

**FINAL DRAINAGE REPORT  
FOR  
MVEA YODER ELECTRIC SUBSTATION  
EL PASO COUNTY, COLORADO**

**SEPTEMBER 2018**

Prepared For:  
**MOUNTAIN VIEW ELECTRIC ASSOCIATION**  
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Job No. 1802.00

**FINAL DRAINAGE REPORT  
FOR  
MVEA YODER ELECTRIC SUBSTATION**

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**REQUIRED MAPS AND DRAWINGS**

VICINITY MAP

S.C.S. SOILS MAP

FEMA FIRM MAP

HYDROLOGIC CALCULATIONS

DRAINAGE PLAN

**CERTIFICATION STATEMENT:**

Engineers Statement

This attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

\_\_\_\_\_  
Quentin Armijo, P.E. 37170

\_\_\_\_\_  
Seal

Developers Statements

I, Mountain View Electric Association, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

Mountain View Electric Association

Business Name

By: \_\_\_\_\_

Title: \_\_\_\_\_

Address: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_  
El Paso County Approval:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 & 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

\_\_\_\_\_  
Jennifer Irvine,  
County Engineer / ECM Administrator

\_\_\_\_\_  
Date

Conditions:

**FINAL DRAINAGE REPORT  
FOR  
MVEA YODER ELECTRIC SUBSTATION**

**PURPOSE**

The purpose of this Final Drainage Report is to identify and analyze the proposed drainage patterns, determine proposed runoff quantities, size drainage structures for conveyance of developed runoff, and present solutions to drainage impacts on-site and off-site resulting from this development. In an effort to protect receiving water and as part of the “four step process to minimize adverse impacts of urbanization” this site was analyzed in the following manner:

1. Reduce Runoff- The new improvements to the site, which consist of adding gravel to the yard will be routed to a proposed private Sediment Basin. By capturing these flows in the sediment basin the developed runoff will be detained and reduce the quantity of downstream runoff.
2. Treat Slowly Release WQCV- The sediment basins will capture the developed flows and allow them to infiltrate, thereby also allowing solids and contaminants to settle out thus stopping downstream transport.
3. Stabilize Stream Channel- By reducing the rate of runoff the site is helping to stabilize the downstream Upper Pond creek and ultimately Chico Creek.
4. Source Controls- As this development will not include outdoor storage or the potential for the introduction of contaminants to the County’s MS4, since it is not an industrial or commercial site, no source controls are proposed or necessary.

**GENERAL DESCRIPTION**

This Final Drainage Report (FDR) is an analysis of approximately 5.0 acres of undeveloped land located just east of the residential house at 1625 N. Yoder Road. This site is being developed by our client to include an electric substation. The development will also include improving the dirt access road to gravel. The site is located in the southwest quarter of Section 3, Township 14 South, Range 61 West of the 6<sup>th</sup> Principal

Meridian currently within El Paso County, Colorado. The site is bounded to the north, west, & south by a 40 acres single family lots, and to the east by undeveloped open space. The site is contained within the Upper Pond Creek Basin.

Soils for this project are delineated by the map in the appendix as Bresser sandy loam (11) 0 to 3 percent slopes and Truckton sandy loam (97), 3 to 9 percent slopes. Soils in the study area are shown as mapped by S.C.S. in the “Soils Survey of El Paso County Area” and contains soils of Hydrologic Group B and A respectively.

### **FLOODPLAIN STATEMENT**

No portion of this site is within a designated F.E.M.A. floodplain, as determined by Flood Insurance Rate Map No. 08041C0875 F, dated March 17, 1997 (see appendix).

### **EXISTING DRAINAGE CONDITIONS**

The site has not been previously developed and is currently part of a 40 acre single family parcel. The site consists mostly of natural vegetative grass and weeds, with some areas of bare ground. There is a natural ridge that runs north south through the site and splits it. The site has been broken down into three existing design points 1, 2 & 3, two existing onsite basins EXA & EXB and three existing offsite basins OS-1, OS-2 & OS-3 in order to show the historic drainage flows. Below is a description of them. See appendix for calculations.

Offsite Basin OS-1 (11.85 acres;  $Q_5=2.7$  cfs and  $Q_{100}=17.4$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from north to south and drains onto Basin EXA.

Basin EXA (3.83 acres;  $Q_5=1.1$  cfs and  $Q_{100}=7.4$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from north to south. The combined flow ( $Q_5=3.5$  cfs and  $Q_{100}=23.0$  cfs) of Basin OS-1 and EXA sheet flows south in an existing broad swale and then to a low point at the south boundary (Design Point 1) where it ponds and then overtops the weir offsite.

Offsite Basin OS-2 (0.33 acres;  $Q_5=0.1$  cfs and  $Q_{100}=0.7$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from northwest to southeast and partially drains onto Basin EXB.

Basin EXB (1.17 acres;  $Q_5=0.4$  cfs and  $Q_{100}=2.7$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from northwest to southeast. The combined flow ( $Q_5=0.5$  cfs and  $Q_{100}=3.4$  cfs) of Basin OS-2 and EXB sheet flows southeast into an existing offsite natural channel (Design Point 2).

Offsite Basin OS-3 (7.79 acres;  $Q_5=1.8$  cfs and  $Q_{100}=11.5$  cfs) consist of undeveloped open space prairie and a section of a dirt access road. Drainage in this basin sheet flows from north to south and drains across the existing dirt access road along the south part of the basin and continues south onto the adjacent lot.

### **PROPOSED DRAINAGE CONDITIONS**

Runoff in the developed conditions will closely flow the historic drainage patterns with the exception of adding 3 Sediment Basin to capture and treat the runoff from the developed substation yard and gravel access road. For analysis the site has been broken down into four design points 1, 1A, 1B & 2, four onsite basins A, A1, A2, & B and the same three existing offsite basins OS-1, OS-2 & OS-3. Below is a description of the runoff in the developed conditions and how it will be safely routed and treated. See appendix for calculations.

Offsite Basin OS-1 (11.85 acres;  $Q_5=2.7$  cfs and  $Q_{100}=17.4$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from north to south and drains onto Basin A1.

Basin A1 (1.70 acres;  $Q_5=0.5$  cfs and  $Q_{100}=3.4$  cfs) consist of undeveloped open space prairie that will be inside the site boundary but will not have any improvements other

than a small portion of the proposed gravel road & ditch section and placing a 2' high berm on the north side of the yard to direct runoff to a broad swale, so the offsite flow can be routed around the substation yard. Drainage in this basin sheet flows to the broad swale (Design Point 1A). The combined flow ( $Q_5=3.1$  cfs and  $Q_{100}=19.9$  cfs) of Basin OS-1 and A1 is directed south in the broad swale and ditch section to a proposed sediment basin. In a major storm event an armored 21' weir a set 0.5' below the top will allow for the flow to be safely routed downstream offsite to Design Point 1 south boundary (Design Point 1B).

Basin A (1.65 acres;  $Q_5=1.3$  cfs and  $Q_{100}=4.3$  cfs) will consist of the proposed substation yard with ground cover comprised of loose gravel. Drainage in this basin sheet flows south to a proposed 2' deep depression acting as a sediment trap (Design Point 1B). At the depression the runoff from the gravel covered yard will be treated by allowing it to infiltrate and also detaining developed flows from the new improvements within the electric yard. In a major storm event an armored 5' weir a set 0.5' below the top will allow for the flow to be safely routed downstream offsite to Design Point 1.

Basin A2 (0.47 acres;  $Q_5=0.2$  cfs and  $Q_{100}=1.1$  cfs) will consist undeveloped land with some gravel drive in the area just south of the proposed sediment trap. Drainage in this basin sheet flows south to Design Point 1. The combined flow of Basins OS-1, A, A1, & A2 at Design Point 1 is  $Q_5=4.2$  cfs and  $Q_{100}=23.7$  cfs without the sediment traps. With the sediment traps the overall flow will be reduced to  $Q_5=0.2$  cfs and  $Q_{100}=1.1$  cfs by capturing Basins OS-1, A, & A1 flow and allowing it to infiltrate. This developed runoff is less than the existing of  $Q_5=3.5$  cfs and  $Q_{100}=23.0$  cfs. Water rights are not impacted as the runoff is allowed to infiltrate and continue to downstream users.

As in the historic condition Offsite Basin OS-2 (0.33 acres;  $Q_5=0.1$  cfs and  $Q_{100}=0.7$  cfs) consist of undeveloped open space prairie. Drainage in this basin sheet flows from northwest to southeast and partially drains onto Basin EXB.

Basin B (1.17 acres;  $Q_5=0.4$  cfs and  $Q_{100}=2.7$  cfs) consist of undeveloped open space prairie inside the property, but is not being improved. Drainage in this basin sheet flows from northwest to southeast. The combined flow ( $Q_5=0.5$  cfs and  $Q_{100}=3.4$  cfs) of Basin OS-2 and EXB sheet flows southeast into an existing offsite natural channel (Design Point 2).

Offsite Basin OS-3 (7.79 acres;  $Q_5=1.9$  cfs and  $Q_{100}=11.6$  cfs) consist of undeveloped open space prairie and a section of a gravel access road. Drainage in this basin sheet flows from north to south in order to limit the small increase ( $Q_5=0.1$  cfs and  $Q_{100}=0.1$  cfs) in developed flow and to slow the runoff down before crossing the gravel road and washing the gravel away we have installed a sediment basin. The runoff will be allowed to infiltrate thus reducing the runoff while treating it for water quality. In a major storm event an armored 12.5' weir a set 0.5' below the top will allow for the flow to be safely routed downstream across the gravel access road along the south part of the basin and continues south onto the adjacent lot. With the gravel road tilted north back onto the site the washing off gravel onto the adjacent south property should not be an issue. If it is washed offsite the owner shall clean it up as soon as possible and the owner is tasked with maintaining the gravel and keeping it from being washed offsite.

Is this the owner of the property or MVEA? Please clarify.

### **HYDROLOGIC CALCULATIONS**

Hydrologic calculations were performed using the El Paso County Storm Drainage Design Criteria Manual - Volumes 1 & 2, latest editions. The Rational Method was used to estimate storm water runoff anticipated from design storms with 5-year and 100-year recurrence intervals. The Urban Drainage Criteria Manual was used to calculate the detention and water quality volume.

### **HYDRAULIC CALCULATIONS**

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County Storm Drainage Design Criteria Manual – Volumes 1 & 2, latest editions. The pertinent data sheets are included in the appendix of this report.

**EROSION CONTROL**

An erosion control plan is included with this drainage report. Vehicle Tracking Control (VTC) will be placed at any entrance to the site. A Concrete Washout (CW) will be placed on site, as well as a Materials Staging Area (SSA) and a Dirt Stockpile (SP) location. Silt Fence (SF) will be placed around the SP and Sediment Control Logs (SCL) are to be placed at the southern border of the site to keep runoff in place.

**MAINTENANCE**

The Sediment Basins are private and therefore must be maintained by the owner. A systematic inspection of all the above mentioned protective devices shall be performed using the operation and maintenance manual associated with this project once every 14 days. Additional inspections may be required prior to anticipated precipitation events and after precipitation events. Post-storm event inspections must be conducted within 24 hours after the end of any precipitation or snowmelt event that causes surface erosion. Provided the timing is appropriate, the post storm inspections may be used to fulfill the 14-day routine inspection requirement. A more frequent inspection schedule than the minimum inspections described may be necessary to ensure that BMPs continue to operate as needed to comply with the plan.

**CONSTRUCTION COST OPINION**

**Public Non Reimbursable**

**NOT APPLICABLE**

**Private Non Reimbursable**

1. Sediment Trap	3 EA	\$ 3,000	<u>    \$ 9,000</u>
			<b>Total \$ 9,000</b>

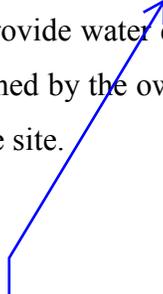
**DRAINAGE FEES**

The existing site is in the Upper Pond Creek Basin. This is a no fee basin.

## **SUMMARY**

Development of this site will not adversely affect the surrounding development. Proposed flows, as detailed in this report, will follow the drainage patterns outlined in this report showing how runoff will be safely routed downstream. The Extended Detention Basins will control developed flow to historic levels and provide water quality for this site. These water features will need to be periodically maintained by the owner in order to maintain their effectiveness in cleaning the discharge from the site.

**PREPARED BY:**  
**TERRA NOVA ENGINEERING, INC.**



Extended Detention Basins are not part of the design for this project. Please correct.

Quentin Armijo, P.E.  
Senior Project Manager  
Jobs/1802.00/drainage/180200 - FDR.doc

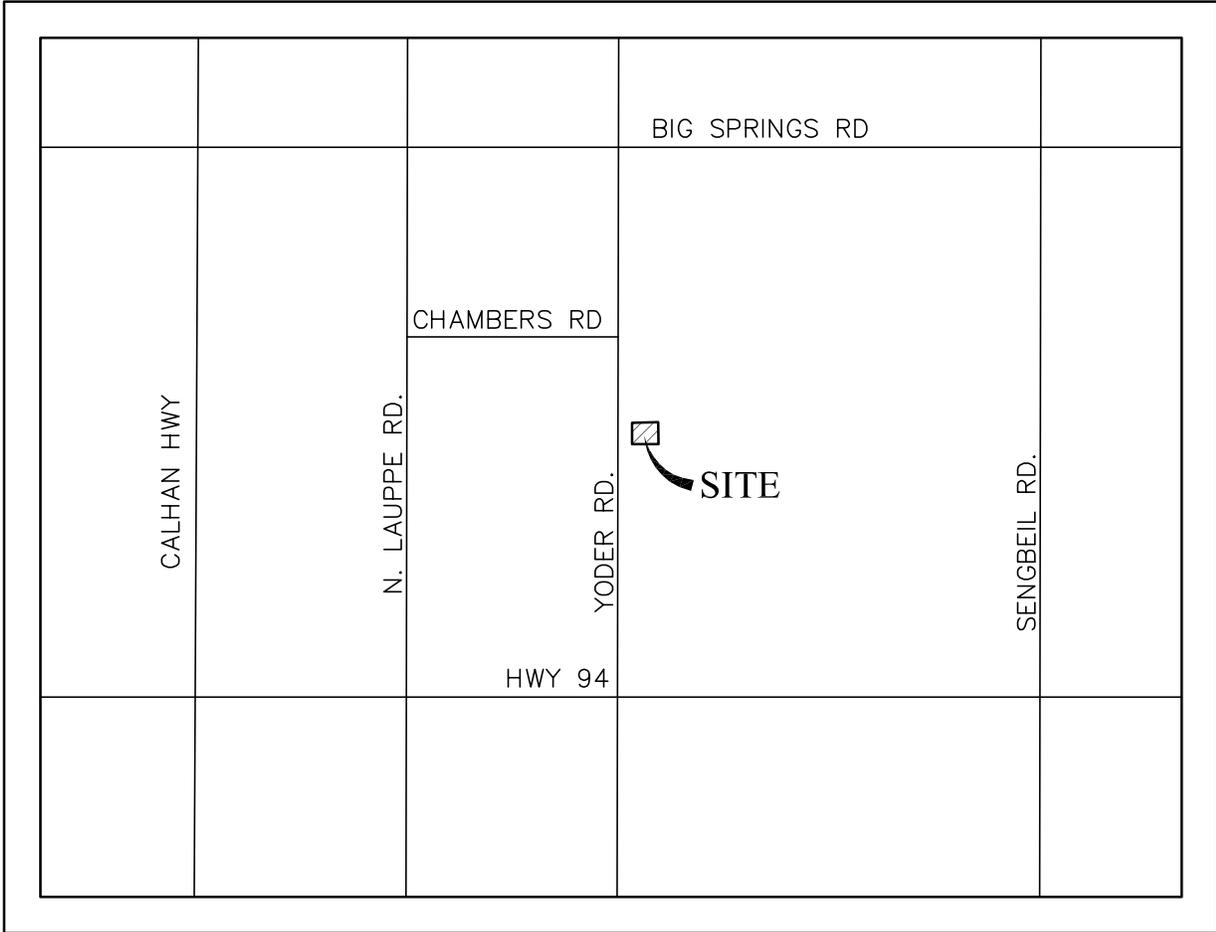
## **REFERENCE**

“El Paso County Drainage Criteria Manual-Volumes 1 & 2, latest edition”

SCS Soils Map for El Paso County

Federal Emergency Management Agency (FEMA) flood maps

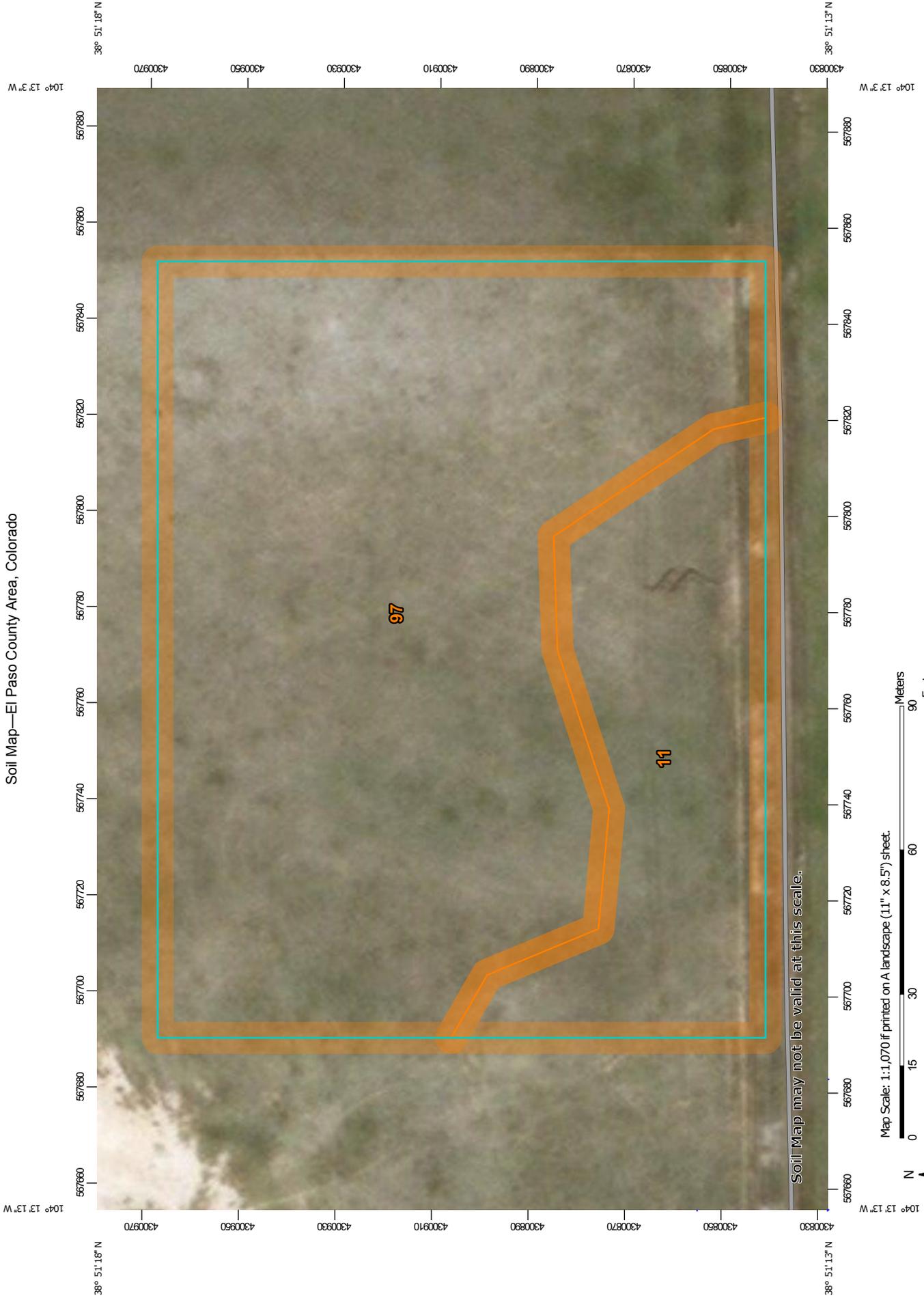
## **VICINITY MAP**



VICINITY MAP  
N.T.S.

**S.C.S. SOILS MAP**

Soil Map—El Paso County Area, Colorado



## MAP LEGEND

- Area of Interest (AOI)
- Area of Interest (AOI)
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points
- Special Point Features**
  - Blowout
  - Borrow Pit
  - Clay Spot
  - Closed Depression
  - Gravel Pit
  - Gravelly Spot
  - Landfill
  - Lava Flow
  - Marsh or swamp
  - Mine or Quarry
  - Miscellaneous Water
  - Perennial Water
  - Rock Outcrop
  - Saline Spot
  - Sandy Spot
  - Severely Eroded Spot
  - Sinkhole
  - Slide or Slip
  - Sodic Spot
- Water Features**
  - Streams and Canals
- Transportation**
  - Rails
  - Interstate Highways
  - US Routes
  - Major Roads
  - Local Roads
- Background**
  - Aerial Photography
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
 Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

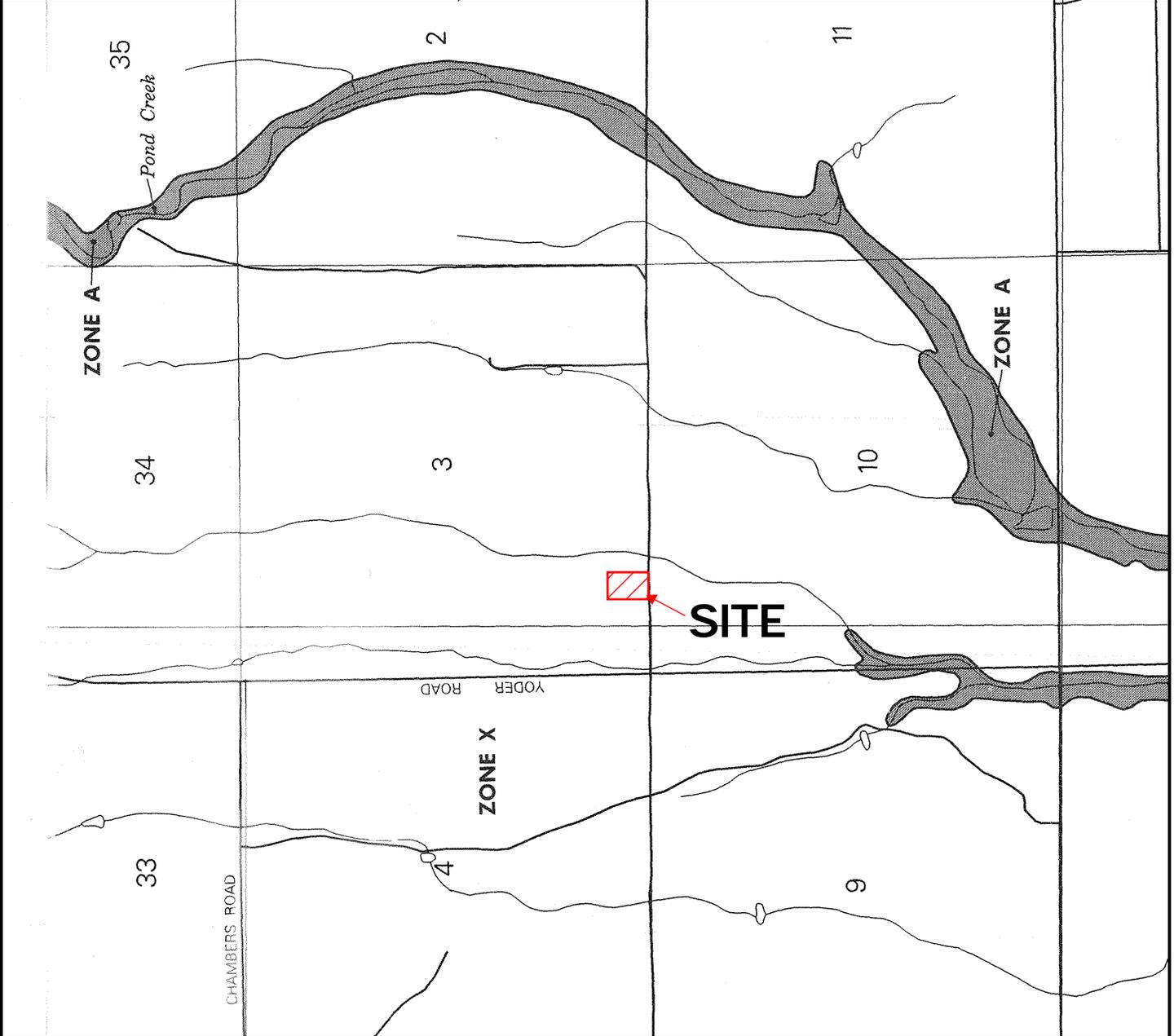
Date(s) aerial images were photographed: May 22, 2016—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
11	Bresser sandy loam, cool, 0 to 3 percent slopes	1.2	24.5%
97	Truckton sandy loam, 3 to 9 percent slopes	3.8	75.5%
<b>Totals for Area of Interest</b>		<b>5.0</b>	<b>100.0%</b>

**FEMA FIRM MAP**



**NATIONAL FLOOD INSURANCE PROGRAM**

**FIRM**  
**FLOOD INSURANCE RATE MAP**  
 EL PASO COUNTY,  
 COLORADO AND  
 INCORPORATED AREAS

**PANEL 875 OF 1300**  
 (SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:  
 COMMUNITY NUMBER PANEL SUFFIX  
 EL PASO COUNTY, UNINCORPORATED AREAS 080059 0875 F

**MAP NUMBER 08041C0875 F**  
**EFFECTIVE DATE: MARCH 17, 1997**



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps, check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)

## **HYDROLOGIC CALCULATIONS**

**MVEA YODER ELECTRIC SUBSTATION**  
**(Area Runoff Coefficient Summary)**

**HISTORIC**

BASIN	DEVELOPED			UNDEVELOPED			WEIGHTED		
	TOTAL AREA (Acres)	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	C <sub>100</sub>
OS-1	11.85	0.00	0.30	0.50	11.85	0.09	0.36	0.09	0.36
OS-2	0.33	0.00	0.30	0.50	0.33	0.09	0.36	0.09	0.36
OS-3	7.79	0.00	0.30	0.50	7.79	0.09	0.36	0.09	0.36
EXA	3.83	0.00	0.30	0.50	3.83	0.09	0.36	0.09	0.36
EXB	1.17	0.00	0.30	0.50	1.17	0.09	0.36	0.09	0.36

QNA

Date: 8/3/2018

Checked by: \_\_\_\_\_

**DEVELOPED**

BASIN	DEVELOPED			UNDEVELOPED			WEIGHTED		
	TOTAL AREA (Acres)	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	C <sub>100</sub>
OS-1	11.85	0.00	0.30	0.50	11.85	0.09	0.36	0.09	0.36
OS-2	0.33	0.00	0.30	0.50	0.33	0.09	0.36	0.09	0.36
OS-3	7.79	0.23	0.30	0.50	7.56	0.09	0.36	0.10	0.36
A	1.65	1.07	0.30	0.50	0.58	0.09	0.36	0.23	0.45
A1	1.70	0.06	0.30	0.50	1.65	0.09	0.36	0.10	0.36
A2	0.47	0.11	0.30	0.50	0.36	0.09	0.36	0.14	0.39
B	1.17	0.00	0.30	0.50	1.17	0.09	0.36	0.09	0.36

QNA

Date: 8/3/2018

Checked by: \_\_\_\_\_

**MVEA YODER SUBSTATION  
AREA DRAINAGE SUMMARY**

**HISTORIC**

BASIN	AREA TOTAL (Acres)	WEIGHTED		OVERLAND			STREET / CHANNEL FLOW				INTENSITY		TOTAL FLOWS		
		C <sub>5</sub>	C <sub>100</sub>	Length (ft)	Height (ft)	T <sub>c</sub> (min)	Length (ft)	Slope (%)	Velocity (fps)	T <sub>t</sub> (min)	T <sub>t</sub> TOTAL (min)	I <sub>5</sub> (in/hr)	I <sub>100</sub> (in/hr)	Q <sub>5</sub> (c.f.s.)	Q <sub>100</sub> (c.f.s.)
OS-1	11.85	0.09	0.36	100	1.2	17.8	1565	2.8%	2.2	11.9	29.6	2.5	4.1	2.7	17.4
OS-2	0.33	0.09	0.36	100	5.0	11.1	205	1.7%	2.1	1.6	12.7	3.7	6.4	0.1	0.7
OS-3	7.79	0.09	0.36	100	1.2	17.8	1555	2.6%	2.2	11.8	29.6	2.5	4.1	1.8	11.5
EXA	3.83	0.09	0.36	86	1.9	13.5	531	1.3%	2.0	4.4	17.9	3.2	5.4	1.1	7.4
EXB	1.17	0.09	0.36	100	5.0	11.1	170	2.4%	2.6	1.1	12.2	3.8	6.5	0.4	2.7

Calculated by: QNA  
Date: 8/3/2018  
Checked by: \_\_\_\_\_

**DEVELOPED**

BASIN	AREA TOTAL (Acres)	WEIGHTED		OVERLAND			STREET / CHANNEL FLOW				INTENSITY		TOTAL FLOWS		
		C <sub>5</sub>	C <sub>100</sub>	Length (ft)	Height (ft)	T <sub>c</sub> (min)	Length (ft)	Slope (%)	Velocity (fps)	T <sub>t</sub> (min)	T <sub>t</sub> TOTAL (min)	I <sub>5</sub> (in/hr)	I <sub>100</sub> (in/hr)	Q <sub>5</sub> (c.f.s.)	Q <sub>100</sub> (c.f.s.)
OS-1	11.85	0.09	0.36	100	1.2	17.8	1565	2.8%	2.2	11.9	29.6	2.5	4.1	2.7	17.4
OS-2	0.33	0.09	0.36	100	5.0	11.1	205	1.7%	2.0	1.7	12.8	3.7	6.3	0.1	0.7
OS-3	7.79	0.10	0.36	100	1.2	17.8	1555	2.6%	2.2	11.8	29.6	2.5	4.1	1.9	11.6
A	1.65	0.23	0.45	100	1.3	13.7	173	1.4%	2.0	1.4	15.2	3.5	5.9	1.3	4.3
A1	1.70	0.10	0.36	100	5.0	11.1	550	0.9%	1.5	6.1	17.2	3.3	5.5	0.5	3.4
A2	0.47	0.14	0.39	100	3.0	13.1	62	1.6%	2.1	0.5	13.6	3.6	6.2	0.2	1.1
B	1.17	0.09	0.36	100	3.0	13.1	63	5.4%	3.7	0.3	13.4	3.6	6.2	0.4	2.6

Calculated by: QNA  
Date: 8/3/2018  
Checked by: \_\_\_\_\_

**MVEA YODER SUBSTATION  
SURFACE ROUTING SUMMARY**

<b>HISTORIC</b>									
<b>Design Point(s)</b>	<b>Contributing Basins</b>	<b>Area (Acres)</b>	<b>Equivalent CA<sub>5</sub></b>	<b>Equivalent CA<sub>100</sub></b>	<b>Maximum T<sub>C</sub></b>	<b>Intensity</b>		<b>Flow</b>	
						<b>I<sub>5</sub></b>	<b>I<sub>100</sub></b>	<b>Q<sub>5</sub></b>	<b>Q<sub>100</sub></b>
1	OS-1 & EXA	15.69	1.41	5.65	29.6	2.5	4.1	3.5	23.0
2	OS-2, & EXB	1.49	0.13	0.54	12.7	3.7	6.4	0.5	3.4
3	OS-2, & EXB	7.79	0.70	2.81	29.6	2.5	4.1	1.8	11.5

<b>DEVELOPED</b>									
<b>Design Point(s)</b>	<b>Contributing Basins</b>	<b>Area (Acres)</b>	<b>Equivalent CA<sub>5</sub></b>	<b>Equivalent CA<sub>100</sub></b>	<b>Maximum T<sub>C</sub></b>	<b>Intensity</b>		<b>Flow</b>	
						<b>I<sub>5</sub></b>	<b>I<sub>100</sub></b>	<b>Q<sub>5</sub></b>	<b>Q<sub>100</sub></b>
1A	OS-1 & A1	13.55	1.23	4.89	29.6	2.5	4.1	3.1	19.9
1B	A	1.65	0.37	0.74	15.2	3.5	5.9	1.3	4.3
1*	OS-1, A, A1, & A2	15.67	0.07	5.82	29.6	2.5	4.1	0.2	23.7
1**	OS-1, A, A1, & A2	15.67	0.07	0.19	13.6	3.6	6.2	0.2	1.1
2	OS-1, & B	1.49	0.13	0.54	12.8	3.7	6.3	0.5	3.4
3	OS-3	7.79	0.75	2.84	29.6	2.5	4.1	1.9	11.6

1\* is the total flow without sediment basins.

1\*\* is the total flow with sediment basins.

Date: 9/18/2018

Checked by: \_\_\_\_\_

# **MVEA YODER SUBSTAION WEIR OUTLETS**

## **Broad Crested Weir**

### **DESIGN POINT 1A**

The general form of the equation for horizontal crested weirs is  $Q = CLH^{3/2}$  where:

Q = Weir flow discharge (cfs)	<b>19.9</b>	
C = Weir flow coefficient	2.640	
H = Depth of flow over the weir (ft)	0.50	Opening Height
L = Length of the weir (ft)	<b>21.4</b>	Length

### **DESIGN POINT 1B**

The general form of the equation for horizontal crested weirs is  $Q = CLH^{3/2}$  where:

Q = Weir flow discharge (cfs)	<b>4.3</b>	
C = Weir flow coefficient	2.640	
H = Depth of flow over the weir (ft)	0.50	Opening Height
L = Length of the weir (ft)	<b>4.7</b>	Length

### **DESIGN POINT 3**

The general form of the equation for horizontal crested weirs is  $Q = CLH^{3/2}$  where:

Q = Weir flow discharge (cfs)	<b>11.6</b>	
C = Weir flow coefficient	2.640	
H = Depth of flow over the weir (ft)	0.50	Opening Height
L = Length of the weir (ft)	<b>12.4</b>	Length

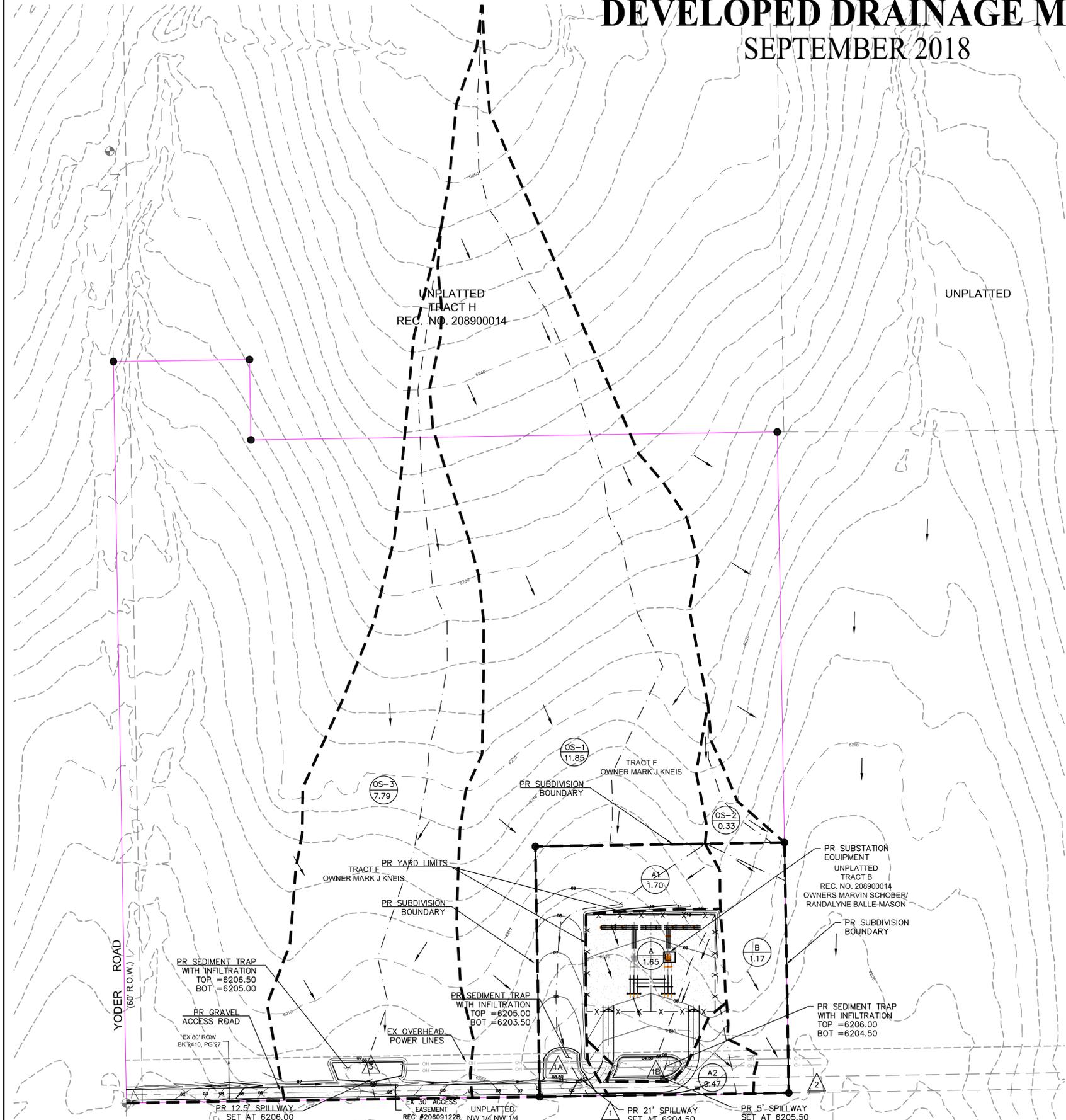
## **DRAINAGE MAPS**

# MVEA YODER SUBSTATION

## EL PASO COUNTY, CO

# DEVELOPED DRAINAGE MAP

### SEPTEMBER 2018

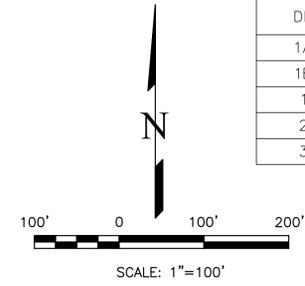


#### DESIGN POINT SUMMARY

DP	CONTRIBUTING BASINS	AREA AC.	Q5 CFS	Q100 CFS
1A	OS-A & A1	13.55	3.1	19.9
1B	A	1.65	1.3	4.3
1	OS-A, A, A1, & A2	15.69	4.2	23.7
2	OS-1 & B	1.49	0.5	3.4
3	OS-3	7.79	1.9	11.6

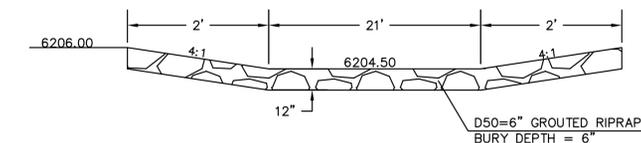
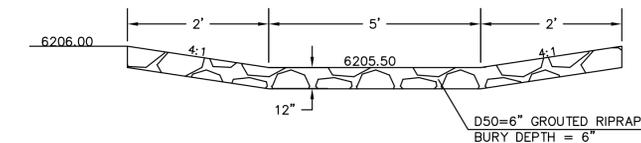
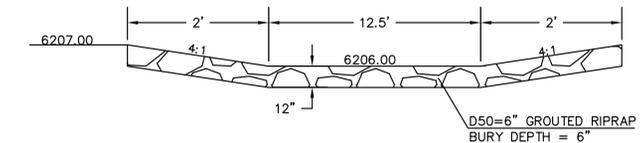
#### PROPOSED CONDITIONS

BASIN	ACRES	Q5 CFS	Q100 CFS
OS-1	11.85	2.7	17.4
OS-2	0.33	0.1	0.7
OS-3	7.79	1.9	11.6
A	1.65	1.3	4.3
A1	1.70	0.5	3.4
A2	0.47	0.2	1.1
B	1.17	0.4	2.6



#### LEGEND

- 10' EX CONTOUR: - - - - - 6810 - - - - -
- 2' EX CONTOUR: - - - - - 6802 - - - - -
- 10' PROP. CONTOUR: ———— 6810 ————
- 2' PROP. CONTOUR: ———— 6802 ————
- PROPOSED FLOW DIRECTION: →
- BASIN BOUNDARY: - - - - -
- TIME OF CONCENTRATION: ————
- BASIN ID: (A)
- ACREAGE: (0.37)
- DESIGN POINT: (3)
- PROPOSED: PR
- EXISTING: EX



REVISIONS	NO.	DESCRIPTION	DATE

UNTL SUCH TIME AS THESE DRAWINGS ARE APPROVED BY THE ENGINEERING BOARD OF THE STATE OF COLORADO. TERRA NOVA ENGINEERING, INC. APPROVES THEIR USE ONLY FOR THE PROJECT AND SITE SPECIFICALLY IDENTIFIED BY WRITTEN AUTHORIZATION.

PREPARED FOR:  
**MVEA**  
 ATTN: DAVE WALDNER  
 11140 E. WOODMEN RD  
 PEYTON, CO 80831  
 (719) 495-2283

**Terra Nova**  
 Engineering, Inc.  
 721 S. 2900 STREET  
 COLORADO SPRINGS, CO 80904  
 OFFICE: 719-635-6422  
 FAX: 719-635-6426  
 www.tnengine.com

**MVEA YODER SUBSTATION**  
 DEVELOPED DRAINAGE MAP

DESIGNED BY QNA
DRAWN BY QNA
CHECKED BY
H-SCALE 1"=100'
V-SCALE NA
JOB NO. 1802.00
DATE ISSUED 9/18/18
SHEET NO. 1 OF 1

# MVEA YODER SUBSTATION

## EL PASO COUNTY, CO

# EXISTING DRAINAGE MAP

### SEPTEMBER 2018

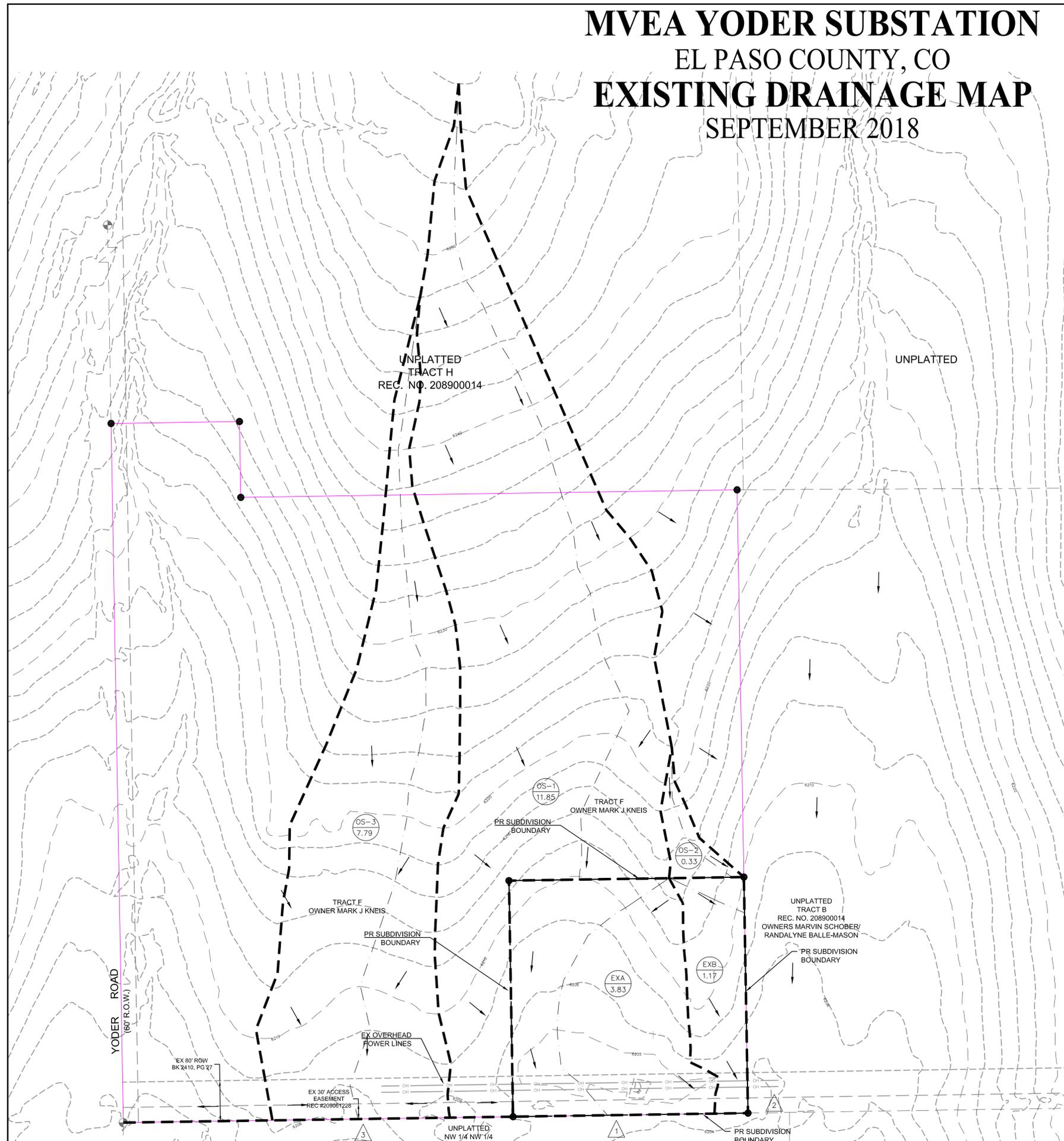
#### DESIGN POINT SUMMARY

DP	CONTRIBUTING BASINS	AREA AC.	Q5 CFS	Q100 CFS
1	OS-1 & EXA	15.69	3.5	23.0
2	OS-2 & EXB	1.49	0.5	3.4
3	OS-3	7.79	1.8	11.5

#### PROPOSED CONDITIONS

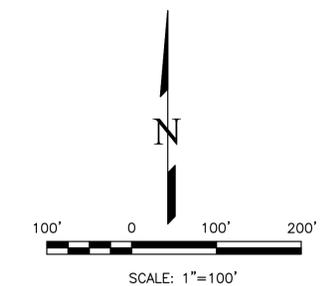
BASIN	ACRES	Q5 CFS	Q100 CFS
OS-1	11.85	2.7	17.4
OS-2	0.33	0.1	0.7
OS-3	7.79	1.8	11.5
EXA	3.83	1.1	7.4
EXB	1.17	0.4	2.7

This table shows the existing conditions. The table name should be "existing conditions" not "proposed conditions".



#### LEGEND

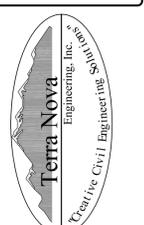
- 10' EX CONTOUR: --- 6810 ---
- 2' EX CONTOUR: --- 6802 ---
- FLOW DIRECTION: →
- BASIN BOUNDARY: - - - - -
- TIME OF CONCENTRATION: - - - - -
- BASIN ID: (A)
- ACREAGE: (0.37)
- DESIGN POINT: (3)
- EXISTING: EX
- PROPOSED: PR



REVISIONS NO.	DESCRIPTION	DATE

UNTIL SUCH TIME AS THESE DRAWINGS ARE APPROVED BY THE REVIEWING AGENCIES TERRA NOVA ENGINEERING, INC. APPROVES THEIR USE ONLY FOR THE PROJECT AND PURPOSE OF THE WRITTEN AUTHORIZATION.

PREPARED FOR:  
**MVEA**  
 ATTN: DAVE WALDNER  
 11140 E. WOODMEN RD  
 PEYTON, CO 80831  
 (719) 495-2283



721 S. 2900 STREET  
 COLORADO SPRINGS, CO 80904  
 OFFICE: 719-635-6422  
 FAX: 719-635-6426  
 www.tnaseinc.com

**MVEA YODER SUBSTATION**

EXISTING DRAINAGE MAP

DESIGNED BY QNA  
 DRAWN BY QNA  
 CHECKED BY  
 H-SCALE 1"=100'  
 V-SCALE NA  
 JOB NO. 1802.00  
 DATE ISSUED 9/18/18  
 SHEET NO. 1 OF 1

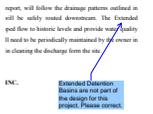
# Markup Summary

dsdgrimm (5)



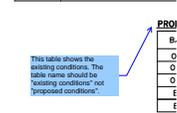
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**Author:** dsdgrimm  
**Date:** 10/25/2018 11:05:58 AM  
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Is this the owner of the property or MVEA? Please clarify.



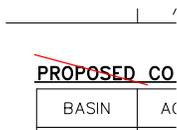
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Extended Detention Basins are not part of the design for this project. Please correct.



**Subject:** Engineer  
**Page Label:** 27  
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**Author:** dsdgrimm  
**Date:** 10/25/2018 11:06:01 AM  
**Color:** ■

This table shows the existing conditions. The table name should be "existing conditions" not "proposed conditions".



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**Page Label:** 27  
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**Color:** ■



**Subject:** Line  
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**Author:** dsdgrimm  
**Date:** 10/25/2018 11:06:04 AM  
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