

LSC TRANSPORTATION CONSULTANTS, INC. 2504 East Pikes Peak Avenue, Suite 304 Colorado Springs, CO 80909 (719) 633-2868 FAX (719) 633-5430 E-mail: <u>lsc@lsctrans.com</u> Website: http://www.lsctrans.com

Copper Chase at Sterling Ranch Traffic Technical Memorandum PUDSP-22-002 (LSC #184990) July 25, 2023

Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

7/31/23 Date

Copper Chase at Sterling Ranch Traffic Impact Study

Prepared for:

Mr. Jim Byers Challenger Homes 8605 Explorer Drive, Suite 250 Colorado Springs, CO 80920

JULY 25, 2023

LSC Transportation Consultants Prepared by: Kirstin D. Ferrin, P.E. Reviewed by: Jeffrey C. Hodsdon, P.E.

LSC #184990 PUDSP-22-002



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July 25, 2023

Mr. Jim Byers Challenger Homes 8605 Explorer Drive, Suite 250 Colorado Springs, CO 80920

> RE: Copper Chase at Sterling Ranch El Paso County, CO Updated Traffic Impact Study PUDSP-22-002 LSC #184990

Dear Mr. Byers:

LSC Transportation Consultants, Inc. has prepared this updated traffic impact study (TIS) for the proposed Copper Chase at Sterling Ranch residential development. As shown in Figure 1, the site is located east of Vollmer Road and north of the future extension of Marksheffel Road in El Paso County, Colorado.

LSC prepared a traffic technical memorandum for this development dated May 26, 2022, however since completion of that memorandum LSC has updated the Sterling Ranch Master Traffic Impact Study and prepared multiple reports various for preliminary plans and filings within Sterling Ranch and the surrounding area which impact the recommendations and conclusions. This report replaces the May 2022 memorandum and is intended as site-specific, PUD Development/PUD Plan report for Copper Chase at Sterling Ranch.

REPORT CONTENTS

This report presents:

- A list of previous Sterling Ranch traffic reports and the context of this project;
- A summary of the proposed land use and access plan;
- The existing roadway and traffic conditions in the site's vicinity including the roadway widths, surface conditions, lane geometries, traffic controls, and posted speed limits;
- The projected average weekday and peak-hour vehicle trips to be generated by the proposed future development;
- The assignment of the projected site-generated traffic volumes to the area roadways;

- Estimates of projected short-term long-term Background traffic volumes;
- The projected short-term and long-term total traffic volumes on the area roadways;
- The projected levels of service at the key intersections within the study area;
- A list of deviation requests
- Findings and recommendations for study area roadways and intersections, including number of lanes, auxiliary turn lanes, intersection traffic control, etc.; and
- The recommended street classifications
- The project's obligation to the County roadway improvement fee program.

REPORT SCENARIOS

Short-Term Scenario

The short-term scenario includes the roadway segments to be added in the short term only as shown in Figure 2. This scenario includes traffic to be traffic generated by the currently proposed Copper Chase at Sterling Ranch and traffic to be generated in the short term by buildout of Homestead at Sterling Ranch, Branding Iron at Sterling Ranch, Sterling Ranch Filings 2-4, Homestead North at Sterling Ranch Filings 1-3, the Retreat at TimberRidge Filings 1-3, Sterling Ranch East Filings 1 and 2, and the planned FourSquare at Sterling Ranch East development. Trips projected from these other short-term developments outside of the currently proposed Copper Chase at Sterling Ranch are included as short-term "background traffic" in this report.

Long-Term Scenario

The long-term scenario is essentially the same as the 2043 Long-Term scenario contained in the LSC February 10, 2023 Master TIS with additional detail added for this application – including the analysis of minor intersections and street segments that are part of the currently proposed development. The study area of this report is more focused than the Sketch Plan.

RECENT TRAFFIC REPORTS

- LSC completed an updated master traffic study (MTIS) for the entire Sterling Ranch development, dated March 17, 2023. Appendix Table 1 includes a link to the El Paso County Electronic Development Application Review Program (EDARP) page where a copy of the latest version of that MTIS can be obtained.
- A list of other traffic studies within Sterling Ranch and in the vicinity of area of study completed within the past five years (that LSC is aware of) is attached for reference (Appendix Table 1).
- El Paso County is currently studying the Briargate Stapleton Corridor as part of a Pikes Peak Rural Transportation Authority (PPRTA) study. A draft version of the Briargate-Stapleton Corridor Study by Wilson & Company was published December 9, 2021.

EXISTING ROAD AND TRAFFIC CONDITIONS

The adjacent streets are shown in Figure 1 and are described below. Copies of the 2016 El Paso County Major Transportation Corridors Plan (MTCP), 2040 Roadway Plan, and 2016 MTCP 2060 Corridor Preservation Plan with the site location identified on them have been attached to this report.

Vollmer Road is currently a five-lane urban street within the City of Colorado Springs limits between Black Forest Road and Cowpoke Road and a two-lane, rural, paved roadway north of Cowpoke Road extending to north of Hodgen Road. Improvements to a section of Vollmer Road in the vicinity of the future Marksheffel Road are currently under construction. In the southbound direction, Vollmer Road has a posted speed limit of 45 mph. South of Cowpoke Road, Vollmer Road has a 40-mph posted speed limit. The 2040 El Paso County *Major Transportation Corridors Plan (MTCP)* and the 2023 Sterling Ranch master traffic study show Vollmer Road as a four-lane Urban Minor Arterial in the vicinity of the site. Note: The new Connect COS City of Colorado Springs transportation plan shows Vollmer as a Principal Arterial.

Marksheffel Road is a Principal Arterial extending north from the City of Fountain to Woodmen Road. Marksheffel Road is planned to ultimately be widened to six lanes and extended north and west from Woodmen Road to connect to Research Parkway at Black Forest Road. Marksheffel Road is shown as a four-lane Principal Arterial through the site on the El Paso County *MTCP*. The City of Colorado Springs intends to take ownership and maintenance of Marksheffel Road when it is constructed from Vollmer to the east and south to where it will connect to the segment constructed north of Woodmen Road in the City.

The section of Marksheffel Road adjacent to Sterling Ranch is currently under construction on 107 feet of right-of-way to the City's required cross section(s) and criteria. It is anticipated that Marksheffel Road will be connected between Vollmer Road and Woodmen Road by the end of 2023.

Briargate Parkway is a six-lane, Principal Arterial that extends east from I-25 to Grand Lawn Circle (about one-half mile east of Powers Boulevard). Briargate Parkway is planned ultimately to extend to Towner Drive. The segment of Briargate Parkway between Vollmer Road and Sterling Ranch Road is planned to be constructed in the short term with the Sterling Ranch East Preliminary Plan.

Sterling Ranch Road is a planned Non-Residential Collector shown extending through the Sterling Ranch development between Marksheffel Road and the north end of the Sketch Plan area (Arroya Road). The segment between Marksheffel Road and Dines Boulevard has been constructed and the segment between Dines Boulevard and Briargate Parkway will be constructed in the short term with the Sterling Ranch East Preliminary Plan.

LAND USE AND ACCESS

Copper Chase at Sterling Ranch is planned to include 138 lots for single-family homes and is located east of Vollmer Road and north of the segment of Marksheffel Road currently under construction. Figure 3 shows the proposed site plan.

Access is proposed to Alzada Drive and Bynum Drive. Both of these existing "Urban Local" roadways are part of the approved Sterling Ranch Filing No. 2. The Copper Chase at Sterling Ranch access points are consistent with the access assumed in the TIS for that filing.

Copper Chase at Sterling Ranch was included in the Sterling Ranch Master TIS as Traffic Analysis Zone (TAZ) 5. Traffic projected to be generated by land uses within this zone was also included as part of the short-term background traffic volumes in the *Sterling Ranch East Rezoning and Preliminary Plan TIS*. The land use and access currently proposed are consistent with what was assumed in the *Master TIS and the Sterling Ranch East Rezoning and Preliminary Plan TIS*.

Intersection Sight Distance

Figure 4 shows a sight-distance analysis at the proposed intersections to Alzada Drive and Bynum Drive. Based on a design speed of 25 miles per hour (mph) and the criteria contained in Table 2-21 of the *Engineering Criteria Manual (ECM)*, the required intersection sight distance at the future intersections is 280 feet. Based on the criteria contained in Table 2-17 of the *ECM*, the required stopping sight distance approaching this intersection is 155 feet. As shown in Figure 3, both proposed intersections analyzed will meet the criteria.

Pedestrian and Bicycle Analysis

Figure 5 shows the location of all planned trails and sidewalks in the vicinity of the site. Connections are also proposed to the planned future Sand Creek Regional Trail (west of Dines Boulevard), as shown in the attached map.

A detached sidewalk will be provided along the west side of Sterling Ranch Road. The multi-use paved shoulder on Sterling Ranch Road will accommodate bicycles.

There are no existing schools within two miles of the site. However, multiple school sites are planned within Sterling Ranch northeast of the intersection of Sterling Ranch Road/Dines Boulevard. School pedestrians would travel to/from the intersection of Sterling Ranch/Dines on the sidewalks and trails identified in Figure 4. The need for additional pedestrian facilities and/or school crossings should be identified when the school site(s) are developed.

Safety Analysis

Most of the roadways in the vicinity of the site have not yet been constructed. The Colorado State Patrol (CSP) provided LSC with crash history data for Vollmer Road between Tahiti Drive and Burgess Road from September 2019 through September 2022. During the reported time period, there were twelve reported crashes. Of the twelve reports, ten were single-vehicle non-intersection-related crashes on Vollmer Road. One crash involved a southbound vehicle that turned right onto Poco Road and crashed into several cars parked on Poco Road partially in the lane. The only intersection related crash occurred in June 2022. A vehicle heading northbound on Vollmer Road was slowing to turn left at Lochwinnoch Road and the vehicle behind them attempted to pass on the left side. The crash history data has been attached.

TRIP GENERATION

Copper Chase at Sterling Ranch site-generated vehicle trips have been estimated using the nationally-published trip-generation rates from *Trip Generation, 11th Edition, 2021* by the Institute of Transportation Engineers (ITE). Table 1 shows the trip-generation estimate. The trip-generation estimate is consistent with the estimate assumed in the *Sterling Ranch Master TIS* and the *Sterling Ranch East Rezoning and Preliminary Plan TIS* for the same parcels.

Copper Chase at Sterling Ranch is expected to generate 1,301 vehicle trips on the average weekday, with about half entering and half exiting the site during a 24-hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 25 vehicles would enter and 71 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between 4:15 and 6:15 p.m., about 82 vehicles would enter and 48 vehicles would exit the site.

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the street and roadway system serving the site is an important factor in determining the site's traffic impacts. The distribution estimates for short-term and long-term residential related traffic are shown in Figure 6. The short-term directional-distribution estimate assumes the short-term roadway network shown in Figure 2 only and the long-term directional distribution estimate assumes buildout of the roadway network. The directional-distribution estimates are based, in part, on the estimates contained in the sketch plan TIS report. Factors include: the location of the site with respect to the Colorado Springs metropolitan area, the planned access system for the site, the street and roadway system serving the site, and the land uses proposed for the site.

When the distribution percentages (from Figure 6) are applied to the new, external trip-generation estimates (from Table 1), the resulting site-generated traffic volumes can be determined. Figures 7 and 8 show the short-term and long-term site-generated traffic volumes, respectively. The short-term site-generated traffic volumes assume only the street network

shown in Figure 2 and the long-term site-generated traffic volumes assume buildout of the area roadway network.

BACKGROUND TRAFFIC VOLUMES

Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation of site-generated traffic volumes. Background traffic (for a specified horizon year) includes the through traffic and the traffic generated by nearby developments (existing and planned, including traffic generated by existing and planned developments within the greater Sterling Ranch overall development) but assumes zero traffic generated by land uses within Copper Chase at Sterling Ranch.

Short-Term Scenario Background Conditions

Figure 9 shows the projected volumes for the short-term Background scenario. This scenario includes traffic to be traffic generated in the short term by buildout of Homestead at Sterling Ranch, Branding Iron at Sterling Ranch, Sterling Ranch Filings 2-4, Homestead North at Sterling Ranch Filings 1-3, the Retreat at TimberRidge Filings 1-3, Sterling Ranch East Filings 1 and 2, and the planned FourSquare at Sterling Ranch East development.

Long-Term Scenario Background Conditions

Figure 10 shows the projected 2043 Background daily traffic volumes on key street segments at the key area intersections. These volumes assume buildout of the area street network, including the completion of Marksheffel Road between Vollmer Road and Black Forest Road, Briargate Parkway between Meridian Road and Black Forest Road, and Sterling Ranch Road between Marksheffel Road and Briargate Parkway.

The 2043 Background traffic volumes are estimates by LSC, based on the traffic projections in the LSC February 10, 2023 Master TIS report. The 2043 Background daily traffic volumes assume buildout of all other land uses within the Sterling Ranch Master Plan.

TOTAL TRAFFIC VOLUMES

Short-Term Scenario Total Conditions

Figure 11 shows the projected volumes for the short-term total scenario. These volumes are the sum of the short-term Background scenario (from Figure 9) plus the short-term site-generated traffic volumes (from Figure 7).

Long-Term Scenario Total Conditions

Figure 12 shows the projected volumes for the 2043 total scenario. These volumes are the sum of the 2043 Background scenario (from Figure 10) plus the long-term site-generated traffic volumes (from Figure 8).

LEVELS OF SERVICE

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections. Table 1 shows the level of service delay ranges.

	Signalized Intersections	Unsignalized Intersections										
	Average Control Delay	Average Control Delay (seconds										
Level of Service	(seconds per vehicle)	per vehicle) ⁽¹⁾										
А	10.0 sec or less	10.0 sec or less										
В	10.1-20.0 sec	10.1-15.0 sec										
С	20.1-35.0 sec	15.1-25.0 sec										
D	35.1-55.0 sec	25.1-35.0 sec										
E	55.1-80.0 sec	35.1-50.0 sec										
F	80.1 sec or more	50.1 sec or more										
(1) For unsignalized inte	rsections, if V/C ratio is grea	ter than 1.0 the level of service is										

Table 2: Intersection Levels of Service Delay Ranges

LOS F, regardless of the projected average control delay per vehicle.

The study area stop-sign-controlled intersections have been analyzed based on the unsignalized-intersection analysis procedures from the *Highway Capacity Manual, 6th Edition* by the Transportation Research Board. The future signalized intersections of Vollmer Road/Marksheffel Road and Sterling Ranch Road /Marksheffel Road have been analyzed using Synchro.

Vollmer Road /Marksheffel Road

Marksheffel Road is planned to be constructed between Vollmer Road and Sterling Ranch Road in the short-term future. The intersection of Marksheffel/Vollmer is initially planned to be stop-sign controlled. Based on the projected short-term total traffic volumes, the westbound left-turn movement is projected to operate at LOS D during the morning peak hour and LOS E during the afternoon peak hour.

By 2043, it was assumed that Marksheffel Road would be constructed west to Briargate Parkway and that the intersection of Vollmer/Marksheffel will be converted to traffic-signal control. The intersection of Vollmer/Marksheffel is projected to operate at an overall LOS C or better during the peak hours as a signalized intersection, based on the projected 2043 total traffic volumes.

Sterling Ranch Road /Marksheffel Road

Marksheffel Road is planned to be constructed between Vollmer Road and Sterling Ranch Road in the short-term future. The intersection of Marksheffel/Sterling Ranch is initially planned to be stop-sign controlled. Based on the projected short-term total traffic volumes, the southbound left-turn movement is projected to operate at LOS C during the morning peak hour and LOS D during the afternoon peak hour.

By 2043, it was assumed that Sterling Ranch would be constructed south of Marksheffel Road and that the intersection of Sterling Ranch/Marksheffel will be converted to traffic-signal control. The intersection of Sterling Ranch/Marksheffel is projected to operate at an overall LOS C or better during the peak hours as a signalized intersection, based on the projected 2043 total traffic volumes.

SIGNAL WARRANT THRESHOLD ANALYSIS – AM AND PM PEAK HOURS

The intersections of Marksheffel/Vollmer and Marksheffel/Sterling Ranch were analyzed to determine if the thresholds for Four-Hour and/or Eight-Hour Vehicular-Volume Traffic-Signal Warrant thresholds would be reached or exceeded, based on the projected short-term traffic volumes.

The off-peak-hour volumes are estimates by LSC based on the peak-hour traffic volumes, 72-hour machine counts conducted by LSC on Vollmer Road in November 2020, and vehicle time-of-day distribution data for single-family homes published by the Institute of Transportation Engineers.

Marksheffel/Vollmer

Table 3 shows the results of the analysis for the intersection of Marksheffel/Vollmer. As shown in Table 3, in the short-term, only one of the hours analyzed is projected to meet the thresholds for an Eight-Hour Vehicular-Volume Traffic-Signal Warrant and none of the hours analyzed are projected to meet the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant. This analysis indicates that traffic-signal warrant(s) will likely **not** be met at the intersection of Marksheffel/Vollmer in the short-term.

Marksheffel/Sterling Ranch

Table 4 shows the results of the analysis for the intersection of Marksheffel/Sterling Ranch. As shown in Table 4, in the short-term, only three of the hours analyzed are projected to meet the

thresholds for an Eight-Hour Vehicular-Volume Traffic-Signal and only one of the hours analyzed are projected to meet the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant. This analysis indicates that traffic-signal warrant(s) will likely **not** be met at the intersection of Marksheffel/Sterling Ranch in the short-term.

SUBDIVISION STREET CLASSIFICATIONS

All of the internal streets within Copper Chase at Sterling Ranch should be classified as Urban Local. Figure 13 shows the recommended street classifications for the internal streets and the streets in the vicinity of the site.

DEVIATON REQUESTS

The following deviation requests to the criteria contained in the El Paso County *Engineering Criteria Manual (ECM)* have been included with this submittal:

- A deviation from the maximum mid-block ped ramp spacing
- A deviation from the number of ped ramps at a "T" intersection
- A deviation from the minimum center-line radius and minimum intersection-spacing standards.
- A deviation request for tangent length between broken back curves & for K-values at stop conditions on a local road

ROADWAY IMPROVEMENTS

Table 6 from the *Sterling Ranch East Rezoning and Preliminary Plan TIS* contained a summary of needed area improvements. Appendix Table 2 is a copy of this table with the improvements needed either prior to or with Copper Chase at Sterling Ranch highlighted. Please see Figure 14 for a map of the key street-segment locations. These recommendations are consistent with the LSC Sketch Plan TIS report.

The following auxiliary lanes shown will be required with Copper Chase at Sterling Ranch development if not completed with Sterling Ranch Filing No. 2. Note: These are shown on the construction plans by JR Engineering for these adjacent roadways and construction is underway

- Marksheffel/Vollmer
 - 155-foot northbound right-turn deceleration lane on Vollmer approaching Marksheffel, plus a160-foot taper
 - 310-foot southbound left-turn lane on Vollmer approaching Marksheffel, plus a 160-foot taper.
 - 425-foot westbound left-turn lane on Marksheffel approaching Vollmer, plus a 200-foot taper
 - 235-foot westbound right-turn deceleration lane on Marksheffel approaching Vollmer, plus a 200-foot taper

- Sterling Ranch/Marksheffel
 - 470-foot eastbound left-turn lane on Marksheffel approaching Sterling Ranch, plus a 200-foot taper
 - A 285-foot southbound left-turn lane on Sterling Ranch approaching Marksheffel, plus a 90-foot reverse-curve taper
 - A 155-foot southbound right-turn lane on Sterling Ranch approaching Marksheffel, plus a 160-foot taper
- Sterling Ranch/Bynum
 - A 155-foot northbound left-turn lane on Sterling Ranch Road approaching Bynum, plus a 90-foot reverse-curve taper

ROADWAY IMPROVEMENT FEE PROGRAM

This project will be required to participate in the El Paso County Road Improvement Fee Program. Copper Chase at Sterling Ranch will join the ten-mil PID. The 2019 ten-mil PID building permit fee portion associated with this option is \$1,221 per single-family dwelling unit. Based on 158 lots, the total building permit fee would be \$168,498. Note: program fees are subject to change.

* * * * *

Please contact me if you have any questions regarding this report.

Respectfully Submitted,

LSC TRANSPORTATION CONSULTANTS, INC.

By: Jeffrey C. Hodsdon, P.E. Principal

JCH/KDF:jas

Enclosures: Tables 1, 3, and 4 Figures 1-14 Trail Map Level of Service Reports Appendix Tables 1-2



				•	Table nch East Pha er Chase at \$	se 1 Prelimin	-						
Trip Generation Rates ⁽¹⁾ Total Trip Generated ITE AM Peak Hour PM Peak Hour AM Peak Hour PM Peak Hour													ak Hour
Code	ITE Land Use	Quantity	Unit	Daily	In	Out	In	Out	Daily	In	Out	In	Out
210	Single-Family Detached Housing	138	DU ⁽²⁾	9.43	0.18	0.52	0.59	0.35	1,301	25	71	82	48
()	e: "Trip Generation, 11th Edition, 2021" b Dwelling Unit	y the Institute of T	ransportatior	n Engineers (I	ITE)								
Source: L	SC Transportation Consultants, Inc.												Apr-23

Table 3 Traffic Signal Warrant Analysis Marksheffel Road/Vollmer Road

								Mor	ant 1: Eigh	t Hour Vol	Warrant 2: Four Hour Vehicular Volume Evaluation							
								Wall	ant i. Eigi	IL HOUL VE	Chart Tarm B							
						Warrant Threshold Met?								Short-Term E	r - T			
	Short-Term Background Traffic			at Sterling Ranch								-Term			Warrant	Warrant	Warrant	Warrant
			Generated Traffic		Short-Term Total Traffic		Warrant Thresholds				Background		Short-Term Total		Threshold	Threshold	Threshold	Threshol
	Major ⁽²⁾	Minor ⁽³⁾	Major	Minor	Major	Minor		ition A	Condi		Conditio		Conditio	Conditio	Minor	Met?	Minor	Met?
Hour	Vollmer	Marksheffel	Vollmer	Marksheffel	Vollmer	Marksheffel	Major	Minor	Major	Minor	n A	n B	n A	nB	Minimum	WB	Minimum	WB
	(4)																	
Short-Term To		<u> </u>		4	10		000	450	000	75				N		N		
12-1 AM	45	2	1	1	46	3	600 600	150 150	900 900	75	No	No	No	No	Low Volume	No	Low Volume	No No
1-2 AM 2-3 AM	23 16	2	1	0	24 17	3	600	150	900	75 75	No No	No No	No No	No No	Low Volume Low Volume	No No	Low Volume	NO NO
2-3 AM 3-4 AM	25	2	1	0	26	3	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
4-5 AM	40	8	1	2	41	10	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
5-6 AM	105	19	1	6	106	25	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
6-7 AM	312	57	7	17	319	74	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
7-8 AM	754	99	13	29	767	128	600	150	900	75	No	No	No	No	223	No	217	No
8-9 AM	841	83	14	24	855	107	600	150	900	75	No	No	No	No	190	No	186	No
9-10 AM	728	52	13	15	741	67	600	150	900	75	No	No	No	No	236	No	230	No
10-11 AM	843	52	18	15	861	67	600	150	900	75	No	No	No	No	189	No	185	No
11-12 PM	947	50	22	15	969	65	600	150	900	75	No	No	No	No	163	No	158	No
12-1 PM	791	49	22	15	813	64	600	150	900	75	No	No	No	No	205	No	197	No
1-2 PM	783	52	23	16	806	68	600	150	900	75	No	No	No	No	209	No	199	No
2-3 PM	901	55	26	17	927	72	600	150	900	75	No	No	No	No	175	No	168	No
3-4 PM	956	53	32	16	988	69	600	150	900	75	No	No	No	No	161	No	153	No
4-5 PM	992	66	40	20	1032	86	600	150	900	75	No	No	No	Yes	152	No	140	No
5-6 PM 6-7 PM	829 565	65 52	40 32	20	869 597	85 68	600 600	150 150	900 900	75 75	No No	No No	No No	No No	193 308	No No	183 292	No No
6-7 PM 7-8 PM	565 353	52 38	32 23	16	597 376		600 600	150	900	75 75	NO NO		NO NO					NO NO
7-8 PM 8-9 PM	353 286	38	23	11 8	376	49 35	600 600	150	900	75 75	NO NO	No No	NO NO	No No	Low Volume Low Volume	No No	Low Volume	NO NO
9-10 PM	183	21	17	0 6	200	27	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	NO
10-11 PM	103	10	8	3	111	13	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
11-12 AM	54	6	5	2	59	8	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
=			-			Numbers		the Warran		-	0	0	0	1		0		0
										rant Met?	N	-	N N		1	No		No

Notes:

(1) Thresholds are based on 2 or more lanes on the major approach and 1 lane on the minor approach (Warrant evaluation assuming the westbound left turn only for the minor street)
(2) The major street traffic includes all movements (left, through, and right)
(3) The minor street traffic includes only the left turns from the minor street

(5) Off peak hour traffic volumes are based on the projected peak hour traffic volumes, 72-hour machine counts conducted on Vollmer Road in November 2020 and vehicle time-of-day distribution data for single-family residential published by the Institute of Transportation Engineers

Source: LSC Transportation Consultants, Inc.

Apr-23

Table 4 Traffic Signal Warrant Analysis Marksheffel Road/Sterling Ranch Road

												Warr	ant Analys	IS	r			
								Warr	ant 1: Eigl	Warrant 2: Four Hour Vehicular Volume Evaluation								
								Tun	unt n. Eigi		Short-Term Background Short-Ter							
Short-Term Copper Chase at Sterling Ranch										t?	Short-Term	ackground						
				•			Short-Term											
	Background Traffic		Generated Traffic		Short-Term Total Traffic		Warrant Thresholds				Background		Short-Term Tota		Warrant	Warrant	Warrant	Warra
		Minor ⁽³⁾				Minor	0		0	141 a.u. D	Threshold	Threshold	Threshold	Thresh
	Major ⁽²⁾	Sterling	Major	Minor Sterling	Major	Sterling		ition A		ition B		Conditio	Conditio		-	Met? WB	Minor	Met?
Hour	Marksheffel	Ranch	Marksheffel	Ranch	Marksheffel	Ranch	Major	Minor	Major	Minor	n A	n B	n A	n B	Minimum	WB	Minimum	WB
	otal Traffic ⁽⁴⁾			4	0.1	-	000	450	000	75				N				
12-1 AM	28 13	6	3	1	31 14	7	600 600	150 150	900 900	75 75	No	No	No No	No	Low Volume	No	Low Volume	No No
1-2 AM 2-3 AM	13	6	1	0		0	600	150	900	-	No	No		No	Low Volume	No	Low Volume	No
2-3 AM 3-4 AM	11	0	1	0	12 15	7	600	150	900	75 75	No No	No No	No No	No No	Low Volume	No No	Low Volume	NO
4-5 AM	21	23	1	3	22	26	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
5-6 AM	49	58	3	7	52	65	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
6-7 AM	147	171	8	21	155	192	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
7-8 AM	335	295	16	36	351	331	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
8-9 AM	377	249	19	30	396	279	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
9-10 AM	326	156	16	19	342	175	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
10-11 AM	389	156	21	19	410	175	600	150	900	75	No	No	No	No	Low Volume	No	385	No
11-12 PM	455	148	27	18	482	166	600	150	900	75	No	No	No	No	363	No	349	No
12-1 PM	460	148	28	19	488	167	600	150	900	75	No	No	No	No	360	No	346	No
1-2 PM	476	156	31	20	507	176	600	150	900	75	No	No	No	No	Low Volume	No	337	No
2-3 PM	550	164	36	21	586	185	600	150	900	75	No	No	No	No	315	No	297	No
3-4 PM	620	158	44	20	664	178	600	150	900	75	Yes	No	Yes	No	282	No	264	No
4-5 PM	702	198	55	25	757	223	600	150	900	75	Yes	No	Yes	No	249	No	222	Yes
5-6 PM	640	195	54	25	694	220	600	150	900	75	Yes	No	Yes	No	274	No	252	No
6-7 PM	490	156	45	20	535	176	600	150	900	75	No	No	No	No	Low Volume	No	323	No
7-8 PM	335	114	32	14	367	128	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
8-9 PM 9-10 PM	316 218	82 63	33 24	10 8	349 242	92 71	600 600	150 150	900 900	75 75	No No	No No	No No	No No	Low Volume	No No	Low Volume	No No
10-11 PM	218 112	29	12	8	124	33	600	150	900	75	NO	NO	NO	NO		NO NO	Low Volume	NO
10-11 PM 11-12 AM	64	29 18	12	4	71	20	600	150	900	75	NO	No	No	NO	Low Volume Low Volume	NO	Low Volume	NO
	- 04	10	1	2	71	-		the Warran		-		0	3	0		0		1
						Number	SUINDURSI	ine warran		rrant Met?	-	v	v	0	4	No		No

Notes:

(1) Thresholds are based on 2 or more lanes on the major approach and 1 lane on the minor approach (Warrant evaluation assuming the southbound left turn only for the minor street)

(2) The major street traffic includes all movements (left, through, and right)

(3) The minor street traffic includes only the left turns from the minor street

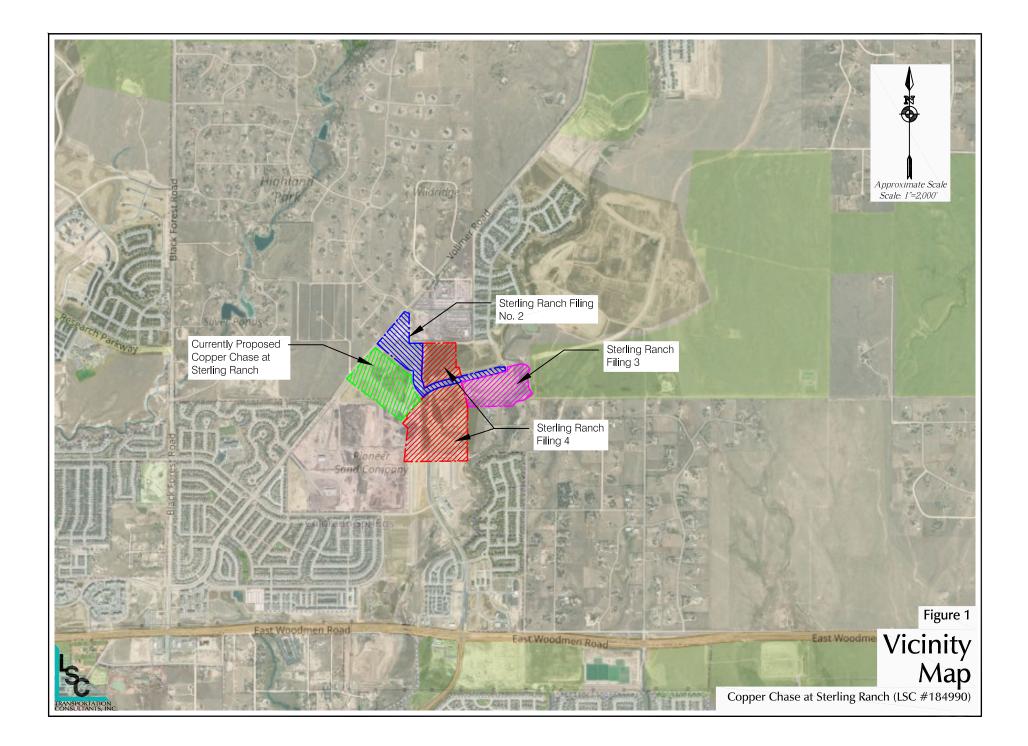
(4) Off peak hour traffic volumes are based on the projected peak hour traffic volumes, 72-hour machine counts conducted on Vollmer Road in November 2020 and vehicle time-of-day distribution data for single-family residential

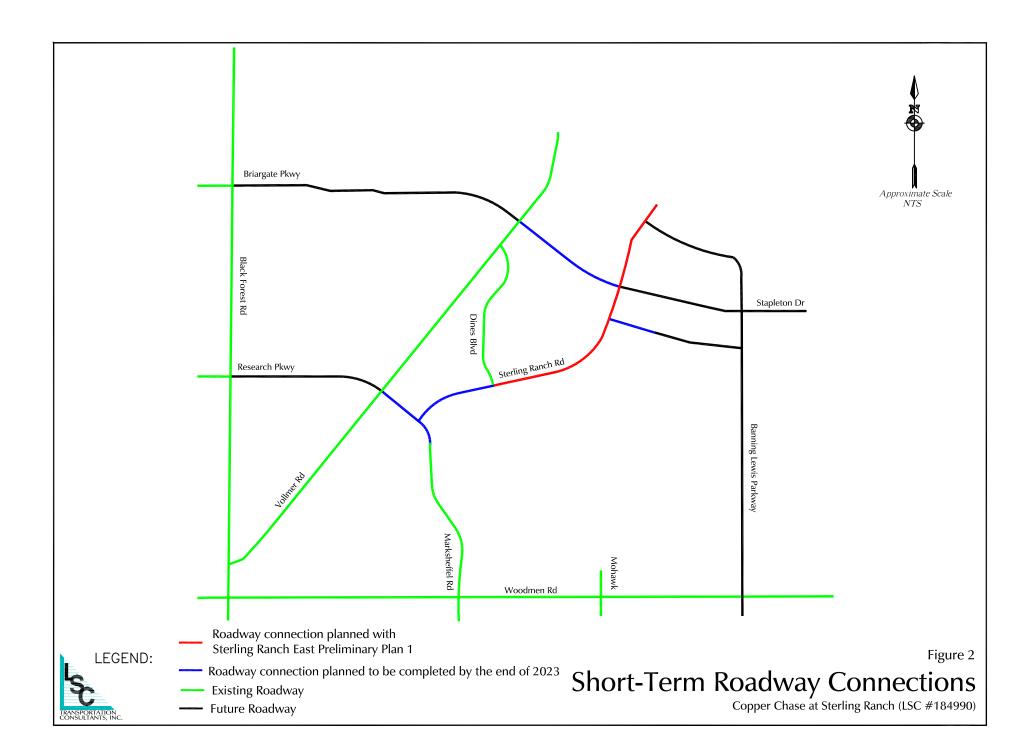
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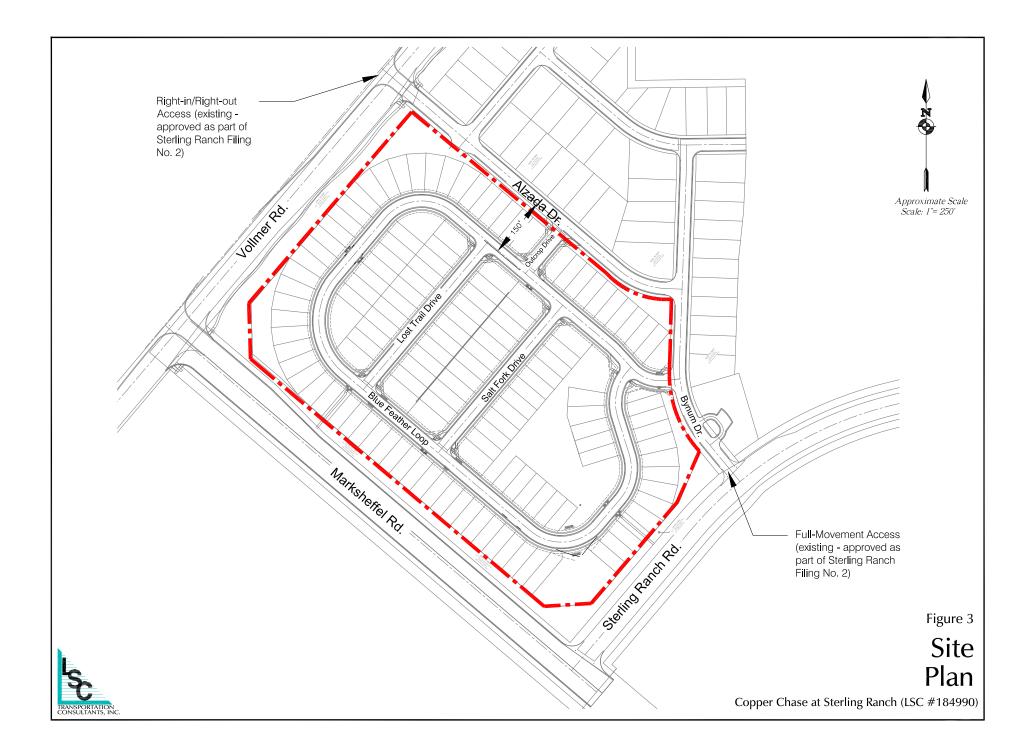
Source: LSC Transportation Consultants, Inc.

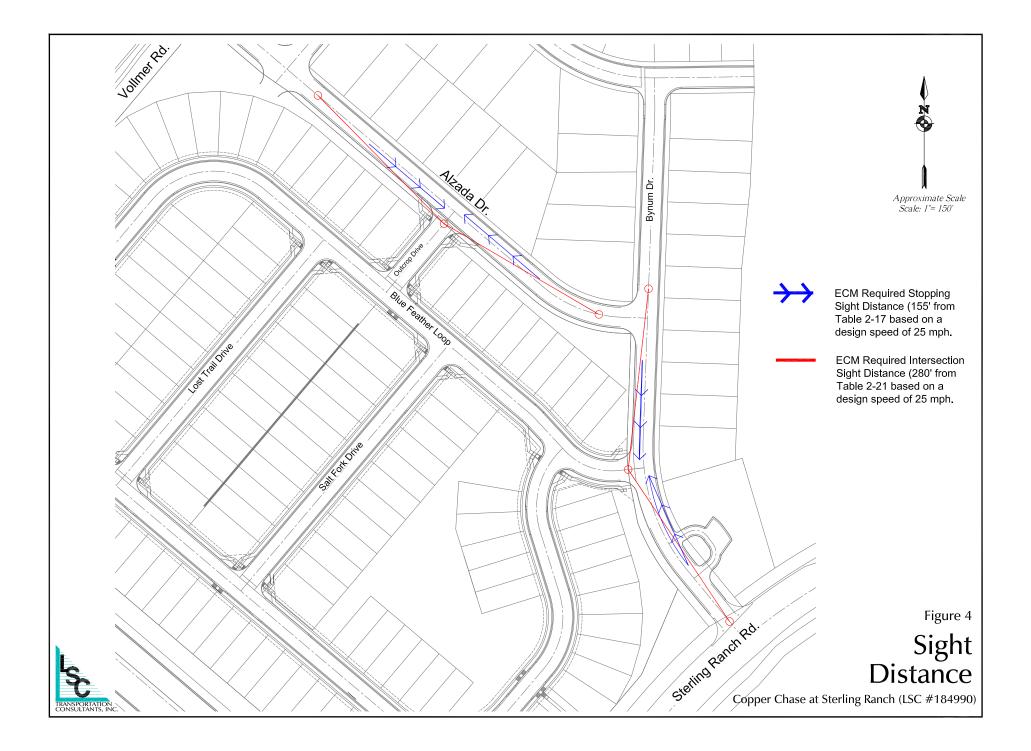
Apr-23

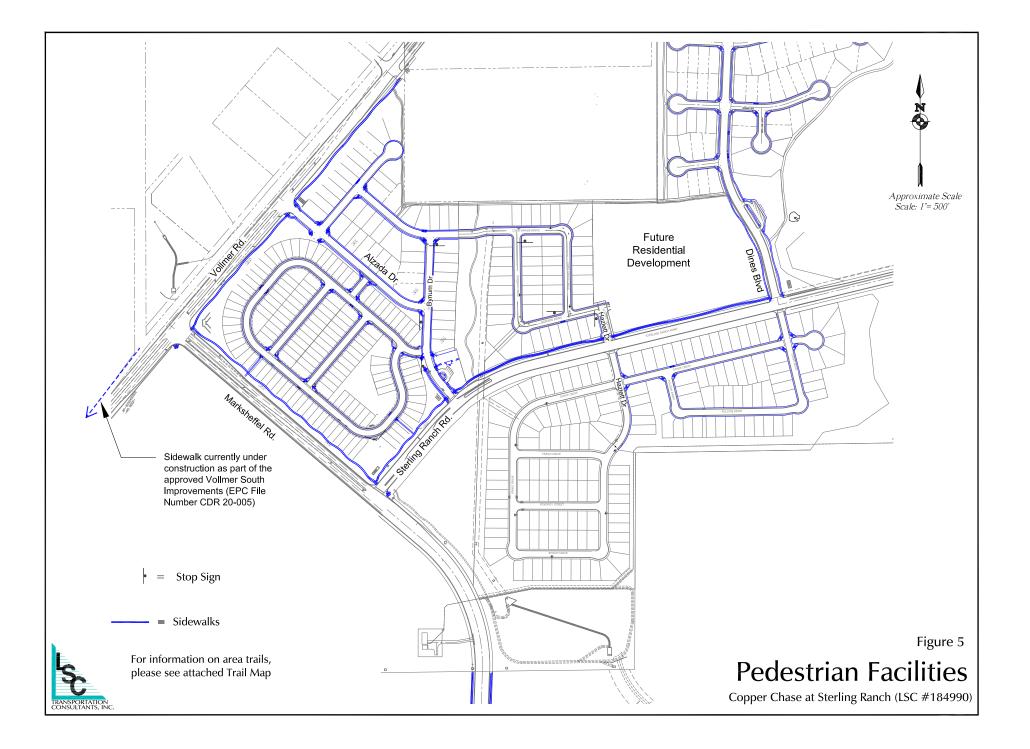


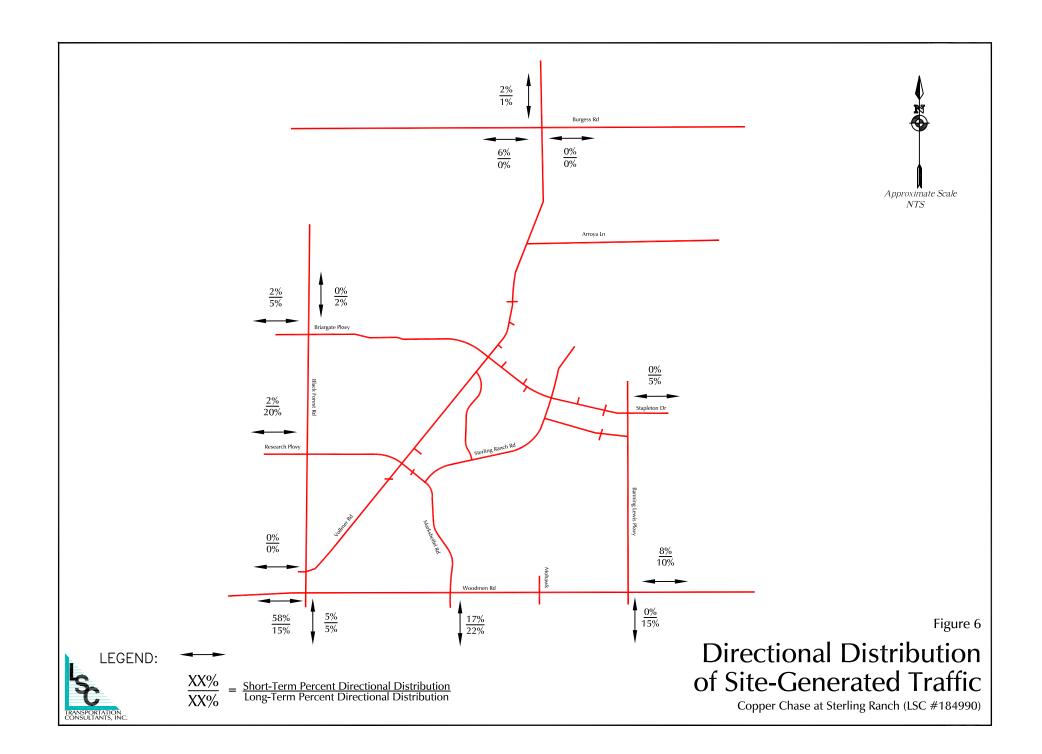


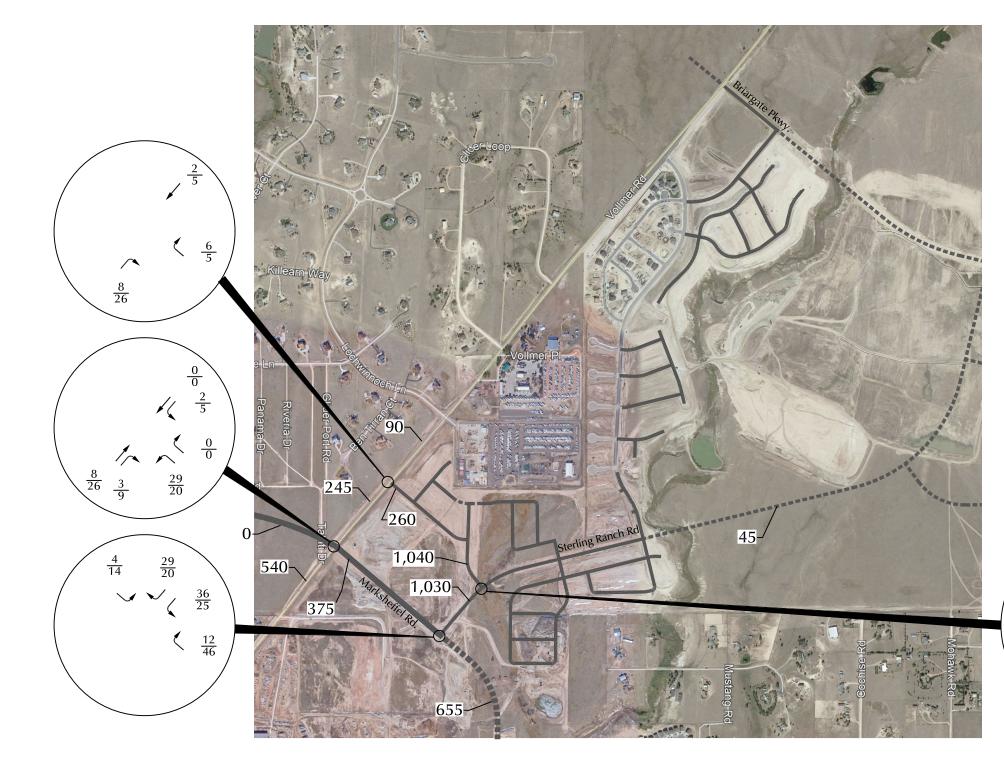












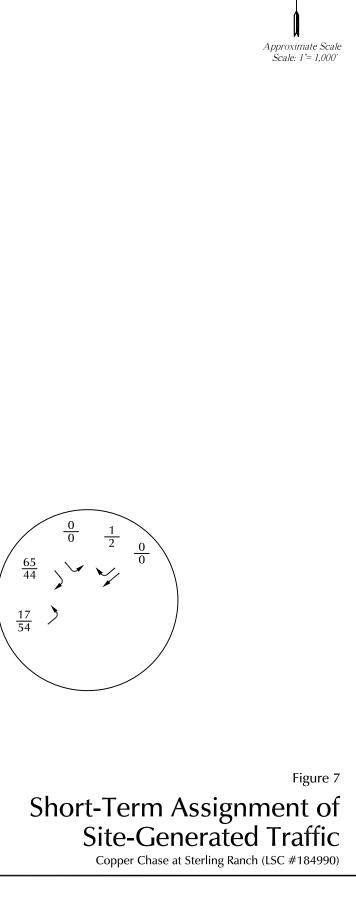
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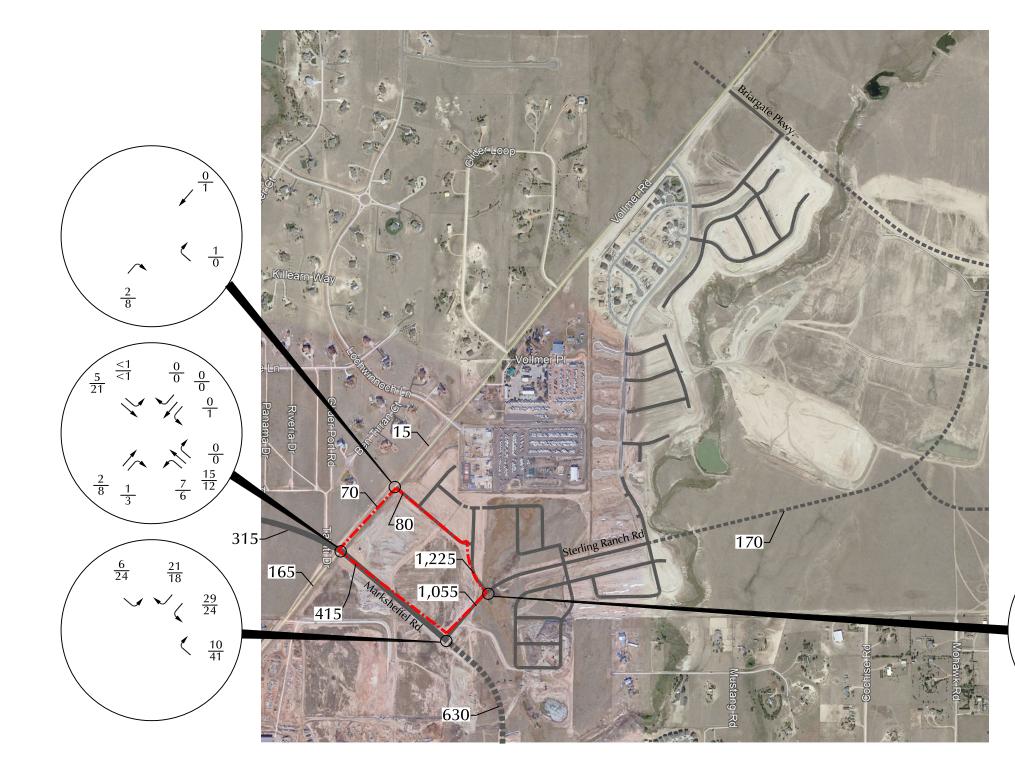


AM Weekday Peak-Hour Traffic (vehicles per hour) PM Weekday Peak-Hour Traffic (vehicles per hour)

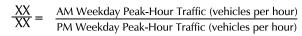
X,XXX= Average Daily Traffic (vehicles per day)

TRANSPORTATION CONSULTANTS, IN



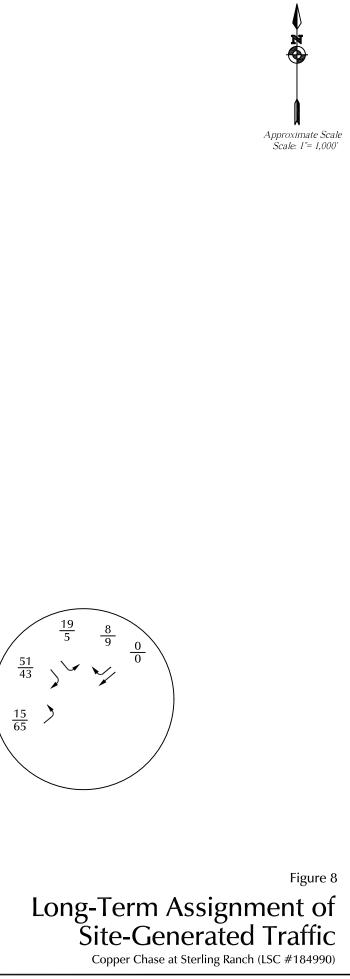


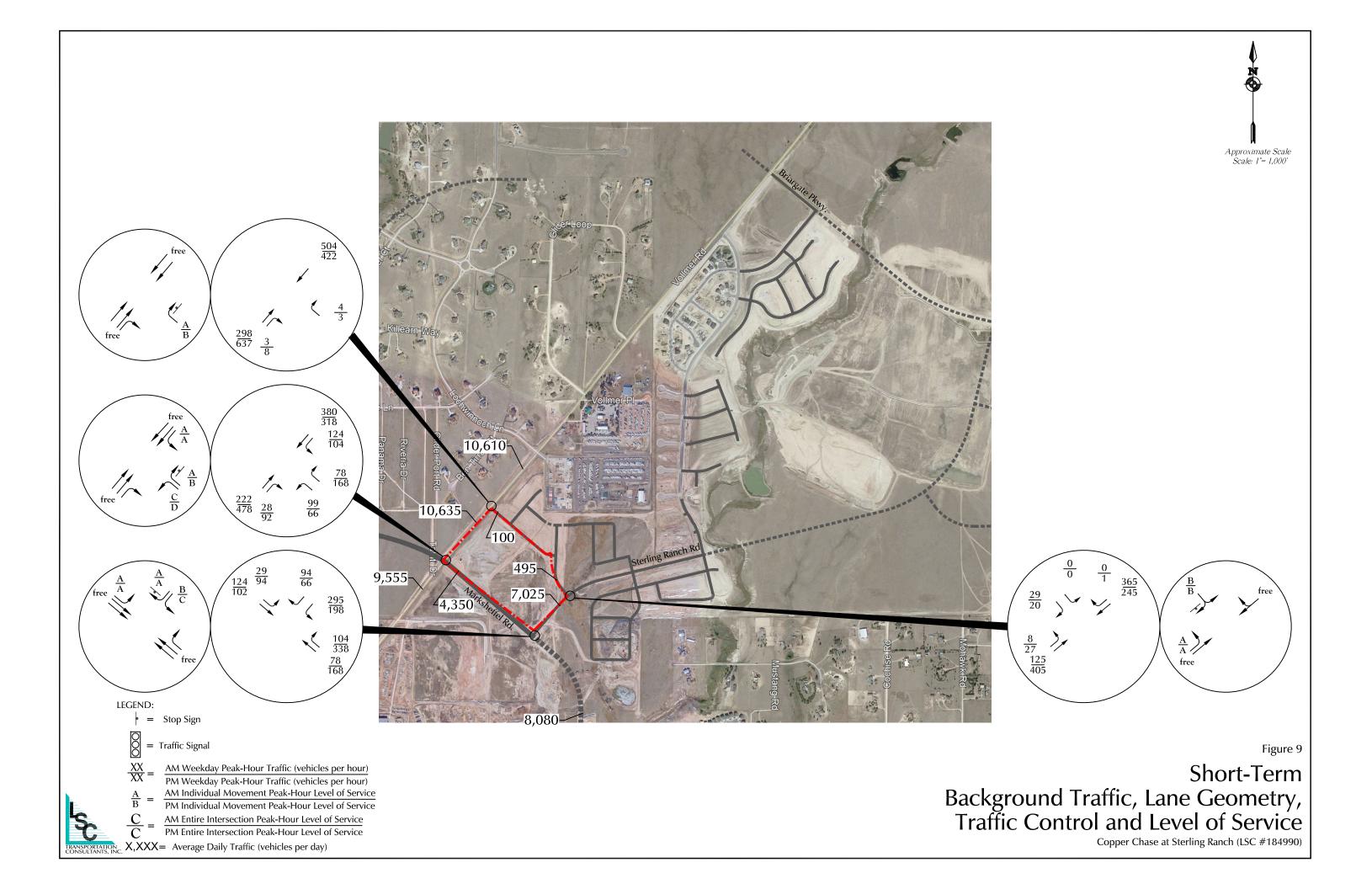
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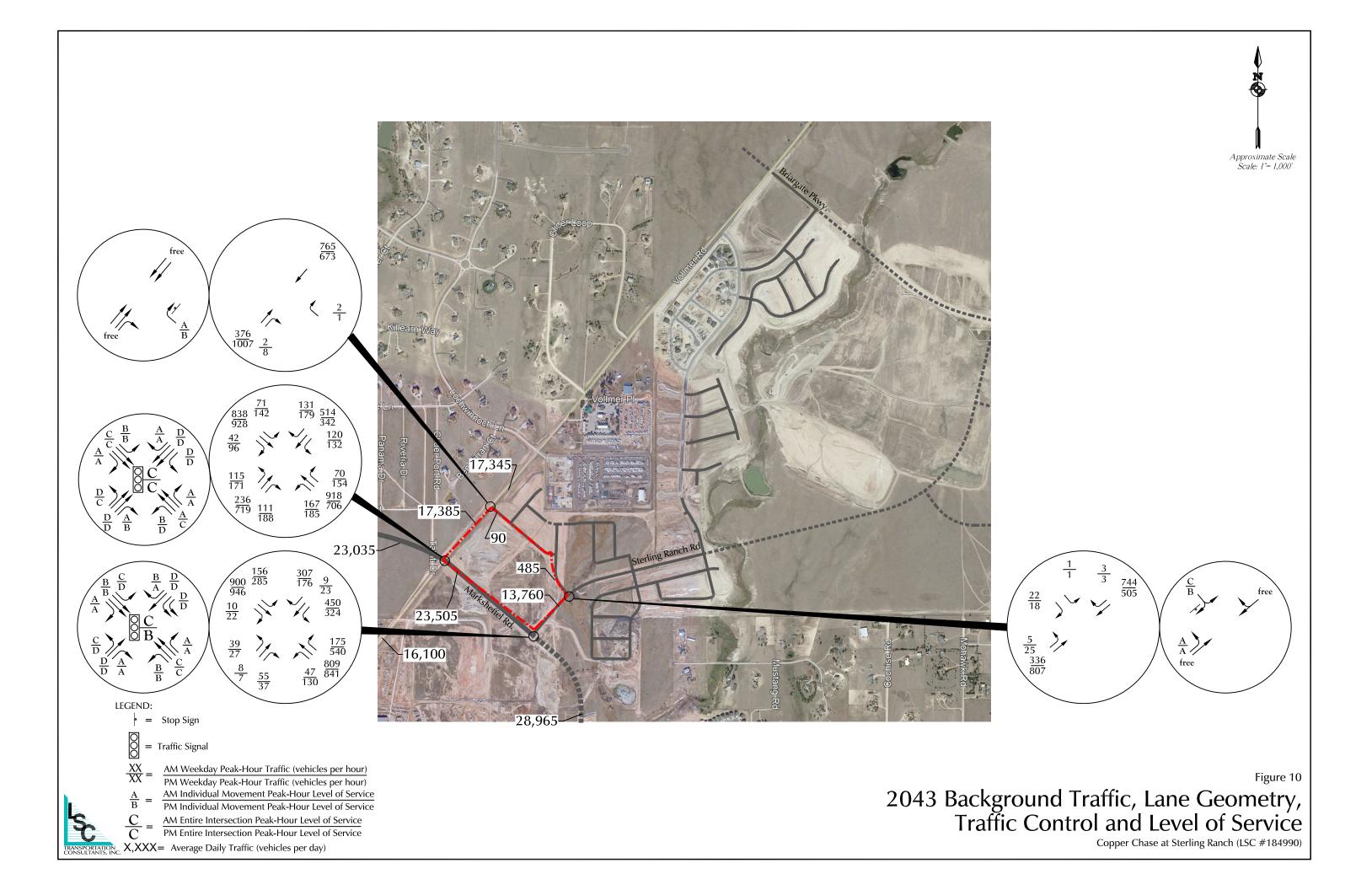


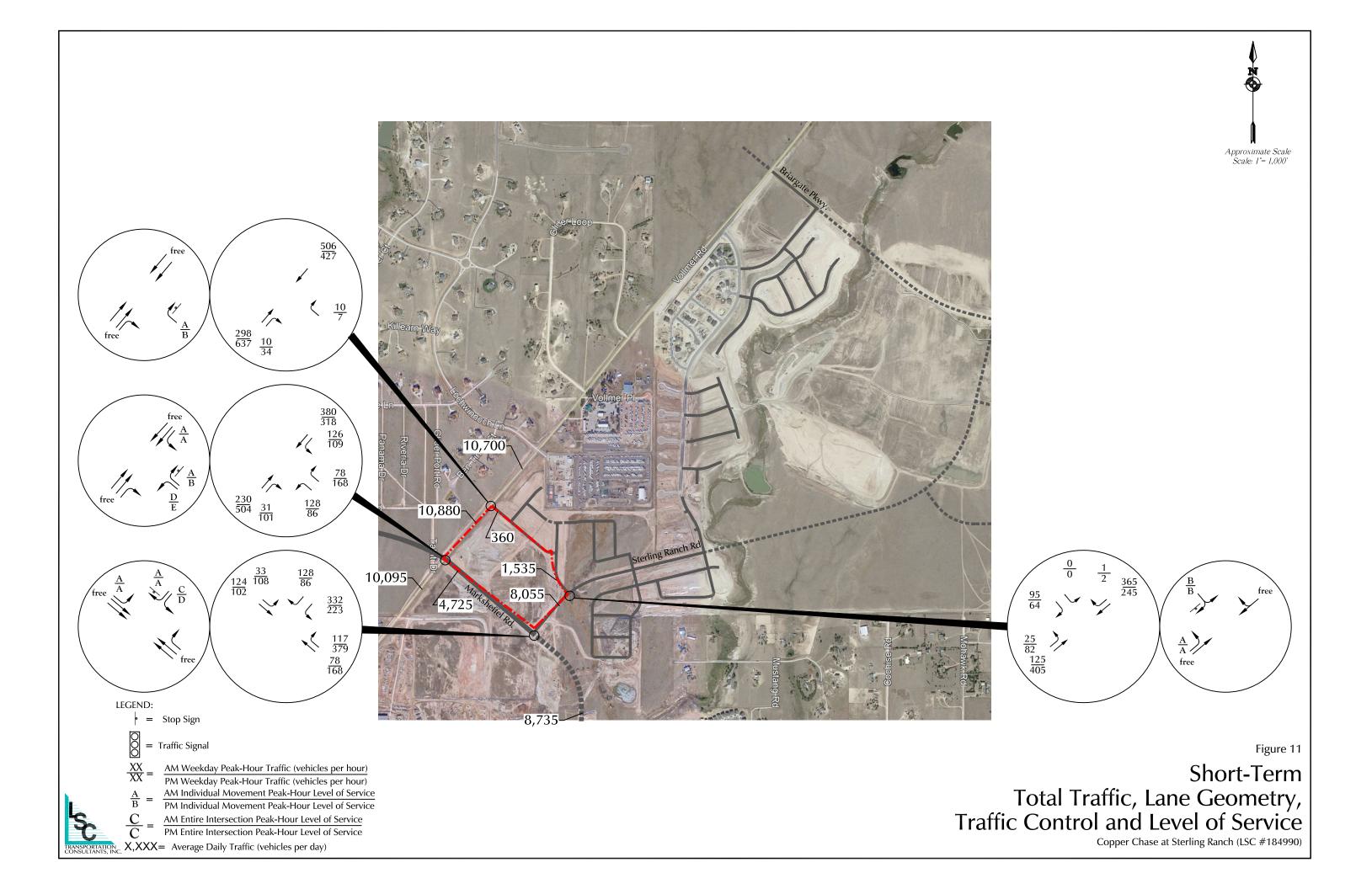
X,XXX= Average Daily Traffic (vehicles per day)

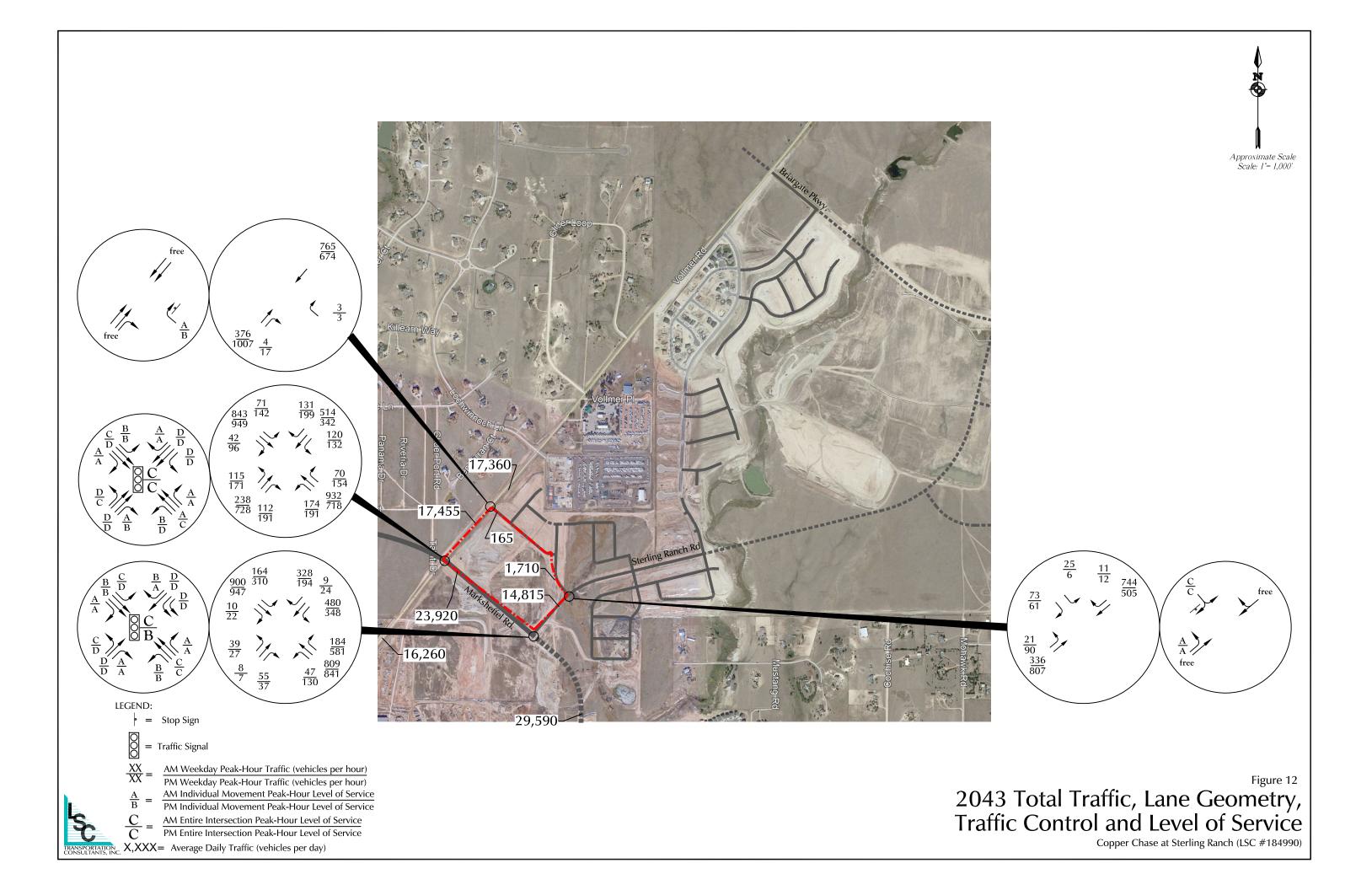
TRANSPORTATION TRANSPORTATION

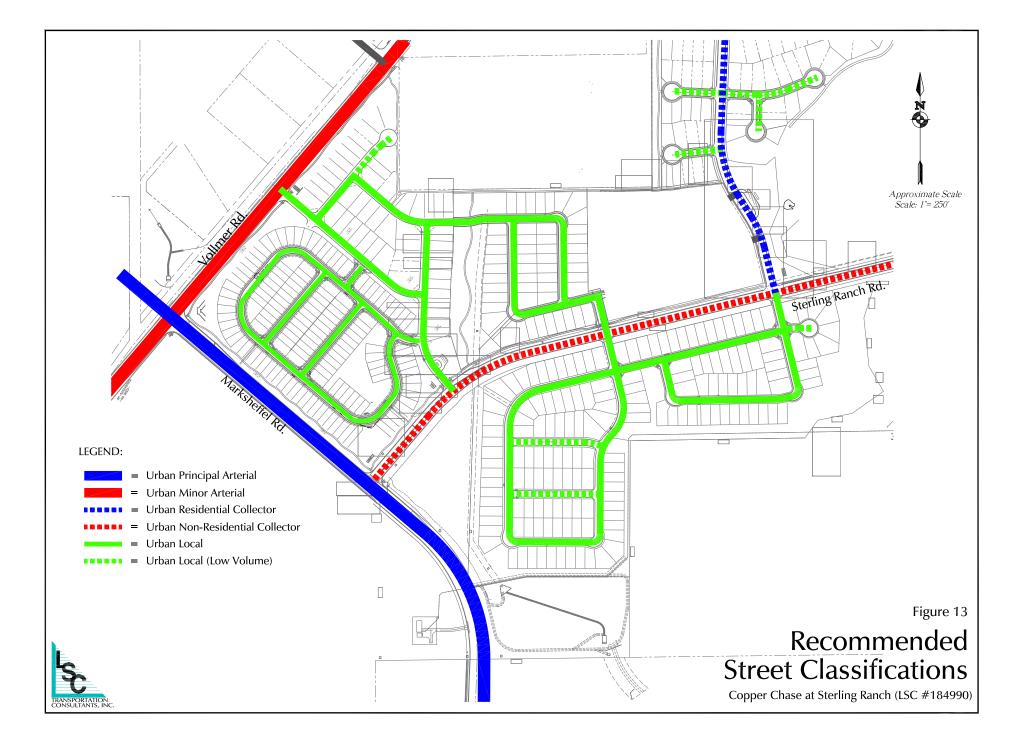


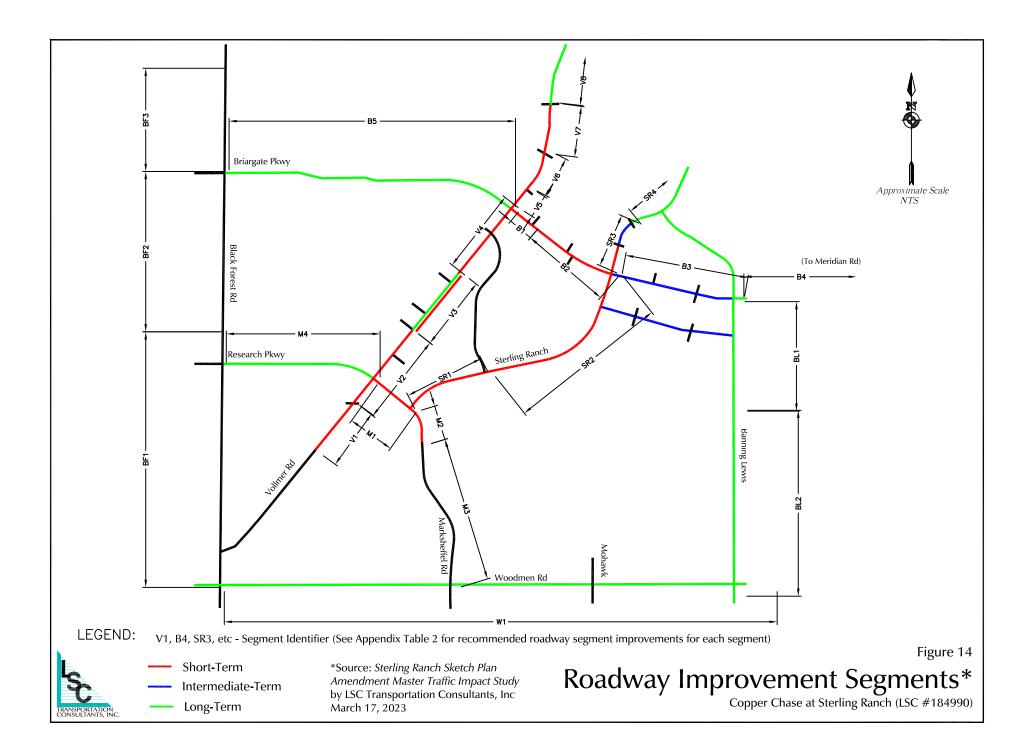


















Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	- 11	1	٦	^
Traffic Vol, veh/h	99	78	222	28	124	380
Future Vol, veh/h	99	78	222	28	124	380
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	155	300	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	116	92	261	33	146	447

Major/Minor	Minor1	М	ajor1	Ν	/lajor2					
Conflicting Flow All	777	131	0	0	294	0				
Stage 1	261	-	-	-	-	-				
Stage 2	516	-	-	-	-	-				
Critical Hdwy	6.84	6.94	-	-	4.14	-				
Critical Hdwy Stg 1	5.84	-	-	-	-	-				
Critical Hdwy Stg 2	5.84	-	-	-	-	-				
Follow-up Hdwy	3.52	3.32	-	-	2.22	-				
Pot Cap-1 Maneuver	334	894	-	-	1264	-				
Stage 1	759	-	-	-	-	-				
Stage 2	564	-	-	-	-	-				
Platoon blocked, %			-	-		-				
Mov Cap-1 Maneuver	r 295	894	-	-	1264	-				
Mov Cap-2 Maneuver	r 295	-	-	-	-	-				
Stage 1	759	-	-	-	-	-				
Stage 2	499	-	-	-	-	-				

Approach	WB	NB	SB
HCM Control Delay, s	18.1	0	2
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	295	894	1264	-	
HCM Lane V/C Ratio	-	-	0.395	0.103	0.115	-	
HCM Control Delay (s)	-	-	24.9	9.5	8.2	-	
HCM Lane LOS	-	-	С	А	А	-	
HCM 95th %tile Q(veh)	-	-	1.8	0.3	0.4	-	

Int Delay, s/veh	7.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	٦	^	- 11	1	٦	1	
Traffic Vol, veh/h	29	124	78	104	295	99)
Future Vol, veh/h	29	124	78	104	295	99	
Conflicting Peds, #/hr	0	0	0	0	0	0)
Sign Control	Free	Free	Free	Free	Stop	Stop)
RT Channelized	-	None	-	None	-	None	ł
Storage Length	300	-	-	205	155	0	
Veh in Median Storage	, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	;
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	34	146	92	122	347	116	j

Major/Minor	Major1	Ma	ajor2	N	1inor2	
Conflicting Flow All	214	0	-	0	233	46
Stage 1	-	-	-	-	92	-
Stage 2	-	-	-	-	141	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1353	-	-	-	735	1014
Stage 1	-	-	-	-	921	-
Stage 2	-	-	-	-	871	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	r 1353	-	-	-	717	1014
Mov Cap-2 Maneuver	r -	-	-	-	717	-
Stage 1	-	-	-	-	898	-
Stage 2	-	-	-	-	871	-
Approach	EB		WB		SB	
LIOM Constral Delay			0		40.0	_

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1	SBLn2
Capacity (veh/h)	1353	-	-	- 717	1014
HCM Lane V/C Ratio	0.025	-	-	- 0.484	0.115
HCM Control Delay (s)	7.7	-	-	- 14.6	9
HCM Lane LOS	А	-	-	- B	А
HCM 95th %tile Q(veh)	0.1	-	-	- 2.7	0.4

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		^
Traffic Vol, veh/h	0	4	298	3	0	504
Future Vol, veh/h	0	4	298	3	0	504
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	351	4	0	593

Major/Minor	Minor1	Μ	ajor1	Ma	jor2	
Conflicting Flow All	-	176	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	837	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		837	-	-	-	-
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	9.3	0	0
HCM LOS	А		

Minor Lane/Major Mvmt	NBT	NBRW	VBLn1	SBT
Capacity (veh/h)	-	-	837	-
HCM Lane V/C Ratio	-	-	0.006	-
HCM Control Delay (s)	-	-	9.3	-
HCM Lane LOS	-	-	А	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	0.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	l
Lane Configurations	٦	1	et -		Y		
Traffic Vol, veh/h	8	125	365	0	0	29)
Future Vol, veh/h	8	125	365	0	0	29)
Conflicting Peds, #/hr	0	0	0	0	0	0)
Sign Control	Free	Free	Free	Free	Stop	Stop)
RT Channelized	-	None	-	None	-	None	;
Storage Length	150	-	-	-	0	-	•
Veh in Median Storage	, # -	0	0	-	0	-	-
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	;
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	9	147	429	0	0	34	ļ

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	429	0	-	0	594	429
Stage 1	-	-	-	-	429	-
Stage 2	-	-	-	-	165	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1130	-	-	-	468	626
Stage 1	-	-	-	-	657	-
Stage 2	-	-	-	-	864	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	464	626
Mov Cap-2 Maneuver	-	-	-	-	464	-
Stage 1	-	-	-	-	652	-
Stage 2	-	-	-	-	864	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		11.1	
HCM LOS					В	
Minor Lane/Major Mvn	at	EBL	EBT	WBT	WBR	
	ni –		EDI	VVDI		
Capacity (veh/h)		1130	-	-	-	626
HCM Lane V/C Ratio	`	0.008	-	-		0.055
HCM Control Delay (s) HCM Lane LOS)	8.2	-	-	-	11.1
		A 0	-	-	-	B 0.2
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Int Delay, s/veh	4.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	-
Lane Configurations	٦	1	- 11	1	٦	^	
Traffic Vol, veh/h	66	168	478	92	104	318	
Future Vol, veh/h	66	168	478	92	104	318	}
Conflicting Peds, #/hr	0	0	0	0	0	0)
Sign Control	Stop	Stop	Free	Free	Free	Free	;
RT Channelized	-	None	-	None	-	None	;
Storage Length	300	-	-	155	300	-	-
Veh in Median Storage	,# 0	-	0	-	-	0)
Grade, %	0	-	0	-	-	0)
Peak Hour Factor	85	85	85	85	85	85	5
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	78	198	562	108	122	374	ŀ

Major/Minor	Minor1	Μ	lajor1	Ν	1ajor2	
Conflicting Flow All	993	281	0	0	670	0
Stage 1	562	-	-	-	-	-
Stage 2	431	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	242	716	-	-	916	-
Stage 1	534	-	-	-	-	-
Stage 2	623	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	r 210	716	-	-	916	-
Mov Cap-2 Maneuve	r 210	-	-	-	-	-
Stage 1	534	-	-	-	-	-
Stage 2	540	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	17.5	0	2.3
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1V	VBLn2	SBL	SBT
Capacity (veh/h)	-	-	210	716	916	-
HCM Lane V/C Ratio	-	-	0.37	0.276	0.134	-
HCM Control Delay (s)	-	-	31.9	11.9	9.5	-
HCM Lane LOS	-	-	D	В	А	-
HCM 95th %tile Q(veh)	-	-	1.6	1.1	0.5	-

Int Delay, s/veh	5.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	^	- 11	1	٦	1
Traffic Vol, veh/h	94	102	168	338	198	66
Future Vol, veh/h	94	102	168	338	198	66
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	205	155	0
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	111	120	198	398	233	78

Major/Minor	Major1	Ма	ajor2	Ν	linor2	
Conflicting Flow All	596	0	-	0	480	99
Stage 1	-	-	-	-	198	-
Stage 2	-	-	-	-	282	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	r 976	-	-	-	515	937
Stage 1	-	-	-	-	816	-
Stage 2	-	-	-	-	741	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve	er 976	-	-	-	456	937
Mov Cap-2 Maneuve	er -	-	-	-	456	-
Stage 1	-	-	-	-	723	-
Stage 2	-	-	-	-	741	-
Approach	EB		WB		SB	
HCM Control Delay,			0	_	18	
HCM LOS	3 4.4		0		C	
					U	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1	SBLn2
Capacity (veh/h)	976	-	-	- 456	937
HCM Lane V/C Ratio	0.113	-	-	- 0.511	0.083
HCM Control Delay (s)	9.2	-	-	- 20.9	9.2
HCM Lane LOS	А	-	-	- C	Α
HCM 95th %tile Q(veh)	0.4	-	-	- 2.8	0.3

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		- † †
Traffic Vol, veh/h	0	3	637	8	0	422
Future Vol, veh/h	0	3	637	8	0	422
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	4	749	9	0	496

Major/Minor	Minor1	М	ajor1	Ма	ijor2	
Conflicting Flow All	-	375	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	623	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuve		623	-	-	-	-
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
• •	14/5				~ ~	

Approach	WB	NB	SB	
HCM Control Delay, s	10.8	0	0	
HCM LOS	В			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBT
Capacity (veh/h)	-	-	623	-
HCM Lane V/C Ratio	-	-	0.006	-
HCM Control Delay (s)	-	-	10.8	-
HCM Lane LOS	-	-	В	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u>ک</u>	1	et –		Y	
Traffic Vol, veh/h	27	405	245	1	0	20
Future Vol, veh/h	27	405	245	1	0	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	32	476	288	1	0	24

Major/Minor	Major1	Ν	1ajor2	1	Minor2	
Conflicting Flow All	289	0	-	0	829	289
Stage 1	-	-	-	-	289	-
Stage 2	-	-	-	-	540	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1273	-	-	-	340	750
Stage 1	-	-	-	-	760	-
Stage 2	-	-	-	-	584	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	332	750
Mov Cap-2 Maneuver	r -	-	-	-	332	-
Stage 1	-	-	-	-	741	-
Stage 2	-	-	-	-	584	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.5		0		10	
HCM LOS					В	
Minor Lane/Major Mvr	mt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1273	-	-	-	750
HCM Lane V/C Ratio		0.025	-	-	-	0.031
HCM Control Delay (s	5)	7.9	-	-	-	10
HCM Lane LOS		А	-	-	-	В
HCM 95th %tile Q(vel	h)	0.1	-	-	-	0.1

Int Delay, s/veh	5.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	- 11	1	٦	- † †
Traffic Vol, veh/h	128	78	230	31	126	380
Future Vol, veh/h	128	78	230	31	126	380
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	155	300	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	151	92	271	36	148	447

Minor1	Μ	ajor1	Ν	lajor2	
791	136	0	0	307	0
271	-	-	-	-	-
520	-	-	-	-	-
6.84	6.94	-	-	4.14	-
5.84	-	-	-	-	-
5.84	-	-	-	-	-
3.52	3.32	-	-	2.22	-
327	888	-	-	1250	-
750	-	-	-	-	-
561	-	-	-	-	-
		-	-		-
288	888	-	-	1250	-
288	-	-	-	-	-
750	-	-	-	-	-
495	-	-	-	-	-
	791 271 520 6.84 5.84 3.52 327 750 561 288 288 288 750	791 136 271 - 520 - 6.84 6.94 5.84 - 3.52 3.32 327 888 750 - 561 - 288 888 288 - 750 -	791 136 0 271 - - 520 - - 6.84 6.94 - 5.84 - - 5.84 - - 3.52 3.32 - 327 888 - 750 - - 288 888 - 288 - - 750 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - <t< td=""><td>$\begin{array}{c ccccccccccccccccccccccccccccccccccc$</td><td>$\begin{array}{c ccccccccccccccccccccccccccccccccccc$</td></t<>	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

Approach	WB	NB	SB
HCM Control Delay, s	22.5	0	2.1
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRW	BLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	288	888	1250	-	
HCM Lane V/C Ratio	-	- (0.523	0.103	0.119	-	
HCM Control Delay (s)	-	-	30.4	9.5	8.3	-	
HCM Lane LOS	-	-	D	А	Α	-	
HCM 95th %tile Q(veh)	-	-	2.8	0.3	0.4	-	

Int Delay, s/veh	8.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ľ	^	- 11	1	٦	1
Traffic Vol, veh/h	33	124	78	117	332	128
Future Vol, veh/h	33	124	78	117	332	128
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	205	155	0
Veh in Median Storage,	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	146	92	138	391	151

Major/Minor	Major1	Ма	ajor2	Ν	linor2	
Conflicting Flow All	230	0	-	0	243	46
Stage 1	-	-	-	-	92	-
Stage 2	-	-	-	-	151	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1335	-	-	-	724	1014
Stage 1	-	-	-	-	921	-
Stage 2	-	-	-	-	861	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1335	-	-	-	703	1014
Mov Cap-2 Maneuver	-	-	-	-	703	-
Stage 1	-	-	-	-	894	-
Stage 2	-	-	-	-	861	-
Approach	EB		WB		SB	
HCM Control Delay s			0		14.3	

now control Dolay, 5	1.0	v	14.0	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1	SBLn2
Capacity (veh/h)	1335	-	-	- 703	1014
HCM Lane V/C Ratio	0.029	-	-	- 0.556	0.149
HCM Control Delay (s)	7.8	-	-	- 16.3	9.2
HCM Lane LOS	А	-	-	- C	А
HCM 95th %tile Q(veh)	0.1	-	-	- 3.4	0.5

Int Delay, s/veh	0.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	-
Lane Configurations		1	^	1		- † †	•
Traffic Vol, veh/h	0	10	298	10	0	506	
Future Vol, veh/h	0	10	298	10	0	506	;
Conflicting Peds, #/hr	0	0	0	0	0	0)
Sign Control	Stop	Stop	Free	Free	Free	Free	,
RT Channelized	-	None	-	None	-	None	ļ
Storage Length	-	0	-	200	-	-	-
Veh in Median Storage,	, # 0	-	0	-	-	0)
Grade, %	0	-	0	-	-	0)
Peak Hour Factor	85	85	85	85	85	85	;
Heavy Vehicles, %	2	2	2	2	2	2)
Mvmt Flow	0	12	351	12	0	595	;

Major/Minor	Minor1	М	ajor1	Ma	ajor2	
Conflicting Flow All	-	176	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	837	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	· -	837	-	-	-	-
Mov Cap-2 Maneuver	· -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annroach	W/R		NR		SB	

Approach	WB	NB	SB	
HCM Control Delay, s	9.4	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBT
Capacity (veh/h)	-	-	837	-
HCM Lane V/C Ratio	-	-	0.014	-
HCM Control Delay (s)	-	-	9.4	-
HCM Lane LOS	-	-	Α	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	2.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et		Y	
Traffic Vol, veh/h	25	125	365	1	0	95
Future Vol, veh/h	25	125	365	1	0	95
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	29	147	429	1	0	112

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	430	0	· -	0	635	430
Stage 1	-	-	-	-	430	-
Stage 2	-	-	-	-	205	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1129	-	-	-	443	625
Stage 1	-	-	-	-	656	-
Stage 2	-	-	-	-	829	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	431	625
Mov Cap-2 Maneuver	-	-	-	-	431	-
Stage 1	-	-	-	-	639	-
Stage 2	-	-	-	-	829	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.4		0		12	
HCM LOS					В	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1129	-	-	-	625
HCM Lane V/C Ratio		0.026	-	-	-	0.179
HCM Control Delay (s))	8.3	-	-	-	12
HCM Lane LOS		А	-	-	-	В

Int Delay, s/veh	5.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	- 11	1	٦	^
Traffic Vol, veh/h	86	168	504	101	109	318
Future Vol, veh/h	86	168	504	101	109	318
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	155	300	-
Veh in Median Storage	,#0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	101	198	593	119	128	374

Major/Minor	Minor1	Μ	ajor1	N	lajor2	
Conflicting Flow All	1036	297	0	0	712	0
Stage 1	593	-	-	-	-	-
Stage 2	443	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	227	699	-	-	884	-
Stage 1	515	-	-	-	-	-
Stage 2	614	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	194	699	-	-	884	-
Mov Cap-2 Maneuver	194	-	-	-	-	-
Stage 1	515	-	-	-	-	-
Stage 2	525	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	22.4	0	2.5
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRWBLr	1WBLn2	SBL	SBT	
Capacity (veh/h)	-	- 19	4 699	884	-	
HCM Lane V/C Ratio	-	- 0.52	2 0.283	0.145	-	
HCM Control Delay (s)	-	- 42	2 12.2	9.8	-	
HCM Lane LOS	-	-	E B	А	-	
HCM 95th %tile Q(veh)	-	- 2	7 1.2	0.5	-	

Int Delay, s/veh	7.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	- 11	- 11	1	٦	1
Traffic Vol, veh/h	108	102	168	379	223	86
Future Vol, veh/h	108	102	168	379	223	86
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	205	155	0
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	127	120	198	446	262	101

Major/Minor	Major1	Ma	ajor2	Ν	linor2	
Conflicting Flow All	644	0	-	0	512	99
Stage 1	-	-	-	-	198	-
Stage 2	-	-	-	-	314	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	937	-	-	-	491	937
Stage 1	-	-	-	-	816	-
Stage 2	-	-	-	-	714	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve	r 937	-	-	-	424	937
Mov Cap-2 Maneuve	r -	-	-	-	424	-
Stage 1	-	-	-	-	705	-
Stage 2	-	-	-	-	714	-
Approach	EB		WB		SB	
Approach	ED		UVD		30	

Approach	ED	WB	30
HCM Control Delay, s	4.9	0	21.6
HCM LOS			С

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1	SBLn2
Capacity (veh/h)	937	-	-	- 424	937
HCM Lane V/C Ratio	0.136	-	-	- 0.619	0.108
HCM Control Delay (s)	9.4	-	-	- 26.3	9.3
HCM Lane LOS	А	-	-	- D	А
HCM 95th %tile Q(veh)	0.5	-	-	- 4.1	0.4

Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		- † †
Traffic Vol, veh/h	0	7	637	34	0	427
Future Vol, veh/h	0	7	637	34	0	427
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	8	749	40	0	502

Major/Minor	Minor1	Μ	lajor1	Ma	ajor2	
Conflicting Flow All	-	375	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	623	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	· -	623	-	-	-	-
Mov Cap-2 Maneuver	· -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	10.9	0	0
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBT
Capacity (veh/h)	-	-	623	-
HCM Lane V/C Ratio	-	-	0.013	-
HCM Control Delay (s)	-	-	10.9	-
HCM Lane LOS	-	-	В	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et –		Y	
Traffic Vol, veh/h	82	405	245	2	0	64
Future Vol, veh/h	82	405	245	2	0	64
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	96	476	288	2	0	75

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	290	0	-	0	957	289
Stage 1	-	-	-	-	289	-
Stage 2	-	-	-	-	668	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1272	-	-	-	286	750
Stage 1	-	-	-	-	760	-
Stage 2	-	-	-	-	510	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1272	-	-	-	265	750
Mov Cap-2 Maneuver	-	-	-	-	265	-
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	510	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.4		0		10.3	
HCM LOS					В	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1272	-	-	-	750
HCM Lane V/C Ratio		0.076	-	-	-	0.1
HCM Control Delay (s)	1	8.1	-	-	-	10.3
		0.1				
HCM Lane LOS)	A	-	-	-	В

Timings 12: Vollmer Rd & Marksheffel Rd

	٦	-	•	*	+	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- ††	1	ሻ	- ††	1	ሻ	- † †	1	ሻ	- ††	1
Traffic Volume (vph)	71	838	42	167	918	70	115	236	111	120	514	131
Future Volume (vph)	71	838	42	167	918	70	115	236	111	120	514	131
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	12.0	66.0	66.0	12.0	66.0	66.0	12.0	30.0	30.0	12.0	30.0	30.0
Total Split (%)	10.0%	55.0%	55.0%	10.0%	55.0%	55.0%	10.0%	25.0%	25.0%	10.0%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize? Recall Mode	Yes	Yes	Yes C-Max	Yes None	Yes C-Max	Yes	Yes	Yes Max	Yes	Yes None	Yes Max	Yes
Act Effct Green (s)	None 67.7	C-Max 61.0	61.0	69.0	63.4	C-Max 63.4	None 32.0	25.0	Max 25.0	32.0	25.0	Max 25.0
Actuated g/C Ratio	0.56	01.0	0.51	0.58	0.53	0.53	0.27	0.21	25.0 0.21	32.0 0.27	25.0 0.21	0.21
v/c Ratio	0.56	0.51	0.01	0.58	0.53	0.53	0.27	0.21	0.21	0.27	0.21	0.21
Control Delay	11.8	20.5	0.05	16.4	9.5	0.08	48.9	42.0	8.8	36.1	51.2	9.6
Queue Delay	0.0	20.5	0.1	0.0	9.5	0.2	40.9	42.0	0.0	0.0	0.0	9.0
Total Delay	11.8	20.5	0.0	16.4	9.5	0.0	48.9	42.0	8.8	36.1	51.2	9.6
LOS	B	20.5 C	A	10.4 B	9.5 A	0.2 A	40.9 D	42.0 D	0.0 A	50.1 D	51.2 D	9.0 A
Approach Delay	D	18.9	~	U	10.0	~	U	35.7	~	U	41.7	Л
Approach LOS		10.5 B			A			00.7 D			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12												
Offset: 0 (0%), Referenced	to phase 2	:EBTL an	d 6:WBTI	_, Start of	Green							
Natural Cycle: 70												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 2					ntersectio							
Intersection Capacity Utiliz	ation 69.7%	0		10	CU Level	ot Service	эC					
Analysis Period (min) 15												

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd

√ Ø1	₩ 102 (R)	Ø 3	Ø4
12 s	66 s	12 s	30 s
≯ _{ø5}	∮ Ø6 (R)	Ø7	↑ _{Ø8}
12 s	66 s	12 s	30 s

Timings 13: Sterling Ranch Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- ሻ	- ††	1	ሻ	- ††	1	<u> </u>	↑	1	ሻሻ	↑	1
Traffic Volume (vph)	158	900	10	47	809	175	39	8	55	450	9	307
Future Volume (vph)	158	900	10	47	809	175	39	8	55	450	9	307
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	12.0	51.0	51.0	12.0	51.0	51.0	12.0	25.0	25.0	32.0	45.0	45.0
Total Split (%)	10.0%	42.5%	42.5%	10.0%	42.5%	42.5%	10.0%	20.8%	20.8%	26.7%	37.5%	37.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag									
Lead-Lag Optimize?	Yes											
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	74.8 0.62	65.7 0.55	65.7 0.55	66.7 0.56	59.8 0.50	59.8 0.50	14.6 0.12	10.0 0.08	10.0 0.08	22.5 0.19	25.0 0.21	25.0 0.21
Actuated g/C Ratio	0.62	0.55	0.55	0.56	0.50	0.50	0.12	0.08	0.08	0.19	0.21	0.21
v/c Ratio Control Delay	20.0	0.49 15.2	0.01	12.0	22.8	0.21 3.7	32.9	0.05 51.6	1.8	0.74 53.2	35.8	13.3
Queue Delay	20.0	0.0	0.0	0.0	22.0	0.0	52.9 0.0	0.0	0.0	55.Z 0.0	0.0	0.0
Total Delay	20.0	15.2	0.0	12.0	22.8	3.7	32.9	51.6	1.8	53.2	35.8	13.3
LOS	20.0 C	15.2 B	0.0 A	12.0 B	22.0 C	3.7 A	52.9 C	51.0 D	1.0 A	55.2 D	55.6 D	13.3 B
Approach Delay	U	15.8	A	D	19.1	A	U	17.4	A	U	37.0	D
Approach LOS		15.0 B			19.1 B			В			57.0 D	
Intersection Summary Cycle Length: 120	_		_	_		_	_		_	_		
Actuated Cycle Length: 120)											
Offset: 0 (0%), Referenced		:EBTL an	d 6:WBTI	L. Start o	f Green							
Natural Cycle: 75				,								
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 2	22.5			li	ntersectio	n LOS: C						
Intersection Capacity Utiliza		, D		l	CU Level	of Service	эB					
Analysis Period (min) 15												

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd

Ø 1	₩Ø2 (R)	4 Ø3	♦ Ø4	
12 s	51 s	12 s	45 s	
	● ● Ø6 (R)	Ø7		√ Ø8
12 s	51 s	32 s		25 s

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		- † †
Traffic Vol, veh/h	0	2	376	2	0	765
Future Vol, veh/h	0	2	376	2	0	765
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	396	2	0	805

Major/Minor	Minor1	Μ	lajor1	Ma	ajor2	
Conflicting Flow All	-	198	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	810	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuve		810	-	-	-	-
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
•					00	

Approach	WB	NB	SB	
HCM Control Delay, s	9.5	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRW	'BLn1	SBT
Capacity (veh/h)	-	-	810	-
HCM Lane V/C Ratio	-	- (0.003	-
HCM Control Delay (s)	-	-	9.5	-
HCM Lane LOS	-	-	А	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et		Y	
Traffic Vol, veh/h	5	336	744	3	6	22
Future Vol, veh/h	5	336	744	3	6	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	354	783	3	6	23

Major/Minor	Major1	Ν	lajor2		Minor2	
Conflicting Flow All	786	0	-	0	1149	785
Stage 1	-	-	-	-	785	-
Stage 2	-	-	-	-	364	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	833	-	-	-	219	393
Stage 1	-	-	-	-	449	-
Stage 2	-	-	-	-	703	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	218	393
Mov Cap-2 Maneuver	-	-	-	-	218	-
Stage 1	-	-	-	-	446	-
Stage 2	-	-	-	-	703	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		16.8	
HCM LOS			-		С	
			EDT			2014
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	
Capacity (veh/h)		833	-	-	-	335
HCM Lane V/C Ratio		0.006	-	-		0.088
HCM Control Delay (s)	9.3	-	-	-	16.8
HCM Lane LOS	,	A	-	-	-	С
HCM 95th %tile Q(veh	1)	0	-	-	-	0.3

Timings 12: Vollmer Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- ††	1	- ከ	- ††	1	<u> </u>	- † †	1	<u>۲</u>	- ††	1
Traffic Volume (vph)	142	928	96	185	706	154	171	719	188	132	342	199
Future Volume (vph)	142	928	96	185	706	154	171	719	188	132	342	199
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	20.0	50.0	50.0	19.0	49.0	49.0	15.0	36.0	36.0	15.0	36.0	36.0
Total Split (%)	16.7%	41.7%	41.7%	15.8%	40.8%	40.8%	12.5%	30.0%	30.0%	12.5%	30.0%	30.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	57.7	47.0	47.0	60.3	48.3	48.3	41.2	31.4	31.4	40.8	31.2	31.2
Actuated g/C Ratio	0.48	0.39	0.39	0.50	0.40	0.40	0.34	0.26	0.26	0.34	0.26	0.26
v/c Ratio	0.43	0.70	0.15	0.70	0.52	0.22	0.50	0.82	0.38	0.68	0.39	0.37
Control Delay	18.5	34.4	4.5	49.4	22.3	5.3	31.4	50.1	13.4	42.7	38.2	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.5	34.4	4.5	49.4	22.3	5.3	31.4	50.1	13.4	42.7	38.2	6.7
LOS	В	С	А	D	С	А	С	D	В	D	D	А
Approach Delay		30.0			24.6			40.8			29.8	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120)											
Offset: 0 (0%), Referenced		:EBTL an	d 6:WBTI	. Start of	Green							
Natural Cycle: 70				_,								
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.82												
Intersection Signal Delay: 3	1.5			Ir	ntersectio	n LOS: C						
Intersection Capacity Utiliza)			CU Level							
Analysis Period (min) 15		- 				27 231 110						

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd

√ Ø1	₩ ₩ 2 (R)	Ø 3	Ø4
19 s	50 s	15 s	36 s
≯ _{ø5}	●	Ø7	1 08
20 s	49 s	15 s	36 s

Timings 13: Sterling Ranch Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- † †	1	ሻ	- † †	1	ሻ	↑	1	ካካ	↑	1
Traffic Volume (vph)	285	946	22	130	841	540	27	7	37	324	23	176
Future Volume (vph)	285	946	22	130	841	540	27	7	37	324	23	176
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	20.0	68.0	68.0	12.0	60.0	60.0	10.0	15.0	15.0	25.0	30.0	30.0
Total Split (%)	16.7%	56.7%	56.7%	10.0%	50.0%	50.0%	8.3%	12.5%	12.5%	20.8%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	80.2	69.1	69.1	70.0	63.1	63.1	11.0	10.0	10.0	20.0	23.0	23.0
Actuated g/C Ratio	0.67	0.58	0.58	0.58	0.53	0.53	0.09	0.08	0.08	0.17	0.19	0.19
v/c Ratio	0.71	0.49	0.02	0.41	0.48	0.52	0.20	0.05	0.12	0.60	0.07	0.41
Control Delay	36.2	10.4	0.0	12.9	20.7	3.4	37.8	51.4	0.8	51.2	39.1	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.2	10.4	0.0	12.9	20.7	3.4	37.8	51.4	0.8	51.2	39.1	8.5
LOS	D	B	А	В	C	А	D	D	А	D	D	A
Approach Delay		16.1			13.9			19.6			36.3	
Approach LOS		В			В			В			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 0 (0%), Referenced	to phase 2	:EBTL an	d 6:WBTI	_, Start of	f Green							
Natural Cycle: 80												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.71												
Intersection Signal Delay: 1					ntersectio							
Intersection Capacity Utiliza	ation 70.1%	,)		10	CU Level	of Servic	e C					
Analysis Period (min) 15												

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd

√ Ø1		↑ Ø3 ↓ Ø4	
12 s	68 s	10 s 30 s	
▶ ø₅	🗸 🕈 Ø6 (R)	▶ Ø7	≪ †ø8
20 s	60 s	25 s	15 s

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		- † †
Traffic Vol, veh/h	0	2	1007	8	0	673
Future Vol, veh/h	0	2	1007	8	0	673
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	1060	8	0	708

Major/Minor	Minor1	Μ	ajor1	Ма	ijor2	
Conflicting Flow All	-	530	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	493	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	· -	493	-	-	-	-
Mov Cap-2 Maneuver	· -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.3	0	0
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	VBLn1	SBT
Capacity (veh/h)	-	-	493	-
HCM Lane V/C Ratio	-	-	0.004	-
HCM Control Delay (s)	-	-	12.3	-
HCM Lane LOS	-	-	В	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	0.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et 👘		Y	
Traffic Vol, veh/h	25	807	505	3	1	18
Future Vol, veh/h	25	807	505	3	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	26	849	532	3	1	19

Major/Minor	Major1	Ν	lajor2		Minor2	
Conflicting Flow All	535	0	-	0		534
Stage 1	-	-	-	-	534	-
Stage 2	-	-	-	-	901	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1033	-	-	-	147	546
Stage 1	-	-	-	-	588	-
Stage 2	-	-	-	-	396	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	143	546
Mov Cap-2 Maneuver	-	-	-	-	143	-
Stage 1	-	-	-	-	573	-
Stage 2	-	-	-	-	396	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		12.9	
HCM LOS					В	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1033	-	-	-	475
HCM Lane V/C Ratio		0.025	-	-	-	0.042
HCM Control Delay (s)	8.6	-	-	-	12.9
HCM Lane LOS		А	-	-	-	В
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.1

Timings 12: Vollmer Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	ሻ	<u></u>	1	۳	<u></u>	1	ሻ	- † †	1	ሻ	- † †	1
Traffic Volume (vph)	71	843	42	174	932	70	115	238	112	120	514	131
Future Volume (vph)	71	843	42	174	932	70	115	238	112	120	514	131
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	12.0	66.0	66.0	12.0	66.0	66.0	12.0	30.0	30.0	12.0	30.0	30.0
Total Split (%)	10.0%	55.0%	55.0%	10.0%	55.0%	55.0%	10.0%	25.0%	25.0%	10.0%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	67.7	61.0	61.0	69.0	63.4	63.4	32.0	25.0	25.0	32.0	25.0	25.0
Actuated g/C Ratio	0.56	0.51	0.51	0.58	0.53	0.53	0.27	0.21	0.21	0.27	0.21	0.21
v/c Ratio	0.25	0.49	0.05	0.56	0.52	0.08	0.64	0.34	0.28	0.41	0.73	0.32
Control Delay	11.9	20.5	0.1	17.8	9.9	0.2	48.9	42.0	8.8	36.2	51.2	9.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.9	20.5	0.1	17.8	9.9	0.2	48.9	42.0	8.8	36.2	51.2	9.6
LOS	В	С	А	В	А	А	D	D	А	D	D	A
Approach Delay		19.0			10.5			35.7			41.7	
Approach LOS		В			В			D			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12	0											
Offset: 0 (0%), Referenced	I to phase 2	:EBTL an	d 6:WBTI	_, Start o	f Green							
Natural Cycle: 70												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay:	23.5			li	ntersectio	n LOS: C						
Intersection Capacity Utiliz	ation 70.2%	Ď		l	CU Level	of Service	эC					
Analysis Period (min) 15												
,												

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd

√ Ø1	₩ 102 (R)	Ø 3	Ø4
12 s	66 s	12 s	30 s
≯ _{ø5}	∮ Ø6 (R)	Ø7	↑ _{Ø8}
12 s	66 s	12 s	30 s

Timings 13: Sterling Ranch Rd & Marksheffel Rd

	٦	-	\mathbf{i}	4	+	•	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	1	ሻ	^	1	ሻ	↑	1	ሻሻ	↑	1
Traffic Volume (vph)	164	900	10	47	809	184	39	8	55	480	9	328
Future Volume (vph)	164	900	10	47	809	184	39	8	55	480	9	328
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	12.0	51.0	51.0	12.0	51.0	51.0	12.0	25.0	25.0	32.0	45.0	45.0
Total Split (%)	10.0%	42.5%	42.5%	10.0%	42.5%	42.5%	10.0%	20.8%	20.8%	26.7%	37.5%	37.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag Lead-Lag Optimize?	Lead Yes	Lag Yes	Lag Yes	Lead Yes	Lag Yes	Lag Yes	Lead Yes	Lag Yes	Lag Yes	Lead Yes	Lag Yes	Lag Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	74.5	65.1	65.1	65.7	58.8	58.8	14.6	10.0	10.0	23.1	25.5	25.5
Actuated g/C Ratio	0.62	0.54	0.54	0.55	0.49	0.49	0.12	0.08	0.08	0.19	0.21	0.21
v/c Ratio	0.02	0.34	0.04	0.55	0.49	0.49	0.12	0.00	0.00	0.19	0.21	0.21
Control Delay	21.9	15.6	0.01	12.3	23.6	3.7	32.6	51.6	1.8	54.0	35.2	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.9	15.6	0.0	12.3	23.6	3.7	32.6	51.6	1.8	54.0	35.2	15.6
LOS	21.5 C	10.0 B	A	12.3 B	20.0 C	0.7 A	02.0 C	D	A	04.0 D	00.2 D	B
Approach Delay	Ū	16.4	71	U	19.6	73	Ŭ	17.3		U	38.4	-
Approach LOS		В			B			B			D	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 12	0											
Offset: 0 (0%), Referenced		:EBTL an	d 6:WBTI	L, Start o	f Green							
Natural Cycle: 75												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay:	23.4			li	ntersectio	n LOS: C						
Intersection Capacity Utiliz	ation 64.3%	b		l	CU Level	of Service	e C					
Analysis Period (min) 15												

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd

Ø1	₩ Ø2 (R)	1 Ø3	∲ Ø4	
12 s	51 s	12 s	45 s	
	● ● Ø6 (R)	Ø7		√ Ø8
12 s	51 s	32 s		25 s

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		- 11
Traffic Vol, veh/h	0	3	376	4	0	765
Future Vol, veh/h	0	3	376	4	0	765
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	396	4	0	805

Major/Minor	Minor1	Μ	ajor1	Ма	ajor2	
Conflicting Flow All	-	198	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	810	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		810	-	-	-	-
Mov Cap-2 Maneuver	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
					~ ~	

Approach	WB	NB	SB	
HCM Control Delay, s	9.5	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBT
Capacity (veh/h)	-	-	810	-
HCM Lane V/C Ratio	-	-	0.004	-
HCM Control Delay (s)	-	-	9.5	-
HCM Lane LOS	-	-	Α	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	1.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et 👘		Y	
Traffic Vol, veh/h	21	336	744	11	25	73
Future Vol, veh/h	21	336	744	11	25	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	22	354	783	12	26	77

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	795	0	-	0	1187	789
Stage 1	-	-	-	-	789	-
Stage 2	-	-	-	-	398	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-		-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	826	-	-	-		391
Stage 1	-	-	-	-	448	-
Stage 2	-	-	-	-	678	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve		-	-	-	202	391
Mov Cap-2 Maneuve	er -	-	-	-	202	-
Stage 1	-	-	-	-	436	-
Stage 2	-	-	-	-	678	-
Approach	EB		WB		SB	
HCM Control Delay,	s 0.6		0		21.8	
HCM LOS					С	
Minor Lane/Major M	/mt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	826	-	-	-	316
HCM Lane V/C Ratio)	0.027	-	-	-	0.326
HCM Control Delay (9.5	-	-	-	21.8
HCM Lane LOS	-7	A	-	-	-	C
		~	-	-	-	0

Timings 12: Vollmer Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	ľ	<u></u>	1	ľ	- † †	1	۲ ۲	<u></u>	1	ľ	- † †	5
Traffic Volume (vph)	142	949	96	191	718	154	171	728	191	132	342	19
Future Volume (vph)	142	949	96	191	718	154	171	728	191	132	342	199
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Pern
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	20.0	50.0	50.0	19.0	49.0	49.0	15.0	36.0	36.0	15.0	36.0	36.0
Total Split (%)	16.7%	41.7%	41.7%	15.8%	40.8%	40.8%	12.5%	30.0%	30.0%	12.5%	30.0%	30.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.(
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	57.4	46.7	46.7	60.6	48.3	48.3	41.2	31.4	31.4	40.8	31.2	31.2
Actuated g/C Ratio	0.48	0.39	0.39	0.50	0.40	0.40	0.34	0.26	0.26	0.34	0.26	0.26
v/c Ratio	0.43	0.73	0.15	0.73	0.53	0.23	0.50	0.83	0.39	0.68	0.39	0.3
Control Delay	18.7	35.3	4.5	53.2	22.5	5.5	31.4	50.7	13.7	43.5	38.2	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.7	35.3	4.5	53.2	22.5	5.5	31.4	50.7	13.7	43.5	38.2	6.7
LOS	В	D	А	D	С	А	С	D	В	D	D	ŀ
Approach Delay		30.8			25.5			41.2			29.9	
Approach LOS		С			С			D			С	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 0 (0%), Referenced to	phase 2	:EBTL an	d 6:WBT	L, Start of	f Green							
Natural Cycle: 70												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.83												
Intersection Signal Delay: 32					ntersectio							
Intersection Capacity Utilizati	on 80.9%	, ,		10	CU Level	of Service	эD					
Analysis Period (min) 15												

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd

√ Ø1	₩ ₩ Ø2 (R)	▲ Ø3	Ø4
19 s	50 s	15 s	36 s
	●	Ø7	1 08
20 s	49 s	15 s	36 s

Timings 13: Sterling Ranch Rd & Marksheffel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- ††	1	ሻ	- † †	1	٦	↑	1	ካካ	↑	1
Traffic Volume (vph)	310	946	22	130	841	581	27	7	37	348	24	194
Future Volume (vph)	310	946	22	130	841	581	27	7	37	348	24	194
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	20.0	68.0	68.0	12.0	60.0	60.0	10.0	15.0	15.0	25.0	30.0	30.0
Total Split (%)	16.7%	56.7%	56.7%	10.0%	50.0%	50.0%	8.3%	12.5%	12.5%	20.8%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	80.7	69.1	69.1	69.5	62.6	62.6	11.0	10.0	10.0	20.0	23.0	23.0
Actuated g/C Ratio	0.67	0.58	0.58	0.58	0.52	0.52	0.09	0.08	0.08	0.17	0.19	0.19
v/c Ratio	0.76	0.49	0.02	0.41	0.48	0.55	0.20	0.05	0.12	0.64	0.07	0.44
Control Delay	40.3	10.5	0.0	12.9	20.9	3.6	37.8	51.4	0.8	52.4	39.2	8.5
Queue Delay	0.0 40.3	0.0 10.5	0.0 0.0	0.0 12.9	0.0 20.9	0.0 3.6	0.0 37.8	0.0 51.4	0.0 0.8	0.0 52.4	0.0 39.2	0.0 8.5
Total Delay LOS	40.3 D	10.5 B	0.0 A	12.9 B	20.9 C	3.0 A	37.8 D	51.4 D	0.8 A	52.4 D	39.2 D	6.5 A
Approach Delay	U	ы 17.6	A	D	13.8	A	U	19.6	A	U	36.8	A
Approach LOS		17.0 B			13.0 B			19.0 B			30.0 D	
		D			D			D			U	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120				• •••••								
Offset: 0 (0%), Referenced	to phase 2	:EBIL an	d 6:WB11	L, Start o	r Green							
Natural Cycle: 90	a malline a Const											
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.76	0.0											
Intersection Signal Delay: 1					ntersectio		- D					
Intersection Capacity Utiliza	auon 74.0%)		I	CU Level	of Service	вD					
Analysis Period (min) 15												

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd

√ Ø1		↑ Ø3 ↓ Ø4	
12 s	68 s	10 s 30 s	
▶ ø₅	🗸 🕈 Ø6 (R)	▶ Ø7	≪ †ø8
20 s	60 s	25 s	15 s

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	- 11	1		^
Traffic Vol, veh/h	0	3	1007	17	0	674
Future Vol, veh/h	0	3	1007	17	0	674
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	1060	18	0	709

Major/Minor	Minor1	Μ	lajor1	Ма	ijor2	
Conflicting Flow All	-	530	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	493	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	· -	493	-	-	-	-
Mov Cap-2 Maneuver	· -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.3	0	0
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBT
Capacity (veh/h)	-	-	493	-
HCM Lane V/C Ratio	-	-	0.006	-
HCM Control Delay (s)	-	-	12.3	-
HCM Lane LOS	-	-	В	-
HCM 95th %tile Q(veh)	-	-	0	-

Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	et –		Y	
Traffic Vol, veh/h	90	807	505	12	6	61
Future Vol, veh/h	90	807	505	12	6	61
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	95	849	532	13	6	64

Major/Minor	Major1	Ν	/lajor2		Minor2		
Conflicting Flow All	545	0	-		1578	539	3
Stage 1	-	-	-	-	539	-	-
Stage 2	-	-	-	-	1039	-	-
Critical Hdwy	4.12	-	-	-	6.42	6.22	2
Critical Hdwy Stg 1	-	-	-	-	5.42	-	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-	-
Follow-up Hdwy	2.218	-	-	-	3.518		
Pot Cap-1 Maneuver	1024	-	-	-	120	542	2
Stage 1	-	-	-	-	585	-	-
Stage 2	-	-	-	-	341	-	-
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver		-	-	-	109	542	2
Mov Cap-2 Maneuver	-	-	-	-	109	-	-
Stage 1	-	-	-	-	531	-	-
Stage 2	-	-	-	-	341	-	-
Approach	EB		WB		SB		
HCM Control Delay, s	0.9		0		15.9		
HCM LOS					С		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1	1
Capacity (veh/h)		1024	-	-	-	400)
HCM Lane V/C Ratio		0.093	-	-	-	0.176	
HCM Control Delay (s)	8.9	-	-	-	15.9	9
HCM Lane LOS		А	-	-	-	С)
						-	



Appendix Table 1 Area Traffic Impact Studies Copper Chase Sterling Ranch										
Study	PCD File No ⁽¹⁾	Consultant	Date							
Sterling Ranch Reports										
Sterling Ranch Updated Traffic Impact Analysis	<u>SKP07007</u>	LSC Transportation Consultants, Inc	June 5, 2008							
Sterling Ranch Phase 1 Traffic Impact Study	<u>P151</u>	LSC Transportation Consultants, Inc	March 16, 2015							
Sterling Ranch Phases 1-3 Transportation Memorandum	<u>SP1415</u>	LSC Transportation Consultants, Inc	October 2, 2017							
Branding Iron at Sterling Ranch Filing No. 1 and Homestead at Sterling Ranch Filing No. 1 Transportation	<u>SF1724</u> <u>SF1725</u>	LSC Transportation Consultants, Inc	December 19, 2017							
Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF1820</u>	LSC Transportation Consultants, Inc	April 3, 2018							
Sterling Ranch Phase 2 Preliminary Plan Traffic Impact Study	<u>SP203</u>	LSC Transportation Consultants, Inc	December 20, 2018							
Homestead at Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF194</u>	LSC Transportation Consultants, Inc	March 3, 2020							
Branding Iron at Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF1918</u>	LSC Transportation Consultants, Inc	May 6, 2020							
Sterling Ranch Filing No. 2 and Phase 2 Traffic Impact Study	<u>SF2015</u> <u>SP191</u>	LSC Transportation Consultants, Inc	June 23, 2021							
Sterling Ranch Filing No. 3 Transportation Memorandum	<u>SF2132</u>	LSC Transportation Consultants, Inc	April 19, 2022							
Homestead North Phase 1 Updated Transportation Memorandum	<u>SP208</u>	LSC Transportation Consultants, Inc	January 11, 2022							
Homestead North Filing No. 1 Traffic Technical Memorandum	<u>SF2213</u>	LSC Transportation Consultants, Inc	February 2, 2022							
Homestead North Filing No. 2 Traffic Technical Memorandum	<u>SF2218</u>	LSC Transportation Consultants, Inc	April 15, 2022							
Homestead North Filing 3 Traffic Impact Study	<u>SF2229</u>	LSC Transportation Consultants, Inc	June 17, 2022							
The Villages at Sterling Ranch East Preliminary Plan/Traffic Generation Analysis	PUDSP226	SM Rocha, LLC	July 1, 2022							
Sterling Ranch Sketch Plan Amendment Master Traffic Impact Study	<u>SKP224</u>	LSC Transportation Consultants, Inc	March 17, 2023							
Sterling Ranch East - Rezoning & Preliminary Plan Traffic Impact Study	<u>SP-22-004, P-22-012, P-22-013</u>	LSC Transportation Consultants, Inc	March 17, 2023 ⁽²⁾							
Sterling Ranch East Filing Nos 1 & 2 Traffic Technical Memorandum	SF2235 SF2237	LSC Transportation Consultants, Inc	February 10, 2023							
Sterling Ranch Filing No. 4 Transportation Memorandum	<u>SF2230</u>	LSC Transportation Consultants, Inc	February 21, 2023							
Foursquare at Sterling Ranch East Transportation Memorandum	<u>SF2236</u>	LSC Transportation Consultants, Inc	April 20, 2023							
Retreat at TimberRidge Reports										
The Retreat at TimberRidge Traffic Impact Analysis	<u>PUD173</u>	LSC Transportation Consultants, Inc	January 25, 2018							
The Retreat at TimberRidge Preliminary Plan Traffic Technical Memorandum	SP182	LSC Transportation Consultants, Inc	June 29, 2018							
The Retreat at TimberRidge Filing No. 1 Traffic Technical Memorandum	SF199	LSC Transportation Consultants, Inc	April 3, 2020							
The Retreat at TimberRidge Filing No. 2 Updated Traffic Technical Memorandum	<u>SF2121</u>	LSC Transportation Consultants, Inc	October 4, 2021							
The Retreat at TimberRidge Filing No. 3 Traffic Technical Memorandum	SF2241	LSC Transportation Consultants, Inc	July 1, 2022							
	512241	Lise manisportation consultants, inc	July 1, 2022							
Other Area Reports										
Wolf Ranch School Site Traffic Impact Study	<u>OAR1720</u>	Matrix Design Group, Inc.	5-May-17							
The Ranch Sketch Plan Traffic Impact Analysis	<u>SKP186</u>	LSC Transportation Consultants, Inc	July 9, 2019							
Lodge III Traffic Impact Study	OAR	LSC Transportation Consultants, Inc	December 13, 2019							
Continental 613 Traffic Impact Study	<u>OAR2177</u>	LSC Transportation Consultants, Inc	July 16, 2021							
Solace at Black Forest Traffic Impact and Access Analysis	<u>OAR2134</u>	LSC Transportation Consultants, Inc	August 13, 2021							
Traffic Impact Study Addendum for Percheron	<u>OAR2173</u>	SM Rocha, LLC	October, 2021							
Woodmen East Commercial Center Traffic Impact Analysis	<u>OAR2191</u>	LSC Transportation Consultants, Inc	December 8, 2021							
Traffic Impact Study for Jaynes Property	<u>SKP225</u>	SM Rocha, LLC	May, 2022							
Traffic Impact Study for Rhetoric Site	<u>P2216</u>	SM Rocha, LLC	June, 2022							
Briargate-Stapleton Corridor Study (DRAFT)	briargate-stapleton.com	Wilson & Company	December 9, 2021							
Notes: 1) Follow the links listed below to obtain the most recent version of each listed study. To obtain a copy of the versi 2) With minor revision 4/3/2023 Source: LSC Transportation Consultants, Inc.	ion of each study used in preparing	this report please contact LSC Transport	ation Consultants, Inc. Apr-2							

		Appendix Table 2 ⁽¹⁾ (Page 1 of 2)							
Sterling Ranch									
Segment ID ⁽²⁾ See Figure 14 for map)	Roadwa Improvement Description	y Segment Improvements Timing	Design ADT (vpd)	Projected 2042 ADT (vpd)	Responsibility				
V1 northbound V1 southbound	Per the City of Colorado Springs, an outside paved shoulder will need to be added along the east side of Vollmer Road from Dry Needle Place up to the south end of segment V2 improvements.	With Sterling Ranch Filing No. 4 but potentially complete concurrently with the construction of the right-turn lane at Pioneer Landscape Center access for the Sterling Ranch Recycling Facility (PCD No. PPR2241)	5,500 (Directional northbound) 10,000 (Directional southbound)	16,275	Sterling Ranch				
V1	Improve Vollmer Road between Dry Needle Place and the Sterling Ranch south boundary to a standard 4-Lane Urban Minor Arterial Cross Section (add a second northbound through lane and painted center median). ⁽³⁾	The need driven by anticipated traffic from each development impacting this section of Vollmer Road.	20,000		Sterling Ranch, if necessary, prior to construction by others.				
V2	Improve Vollmer Road between the Sterling Ranch south boundary to Lochwinnoch Lane/Sterling property boundary to a standard 4-Lane Urban Minor Arterial Cross Section. ⁽³⁾	Short-Term Future (With Sterling Ranch Fil No. 2 Or Sterling Ranch Phase 2)	20,000 (Note: Existing Capacity 8,000 ⁽³⁾)	17,475	Sterling Ranch				
V3	Short Term: Improve Vollmer Road from Lochwinnoch Lane to Sterling Ranch boundary (northeast of Glider Loop) to provide 36' of pavement (existing pavement 1 approx. 23.38') and stripe for one through lane plus a 6' paved, striped outside shoulder in each direction. ⁽³⁾	Short-Term Future (With Homestead North)	11,000 (Note: Existing Capacity 8,000)	17,380	Sterling Ranch				
•5	Long Term: Improve Vollmer Road from Lochwinnoch Lane to Sterling Ranch boundary (northeast of Glider Loop) to a standard 4-Lane Urban Minor Arterial Cross Section. ⁽³⁾	Long-Term Future	20,000		Sterling Ranch with potential County assistance with ROW acquisition - pursua to the recent development agreement between Sterling Ranch and EPC.				
V4	Improve Vollmer Road from Sterling Ranch boundary (northeast of Glider Loop) to Briargate Parkway to a standard 4- Lane Urban Minor Arterial Cross Section. ⁽³⁾	Short-Term Future May 2024Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: With Homestead North Filing 1)	20,000	16,445	Sterling Ranch				
V5	Improve Vollmer Road from Briargate Parkway to Jane Kirkham Drive to a standard 4-Lane Urban Minor Arterial Cross Section. ⁽³⁾	Short-Term Future– May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: prior note: With Homestead North Filing 1)	20,000	11,690	Sterling Ranch				
V6	Improve Vollmer Road from Jane Kirkham Drive to Sam Bass Drive to a standard 4-Lane Urban Minor Arterial Cross Section. ⁽³⁾	Short-Term Future– May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: prior note: With Homestead North Filing 2)	20,000	11,425	Sterling Ranch				
V7	Improve Vollmer Road between Sam Bass Drive and Poco Road to a 4-lane Urban Minor Arterial but with necessary lane transitions, redirect tapers, etc. south of Poco to adequately transition between the 4-Lane Urban Minor Arterial Cross Section and the 2-Lane Rural Arterial Cross Section north of Poco Road.	Short-Term Future – May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: With Homestead North Filing 3)	20,000	10,030	Sterling Ranch				
V8	Improve Vollmer Road from Poco Road to Shoup Road to a Rural 2-Lane Arterial Cross Section. ⁽³⁾	Long-Term Future	10,000	11,790	El Paso County Project ID U-12				
o tes: (1) Source	table (see Part 2 on next page) e: This Appendix Table 2 is a copy of Table 6 from the Sterling Ranch East Phase 1 Rezoning and Preliminary Plan TIS, March have been highlighted in green. gure 14	h 17, 2023 (with minor revisions April 3, 2023) with LSC notes regarding t	nis Copper Chase pr	oject. Improven	nents needed prior to Copper Chase at Sterl				
(3) Adequ 20:1	ate transition/redirect tapers would be needed between the various cross sections on Vollmer Road. Based on the criteria	a contained in Table 2-29 of the <i>El Paso Engineering Criteria Manual</i> , an a	ppropriate taper rat	tio for a roadwa	y with a design speed of 40 miles per hour is				
	e: Table 20 Road Impact Fee Study Updated November 16, 2016 nsportation Consultants, Inc. (April 26, 2023)								

	(Page 2 of 2) Sterling Ranch							
Roadway Segment Improvements								
egment ID ⁽²⁾ ee Figure 14 for map)	Improvement Description	Timing	Design ADT (vpd)	Projected 2042 ADT (vpd)	Responsibility			
SR1	Construct Sterling Ranch Road as an Urban Non-Residential Collector from Marksheffel Road to Dines Boulevard.	Short Term - with Sterling Ranch Fil No. 2	20,000	<mark>14,840</mark>	Sterling Ranch			
SR2	Construct Sterling Ranch Road as an Urban Non-Residential Collector from Dines Boulevard to Briargate Parkway.	Short-Term	20,000	10,275	Sterling Ranch			
SR3	Construct Sterling Ranch Road as an Urban Collector from Briargate Parkway to Vancouver Street.	Short Term	10,000	9,300	Sterling Ranch			
SR4 M1	Construct Sterling Ranch Road from Vancouver Street north to Arroya (or ultimate north terminus). Construct Marksheffel Road as an Urban Principal Arterial to City of Colorado Springs standards in 107' of right-of-way between Vollmer Road and Sterling Ranch Road.	Long-Term Future Updated 10/15/2022: to be completed by the end of 2022 (prior note: With Sterling Ranch Fil No. 2	10,000 <mark>40,000</mark>	4,260 23,370	Sterling Ranch <mark>Sterling Ranch</mark>			
<mark>M2</mark>	Construct Marksheffel Road as an Urban Principal Arterial to City of Colorado Springs standards in 107' of right-of-way between Sterling Ranch Road and the south boundary of the Sterling Ranch Master Plan Area. 10/16/2022 NOTE: With the completion of M2 in 2023, the connection between Vollmer and Woodmen Road (via M3) will be completed.	Short Term Updated 10/15/2022: to be completed in 2023 (prior note: With Sterling Ranch Phase 2)	40,000	<mark>29,600</mark>	Sterling Ranch			
М3	Construct Marksheffel Road between the south boundary of the Sterling Ranch Master Plan Area and Woodmen Road. (Note this segment is located within the City of Colorado Springs). 10/16/2022 NOTE: With the completion of M2 in 2023, the connection between Vollmer and Woodmen Road (via M3) will be completed.	Updated 10/15/2022: <u>Completed</u> (by Others)	40,000	24,525	Others (Completed)			
M4	Construct Marksheffel Road between Black Forest Road and Vollmer Road.	Long-Term Future	40,000	27,910	Others			
В1	Construct the south half section of Briargate Pkwy (4-Lane Principal Arterial) between Vollmer Road and Wheatland Drive [now full section by 2023].	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 with Homestead at Sterling Ranch Filing No. 1 (prior note: With Homestead at Sterling Ranch Fil 2)	20,000	- 24,745 -	Sterling Ranch			
ы	nstruct the north half section of Briargate Pkwy (4-Lane Principal Arterial) between Vollmer Road and Wheatland ive [now full section by 2023] .	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 with Homestead at Sterling Ranch Filing No. 1 (prior note: Long-Term Future)	40,000	24,743	Sterling Ranch			
B2	Construct Briargate Pkwy (full section) as a 4-Lane Principal Arterial between Wheatland Dr and Sterling Ranch Road.	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 or Spring 2024 (prior note: Long-Term Future)	40,000	26,375	Sterling Ranch			
B3	Construct Briargate Pkwy as a 4-Lane Principal Arterial between Sterling Ranch Road and Banning Lewis Parkway.	Intermediate Term	40,000	22.365	Sterling Ranch			
B4	Construct Stapleton Road as a 4-Lane Principal Arterial between Banning Lewis Parkway and Meridian Road (including upgrade of existing rural two-lane segment between Towner and Meridian).	Long-Term Future	40,000	17,945	Others			
B5	Construct Briargate Pkwy as a 4-Lane Principal Arterial between Black Forest Road and Vollmer Road.	Long-Term Future	40,000	24,340	Others; PPRTA A List Project			
BL1	Construct Banning Lewis Parkway as a 4-Lane Principal Arterial between the south Sterling Ranch boundary and Briargate Pkwy.	Long-Term Future	40,000	<mark>20,320</mark>	Financial assurances for half-section, Ste Ranch half-section or full-section w/ co recovery			
BL2	Construct Banning Lewis Parkway as a 4-Lane Principal Arterial between Woodmen Road and the south Sterling Ranch boundary. (Note this segment will be located within the City of Colorado Springs)	Long-Term Future	40,000	28,480	Others			
W1	Widen Woodmen Road from 4-lane to 6-lane section from Powers Boulevard to US 24.	Long-Term Future	72,000	66,690	PPRTA A-List Project; City of Colorad Springs ConnectCOS Index No.476			
B1	Widen Black Forest Road between Woodmen Road to just north of Research Road to two northbound and southbound through lanes.	Black Forest Widening Project	40,000	28,420	City of Colorado Springs			
B2	Widen Black Forest Road from just north of Research Road to Briargate Parkway.	Long-Term Future	40,000	25,145	Others/City of Colorado Springs			
В3	Widen Black Forest Rd from Briargate Pkwy to Old Ranch Rd as a 4-lane Principal Arterial with bike and pedestrian facilities.	Long-Term Future	40,000	19,135	PPRTA B List Project ConnectCOS Index No. 479			
rt 2/2 of this tes:	table							
• •	e: This Appendix Table 2 is a copy of Table 6 from the Sterling Ranch East Phase 1 Rezoning and Preliminary Plan TIS, March have been highlighted in green.	17, 2023 (with minor revisions April 3, 2023) with LSC notes regarding th	is Copper Chase pr	oject. Improveme	ents needed prior to Copper Chase at Ste			