SCALE: 1" = 100'

STERLING RANCH PHASE 2
PROPOSED DRAINAGE MAP
JOB NO. 25188.00
09/10/21
SHEET 2 OF 2

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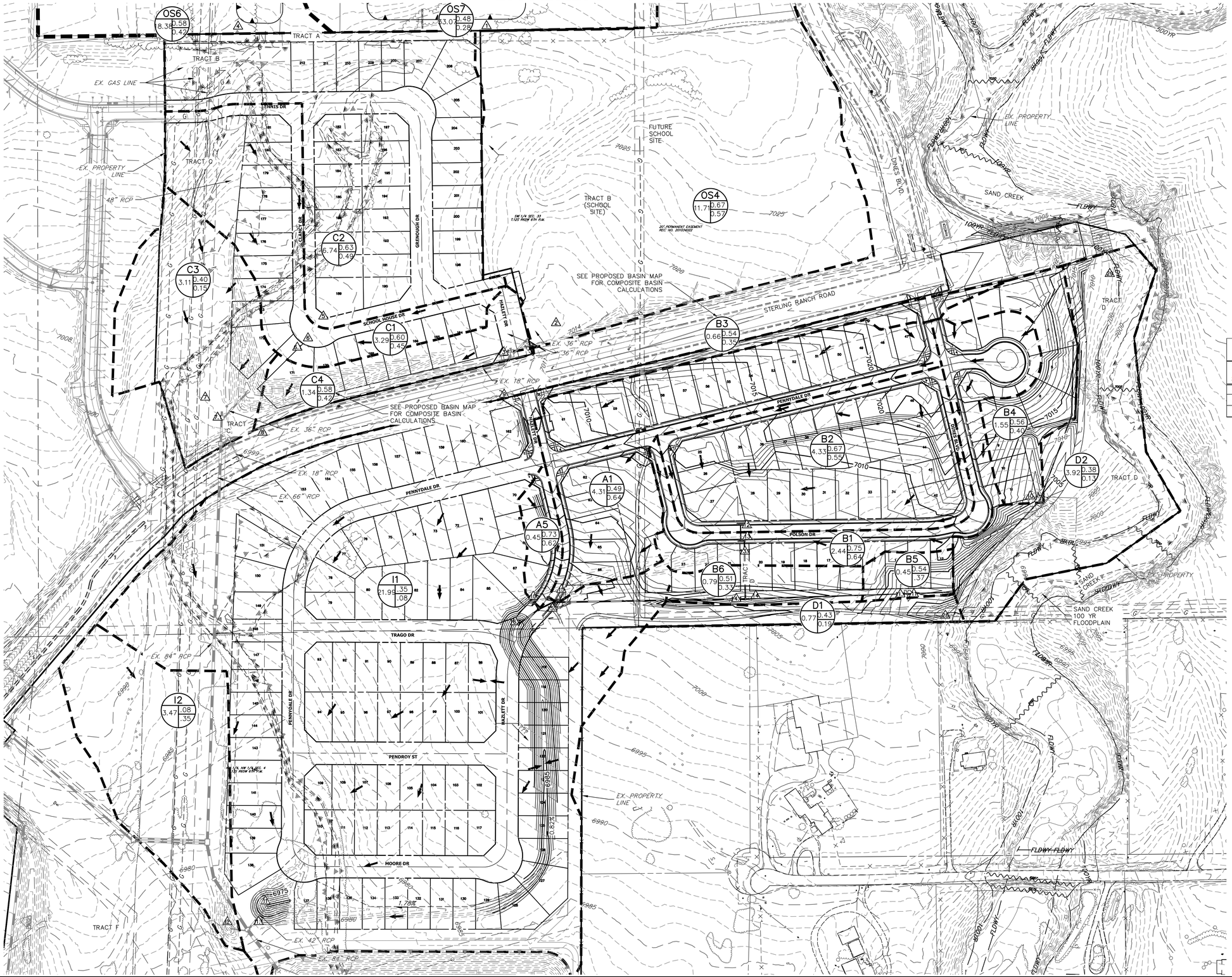
STERLING RANCH PHASE 2
INTERIM - PROPOSED DRAINAGE MAP



LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C –100 YR
D: C–5 YR

DESIGN POINT

PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

FL EXISTING

SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100 PROPOSED 6100

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
I1	21.99	1%	0.08	0.35	31.9	4.4	31.2
I2	3.47	0%	0.08	0.35	31.1	0.7	4.9

DESIGN POINT

DP	Q5 Total	Q100 Total
I1	4.4	31.2
I1.1	22.1	68.4
I2	0.7	4.9

N

100 50 0 100 200

ORIGINAL SCALE: 1" = 100'

STERLING RANCH PHASE 2
INTERIM CONDITION DRAINAGE MAP
JOB NO. 25188.00
9/10/21
SHEET 1 OF 1

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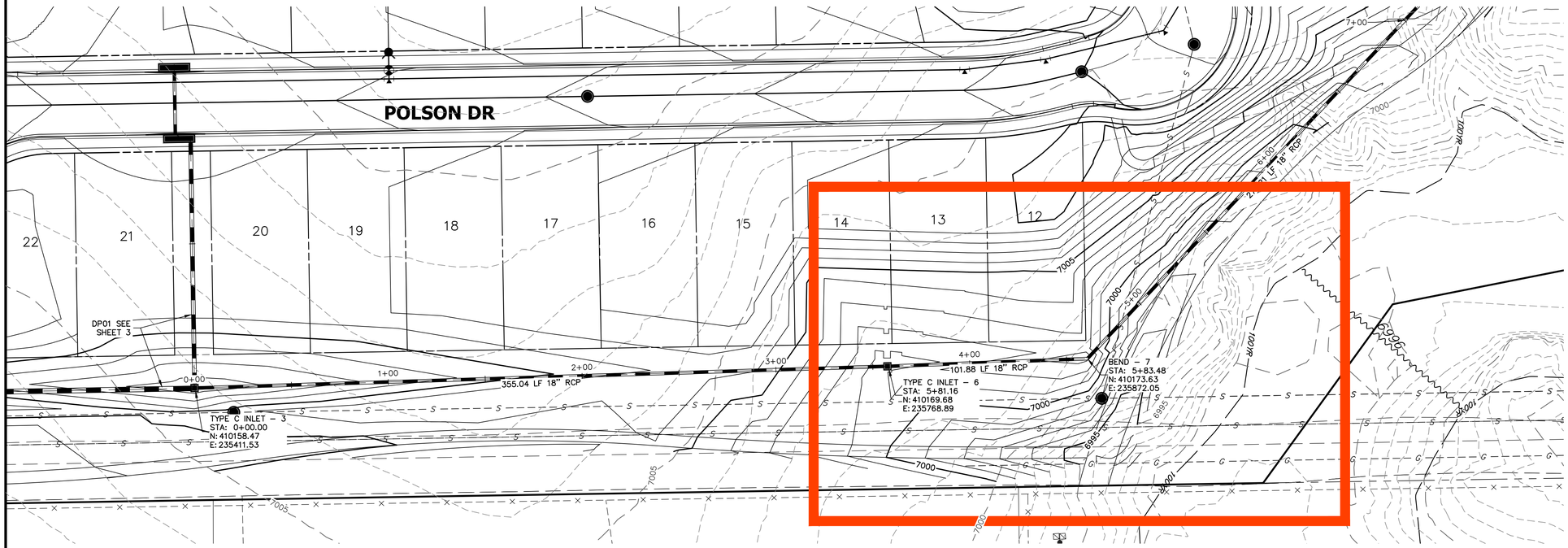
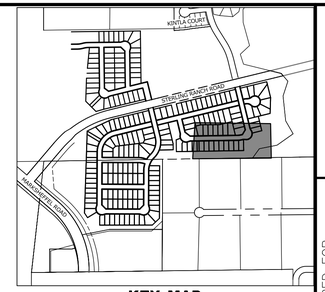
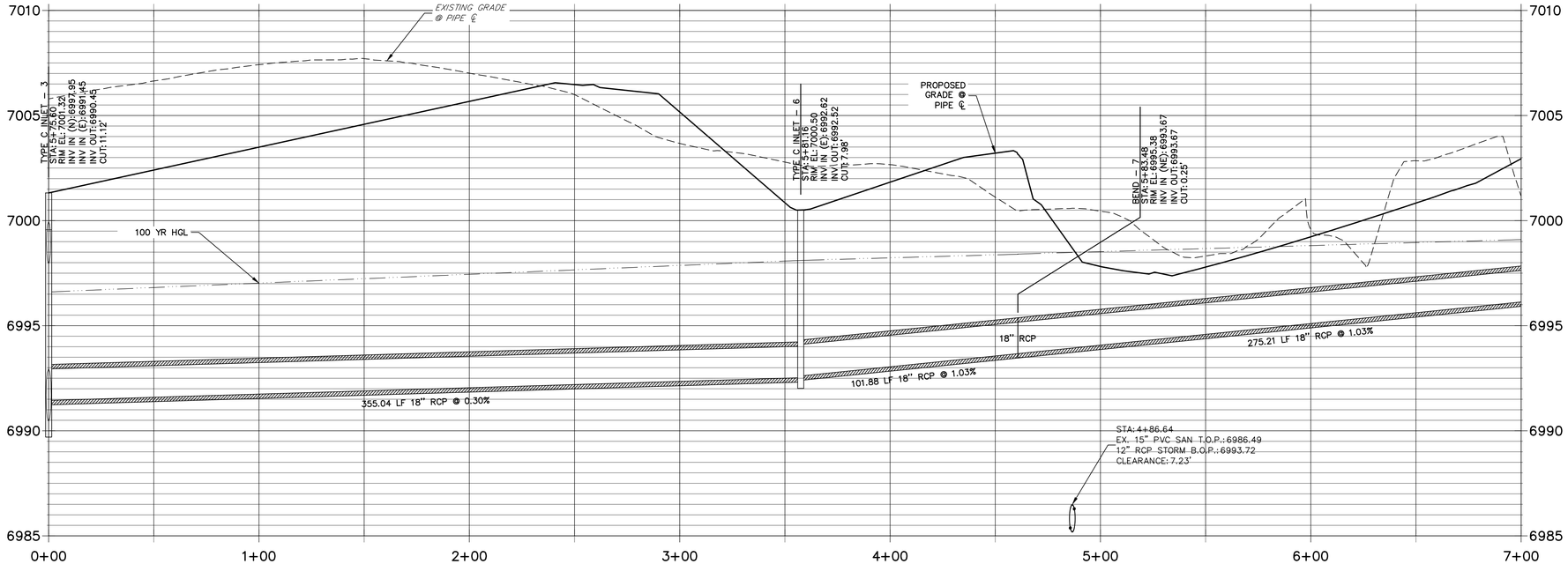




KEY MAP
SCALE: NTS

EXHIBIT 1 - GRADING CLOSE UP

DP-02 PROFILE (1)
STA 0+00.00 TO 7+00.00



30 15 0 30 60

HORIZONTAL
ORIGINAL SCALE: 1" = 30'
VERTICAL
ORIGINAL SCALE: 1" = 3'

Know what's **below.**
Call before you dig.

ENGINEER'S STATEMENT

PREPARED UNDER MY DIRECT SUPERVISION AND ON BEHALF OF JR ENGINEERING

MIKE A. BRAMLETT, P.E.
COLORADO P.E. 32314
FOR AND ON BEHALF OF JR ENGINEERING, LLC

DATE

COLORADO REGISTERED MIKE A. BRAMLETT 32314 PROFESSIONAL ENGINEER

FILE NO. SF–21–XXX

UNTIL SUCH TIME AS THESE DRAWINGS ARE APPROVED BY THE APPROPRIATE REVIEWING AGENCIES, JR ENGINEERING APPROVES THEIR USE ONLY FOR THE PURPOSES DESIGNATED BY WRITTEN AUTHORIZATION.

PREPARED FOR
SR LAND, LLC
20 BOULDER CRESCENT
SUITE 201
COLORADO SPRINGS, CO 80903
JAMES F. MORLEY
(719) 471–1742

J·R ENGINEERING
A Westrian Company
Centennial 303–740–9393 • Colorado Springs 719–593–2593
Fort Collins 970–491–9888 • www.jrengineering.com

No.	REVISION	BY	DATE

H–SCALE	1" = 30'	DESIGNED BY	ARJ
V–SCALE	1" = 3'	DRAWN BY	ARJ
DATE	06/18/21	CHECKED BY	

STERLING RANCH FILING 3

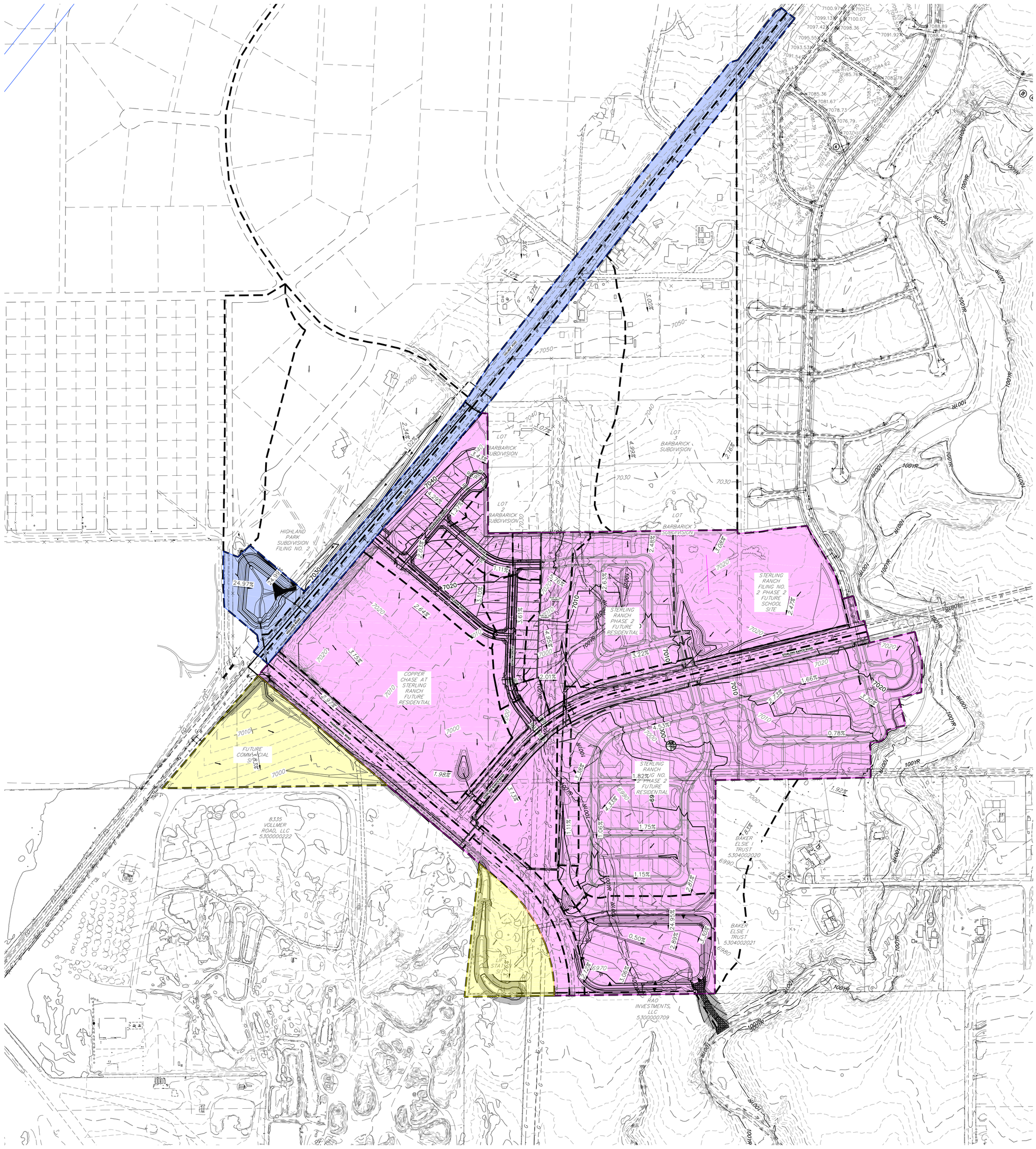
STORM SEWER PLAN

SHEET OF

JOB NO. 25188.00

STERLING RANCH FILING 2/PHASE 2

PERMANENT BMP APPLICABILITY MAP



LEGEND:

- PROPOSED STORM SEWER
- 5000 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5000 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN
- A = BASIN DESIGNATION
  B = AREA IN ACRES
  C = 5-YR RUNOFF COEFFICIENT
  D = 100-YR RUNOFF COEFFICIENT
- 1 DESIGN POINT
- HP HIGH POINT
- LP LOW POINT
- DRAINAGE ARROW
- EXISTING DRAINAGE ARROW
- PROPOSED DRAINAGE SWALE
- AREA DETAINED IN POND W5
- AREA DETAINED IN POND W4
- FUTURE BMP

200 100 0 200 400

ORIGINAL SCALE: 1" = 200'

MS4 PERMIT EXCLUSION AREA
STERLING RANCH FILING 2 / PHASE 2
JOB NO. 25188.01
06/02/2020
SHEET 1 OF 1

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