April 6, 2021

COLA, LLC 555 Middle Creek Parkway, Suite 200 Colorado Springs, Colorado 80921

- Attn: Stephen Schoonover
- Re: Pavement Recommendations Addendum Trails at Aspen Ridge, Filing No.1 Bradley Road Acceleration Deceleration Lane El Paso County, Colorado





505 ELKTON DRIVE COLORADO SPRINGS, CO 60907 PHONE (719) 531-5599 FAX (719) 531-5238

APPROVED Engineering Department

04/28/2021 2:31:29 PM dsdnijkamp EPC Planning & Community Development Department

Reference: Pavement Recommendations, by Entech Engineering, Inc., dated March 10, 2020, Revised July 16, 2020, Entech Job No.191931

Dear Mr. Schoonover:

As requested, personnel of Entech Engineering, Inc. have reviewed our above-referenced report and have provided a flexible pavement design section for the proposed Bradley Road acceleration/deceleration lane (right turn lane) for the roadway referenced above. This design is based on the soil parameters used in the above referenced report.

A pavement section was calculated for flexible asphalt pavement. Design parameters from the pavement analysis were taken from the previous pavement report. The El Paso County "Pavement Design Criteria Manual" was also referenced for other requirements. In the ESAL Calculation Memorandum letter by LSC Transportation Consultants, Inc., dated February 9, 2021, LSC# 184362, an 18K ESAL value of 855,000 was recommended for the accel/decel lanes.

An asphalt pavement section of 6 inches of asphalt on 15 inches of aggregate base course is recommended for the site. An alternative would be 6 inches of asphalt over 14 inches of cement-treated subgrade. The subgrade should be stabilized with 4 percent cement mixed into the subgrade to a depth of 14 inches. High sulfates must be mitigated if the CTS option is chosen. Design calculations are shown in Figures 1 through 3.

Other recommendations in the pavement analysis report remain valid. The roadway construction shall meet the El Paso County roadway pavement specifications.

We trust that this has provided you with the information you required. If you have any questions or need additional information, please do not hesitate to contact us.

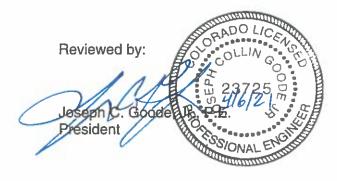
Respectfully Submitted,

ENTECH ENGINEERING, INC.

Daniel P. Stegman

DPS/bs Encl.

Entech Job No. 191931 2MSW/paveltr/2020/191931 pr - add



EPC file number SF192

## **FLEXIBLE PAVEMENT DESIGN**

#### DESIGN DATA COLA, LLC - BRADLEY ROAD RIGHT TURN (OUT) ACCEL LANE TRAILS AT ASPEN RIDGE, FILING 1 - PRINCIPAL ARTERIAL - 4-LANE SOIL TYPE 1 Equivalent (18 kip) Single Axle Load Applications (ESAL): $ESAL(W_{18}) =$ 855,000 Hveem Stabilometer (R Value) Results: R = 6 Standard Deviation $S_0 =$ 0.45 Loss in Serviceability 2.5 $\Delta psi =$ Reliability Reliability = 90 Reliability (z-statistic) $Z_R =$ -1.28 Soil Resilient Modulus $M_R =$ 3126 Weighted Structural Number (WSN): WSN =4.23 DESIGN TABLES AND EQUATIONS $S_1 = [(R - 5) / 11.29] + 3$ $M_{\rm R} = 10^{[(S_1 + 18.72)/6.24]}$

 $k = M_R / 19.4$ 

Where:

M<sub>R</sub> = resilient modulus (psi)

S1 = the soil support value

R = R-value obtained from the Hveem stabilometer

CBR = California Bearing Ratio

Reliability (%)	Z <sub>R</sub> (z-statistic)
80	-0.84
85	-1.04
90	-1.28
93	-1.48
94	-1.56
95	-1.65
96	-1.75
97	-1.88
98	-2.05
99	-2.33
99.9	-3.09
99.99	-3.75

 $\log_{10}W_{18} = Z_R^* S_0^+ 9.36^* \log_{10}(SN+1) - 0.20 + -$ 

$$\log_{10} \left[ 4.2 - 1.5 \right]$$

$$0.40 + \frac{1094}{(SN+1)^{5.19}}$$

+ 2.32\*log<sub>10</sub>M<sub>R</sub>- 8.07

Job No. 191931 Fig. No. C-1

Left	Right	Difference
5.93	5.93	0.0

## **DESIGN CALCULATIONS**

<u>DESIGN DATA</u>	COLA, LLC - BRADLEY ROAD RIGHT TURN (OUT	Γ) ACCEL L	ANE
	TRAILS AT ASPEN RIDGE, FILING 1 - PRINCIPAL	ARTERIAL	- 4-LANE
	SOIL TYPE 1		
Equivaler	at (18 kip) Single Axle Load Applications (ESAL):	ESAL =	855,000
Hveem St	abilometer (R Value) Results:	R =	6
Weighted	Structural Number (WSN):	WSN =	4.23

#### **DESIGN EQUATION**

 $WSN = C_1D_1 + C_2D_2$ 

 $C_1 = 0.44$  Strength Coefficient - Hot Bituminous Asphalt  $C_2 = 0.11$  Strength Coefficient - Aggregate Base Course

 $D_1$  = Depth of Asphalt (inches)  $D_2$  = Depth of Base Course (inches)

#### FOR FULL DEPTH ASPHALT SECTION (CURRENTLY NOT ALLOWED)

 $D_1 = (WSN)/C_1 = 9.6$  inches of Full Depth Asphalt Use 10.0 inches Full Depth

#### FOR ASPHALT + AGGREGATE BASE COURSE SECTION

Asphalt Thickness (t) = 6 inches  $D_2 = ((WSN) - (t)(C_1))/C_2 = 14.5$  inches of Aggregate Base Course, use 15.0 inches

#### **RECOMMENDED ALTERNATIVES**

- 1. 6.0 inches of Asphalt + 15.0 inches of Aggregate Base Course, or
- 2. 10.0 inches of Full Depth Asphalt

Job No. 191931 Fig. No. C-2

## **DESIGN CALCULATIONS**

#### CEMENT TREATED SECTIONS

DESIGN DATA:COLA, LLC - BRADLEY RD RIGHT TURN ACCEL (OUT) LANE<br/>TRAILS AT ASPEN RIDGE, FILING 1 - PRINCIPAL ARTERIAL - 4-LANE<br/>SOIL TYPE 1Equivalent (18 kip) Single Axle Load Applications (ESAL):ESAL = 855,000<br/>R = 6<br/>Weighted Structural Number (WSN):

#### **DESIGN EQUATION**

 $WSN = C_1D_1 + C_2D_2$ 

 $C_1 = 0.44$  Strength Coefficient - Hot Bituminous Asphalt  $C_2 = 0.12$  Strength Coefficient - Cement Treated Subgrade.

 $D_1 = Depth of Asphalt (inches)$ 

D<sub>2</sub> = Depth of Cement Treated Subgrade (inches)

#### FOR FULL DEPTH ASPHALT SECTION - (CURRENTLY NOT ALLOWED)

 $D_1 = (WSN)/C_1 = 9.6$  inches of Full Depth Asphalt Use 10.0 inches Full Depth

#### FOR ASPHALT + CEMENT TREATED SUBGRADE SECTION

Asphalt Thickness (t) = 6 inches  $D_2 = ((WSN) - (t)(C_1))/C_2 = 13.3$  inches Use 15.0 inches of Cement Treated Subgrade.

#### **RECOMMENDED ALTERNATIVES**

- 1. 6.0 inches of Asphalt + 15.0 inches of Cement Treated Subgrade.
- 2. 10.0 inches of Full Depth Asphalt

Job No. 191931 Fig. No. C-3 March 10, 2020

 $\Leftrightarrow$ 



505 ELKTON DRIVE COLORADO SPRINGS, CO 80907 PHONE (719) 531-5599 FAX (719) 531-5238

- COLA, LLC 555 Middle Creek Parkway, Suite 200 Colorado Springs, Colorado 90921
- Attn: Steven Schoonover
- Re: Cement Stabilized Subgrade Results Laboratory Testing Trails at Aspen Ridge, Filing 1 El Paso County, Colorado
- Ref: Pavement Recommendations Report by Entech Engineering, Inc., dated March 10, 2020, Entech Job No. 191931

Dear Mr. Schoonover:

As requested, personnel of Entech Engineering, Inc. have performed strength and Swell/Consolidation testing on two sets of soil/cement composite samples for the above reference project. Testing was performed on soil samples prepared with 2% and 4% Portland Cement Type 1/2, from Martin Marietta, near Pueblo, Colorado.

The 5-day average strength values of the 2% mix was 183 psi. The 5-day average strength values of the 4% mix was 204 psi. Swells remolded with 2% cement resulted in 2.2 and 2.8 percent volume changes, which exceed the maximum allowable swell for the site soils. Swells remolded with 4 percent cement resulted in volume changes of 1.4 and 1.3 percent, which are beneath the allowable swell of 2 percent. A 4% mix is recommended based on the laboratory test results. The testing results are attached.

Pending the results of the field density testing, microfracturing of the stabilized subgrade may be required. Soil strengths in excess of 200 psi require microfracturing.

We trust this has provided you with the information you required. if you have any questions or need additional information, please do not hesitate to contact us.

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Daniel P. Stegman

DPS/ao

Encl.

Entech Job No. 191931 AAprojects/2019/191931/191931 cssr - lab



# SUMMARY OF CTS TEST RESULTS LAB TESTING

CLIENT COLA, LLC		JOB NO	191931
PROJECT TRAILS AT /	ASPEN RIDGE	DATE	12/19/19
FIELD SAMPLE ID	TB-1 @ 0-3'	BY	BL
SOIL ADDITIVE TYPE I/II CEMENT			
AASHTO CLASS.	A-7-6		
SOIL TYPE NO.	#1		

ADDITIVE %	DATE SAMPLED	WATER %	DENSITY (dry)	AGE (days)	STRENGTH (psi)		
2	12/13/19	12.6	104.8	5	183		
2	12/13/19	12.6	104.6	5	178		
2	12/13/19	12.6	104.7	5	189		
	AVERAGE:						
4	12/13/19	12.6	104.7	5	214		
4	12/13/19	12.6	104.6	5	190		
4	12/13/19	12.6	104.5	5	207		
				AVERAGE:	204		

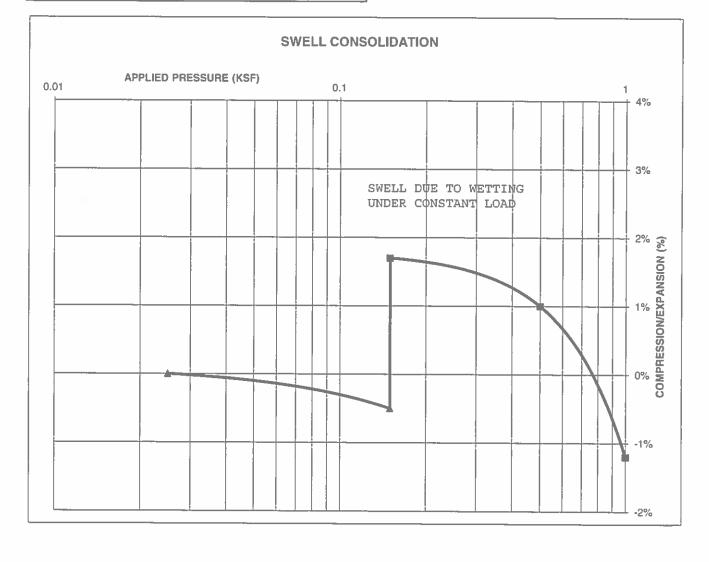
CURING METHOD

100° HUMIDIFIED OVEN

TEST BORING #	16	DEPTH(ft)	1-2
DESCRIPTION	CL	SOIL TYPE	1
NATURAL UNIT DRY	WEIGH	HT (PCF)	112
NATURAL MOISTUR	E CONT	FENT Í	12.9%
SWELL/CONSOLIDA			2.2%

<u>JOB NO.</u>	191931
<u>CLIENT</u>	COLA, LLC
PROJECT	TRAILS AT ASPEN RIDGE
	REMOLDED W/ +2% CEMENT

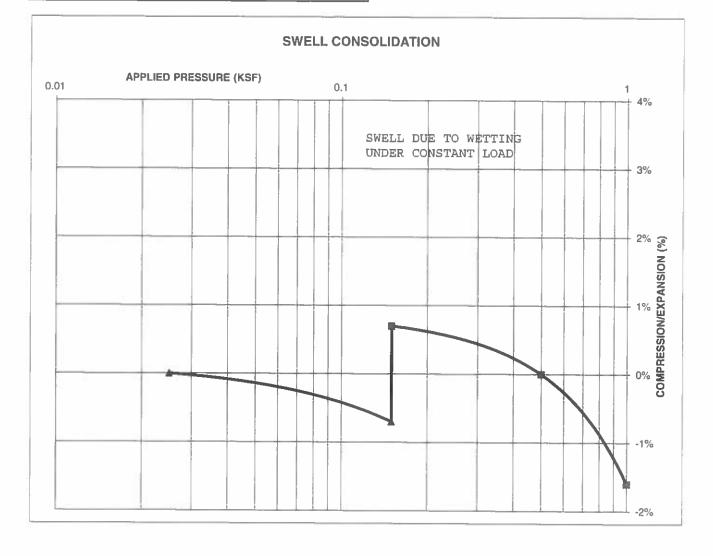
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ENTECH ENGINEERING, INC.				JOB NO 191931				
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	厂	DRAWN:	DATE	CHECKED:	DATE	J	FIG NO.

TEST BORING #	16	DEPTH(ft)	1-2
DESCRIPTION	CL	SOIL TYPE	1
NATURAL UNIT DRY	WEIGH	HT (PCF)	110
NATURAL MOISTUR	E CON	TENT	18.3%
SWELL/CONSOLIDA	TION (9	%)	1.4%

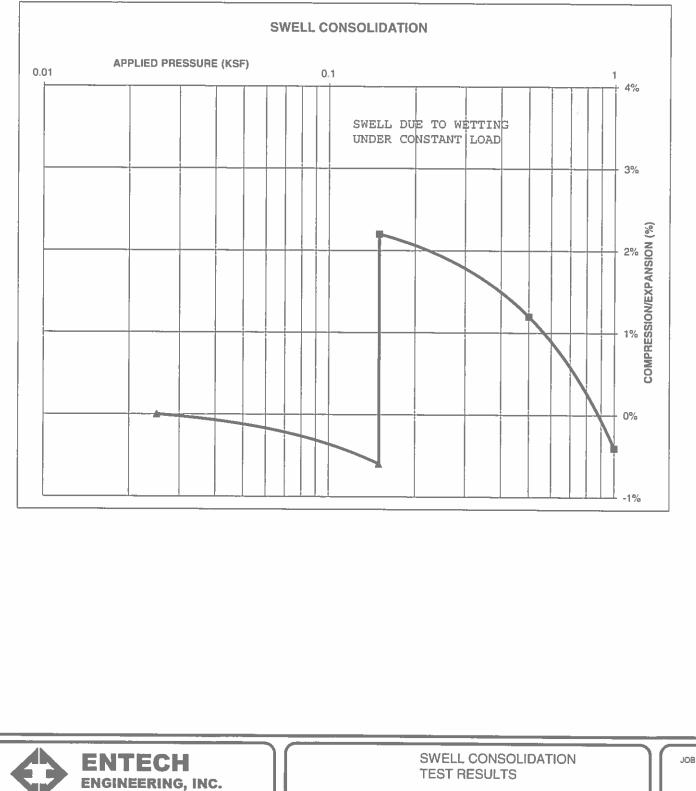
JOB NO.	191931
<u>CLIENT</u>	COLA, LLC
PROJECT	TRAILS AT ASPEN RIDGE
	REMOLDED W/ +4% CEMENT



ENTECH ENGINEERING, INC.		SWELL CONSOLIDATION TEST RESULTS						
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAW	4;	DATE:	CHECKED:	2/3/2	JI	FIG NO

TE	EST BORING #	21	DEPTH(ft)	1-2
	ESCRIPTION	CL	SOIL TYPE	1
N/	ATURAL UNIT DRY	WEIGH	HT (PCF)	117
N/	ATURAL MOISTUR	E CON	<b>FENT</b> of a	12.2%
SI	<b>VELL/CONSOLIDA</b>	TION (?	%)	2.8%

<u>JOB NO.</u>	191931
<u>CLIENT</u>	COLA, LLC
PROJECT	TRAILS AT ASPEN RIDGE
	REMOLDED W/ +2% CEMENT



JOB NO. 191931 FIG NO. **3** 

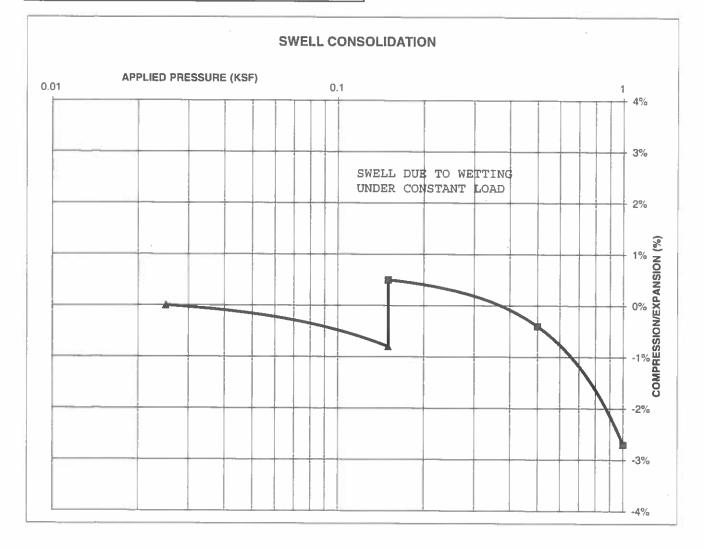
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505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907 DBAWN:

WN: DATE: CHECKED

TEST BORING #	21	DEPTH(ft)	1-2
DESCRIPTION	CL	SOIL TYPE	1
NATURAL UNIT DRY	109		
NATURAL MOISTUR	17.3%		
SWELL/CONSOLIDA	1.3%		

JOB NO.	191931
<u>CLIENT</u>	COLA, LLC
PROJECT	TRAILS AT ASPEN RIDGE
	REMOLDED W/ +4% CEMENT



$\diamond$	ENTECH ENGINEERING, INC. 505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907		SWELL CONSOLIDATION TEST RESULTS				$\left  \right $	JOB NO: 191931
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