



COLORADO
Division of Water Resources

Department of Natural Resources
Water Division 2 - Main Office

November 15, 2022

SR Land, LLC
% Jim Morley
20 Boulder Crescent, Suite 200
Colorado Springs, CO 80903

When replying, please refer to:
Homestead North F3 - Pond A Dam
Non-Jurisdictional Detention Pond NOI
Water Division 2, Water District 10

SUBJECT: Signed Notice of Intent to Construct a Non-Jurisdictional Water Impoundment Structure

Dear Mr. Morley,

Our office is in receipt of a Notice of Intent (NOI) to Construct a Non-Jurisdictional Water Impoundment Structure for the subject dam. The impoundment is to be located adjacent to Sand Creek, tributary to Fountain Creek, tributary to the Arkansas River, with the filling source to be Stormwater for Temporary Detention.

In accordance with Rule 11.1 of the Colorado Rules and Regulations for Dam Safety and Dam Construction, the hazard of this dam has been assessed as Low based on the construction drawing plans submitted with the NOI. A copy of the signed NOI is attached. An electronic copy will be maintained with the Division of Water Resources.

Please note the following:

- This structure must be designed and constructed to standards outlined in 37-92-602(8) for stormwater detention facilities.
- Because this structure is located on a tributary to Fountain Creek, the structure can only operate pursuant a Colorado Discharge Permit System Municipal, Separate Storm Sewer System Permit issued by the Department of Public Health and Environment Pursuant to Article 8 of Title 25, C.R.S.
- In the event groundwater is encountered during construction of the pond, the pond must be backfilled so as not to expose groundwater until such time as: 1) a well permit has been obtained for the groundwater pond pursuant to CRS §37-90-137, or 2) the pond is lined in accordance with the document, "State Engineer Guidelines for Lining Criteria for Gravel Pits," dated August 1999.

The requirements and recommendations provided herein are based on our review of the safety and water administration aspects of the proposed dam and the information provided in the



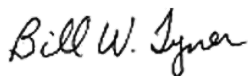
submitted NOI. These requirements and recommendations create no liability for the State of Colorado should the dam fail for any reason. Please be aware that it is in the owner's best interest to construct, operate, and maintain the structure in a safe manner, as he or she may be held liable in civil court for any downstream damages resulting from failure of the dam. A copy of Specifications for Construction of Non-Jurisdictional Dams is provided to assist you in the construction of a sound structure.

Finally, please be aware of any other permitting or regulatory requirements associated with the construction of a water impoundment structure, including but not limited to county and/or municipal regulations, and wetland permitting through the U.S. Army Corps of Engineers (see www.usace.army.mil for regional contact information).

The plans reviewed in this determination are submitted as part of the Developmental Approval process. Prior to the operation of this structure, please provide notice of completion of construction and as-constructed plans in PDF form including as constructed Stormwater Detention and Infiltration Data Sheet. Additionally, prior to the operation of this structure, notice must be provided pursuant to 37-92-602(8)(d) to the substitute water supply plan notification list maintained by the state engineer pursuant to section 37-92-308 (6) for the water division in which the facility is located.

If you have any questions regarding this approval, please contact Water Commissioner, Jacob Olson, at (719) 227-5291 or via email to jacob.olson@state.co.us, or Dam Safety Engineer, Brian McCormick, at (719)-227-5294, or via email to brian.mccormick@state.co.us,

Sincerely,



Bill W. Tyner, P.E.
Division Engineer, Division 2

Enc:

Signed Notice of Intent to Construct a Non-Jurisdictional Water Impoundment Structure
Specifications for Construction of Non-Jurisdictional Dams
Completion of Construction Form

Ec:

Brian McCormick, P.E., Dam Safety Engineer
Jacob Olson, District 10 Water Commissioner
Laserfiche File

August 16, 2022



Colorado Division of Water Resources

310 E. Abriendo Ave., Suite B
Pueblo, CO 81004

RE: Non-Jurisdictional Water Impoundment Structure Notice
Homestead North at Sterling Ranch Filing 2, El Paso County

JR Engineering is performing civil engineering services for the proposed Homestead North at Sterling Ranch Filing 2 development northeast of the intersection of Vollmer Road and future Briargate Parkway in El Paso County. The development is comprised of urban residential lots, tracts roadways and utilities.

As part of this development, one (1) full spectrum detention pond is proposed. The pond is identified as Pond A. The pond will have an embankment on the downhill side but, in my opinion, the pond is non-jurisdictional and provide no public exposure in the event of embankment failure as they are adjacent to the Sand Creek drainageway. Groundwater is not anticipated to be encountered based on the depth of excavation and soils report completed by Entech Engineering. In the event groundwater is encountered, your office will be notified.

I have attached the NOI and the grading/pond plans for the pond. I have also attached an overall vicinity map to help define the location of the proposed pond.

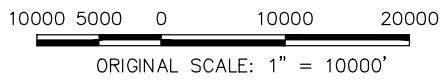
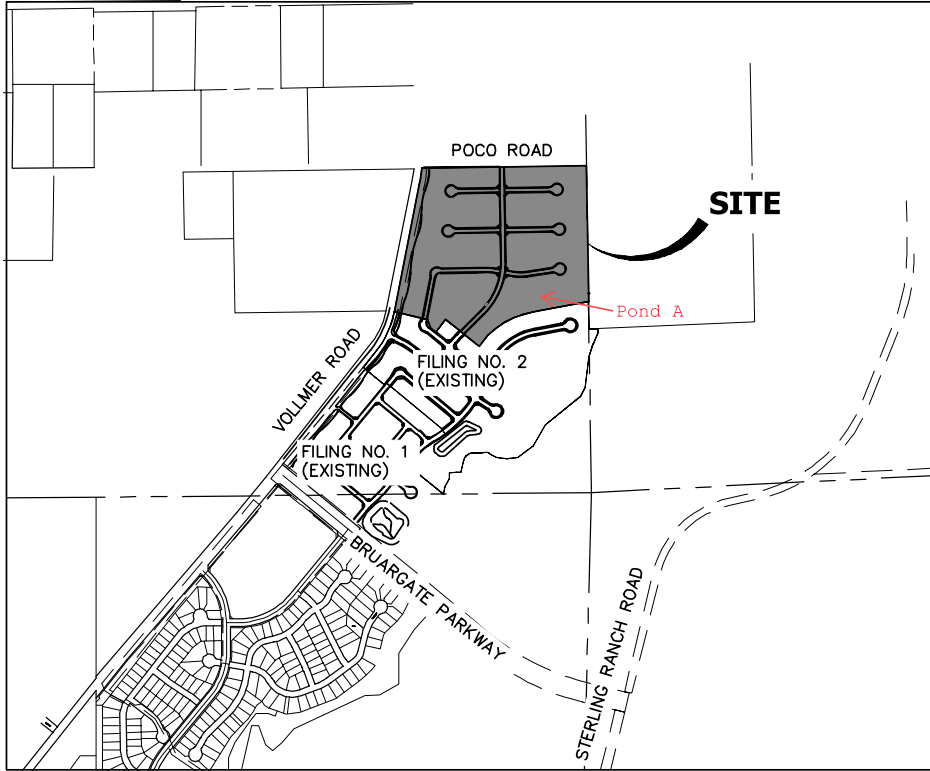
If additional information or clarification is needed to support this submittal, please feel free to contact me.

Respectfully submitted,

JR ENGINEERING, LLC

A handwritten signature in blue ink that reads "Mike Bramlett".

Mike Bramlett, PE
Client Manager
Ph: (303) 267-6240
Cell: (719) 659-7679
Email: mbramlett@jrengineering.com



HOMESTEAD NORTH AT
 STERLING RANCH FIL. NO. 3
 VICINITY MAP
 JOB NO. 2518812
 6/27/22
 SHEET 1 OF 1



Centennial 303-740-9993 • Colorado Springs 719-583-2993
 Fort Collins 970-491-9888 • www.jrengineering.com



NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE¹

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

OWNER INFORMATION

Name: SR LAND, LLC Telephone/E-Mail: (719) 491-3024 / JMORLEY3870@AOL.COM
Address: 20 BOULDER CRESCENT, SUITE 200 COLORADO SPRINGS CO 80903
Responsible Person: JIM MORLEY Telephone/E-Mail: (719) 491-3024 / JMORLEY3870@AOL.COM
Address: 20 BOULDER CRESCENT, SUITE 200 COLORADO SPRINGS CO 80903
Contractor: TO BE DETERMINED. Telephone/E-Mail: (TO BE DETERMINED.)

STRUCTURE INFORMATION

Name of Dam: HOMESTEAD NORTH F3 - POND A Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, and GPS Point taken at crest of dam above streamline/outlet)

- Section: 28, Township: 12S, Range: 65W, 6th P.M.
- Northing 4313958.13 meters, Easting 529075.13 meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height²: 5.9 ft., Length: 350 ft., Crest Width: 10 ft., Slopes: U/S: 4 (H:1V), D/S 4 (H:1V)

Reservoir:

- Surface Area¹: 0.8 acres, Capacity¹: 2.4 acre-feet, Drainage Area*: 30.0 acres
*(If drainage area is unknown leave blank and a spillway size will be assigned):

Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 30 ft., Side Slopes: 4 H:1V, Freeboard³: 2.0' ft

Outlet Conduit Type: RCP, Size: 30" inches, Location: SAND CREEK

Stream Name or Water Source⁴: SAND CREEK Proposed Water Use: FULL SPECTRUM DETENTION POND

Water Court Case or WDID : (Water District Identification Number)

Signature of Owner (handwritten signature)

Date: 8/16/22

Date

Office Use Only

DIVISION ENGINEER'S REQUIREMENTS:

Dam I.D. 100594

Signature of Division Engineer (handwritten signature)

Date: 11/15/2022

Signature of Division Engineer

Date

1 A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less and a surface area of 20 acres or less and a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.

2 "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.

3 "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.

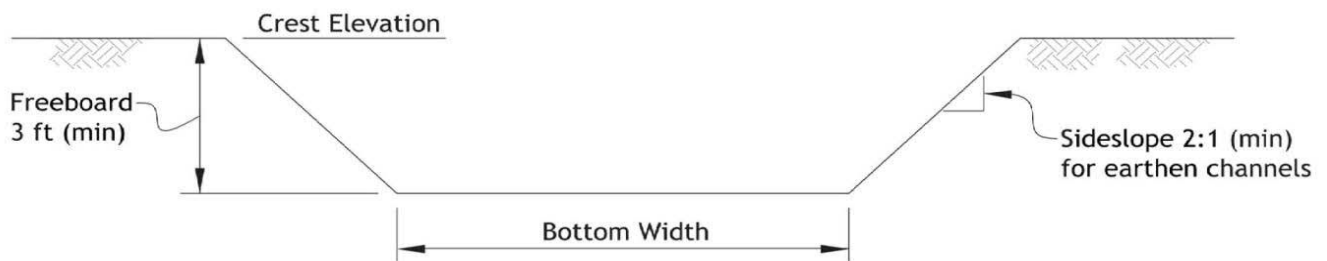
4 If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at www.water.state.co.us)



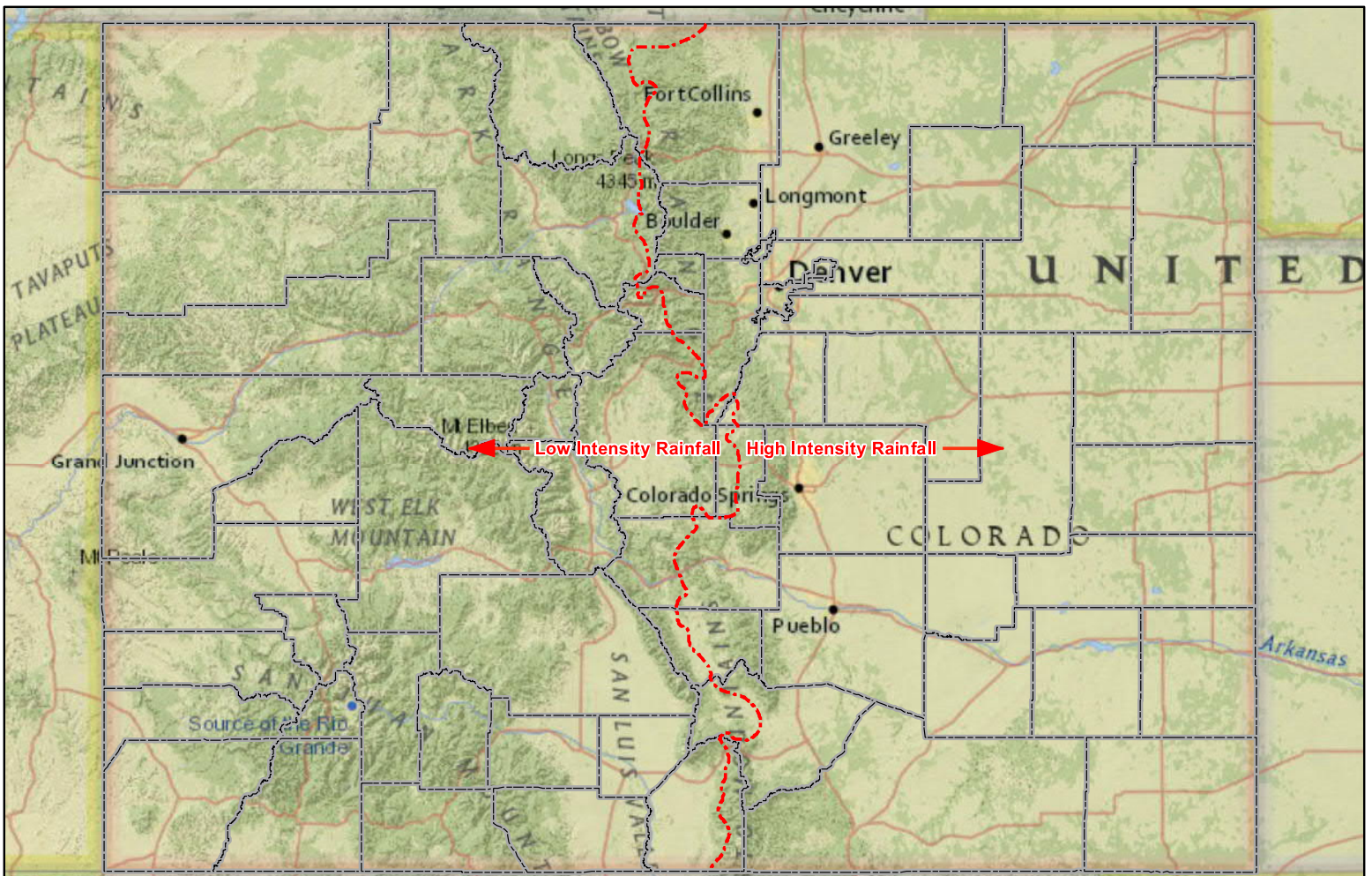
Table 1 DAM SAFETY BRANCH Spillway Sizing Guidelines for Non-Jurisdictional Dams

Drainage Area (Acres)	Minimum Recommended Bottom Width ¹ (Feet) Low Intensity Rainfall Zone	Minimum Recommended Bottom Width ¹ (Feet) High Intensity Rainfall Zone
175	8	8
225	8	10
275	8	12
325	8	15
375	10	17
425	11	19
475	12	21
525	13	24
575	15	26
625	16	28
675	17	30
725	19	33
775	20	35
825	21	37
875	22	39
925	24	42
975	25	44
1025	26	46
1075	28	48
1125	29	51
1175	30	53
1225	31	55
1275	33	57
1325	34	59
1375	35	62
1425	37	64
1475	38	66

¹Minimum recommended bottom width for drainage areas less than 175 acres is 8 feet



Spillway Section



0 27.5 55 Miles 1" = 55 Miles

Map Key - - - - - Rainfall Divide Line

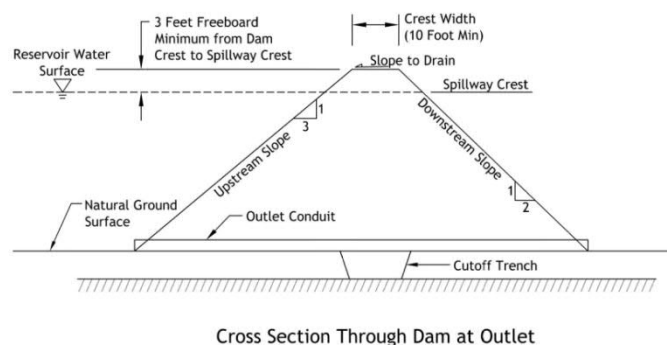
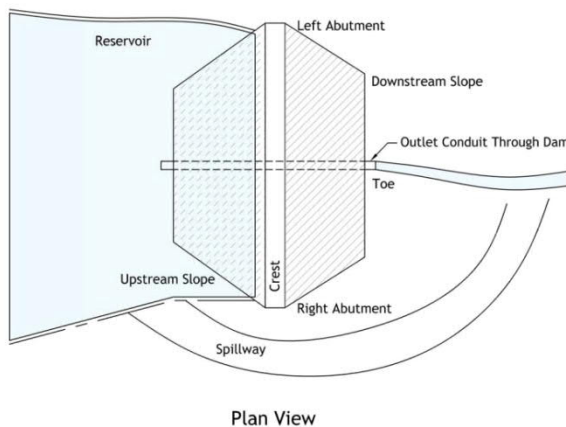


COLORADO
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Department of Natural Resources

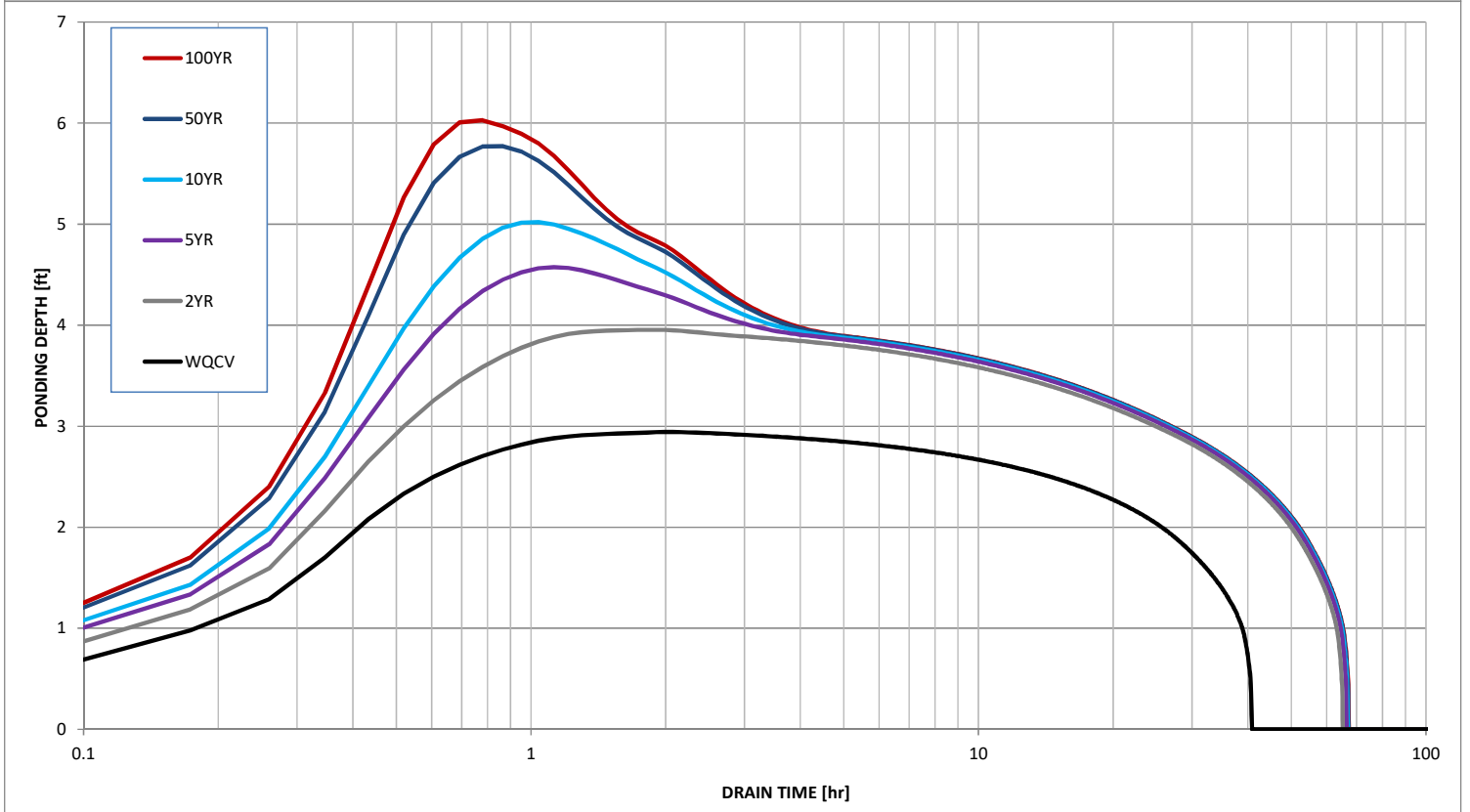
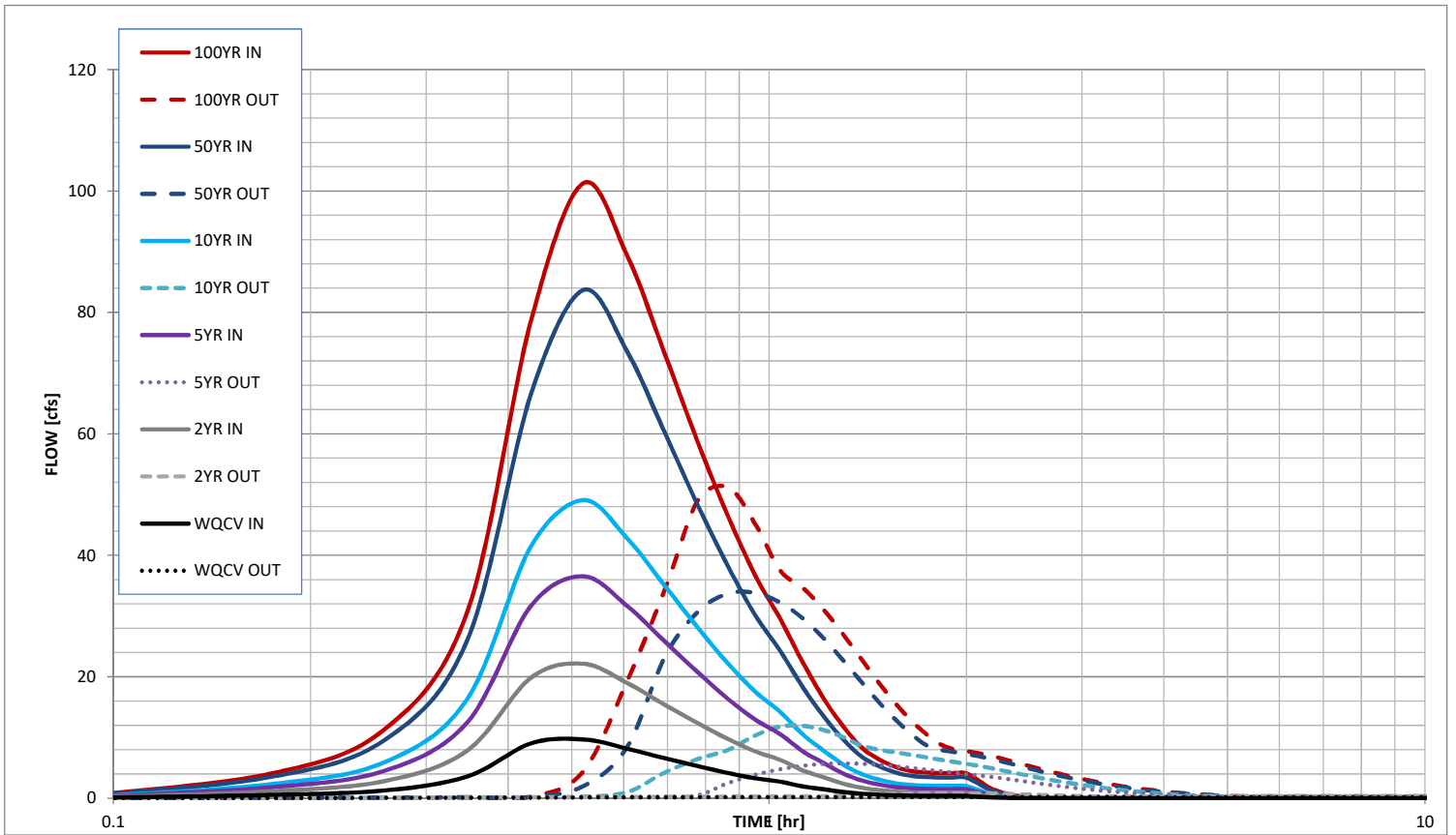
Rainfall Intensity Zones for Non-Jurisdictional Dam Spillway Sizing

DAM SAFETY BRANCH Specifications for Construction of Non-Jurisdictional Dams

- Site Selection:
 - Foundation soils should be firm to provide adequate support for the embankment and should have low permeability to allow for water retention. Site selection should consider potential downstream property damage in the event of a dam failure. Construction of dams in boggy areas, areas with non-uniform fractured rock, or sands/gravels is not recommended and an engineer should be hired to evaluate the site conditions. Any part of the reservoir basin excavated below grade cannot expose groundwater.
- Embankment Design:
 - Backfill material to be used for construction of the cutoff trench and embankment should be a suitable clay material and contain no material larger than 6 inches in diameter.
 - The upstream slope should be constructed with a slope no steeper than 3:1, and the downstream slope should be no steeper than 2:1 (see cross section below). The dam crest should have a minimum width of 10 feet and the surface should be graded with positive drainage toward the reservoir basin.
 - It is recommended that rock rip rap or other suitable material be placed on the upstream slope of the embankment to protect it from wave action. A suitable gravel or geosynthetic material should be placed under the rip rap to prevent fine material from washing out from behind the larger rock.
 - The embankment should be fenced to restrict livestock from accessing the dam since they damage the protective vegetation and increase erosion.
- Embankment Construction
 - The topsoil and all organic material should be removed from the foundation of the proposed dam site. Organic soil should only be reused for placement on the completed embankment to promote the re-growth of vegetation.
 - A cutoff trench should be excavated under the full length of the centerline of the dam with sloping sides (1:1 min.), a minimum bottom width of 3 feet and a depth of 3 feet.
 - The foundation of the dam should be scarified/ripped to a depth of 6-inches to provide proper contact between the native foundation and embankment. This surface should then be moisture treated before placement of fill.
 - Fill material should be placed in layers not exceeding 12 inches in thickness prior to compaction. Suitable backfill material should have enough clay and moisture content to roll a small ball by hand. If this cannot be done, the soil is likely too dry or does not have adequate clay content.
 - Each lift should be thoroughly compacted using a sheeps foot compactor. Care should be taken not to allow the top layers of the soil to dry out between placement of lifts.
 - Fill should be placed in uniform lifts that cover the entire embankment length and width.
- Outlet
 - Unless a waiver is granted in writing by the Division Engineer, all non-jurisdictional dams require an outlet conduit positioned at the natural low point of the reservoir basin. A minimum diameter of 12 inches is recommended and should be controlled at the upstream end by a valve and trash rack.
- Emergency Spillway
 - The spillway should have sufficient width to provide capacity to route the runoff from the drainage basin above the dam during rainfall/runoff events.
 - The emergency spillway should be located on natural ground far enough away to prevent erosion of the dam embankment. A spillway over the dam embankment is not acceptable.
 - A minimum of 3 feet of freeboard is required from the bottom of the emergency spillway to the top of the dam.
 - To determine the minimum spillway width, see the attached table for your area and drainage basin size.
- Example Plan View and Cross Section



Stormwater Detention and Infiltration Design Data Sheet

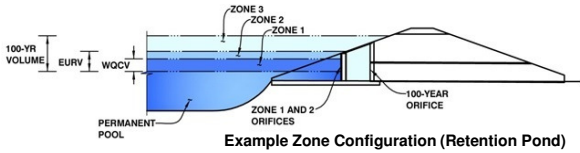


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)

Project: Homestead North at Sterling Ranch Filing No. 3

Basin ID: Pond A



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.01	0.452	Orifice Plate
Zone 2 (EURV)	4.46	0.823	Orifice Plate
Zone 3 (100-year)	6.03	1.141	Weir&Pipe (Restrict)
Total (all zones)		2.416	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
 Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = inches
 Orifice Plate: Orifice Area per Row = sq. inches

Calculated Parameters for Plate
 WQ Orifice Area per Row = ft²
 Elliptical Half-Width = feet
 Elliptical Slot Centroid = feet
 Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.50	3.00	3.25				
Orifice Area (sq. inches)	1.86	1.86	1.86	2.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft ²
Vertical Orifice Centroid =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	<input type="text" value="4.50"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	<input type="text" value="5.00"/>	<input type="text" value="N/A"/>	feet
Overflow Weir Gate Slope =	<input type="text" value="0.00"/>	<input type="text" value="N/A"/>	H:V
Horiz. Length of Weir Sides =	<input type="text" value="5.00"/>	<input type="text" value="N/A"/>	feet
Overflow Gate Type =	<input type="text" value="Type C Gate"/>	<input type="text" value="N/A"/>	
Debris Clogging % =	<input type="text" value="50%"/>	<input type="text" value="N/A"/>	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Gate Upper Edge, H _g =	<input type="text" value="4.50"/>	<input type="text" value="N/A"/>	feet
Overflow Weir Slope Length =	<input type="text" value="5.00"/>	<input type="text" value="N/A"/>	feet
Grate Open Area / 100-yr Orifice Area =	<input type="text" value="5.01"/>	<input type="text" value="N/A"/>	
Overflow Grate Open Area w/o Debris =	<input type="text" value="17.40"/>	<input type="text" value="N/A"/>	ft ²
Overflow Grate Open Area w/ Debris =	<input type="text" value="8.70"/>	<input type="text" value="N/A"/>	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	<input type="text" value="0.00"/>	<input type="text" value="N/A"/>	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	<input type="text" value="30.00"/>	<input type="text" value="N/A"/>	inches
Restrictor Plate Height Above Pipe Invert =	<input type="text" value="20.00"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	<input type="text" value="3.48"/>	<input type="text" value="N/A"/>	ft ²
Outlet Orifice Centroid =	<input type="text" value="0.94"/>	<input type="text" value="N/A"/>	feet
Half-Central Angle of Restrictor Plate on Pipe =	<input type="text" value="1.91"/>	<input type="text" value="N/A"/>	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
 Spillway Crest Length = feet
 Spillway End Slopes = H:V
 Freeboard above Max Water Surface = feet

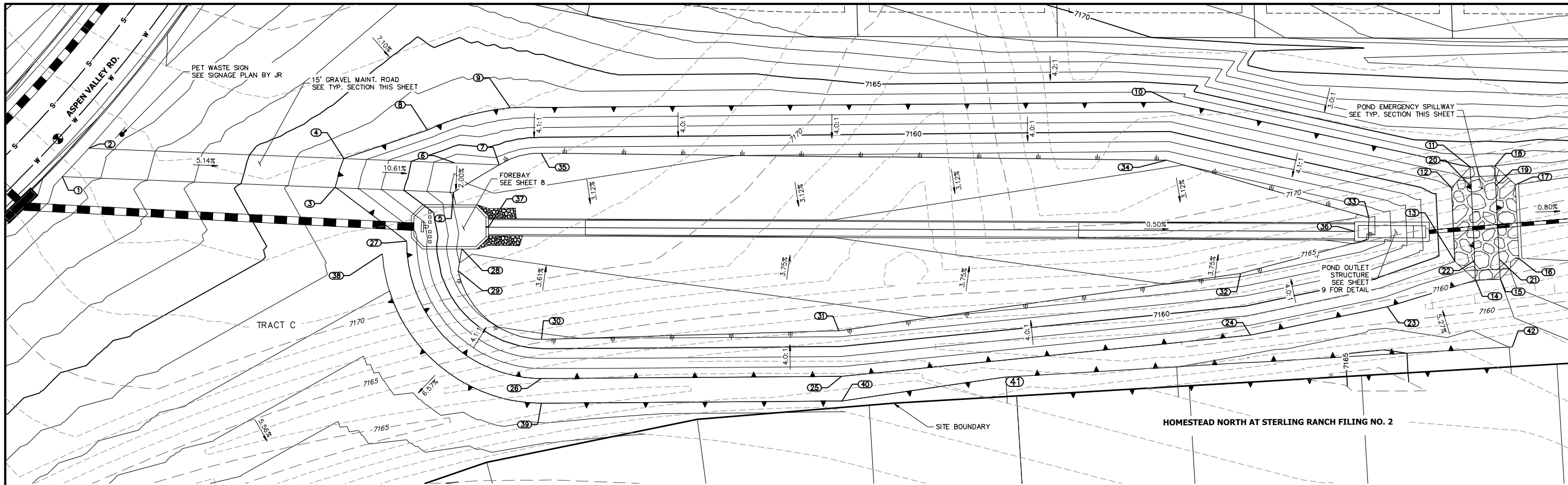
Calculated Parameters for Spillway

Spillway Design Flow Depth = feet
 Stage at Top of Freeboard = feet
 Basin Area at Top of Freeboard = acres
 Basin Volume at Top of Freeboard = acre-ft

Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
CUHP Runoff Volume (acre-ft) =	0.452	1.275	1.247	1.860	2.411	3.199	3.813	4.614	8.451
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.247	1.860	2.411	3.199	3.813	4.614	8.451
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	3.2	9.0	13.6	24.4	30.6	39.2	76.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.11	0.30	0.46	0.82	1.02	1.31	2.56
Peak Inflow Q (cfs) =	N/A	N/A	17.8	27.5	34.8	47.5	56.5	67.8	122.0
Peak Outflow Q (cfs) =	0.2	0.4	0.4	5.4	11.5	23.1	31.2	37.0	96.3
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.6	0.8	0.9	1.0	0.9	1.3
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	0.3	0.6	1.3	1.8	2.1	2.3
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	68	67	70	68	66	64	62	53
Time to Drain 99% of Inflow Volume (hours) =	40	72	72	76	75	74	73	71	67
Maximum Ponding Depth (ft) =	3.01	4.46	4.31	4.79	4.99	5.29	5.47	5.82	6.60
Area at Maximum Ponding Depth (acres) =	0.46	0.65	0.64	0.68	0.70	0.73	0.74	0.78	0.85
Maximum Volume Stored (acre-ft) =	0.455	1.281	1.177	1.495	1.640	1.848	1.980	2.246	2.879



UNTIL SUCH TIME AS THE WORK IS APPROVED BY THE APPROPRIATE AGENCIES, JR ENGINEERING APPROVES THEIR USE ONLY FOR THE PURPOSES DESIGNATED BY WRITTEN AUTHORIZATION.

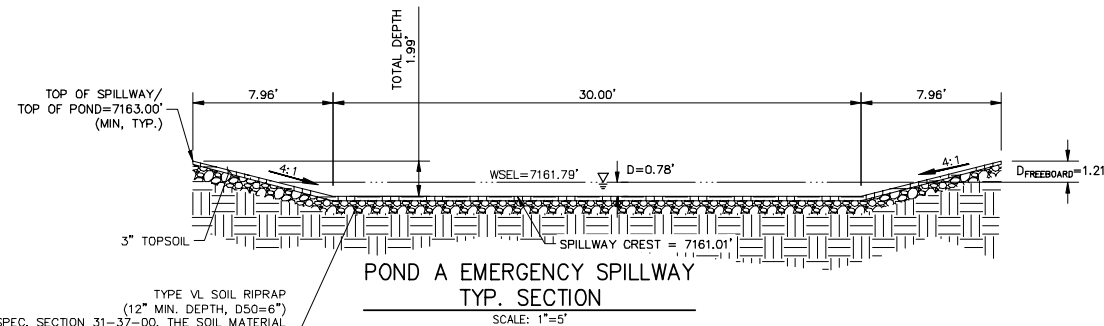
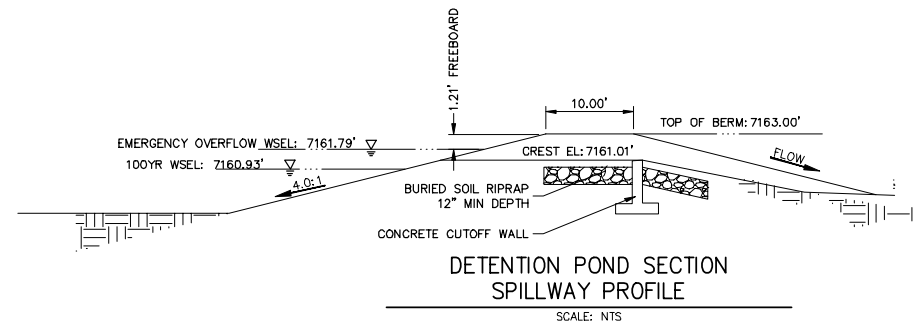
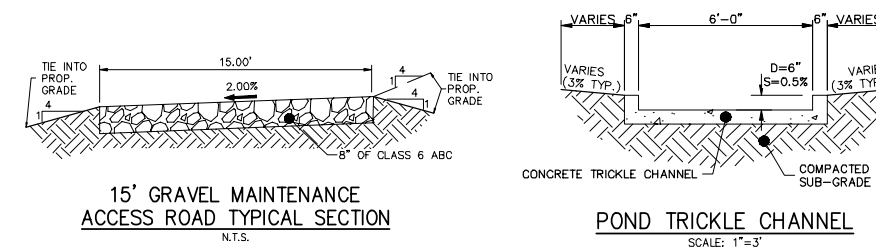
PREPARED FOR
SR LAND, LLC
 20 BOULDER CREST SITE 200
 COLORADO SPRINGS, CO 80903
 ATTN: JAMES F. MORLEY
 JFMORLEY73870@AOL.COM

JR ENGINEERING
 A Western Company
 Centennial 303-740-6888 • Colorado Springs 719-589-2589
 Fort Collins 970-491-3888 • www.jrengineering.com

POINT TABULATION			
ID NO.	DESCRIPTION	NORTHING/EASTING	ELEVATION
1	MAINT. ROAD	N: 416925.58 E: 238346.46	7168.46
2	MAINT. ROAD	N: 416938.44 E: 238355.71	7168.53
3	MAINT. ROAD/ TOP	N: 416935.24 E: 238456.61	7163.00
4	MAINT. ROAD/ TOP	N: 416947.47 E: 238458.76	7163.00
5	MAINT. ROAD/TOE	N: 416939.44 E: 238504.56	7157.84
6	MAINT. ROAD	N: 416951.39 E: 238503.52	7158.08
7	MAINT. ROAD/ TOE	N: 416952.98 E: 238521.59	7158.34
8	TOP	N: 416963.20 E: 238490.88	7163.00
9	TOP	N: 416976.40 E: 238523.13	7163.00
10	TOP	N: 417012.92 E: 238789.24	7163.00
11	SPILLWAY TOP	N: 417002.06 E: 238912.15	7163.00
12	SPILLWAY CREST	N: 416992.84 E: 238905.69	7161.01
13	SPILLWAY CREST	N: 416963.30 E: 238910.89	7161.01
14	SPILLWAY TOP	N: 416956.84 E: 238920.11	7163.00
15	SPILLWAY TOP	N: 416958.57 E: 238929.96	7163.00

POINT TABULATION			
ID NO.	DESCRIPTION	NORTHING/EASTING	ELEVATION
16	SPILLWAY CREST	N: 416967.79 E: 238936.42	7161.01
17	SPILLWAY CREST	N: 416997.34 E: 238931.22	7161.01
18	SPILLWAY TOP	N: 417003.80 E: 238922.00	7163.00
19	SPILLWAY CREST	N: 416995.96 E: 238923.38	7161.01
20	SPILLWAY CREST	N: 416994.22 E: 238913.53	7161.01
21	SPILLWAY CREST	N: 416966.41 E: 238928.58	7161.01
22	SPILLWAY CREST	N: 416964.68 E: 238918.73	7161.01
23	TOP	N: 416941.58 E: 238885.99	7163.00
24	TOP	N: 416923.08 E: 238832.47	7163.00
25	TOP	N: 416885.45 E: 238670.18	7163.00
26	TOP	N: 416869.09 E: 238549.63	7163.00
27	TOP	N: 416917.40 E: 238488.30	7163.00
28	TOE/ FOREBAY	N: 416917.09 E: 238510.82	7157.49
29	TOE	N: 416908.16 E: 238510.50	7158.27
30	TOE	N: 416884.87 E: 238547.58	7159.09

POINT TABULATION			
ID NO.	DESCRIPTION	NORTHING/EASTING	ELEVATION
31	TOE	N: 416903.70 E: 238666.80	7158.39
32	TOE	N: 416947.47 E: 238825.40	7156.68
33	TOE	N: 416976.57 E: 238873.97	7156.06
34	TOE	N: 416989.21 E: 238789.73	7157.09
35	TOE	N: 416959.79 E: 238537.88	7158.41
36	TRICKLE CHANNEL	N: 416970.22 E: 238868.98	7155.44
37	TRICKLE CHANNEL	N: 416927.35 E: 238520.44	7157.20
38	BERM	N: 416911.05 E: 238479.19	7163.00
39	BERM	N: 416859.17 E: 238550.91	7163.00
40	BERM	N: 416875.54 E: 238671.52	7163.00
41	BERM	N: 416893.67 E: 238736.21	7163.00
42	BERM	N: 416931.84 E: 238938.32	7163.97



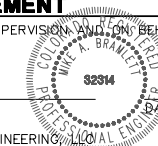
TYPE VI. SOIL RIPRAP
 (12" MIN. DEPTH, D50=6")
 PER MHD SPEC. SECTION 31-37-00, THE SOIL MATERIAL
 SHALL BE NATIVE OR TOPSOIL MIXED WITH 65% RIPRAP
 AND 35% SOIL BY VOLUME. SOIL RIPRAP SHALL CONSIST OF
 UNIFORM MIXTURE OF SOIL AND RIPRAP WITHOUT VOIDS.



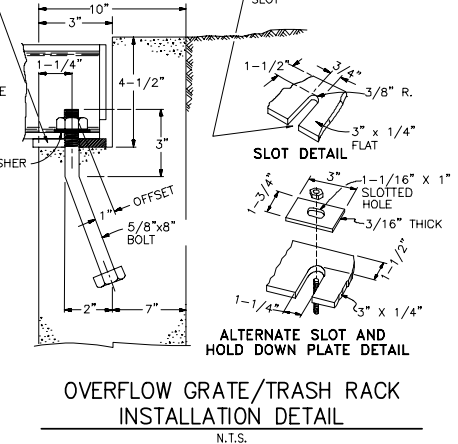
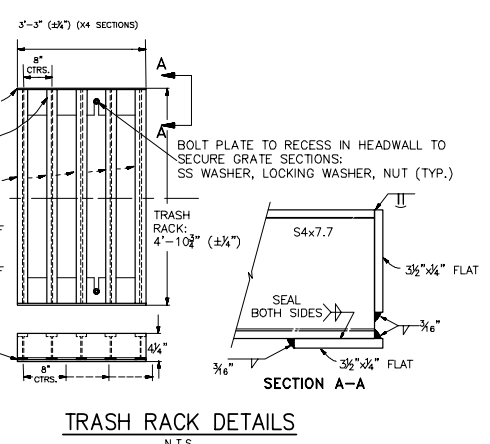
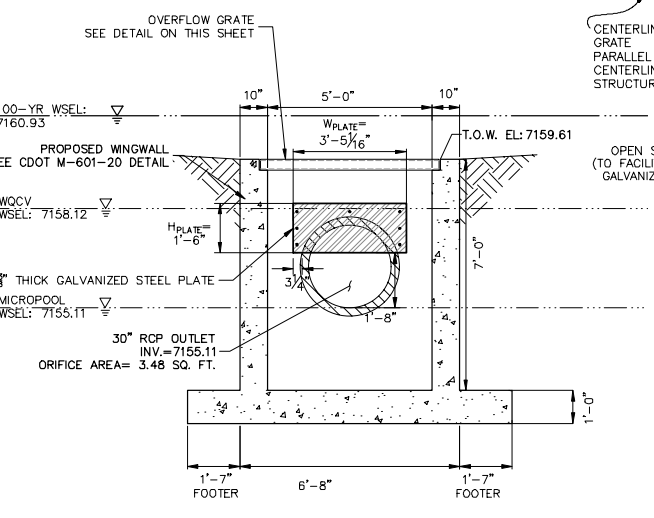
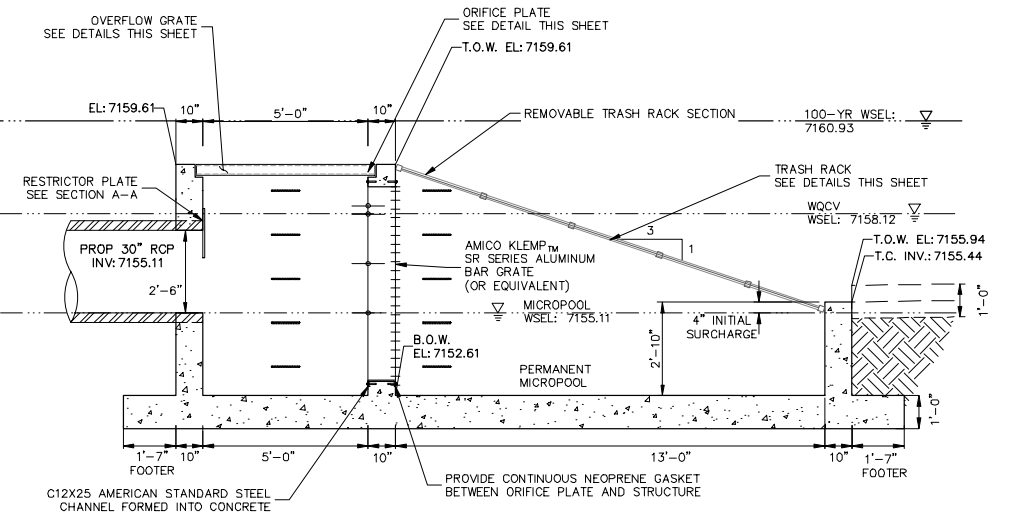
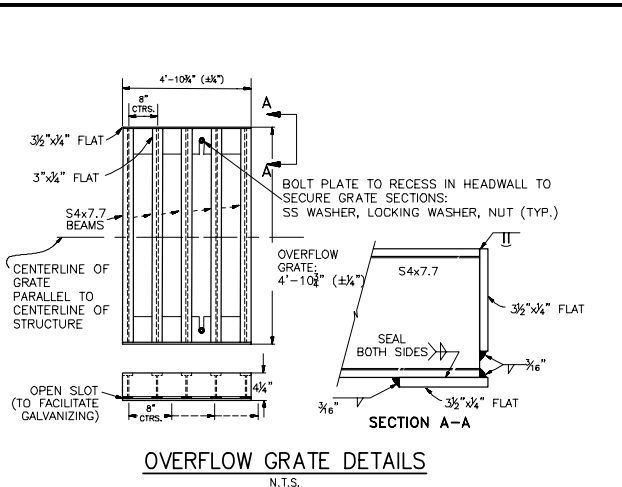
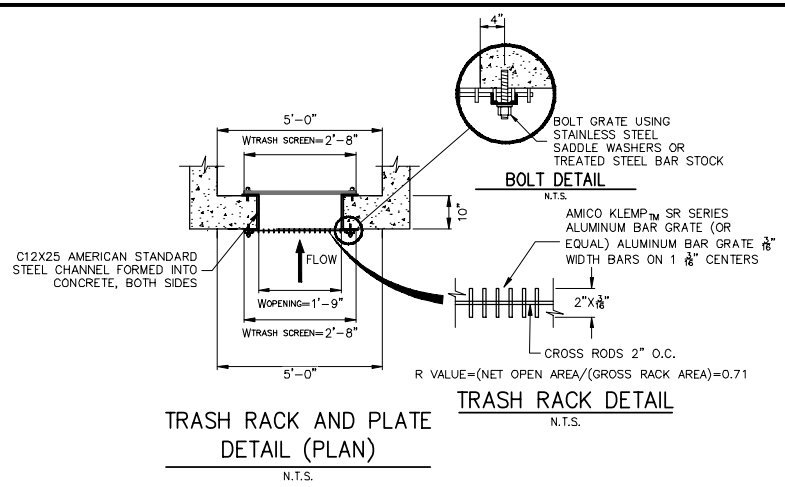
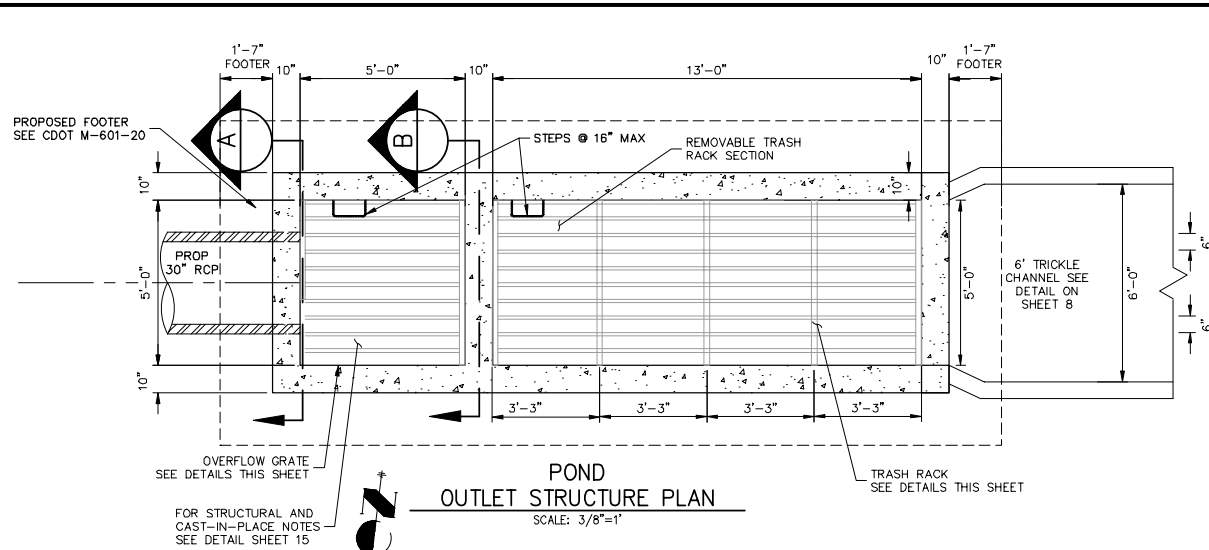
ENGINEER'S STATEMENT

PREPARED UNDER MY DIRECT SUPERVISION AND ON BEHALF OF JR ENGINEERING

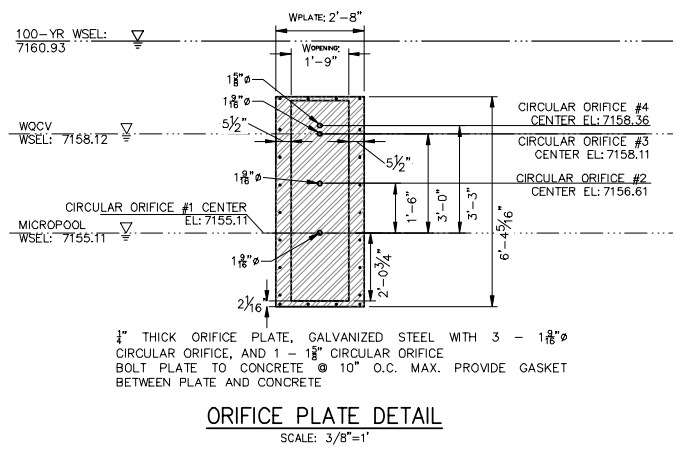
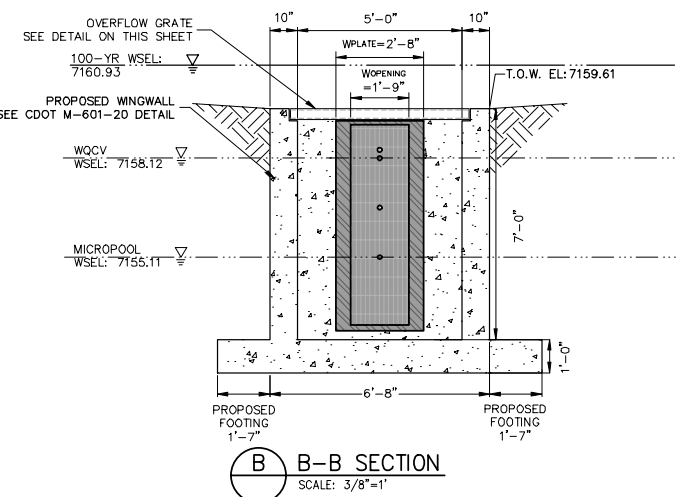
MIKE A. BRAMLETT, P.E.
 COLORADO P.E. 32314
 FOR AND ON BEHALF OF JR ENGINEERING



BY	DATE	No.	REVISION	H-SCALE	V-SCALE	DATE	DESIGNED BY	DRAWN BY	CHECKED BY
				1"=20'	N/A	08/05/22	APL	APL	
HOMESTEAD NORTH AT STERLING RANCH FILING NO. 3									
POND PLANS									
SHEET 6 OF 13									
JOB NO. 2518812									



- POND C OUTLET STRUCTURE NOTES:**
- ORIFICE PLATE:**
1. PROVIDE CONTINUOUS NEOPRENE GASKET MATERIAL BETWEEN THE ORIFICE PLATE AND CONCRETE AND BETWEEN THE RESTRICTOR PLATE AND CONCRETE.
 2. BOLT PLATE TO CONCRETE 12" MAX. ON CENTER.
- TRASH RACKS:**
3. TRASH RACKS SHALL BE 1 1/2" SCH.40 STEEL PIPE, GALVANIZED, @ 6" CENTERS. SUPPORT BARS SHALL BE 1/2" x 2" STEEL RECTANGULAR BARS, GALVANIZED, @ 36". ALL TRASH RACKS SHALL BE MOUNTED USING STAINLESS STEEL HARDWARE.
 4. REMOVABLE TRASH RACK SECTIONS SHALL BE MOUNTED USING STAINLESS STEEL HARDWARE AND PROVIDED WITH HINGED & LOCKABLE OR BOLTABLE ACCESS PANELS AS SHOWN ON THE PLANS.
 5. STEEL TRASH RACKS SHALL BE HOT DIP GALVANIZED AND MAY BE HOT POWDER COATED AFTER GALVANIZING.
 6. STRUCTURAL STEEL FOR GRATES, ORIFICE PLATES, AND BARS SHALL BE GALVANIZED AND SHALL BE IN ACCORDANCE WITH CDOT STANDARD SPECIFICATIONS, SUBSECTION 712.06.
 7. ALL HARDWARE, BOLTS, AND FASTENERS SHALL BE STAINLESS STEEL.
 8. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ALL PLATES AND GRATING FOR ENGINEER'S APPROVAL PRIOR TO CONSTRUCTION.



ENGINEER'S STATEMENT
 PREPARED UNDER MY DIRECT SUPERVISION AND ON BEHALF OF JR ENGINEERING
 MIKE A. BRAMLETT, P.E.
 COLORADO P.E. 32314
 FOR AND ON BEHALF OF JR ENGINEERING

UNTIL SUCH TIME AS THE WORK IS APPROVED BY THE APPROPRIATE ENGINEERING AGENCIES, JR ENGINEERING APPROVES THEIR USE ONLY FOR THE PURPOSES DESIGNATED BY WRITTEN AUTHORIZATION.

PREPARED FOR
SR LAND, LLC
 20 BOULDER CREEK STE 200
 COLORADO SPRINGS, CO 80903
 ATTN: JAMES F. MORLEY
 JFMORLEY3570@AOL.COM

JR ENGINEERING
 A Western Company
 Centennial 303-740-0888 • Colorado Springs 719-589-2888
 Fort Collins 970-491-3888 • www.jrengineering.com

NO.	REVISION	DATE	DESIGNED BY	DRAWN BY	CHECKED BY
1		08/05/22	APL	APL	

HOMESTEAD NORTH AT STERLING RANCH FILING NO. POND PLANS

SHEET 9 OF 13
 JOB NO. 2518812