EPC STORMWATER REVIEW COMMENTS
IN ORANGE BOXES WITH BLACK TEXT

FINAL DRAINAGE REPORT
FOR
SADDLEHORN RANCH – FILING 5 EARLY GRADING

Prepared For:
ROI Property Group, LLC
2495 Rigdon Street
Napa, CA 94558
(707) 365-6891

September 1, 2022
Project No. 25142.07

Prepared By:
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5475 Tech Center Drive
Colorado Springs, CO 80919
719-593-2593

El Paso County PCD File No.:
EGPXXX
EGP226

Final Drainage Report
Filing 5 - Saddlehorn Ranch Early Grading

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

25043

Bryan Law, Colorado P.E. # 25043
For and On Behalf of JR Engineering, LLC

9/8/22
Date

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: ROI Property Group, LLC

By: FOR ROI (BILL GUMAN

Title: REPRESENTATIVE

Address: 2495 Rigdon Street
Napa, CA 94558

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

Joshua Palmer, P.E.
County Engineer/ ECM Administrator

Date

Conditions:

CONTENTS

PURPOSE

This document is the Final Drainage report for Filing 5 of Saddlehorn Ranch Early Grading. The purpose of this report is to:

1. Identify on-site and off-site drainage patterns.
2. Recommend storm water facilities to collect and convey storm runoff from the proposed development during early grading operations to appropriate discharge and/or detention locations.
3. Recommend water quality and detention facilities to control discharge release rates to below historic.
4. Demonstrate compliance with surrounding major drainage basin planning studies, master development drainage plans and flood insurance studies.

GENERAL LOCATION AND DESCRIPTION

Location

The proposed Saddlehorn Ranch Filing 5, known as "Filing 5" from herein, is a parcel of land located in Section 3 and 10, Township 13 South, Range 64 West of the 6th Principal Meridian in El Paso County, Colorado. Saddlehorn Ranch is an 824 acre, rural, single family-development. Filing 5 is 126.73 acres and is comprised of 41 lots of the overall Saddlehorn Ranch development. Saddlehorn Ranch is bound by Judge Orr Road to the North and Curtis Road to the West. To the East, Saddlehorn Ranch is bound by undeveloped land owned by Brent Houser Enterprises, LLC. To the south, Saddlehorn Ranch is bound by undeveloped properties owned by Carolyn Gudzunas and Faye Reynolds. Filing 5 is bound by future Filing 4 to the north, Drainageway MS-06 to the west, and unplatted vacant land to the east and to the south. A vicinity map is presented in Appendix A.

Currently, there are two major Drainageway that will receive flows from Filing 5: Gieck Ranch (WF-R7A) and Haegler Ranch Main Stem 6 (MS-06). These Drainageways were analyzed, both hydrologically and hydraulically, in the following reports:

- Haegler Ranch Basin Drainage Basin Planning Study (DBPS), May 2009.
- Santa Fe Springs – Haegler Ranch Drainage Basin Letter of Map Revision, June 2004.
- Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch, May 2020.
- Geick Ranch Drainage Basin Planning Study (DBPS), October 2007

The impact of these Drainageways and planning studies on the proposed development will be discussed later in the report.

Description of Property

Filing 5 is currently unoccupied and undeveloped. The existing ground cover is sparse vegetation and open space, typical of a Colorado rolling range land condition. In general, Filing 5 slopes from south to southeast and the existing drainageways follow this topography.

Per a NRCS web soil survey of the area, Filing 5 is made up of Group A soils. Group A soils have a high infiltration rate when thoroughly wet. A NRCS soil survey map has been presented in Appendix A.

Floodplain Statement

Based on the FEMA FIRM Map number 08041C0558G, dated December 7, 2018, Filing 5 lies within Zone AE and Zone X. Zone AE is defined as area subject to inundation by the 1-percent-annual-chance flood event. Zone X is defined as area outside the Special Flood Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. All proposed residential development within Filing 5 will occur in Zone X. The FIRM Map has been presented in Appendix A.

DRAINAGE BASINS AND SUB-BASINS

Address CLOMR/LOMR requirements

Existing Major Basin Descriptions

Filing 5 lies within Haegler Ranch Drainage Basin based on the "*Haegler Ranch Drainage Basin Planning Study*" prepared by URS Corporation in May 2009.

The Haegler Ranch Drainage Basin covers approximately 16.6 square miles in unincorporated El Paso County, CO. The Haegler Ranch Drainage Basin is tributary to Black Squirrel Creek. In its existing condition, the basin is comprised of rolling rangeland with poor vegetative cover associated with Colorado's semi-arid climate. The natural Drainageways within the basin are typically shallow and wide with poorly defined flow paths in most areas. Anticipated land use for the basin includes residential and commercial development. Residential developments will range from 0.125 – 5 acre lots with a mix of low, medium and high density developments.

As part of its drainage research, JR Engineering reviewed the following drainage studies, reports and LOMRs:

- Haegler Ranch Drainage Basin Planning Study prepared by URS Corporation in May 2009
- Santa Fe Springs – Haegler Ranch Drainage Basin Letter of Map Revision prepared by Tri-Core Engineering in June 2004.
- Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch, prepared by JR Engineering, May 2020.
- Gieck Ranch Drainage Basin Planning Study (DBPS), October 2007

The "*Haegler Ranch Drainage Basin Planning Study*" was used to establish a stormwater management plan for the existing and future stormwater infrastructure needs within the Haegler Ranch Drainage Basin. Based on provided drainage maps and analysis, in the existing condition Haegler Ranch contributes a total

of 710 cfs onto the site. Of the 710 cfs, 590 cfs crosses Curtis Road in an existing 24" CMP onto the site. Major Drainageway MS-06 conveys the stormwater through the site and to its off-site confluence with Major Drainageway MS-05. The remaining 210 cfs crosses Curtis Road in an existing 36" CMP onto the site. Major Drainageway T-6 conveys the stormwater through the site and to its off-site confluence with Major Drainageway MS-05. Both Curtis Road culverts are undersized for existing and future flows and overtopping occurs locally near the culvert crossings.

Based on flood impacts, stream stability and cost effectiveness, this study recommended a sub-regional detention approach. This allows future development anywhere in the basin with the construction of an associated sub-regional pond. However, based on the *Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch,* Filing 5 will utilize one on-site full spectrum water quality and detention ponds instead. This full spectrum detention pond will limit developed discharge into Drainageway MS-06 to less than historic rates.

The Santa Fe Springs – Haegler Ranch Drainage Basin LOMR was executed on Haegler Ranch Tributary 2, 3, and 4. The LOMR revised the onsite effective flood zone from Zone A to Zone AE. See FIRM Map Panel 08041C0558G for limits of LOMR study and revised flood zones, presented in Appendix E.

The Gieck Ranch Drainage Basin covers approximately 22 square miles and begins approximately five miles northeast of the Town of Falcon and travels approximately 15 miles to the southeast. The Gieck Ranch Drainage Basin is tributary to Black Squirrel Creek which drains south to the Arkansas River near the city of Pueblo, Colorado. The majority of the area within the basin is undeveloped and is characterized as rolling range land typically associated with Colorado's semi-arid climates. Anticipated land use for the basin includes residential, industrial, agricultural and commercial development. Residential developments will range from 0.125 – 5 acre lots with a mix of low, medium and high density developments.

See Table 2 for comparison of Drainageway identification and the naming convention used within the context of this report. See Table 3 for a comparison of 100-year flows as calculated in the aforementioned DBPS and LOMR. An existing conditions drainage map is presented in Appendix E.

Table 1: Major Drainageway Naming Convention

Major Drainageway Naming Conventions			
Saddlehorn Ranch MDDP/PDR:	Per Haegler Ranch DBPS:	Per Geick Ranch DBPS:	Per Sante Fe Springs LOMR:
MS-06	Main Stem (MS-06)	N/A*	Haegler Ranch Tributary 3
WF-R7A	N/A*	West Fork (Middle)/WF-R7A	N/A*

Table 2: Major Drainageway – Ex. 100-Year Flow Comparison

Major Drainageways: 100-Year Flow Comparison				
Drainageway Name	Contributing Area (sq. mi.)	Q_{100} Per Haegler Ranch DBPS:	Q_{100} Per Geick Ranch DBPS:	Q_{100} Per Sante Fe Springs LOMR:
MS-06 @ Curtis Road	1.05	590 cfs	N/A*	505 cfs
WF-R7A @ Judge Orr Road	1.50	N/A*	1,017 cfs	N/A*

*N/A: Flow regime outside limits of study.

The *Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch* proposed the overall drainage facility design for Saddlehorn Ranch. Within the context of this report, onsite drainage basins the associated full spectrum water quality pond were established. As it pertains to Filing 5, two full spectrum water quality ponds are recommended. Roadside ditches and local street culverts will be utilized to capture and convey Filing 5's runoff to the water quality ponds. Both ponds A and B will discharge into Drainageway MS-06, while a portion of the proposed lots will release directly into Drainageway WF-R7A. All ponds are full spectrum and will release at less than historic rates.

Existing Sub-basin Drainage

On-site, existing sub-basin drainage patterns are generally from northwest to southeast by way of Drainageway MS-06 and Drainageway WF-R7A. On-site areas flow directly into these drainageways, which also bypass off-site flows through the site.

On-site, existing drainage basins were established based upon existing topography and the limits of the 100-year floodplain. These existing sub-basins were analyzed in the *Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch.* An existing drainage map has been provided in Appendix E.

Provide description of existing sub-basins and design points

Proposed Sub-basin Drainage

The proposed Filing 5 basin delineation is as follows;

flow across adjacent property and (?)

Basin A consists solely of Basin A1 for a total of 15.08 acres. In its existing condition, Basin A is rolling rangeland and runoff generally flows southeast towards the southern property line where it will ultimately outfall into Drainageway MS-06. In the proposed condition, Basin A will be rural 2.5 acre lots, paved roadway, and will include Pond A. Runoff from this basin will be collected in road side ditches and conveyed to Pond A located in the southeast corner of the Filing 5 development. Pond A will be a full spectrum water quality and detention pond, and will release at less than historic rates into Drainageway MS-06.

ultimately... State how it will function with EGP construction

Basin B consists of Sub-basins B1-B6 combining for a total of 60.42 acres. In its existing condition, Basin B is rolling rangeland and runoff generally flows southwest to Drainageway MS-06. In the

Address how much acreage and flow is diverted and its effect on the drainageway

proposed condition, Basin B will be rural 2.5 acre lots, paved roadway, and will include Pond B. Runoff from this basin will be collected in road side ditches and conveyed west to Pond B located in the south west corner of the Filing 5 development. Pond B is a full spectrum water quality and detention pond, and will release at less than historic rates into Drainageway MS-06.

Basin C consists of Sub-basins C1-C2 combining for a total of 5.45 acres. In its existing condition, Basin C is rolling rangeland and runoff generally flows south west towards Drainageway MS-06. In the proposed condition, Basin C will be rural 2.5 acre lots and paved roadway. Runoff from this basin will be collected in road side ditches and conveyed to the existing Pond C located in the southern portion of the Filing 4 development along Del Cambre Trail. Pond C is a full spectrum water quality and detention pond, and will release at less than historic rates into Drainageway MS-06. All calculations pertaining to Pond C can be found in the *Final Drainage Report for Saddlehorn Ranch – Filing 3*, prepared by JR Engineering, February 4, 2022.

Basin UD consists of Sub-basins UD1-UD4 combining for a total of 45.81 acres. In their existing condition, these basins are rolling rangeland. Runoff from Basins UD2, UD3, & UD 4 generally flows south and west to Drainageway MS-06. Basin UD1 flows east to Drainageway WF-R7A. In the proposed condition, these basins will be rural 2.5 acre lots with an Imperviousness = 6.2% and will be excluded from permanent stormwater quality management per Section I.7.1.B.5 of the ECM – Stormwater Quality Policy and Procedures.

less than 10%?

A summary table of proposed basin parameters and flow rates are presented in Appendix B.

Basin A runoff will overland flow into Pond A, or be captured by roadside swales and conveyed to the proposed Pond A. This full spectrum pond will release treated flows at less than historic rates to minimize adverse impacts downstream. Basin B will be captured in roadside swales and conveyed to the proposed Pond B. Pond A and Pond B will both discharge into Drainageway MS-06.

See Table 3 below for proposed Filing 5 pond parameters.

Table 3: Pond Summary

Tributary Sub-Basin	Pond Name	Tributary Acres	WQ Volume (ac-ft)	Total Detention Volume (ac-ft)	Provided Volume (ac-ft)	Maximum 100-Year Discharge (cfs)
A	POND A	15.08	0.085	0.199	0.279	7.5
B	POND B	60.42	0.382	1.144	1.295	21.6

Early Grading Drainage

During early grading operations, runoff will be captured in roadside ditches and conveyed into one of two sediment basins. Basin A runoff will be conveyed to Sediment Basin 2. Basin B

runoff will be conveyed to Sediment Basin 1. Sediment Basin 1 is designed to treat a tributary area of 60.42 acre, 20.98 acre of disturbed area, and 39.44 acre of undisturbed area. The required volume of Sediment Basin 1 in order to treat the 60.42 acre is 1.320 Ac-ft. Sediment Basin 1 exceeds this with a provided volume of 2.315 Ac-ft. Sediment Basin 1 was designed to drain its entire volume within 40 hrs via a temporary outlet structure. This temporary outlet structure was designed as a singular column with five 1.25" dia holes allowing for water to drain.

Sediment Basin 2 is designed to treat a tributary area of 15.08 acre, 1.55 acre of disturbed area, and 13.53 acre of undisturbed area. The required volume of Sediment Basin 2 in order to treat the 15.08 acre is 0.219 Ac-ft. Sediment Basin 2 has a provided volume of 0.279 Ac-ft. Sediment Basin 2 was designed to drain its entire volume within 40 hrs via a temporary outlet structure. This temporary outlet structure was designed as a singular column with five 1.25" dia holes allowing for water to drain.

Once the project progresses past this early grading phase, both Sediment Basin 1 and Sediment Basin 2 will be converted to a full spectrum water quality detention ponds. Sediment Basin 1 will be converted to Pond B, and Sediment Basin 2 will be converted to Pond A. Each Pond will be fitted with a concrete forebay along with appropriately sized riprap. The water will then drain through a concrete trickle channel to the proposed permanent outlet structure. Both temporary outlet structures will be replaced with permanent outlet structures, each with appropriately sized riprap spreaders. Both ponds will release treated flows at less than historic rates to minimize adverse impacts downstream. Both ponds will discharge into Major Drainageway MS-06. The final design for both Pond A and Pond B will be included in the Final drainage Report.

See Table 4 below for proposed Filing 5 Early Grading sediment basin parameters

Table 4: Sediment Basin Summary

Tributary Sub-Basin	Sediment Basin Name	Tributary Acres	Total Detention Volume (ac-ft)	Provided Volume (ac-ft)	Maximum Discharge (cfs)
A	Sediment Basin 2	15.08	0.219	0.279	0.0331
B	Sediment Basin 1	60.42	1.320	2.315	0.1997

DRAINAGE DESIGN CRITERIA

Development Criteria Reference

Storm drainage analysis and design criteria for the project were taken from the "*City of Colorado Spring/El Paso County Drainage Criteria Manual*" Volumes 1 and 2 (EPCDCM), dated October 12, 1994, the "*Urban Storm Drainage Criteria Manual"* Volumes 1 - 3 (USDCM) and Chapter 6 and Section

3.2.1 of Chapter 13 of the "Colorado Springs Drainage Criteria Manual (CCSDCM), dated May 2014, as adopted by El Paso County.

Hydrologic Criteria

All hydrologic data was obtained from the "El Paso Drainage Criteria Manual" Volumes 1 and 2, and the "Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual" Volumes 1, 2, and 3. Onsite drainage improvements were designed based on the 5 year (minor) storm event and the 100-year (major) storm event. Rational Method calculations were prepared, in accordance with Section 13.3.2.1. of the CCSDCM, for the sub-basins that directly impact the sizing of ditches and local street culverts. Rational method calculations are presented in Appendix B.

Urban Drainage and Flood Control District's UD-Detention, Version 4.06 workbook was used for pond sizing. Required detention volumes and allowable release rates were designed per USDCM and CCS/EPCDCM. Pond sizing spreadsheets are presented in Appendix D.

Hydraulic Criteria

Autodesk Inc.'s Hydraflow Express Extension (Volume 10.5) was used for roadside ditch design. Ditches were checked for velocity and capacity per the CCS/EPCDCM Section 12.3.2.2. In order to check both capacity and velocity, a cross section analysis was performed on the roadside swales using the basin's maximum runoff Q and the proposed uniform slope of the swale. Swale cross sections have been presented in Appendix C.

Autodesk Inc.'s Hydraflow Express Extension (Volume 10.5) was used for local road crossing culvert design. Culvert size was determined based on 100-year flows and hydraulic criteria from EPCDCM Chapter 9 –Culvert Design. All local road crossing culvert design reports are presented in Appendix C.

DRAINAGE FACILITY DESIGN

General Concept

The proposed stormwater conveyance system was designed to convey the developed Filing 5 runoff during interim early grading to one of two Sediment Basins via roadside ditches and local street culverts. These Sediment Basins were designed to release at less than historic rates to minimize adverse impacts downstream during early grading.

The proposed early grading improvements are over designed for the current state of the project site. The roadside swales along with the proposed culverts are designed to treat runoff for the completed development. During early grading operations, the site will have minimal composite impervious surfaces without the proposed roads and vacant lots. This will allow more runoff to infiltrate the ground, reducing the amount of runoff that needs to be caught by the roadside swales and sediment basins.

Once the project progresses past early grading operations, Sediment Basin 1 and Sediment Basin 2 will each be converted into Pond B and Pond A respectively. The temporary outlet structures will be replaced with permanent outlet structures. Each Pond will have a concrete forebay and trickle channel. Both ponds

will release treated flows at less than historic rates to minimize adverse impacts downstream. Both ponds will discharge into Major Drainageway MS-06. The final design for Ponds A and B will be included in the Final Drainage report.

Specific Details

Four Step Process to Minimize Adverse Impacts of Urbanization

In accordance with the El Paso County Drainage Criteria Manual Volume 2, this site has implemented the four step process to minimize adverse impacts of urbanization. The four step process includes reducing runoff volumes, stabilizing drainageways, treating the water quality capture volume (WQCV), and consider the need for Industrial Commercial BMP's.

Step 1, Reducing Runoff Volumes: The development of the project site is proposed single family residential lots (2.5 ac. min.) with open spaces and lawn areas interspersed within the development which helps disconnect impervious areas and reduce runoff volumes. Roadways utilize soil riprap lined roadside ditches further disconnecting impervious areas. These practices will also allow for increased infiltration and reduce runoff volume.

Step 2, Stabilize Drainageways: Filing 5 utilizes roadside ditches with culvert crossings throughout. These roadside ditches direct the on-site development flows to the proposed detention ponds within the project that releases at or below historic rates into Drainageway MS-06. Based upon the proposed reduction in released flows compared to the pre-developed flows, no impacts to downstream Drainageway MS-06 or Drainageway WF-R7A are anticipated.

Step 3, Provide WQCV: Runoff from this development is treated through capture and slow release of the WQCV in a full spectrum water quality and detention pond that is designed per current El Paso County drainage criteria.

Step 4 Consider the need for Industrial and Commercial BMP's: No industrial or commercial uses are proposed within this development. However, a site specific storm water quality and erosion control plan and narrative are prepared in conjunction with this report. Site specific temporary source control BMPs as well as permanent BMP's are detailed in this plan and narrative to protect receiving waters.

Water Quality

In accordance with Section 13.3.2.1 of the CCS/EPCDCM, full spectrum water quality and detention are provided for all developed basins. Outlet structure release rates are limited to less than historic rates to minimize adverse impacts to downstream stormwater facilities. Complete pond and outlet structure designs are presented in Appendix D.

for information

Erosion Control Plan

The El Paso County Drainage Criteria Manual specifies an Erosion Control Plan and associated cost estimate must be submitted since this project is disturbing more than 1 acre. The Early Erosion Control Plans for Filing 5 have been submitted concurrently with this report.

Saddlehorn Ranch?

Operation & Maintenance

In order to ensure the function and effectiveness of the stormwater infrastructure, maintenance activities such as inspection, routine maintenance, restorative maintenance, rehabilitation and repair, are required. All proposed drainage structures within the any platted County ROW will be owned and maintained by El Paso County. All proposed drainage structures within easements or tracts will be owned and maintained by the 824 Acre Metropolitan No. 1. Vegetation in the natural and improved portions of Drainageway MS-06 with the Filing 5 improvements is the responsibility of 824 Acre Metropolitan District No. 1. This includes all mowing, seeding and weed control activities. An Inspection & Maintenance Plan is submitted concurrently with this drainage report that details the required maintenance activities and intervals to ensure proper function of all stormwater infrastructure in the future.

Drainage and Bridge Fees

Drainage and Bridge Fees are not due with the early grading permit application. An estimate of basin fees for the proposed development within Haegler Ranch drainage basin will be calculated and provided with the Filing 5 Final Drainage Report.

SUMMARY

The proposed development remains consistent with pre-development drainage conditions with the construction of the recommended drainage improvements, including ditches, culverts, detention ponds and drainage channel improvements. The proposed development will not adversely affect the offsite major drainageways or surrounding development. This report meets the latest El Paso County Drainage Criteria requirements for this site and is in accordance with the PDR/MDDP for Saddlehorn Ranch.

Delete if not included with EGP

REFERENCES:

1. City of Colorado Springs Drainage Criteria Manual Volume 1, City of Colorado Springs, CO, May 2014.

2. Urban Storm Drainage Criteria Manual, Urban Drainage and Flood Control District, Latest Revision.

3. Master Development Drainage Plan and Preliminary Drainage Report for Saddlehorn Ranch, JR Engineering, May 2020.

4. Haegler Ranch Drainage Basin Planning Study, URS Corporation, May 2009.

5. The Santa Fe Springs – Haegler Ranch Drainage Basin LOMR, Federal Emergency Management Agency, October 20, 2004.

6. Final Drainage Report for Saddlehorn Ranch – Filing 3, JR Engineering, February 4, 2022

APPENDIX A

FIGURES AND EXHIBITS

5000 2500 0 5000 10000

ORIGINAL SCALE: 1" = 5000'

VICINITY MAP
SADDLEHORN RANCH
25142.07
08/29/21
SHEET 1 OF 1

J·R ENGINEERING
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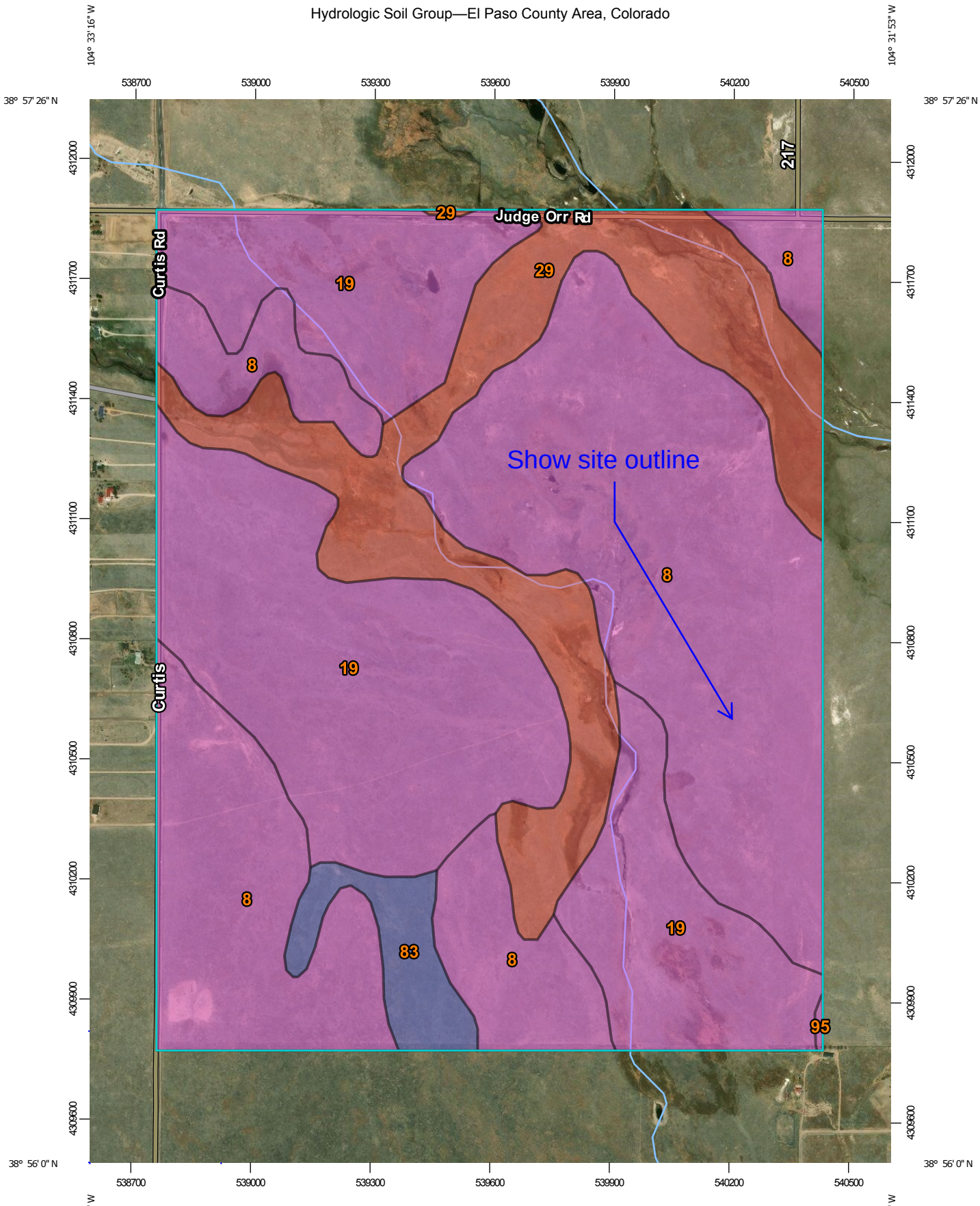

104° 33' 16" W
104° 31' 53" W
538700
539000
539300
539600
539900
540200
540500
38° 57' 26" N
38° 56' 0" N
4312000
4311700
4311400
4311100
4310800
4310500
4310200
4309900
4309600
217
Judge Orr Rd
Curtis Rd
Curtis
29
19
8
83
95
Show site outline
Map Scale: 1:13,000 if printed on A portrait (8.5" x 11") sheet.
N
Meters
0 150 300 600 900
Feet
0 500 1000 2000 3000
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Lines

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Points

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 16, Sep 10, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2016—Aug 17, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	388.3	44.6%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	307.3	35.3%
29	Fluvaquentic Haplaquolls, nearly level	D	150.0	17.2%
83	Stapleton sandy loam, 3 to 8 percent slopes	B	24.6	2.8%
95	Truckton loamy sand, 1 to 9 percent slopes	A	0.6	0.1%
Totals for Area of Interest			**870.8**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

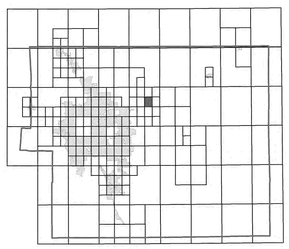
Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

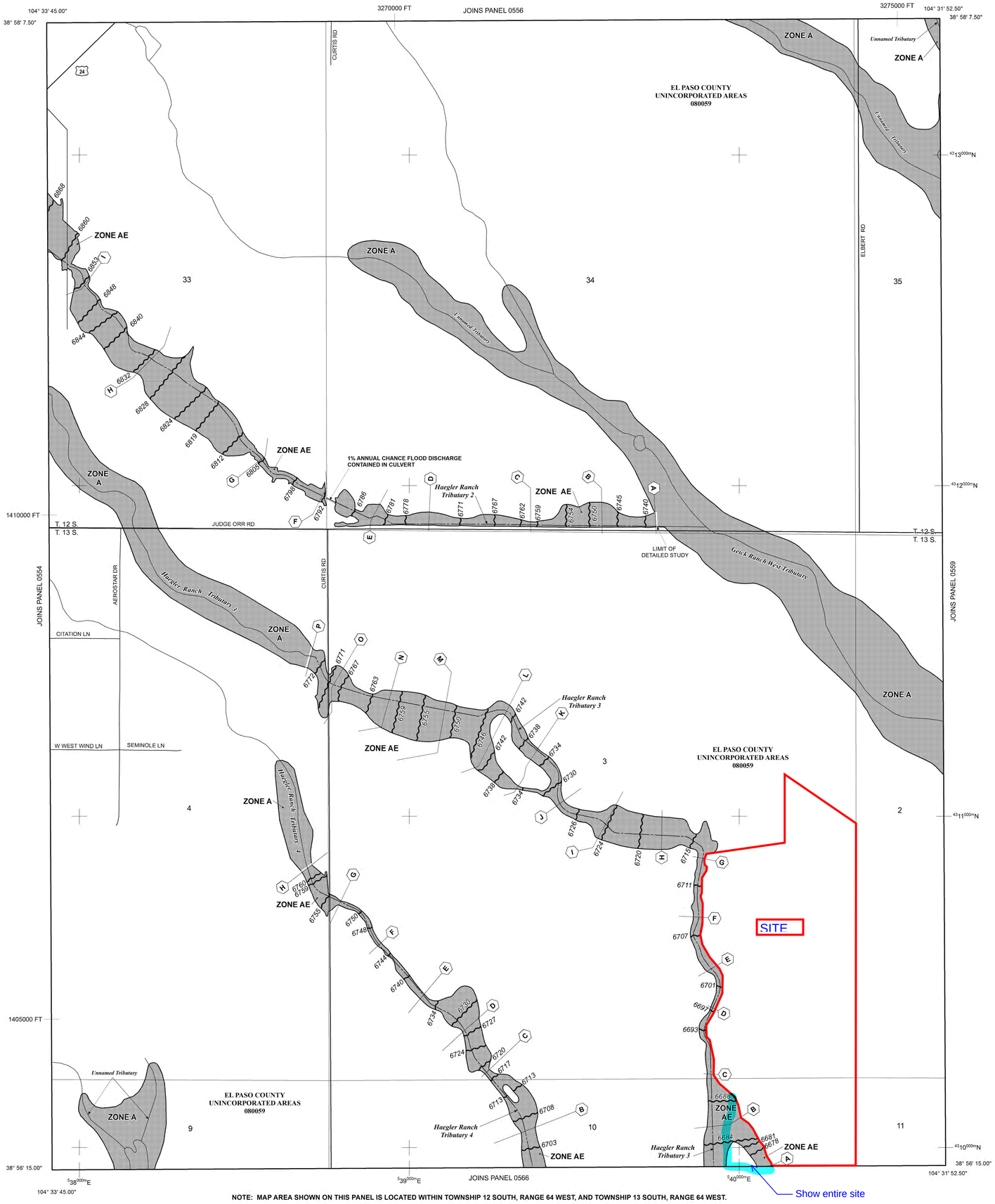
Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 64 WEST, AND TOWNSHIP 13 SOUTH, RANGE 64 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

	Floodplain boundary
	Floodway boundary
	Zone D Boundary
	CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513	Base Flood Elevation line and value; elevation in feet*
(EL 987)	Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A — A	Cross section line
23 — 23	Transect line
97° 07' 30.00" 32° 22' 30.00"	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4275000mN	1000-meter Universal Transverse Mercator grid ticks, zone 13
6000000 FT	5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
DX5510	Bench mark (see explanation in Notes to Users section of this FIRM panel)
M1.5	River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'
250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

PANEL 0558G

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO
AND INCORPORATED AREAS

PANEL 558 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY	080059	0558	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0558G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency

APPENDIX B

HYDROLOGIC CALCULATIONS

COMPOSITE % IMPERVIOUS CALCULATIONS

Subdivision: Saddlehorn Ranch Filing 5 Early Grading
Location: El Paso County

Project Name: Saddlehorn Ranch
Project No.: 25142.07
Calculated By: WKN
Checked By: TBD
Date: 8/12/22

Basin ID	Total Area (ac)	Paved Roads			2.5 Acre Rural Lots			Lawns			Basins Total Weighted % Imp.
		% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	
A1	15.08	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	15.08	2.0%	2.0%
B1	12.57	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	12.57	2.0%	2.0%
B2	12.64	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	12.64	2.0%	2.0%
B3	10.83	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	10.83	2.0%	2.0%
B4	9.16	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	9.16	2.0%	2.0%
B5	13.72	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	13.72	2.0%	2.0%
B6	1.50	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	1.50	2.0%	2.0%
C1	1.26	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	1.26	2.0%	2.0%
C2	4.19	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	4.19	2.0%	2.0%
UD1	8.14	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	8.14	2.0%	2.0%
UD2	25.14	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	25.14	2.0%	2.0%
UD3	9.85	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	9.85	2.0%	2.0%
UD4	2.68	45%	0.00	0.0%	6.2%	0.00	0.0%	2%	2.68	2.0%	2.0%
TOTAL	126.76										2.0%

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis--Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

2.5 Acre Rural Lots - Comp. % Impervious Calculation			
Total Area (ac)	Area (ac) - Roofs (90%)	Area (ac)- Drives (100%)	Area (ac) - Lawns (2%)
2.50	0.068	0.046	2.39
Comp % Imperviousness	6.20%		

Roads w/ Roadside Ditches - Comp. % Impervious Calculation		
Area* (ac)	Area - Ditch (5%)	Area - Roads (100%)
0.2124	0.1320	0.0804
Comp % Imperviousness	0.41	

*Area based on 250 LF roadway from CL to outside edge of roadside ditch
the above conservatively rounded to 45%.

Per ECM Table 3-1 Appendix L, 2.5 acres lots have 11% imperviousness

Add note about plat restriction to 10% imperviousness

COMPOSITE RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Saddlehorn Ranch Filing 5 Early Grading
Location: El Paso County

Project Name: Saddlehorn Ranch
Project No.: 25142.07
Calculated By: WKN
Checked By: TBD
Date: 8/12/22

Basin ID	Total Area (ac)	Basins Total Weighted % Imp.	Hydrologic Soil Group			Hydrologic Soil Group			Minor Coefficients			Major Coefficients			Basins Total Weighted C_5	Basins Total Weighted C_{100}
			Area A (ac)	Area B (ac)	Area C/D (ac)	% A (ac)	% B (ac)	% C/D (ac)	$C_{5,A}$	$C_{5,B}$	$C_{5,C/D}$	$C_{100,A}$	$C_{100,B}$	$C_{100,C/D}$		
A1	15.08	2.0%	15.08	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B1	12.57	2.0%	12.57	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B2	12.64	2.0%	12.64	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B3	10.83	2.0%	10.83	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B4	9.16	2.0%	9.16	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B5	13.72	2.0%	13.72	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
B6	1.50	2.0%	1.50	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
C1	1.26	2.0%	1.26	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
C2	4.19	2.0%	4.19	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
UD1	8.14	2.0%	8.14	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
UD2	25.14	2.0%	25.14	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
UD3	9.85	2.0%	9.85	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
UD4	2.68	2.0%	2.68	0.00	0.00	100%	0%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.13
TOTAL	126.76	2.0%	126.76	0.00	0.00	100%	0%	0%	---	---	---	---	---	---	0.01	0.13

Table 6-4. Runoff coefficient equations based on NRCS soil group and storm return period

NRCS Soil Group	Storm Return Period						
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	500-Year
A	$C_A = 0.84i^{1.302}$	$C_A = 0.86i^{1.276}$	$C_A = 0.87i^{1.232}$	$C_A = 0.84i^{1.124}$	$C_A = 0.85i+0.025$	$C_A = 0.78i+0.110$	$C_A = 0.65i+0.254$
B	$C_B = 0.84i^{1.169}$	$C_B = 0.86i^{1.088}$	$C_B = 0.81i+0.057$	$C_B = 0.63i+0.249$	$C_B = 0.56i+0.328$	$C_B = 0.47i+0.426$	$C_B = 0.37i+0.536$
C/D	$C_{C/D} = 0.83i^{1.122}$	$C_{C/D} = 0.82i+0.035$	$C_{C/D} = 0.74i+0.132$	$C_{C/D} = 0.56i+0.319$	$C_{C/D} = 0.49i+0.393$	$C_{C/D} = 0.41i+0.484$	$C_{C/D} = 0.32i+0.588$

Where:

i = % imperviousness (expressed as a decimal)

C_A = Runoff coefficient for Natural Resources Conservation Service (NRCS) HSG A soils

C_B = Runoff coefficient for NRCS HSG B soils

$C_{C/D}$ = Runoff coefficient for NRCS HSG C and D soils.

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Saddlehorn Ranch Filing 5 Early Grading
Location: El Paso County

Project Name: Saddlehorn Ranch
Project No.: 25142.07
Calculated By: WKN
Checked By: TBD
Date: 8/12/22

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A1	15.08	A	2%	0.01	0.13	300	4.8%	20.4	1330	2.9%	7.0	1.2	18.6	39.0	1630.0	39.7	39.0
B1	12.57	A	2%	0.01	0.13	282	4.9%	19.6	1160	1.3%	15.0	1.7	11.5	31.2	1442.0	44.3	31.2
B2	12.64	A	2%	0.01	0.13	20	20.0%	3.3	1561	4.6%	15.0	3.2	8.1	11.4	1581.0	38.7	11.4
B3	10.83	A	2%	0.01	0.13	300	2.0%	27.2	1117	3.1%	15.0	2.6	7.0	34.3	1417.0	37.1	34.3
B4	9.16	A	2%	0.01	0.13	300	3.3%	23.1	997	1.5%	15.0	1.8	9.0	32.1	1297.0	40.3	32.1
B5	13.72	A	2%	0.01	0.13	41	9.0%	6.1	3242	1.6%	15.0	1.9	28.9	35.1	3283.0	72.4	35.1
B6	1.50	A	2%	0.01	0.13	76	4.0%	10.9	448	0.5%	20.0	1.4	5.3	16.2	524.0	37.0	16.2
C1	1.26	A	2%	0.01	0.13	143	2.4%	17.7	184	1.0%	15.0	1.5	2.0	19.7	327.0	29.0	19.7
C2	4.19	A	2%	0.01	0.13	154	3.0%	17.1	455	1.0%	15.0	1.5	5.1	22.1	609.0	33.8	22.1
UD1	8.14	A	2%	0.01	0.13	300	3.6%	22.4	267	5.5%	7.0	1.6	2.7	25.1	567.0	27.7	25.1
UD2	25.14	A	2%	0.01	0.13	300	1.7%	28.7	367	4.1%	7.0	1.4	4.3	33.0	667.0	28.9	28.9
UD3	9.85	A	2%	0.01	0.13	300	1.8%	28.2	552	1.3%	7.0	0.8	11.4	39.5	852.0	34.2	34.2
UD4	2.68	A	2%	0.01	0.13	300	5.1%	19.9	360	2.1%	7.0	1.0	5.9	25.8	660.0	30.1	25.8

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$ Equation 6-4

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_o = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_o}$
K = NRCS conveyance factor (see Table 6-2).

Equation 6-2 $$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$ Equation 6-3

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$ Equation 6-5

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Saddlehorn Ranch Filing 5 Early Grading
Location: El Paso County
Design Storm: 5-Year

Project Name: Saddlehorn Ranch
Project No.: 25142.07
Calculated By: WKN
Checked By: TBD
Date: 8/12/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C* A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C* A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C* A (ac)	Slope (%)	Q_{pipe} (cfs)	C* A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	B1	12.57	0.01	31.2	0.08	2.42	0.2					0.2	0.08	1.1					791	2.1	6.3	Roadside Swale Swale conveyance to DP 1.0
	2	B2	12.64	0.01	11.4	0.08	3.94	0.3					0.3	0.08	1.1					0	2.1	0.0	Roadside Swale Swale conveyance to DP 1.0
	1.0								37.4	0.16	2.15	0.3	0.3	0.16	2.99					804	3.5	3.9	Sum of DP 1 and DP 2 Swale conveyance to DP 1.1
	3	B3	10.83	0.01	34.3	0.06	2.28	0.1					0.1	0.06	1.0					0	2.0	0.0	Roadside Swale Swale conveyance to DP 1.1
	1.1								41.3	0.22	2.00	0.4	0.4	0.22	1.0					513	2.0	4.3	Sum of DP 1.0 & DP 3 Swale conveyance to DP 1.2
	4	B4	9.16	0.01	32.1	0.05	2.38	0.1					0.1	0.05	1.9					0	2.7	0.0	Roadside Swale Swale conveyance to DP 1.2
	1.2								45.6	0.27	1.85	0.5	0.5	0.27	0.6					488	1.5	5.3	Sum of DP 1.1 & DP 4 Swale conveyance to DP 1.3
	5	B5	13.72	0.01	35.1	0.08	2.25	0.2					0.2	0.08	1.1					0	2.1	0.0	Swale Trickle Channel conveyance to DP 1.4
	1.3								50.8	0.35	1.69	0.6	0.6	0.35	0.5					466	1.4	5.5	Sum of DP 1.3 and DP 5 Pond conveyance to DP 1.4
								0.03					0.03	0.01	1.1					0	2.1	0.0	Proposed Pond B Pond conveyance to DP 1.4
	1.4								50.8	0.36	1.69	0.6	0.6	0.36	1.0					136	2.0	1.1	Sum of DP 1.4 and DP 6 Oulet structure release into Drainageway MS-06
	C1	C1	1.26	0.01	19.7	0.01	3.11	0.0					0.03	0.01	1.9						2.7		Roadside Swale Swale conveyance to Pond C. See Filing 4 for calculations
	C2	C2	4.19	0.01	22.1	0.03	2.94	0.1					0.1	0.03	1.9						2.7		Roadside Swale Swale conveyance to Pond C. See Filing 4 for calculations
	11	A1	15.08	0.01	39.0	0.15	2.09	0.3															Overland Flow Sheet flow into Sediment Basin 2
	UD1	UD1	8.14	0.01	25.1	0.05	2.75	0.1															Overland Flow Sheet flow into Drainageway WF-R7A
	UD2	UD2	25.14	0.01	28.9	0.15	2.54	0.4															Overland Flow Sheet flow into Drainageway MS-06
	UD3	UD3	9.85	0.01	34.2	0.06	2.28	0.1															Overland Flow Sheet flow into Drainageway MS-06
	UD4	UD4	2.68	0.01	25.8	0.03	2.71	0.1															Overland Flow Sheet flow into Drainageway MS-06

increase decimal places

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Saddlehorn Ranch Filing 5 Early Grading
Location: El Paso County
Design Storm: 100-Year

Project Name: Saddlehorn Ranch
Project No.: 25142.07
Calculated By: WKN
Checked By: TBD
Date: 8/12/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	i (in/hr)	Q (cfs)	tc (min)	C*A (ac)	i (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	B1	12.57	0.13	31.2	1.58	4.07	6.4					6.4	1.58	1.1					791	2.1	6.3	Roadside Swale Swale conveyance to DP 1.0
	2	B2	12.64	0.13	11.4	1.59	6.61	10.5					10.5	1.59	1.1					0	2.1	0.0	Roadside Swale Swale conveyance to DP 1.0
	1.0								37.4	3.17	3.61	11.4	11.4	3.17	2.99					804	3.5	3.9	Sum of DP 1 and DP 2 Swale conveyance to DP 1.1
	3	B3	10.83	0.13	34.3	1.36	3.83	5.2					5.2	1.36	1.0					0	2.0	0.0	Roadside Swale Swale conveyance to DP 1.1
	1.1								41.3	4.53	3.36	15.2	15.2	4.53	1.0					513	2.0	4.3	Sum of DP 1.0 & DP 3 Swale conveyance to DP 1.2
	4	B4	9.16	0.13	32.1	1.15	3.99	4.6					4.6	1.15	1.9					0	2.7	0.0	Roadside Swale Swale conveyance to DP 1.2
	1.2								45.6	5.68	3.11	17.7	17.7	5.68	0.6					488	1.5	5.3	Sum of DP 1.1 & DP 4 Swale conveyance to DP 1.3
	5	B5	13.72	0.13	35.1	1.73	3.77	6.5					6.5	1.73	1.1					0	2.1	0.0	Swale Trickle Channel conveyance to DP 1.4
	1.3								50.8	7.41	2.83	21.0	21.0	7.41	0.5					466	1.4	5.5	Sum of DP 1.3 and DP 5 Pond conveyance to DP 1.4
	6	B6	1.50	0.13	16.2	0.19	5.72	1.1					1.09	0.19	1.1					0	2.1	0.0	Proposed Pond B Pond conveyance to DP 1.4
	1.4								56.3	7.60	2.58	19.6	19.6	7.60	1.0					136	2.0	1.1	Sum of DP 1.4 and DP 6 Oulet structure release into Drainageway MS-06
	C1	C1	1.26	0.13	19.7	0.16	5.22	0.8					0.8	0.16	1.9						2.7		Roadside Swale Swale conveyance to Pond C. See Filing 4 for calculations
	C2	C2	4.19	0.13	22.1	0.53	4.93	2.6					2.6	0.53	1.9						2.7		Roadside Swale Swale conveyance to Pond C. See Filing 4 for calculations
	11	A1	15.08	0.13	39.0	1.96	3.50	6.9															Overland Flow Sheet flow into Sediment Basin 2
	UD1	UD1	8.14	0.13	25.1	1.03	4.61	4.7															Overland Flow Sheet flow into Drainageway WF-R7A
	UD2	UD2	25.14	0.13	28.9	3.17	4.26	13.5															Overland Flow Sheet flow into Drainageway MS-06
	UD3	UD3	9.85	0.13	34.2	1.24	3.83	4.8															Overland Flow Sheet flow into Drainageway MS-06
	UD4	UD4	2.68	0.13	25.8	0.35	4.54	1.6															Overland Flow Sheet flow into Drainageway MS-06

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

APPENDIX C

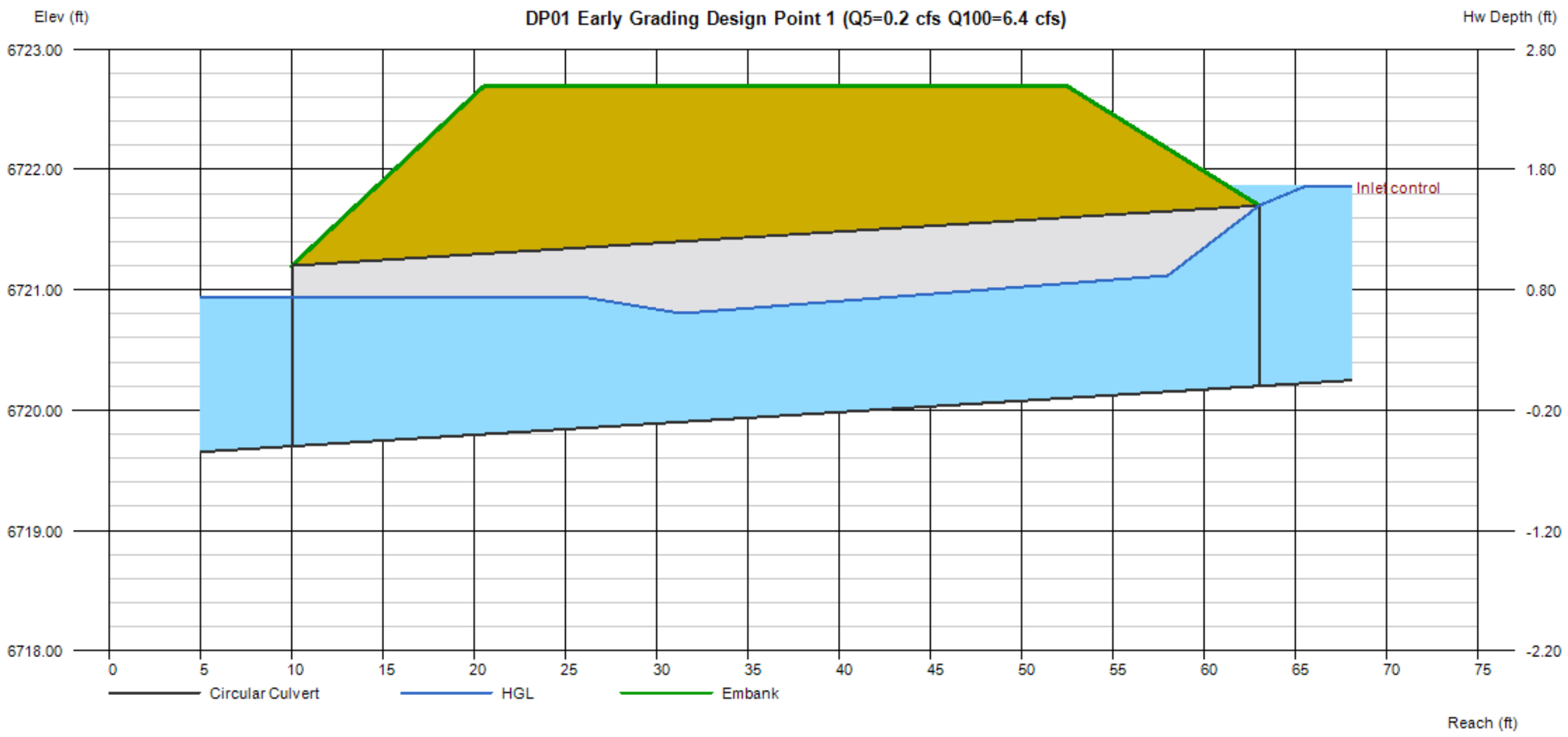
HYDRAULIC CALCULATIONS

Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Aug 12 2022

DP01 Early Grading Design Point 1 (Q5=0.2 cfs Q100=6.4 cfs)

Invert Elev Dn (ft)	= 6719.70	**Calculations**	
Pipe Length (ft)	= 53.00	Qmin (cfs)	= 0.20
Slope (%)	= 0.94	Qmax (cfs)	= 6.40
Invert Elev Up (ft)	= 6720.20	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 18.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 18.0	Qtotal (cfs)	= 6.40
No. Barrels	= 1	Qpipe (cfs)	= 6.40
n-Value	= 0.014	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (ft/s)	= 4.10
Culvert Entrance	= Projecting	Veloc Up (ft/s)	= 5.25
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (ft)	= 6720.94
		HGL Up (ft)	= 6721.18
Embankment		Hw Elev (ft)	= 6721.86
Top Elevation (ft)	= 6722.69	Hw/D (ft)	= 1.11
Top Width (ft)	= 32.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 20.00		



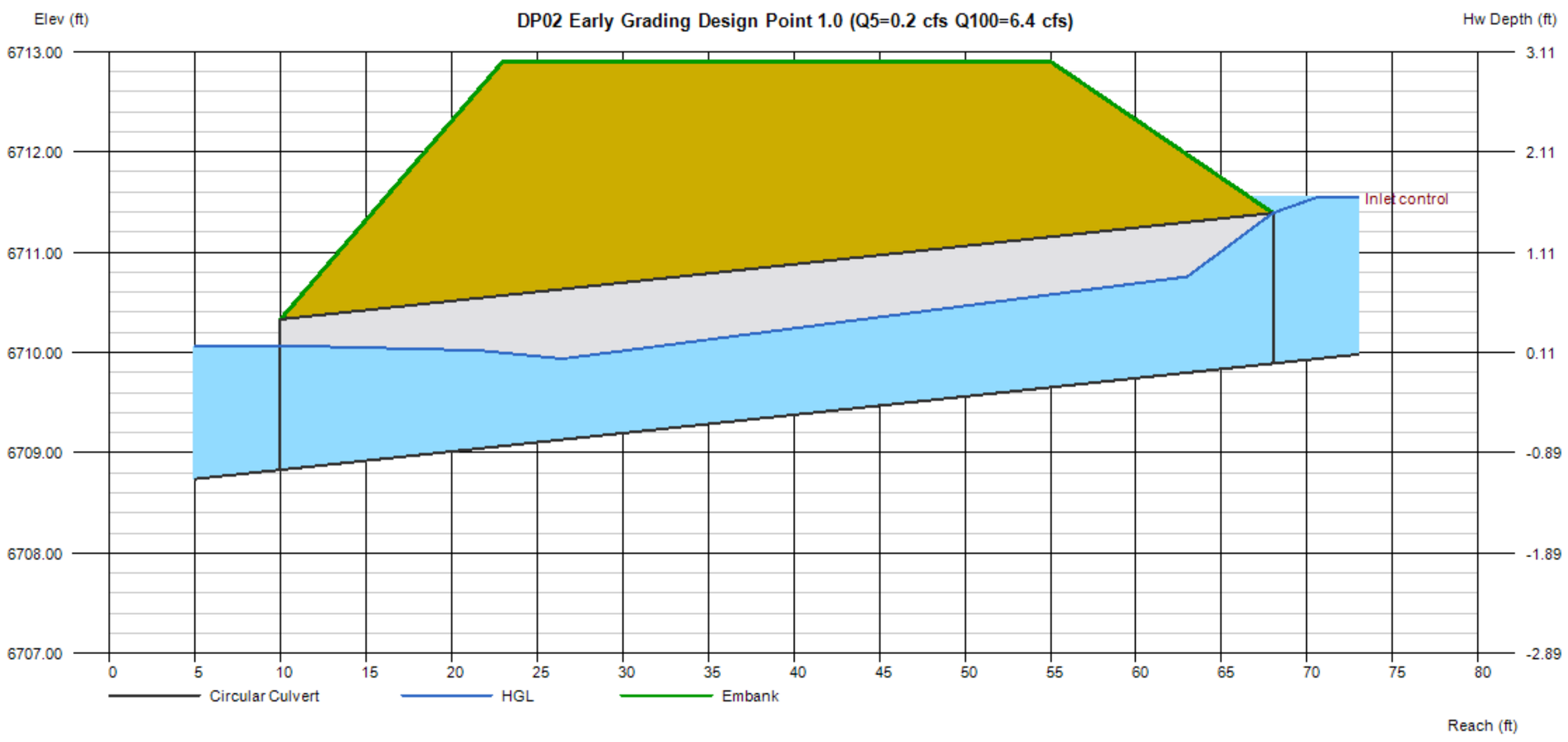
Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Aug 12 2022

Flows do not match hydrology spreadsheet for DP 1.0. Please update sheet with flows shown on hydrology spreadsheet (0.3 & 11.4 cfs)

DP02 Early Grading Design Point 1.0 (Q5=0.2 cfs Q100=6.4 cfs)

Invert Elev Dn (ft)	= 6708.83	**Calculations**	
Pipe Length (ft)	= 58.00	Qmin (cfs)	= 0.20
Slope (%)	= 1.83	Qmax (cfs)	= 6.40
Invert Elev Up (ft)	= 6709.89	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 18.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 18.0	Qtotal (cfs)	= 6.40
No. Barrels	= 1	Qpipe (cfs)	= 6.40
n-Value	= 0.014	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (ft/s)	= 4.10
Culvert Entrance	= Projecting	Veloc Up (ft/s)	= 5.25
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (ft)	= 6710.07
		HGL Up (ft)	= 6710.87
Embankment		Hw Elev (ft)	= 6711.54
Top Elevation (ft)	= 6712.90	Hw/D (ft)	= 1.10
Top Width (ft)	= 32.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 20.00		



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Aug 12 2022

Flows do not match hydrology spreadsheet for DP 1.1. Please update sheet with flows shown on hydrology spreadsheet (0.4 & 15.2 cfs)

DP03 Early Grading Design Point 1.1 (Q5=0.3 cfs Q100=11.4 cfs)

Invert Elev Dn (ft)	=	6684.74
Pipe Length (ft)	=	53.00
Slope (%)	=	0.87
Invert Elev Up (ft)	=	6685.20
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.014
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

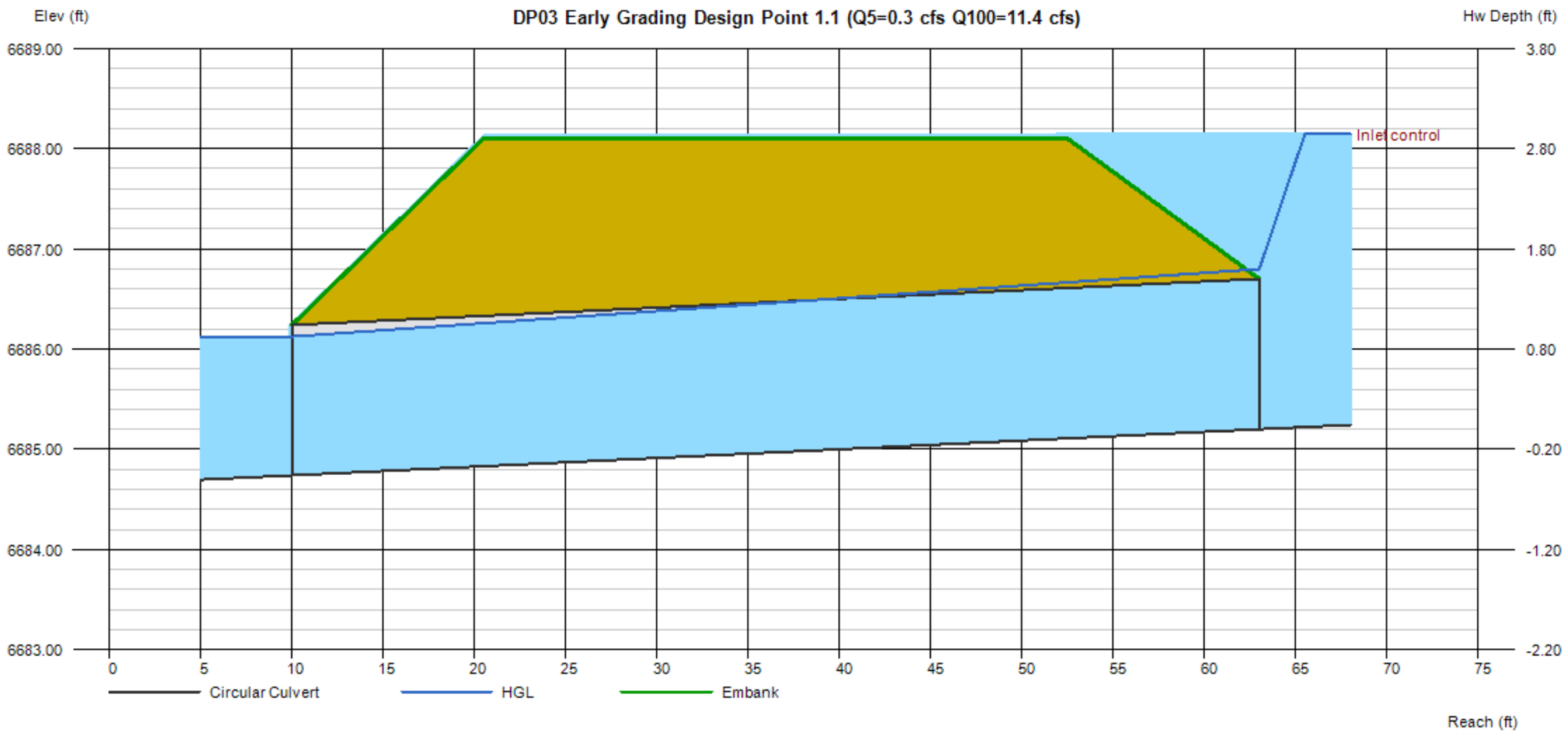
Top Elevation (ft)	=	6688.10
Top Width (ft)	=	32.00
Crest Width (ft)	=	20.00

Calculations

Qmin (cfs)	=	0.30
Qmax (cfs)	=	11.41
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	11.40
Qpipe (cfs)	=	11.00
Qovertop (cfs)	=	0.40
Veloc Dn (ft/s)	=	6.45
Veloc Up (ft/s)	=	6.22
HGL Dn (ft)	=	6686.13
HGL Up (ft)	=	6686.80
Hw Elev (ft)	=	6688.15
Hw/D (ft)	=	1.96
Flow Regime	=	Inlet Control

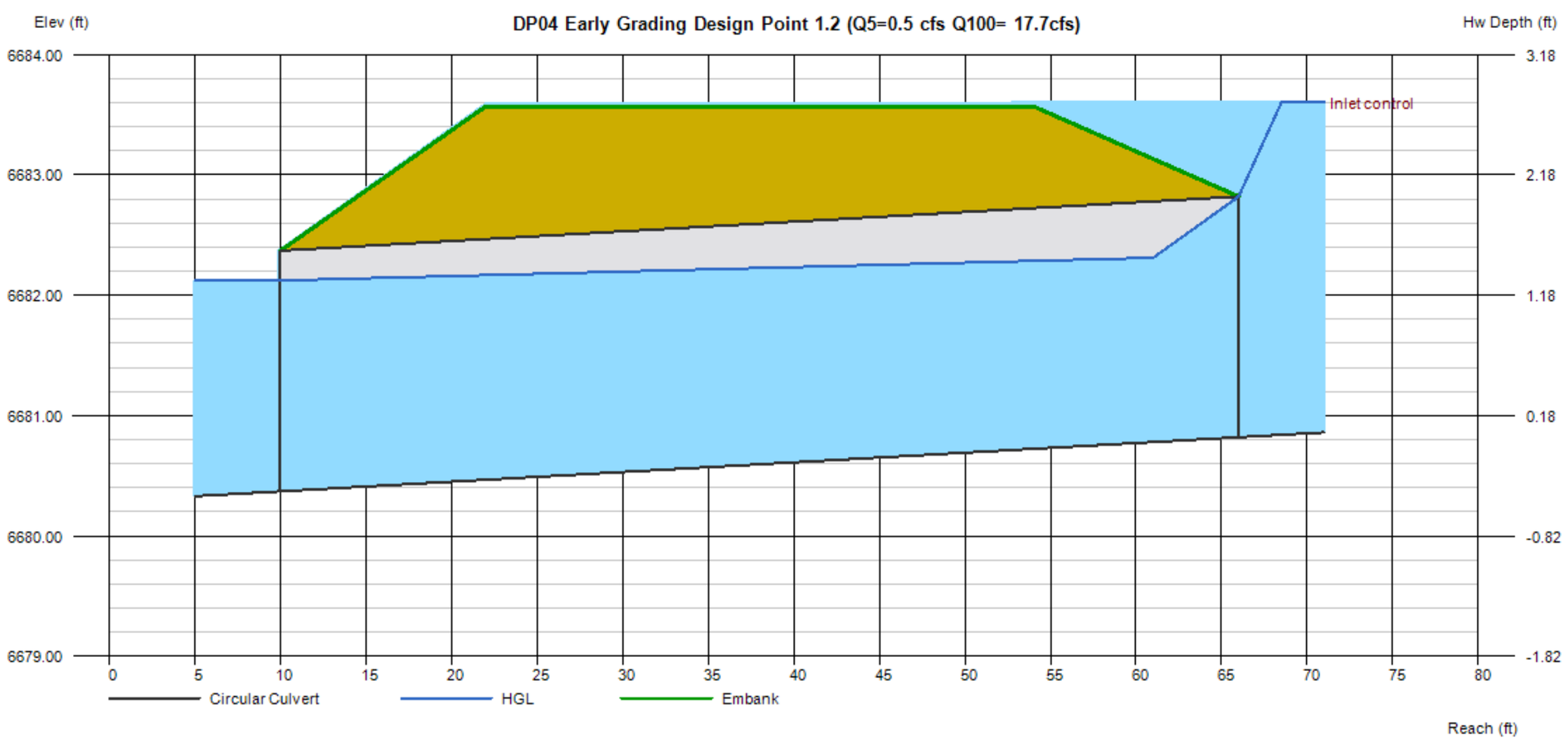


Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Aug 12 2022

DP04 Early Grading Design Point 1.2 (Q5=0.5 cfs Q100= 17.7cfs)

Invert Elev Dn (ft)	= 6680.37	**Calculations**		
Pipe Length (ft)	= 56.00	Qmin (cfs)	= 0.50	
Slope (%)	= 0.80	Qmax (cfs)	= 17.70	
Invert Elev Up (ft)	= 6680.82	Tailwater Elev (ft)	= (dc+D)/2	
Rise (in)	= 24.0			
Shape	= Circular	**Highlighted**		
Span (in)	= 24.0	Qtotal (cfs)	= 17.70	
No. Barrels	= 1	Qpipe (cfs)	= 17.48	
n-Value	= 0.014	Qovertop (cfs)	= 0.22	
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (ft/s)	= 5.99	
Culvert Entrance	= Projecting	Veloc Up (ft/s)	= 6.87	
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (ft)	= 6682.12	
		HGL Up (ft)	= 6682.33	
Embankment		Hw Elev (ft)	= 6683.60	
Top Elevation (ft)	= 6683.57	Hw/D (ft)	= 1.39	
Top Width (ft)	= 32.00	Flow Regime	= Inlet Control	
Crest Width (ft)	= 20.00			



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1 Swale (5-Year)(FR:0.64)

Triangular

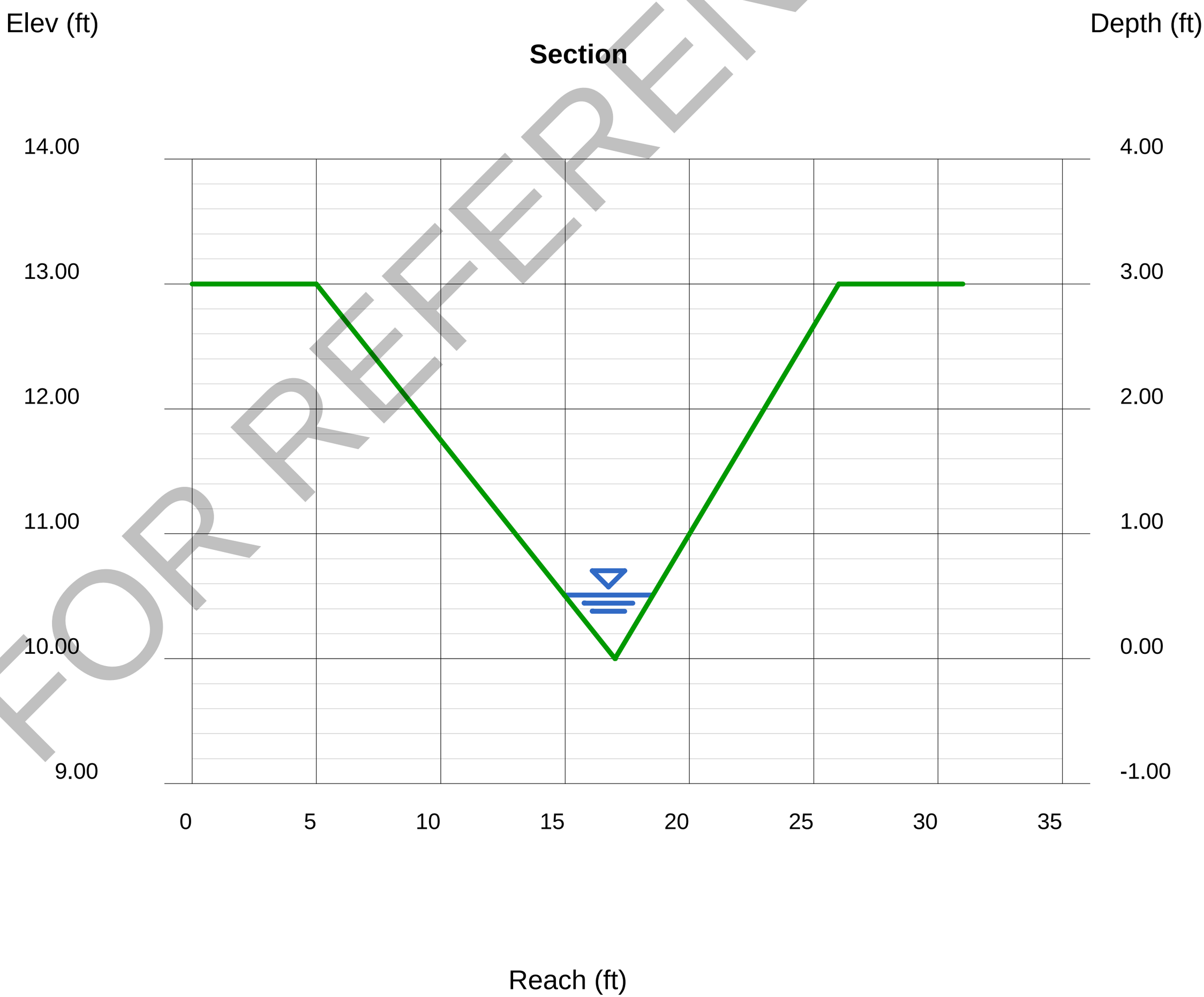
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 1.70

Highlighted

Depth (ft)	= 0.51
Q (cfs)	= 1.700
Area (sqft)	= 0.91
Velocity (ft/s)	= 1.87
Wetted Perim (ft)	= 3.72
Crit Depth, Yc (ft)	= 0.43
Top Width (ft)	= 3.57
EGL (ft)	= 0.56



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1 Swale (100-Year)(FR:0.75)

Triangular

Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 10.50

Highlighted

Depth (ft)	= 1.00
Q (cfs)	= 10.50
Area (sqft)	= 3.50
Velocity (ft/s)	= 3.00
Wetted Perim (ft)	= 7.29
Crit Depth, Yc (ft)	= 0.90
Top Width (ft)	= 7.00
EGL (ft)	= 1.14

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.0 Swale (5-Year)(FR:0.80)

Triangular

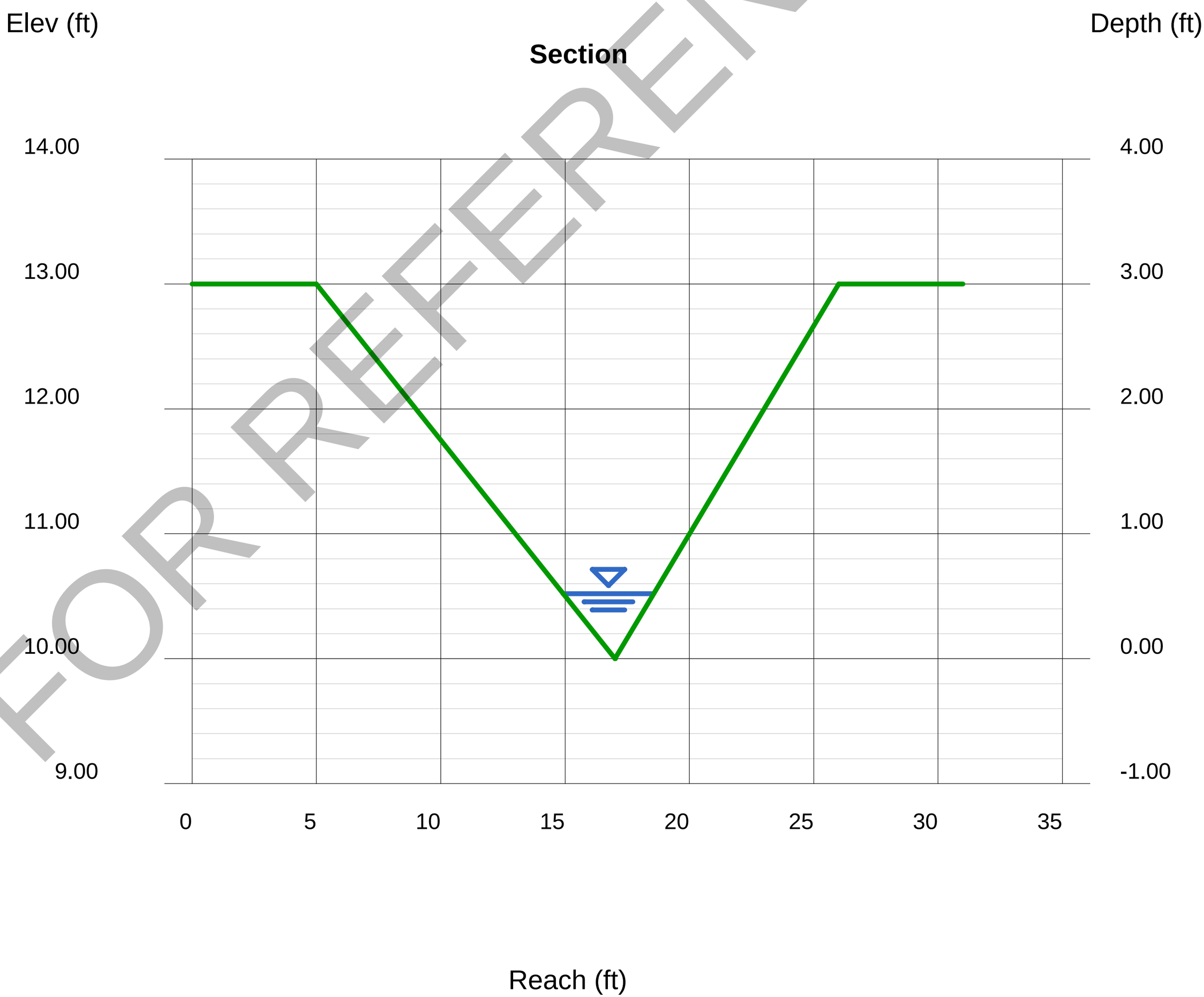
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 3.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 3.10

Highlighted

Depth (ft)	= 0.52
Q (cfs)	= 3.100
Area (sqft)	= 0.95
Velocity (ft/s)	= 3.28
Wetted Perim (ft)	= 3.79
Crit Depth, Yc (ft)	= 0.55
Top Width (ft)	= 3.64
EGL (ft)	= 0.69



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.0 Swale (100-Year)(FR:0.92)

Triangular

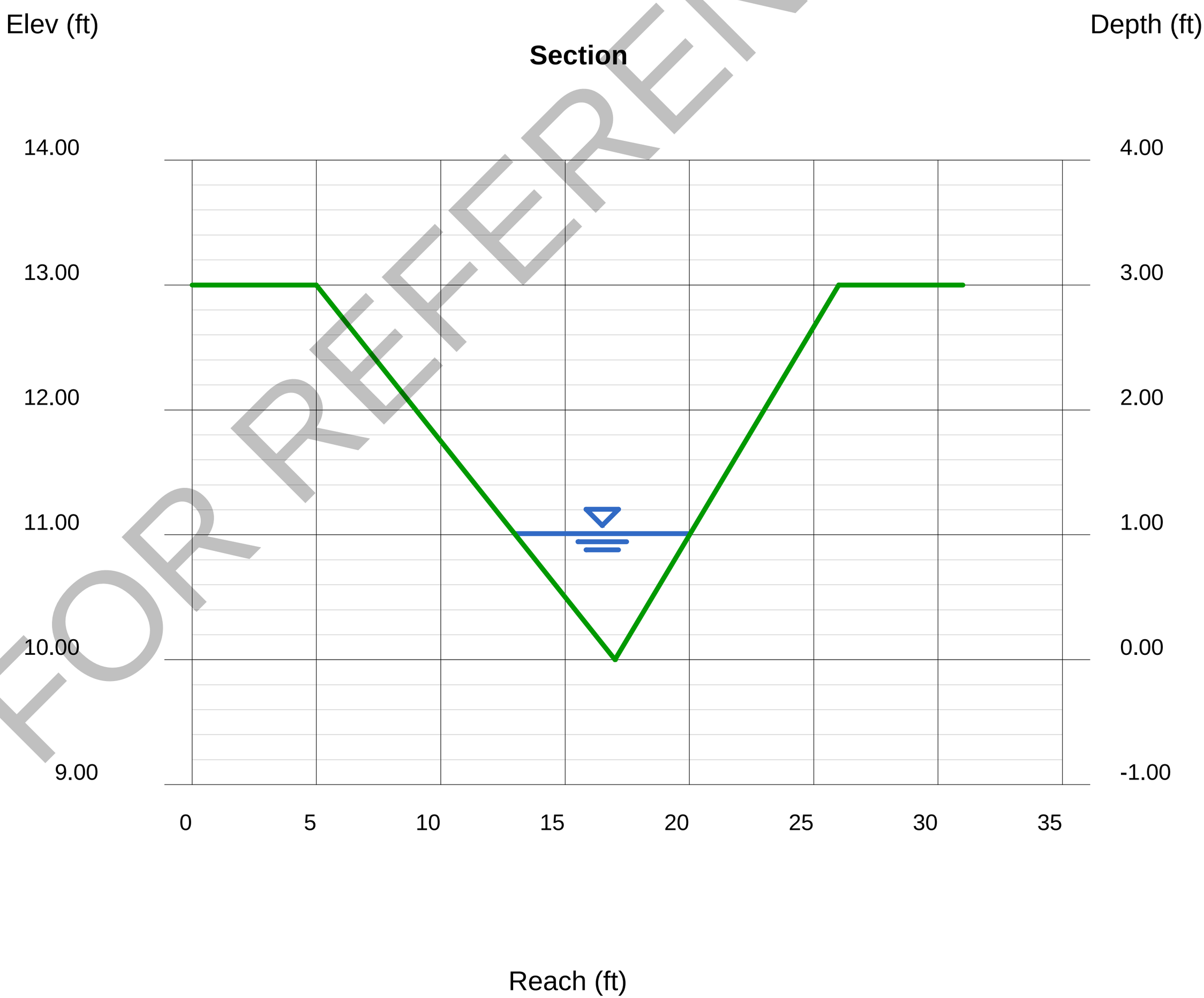
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 3.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 18.70

Highlighted

Depth (ft)	= 1.01
Q (cfs)	= 18.70
Area (sqft)	= 3.57
Velocity (ft/s)	= 5.24
Wetted Perim (ft)	= 7.36
Crit Depth, Yc (ft)	= 1.13
Top Width (ft)	= 7.07
EGL (ft)	= 1.44



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.1 Swale (5-Year)(FR:0.50)

Triangular

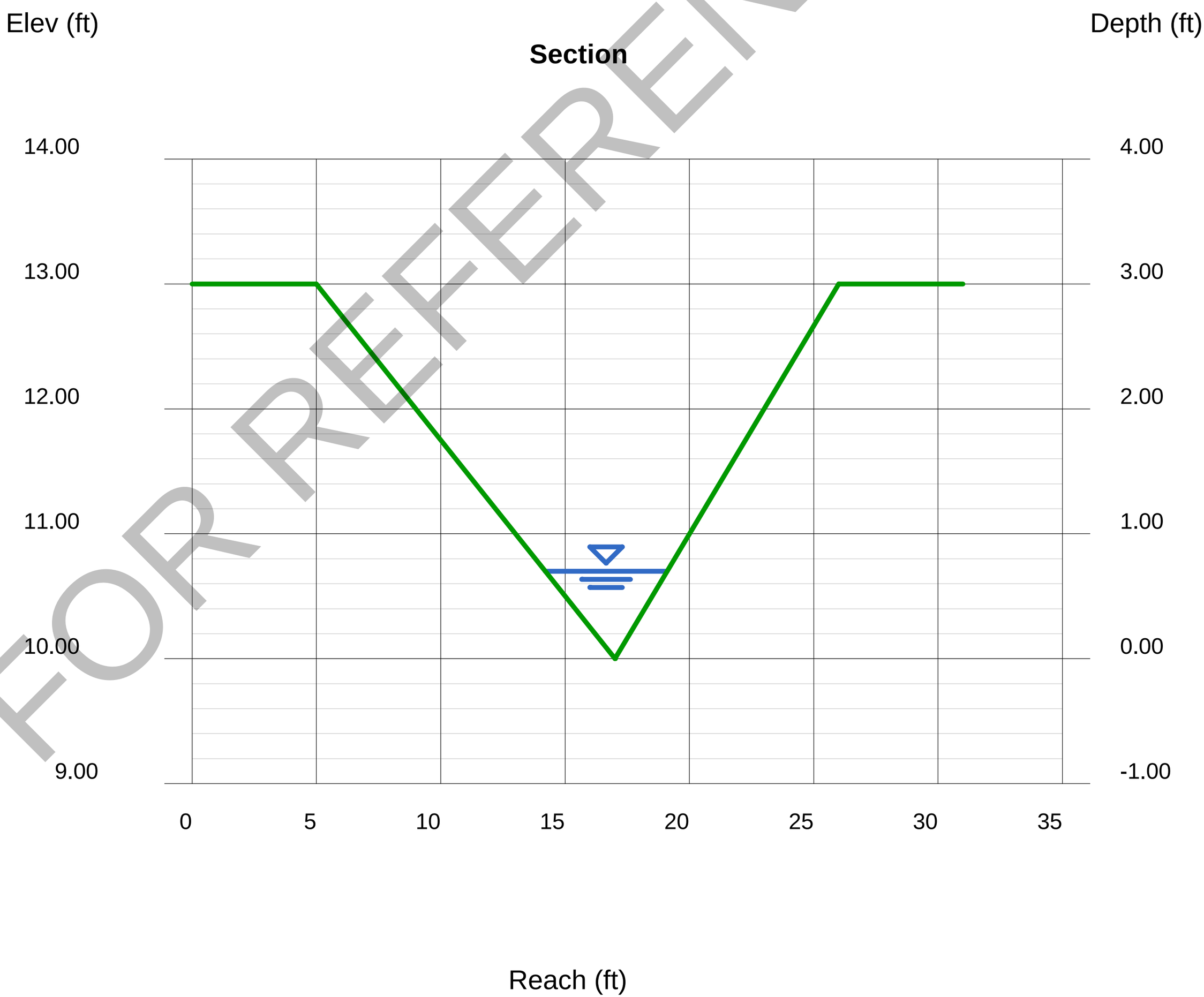
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 4.10

Highlighted

Depth (ft)	= 0.70
Q (cfs)	= 4.100
Area (sqft)	= 1.71
Velocity (ft/s)	= 2.39
Wetted Perim (ft)	= 5.10
Crit Depth, Yc (ft)	= 0.62
Top Width (ft)	= 4.90
EGL (ft)	= 0.79



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.1 Swale (100-Year)(FR:0.56)

Triangular

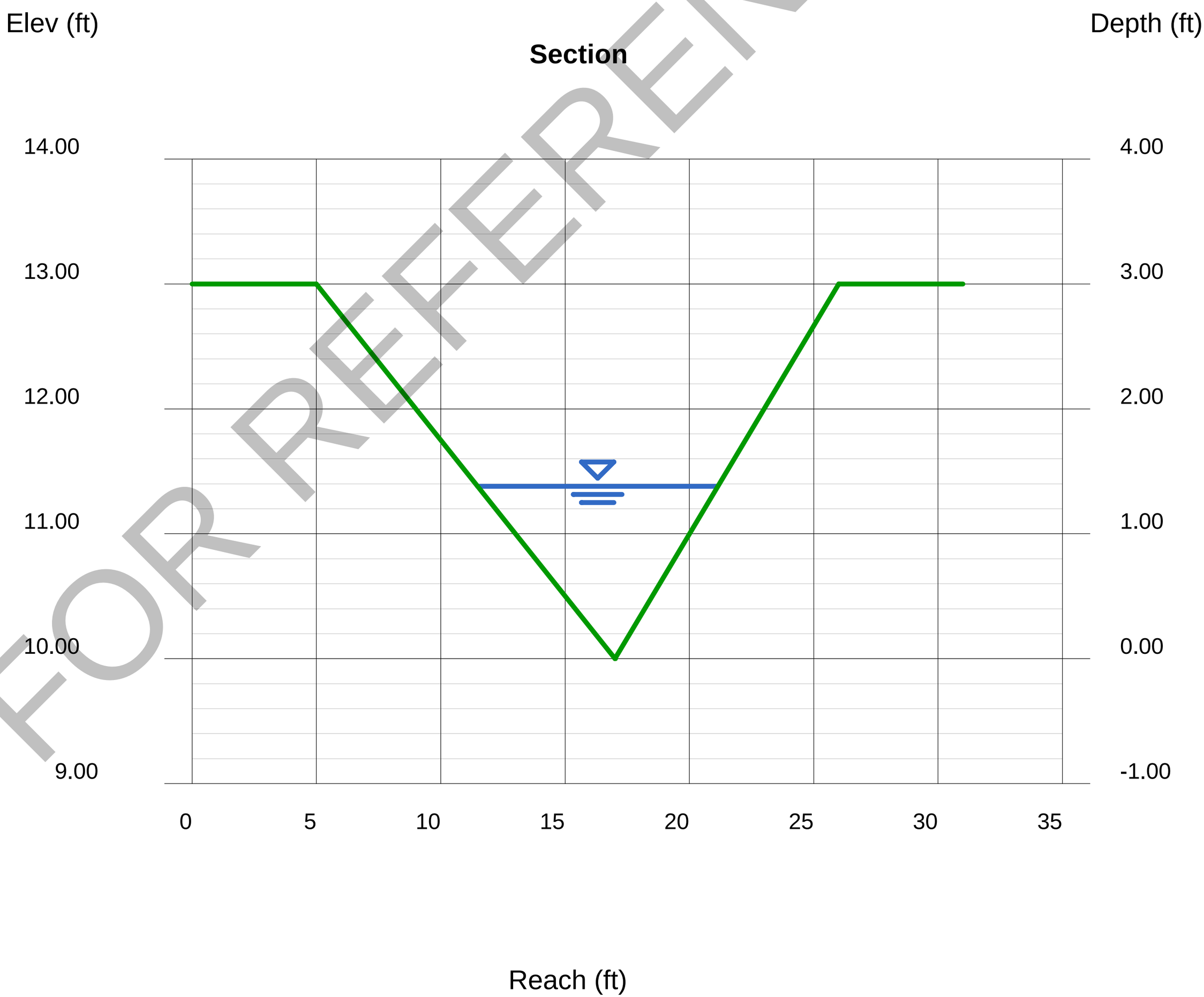
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 24.90

Highlighted

Depth (ft)	= 1.38
Q (cfs)	= 24.90
Area (sqft)	= 6.67
Velocity (ft/s)	= 3.74
Wetted Perim (ft)	= 10.05
Crit Depth, Yc (ft)	= 1.26
Top Width (ft)	= 9.66
EGL (ft)	= 1.60



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.2 Swale (5-Year)(FR:0.77)

Triangular

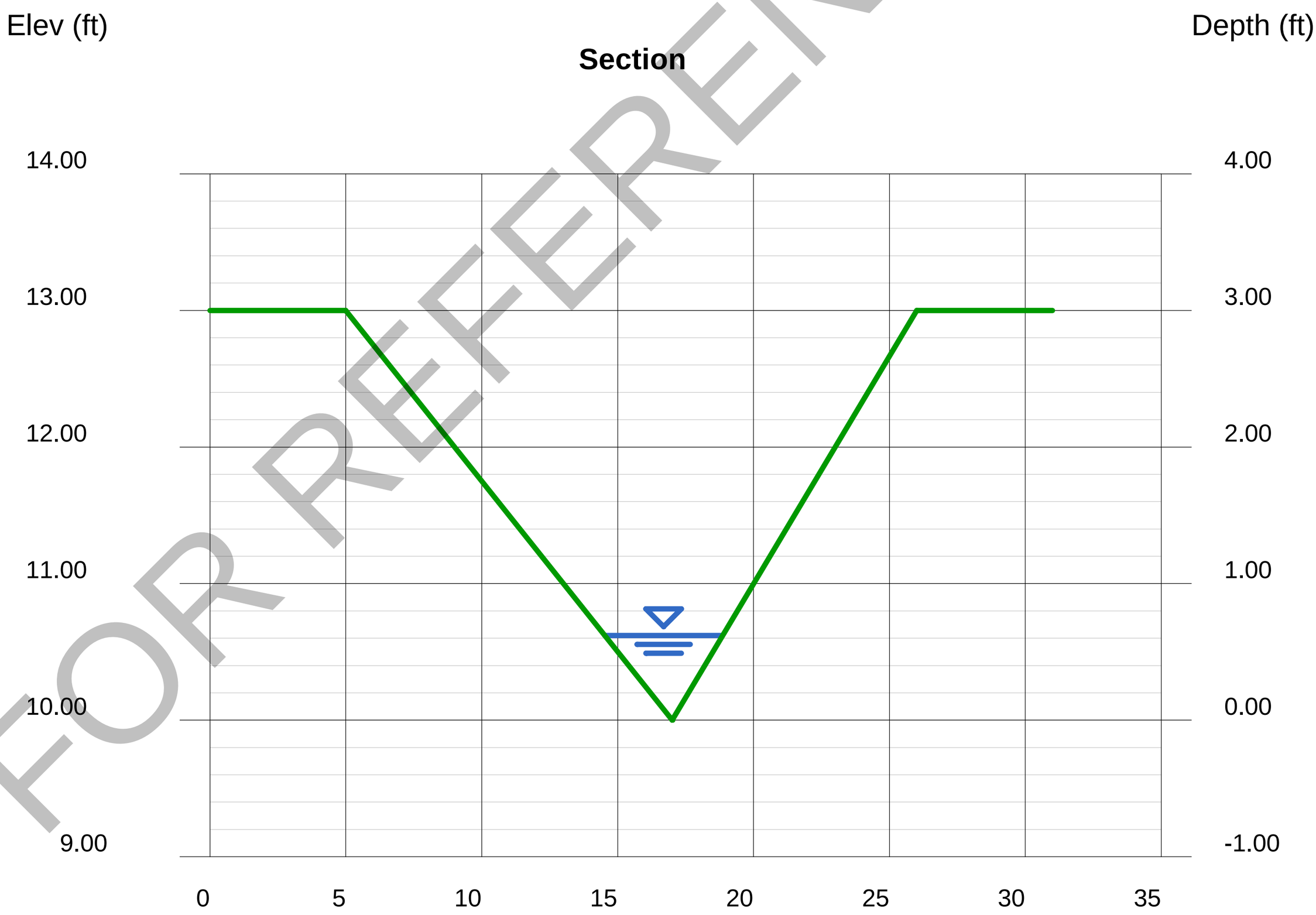
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 2.40
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 4.60

Highlighted

Depth (ft)	= 0.62
Q (cfs)	= 4.600
Area (sqft)	= 1.35
Velocity (ft/s)	= 3.42
Wetted Perim (ft)	= 4.52
Crit Depth, Yc (ft)	= 0.65
Top Width (ft)	= 4.34
EGL (ft)	= 0.80



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.2 Swale (100-Year)(FR:0.85)

Triangular

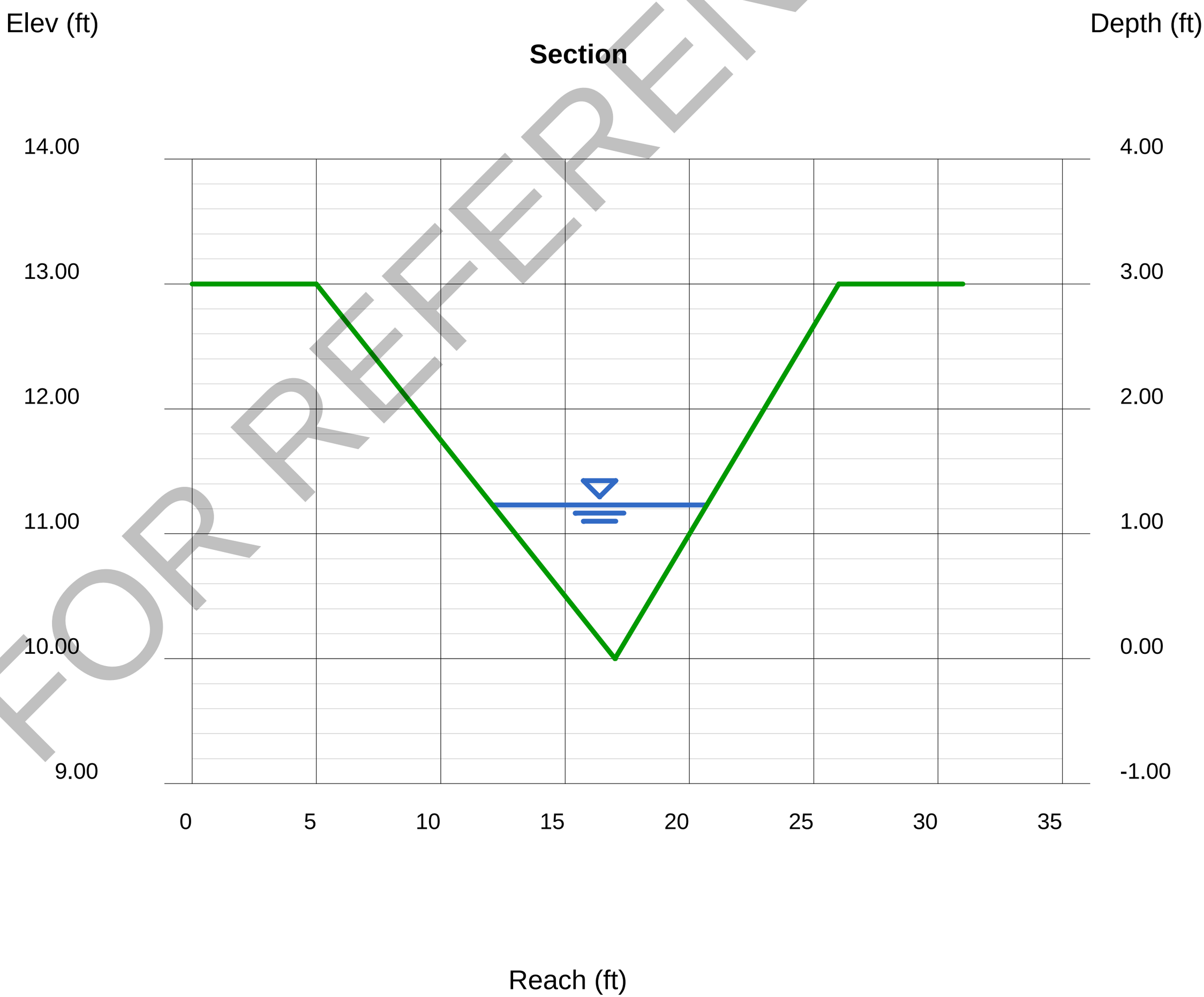
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 2.40
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 28.40

Highlighted

Depth (ft)	= 1.23
Q (cfs)	= 28.40
Area (sqft)	= 5.30
Velocity (ft/s)	= 5.36
Wetted Perim (ft)	= 8.96
Crit Depth, Yc (ft)	= 1.33
Top Width (ft)	= 8.61
EGL (ft)	= 1.68



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.3 Swale (5-Year)(FR:0.42)

Trapezoidal

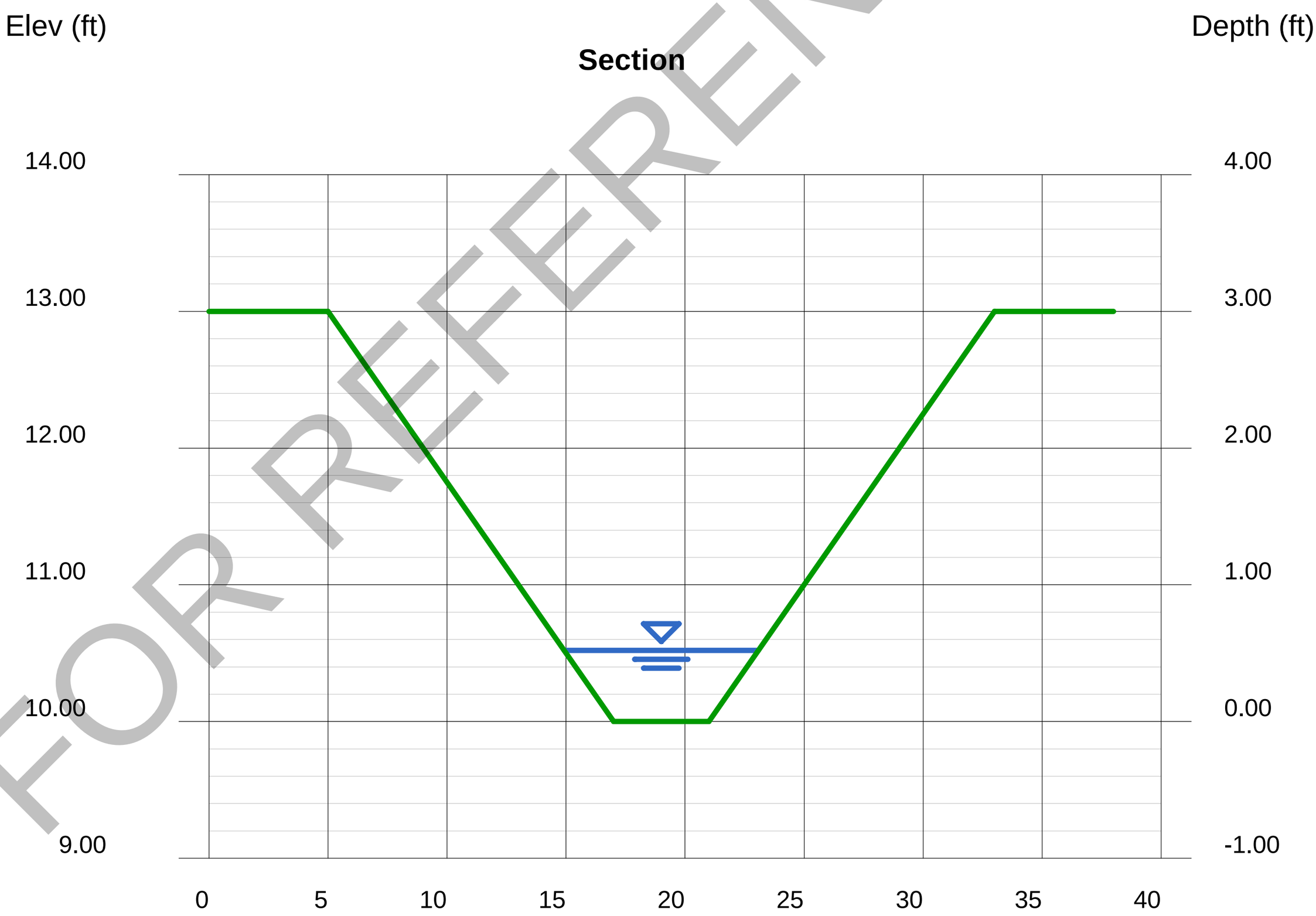
Bottom Width (ft)	= 4.00
Side Slopes (z:1)	= 4.00, 4.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 0.50
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 5.70

Highlighted

Depth (ft)	= 0.52
Q (cfs)	= 5.700
Area (sqft)	= 3.16
Velocity (ft/s)	= 1.80
Wetted Perim (ft)	= 8.29
Crit Depth, Yc (ft)	= 0.36
Top Width (ft)	= 8.16
EGL (ft)	= 0.57



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 1.3 Swale (100-Year)(FR:0.47)

Trapezoidal

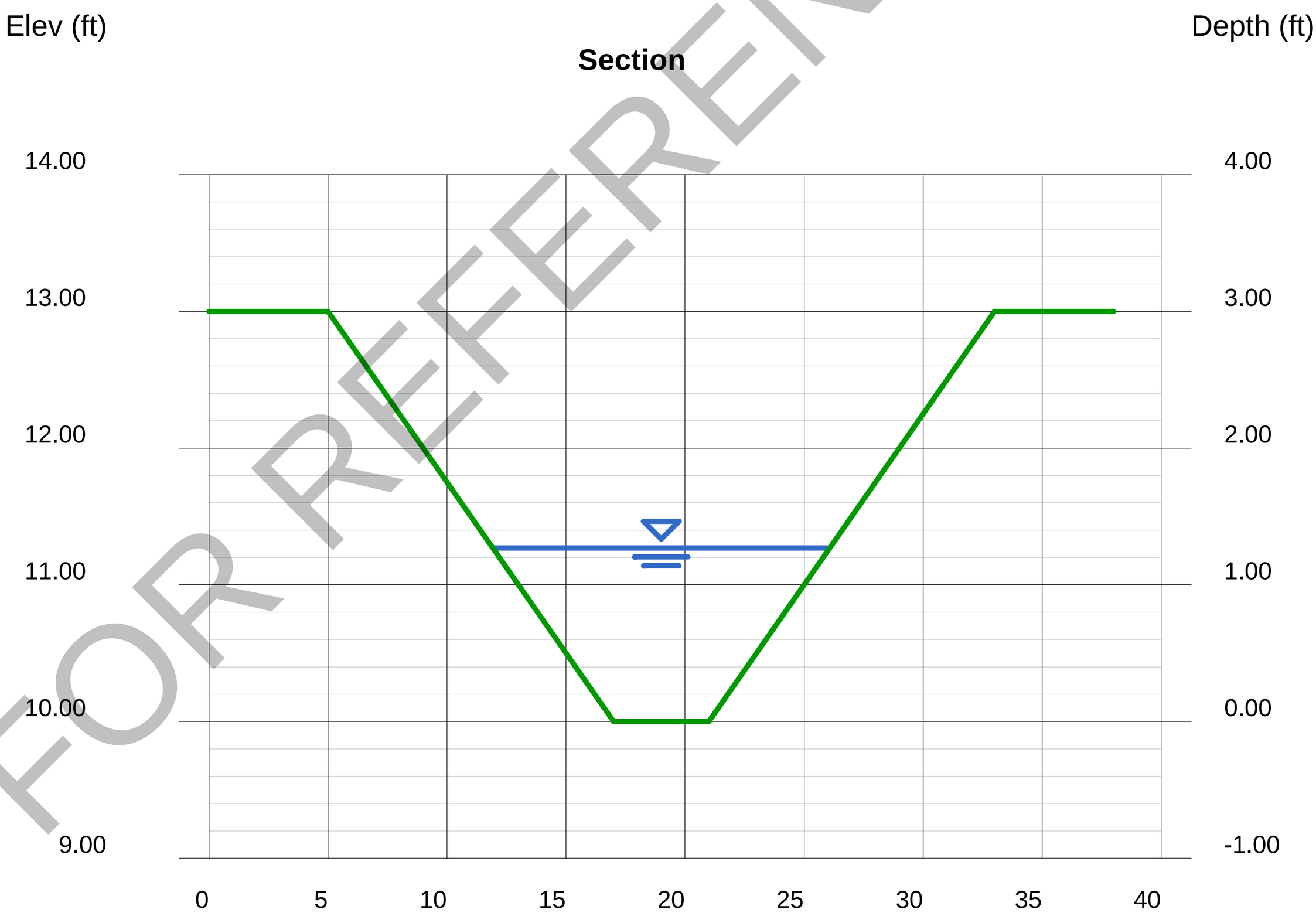
Bottom Width (ft)	= 4.00
Side Slopes (z:1)	= 4.00, 4.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 0.50
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 34.30

Highlighted

Depth (ft)	= 1.27
Q (cfs)	= 34.30
Area (sqft)	= 11.53
Velocity (ft/s)	= 2.97
Wetted Perim (ft)	= 14.47
Crit Depth, Yc (ft)	= 0.97
Top Width (ft)	= 14.16
EGL (ft)	= 1.41



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 5 Swale (5-Year)(FR:0.63)

Triangular

Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.80
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 2.10

Highlighted

Depth (ft)	= 0.49
Q (cfs)	= 2.100
Area (sqft)	= 0.84
Velocity (ft/s)	= 2.50
Wetted Perim (ft)	= 3.57
Crit Depth, Yc (ft)	= 0.47
Top Width (ft)	= 3.43
EGL (ft)	= 0.59

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 5 Swale (100-Year)(FR:0.68)

Triangular

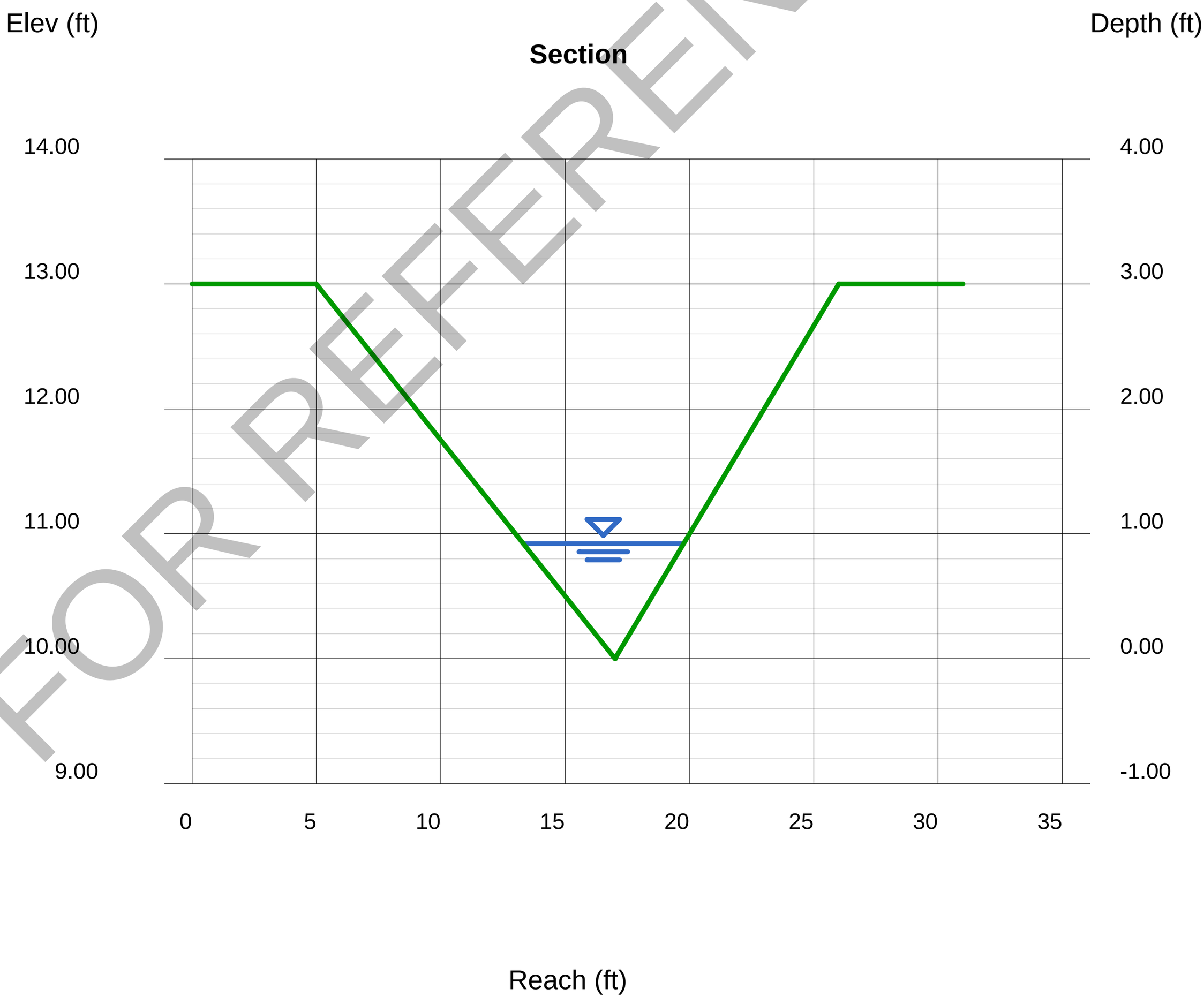
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 1.80
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 11.20

Highlighted

Depth (ft)	= 0.92
Q (cfs)	= 11.20
Area (sqft)	= 2.96
Velocity (ft/s)	= 3.78
Wetted Perim (ft)	= 6.70
Crit Depth, Yc (ft)	= 0.92
Top Width (ft)	= 6.44
EGL (ft)	= 1.14



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 11 Swale (5-Year)(FR:0.70)

Triangular

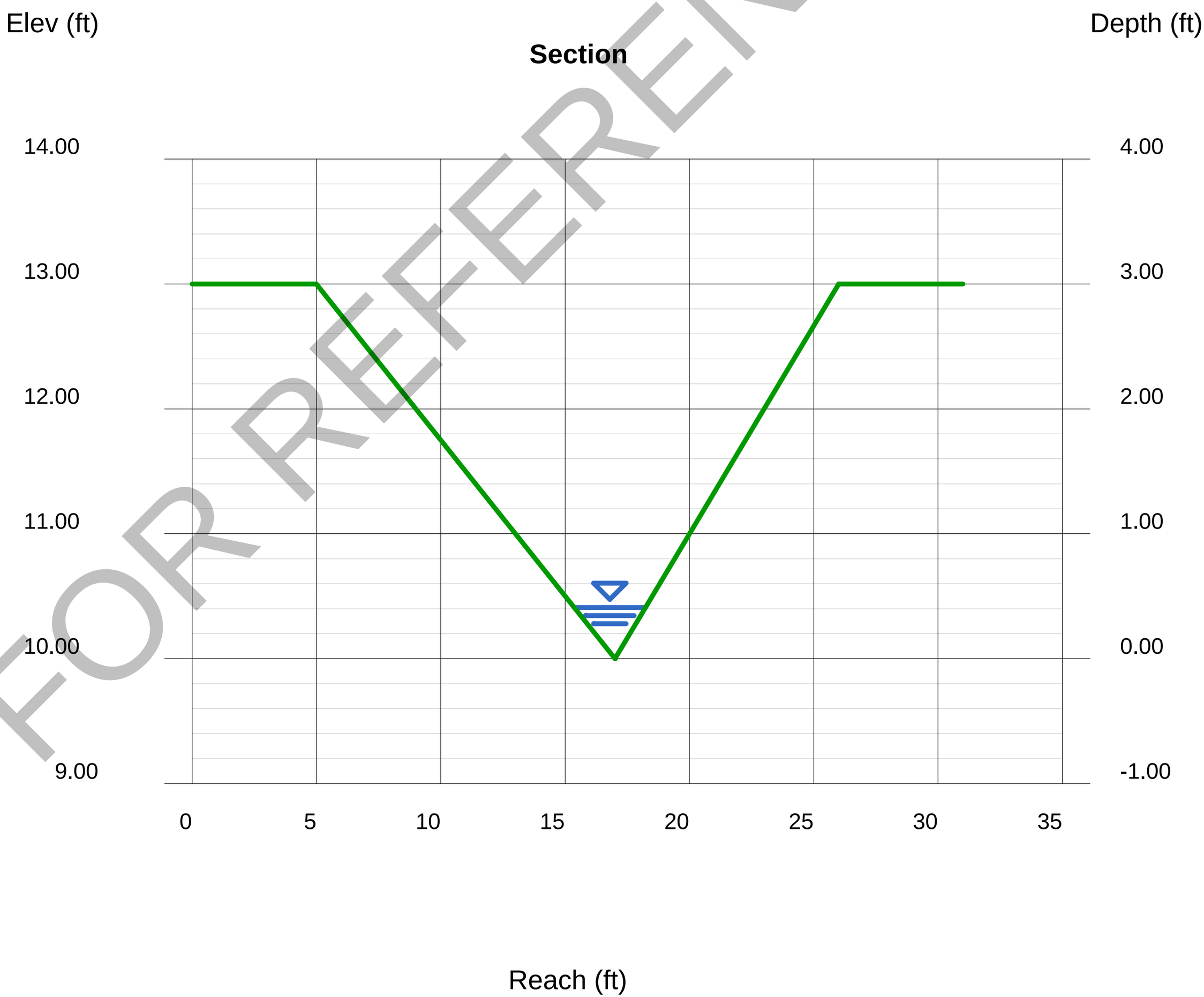
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 2.40
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 1.50

Highlighted

Depth (ft)	= 0.41
Q (cfs)	= 1.500
Area (sqft)	= 0.59
Velocity (ft/s)	= 2.55
Wetted Perim (ft)	= 2.99
Crit Depth, Yc (ft)	= 0.41
Top Width (ft)	= 2.87
EGL (ft)	= 0.51



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Aug 30 2022

DP 11 Swale (100-Year)(FR:0.79)

Triangular

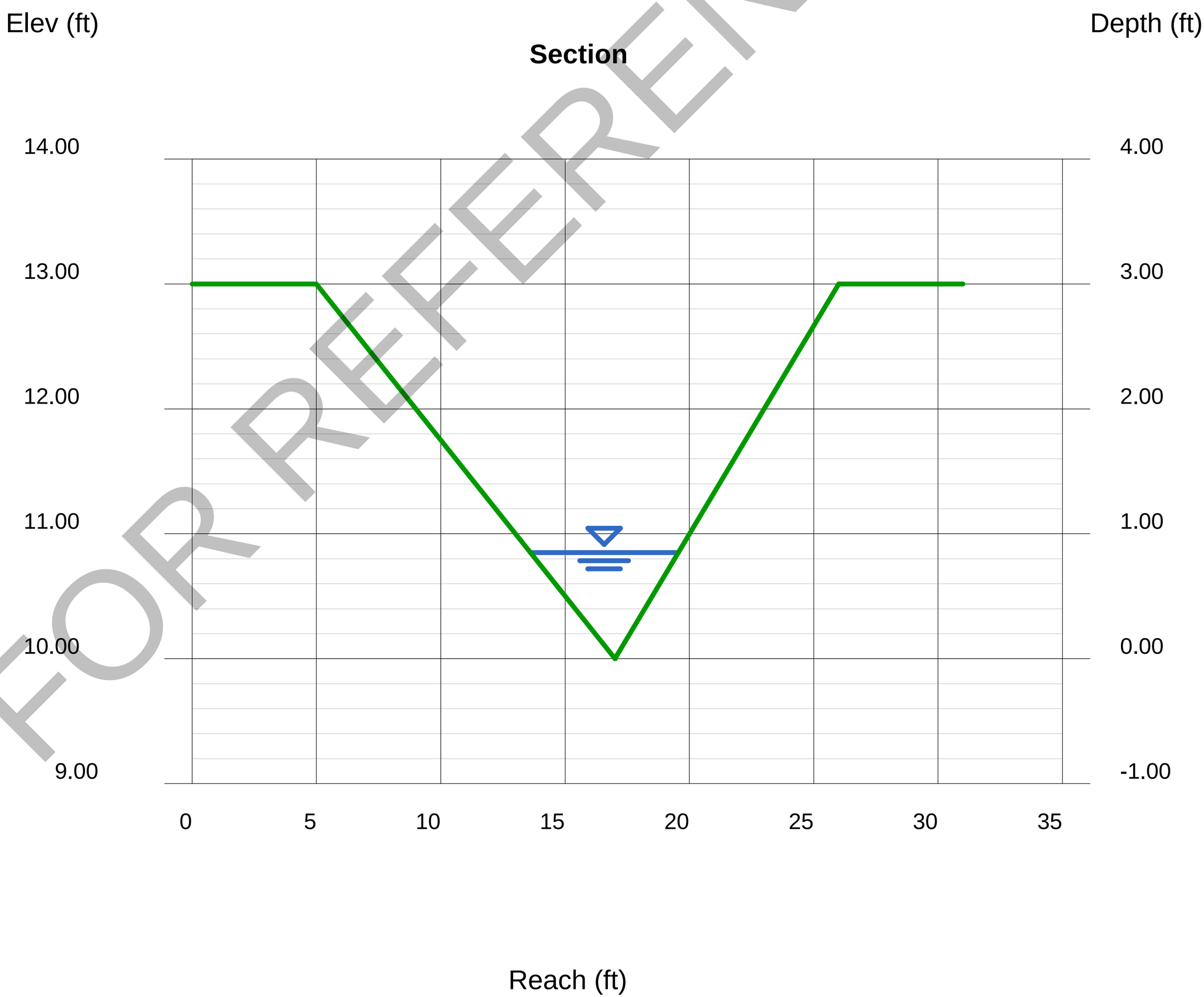
Side Slopes (z:1)	= 4.00, 3.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 10.00
Slope (%)	= 2.40
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 10.40

Highlighted

Depth (ft)	= 0.85
Q (cfs)	= 10.40
Area (sqft)	= 2.53
Velocity (ft/s)	= 4.11
Wetted Perim (ft)	= 6.19
Crit Depth, Yc (ft)	= 0.89
Top Width (ft)	= 5.95
EGL (ft)	= 1.11



APPENDIX D

WATER QUALITY AND DETENTION CALCULATIONS

Saddlehorn-2514207
Required Sediment Pond Volumes
8/29/2022

	Sediment Basin #1	(north)	
Total Area =	60.42	acres	
Developed Area =	20.98	acres	
Undeveloped Area =	39.44	acres	
Required Volume =	(Dev. Area * 1800 ft^3/ac) + (Undev. Area * 500 ft^3/ac)		
=	57,484	ft^3	
	1.320	AC-FT	
	0.660	1/2 VOLUME	
L=2xW	196	L	76645.33 ft3
	98	W	
	19,161	pond bottom min (3' depth assumed)	

	Sediment Basin #2	(South)	
Total Area =	15.08	acres	
Developed Area =	1.55	acres	
Undeveloped Area =	13.53	acres	
Required Volume =	(Dev. Area * 1800 ft^3/ac) + (Undev. Area * 500 ft^3/ac)		
=	9,555	ft^3	
	0.219	AC-FT	
	0.110	1/2 VOLUME	
L=2xW	170	L	57800 ft3
	85	W	
	3,185	pond bottom min (3' depth assumed)	

Provide summary table for each sediment basin showing contributing basin, total area, developed and undeveloped areas.

Saddlehorn (25142.07)
Orifice Sizing

Sediment Basin #1			
Basin Total Volume:	1.320	ac-ft	
Top 1/2	0.660	ac-ft	
	28750	cf	
Drain Time 40 hrs	0.1997	cfs	over 40 hrs
	Assuming	5	holes
	0.0399	cfs	per hole
Equates to a	1.25	diam. hole (in)	
Equates to a	1.23	sq. in. hole	

Solution	5	1 Column - 5 holes
	1.25	Inch diameter holes

Saddlehorn (25142.07)
Orifice Sizing

Sediment Basin #2

Basin Total Volume:	0.219	ac-ft	
Top 1/2	0.110	ac-ft	
	4770	cf	
Drain Time 40 hrs	0.0331	cfs	over 40 hrs
	Assuming	5	holes
	0.0066	cfs	per hole
Equates to a	1.25	diam. hole (in)	
Equates to a	1.23	sq. in. hole	

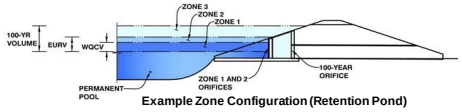
Solution	5	1 Column - 5 holes
	1.25	Inch diameter holes

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Saddlehorn Filing 5

Basin ID: Pond A



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	15.08	acres
Watershed Length =	1,659	ft
Watershed Length to Centroid =	794	ft
Watershed Slope =	0.048	ft/ft
Watershed Imperviousness =	10.10%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

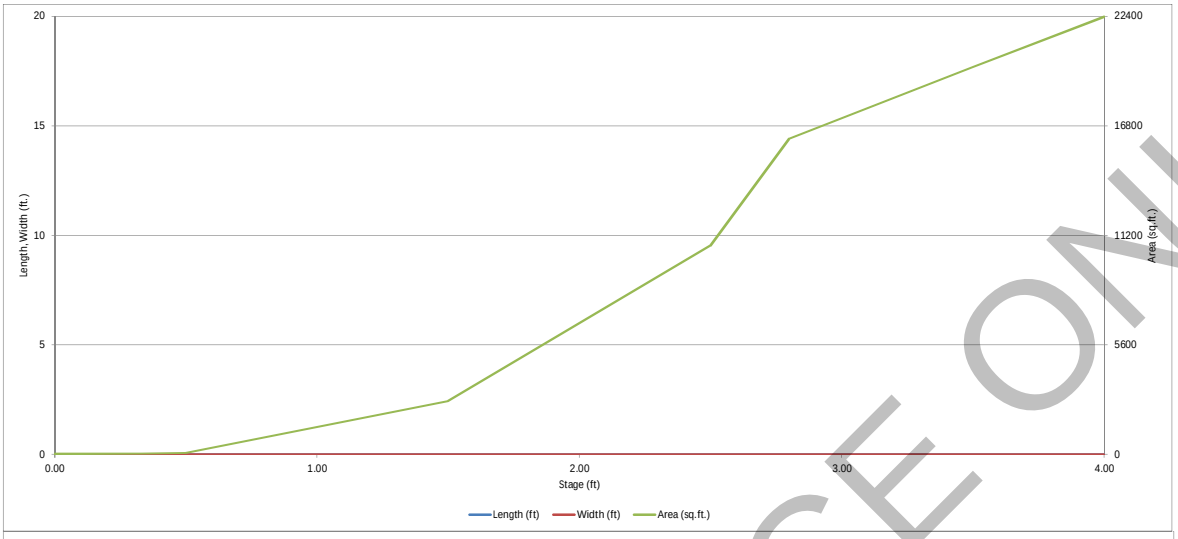
			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.085	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.112	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.055	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.094	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	0.126	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	0.330	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	0.534	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	0.822	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	1.456	acre-feet		inches
Approximate 2-yr Detention Volume =	0.067	acre-feet		
Approximate 5-yr Detention Volume =	0.092	acre-feet		
Approximate 10-yr Detention Volume =	0.121	acre-feet		
Approximate 25-yr Detention Volume =	0.164	acre-feet		
Approximate 50-yr Detention Volume =	0.221	acre-feet		
Approximate 100-yr Detention Volume =	0.358	acre-feet		

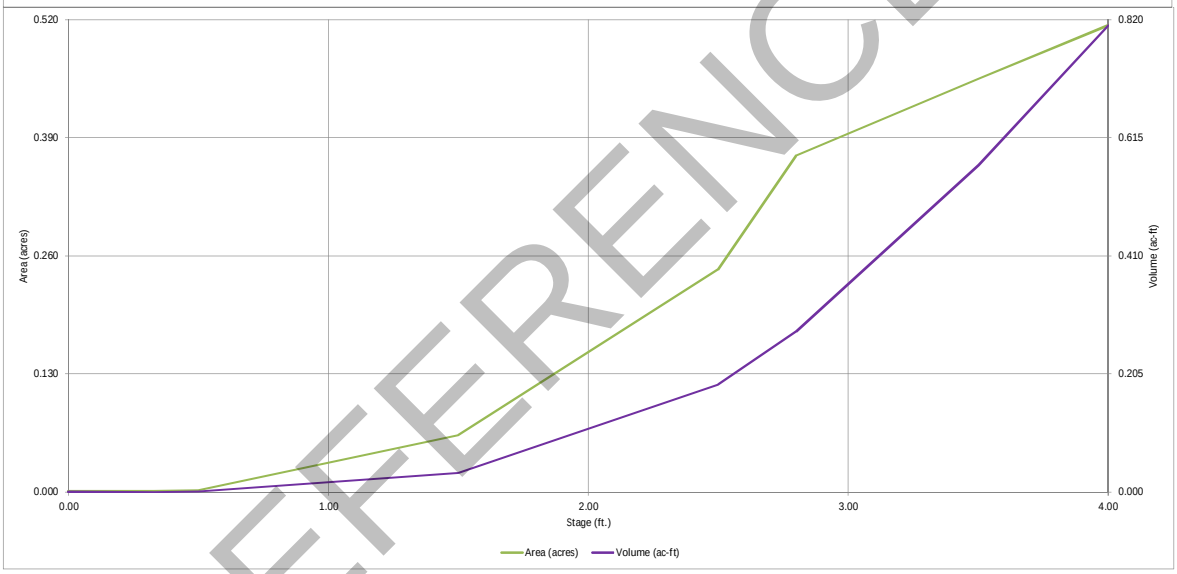
Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.085	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.027	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.245	acre-feet
Total Detention Basin Volume =	0.358	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio (R_{LW}) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	36	0.001		
6661.83	--	0.33	--	--	--	50	0.001	14	0.000
6662	--	0.50	--	--	--	75	0.002	25	0.001
6663	--	1.50	--	--	--	2,724	0.063	1,424	0.033
6664	--	2.50	--	--	--	10,676	0.245	8,124	0.187
6664.3	--	2.80	--	--	--	16,135	0.370	12,146	0.279
6665	--	3.50	--	--	--	19,821	0.455	24,730	0.568
6665.5	--	4.00	--	--	--	22,375	0.514	35,279	0.810



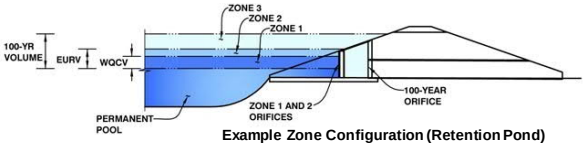


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: Saddlehorn Filing 5

Basin ID: Pond A



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.99	0.085	Orifice Plate
Zone 2 (EURV)	2.16	0.027	Circular Orifice
Zone 3 (100-year)	3.01	0.245	Weir&Pipe (Restrict)
Total (all zones)		0.358	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	2.257E-03	ft²
Depth at top of Zone using Orifice Plate =	1.99	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	8.00	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	0.33	sq. inches (diameter = 5/8 inch)	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.66	1.33					
Orifice Area (sq. inches)	0.33	0.33	0.33					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected			Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	1.99	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.00	N/A	ft²
Depth at top of Zone using Vertical Orifice =	2.16	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.02	N/A	feet
Vertical Orifice Diameter =	0.38	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.17	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	2.17	N/A	feet
Overflow Weir Front Edge Length =	3.00	N/A	feet	Overflow Weir Slope Length =	4.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	8.28	N/A	
Horiz. Length of Weir Sides =	4.00	N/A	feet	Overflow Grate Open Area w/o Debris =	8.35	N/A	ft²
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	8.35	N/A	ft²
Debris Clogging % =	0%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.33	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	1.01	N/A	ft²
Outlet Pipe Diameter =	18.00	N/A	inches	Outlet Orifice Centroid =	0.48	N/A	feet
Restrictor Plate Height Above Pipe Invert =	10.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.68	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	2.80	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.20	feet
Spillway Crest Length =	35.00	feet	Stage at Top of Freeboard =	4.00	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	0.51	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	0.81	acre-ft

Calculated Parameters for Spillway

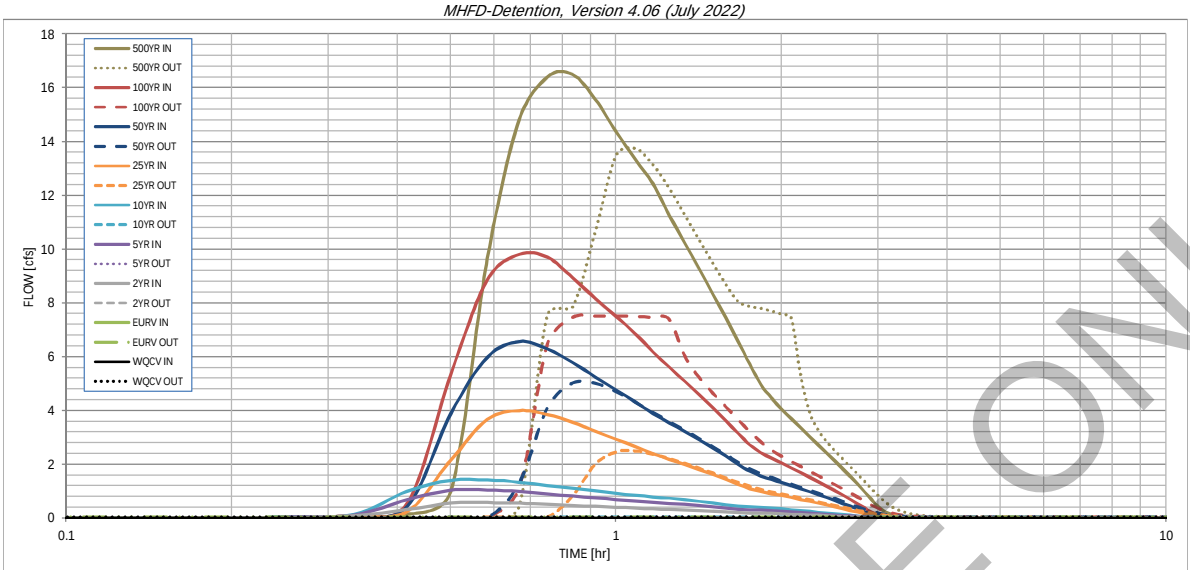
Routed Hydrograph Results

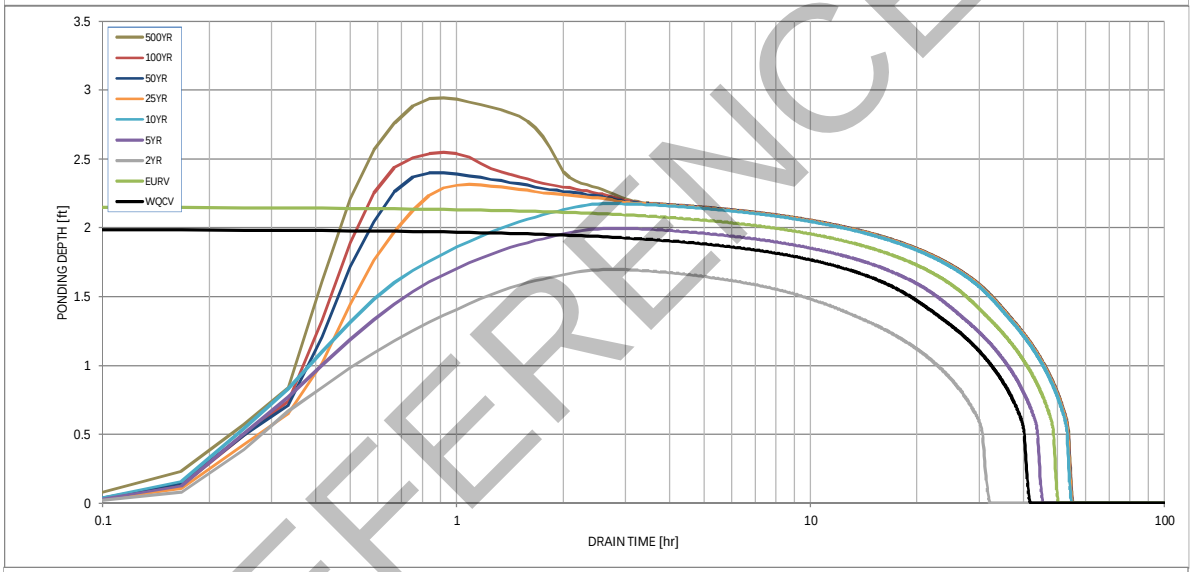
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

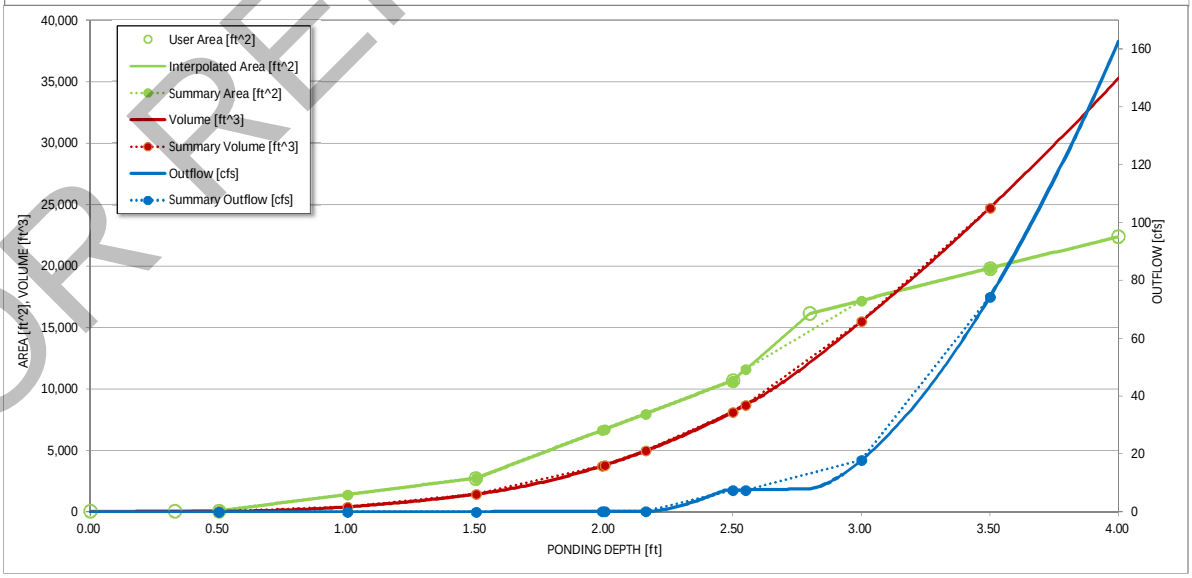
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	0.085	0.112	0.055	0.094	0.126	0.330	0.534	0.822	1.456
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.055	0.094	0.126	0.330	0.534	0.822	1.456
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.1	0.2	0.3	2.5	5.0	8.1	14.7
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.01	0.02	0.16	0.33	0.54	0.98
Peak Inflow Q (cfs) =	N/A	N/A	0.6	1.0	1.4	4.0	6.6	9.8	16.5
Peak Outflow Q (cfs) =	0.0	0.0	0.0	0.0	0.1	2.5	5.0	7.5	13.7
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.2	0.3	1.0	1.0	0.9	0.9
Structure Controlling Flow =	Plate	Vertical Orifice 1	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.0	0.3	0.6	0.9	1.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	45	29	41	50	45	41	36	28
Time to Drain 99% of Inflow Volume (hours) =	40	48	31	43	52	50	49	46	42
Maximum Ponding Depth (ft) =	1.99	2.16	1.70	2.00	2.18	2.31	2.40	2.55	2.94
Area at Maximum Ponding Depth (acres) =	0.15	0.18	0.10	0.15	0.18	0.21	0.23	0.26	0.39
Maximum Volume Stored (acre-ft) =	0.085	0.114	0.048	0.085	0.116	0.143	0.163	0.197	0.332

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	0:15:00	0.00	0.00	0.02	0.04	0.05	0.03	0.04	0.04	0.06
	0:20:00	0.00	0.00	0.08	0.11	0.13	0.08	0.10	0.10	0.14
	0:25:00	0.00	0.00	0.32	0.69	1.00	0.25	0.43	0.55	0.99
	0:30:00	0.00	0.00	0.56	1.04	1.41	2.14	3.88	5.33	9.66
	0:35:00	0.00	0.00	0.58	1.05	1.42	3.67	5.98	8.87	14.81
	0:40:00	0.00	0.00	0.56	0.99	1.34	4.00	6.57	9.81	16.41
	0:45:00	0.00	0.00	0.51	0.90	1.21	3.85	6.29	9.70	16.50
	0:50:00	0.00	0.00	0.47	0.82	1.10	3.57	5.79	8.95	15.57
	0:55:00	0.00	0.00	0.44	0.76	1.01	3.23	5.24	8.17	14.37
	1:00:00	0.00	0.00	0.40	0.69	0.93	2.94	4.77	7.50	13.34
	1:05:00	0.00	0.00	0.38	0.64	0.85	2.66	4.33	6.87	12.44
	1:10:00	0.00	0.00	0.35	0.60	0.79	2.40	3.90	6.19	11.26
	1:15:00	0.00	0.00	0.33	0.55	0.75	2.19	3.55	5.61	10.24
	1:20:00	0.00	0.00	0.30	0.51	0.69	2.00	3.23	5.09	9.27
	1:25:00	0.00	0.00	0.28	0.47	0.63	1.81	2.92	4.59	8.34
	1:30:00	0.00	0.00	0.25	0.42	0.56	1.63	2.61	4.10	7.45
	1:35:00	0.00	0.00	0.23	0.38	0.50	1.44	2.31	3.63	6.58
	1:40:00	0.00	0.00	0.21	0.34	0.45	1.26	2.01	3.16	5.73
	1:45:00	0.00	0.00	0.20	0.31	0.42	1.10	1.74	2.74	4.98
	1:50:00	0.00	0.00	0.19	0.30	0.39	0.99	1.57	2.44	4.46
	1:55:00	0.00	0.00	0.18	0.28	0.37	0.91	1.44	2.23	4.06
	2:00:00	0.00	0.00	0.16	0.26	0.34	0.84	1.33	2.05	3.70
	2:05:00	0.00	0.00	0.15	0.24	0.31	0.77	1.21	1.87	3.36
	2:10:00	0.00	0.00	0.13	0.21	0.28	0.70	1.10	1.69	3.04
	2:15:00	0.00	0.00	0.12	0.19	0.25	0.63	0.99	1.52	2.72
	2:20:00	0.00	0.00	0.11	0.16	0.22	0.56	0.88	1.35	2.42
	2:25:00	0.00	0.00	0.09	0.14	0.19	0.49	0.77	1.19	2.13
	2:30:00	0.00	0.00	0.08	0.12	0.16	0.42	0.66	1.02	1.84
	2:35:00	0.00	0.00	0.07	0.10	0.13	0.35	0.55	0.86	1.55
	2:40:00	0.00	0.00	0.05	0.08	0.11	0.29	0.45	0.70	1.26
	2:45:00	0.00	0.00	0.04	0.06	0.08	0.22	0.34	0.53	0.98
	2:50:00	0.00	0.00	0.03	0.04	0.05	0.16	0.24	0.37	0.69
	2:55:00	0.00	0.00	0.02	0.03	0.04	0.10	0.14	0.22	0.42
	3:00:00	0.00	0.00	0.02	0.02	0.03	0.05	0.07	0.12	0.24
	3:05:00	0.00	0.00	0.02	0.02	0.03	0.03	0.04	0.07	0.15
	3:10:00	0.00	0.00	0.01	0.02	0.02	0.02	0.03	0.04	0.10
	3:15:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.03	0.07
	3:20:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.04
	3:25:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.02	0.03
	3:30:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.02
	3:35:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01
	3:40:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	3:45:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft²]	Area [acres]	Volume [ft³]	Volume [ac-ft]	Total Outflow [cfs]
	0.50	75	0.002	25	0.001	0.01
	1.00	1,400	0.032	393	0.009	0.02
	1.50	2,724	0.063	1,424	0.033	0.03
WQCV	1.99	6,620	0.152	3,714	0.085	0.04
	2.00	6,700	0.154	3,780	0.087	0.04
EURV	2.16	7,972	0.183	4,954	0.114	0.04
	2.50	10,676	0.245	8,124	0.187	7.45
100-YR	2.55	11,586	0.266	8,681	0.199	7.53
	3.00	17,188	0.395	15,478	0.355	17.76
	3.50	19,821	0.455	24,730	0.568	74.32

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

POND A FOREBAY VOLUME REQUIREMENTS

Equation 3-1 $WQCV = a(0.91I^3 - 1.19I^2 + 0.781I)$

a=1 (40 hour drain time)

Proposed Forebay I=.101 $WQCV$= 0.067578

Equation 3-3 $V=(WQCV/12)A$

Proposed Forebay A= 15.08 Acres V= 0.085

3% OF WQCV

Forebay Total Volume= .03(V)

Volume Required For Proposed Forebay= 0.003 AC-FT 111 CF

Volume Provided For Proposed Forebay= 0.008 AC-FT 365 CF

Q_{100} Discharges 2% OF Q_{100}

Q_{100} Proposed Forebay = .02*7.5 CFS= .15 CFS

Weir Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Mar 25 2022

Pond A Spillway

Trapezoidal Weir

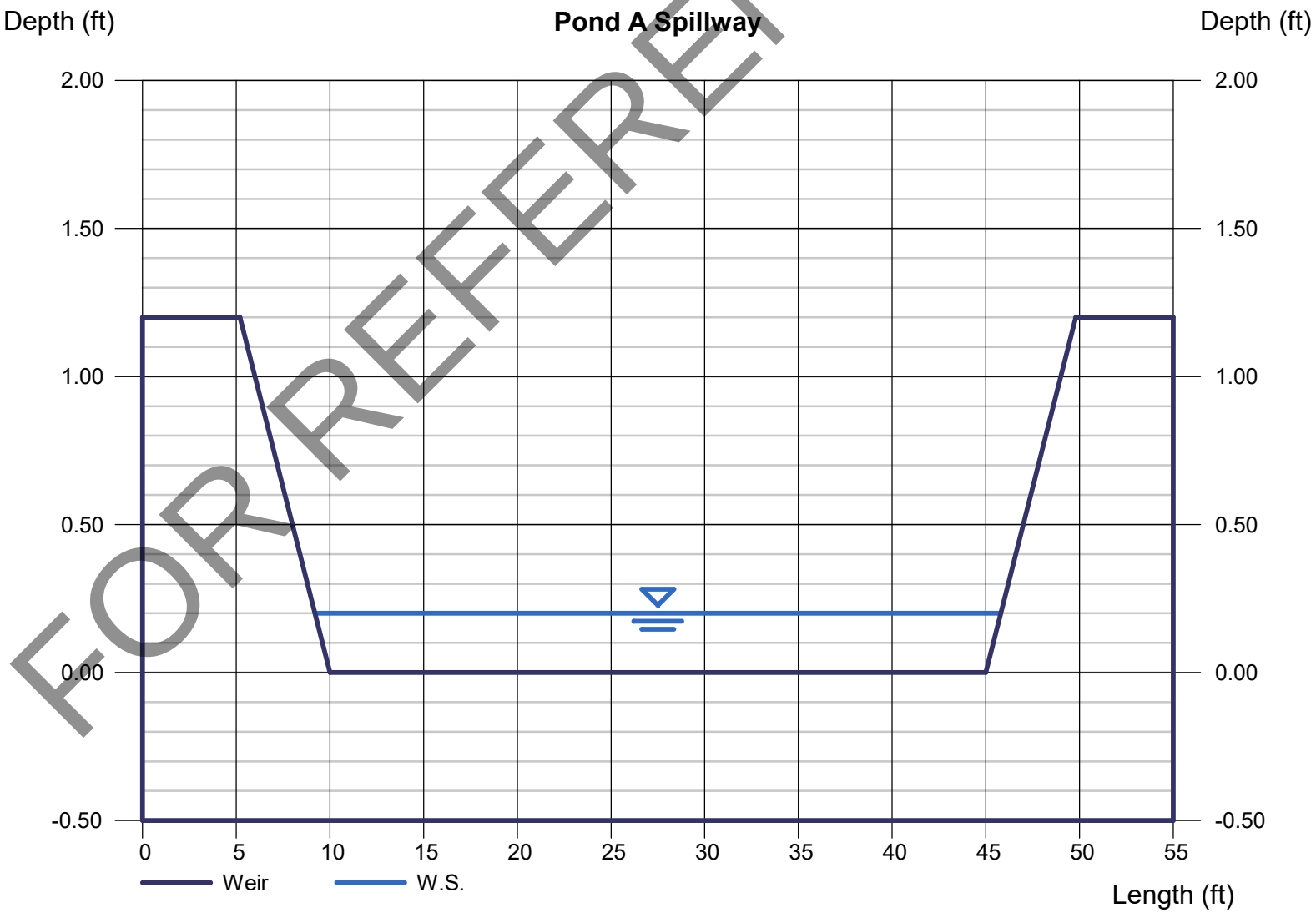
Crest	= Sharp
Bottom Length (ft)	= 35.00
Total Depth (ft)	= 1.20
Side Slope (z:1)	= 4.00

Calculations

Weir Coeff. Cw	= 3.10
Compute by:	Known Q
Known Q (cfs)	= 9.30

Highlighted

Depth (ft)	= 0.20
Q (cfs)	= 9.300
Area (sqft)	= 7.16
Velocity (ft/s)	= 1.30
Top Width (ft)	= 36.60



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Thursday, Sep 1 2022

Pond A Trickle Channel

Rectangular

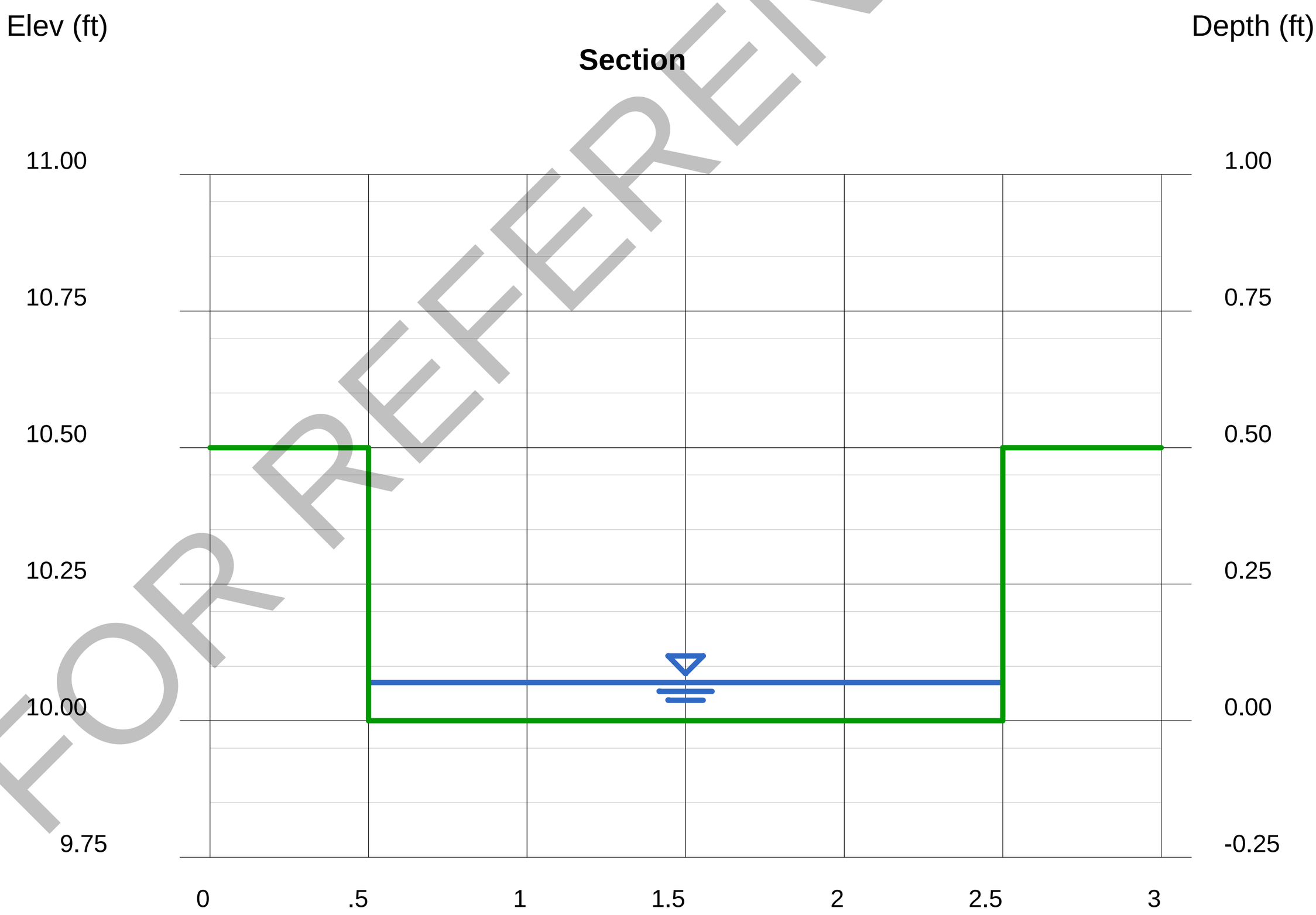
Bottom Width (ft)	= 2.00
Total Depth (ft)	= 0.50
Invert Elev (ft)	= 10.00
Slope (%)	= 0.50
N-Value	= 0.013

Calculations

Compute by:	Known Q
Known Q (cfs)	= 0.15

Highlighted

Depth (ft)	= 0.07
Q (cfs)	= 0.150
Area (sqft)	= 0.14
Velocity (ft/s)	= 1.07
Wetted Perim (ft)	= 2.14
Crit Depth, Yc (ft)	= 0.06
Top Width (ft)	= 2.00
EGL (ft)	= 0.09

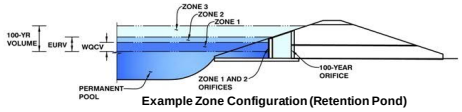


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Saddehorn Filing 5

Basin ID: Pond B



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	60.42	acres
Watershed Length =	3,478	ft
Watershed Length to Centroid =	1,805	ft
Watershed Slope =	0.023	ft/ft
Watershed Imperviousness =	11.50%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

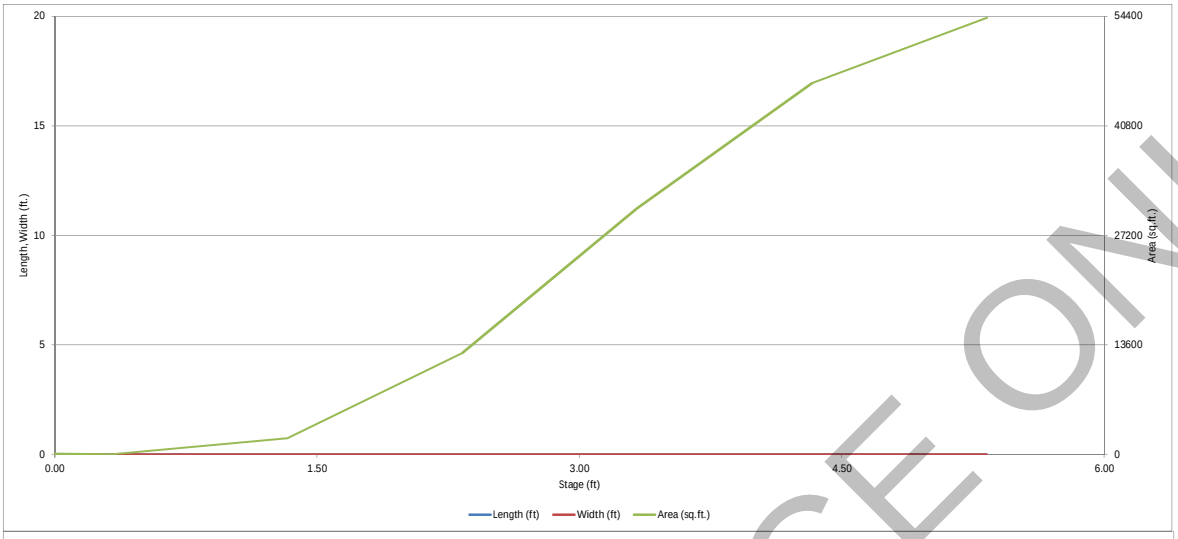
			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.379	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.531	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.279	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.454	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	0.602	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	1.442	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	2.273	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	3.438	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	5.999	acre-feet		inches
Approximate 2-yr Detention Volume =	0.320	acre-feet		
Approximate 5-yr Detention Volume =	0.438	acre-feet		
Approximate 10-yr Detention Volume =	0.572	acre-feet		
Approximate 25-yr Detention Volume =	0.768	acre-feet		
Approximate 50-yr Detention Volume =	1.006	acre-feet		
Approximate 100-yr Detention Volume =	1.563	acre-feet		

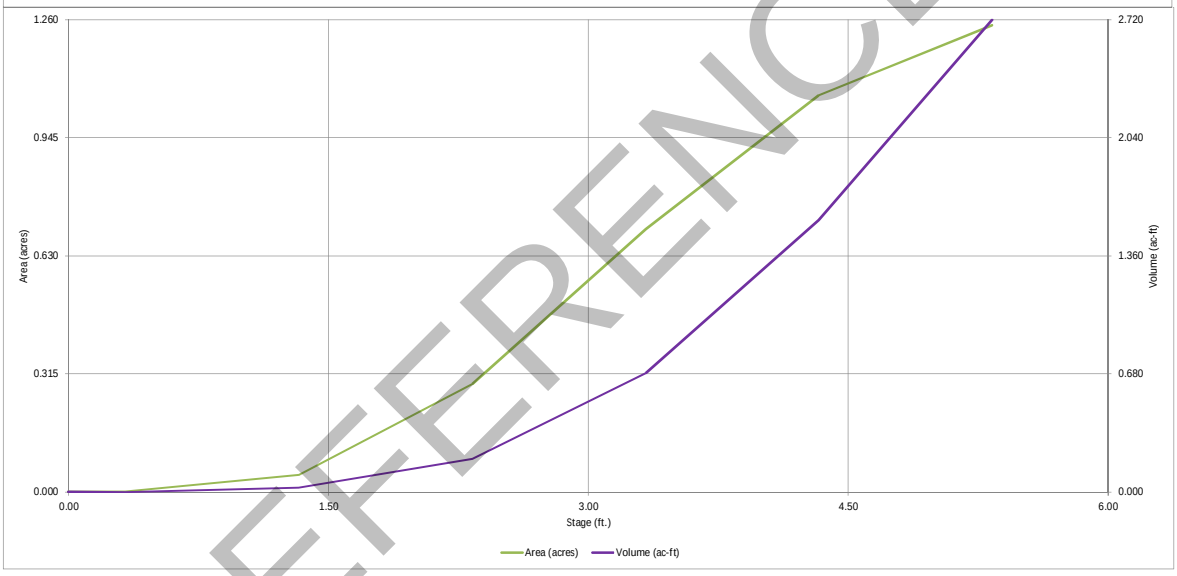
Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.379	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.152	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.033	acre-feet
Total Detention Basin Volume =	1.563	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio (R_{LW}) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	36	0.001		
6675	--	0.33	--	--	--	50	0.001	14	0.000
6676	--	1.33	--	--	--	1,986	0.046	1,032	0.024
6677	--	2.33	--	--	--	12,572	0.289	8,311	0.191
6678	--	3.33	--	--	--	30,573	0.702	29,884	0.686
6679	--	4.33	--	--	--	46,107	1.058	68,223	1.566
6680	--	5.33	--	--	--	54,237	1.245	118,395	2.718



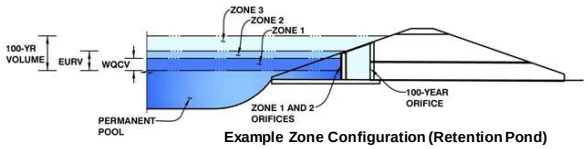


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: Saddehorn Filing 5

Basin ID: Pond B



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.82	0.379	Orifice Plate
Zone 2 (EURV)	3.10	0.152	Circular Orifice
Zone 3 (100-year)	4.33	1.033	Weir&Pipe (Restrict)
Total (all zones)		1.563	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

			Calculated Parameters for Underdrain		
Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

			Calculated Parameters for Plate		
Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	8.264E-03	ft^2
Depth at top of Zone using Orifice Plate =	2.82	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	11.30	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	1.19	sq. inches (diameter = 1-3/16 inches)	Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.94	1.88					
Orifice Area (sq. inches)	1.19	1.19	1.19					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected		Calculated Parameters for Vertical Orifice	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	2.82	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.00	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	3.10	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.02	N/A	feet
Vertical Orifice Diameter =	0.38	N/A	inches				

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected		Calculated Parameters for Overflow Weir	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.11	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	4.36	N/A	feet
Overflow Weir Front Edge Length =	15.00	N/A	feet	Overflow Weir Slope Length =	5.15	N/A	feet
Overflow Weir Grate Slope =	4.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	17.13	N/A	
Horiz. Length of Weir Sides =	5.00	N/A	feet	Overflow Grate Open Area w/o Debris =	53.81	N/A	ft^2
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	53.81	N/A	ft^2
Debris Clogging % =	0%	N/A	%				

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected		Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	3.14	N/A	ft^2
Outlet Pipe Diameter =	24.00	N/A	inches	Outlet Orifice Centroid =	1.00	N/A	feet
Restrictor Plate Height Above Pipe Invert =	24.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

			Calculated Parameters for Spillway	
Spillway Invert Stage=	4.08	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.24 feet
Spillway Crest Length =	85.00	feet	Stage at Top of Freeboard =	8.32 feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	1.25 acres
Freeboard above Max Water Surface =	4.00	feet	Basin Volume at Top of Freeboard =	2.72 acre-ft

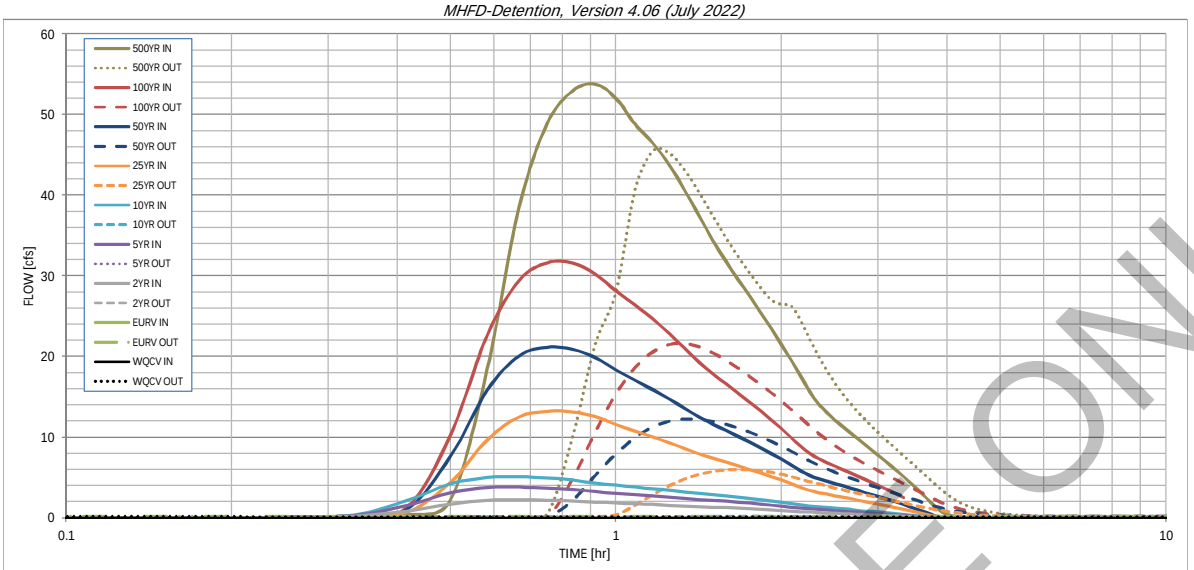
Routed Hydrograph Results

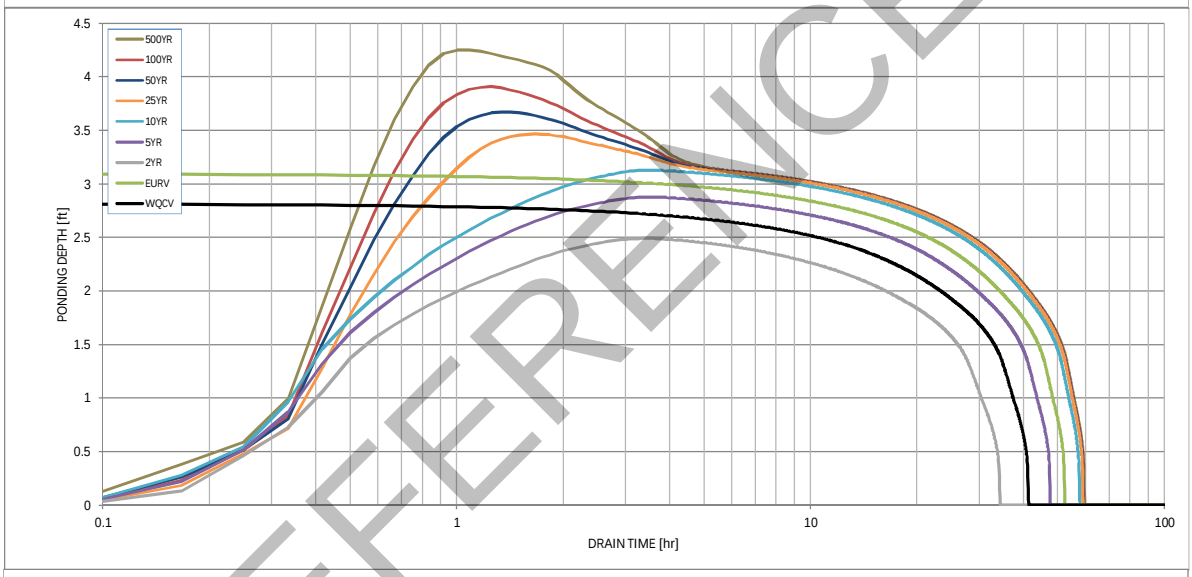
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

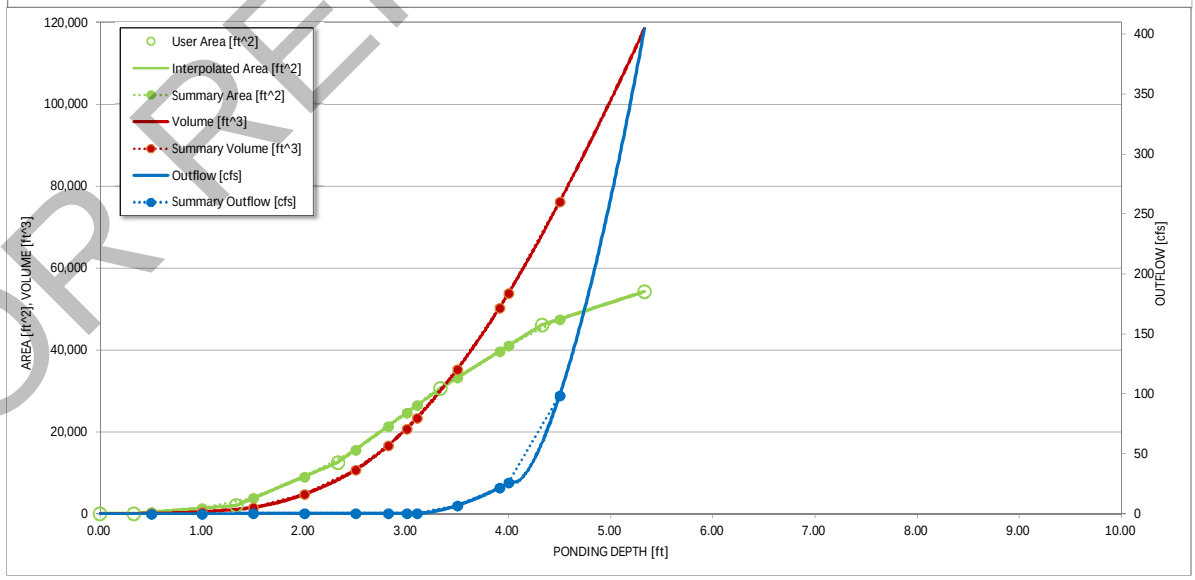
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	0.379	0.531	0.279	0.454	0.602	1.442	2.273	3.438	5.999
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.279	0.454	0.602	1.442	2.273	3.438	5.999
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.3	0.6	0.8	7.5	15.1	25.1	46.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.12	0.25	0.42	0.77
Peak Inflow Q (cfs) =	N/A	N/A	2.3	3.8	5.1	13.2	21.2	31.6	53.8
Peak Outflow Q (cfs) =	0.2	0.2	0.1	0.2	0.2	6.0	12.3	21.6	45.5
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.3	0.3	0.8	0.8	0.9	1.0
Structure Controlling Flow =	Plate	Vertical Orifice 1	Plate	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.0	0.1	0.2	0.4	0.5
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	37	47	31	43	52	50	46	42	35
Time to Drain 99% of Inflow Volume (hours) =	40	50	33	46	55	54	53	51	48
Maximum Ponding Depth (ft) =	2.82	3.10	2.49	2.88	3.13	3.46	3.67	3.91	4.25
Area at Maximum Ponding Depth (acres) =	0.49	0.61	0.35	0.51	0.62	0.75	0.82	0.91	1.03
Maximum Volume Stored (acre-ft) =	0.382	0.536	0.242	0.407	0.548	0.780	0.945	1.144	1.483

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.02
	0:15:00	0.00	0.00	0.05	0.08	0.10	0.07	0.09	0.08	0.14
	0:20:00	0.00	0.00	0.22	0.30	0.36	0.23	0.28	0.29	0.40
	0:25:00	0.00	0.00	0.85	1.57	2.17	0.72	1.10	1.32	2.23
	0:30:00	0.00	0.00	1.73	3.13	4.24	4.42	7.67	10.34	18.52
	0:35:00	0.00	0.00	2.18	3.76	5.02	9.68	15.88	22.71	38.59
	0:40:00	0.00	0.00	2.26	3.82	5.10	12.56	20.11	29.50	48.94
	0:45:00	0.00	0.00	2.22	3.73	4.97	13.23	21.17	31.65	52.96
	0:50:00	0.00	0.00	2.12	3.54	4.70	13.14	20.90	31.61	53.78
	0:55:00	0.00	0.00	1.99	3.28	4.33	12.56	19.89	30.26	52.00
	1:00:00	0.00	0.00	1.87	3.07	4.06	11.59	18.33	28.16	48.82
	1:05:00	0.00	0.00	1.78	2.91	3.83	10.76	17.06	26.39	46.45
	1:10:00	0.00	0.00	1.68	2.75	3.62	10.05	15.90	24.68	43.79
	1:15:00	0.00	0.00	1.58	2.58	3.43	9.33	14.75	22.85	40.63
	1:20:00	0.00	0.00	1.49	2.42	3.24	8.61	13.59	21.01	37.41
	1:25:00	0.00	0.00	1.41	2.29	3.07	7.95	12.51	19.30	34.39
	1:30:00	0.00	0.00	1.35	2.19	2.91	7.40	11.64	17.88	31.84
	1:35:00	0.00	0.00	1.29	2.08	2.75	6.92	10.86	16.65	29.57
	1:40:00	0.00	0.00	1.23	1.96	2.59	6.47	10.13	15.50	27.47
	1:45:00	0.00	0.00	1.16	1.84	2.43	6.02	9.41	14.37	25.44
	1:50:00	0.00	0.00	1.10	1.72	2.27	5.58	8.70	13.26	23.44
	1:55:00	0.00	0.00	1.03	1.59	2.11	5.14	7.99	12.16	21.48
	2:00:00	0.00	0.00	0.95	1.47	1.94	4.70	7.28	11.07	19.54
	2:05:00	0.00	0.00	0.87	1.33	1.76	4.25	6.56	9.97	17.60
	2:10:00	0.00	0.00	0.79	1.21	1.60	3.79	5.84	8.87	15.68
	2:15:00	0.00	0.00	0.73	1.12	1.49	3.42	5.27	8.00	14.19
	2:20:00	0.00	0.00	0.68	1.05	1.39	3.15	4.87	7.38	13.08
	2:25:00	0.00	0.00	0.63	0.98	1.30	2.95	4.55	6.88	12.16
	2:30:00	0.00	0.00	0.59	0.91	1.21	2.75	4.26	6.43	11.34
	2:35:00	0.00	0.00	0.54	0.84	1.12	2.57	3.98	6.01	10.57
	2:40:00	0.00	0.00	0.50	0.78	1.03	2.40	3.71	5.60	9.83
	2:45:00	0.00	0.00	0.46	0.72	0.95	2.23	3.44	5.19	9.12
	2:50:00	0.00	0.00	0.43	0.66	0.87	2.06	3.18	4.81	8.44
	2:55:00	0.00	0.00	0.39	0.60	0.80	1.89	2.92	4.42	7.77
	3:00:00	0.00	0.00	0.36	0.55	0.72	1.73	2.67	4.04	7.11
	3:05:00	0.00	0.00	0.32	0.49	0.65	1.57	2.42	3.66	6.44
	3:10:00	0.00	0.00	0.29	0.44	0.58	1.40	2.16	3.28	5.78
	3:15:00	0.00	0.00	0.25	0.39	0.51	1.24	1.91	2.91	5.12
	3:20:00	0.00	0.00	0.22	0.34	0.44	1.08	1.66	2.53	4.47
	3:25:00	0.00	0.00	0.19	0.28	0.37	0.93	1.41	2.15	3.81
	3:30:00	0.00	0.00	0.16	0.23	0.30	0.77	1.16	1.78	3.15
	3:35:00	0.00	0.00	0.13	0.18	0.24	0.61	0.92	1.40	2.50
	3:40:00	0.00	0.00	0.10	0.14	0.17	0.45	0.67	1.03	1.85
	3:45:00	0.00	0.00	0.07	0.09	0.12	0.30	0.43	0.67	1.21
	3:50:00	0.00	0.00	0.05	0.07	0.09	0.17	0.23	0.37	0.71
	3:55:00	0.00	0.00	0.05	0.06	0.08	0.10	0.14	0.21	0.44
	4:00:00	0.00	0.00	0.04	0.05	0.07	0.08	0.10	0.14	0.29
	4:05:00	0.00	0.00	0.04	0.05	0.06	0.06	0.08	0.10	0.19
	4:10:00	0.00	0.00	0.03	0.04	0.05	0.05	0.06	0.07	0.13
	4:15:00	0.00	0.00	0.03	0.03	0.04	0.04	0.05	0.05	0.09
	4:20:00	0.00	0.00	0.02	0.03	0.04	0.03	0.04	0.04	0.05
	4:25:00	0.00	0.00	0.02	0.02	0.03	0.03	0.03	0.03	0.04
	4:30:00	0.00	0.00	0.01	0.02	0.02	0.02	0.02	0.02	0.03
	4:35:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.02
	4:40:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.02
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	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft²]	Area [acres]	Volume [ft³]	Volume [ac-ft]	Total Outflow [cfs]
	0.50	379	0.009	51	0.001	0.03
	1.00	1,347	0.031	482	0.011	0.05
	1.50	3,786	0.087	1,523	0.035	0.08
	2.00	9,079	0.208	4,739	0.109	0.11
	2.50	15,632	0.359	10,708	0.246	0.14
WQCV	2.82	21,392	0.491	16,632	0.382	0.16
	3.00	24,633	0.565	20,775	0.477	0.17
EURV	3.10	26,433	0.607	23,328	0.536	0.17
	3.50	33,214	0.762	35,305	0.811	6.96
100-YR	3.91	39,583	0.909	50,229	1.153	21.71
	4.00	40,981	0.941	53,854	1.236	25.86
	4.50	47,489	1.090	76,179	1.749	98.81

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

POND B FOREBAY VOLUME REQUIREMENTS

Equation 3-1 $WQCV = a(0.91I^3 - 1.19I^2 + 0.781I)$

a=1 (40 hour drain time)

Proposed Forebay *I=.115* *WQCV=* 0.075346

Equation 3-3 V=(WQCV/12)A

Proposed Forebay A= 60.42 Acres V= 0.379

3% OF WQCV

Forebay Total Volume= .03(V)

Volume Required For Proposed Forebay= 0.011 AC-FT 496 CF

Volume Provided For Proposed Forebay= 0.014 AC-FT 620 CF

Q_{100} Discharges 2% OF Q_{100}

Q_{100} Proposed Forebay = .02*31.6 CFS= 0.63 CFS

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Thursday, Sep 1 2022

Pond B Trickle Channel

Rectangular

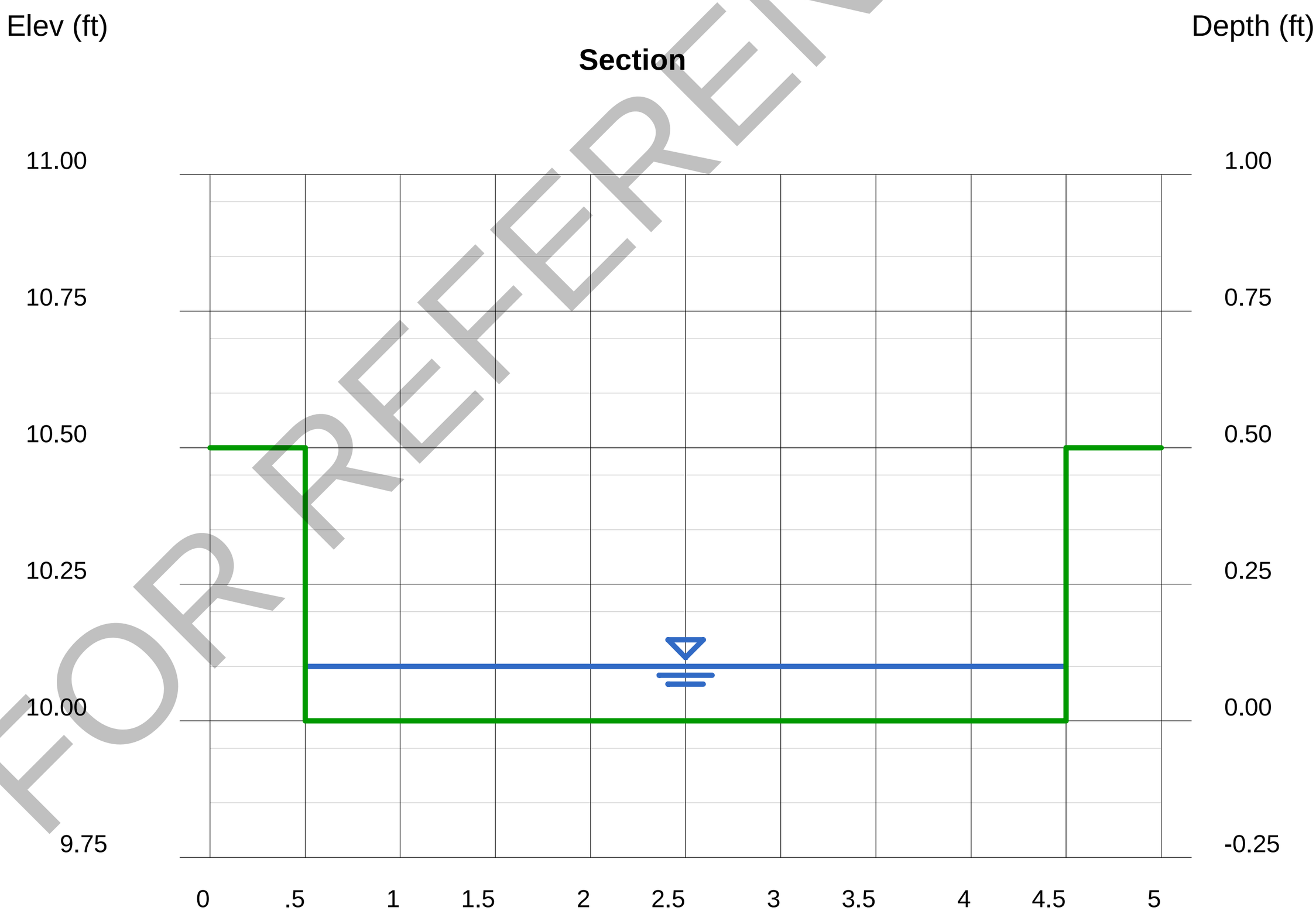
Bottom Width (ft)	= 4.00
Total Depth (ft)	= 0.50
Invert Elev (ft)	= 10.00
Slope (%)	= 0.50
N-Value	= 0.013

Calculations

Compute by:	Known Q
Known Q (cfs)	= 0.63

Highlighted

Depth (ft)	= 0.10
Q (cfs)	= 0.630
Area (sqft)	= 0.40
Velocity (ft/s)	= 1.58
Wetted Perim (ft)	= 4.20
Crit Depth, Yc (ft)	= 0.10
Top Width (ft)	= 4.00
EGL (ft)	= 0.14



Weir Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Tuesday, Mar 29 2022

Pond B Spillway

Trapezoidal Weir

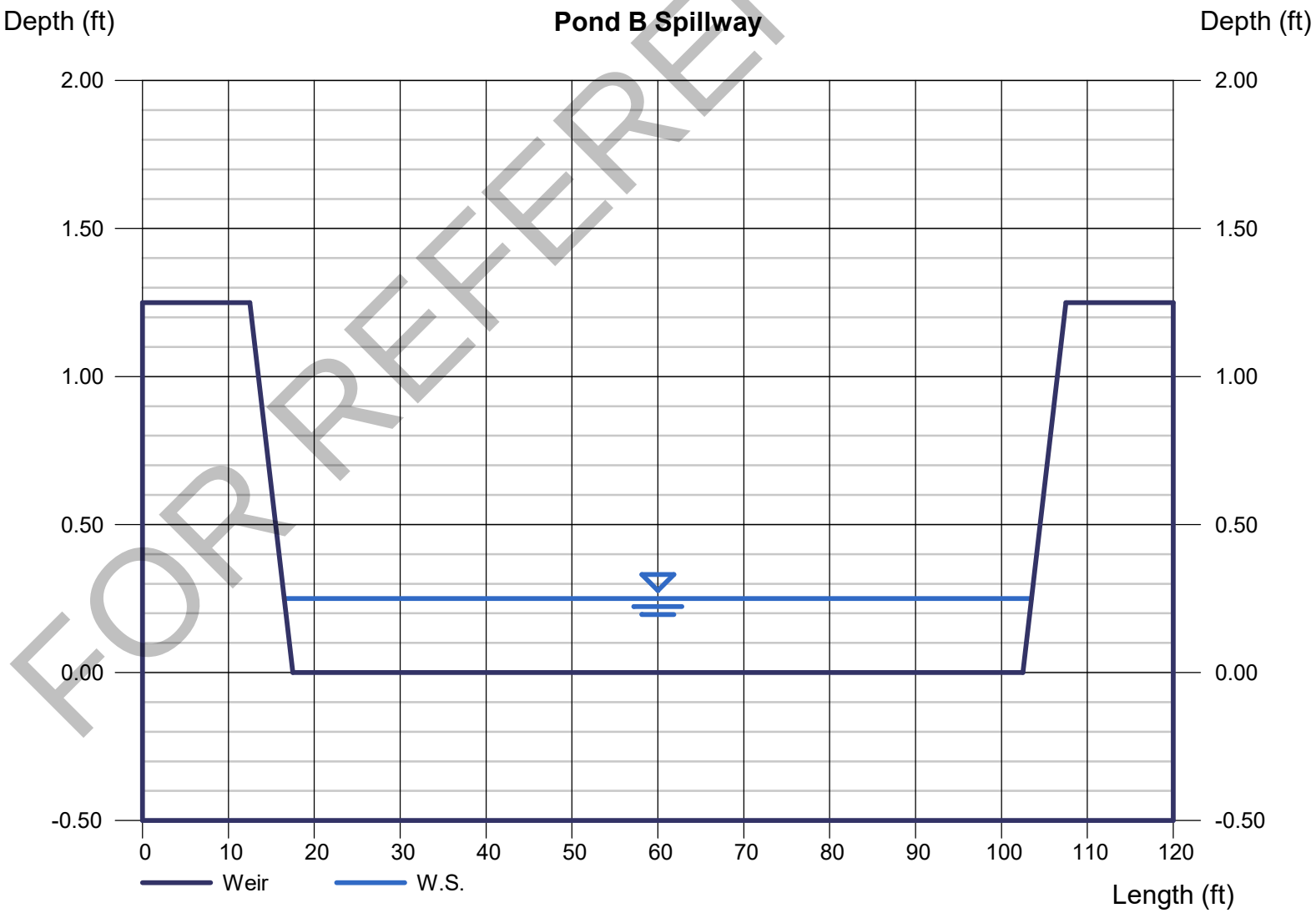
Crest	= Sharp
Bottom Length (ft)	= 85.00
Total Depth (ft)	= 1.25
Side Slope (z:1)	= 4.00

Calculations

Weir Coeff. Cw	= 3.10
Compute by:	Known Q
Known Q (cfs)	= 31.60

Highlighted

Depth (ft)	= 0.25
Q (cfs)	= 31.60
Area (sqft)	= 21.50
Velocity (ft/s)	= 1.47
Top Width (ft)	= 87.00



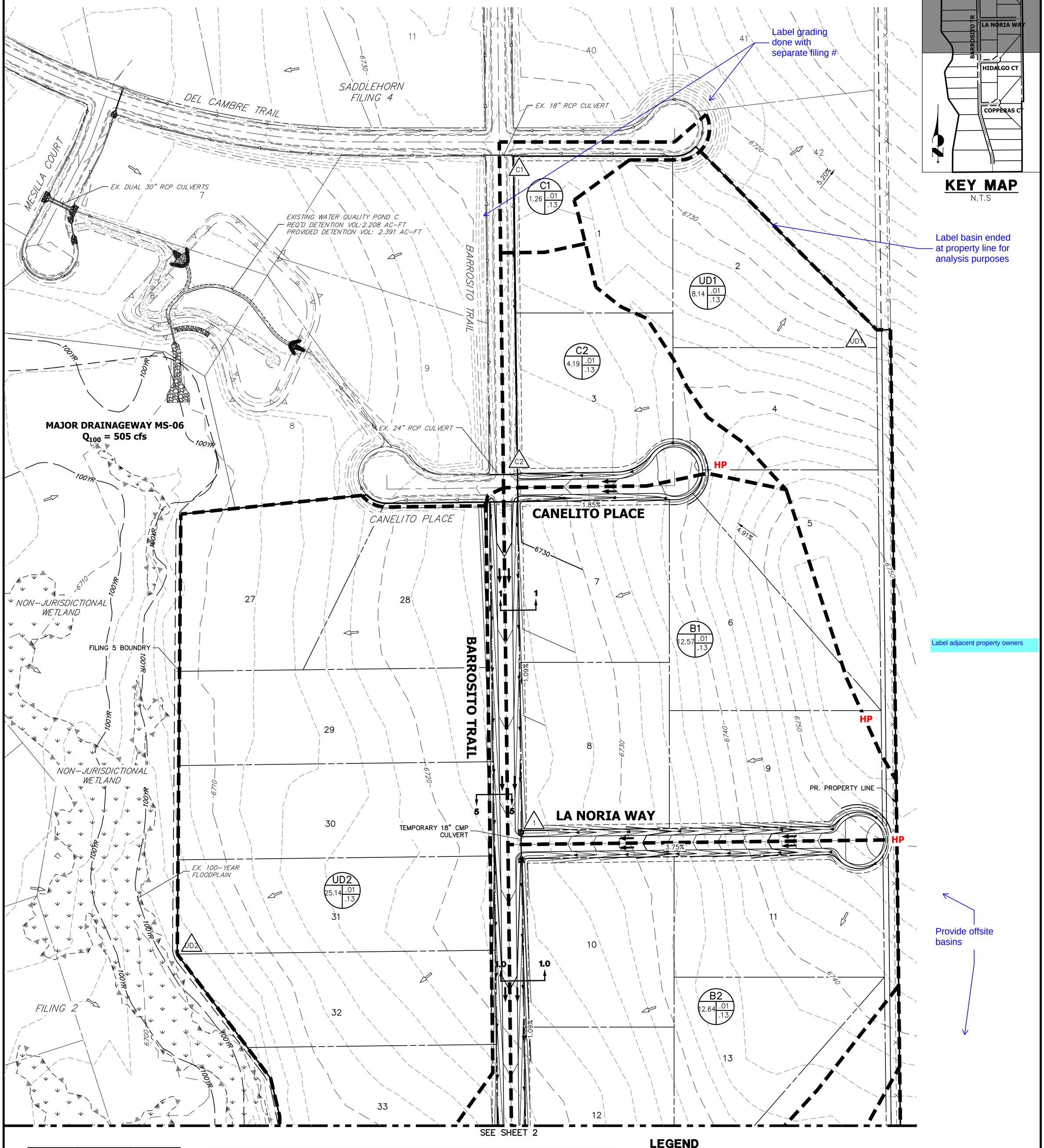
APPENDIX E

REFERENCE MATERIALS

APPENDIX F

DRAINAGE MAPS & PLANS

SADDLEHORN FILING 5 EARLY GRADING
PROPOSED DRAINAGE MAP



KEY MAP
N.T.S

FILING 5 - DESIGN POINT SUMMARY		
Design Point	Q_5 (cfs)	Q_{100} (Cfs)
1	0.2	6.4
2	0.3	10.5
3	0.1	5.2
4	0.1	4.6
5	0.2	6.5
6	0.0	1.1
11	0.3	6.9
C1	0.02	0.8
C2	0.1	2.6
UD1	0.1	4.7
UD2	0.4	13.5
UD3	0.1	4.8
UD4	0.1	1.6
1.0	0.3	11.4
1.1	0.4	15.2
1.2	0.5	17.7
1.3	0.6	21.0
1.4	0.6	19.6

Extend decimal place

FILING 5 - SUB-BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	15.08	2%	0.01	0.13	39.0	0.3	6.9
B1	12.57	2%	0.01	0.13	31.2	0.2	6.4
B2	12.64	2%	0.01	0.13	11.4	0.3	10.5
B3	10.83	2%	0.01	0.13	34.3	0.1	5.2
B4	9.16	2%	0.01	0.13	32.1	0.1	4.6
B5	13.72	2%	0.01	0.13	35.1	0.2	6.5
B6	1.50	2%	0.01	0.13	16.2	0.03	1.1
C1	1.26	2%	0.01	0.13	19.7	0.02	0.8
C2	4.19	2%	0.01	0.13	22.1	0.1	2.6
UD1	8.14	2%	0.01	0.13	25.1	0.1	4.7
UD2	25.14	2%	0.01	0.13	28.9	0.4	13.5
UD3	9.85	2%	0.01	0.13	34.2	0.1	4.8
UD4	2.68	2%	0.01	0.13	25.8	0.1	1.6

LEGEND

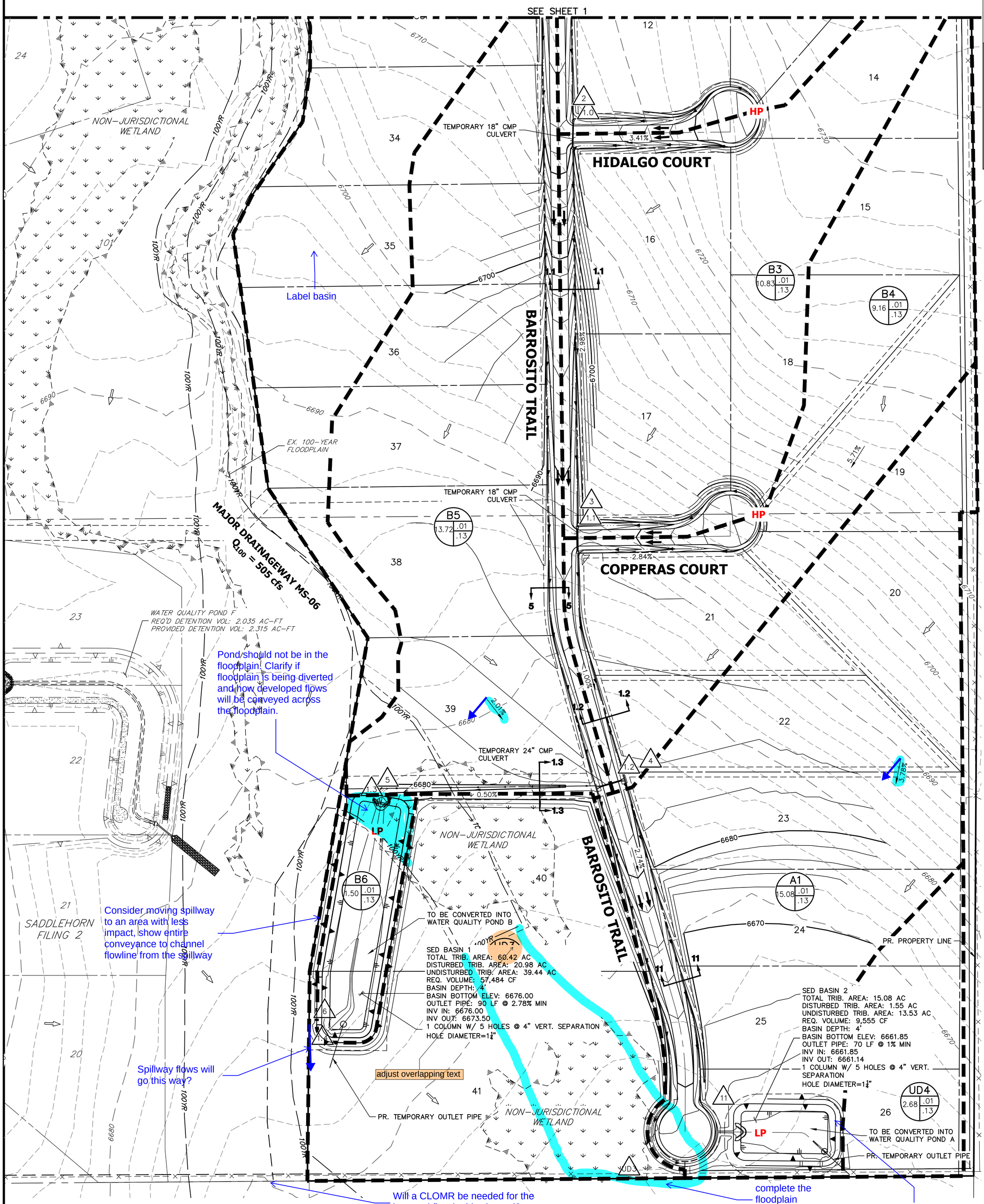
- I.D. (A: BASIN AREA, B: C_5, C: C_{100}) — BASIN DESIGNATION
- I.D.: BASIN IDENTIFIER
- X — DESIGN POINT
- MAJOR BASIN DELINEATION
- SUB-BASIN DELINEATION
- 6700 — EXISTING INDEX CONTOURS
- EXISTING INTERMEDIATE CONTOURS
- 6700 — PROPOSED INDEX CONTOURS
- PROPOSED INTERMEDIATE CONTOURS
- EXISTING FLOW DIRECTION
- PROPOSED FLOW DIRECTION
- HP — PROPOSED HIGH POINT
- LP — PROPOSED LOW POINT

100 50 0 100 200
ORIGINAL SCALE: 1" = 100'

EARLY GRADING PROPOSED DRAINAGE MAP
SADDLEHORN RANCH FILING 5
JOB NO. 2514207
8/26/2022
SHEET 1 OF 2

J·R ENGINEERING
A Westrian Company
Centennial 303-740-9393 • Colorado Springs 719-593-2593
Fort Collins 970-491-9888 • www.jrengineering.com

SADDLEHORN FILING 5 EARLY GRADING
PROPOSED DRAINAGE MAP



FILING 5 - DESIGN POINT SUMMARY

Design Point	Q_5 (cfs)	Q_{100} (Cfs)
1	0.2	6.4
2	0.3	10.5
3	0.1	5.2
4	0.1	4.6
5	0.2	6.5
6	0.0	1.1
11	0.3	6.9
C1	0.02	0.8
C2	0.1	2.6
UD1	0.1	4.7
UD2	0.4	13.5
UD3	0.1	4.8
UD4	0.1	1.6
1.0	0.3	11.4
1.1	0.4	15.2
1.2	0.5	17.7
1.3	0.6	21.0
1.4	0.6	19.6

FILING 5 - SUB-BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	15.08	2%	0.01	0.13	39.0	0.3	6.9
B1	12.57	2%	0.01	0.13	31.2	0.2	6.4
B2	12.64	2%	0.01	0.13	11.4	0.3	10.5
B3	10.83	2%	0.01	0.13	34.3	0.1	5.2
B4	9.16	2%	0.01	0.13	32.1	0.1	4.6
B5	13.72	2%	0.01	0.13	35.1	0.2	6.5
B6	1.50	2%	0.01	0.13	16.2	0.03	1.1
C1	1.26	2%	0.01	0.13	19.7	0.02	0.8
C2	4.19	2%	0.01	0.13	22.1	0.1	2.6
UD1	8.14	2%	0.01	0.13	25.1	0.1	4.7
UD2	25.14	2%	0.01	0.13	28.9	0.4	13.5
UD3	9.85	2%	0.01	0.13	34.2	0.1	4.8
UD4	2.68	2%	0.01	0.13	25.8	0.1	1.6

Missing labels on map

LEGEND

- I.D. (A / B / C): BASIN DESIGNATION
  - I.D.: BASIN IDENTIFIER
  - A: BASIN AREA
  - B: C_5
  - C: C_{100}
- X: DESIGN POINT
- MAJOR BASIN DELINEATION
- SUB-BASIN DELINEATION
- 6700: EXISTING INDEX CONTOURS
- EXISTING INTERMEDIATE CONTOURS
- 6700: PROPOSED INDEX CONTOURS
- PROPOSED INTERMEDIATE CONTOURS
- EXISTING FLOW DIRECTION
- PROPOSED FLOW DIRECTION
- HP: PROPOSED HIGH POINT
- LP: PROPOSED LOW POINT

100 50 0 100 200
ORIGINAL SCALE: 1" = 100'

EARLY GRADING PROPOSED DRAINAGE MAP
SADDLEHORN RANCH FILING 5
JOB NO. 2514207
8/26/2022
SHEET 2 OF 2

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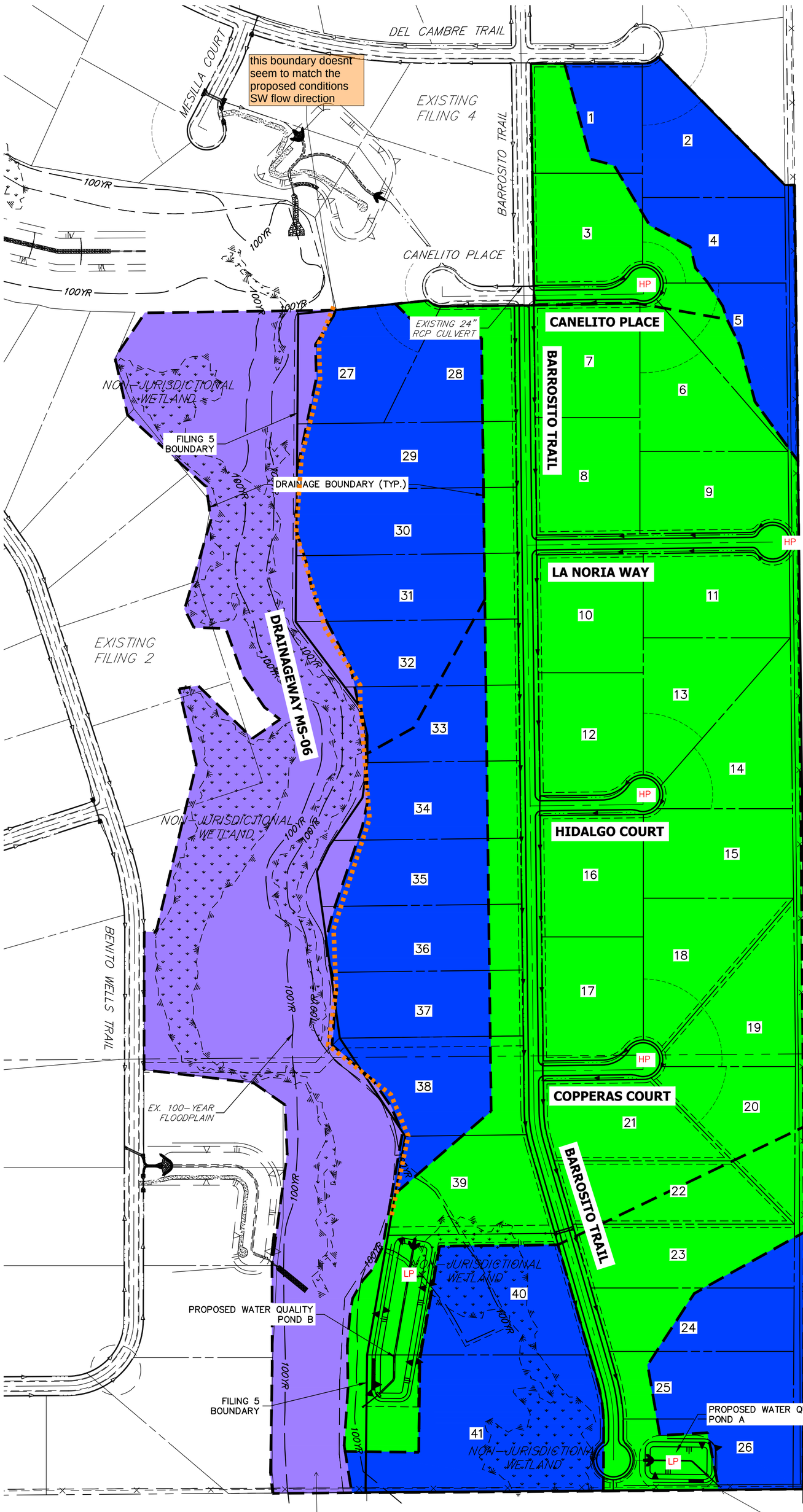
SADDLEHORN RANCH - FILING 5
PERMANENT APPLICABILITY MAP



show offsite discharge path from outlet to drainageway

you are not disturbing this area. It is outside the project limits and does not need to be captured in an exclusion.

LEGEND

- BASIN DELINEATION
- –6100– EXISTING INDEX CONTOURS
- EXISTING INTERMEDIATE CONTOURS
- 6100 PROPOSED INDEX CONTOURS
- PROPOSED INTERMEDIATE CONTOURS
- HP PROPOSED HIGH POINT
- LP PROPOSED LOW POINT
- AREA DETAINED IN PBMP
- AREA NOT DETAINED IN PBMP PER SECTION I.7.1.B.5 (RURAL 2.5+ ACRE LOTS W/ IMPERVIOUSNESS < 10%)
- AREA NOT DETAINED IN PBMP PER SECTION I.7.1.B.7 (LAND DISTURBANCE TO UNDEV. LAND THAT WILL REMAIN UNDEV.)

N

200 100 0 200 400

ORIGINAL SCALE: 1" = 200'

MS4 PERMIT EXCULSION AREAS
SADDLEHORN RANCH FILING 5
JOB NO. 25142.07
9/1/2022
SHEET 1 OF 1

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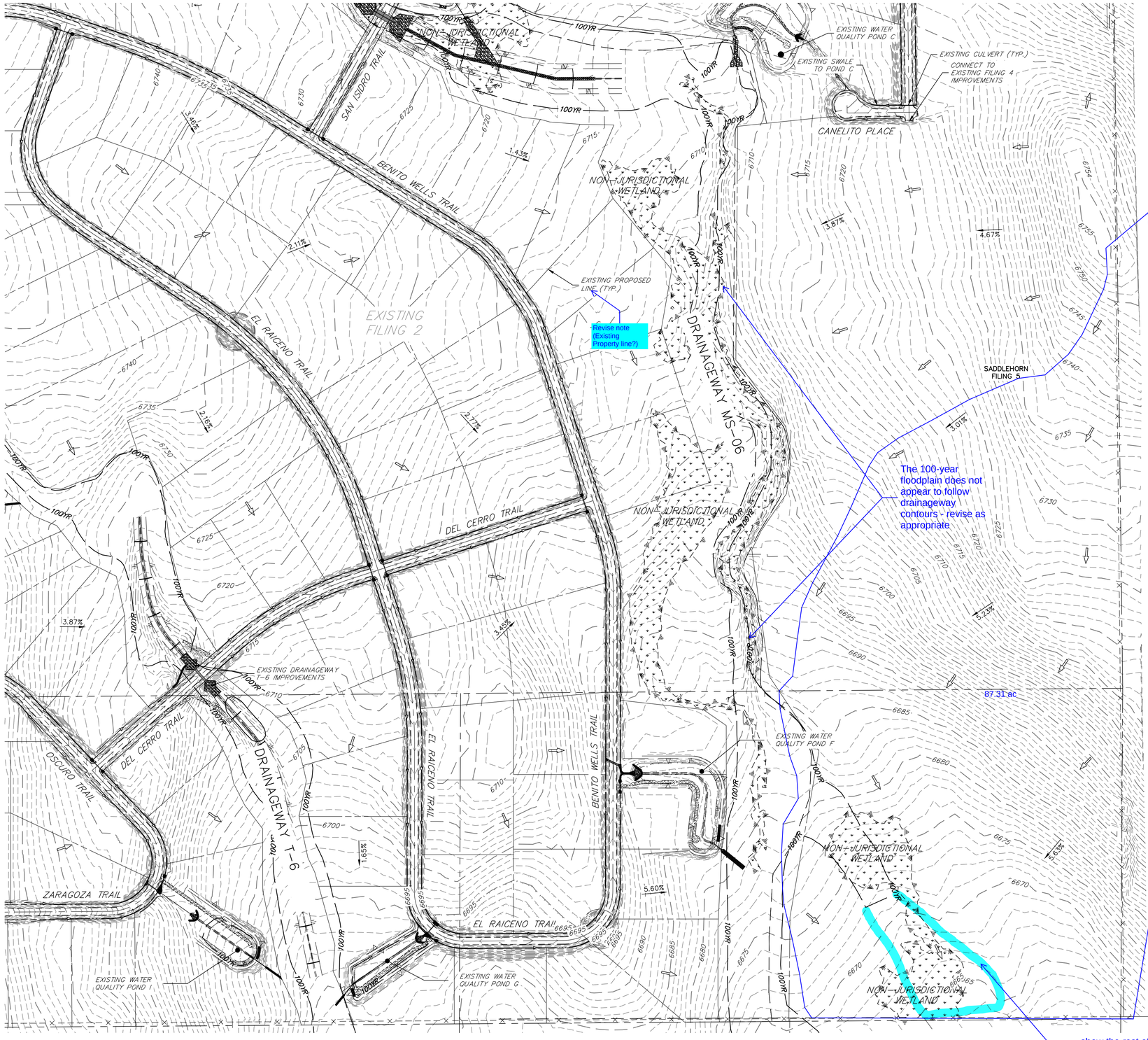
SADDLEHORN RANCH - FILING 5
EXISTING CONDITIONS MAP



Revise note (Existing Property line?)

Address what portion of the basin currently flowing to the south will be diverted to the west through the proposed detention pond and what the impacts to the channel will be. This will be a larger issue with the final plat.

Provide sub-basins, design points, flows, etc. for comparison to developed condition design points

Move map in front of proposed drainage maps.

Label property boundary

The 100-year floodplain does not appear to follow drainageway contours - revise as appropriate

show the rest of the 100-year floodplain to the property line

LEGEND

- EXISTING INDEX CONTOURS (6700)
- EXISTING INTERMEDIATE CONTOURS
- EXISTING FLOW DIRECTION
- 100-YEAR FLOODPLAIN (100YR)

N

200 100 0 200 400

ORIGINAL SCALE: 1" = 200'

EXISTING CONDITIONS MAP
SADDLEHORN FILING 5
PROJ. NO. 25142.07
8/26/22
SHEET 1 OF 1

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