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Copper Chase at Sterling Ranch
Traffic Technical Memorandum
PUDSP-22-002
(LSC #184990)
April 26, 2023

Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

Copper Chase at Sterling Ranch Traffic Impact Study

Prepared for:

Mr. Jim Byers Challenger Homes 8605 Explorer Drive, Suite 250 Colorado Springs, CO 80920

APRIL 26, 2023

LSC Transportation Consultants
Prepared by: Kirstin D. Ferrin, P.E.
Reviewed by: Jeffrey C. Hodsdon, P.E.

LSC #184990 PUDSP-22-002



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April 26, 2023

Mr. Jim Byers Challenger Homes 8605 Explorer Drive, Suite 250 Colorado Springs, CO 80920

> RE: Copper Chase at Sterling Ranch El Paso County, CO Traffic Impact Study PUDSP-22-002 LSC #184990

Dear Mr. Byers:

LSC Transportation Consultants, Inc. has prepared this traffic impact study (TIS) for the proposed Copper Chase at Sterling Ranch residential development. As shown in Figure 1, the site is located east of Vollmer Road and north of the future extension of Marksheffel Road in El Paso County, Colorado.

LSC prepared a traffic technical memorandum for this development dated May 26, 2022, however since completion of that memorandum LSC has updated the Sterling Ranch Master Traffic Impact Study and prepared multiple reports various for preliminary plans and filings within Sterling Ranch and the surrounding area which impact the recommendations and conclusions. This report replaces the May 2022 memorandum and is intended as site-specific, PUD Development/PUD Plan report for Copper Chase at Sterling Ranch.

REPORT CONTENTS

This report presents:

- A list of previous Sterling Ranch traffic reports and the context of this project;
- A summary of the proposed land use and access plan;
- The existing roadway and traffic conditions in the site's vicinity including the roadway widths, surface conditions, lane geometries, traffic controls, and posted speed limits;
- The projected average weekday and peak-hour vehicle trips to be generated by the proposed future development;
- The assignment of the projected site-generated traffic volumes to the area roadways;

- Estimates of projected short-term long-term Background traffic volumes;
- The projected short-term and long-term total traffic volumes on the area roadways;
- The projected levels of service at the key intersections within the study area;
- A list of deviation requests
- Findings and recommendations for study area roadways and intersections, including number of lanes, auxiliary turn lanes, intersection traffic control, etc.; and
- The recommended street classifications
- The project's obligation to the County roadway improvement fee program.

REPORT SCENARIOS

Short-Term Scenario

The short-term scenario includes the roadway segments to be added in the short term only as shown in Figure 2. This scenario includes traffic to be traffic generated by the currently proposed Copper Chase at Sterling Ranch and traffic to be generated in the short term by buildout of Homestead at Sterling Ranch, Branding Iron at Sterling Ranch, Sterling Ranch Filings 2-4, Homestead North at Sterling Ranch Filings 1-3, the Retreat at TimberRidge Filings 1-3, Sterling Ranch East Filings 1 and 2, and the planned FourSquare at Sterling Ranch East development. Trips projected from these other short-term developments outside of the currently proposed Copper Chase at Sterling Ranch are included as short-term "background traffic" in this report.

Long-Term Scenario

The long-term scenario is essentially the same as the 2043 Long-Term scenario contained in the LSC February 10, 2023 Master TIS with additional detail added for this application – including the analysis of minor intersections and street segments that are part of the currently proposed development. The study area of this report is more focused than the Sketch Plan.

RECENT TRAFFIC REPORTS

- LSC completed an updated master traffic study (MTIS) for the entire Sterling Ranch development, dated March 17, 2023. Appendix Table 1 includes a link to the El Paso County Electronic Development Application Review Program (EDARP) page where a copy of the latest version of that MTIS can be obtained.
- A list of other traffic studies within Sterling Ranch and in the vicinity of area of study completed within the past five years (that LSC is aware of) is attached for reference (Appendix Table 1).
- El Paso County is currently studying the Briargate Stapleton Corridor as part of a Pikes Peak Rural Transportation Authority (PPRTA) study. A draft version of the Briargate-Stapleton Corridor Study by Wilson & Company was published December 9, 2021.

EXISTING ROAD AND TRAFFIC CONDITIONS

The adjacent streets are shown in Figure 1 and are described below. Copies of the 2016 El Paso County Major Transportation Corridors Plan (MTCP), 2040 Roadway Plan, and 2016 MTCP 2060 Corridor Preservation Plan with the site location identified on them have been attached to this report.

Vollmer Road is currently a five-lane urban street within the City of Colorado Springs limits between Black Forest Road and Cowpoke Road and a two-lane, rural, paved roadway north of Cowpoke Road extending to north of Hodgen Road. Improvements to a section of Vollmer Road in the vicinity of the future Marksheffel Road are currently under construction. In the southbound direction, Vollmer Road has a posted speed limit of 45 mph. South of Cowpoke Road, Vollmer Road has a 40-mph posted speed limit. The 2040 El Paso County *Major Transportation Corridors Plan (MTCP)* and the 2023 Sterling Ranch master traffic study show Vollmer Road as a four-lane Urban Minor Arterial in the vicinity of the site. Note: The new Connect COS City of Colorado Springs transportation plan shows Vollmer as a Principal Arterial.

Marksheffel Road is a Principal Arterial extending north from the City of Fountain to Woodmen Road. Marksheffel Road is planned to ultimately be widened to six lanes and extended north and west from Woodmen Road to connect to Research Parkway at Black Forest Road. Marksheffel Road is shown as a four-lane Principal Arterial through the site on the El Paso County *MTCP*. The City of Colorado Springs intends to take ownership and maintenance of Marksheffel Road when it is constructed from Vollmer to the east and south to where it will connect to the segment constructed north of Woodmen Road in the City.

The section of Marksheffel Road adjacent to Sterling Ranch is currently under construction on 107 feet of right-of-way to the City's required cross section(s) and criteria. It is anticipated that Marksheffel Road will be connected between Vollmer Road and Woodmen Road by the end of 2023.

Briargate Parkway is a six-lane, Principal Arterial that extends east from I-25 to Grand Lawn Circle (about one-half mile east of Powers Boulevard). Briargate Parkway is planned ultimately to extend to Towner Drive. The segment of Briargate Parkway between Vollmer Road and Sterling Ranch Road is planned to be constructed in the short term with the Sterling Ranch East Preliminary Plan.

Sterling Ranch Road is a planned Non-Residential Collector shown extending through the Sterling Ranch development between Marksheffel Road and the north end of the Sketch Plan area (Arroya Road). The segment between Marksheffel Road and Dines Boulevard has been constructed and the segment between Dines Boulevard and Briargate Parkway will be constructed in the short term with the Sterling Ranch East Preliminary Plan.

LAND USE AND ACCESS

Copper Chase at Sterling Ranch is planned to include 138 lots for single-family homes and is located east of Vollmer Road and north of the segment of Marksheffel Road currently under construction. Figure 3 shows the proposed site plan.

Access is proposed to Alzada Drive and Bynum Drive. Both of these existing "Urban Local" roadways are part of the approved Sterling Ranch Filing No. 2. The Copper Chase at Sterling Ranch access points are consistent with the access assumed in the TIS for that filing.

Copper Chase at Sterling Ranch was included in the Sterling Ranch Master TIS as Traffic Analysis Zone (TAZ) 5. Traffic projected to be generated by land uses within this zone was also included as part of the short-term background traffic volumes in the *Sterling Ranch East Rezoning and Preliminary Plan TIS*. The land use and access currently proposed are consistent with what was assumed in the *Master TIS and the Sterling Ranch East Rezoning and Preliminary Plan TIS*.

Intersection Sight Distance

Figure 4 shows a sight-distance analysis at the proposed intersections to Alzada Drive and Bynum Drive. Based on a design speed of 25 miles per hour (mph) and the criteria contained in Table 2-21 of the *Engineering Criteria Manual (ECM)*, the required intersection sight distance at the future intersections is 280 feet. Based on the criteria contained in Table 2-17 of the *ECM*, the required stopping sight distance approaching this intersection is 155 feet. As shown in Figure 3, both proposed intersections analyzed will meet the criteria.

Pedestrian and Bicycle Analysis

Figure 5 shows the location of all planned trails and sidewalks in the vicinity of the site. Connections are also proposed to the planned future Sand Creek Regional Trail (west of Dines Boulevard), as shown in the attached map.

A detached sidewalk will be provided along the west side of Sterling Ranch Road. The multi-use paved shoulder on Sterling Ranch Road will accommodate bicycles.

There are no existing schools within two miles of the site. However, multiple school sites are planned within Sterling Ranch northeast of the intersection of Sterling Ranch Road/Dines Boulevard. School pedestrians would travel to/from the intersection of Sterling Ranch/Dines on the sidewalks and trails identified in Figure 4. The need for additional pedestrian facilities and/or school crossings should be identified when the school site(s) are developed.

Safety Analysis

Most of the roadways in the vicinity of the site have not yet been constructed. The Colorado State Patrol (CSP) provided LSC with crash history data for Vollmer Road between Tahiti Drive and Burgess Road from September 2019 through September 2022. During the reported time period, there were twelve reported crashes. Of the twelve reports, ten were single-vehicle non-intersection-related crashes on Vollmer Road. One crash involved a southbound vehicle that turned right onto Poco Road and crashed into several cars parked on Poco Road partially in the lane. The only intersection related crash occurred in June 2022. A vehicle heading northbound on Vollmer Road was slowing to turn left at Lochwinnoch Road and the vehicle behind them attempted to pass on the left side. The crash history data has been attached.

TRIP GENERATION

Copper Chase at Sterling Ranch site-generated vehicle trips have been estimated using the nationally-published trip-generation rates from *Trip Generation*, 11th Edition, 2021 by the Institute of Transportation Engineers (ITE). Table 1 shows the trip-generation estimate. The trip-generation estimate is consistent with the estimate assumed in the Sterling Ranch Master TIS and the Sterling Ranch East Rezoning and Preliminary Plan TIS for the same parcels.

Copper Chase at Sterling Ranch is expected to generate 1,301 vehicle trips on the average weekday, with about half entering and half exiting the site during a 24-hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 25 vehicles would enter and 71 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between 4:15 and 6:15 p.m., about 82 vehicles would enter and 48 vehicles would exit the site.

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the street and roadway system serving the site is an important factor in determining the site's traffic impacts. The distribution estimates for short-term and long-term residential related traffic are shown in Figure 6. The short-term directional-distribution estimate assumes the short-term roadway network shown in Figure 2 only and the long-term directional distribution estimate assumes buildout of the roadway network. The directional-distribution estimates are based, in part, on the estimates contained in the sketch plan TIS report. Factors include: the location of the site with respect to the Colorado Springs metropolitan area, the planned access system for the site, the street and roadway system serving the site, and the land uses proposed for the site.

When the distribution percentages (from Figure 6) are applied to the new, external trip-generation estimates (from Table 1), the resulting site-generated traffic volumes can be determined. Figures 7 and 8 show the short-term and long-term site-generated traffic volumes, respectively. The short-term site-generated traffic volumes assume only the street network

shown in Figure 2 and the long-term site-generated traffic volumes assume buildout of the area roadway network.

BACKGROUND TRAFFIC VOLUMES

Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation of site-generated traffic volumes. Background traffic (for a specified horizon year) includes the through traffic and the traffic generated by nearby developments (existing and planned, including traffic generated by existing and planned developments within the greater Sterling Ranch overall development) but assumes zero traffic generated by land uses within Copper Chase at Sterling Ranch.

Short-Term Scenario Background Conditions

Figure 9 shows the projected volumes for the short-term Background scenario. This scenario includes traffic to be traffic generated in the short term by buildout of Homestead at Sterling Ranch, Branding Iron at Sterling Ranch, Sterling Ranch Filings 2-4, Homestead North at Sterling Ranch Filings 1-3, the Retreat at TimberRidge Filings 1-3, Sterling Ranch East Filings 1 and 2, and the planned FourSquare at Sterling Ranch East development.

Long-Term Scenario Background Conditions

Figure 10 shows the projected 2043 Background daily traffic volumes on key street segments at the key area intersections. These volumes assume buildout of the area street network, including the completion of Marksheffel Road between Vollmer Road and Black Forest Road, Briargate Parkway between Meridian Road and Black Forest Road, and Sterling Ranch Road between Marksheffel Road and Briargate Parkway.

The 2043 Background traffic volumes are estimates by LSC, based on the traffic projections in the LSC February 10, 2023 Master TIS report. The 2043 Background daily traffic volumes assume buildout of all other land uses within the Sterling Ranch Master Plan.

TOTAL TRAFFIC VOLUMES

Short-Term Scenario Total Conditions

Figure 11 shows the projected volumes for the short-term total scenario. These volumes are the sum of the short-term Background scenario (from Figure 9) plus the short-term site-generated traffic volumes (from Figure 7).

Copper Chase at Sterling Ranch

Long-Term Scenario Total Conditions

Figure 12 shows the projected volumes for the 2043 total scenario. These volumes are the sum of the 2043 Background scenario (from Figure 10) plus the long-term site-generated traffic volumes (from Figure 8).

LEVELS OF SERVICE

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections. Table 1 shows the level of service delay ranges.

Table 2: Intersection Levels of Service Delay Ranges

	Signalized Intersections	Unsignalized Intersections
Level of Service	Average Control Delay (seconds per vehicle)	Average Control Delay (seconds per vehicle) ⁽¹⁾
Α	10.0 sec or less	10.0 sec or less
В	10.1-20.0 sec	10.1-15.0 sec
С	20.1-35.0 sec	15.1-25.0 sec
D	35.1-55.0 sec	25.1-35.0 sec
Е	55.1-80.0 sec	35.1-50.0 sec
F	80.1 sec or more	50.1 sec or more

⁽¹⁾ For unsignalized intersections, if V/C ratio is greater than 1.0 the level of service is LOS F, regardless of the projected average control delay per vehicle.

The study area stop-sign-controlled intersections have been analyzed based on the unsignalized-intersection analysis procedures from the Highway Capacity Manual, 6th Edition by the Transportation Research Board. The future signalized intersections of Vollmer Road/Marksheffel Road and Sterling Ranch Road /Marksheffel Road have been analyzed using Synchro.

Vollmer Road / Marksheffel Road

Marksheffel Road is planned to be constructed between Vollmer Road and Sterling Ranch Road in the short-term future. The intersection of Marksheffel/Vollmer is initially planned to be stop-sign controlled. Based on the projected short-term total traffic volumes, the westbound left-turn movement is projected to operate at LOS D during the morning peak hour and LOS E during the afternoon peak hour.

By 2043, it was assumed that Marksheffel Road would be constructed west to Briargate Parkway and that the intersection of Vollmer/Marksheffel will be converted to traffic-signal control. The intersection of Vollmer/Marksheffel is projected to operate at an overall LOS C or better during the peak hours as a signalized intersection, based on the projected 2043 total traffic volumes.

Sterling Ranch Road / Marksheffel Road

Marksheffel Road is planned to be constructed between Vollmer Road and Sterling Ranch Road in the short-term future. The intersection of Marksheffel/Sterling Ranch is initially planned to be stop-sign controlled. Based on the projected short-term total traffic volumes, the southbound left-turn movement is projected to operate at LOS C during the morning peak hour and LOS D during the afternoon peak hour.

By 2043, it was assumed that Sterling Ranch would be constructed south of Marksheffel Road and that the intersection of Sterling Ranch/Marksheffel will be converted to traffic-signal control. The intersection of Sterling Ranch/Marksheffel is projected to operate at an overall LOS C or better during the peak hours as a signalized intersection, based on the projected 2043 total traffic volumes.

SIGNAL WARRANT THRESHOLD ANALYSIS – AM AND PM PEAK HOURS

The intersections of Marksheffel/Vollmer and Marksheffel/Sterling Ranch were analyzed to determine if the thresholds for Four-Hour and/or Eight-Hour Vehicular-Volume Traffic-Signal Warrant thresholds would be reached or exceeded, based on the projected short-term traffic volumes.

The off-peak-hour volumes are estimates by LSC based on the peak-hour traffic volumes, 72-hour machine counts conducted by LSC on Vollmer Road in November 2020, and vehicle time-of-day distribution data for single-family homes published by the Institute of Transportation Engineers.

Marksheffel/Vollmer

Table 3 shows the results of the analysis for the intersection of Marksheffel/Vollmer. As shown in Table 3, in the short-term, only one of the hours analyzed is projected to meet the thresholds for an Eight-Hour Vehicular-Volume Traffic-Signal Warrant and none of the hours analyzed are projected to meet the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant. This analysis indicates that traffic-signal warrant(s) will likely **not** be met at the intersection of Marksheffel/Vollmer in the short-term.

Marksheffel/Sterling Ranch

Table 4 shows the results of the analysis for the intersection of Marksheffel/Sterling Ranch. As shown in Table 4, in the short-term, only three of the hours analyzed are projected to meet the

thresholds for an Eight-Hour Vehicular-Volume Traffic-Signal and only one of the hours analyzed are projected to meet the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant. This analysis indicates that traffic-signal warrant(s) will likely **not** be met at the intersection of Marksheffel/Sterling Ranch in the short-term.

SUBDIVISION STREET CLASSIFICATIONS

All of the internal streets within Copper Chase at Sterling Ranch should be classified as Urban Local. Figure 13 shows the recommended street classifications for the internal streets and the streets in the vicinity of the site.

DEVIATON REQUESTS

The following deviation requests to the criteria contained in the El Paso County *Engineering Criteria Manual (ECM)* have been included with this submittal:

- A deviation from the maximum mid-block ped ramp spacing
- A deviation from the number of ped ramps at a "T" intersection
- A deviation from the minimum center-line radius and minimum intersection-spacing standards.

ROADWAY IMPROVEMENTS

Table 6 from the Sterling Ranch East Rezoning and Preliminary Plan TIS contained a summary of needed area improvements. Appendix Table 2 is a copy of this table with the improvements needed either prior to or with Copper Chase at Sterling Ranch highlighted. Please see Figure 14 for a map of the key street-segment locations. These recommendations are consistent with the LSC Sketch Plan TIS report.

The following auxiliary lanes shown will be required with Copper Chase at Sterling Ranch development if not completed with Sterling Ranch Filing No. 2. Note: These are shown on the construction plans by JR Engineering for these adjacent roadways and construction is underway

- Marksheffel/Vollmer
 - 155-foot northbound right-turn deceleration lane on Vollmer approaching Marksheffel, plus a160-foot taper
 - 310-foot southbound left-turn lane on Vollmer approaching Marksheffel, plus a 160-foot taper.
 - 425-foot westbound left-turn lane on Marksheffel approaching Vollmer, plus a 200-foot taper
 - 235-foot westbound right-turn deceleration lane on Marksheffel approaching Vollmer, plus a 200-foot taper

- Sterling Ranch/Marksheffel
 - 470-foot eastbound left-turn lane on Marksheffel approaching Sterling Ranch, plus a 200-foot taper
 - A 285-foot southbound left-turn lane on Sterling Ranch approaching Marksheffel, plus a 90-foot reverse-curve taper
 - A 155-foot southbound right-turn lane on Sterling Ranch approaching Marksheffel, plus a 160-foot taper
- Sterling Ranch/Bynum
 - A 155-foot northbound left-turn lane on Sterling Ranch Road approaching Bynum, plus a 90-foot reverse-curve taper

ROADWAY IMPROVEMENT FEE PROGRAM

This project will be required to participate in the El Paso County Road Improvement Fee Program. Copper Chase at Sterling Ranch will join the ten-mil PID. The 2019 ten-mil PID building permit fee portion associated with this option is \$1,221 per single-family dwelling unit. Based on 158 lots, the total building permit fee would be \$168,498. Note: program fees are subject to change.

* * * * *

Please contact me if you have any questions regarding this report.

Respectfully Submitted,

LSC TRANSPORTATION CONSULTANTS, INC.

By: Jeffrey C. Hodsdon, P.E. Principal

JCH/KDF:jas

Enclosures: Tables 1, 3, and 4

Figures 1-14

Level of Service Reports Appendix Tables 1-2



Table 1 Sterling Ranch East Phase 1 Preliminary Plan Copper Chase at Sterling Ranch

					Trip	Generation R	ates ⁽¹⁾			Total	Trip Gene	rated	
ITE					AM Pe	ak Hour	PM Pea	ık Hour		AM Pea	ak Hour	PM Pea	k Hour
Code	ITE Land Use	Quantity	Unit	Daily	ln	Out	In	Out	Daily	In	Out	In	Out
210	Single-Family Detached Housing	138	DU ⁽²⁾	9.43	0.18	0.52	0.59	0.35	1,301	25	71	82	48
210	Single-ramily Detached Housing	130	DO.	9.43	0.10	0.52	0.59	0.33	1,301	25	/ 1	02	

Notes:

(1) Source: "Trip Generation, 11th Edition, 2021" by the Institute of Transportation Engineers (ITE)

(2) DU = Dwelling Unit

Source: LSC Transportation Consultants, Inc.

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Table 3 Traffic Signal Warrant Analysis Marksheffel Road/Vollmer Road

												Warr	ant Analys	is ⁽¹⁾				
								Warr	ant 1: Figh	ıt Hour Vel	hicular Volu	ıme Evalu	ation		Warrant		ur Vehicular V Jation	/olume
													eshold Me	17	Short-Term E	Background	Short-Tei	rm Total
	Short	Torm	Copper Chase a	t Starling Danch							Short		Conoia ino	•		1		
	Backgrou		Generate		Short-Term	Total Traffia		Warrant T	hrachalda		Backg		Short-Te	rm Total	Warrant	Warrant	Warrant	Warrant
							0			tion B	•				Threshold	Threshold	Threshold	Threshold
	Major ⁽²⁾	Minor ⁽³⁾	Major	Minor	Major	Minor		ition A			Conditio		Conditio	Conditio	Minor	Met? WB	Minor	Met? WB
Hour	Vollmer	Marksheffel	Vollmer	Marksheffel	Vollmer	Marksheffel	Major	Minor	Major	Minor	n A	n B	n A	n B	Minimum	WD	Minimum	WD
Short-Term To	otal Traffic ⁽⁴⁾																	
12-1 AM	45	2	1	1	46	3	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
1-2 AM	23	2	1	1	24	3	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
2-3 AM	16	0	1	0	17	0	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
3-4 AM	25	2	1	1	26	3	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
4-5 AM	40	8	1	2	41	10	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
5-6 AM	105	19	1	6	106	25	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
6-7 AM	312	57	7	17	319	74	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
7-8 AM	754	99	13	29	767	128	600	150	900	75	No	No	No	No	223	No	217	No
8-9 AM	841	83	14	24	855	107	600	150	900	75	No	No	No	No	190	No	186	No
9-10 AM	728	52	13	15	741	67	600	150	900	75	No	No	No	No	236	No	230	No
10-11 AM	843	52	18	15	861	67	600	150	900	75	No	No	No	No	189	No	185	No
11-12 PM	947	50	22	15	969	65	600	150	900	75	No	No	No	No	163	No	158	No
12-1 PM	791	49	22	15	813	64	600	150	900	75	No	No	No	No	205	No	197	No
1-2 PM	783	52	23	16	806	68	600	150	900	75	No	No	No	No	209	No	199	No
2-3 PM	901	55	26	17	927	72	600	150	900	75	No	No	No	No	175	No	168	No
3-4 PM	956	53	32	16	988	69	600	150	900	75	No	No	No	No	161	No	153	No
4-5 PM	992	66	40	20	1032	86	600	150	900	75	No	No	No	Yes	152	No	140	No
5-6 PM	829	65	40	20	869	85	600	150	900	75	No	No	No	No	193	No	183 292	No
6-7 PM	565	52	32	16	597	68	600	150	900	75	No	No	No	No	308	No		No
7-8 PM	353	38	23	11	376	49	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
8-9 PM	286	27	24	8	310	35	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
9-10 PM	183	21	17	6	200	27	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
10-11 PM	103	10	8	3	111	13	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
11-12 AM	54	6	5	2	59	8	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
						Numbers	of Hours	the Warran		rant Met?		0	0 N	1 lo	}	0 No		0 No

- (1) Thresholds are based on 2 or more lanes on the major approach and 1 lane on the minor approach (Warrant evaluation assuming the westbound left turn only for the minor street) (2) The major street traffic includes all movements (left, through, and right) (3) The minor street traffic includes only the left turns from the minor street

- (5) Off peak hour traffic volumes are based on the projected peak hour traffic volumes, 72-hour machine counts conducted on Vollmer Road in November 2020 and vehicle time-of-day distribution data for single-family residential published by the Institute of Transportation Engineers

Source: LSC Transportation Consultants, Inc.

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Table 4 Traffic Signal Warrant Analysis Marksheffel Road/Sterling Ranch Road

							-					Warr	ant Analys	is ⁽¹⁾				
															Warrant	2: Four Hou	ır Vehicular V	olume
								War	ant 1: Eigh	nt Hour Ve	hicular Vol	ume Evalu	ation			Evalu	ıation	
									<u> </u>		W	arrant Thr	eshold Me	t?	Short-Term E	Background	Short-Ter	m Total
	Short-	Torm	Connor Chase a	t Sterling Ranch								-Term		*-				
	Backgroui			ed Traffic	Short-Term	Total Traffia		Warrant T	hresholds		Backg		Short T	erm Total	Warrant	Warrant	Warrant	Warrant
	Backgroui	Minor ⁽³⁾	Generali	l ITAIIIC	Short-reini	Minor		vvarrant i	liresiloius		Dacky	rounu	311011-11	iiii iotai	Threshold	Threshold	Threshold	Threshold
	Major ⁽²⁾	Sterling	Major	Minor Sterling	Maior	Sterling	Cond	ition A	Cond	ition B	Conditio	Conditio	Conditio	Conditio		Met?	Minor	Met?
Hour	Marksheffel	Ranch	Marksheffel	Ranch	Marksheffel	Ranch	Major	Minor	Major	Minor	n A	n B		n B	Minimum	WB	Minimum	WB
-11001	Marksheller	Ranch	Markshellei	Kanch	Markshellel	Ranch	IVIAJOI	WIIIIOI	IVIAJOI	WIIIIOI	II A	II D	n A	II D	Minimum	5	Wilnimum	
Short-Term To	otal Traffic ⁽⁴⁾																	
12-1 AM	28	6	3	1	31	7	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
1-2 AM	13	6	1	1	14	7	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
2-3 AM	11	0	1	0	12	0	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
3-4 AM	14	6	1	1	15	7	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
4-5 AM	21	23	1	3	22	26	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
5-6 AM	49	58	3	7	52	65	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
6-7 AM	147	171	8	21	155	192	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
7-8 AM	335	295	16	36	351	331	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
8-9 AM	377	249	19	30	396	279	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
9-10 AM	326	156	16	19	342	175	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
10-11 AM	389	156	21	19	410	175	600	150	900	75	No	No	No	No	Low Volume	No	385	No
11-12 PM	455	148	27	18	482	166	600	150	900	75	No	No	No	No	363	No	349	No
12-1 PM	460	148	28	19	488	167	600	150	900	75	No	No	No	No	360	No	346	No
1-2 PM	476	156	31	20	507	176	600	150	900	75	No	No	No	No	Low Volume	No	337	No
2-3 PM	550 620	164 158	36 44	21	586 664	185 178	600	150 150	900	75	No Yes	No	No	No	315 282	No No	297 264	No No
3-4 PM 4-5 PM	702	198	55	20 25	757	223	600 600	150	900 900	75 75		No	Yes	No	282	No	204	Yes
4-5 PM 5-6 PM	702 640	198	55	25 25	757 694	223	600	150	900	75 75	Yes Yes	No No	Yes Yes	No No	274	No	252	No Yes
6-7 PM	490	156	45	20	535	176	600	150	900	75	No	No	No	No	Low Volume	No	323	No
7-8 PM	335	114	32	14	367	128	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
8-9 PM	316	82	33	10	349	92	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
9-10 PM	218	63	24	8	242	71	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
10-11 PM	112	29	12	4	124	33	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
11-12 AM	64	18	7	2	71	20	600	150	900	75	No	No	No	No	Low Volume	No	Low Volume	No
				<u> </u>				the Warran				0	3	0		0		1
										rrant Met?		lo		lo -	1	No		No
									wa	wiet:			<u> </u>	••	1	140		110

Notes

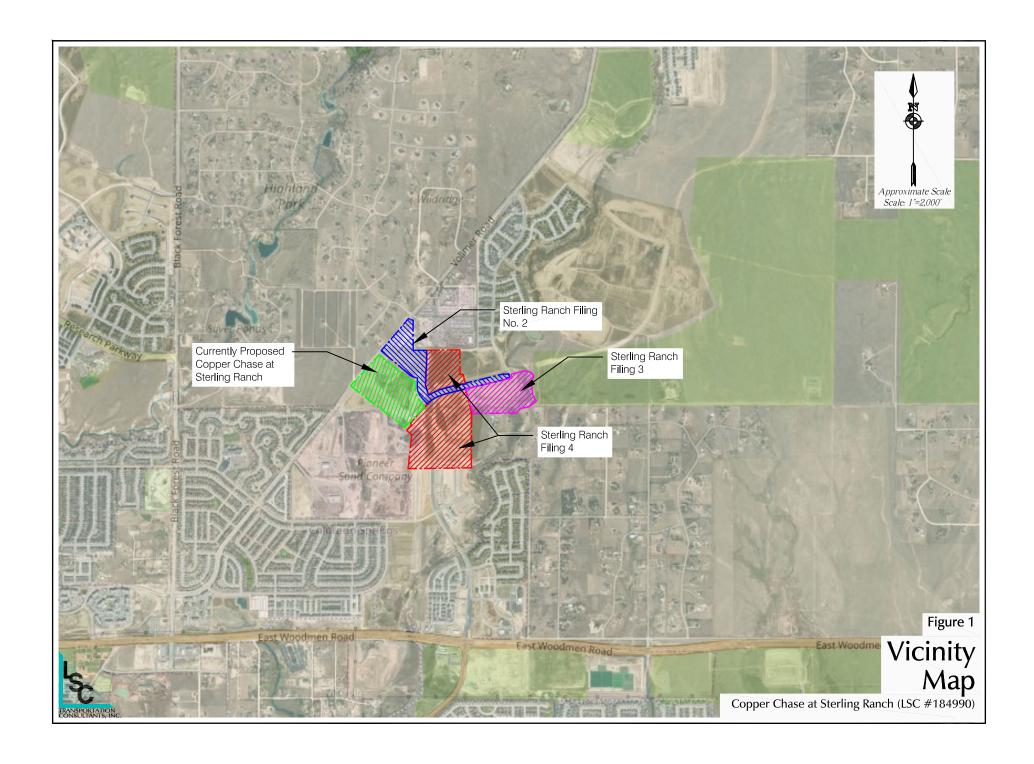
- (1) Thresholds are based on 2 or more lanes on the major approach and 1 lane on the minor approach (Warrant evaluation assuming the southbound left turn only for the minor street)
- (2) The major street traffic includes all movements (left, through, and right)
- (3) The minor street traffic includes only the left turns from the minor street
- (4) Off peak hour traffic volumes are based on the projected peak hour traffic volumes, 72-hour machine counts conducted on Vollmer Road in November 2020 and vehicle time-of-day distribution data for single-family residential published by the Institute of Transportation Engineers

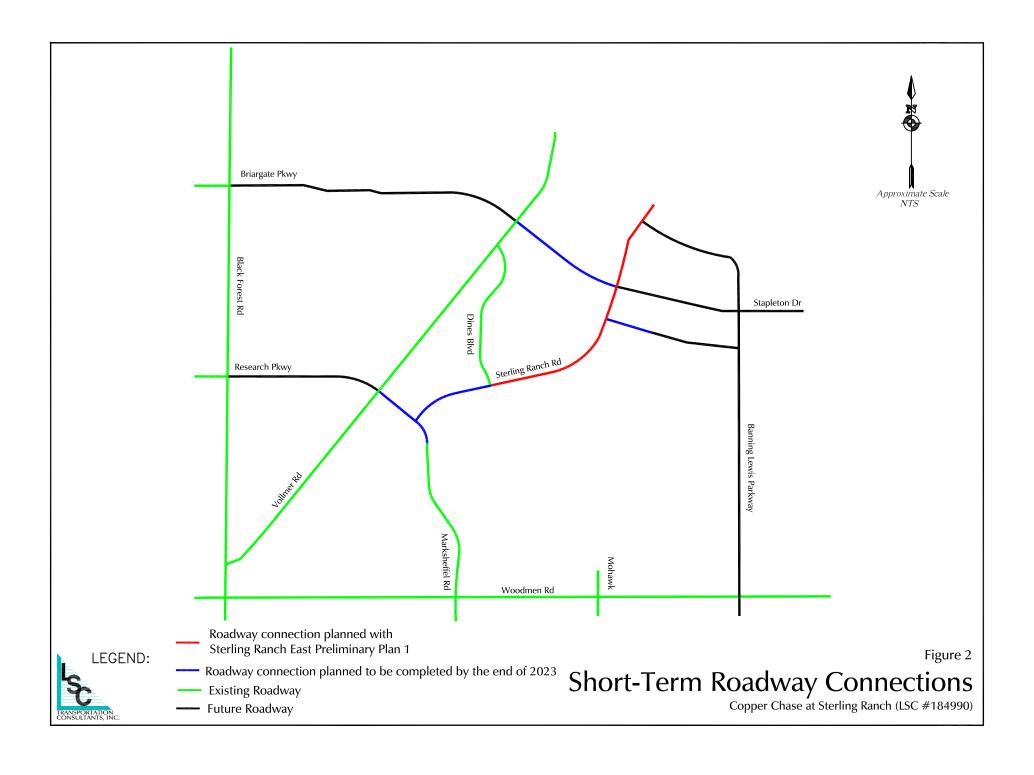
Source: LSC Transportation Consultants, Inc.

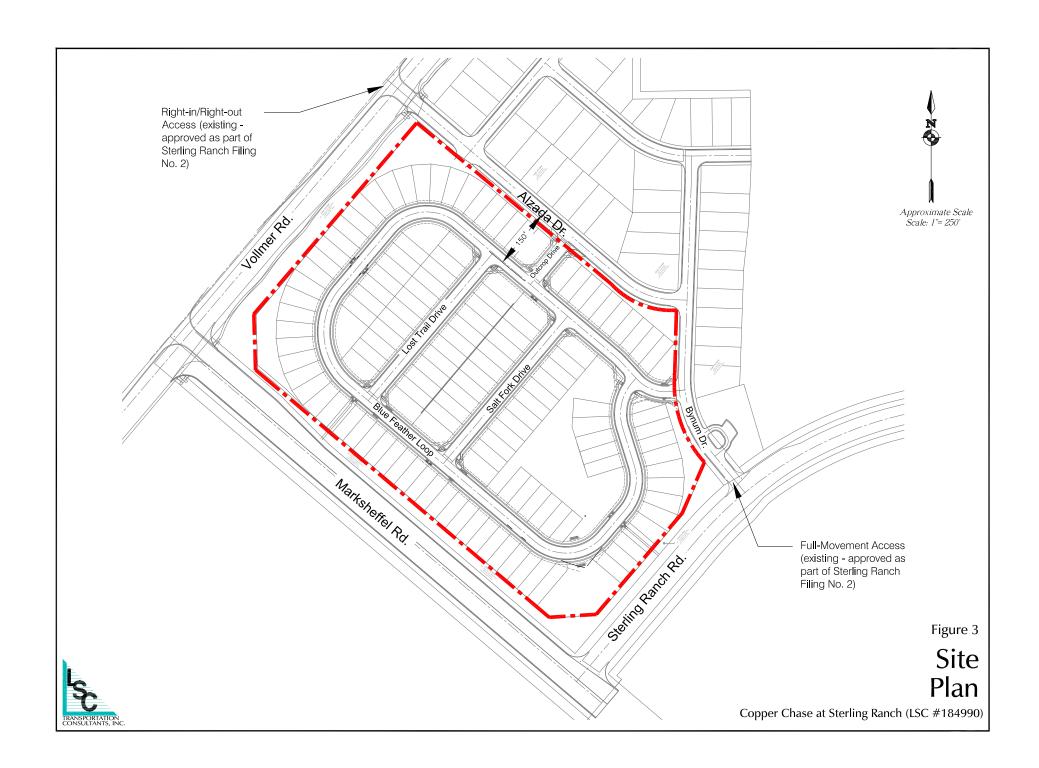
Apr-23

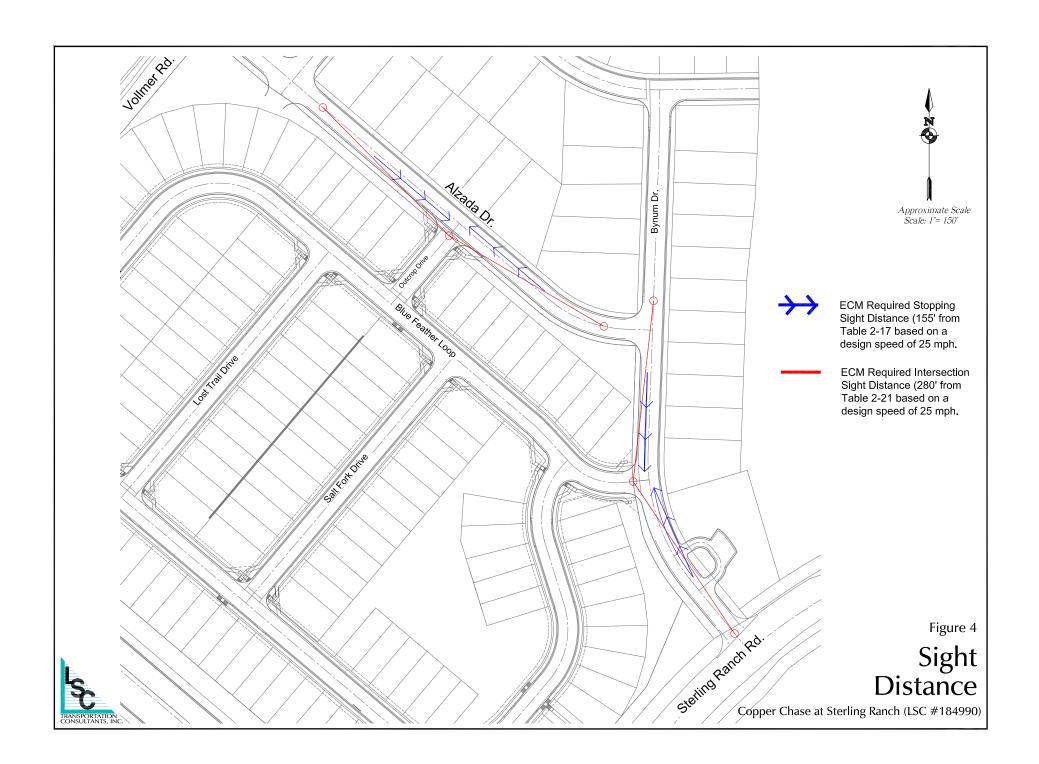
Figures 1-14

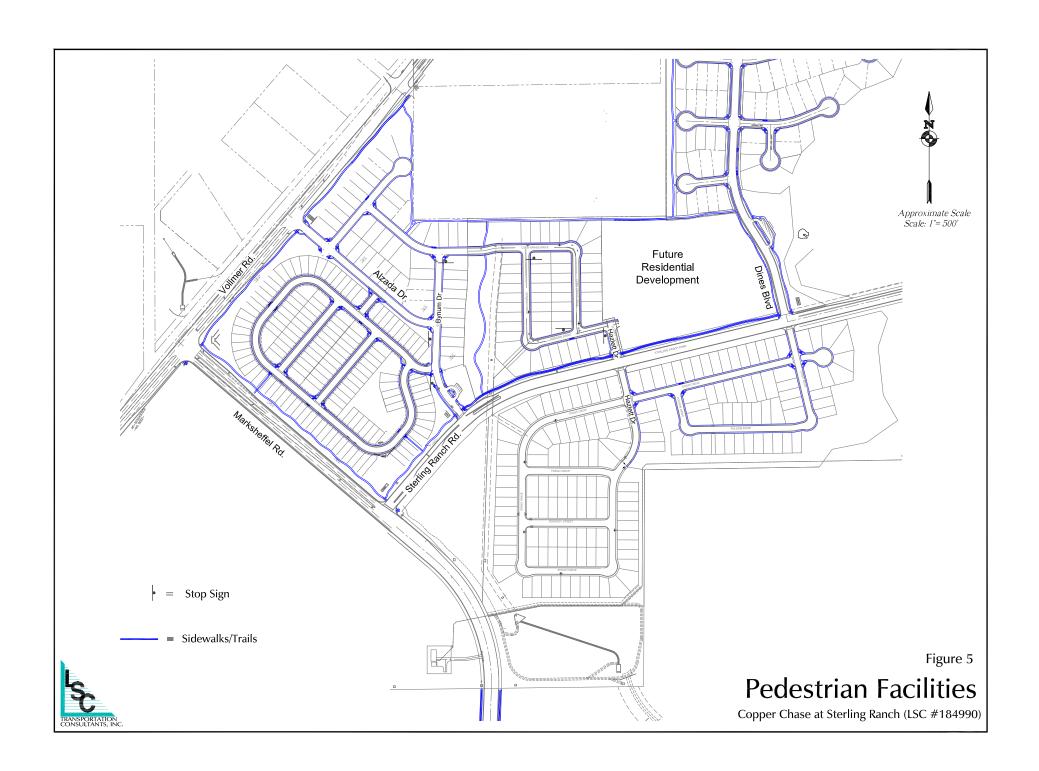


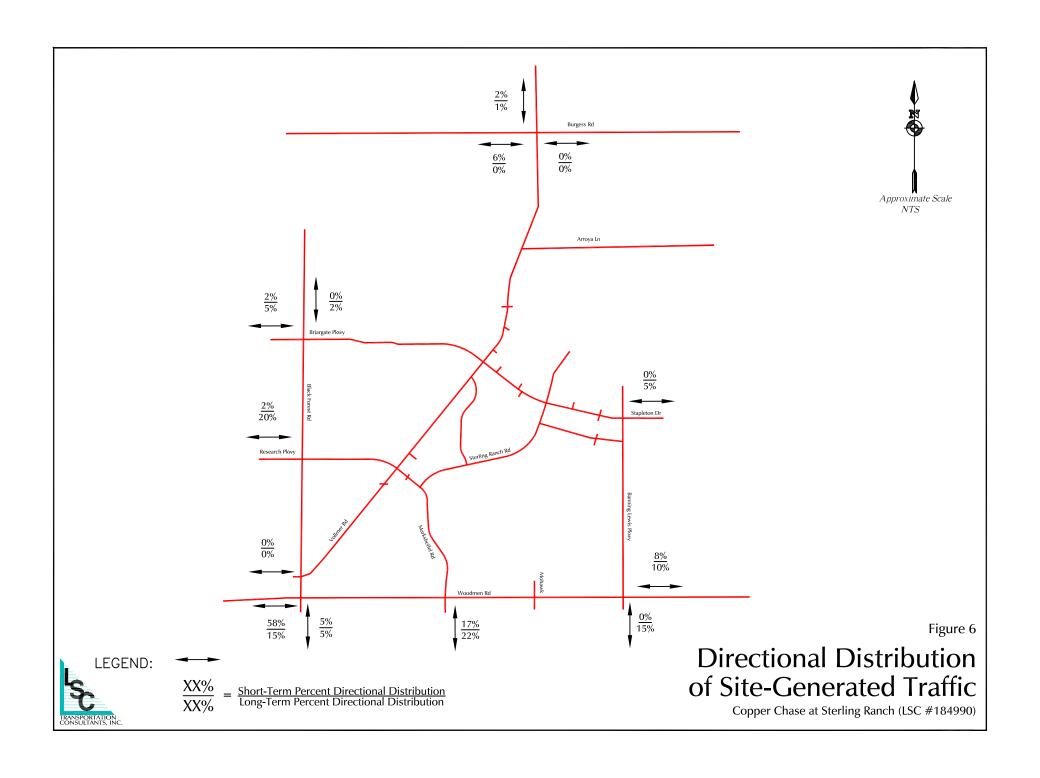


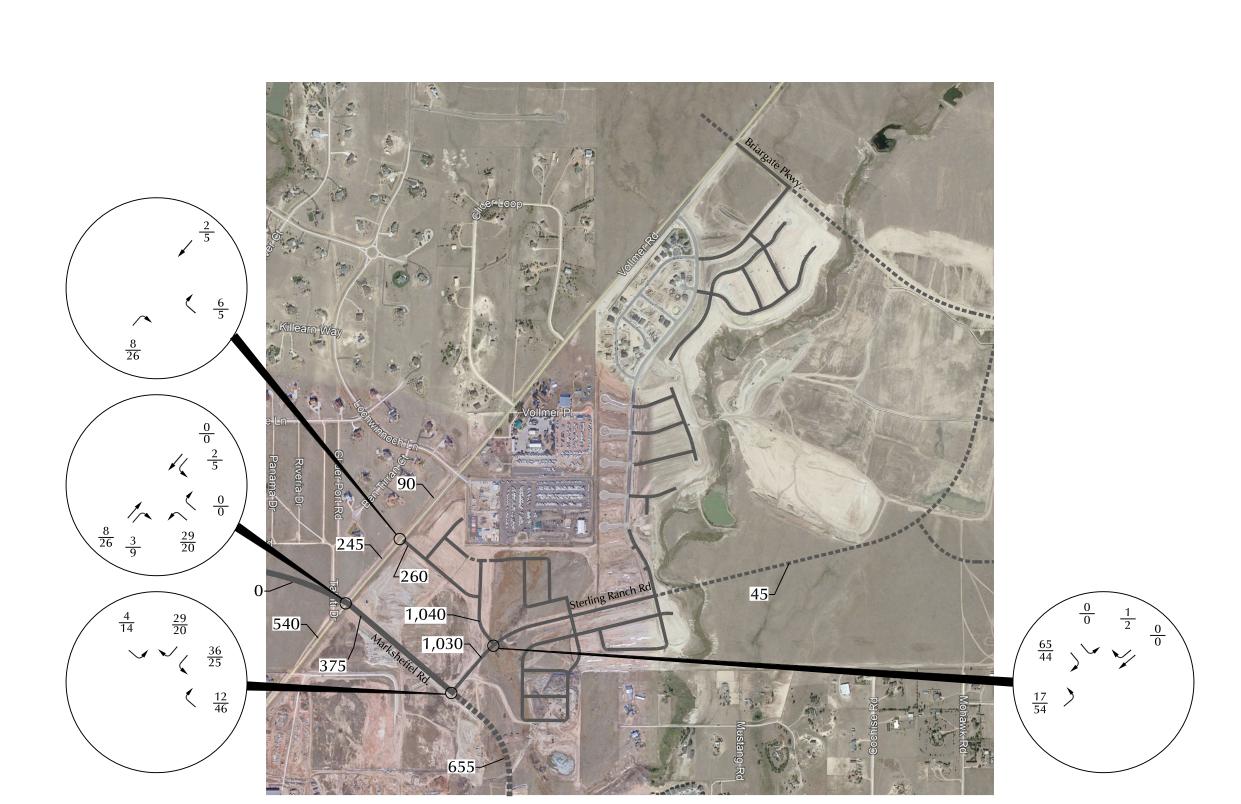












LEGEND:

\frac{XX}{XX} = \frac{AM Weekday Peak-Hour Traffic (vehicles per hour)}{PM Weekday Peak-Hour Traffic (vehicles per hour)}

X,XXX= Average Daily Traffic (vehicles per day)

Figure 7

Approximate Scale Scale: 1"= 1,000'







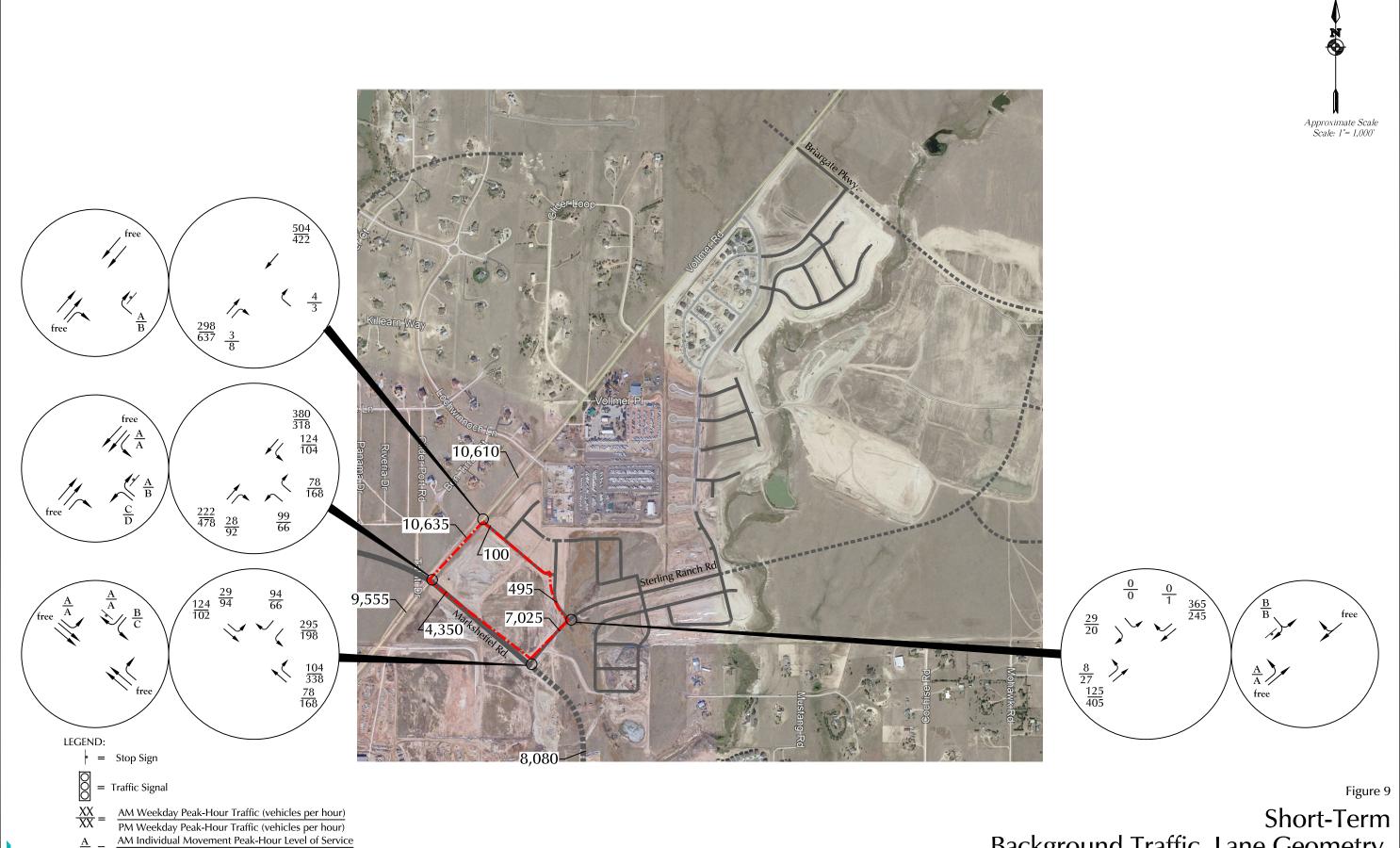
LEGEND:

\frac{XX}{XX} = \frac{AM Weekday Peak-Hour Traffic (vehicles per hour)}{PM Weekday Peak-Hour Traffic (vehicles per hour)}

X,XXX= Average Daily Traffic (vehicles per day)

Figure 8

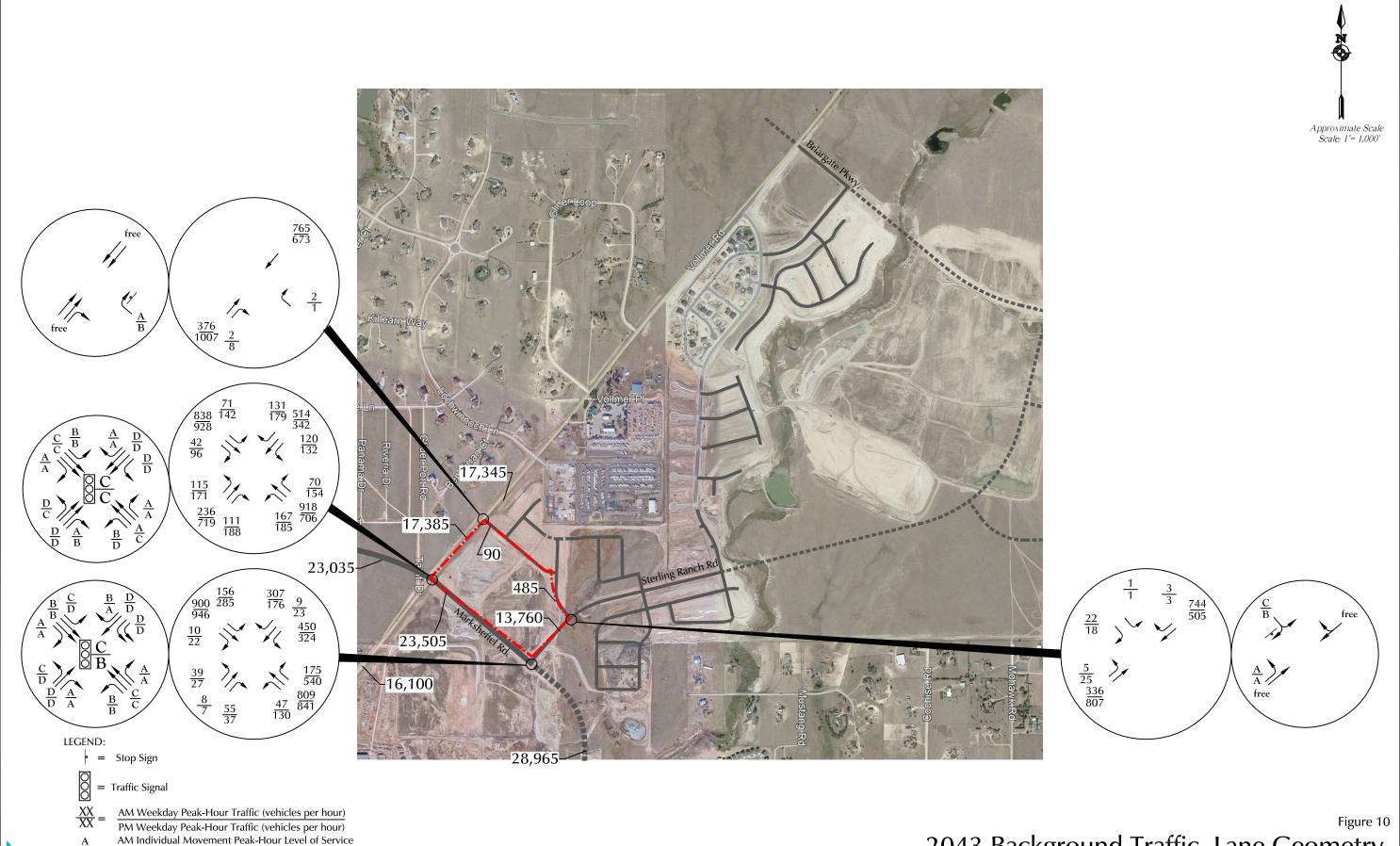
Long-Term Assignment of Site-Generated Traffic



PM Individual Movement Peak-Hour Level of Service AM Entire Intersection Peak-Hour Level of Service PM Entire Intersection Peak-Hour Level of Service

X,XXX = Average Daily Traffic (vehicles per day)

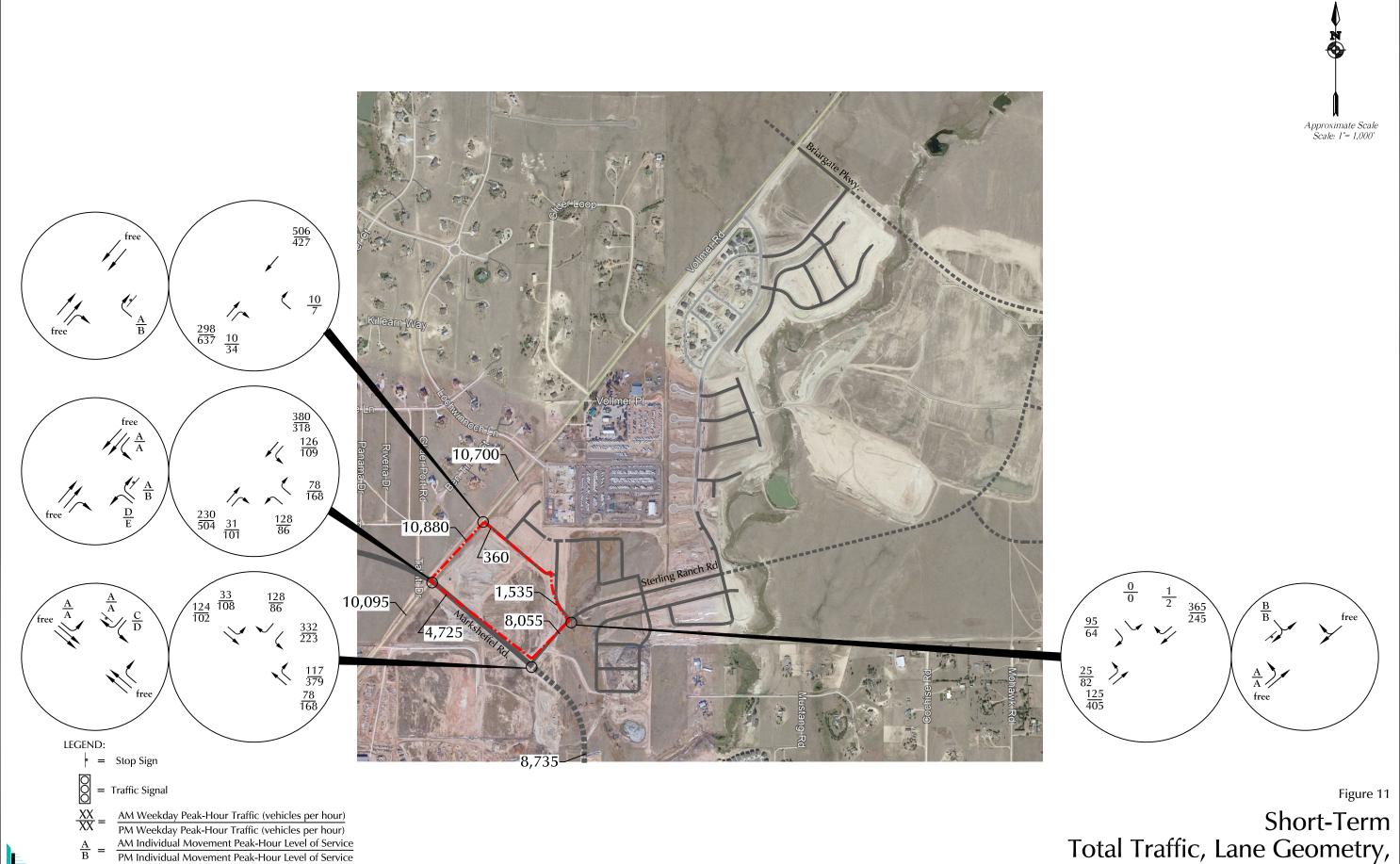
Short-Term Background Traffic, Lane Geometry, Traffic Control and Level of Service



PM Individual Movement Peak-Hour Level of Service AM Entire Intersection Peak-Hour Level of Service PM Entire Intersection Peak-Hour Level of Service

X,XXX = Average Daily Traffic (vehicles per day)

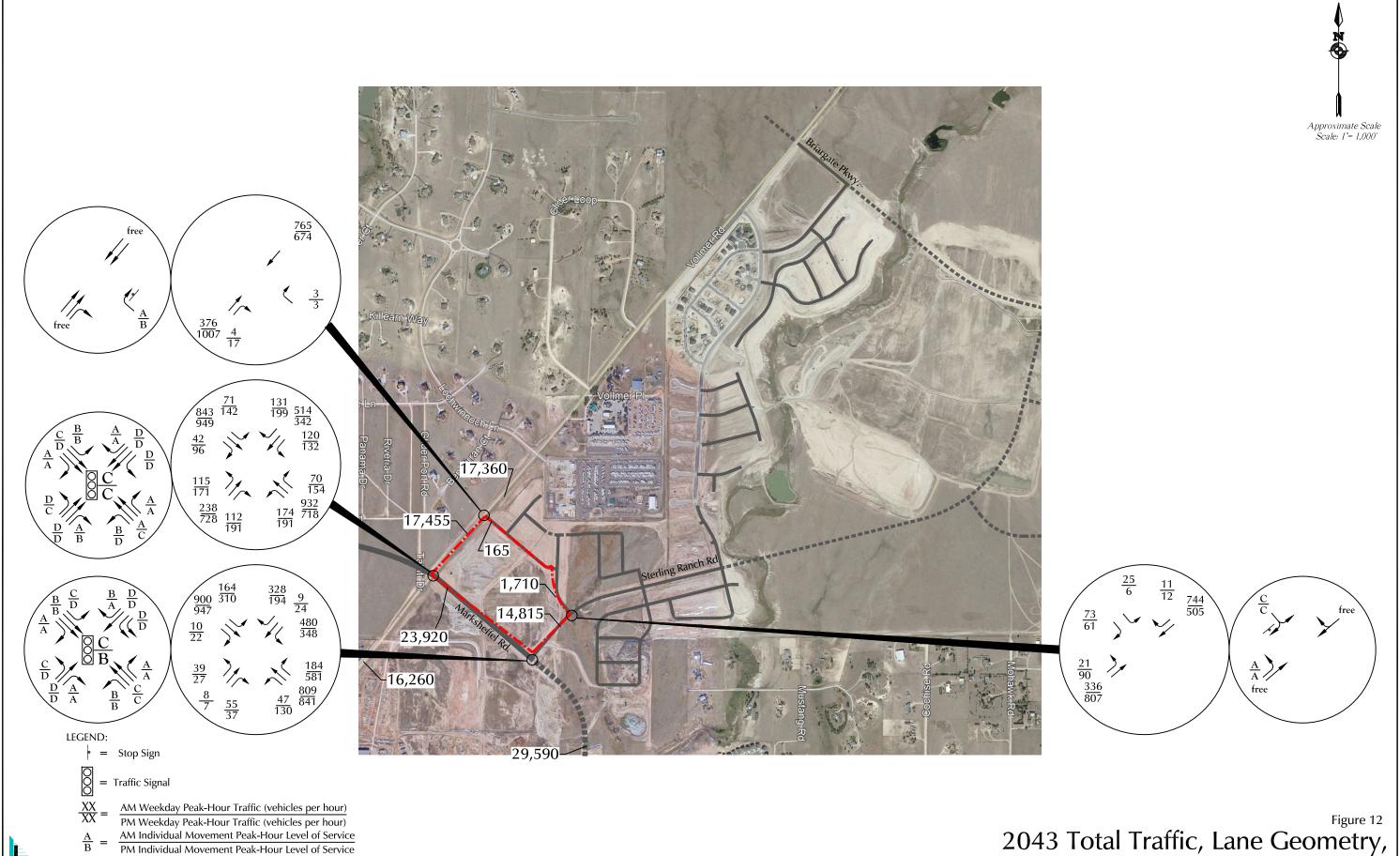
2043 Background Traffic, Lane Geometry, Traffic Control and Level of Service



AM Entire Intersection Peak-Hour Level of Service PM Entire Intersection Peak-Hour Level of Service

X,XXX = Average Daily Traffic (vehicles per day)

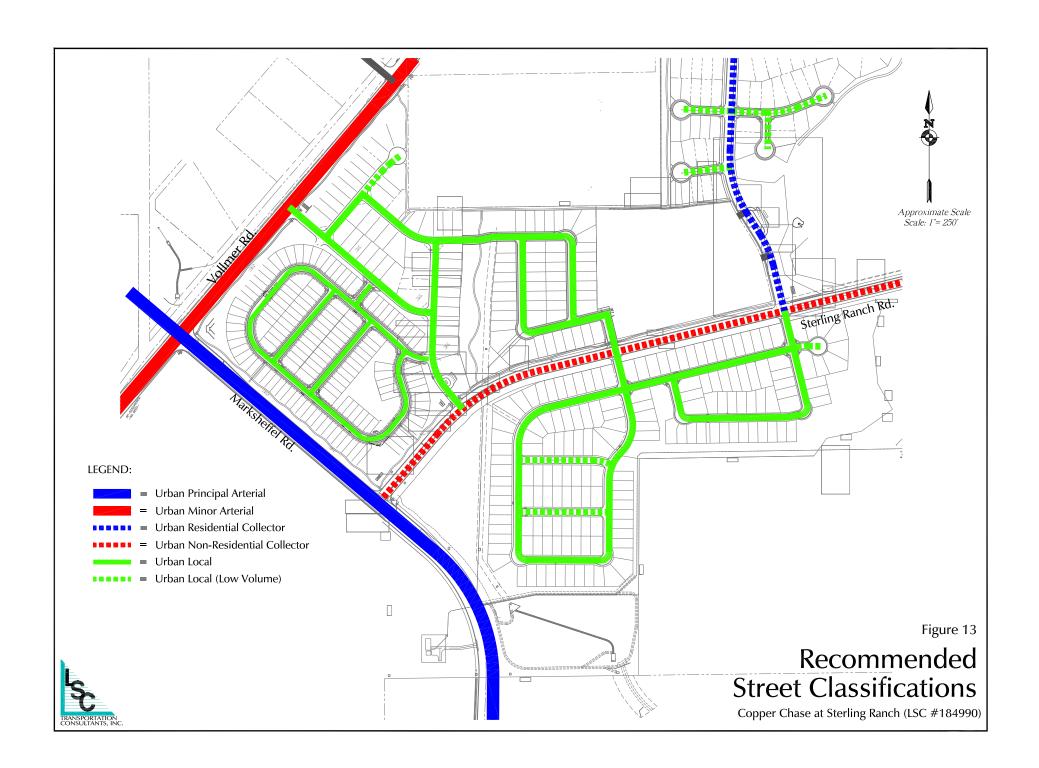
Total Traffic, Lane Geometry, Traffic Control and Level of Service

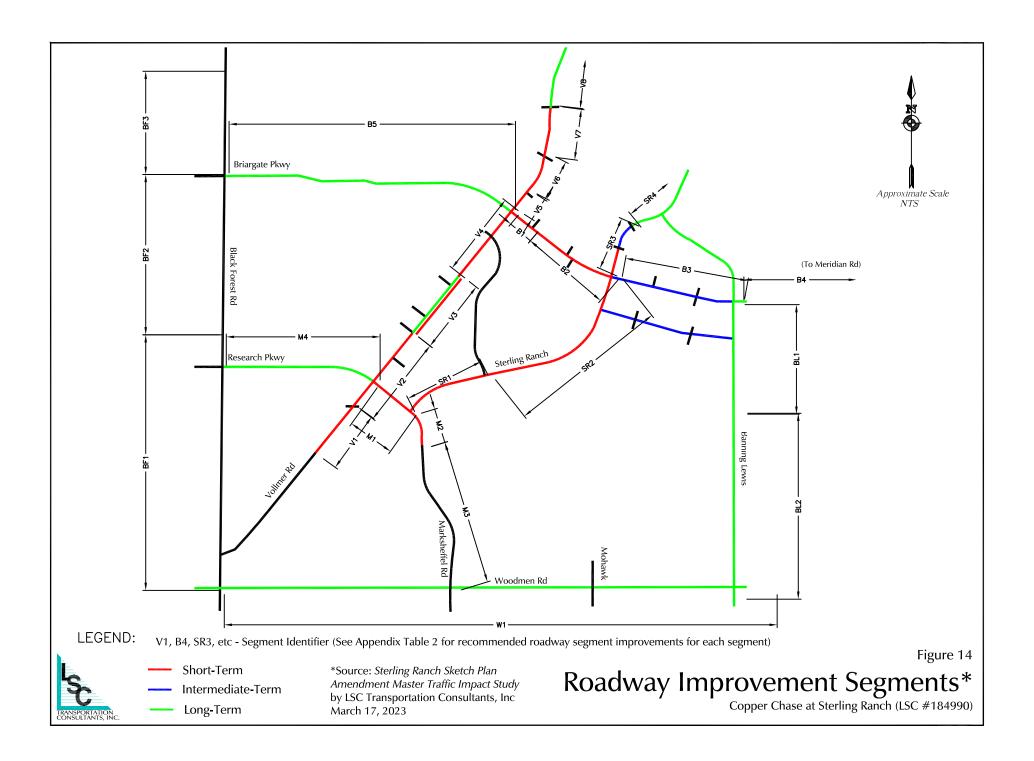


AM Entire Intersection Peak-Hour Level of Service PM Entire Intersection Peak-Hour Level of Service

X,XXX = Average Daily Traffic (vehicles per day)

2043 Total Traffic, Lane Geometry, Traffic Control and Level of Service





Levels of Service



Intersection						
Int Delay, s/veh	4.5					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>ች</u>	70	^	7	104	^
Traffic Vol, veh/h	99	78	222	28	124	380
Future Vol, veh/h	99	78	222	28	124	380
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	155	300	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	116	92	261	33	146	447
	Minor1		//ajor1		Major2	
Conflicting Flow All	777	131	0	0	294	0
Stage 1	261	-	-	-	-	-
Stage 2	516	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	334	894	-	-	1264	-
Stage 1	759	-	-	-	_	-
Stage 2	564	-	_	-	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	295	894	_	_	1264	_
Mov Cap-2 Maneuver	295	-	_	_	-	_
Stage 1	759	_			_	_
Stage 2	499	_			_	
Slaye 2	433	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	18.1		0		2	
HCM LOS	С					
		NIDT	NDD		VDI 0	001
Minor Lane/Major Mvm	<u>it</u>	NBT	NBRV	VBLn1V		SBL
Capacity (veh/h)		-	-	295	894	1264
HCM Lane V/C Ratio		-	-	0.395		
HCM Control Delay (s)		-	-	24.9	9.5	8.2
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh))	-	-	1.8	0.3	0.4

Intersection							
Int Delay, s/veh	7.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	EDL		↑	WBR	SBL Š	SDK *	
Traffic Vol, veh/h	29	↑↑ 124	TT 78	104	295	99	
Future Vol, veh/h	29	124	78	104	295	99	
Conflicting Peds, #/hr	0	0	0	0	293	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None		None	
Storage Length	300	-	_	205	155	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	σ, π -	0	0	<u>-</u>	0	<u>-</u>	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	34	146	92	122	347	116	
	0-1	170	02	122	0-11	. 10	
	Major1		//ajor2		Minor2		
Conflicting Flow All	214	0	-	0	233	46	
Stage 1	-	-	-	-	92	-	
Stage 2	-	-	-	-	141	-	
Critical Hdwy	4.14	_	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	1353	-	-	-	735	1014	
Stage 1	-	-	-	-	921	-	
Stage 2	-	-	-	-	871	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1353	-	-	-	717	1014	
Mov Cap-2 Maneuver	-	-	-	-	717	-	
Stage 1	-	-	-	-	898	-	
Stage 2	-	-	-	-	871	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.5		0		13.2		
HCM LOS					В		
Minor Long/Marian M	-4	EDI	EDT	WDT	WED	CDL 4	ODL 0
Minor Lane/Major Mvn	nt	EBL	EBT	WBT		SBLn1	
Capacity (veh/h)		1353	-	-	-		1014
HCM Lane V/C Ratio		0.025	-	-		0.484	
HCM Control Delay (s)		7.7	-	-	-	14.6	9
HCM Lane LOS	,	A	-	-	-	В	A
HCM 95th %tile Q(veh)	0.1	-	-	-	2.7	0.4

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	7		
Traffic Vol, veh/h	0	4	298	3	0	504
Future Vol, veh/h	0	4	298	3	0	504
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	_	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	5	351	4	0	593
IVIVIII(I IOW	U	3	001	7	U	555
Major/Minor M	linor1	N	//ajor1	N	/lajor2	
Conflicting Flow All	-	176	0	0	-	-
Stage 1	_	-	-	-	-	-
Stage 2	_	_	-	_	_	-
Critical Hdwy	_	6.94	_	_	_	_
Critical Hdwy Stg 1	_	-	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.32	_	_	<u>-</u>	_
Pot Cap-1 Maneuver	0	837	_	_	0	_
•	0	-			0	_
Stage 1			-	-		
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	837	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.3		0		0	
HCM LOS	9.5 A		U		U	
HOW LOS	А					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)				837	_	
HCM Lane V/C Ratio		_	_	0.006	-	
HCM Control Delay (s)		_	_	9.3	_	
HCM Lane LOS		_	_	3.5 A	_	
HCM 95th %tile Q(veh)		-	-	0		
HOW SOUL WILLE CALACT		-	-	U	-	

Intersection						
Int Delay, s/veh	0.7					
			14/5-	14/5-	0	055
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7		₽		Y	
Traffic Vol, veh/h	8	125	365	0	0	29
Future Vol, veh/h	8	125	365	0	0	29
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	147	429	0	0	34
Major/Minor	Major1		/loior?		Minor	
	Major1		Major2		Minor2	400
Conflicting Flow All	429	0	-	0	594	429
Stage 1	-	-	-	-	429	-
Stage 2	-	-	-	-	165	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1130	-	-	-	468	626
Stage 1	-	-	-	-	657	-
Stage 2	-	-	-	-	864	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1130	-	-	-	464	626
Mov Cap-2 Maneuver	-	-	-	-	464	-
Stage 1	-	-	-	-	652	-
Stage 2	-	-	-	-	864	-
Annach	ED		WD		CD	
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		11.1	
HCM LOS					В	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1130			-	626
HCM Lane V/C Ratio		0.008	_	-		0.055
HCM Control Delay (s)		8.2	-	-	-	
HCM Lane LOS						
	١	A	-	-	-	В
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Intersection						
Int Delay, s/veh	4.1					
		WIDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>ች</u>	100	^	7	104	^
Traffic Vol, veh/h	66	168	478	92	104	318
Future Vol, veh/h	66	168	478	92	104	318
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	300	-	-	155	300	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	78	198	562	108	122	374
Major/Minor	Minor1		laier1	,	Major?	
	Minor1		//ajor1		Major2	
Conflicting Flow All	993	281	0	0	670	0
Stage 1	562	-	-	-	-	-
Stage 2	431	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	242	716	-	-	916	-
Stage 1	534	-	-	-	-	-
Stage 2	623	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	210	716	-	-	916	-
Mov Cap-2 Maneuver	210	-	_	_	-	_
Stage 1	534	-	-	-	-	_
Stage 2	540	<u>-</u>	_	_	_	_
Olugo Z	0-10					
Approach	WB		NB		SB	
HCM Control Delay, s	17.5		0		2.3	
HCM LOS	С					
Minor Long/Major M.	. +	NDT	NDDV	VDL ~ 4V	VDL O	CDI
Minor Lane/Major Mvm	IL	NBT		VBLn1V		SBL
Capacity (veh/h)		-	-		716	916
HCM Lane V/C Ratio		-	-		0.276	
HCM Control Delay (s)		-	-	•	11.9	9.5
HCM Lane LOS		-	-	D	В	Α
HCM 95th %tile Q(veh		-	-	1.6	1.1	0.5

Intersection							
Int Delay, s/veh	5.8						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	EDL Š			WDR	ODL 1	SDK 7	
Traffic Vol. veh/h	1 94	↑↑ 102	↑↑ 168	338	า 198	66	
Future Vol, veh/h	94	102	168	338	198	66	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	300	-	_	205	155	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	111	120	198	398	233	78	
Major/Minor N	Major1	N	/lajor2	N	/linor2		
Conflicting Flow All	596	0	//aj012 -	0	480	99	
Stage 1	590	U	-	-	198	99	
Stage 2	_		-	-	282	_	
Critical Hdwy	4.14	_	_	_	6.84	6.94	
Critical Hdwy Stg 1	T. IT	_	_	_	5.84	0.54	
Critical Hdwy Stg 2	_	_	_	_	5.84	_	
Follow-up Hdwy	2.22	_	_	_	3.52	3.32	
Pot Cap-1 Maneuver	976	_	_	_	515	937	
Stage 1	-	-	_	-	816	-	
Stage 2	-	-	_	-	741	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	976	-	-	-	456	937	
Mov Cap-2 Maneuver	-	-	-	-	456	-	
Stage 1	-	-	-	-	723	-	
Stage 2	-	-	-	-	741	-	
Approach	EB		WB		SB		
HCM Control Delay, s	4.4		0		18		
HCM LOS	4.4		U		C		
I IOIVI LOS					U		
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR:	SBLn1	SBLn2
Capacity (veh/h)		976	-	-	-	456	937
HCM Lane V/C Ratio		0.113	-	-	-	0.511	
HCM Control Delay (s)		9.2	-	-	-	20.9	9.2
HCM Lane LOS		Α	-	-	-	С	Α
HCM 95th %tile Q(veh)		0.4	-	-	-	2.8	0.3

Intersection						
Int Dalay, alicela	^					
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	7		^
Traffic Vol, veh/h	0	3	637	8	0	422
Future Vol, veh/h	0	3	637	8	0	422
Conflicting Peds, #/hr	0	0	0	0	0	0
_	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	-	0		-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	4	749	9	0	496
WWWIICTIOW	U	-	140	J	U	450
	linor1		/lajor1	N.	/lajor2	
Conflicting Flow All	-	375	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	_	-	-	_	-
Critical Hdwy Stg 2	_	-	_	-	_	-
Follow-up Hdwy	_	3.32	-	_	_	_
	0	623	_	_	0	_
Pot Cap-1 Maneuver	0	623	-	-	0	-
Pot Cap-1 Maneuver Stage 1	0	-	- -		0	-
Pot Cap-1 Maneuver Stage 1 Stage 2			-	-		-
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, %	0	-	-		0	- - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver	0 0	623	- - -	- - -	0 0	- - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	0	623	-	-	0 0 - -	- - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	0 0 - -	623	- - - -	- - - -	0 0 - -	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	0 0	623	- - -	- - -	0 0 - -	- - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	0 0 - -	623	- - - -	- - - -	0 0 - -	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	0 0 - -	623	- - - -	- - - -	0 0 - -	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach	0 0 - - - - WB	623	- - - -	- - - -	0 0	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	0 0 - - - - WB	623	- - - - - NB	- - - -	0 0 - - - - SB	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach	0 0 - - - - WB	623	- - - - - NB	- - - -	0 0 - - - - SB	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	0 0 - - - - WB 10.8 B	623	- - - - - NB	-	0 0 - - - - SB 0	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	0 0 - - - - WB 10.8 B	623	- - - - - NB 0	- - - - - -	0 0 - - - - SB	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	0 0 - - - - WB 10.8 B	623	- - - - - - NB 0	- - - - - - - - - - - - - - - - - - -	0 0 - - - - SB 0	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0 0 - - - - WB 10.8 B	623 - - - NBT	- - - - - - NB 0	- - - - - - - - - - - - - - - - - - -	0 0 - - - - SB 0	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	0 0 - - - - WB 10.8 B	623 - - - - NBT	- - - - - - NB 0	VBLn1 623 0.006 10.8	0 0 - - - - SB 0	- - - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0 0 - - - - WB 10.8 B	623 - - - - NBT	- - - - - NB 0	- - - - - - - - - - - - - - - - - - -	0 0 - - - - SB 0	- - - -

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7		₽		14	
Traffic Vol, veh/h	27	405	245	1	0	20
Future Vol, veh/h	27	405	245	1	0	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage		0	0	_	0	-
Grade, %	-, "	0	0	_	0	_
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	32	476	288	1	0	24
IVIVIIIL I IOW	JZ	710	200		U	24
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	289	0		0	829	289
Stage 1	-	-	-	-	289	-
Stage 2	_	_	_	_	540	_
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1		<u>-</u>	_	_	5.42	-
Critical Hdwy Stg 2	_		_	_	5.42	_
Follow-up Hdwy	2.218		_		3.518	
Pot Cap-1 Maneuver	1273	-	-	_	340	750
•	1213	_	_	_	760	750
Stage 1	-	-				
Stage 2	-	-	-	-	584	-
Platoon blocked, %	4070	-	-	-	000	7-^
Mov Cap-1 Maneuver	1273	-	-	-	332	750
Mov Cap-2 Maneuver	-	-	-	-	332	-
Stage 1	-	-	-	-	741	-
Stage 2	-	-	-	-	584	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		10	
	0.5		U			
HCM LOS					В	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1273		_		750
HCM Lane V/C Ratio		0.025	-	_	_	0.031
HCM Control Delay (s)		7.9	_	_	_	10
HCM Lane LOS		Α	_	_	_	В
HCM 95th %tile Q(veh)	0.1				0.1
How som wille Q(ven)	U. I			_	U. I

Intersection						
Int Delay, s/veh	5.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	^	7	<u> </u>	^
Traffic Vol, veh/h	128	78	230	31	126	380
Future Vol, veh/h	128	78	230	31	126	380
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	300	-	_	155	300	-
Veh in Median Storage		_	0	-	-	0
Grade, %	0	<u>-</u>	0	_	_	0
Peak Hour Factor	85	85	85	85	85	85
	2	2	2	2	2	2
Heavy Vehicles, %	151	92	271	36	148	447
Mvmt Flow	151	92	2/1	36	148	447
Major/Minor I	Minor1	N	Major1	1	Major2	
Conflicting Flow All	791	136	0	0	307	0
Stage 1	271	-	-	_	-	_
Stage 2	520	_	_	_	_	_
Critical Hdwy	6.84	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	-	_	_	-	_
Critical Hdwy Stg 2	5.84	_	_	_	_	_
Follow-up Hdwy	3.52	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	327	888	_	-	1250	_
•	750	- 000	_	_	1230	_
Stage 1			-	-	-	-
Stage 2	561	-	-	-	-	-
Platoon blocked, %	000	000	-	-	4050	-
Mov Cap-1 Maneuver	288	888	-	-	1250	-
Mov Cap-2 Maneuver	288	-	-	-	-	-
Stage 1	750	-	-	-	-	-
Stage 2	495	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	22.5		0		2.1	
HCM LOS	C		U		۷.۱	
I IOIVI LOO	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		-	-	288	888	1250
HCM Lane V/C Ratio		-	-	0.523	0.103	0.119
HCM Control Delay (s)		-	-	30.4	9.5	8.3
HCM Lane LOS		-	-	D	Α	Α
HCM 95th %tile Q(veh))	-	-	2.8	0.3	0.4

Intersection							
Int Delay, s/veh	8.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ኘ	^	^	7	ሻ	7	
Traffic Vol, veh/h	33	124	78	117	332	128	
Future Vol, veh/h	33	124	78	117	332	128	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	300	-	-	205	155	0	
Veh in Median Storage	, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	39	146	92	138	391	151	
Major/Minor N	Major1	N	Major2	- 1	Minor2		
Conflicting Flow All	230	0	-	0	243	46	
Stage 1	-	-	-	-	92	-	
Stage 2	-	-	-	-	151	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	1335	_	-	-	724	1014	
Stage 1	-	-	-	-	921	-	
Stage 2	-	-	-	-	861	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1335	-	-	-	703	1014	
Mov Cap-2 Maneuver	-	-	-	-	703	-	
Stage 1	-	-	-	-	894	-	
Stage 2	-	-	-	-	861	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.6		0		14.3		
HCM LOS					В		
Minor Lane/Major Mvm	.+	EBL	EBT	WBT	WPD	SBLn1	CDI 22
	ı						
Capacity (veh/h) HCM Lane V/C Ratio		1335	-	-	-	0.556	1014
HCM Control Delay (s)		0.029 7.8	-	-	_	16.3	9.2
HCM Lane LOS		7.0 A	-	-	-	16.3 C	9.2 A
HCM 95th %tile Q(veh)		0.1	-	_	-	3.4	0.5
How both wille Q(Ven)		0.1	_	-		5.4	0.5

Short-Term Total Traffic Synchro 11 Report AM Peak Hour Page 2

Intersection						
Int Delay, s/veh	0.1					
<u> </u>		WED	NOT	NDD	ODI	ODT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	^	7	^	7	_	^
Traffic Vol, veh/h	0	10	298	10	0	506
Future Vol, veh/h	0	10	298	10	0	506
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0		0		-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	12	351	12	0	595
Major/Minor M	inor1	N	Major1	ı	/lajor2	
Conflicting Flow All	-	176	0	0	- -	_
Stage 1	- -	-	-	-	-	-
Stage 2			-	-	-	
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	837	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	837	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
A L	MD		ND		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s	9.4		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NRRV	VBLn1	SBT	
			-			
Capacity (veh/h) HCM Lane V/C Ratio		-		0.014	-	
				9.4	-	
HCM Long LOS		-			-	
HCM Lane LOS		-	-	A	-	
HCM 95th %tile Q(veh)		-	-	0	-	

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	↑	1		₩	
Traffic Vol, veh/h	25	125	365	1	0	95
Future Vol, veh/h	25	125	365	1	0	95
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	150	-	_	-	0	-
Veh in Median Storage,		0	0	_	0	_
Grade, %	, <i>''</i> -	0	0	_	0	_
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	29	147	429	1	0	112
MALL LIOW	29	147	423		U	112
Major/Minor N	/lajor1	N	Major2	1	Minor2	
Conflicting Flow All	430	0	-	0	635	430
Stage 1	-	-	-	-	430	-
Stage 2	-	-	-	-	205	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	-	5.42	-
	2.218	-	-	_	3.518	3.318
Pot Cap-1 Maneuver	1129	_	-	-	443	625
Stage 1	-	_	_	_	656	-
Stage 2	_	_	_	_	829	_
Platoon blocked, %		_	_	_	020	
Mov Cap-1 Maneuver	1129	_	_	_	431	625
Mov Cap-2 Maneuver	-	_	_	_	431	-
Stage 1	_	_	_	_	639	_
Stage 2	<u>-</u>	_	_	_	829	_
Olage 2	_		_	_	023	_
Approach	EB		WB		SB	
HCM Control Delay, s	1.4		0		12	
HCM LOS					В	
Minor Long/Major M.	1	EDI	EDT	WDT	WDD	CDL 4
Minor Lane/Major Mymi	l e	EBL	EBT	WBT		SBLn1
Capacity (veh/h)		1129	-	-	-	625
HCM Lane V/C Ratio		0.026	-	-		0.179
			_	_	_	12
HCM Control Delay (s)		8.3				
		0.3 A 0.1	-	-	-	B 0.6

Intersection							
Int Delay, s/veh	5.3						
		MDD	NDT	NDD	CDI	CDT	į
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	<u>ነ</u>	400	^	101	100	^	
Traffic Vol, veh/h	86	168	504	101	109	318	
Future Vol, veh/h	86	168	504	101	109	318	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-		-	None	
Storage Length	300	-	-	155	300	-	
Veh in Median Storage		-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	101	198	593	119	128	374	
Major/Minor N	/linor1	I	Major1	N	Major2		
Conflicting Flow All	1036	297	0	0	712	0	
Stage 1	593	-	-	-	_	-	
Stage 2	443	-	-	-	_	_	
Critical Hdwy	6.84	6.94	_	_	4.14	-	
Critical Hdwy Stg 1	5.84	-	-	-	-	-	
Critical Hdwy Stg 2	5.84	_	_	_	-	-	
Follow-up Hdwy	3.52	3.32	-	-	2.22	-	
Pot Cap-1 Maneuver	227	699	_	_	884	-	
Stage 1	515	-	-	-	_	_	
Stage 2	614	-	_	-	_	-	
Platoon blocked, %			-	-		_	
Mov Cap-1 Maneuver	194	699	-	_	884	-	
Mov Cap-2 Maneuver	194	-	_	-	-	_	
Stage 1	515	_	_	_	_	_	
Stage 2	525	_	_	_	_	_	
Olago Z	020						
Approach	WB		NB		SB		
HCM Control Delay, s	22.4		0		2.5		
HCM LOS	С						
Minor Lane/Major Mvm	t	NBT	NRRV	WBLn1W	VRI n2	SBL	ĺ
Capacity (veh/h)		-	-		699	884	
HCM Lane V/C Ratio		<u>-</u>		0.522			
HCM Control Delay (s)		_	_	42.2	12.2	9.8	
HCM Lane LOS		<u>-</u>	_	42.2 E	12.2	3.0 A	
LIGHT LUNG LOO							
HCM 95th %tile Q(veh)		_	_	2.7	1.2	0.5	

7.2						
FRI	FRT	WRT	WRR	SRI	SBR	
0	0	0	0	0	0	
Free	Free	Free	Free			
-	None			-	None	
300	-	-	205	155	0	
# -	0	0	-	0	-	
-	0	0	-	0	-	
85	85	85	85	85	85	
2	2	2	2	2	2	
127	120	198	446	262	101	
laior1	N	Maior2	N	Minor2		
		- viajoiz			99	
-	-	_	-		-	
_	-	_	-		-	
4.14	-	-	-		6.94	
-	-	_	-	5.84	-	
-	-	-	-	5.84	-	
2.22	-	-	-	3.52	3.32	
937	-	-	-	491	937	
-	-	-	-	816	-	
-	-	-	-	714	-	
	-	-	-			
937	-	-	-	424	937	
-	-	-	-	424	-	
-	-	-	-	705	-	
-	-	-	-	714	-	
FR		WR		SB		
4.3		U				
				U		
		EBT	WBT	WBR :		
		-	-			937
		-	-			
		-	-	-		9.3
		-	-	-		A
	0.5			_	/ 1	0.4
1	EBL 108 108 0 Free - 300 # - 85 2 127 lajor1 644 4.14 2.22 937 937 BB 4.9	EBL EBT 108 102 108 102 0 0 Free Free - None 300 - # - 0 85 85 2 2 127 120 lajor1 N 644 0 4.14 2.22 - 937 937 937 EB 4.9	EBL EBT WBT 108 102 168 108 102 168 0 0 0 Free Free Free - None 300 # - 0 0 85 85 85 2 2 2 2 127 120 198	EBL EBT WBT WBR 108 102 168 379 108 102 168 379 0 0 0 0 0 0 0 0 Free Free Free Free None None 300 - 205 - 0 0 - - 0 0 - - 0 0 - - 0 0 - 85 85 85 85 2 2 2 2 127 120 198 446	EBL EBT WBT WBR SBL *** *** *** *** 108 102 168 379 223 0 0 0 0 0 Free Free Free Free Stop None - None - 300 - - 205 155 # - 0 0 - 0 - 0 0 - 0 85 85 85 85 85 2 2 2 2 2 2 127 120 198 446 262 128 4 0 - 0 512 424 0 - 0 512 512 444 0 - 0 512 514 4.14 - - - 1.84 5.84 2.22 - - <td> BBL BBT WBT WBR SBL SBR </td>	BBL BBT WBT WBR SBL SBR

Intersection						
Int Delay, s/veh	0.1					
		WED	NET	NDD	ODI	ODT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	7		^
Traffic Vol, veh/h	0	7	637	34	0	427
Future Vol, veh/h	0	7	637	34	0	427
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	8	749	40	0	502
	inor1		//ajor1		/lajor2	
Conflicting Flow All	-	375	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	623	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	_		_
Mov Cap-1 Maneuver	_	623	_	_	_	_
Mov Cap-2 Maneuver	_	-	_	_	_	_
Stage 1	_	_	_	_	_	_
Olage I	_	_	_	_	_	_
Stage 2			-	-	-	-
Stage 2	-					
Stage 2	-					
Stage 2 Approach	WB		NB		SB	
Approach	WB		NB 0		SB 0	
Approach HCM Control Delay, s	WB 10.9					
Approach	WB					
Approach HCM Control Delay, s HCM LOS	WB 10.9		0	MRI n1	0	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	WB 10.9	NBT	0	VBLn1	0 SBT	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	WB 10.9	NBT -	0 NBRV	623	0 SBT	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	WB 10.9	NBT -	0 NBRV -	623 0.013	0 SBT -	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	WB 10.9	NBT - -	NBRV - -	623 0.013 10.9	0 SBT - -	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	WB 10.9	NBT -	0 NBRV -	623 0.013	0 SBT -	

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ች	†	1>		¥	
Traffic Vol, veh/h	82	405	245	2	0	64
Future Vol, veh/h	82	405	245	2	0	64
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	_	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %	- -	0	0	_	0	_
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	96	476	288	2	0	75
IVIVIIIL FIOW	90	470	200		U	75
Major/Minor	Major1	N	Major2	N	Minor2	
Conflicting Flow All	290	0	-	0	957	289
Stage 1	-	-	-	_	289	-
Stage 2	-	-	-	-	668	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	_	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	-	_	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	1272	_	_	_	286	750
Stage 1	-	_	_	_	760	-
Stage 2	_	_	_	_	510	_
Platoon blocked, %		_	_	<u>-</u>	010	
Mov Cap-1 Maneuver	1272	_	_	_	265	750
Mov Cap-1 Maneuver	1212	<u>-</u>	_	<u>-</u>	265	- 100
Stage 1	_		_	_	703	_
•	_	_		_	510	_
Stage 2	_	-	-	-	510	_
Approach	EB		WB		SB	
HCM Control Delay, s	1.4		0		10.3	
HCM LOS					В	
Min and any (Marin Ma	-1	EDI	EDT	MOT	MPD	ODL 4
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR:	
Capacity (veh/h)		1272	-	-	-	750
HCM Lane V/C Ratio		0.076	-	-	-	0.1
HCM Control Delay (s)		8.1	-	-	-	10.3
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh	1)	0.2	-	-	-	0.3

	•	→	•	•	←	•	1	†	/	>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	7	^	7	7	^	7
Traffic Volume (vph)	71	838	42	167	918	70	115	236	111	120	514	131
Future Volume (vph)	71	838	42	167	918	70	115	236	111	120	514	131
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	12.0	66.0	66.0	12.0	66.0	66.0	12.0	30.0	30.0	12.0	30.0	30.0
Total Split (%)	10.0%	55.0%	55.0%	10.0%	55.0%	55.0%	10.0%	25.0%	25.0%	10.0%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	67.7	61.0	61.0	69.0	63.4	63.4	32.0	25.0	25.0	32.0	25.0	25.0
Actuated g/C Ratio	0.56	0.51	0.51	0.58	0.53	0.53	0.27	0.21	0.21	0.27	0.21	0.21
v/c Ratio	0.25	0.49	0.05	0.53	0.52	0.08	0.64	0.34	0.28	0.40	0.73	0.32
Control Delay	11.8	20.5	0.1	16.4	9.5	0.2	48.9	42.0	8.8	36.1	51.2	9.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.8	20.5	0.1	16.4	9.5	0.2	48.9	42.0	8.8	36.1	51.2	9.6
LOS	В	С	Α	В	Α	Α	D	D	Α	D	D	Α
Approach Delay		18.9			10.0			35.7			41.7	
Approach LOS		В			Α			D			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 70

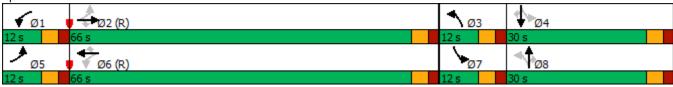
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 23.4 Intersection LOS: C
Intersection Capacity Utilization 69.7% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd



2042 Background Traffic Synchro 11 Report
AM Peak Hour Page 1

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	7	44	7	7	†	7	1/4	†	7
Traffic Volume (vph)	158	900	10	47	809	175	39	8	55	450	9	307
Future Volume (vph)	158	900	10	47	809	175	39	8	55	450	9	307
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	12.0	51.0	51.0	12.0	51.0	51.0	12.0	25.0	25.0	32.0	45.0	45.0
Total Split (%)	10.0%	42.5%	42.5%	10.0%	42.5%	42.5%	10.0%	20.8%	20.8%	26.7%	37.5%	37.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	74.8	65.7	65.7	66.7	59.8	59.8	14.6	10.0	10.0	22.5	25.0	25.0
Actuated g/C Ratio	0.62	0.55	0.55	0.56	0.50	0.50	0.12	0.08	0.08	0.19	0.21	0.21
v/c Ratio	0.43	0.49	0.01	0.15	0.48	0.21	0.21	0.05	0.21	0.74	0.02	0.60
Control Delay	20.0	15.2	0.0	12.0	22.8	3.7	32.9	51.6	1.8	53.2	35.8	13.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.0	15.2	0.0	12.0	22.8	3.7	32.9	51.6	1.8	53.2	35.8	13.3
LOS	С	В	Α	В	С	Α	С	D	Α	D	D	В
Approach Delay		15.8			19.1			17.4			37.0	
Approach LOS		В			В			В			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 22.5 Intersection Capacity Utilization 63.1% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd



2042 Background Traffic Synchro 11 Report
AM Peak Hour Page 2

Intersection						
Int Delay, s/veh	0					
		WED	NOT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	7		^
Traffic Vol, veh/h	0	2	376	2	0	765
Future Vol, veh/h	0	2	376	2	0	765
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	396	2	0	805
Major/Minor	linor1		Jaior1	N.	laior?	
-			Major1		/lajor2	
Conflicting Flow All	-	198	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	810	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	810	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	_	_	_	_	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	9.5		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NRRV	VBLn1	SBT	
Capacity (veh/h)		-	-		-	
HCM Lane V/C Ratio		-		0.003	-	
HCM Control Delay (s)		-			_	
HCM Lane LOS		-				
		-	-	A	-	
HCM 95th %tile Q(veh)		-	-	0	-	

2042 Background Traffic Synchro 11 Report AM Peak Hour Page 3

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				WDN	SBL ₩	SDN
Lane Configurations	ጟ	226	744	2		22
Traffic Vol, veh/h	5	336	744	3	6	22
Future Vol, veh/h	5	336	744	3	6	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	354	783	3	6	23
Maria - /MAi	1-1-4		4-1-0		1:	
	Major1		Major2		Minor2	
Conflicting Flow All	786	0	-	0	1149	785
Stage 1	-	-	-	-	785	-
Stage 2	-	-	-	-	364	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	833	-	-	-	219	393
Stage 1	-	-	-	-	449	-
Stage 2	-	-	-	-	703	-
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	833	-	-	-	218	393
Mov Cap-2 Maneuver	-	_	_	<u>-</u>	218	-
Stage 1	_	_	_	_	446	_
Stage 2		_		_	703	_
Glaye Z	-	<u>-</u>	_	<u>-</u>	103	<u>-</u>
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		16.8	
HCM LOS			•		С	
					J	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		833	_	-	-	335
HCM Lane V/C Ratio		0.006	-	-	-	0.088
HCM Control Delay (s)		9.3	-	-	-	
HCM Lane LOS		Α	_	-	_	С
HCM 95th %tile Q(veh))	0	_	-	_	0.3

2042 Background Traffic Synchro 11 Report
AM Peak Hour Page 4

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	*	44	7	*	44	7
Traffic Volume (vph)	142	928	96	185	706	154	171	719	188	132	342	199
Future Volume (vph)	142	928	96	185	706	154	171	719	188	132	342	199
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	20.0	50.0	50.0	19.0	49.0	49.0	15.0	36.0	36.0	15.0	36.0	36.0
Total Split (%)	16.7%	41.7%	41.7%	15.8%	40.8%	40.8%	12.5%	30.0%	30.0%	12.5%	30.0%	30.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	57.7	47.0	47.0	60.3	48.3	48.3	41.2	31.4	31.4	40.8	31.2	31.2
Actuated g/C Ratio	0.48	0.39	0.39	0.50	0.40	0.40	0.34	0.26	0.26	0.34	0.26	0.26
v/c Ratio	0.43	0.70	0.15	0.70	0.52	0.22	0.50	0.82	0.38	0.68	0.39	0.37
Control Delay	18.5	34.4	4.5	49.4	22.3	5.3	31.4	50.1	13.4	42.7	38.2	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.5	34.4	4.5	49.4	22.3	5.3	31.4	50.1	13.4	42.7	38.2	6.7
LOS	В	С	Α	D	С	Α	С	D	В	D	D	Α
Approach Delay		30.0			24.6			40.8			29.8	
Approach LOS		С			С			D			С	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 31.5 Intersection LOS: C
Intersection Capacity Utilization 79.8% ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd



2042 Background Traffic Synchro 11 Report
PM Peak Hour Page 1

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, T	† †	7	, j	† †	7	*		7	14.44	+	7
Traffic Volume (vph)	285	946	22	130	841	540	27	7	37	324	23	176
Future Volume (vph)	285	946	22	130	841	540	27	7	37	324	23	176
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	20.0	68.0	68.0	12.0	60.0	60.0	10.0	15.0	15.0	25.0	30.0	30.0
Total Split (%)	16.7%	56.7%	56.7%	10.0%	50.0%	50.0%	8.3%	12.5%	12.5%	20.8%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	80.2	69.1	69.1	70.0	63.1	63.1	11.0	10.0	10.0	20.0	23.0	23.0
Actuated g/C Ratio	0.67	0.58	0.58	0.58	0.53	0.53	0.09	0.08	0.08	0.17	0.19	0.19
v/c Ratio	0.71	0.49	0.02	0.41	0.48	0.52	0.20	0.05	0.12	0.60	0.07	0.41
Control Delay	36.2	10.4	0.0	12.9	20.7	3.4	37.8	51.4	0.8	51.2	39.1	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.2	10.4	0.0	12.9	20.7	3.4	37.8	51.4	0.8	51.2	39.1	8.5
LOS	D	В	Α	В	С	Α	D	D	Α	D	D	Α
Approach Delay		16.1			13.9			19.6			36.3	
Approach LOS		В			В			В			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 80

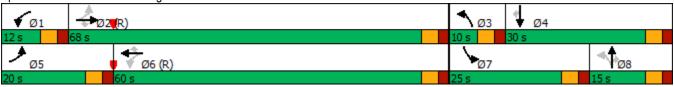
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 18.3 Intersection LOS: B
Intersection Capacity Utilization 70.1% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd



2042 Background Traffic Synchro 11 Report
PM Peak Hour Page 2

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	7		† †
Traffic Vol, veh/h	0	2	1007	8	0	673
Future Vol, veh/h	0	2	1007	8	0	673
Conflicting Peds, #/hr	0	0	0	0	0	0.0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	_	0	_	200	_	-
Veh in Median Storage	, # 0	-	0	-	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	1060	8	0	708
William Com	•	_	1000		•	100
	Minor1		Major1		/lajor2	
Conflicting Flow All	-	530	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	493	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	493	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
, and the second						
Annragah	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	12.3		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		_	_	493	_	
HCM Lane V/C Ratio		_	_	0.004	_	
HCM Control Delay (s)		_	_	12.3	_	
HCM Lane LOS		_	_	В	_	
HCM 95th %tile Q(veh))	-	-	0	-	
J. 222. 70 2(1011)						

2042 Background Traffic Synchro 11 Report PM Peak Hour Page 3

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				WDK		אמט
Lane Configurations	<u>ነ</u>	↑	\$	•	Y	40
Traffic Vol, veh/h	25	807	505	3	1	18
Future Vol, veh/h	25	807	505	3	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage	е, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	26	849	532	3	1	19
IVIVIII(I IOW	20	043	002	3		13
Major/Minor I	Major1	N	Major2	1	Minor2	
Conflicting Flow All	535	0	-	0	1435	534
Stage 1	_	-	-	-	534	-
Stage 2	_	_	-	_	901	_
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1		_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_			
		-				
Pot Cap-1 Maneuver	1033	-	-	-	147	546
Stage 1	-	-	-	-	588	-
Stage 2	-	-	-	-	396	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1033	-	-	-	143	546
Mov Cap-2 Maneuver	-	-	-	-	143	-
Stage 1	_	-	-	-	573	-
Stage 2	_	_	_	_	396	_
otago =						
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		12.9	
HCM LOS					В	
				14/5=	14/5-	0DL /
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	
Capacity (veh/h)		1033	-	-		475
HCM Lane V/C Ratio		0.025	-	-		0.042
HCM Control Delay (s))	8.6	-	-	-	12.9
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1
	,					

2042 Background Traffic Synchro 11 Report PM Peak Hour Page 4

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† †	7	7	† †	7	7	44	7	7	^	7
Traffic Volume (vph)	71	843	42	174	932	70	115	238	112	120	514	131
Future Volume (vph)	71	843	42	174	932	70	115	238	112	120	514	131
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	12.0	66.0	66.0	12.0	66.0	66.0	12.0	30.0	30.0	12.0	30.0	30.0
Total Split (%)	10.0%	55.0%	55.0%	10.0%	55.0%	55.0%	10.0%	25.0%	25.0%	10.0%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	67.7	61.0	61.0	69.0	63.4	63.4	32.0	25.0	25.0	32.0	25.0	25.0
Actuated g/C Ratio	0.56	0.51	0.51	0.58	0.53	0.53	0.27	0.21	0.21	0.27	0.21	0.21
v/c Ratio	0.25	0.49	0.05	0.56	0.52	0.08	0.64	0.34	0.28	0.41	0.73	0.32
Control Delay	11.9	20.5	0.1	17.8	9.9	0.2	48.9	42.0	8.8	36.2	51.2	9.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.9	20.5	0.1	17.8	9.9	0.2	48.9	42.0	8.8	36.2	51.2	9.6
LOS	В	С	Α	В	Α	Α	D	D	Α	D	D	Α
Approach Delay		19.0			10.5			35.7			41.7	
Approach LOS		В			В			D			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 70

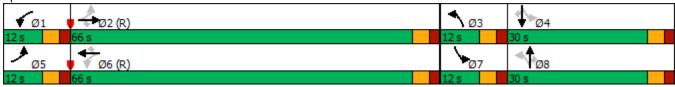
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 23.5 Intersection LOS: C
Intersection Capacity Utilization 70.2% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd



2042 Total Traffic Synchro 11 Report
AM Peak Hour Page 1

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	7	^	7	7	†	7	14.54	†	7
Traffic Volume (vph)	164	900	10	47	809	184	39	8	55	480	9	328
Future Volume (vph)	164	900	10	47	809	184	39	8	55	480	9	328
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	12.0	51.0	51.0	12.0	51.0	51.0	12.0	25.0	25.0	32.0	45.0	45.0
Total Split (%)	10.0%	42.5%	42.5%	10.0%	42.5%	42.5%	10.0%	20.8%	20.8%	26.7%	37.5%	37.5%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag									
Lead-Lag Optimize?	Yes	Yes	Yes									
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	74.5	65.1	65.1	65.7	58.8	58.8	14.6	10.0	10.0	23.1	25.5	25.5
Actuated g/C Ratio	0.62	0.54	0.54	0.55	0.49	0.49	0.12	0.08	0.08	0.19	0.21	0.21
v/c Ratio	0.45	0.49	0.01	0.15	0.49	0.22	0.21	0.05	0.21	0.77	0.02	0.63
Control Delay	21.9	15.6	0.0	12.3	23.6	3.7	32.6	51.6	1.8	54.0	35.2	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.9	15.6	0.0	12.3	23.6	3.7	32.6	51.6	1.8	54.0	35.2	15.6
LOS	С	В	Α	В	С	Α	С	D	Α	D	D	В
Approach Delay		16.4			19.6			17.3			38.4	
Approach LOS		В			В			В			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 23.4 Intersection LOS: C
Intersection Capacity Utilization 64.3% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd



2042 Total Traffic Synchro 11 Report
AM Peak Hour Page 2

Intersection						
Int Delay, s/veh	0					
		WED	NET	NDD	051	057
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^	- 7		^
Traffic Vol, veh/h	0	3	376	4	0	765
Future Vol, veh/h	0	3	376	4	0	765
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	200	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	396	4	0	805
Major/Minor M	1inor1	N	//ajor1	١	/lajor2	
Conflicting Flow All	-	198	0	0	-	_
Stage 1	_	-	-	-	_	_
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	6.94	_	_	_	_
Critical Hdwy Stg 1	_	-	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.32	_	_	_	_
Pot Cap-1 Maneuver	0	810	_	_	0	_
Stage 1	0	-	_	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	U		_	_	U	_
Mov Cap-1 Maneuver	_	810	_	_	_	_
Mov Can-2 Maneuver	_		_	_	_	_
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	- -
		-	- - -	- - -		- - -
Stage 1 Stage 2	-	-	- - -	-	-	- - -
Stage 1		-	- - - NB	-	-	- - -
Stage 1 Stage 2	-	-	- - - NB 0	-	-	-
Stage 1 Stage 2 Approach	- - WB	-		-	- - SB	-
Stage 1 Stage 2 Approach HCM Control Delay, s	- - WB 9.5	-		-	- - SB	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	- - - WB 9.5 A	-	0	-	- - SB 0	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	- - - WB 9.5 A	- - - NBT	0 NBRW	- - VBLn1	SB 0	
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	- - - WB 9.5 A	- - - NBT	0 NBRW	- - - VBLn1 810	SB 0	
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	- - - WB 9.5 A	- - - NBT	0 NBRV -	VBLn1 810 0.004	SB 0	
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	- - - WB 9.5 A	- - - NBT - -	NBRW - -	VBLn1 810 0.004 9.5	SB 0 SBT -	
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	- - WB 9.5 A	- - - NBT	0 NBRV -	VBLn1 810 0.004	SB 0	

2042 Total Traffic Synchro 11 Report AM Peak Hour Page 3

1.9					
EBL	EBT	WBT	WBR	SBL	SBR
			11		73
21	336	744	11	25	73
	0	0	0	0	0
Free	Free	Free	Free		Stop
-	None	-	None	-	None
150	-	-	-	0	-
	0	0	-	0	-
-	0	0	-	0	-
95		95	95	95	95
					2
					77
	001	. 00	14		
			_		
	0	-			789
-	-	-	-		-
-	-	-	-		-
4.12	-	-	-		6.22
-	-	-	-		-
-	-	-	-		-
2.218	-	-	-		
826	-	-	-	208	391
-	-	-	-	448	-
-	-	-	-	678	-
	-	-	-		
826	-	-	-	202	391
	-	-	-	202	-
-	-	-	-	436	-
-	-	-	-		-
		14.0		0.5	
0.6		0			
				С	
nt	EBI	EBT	WBT	WBR :	SBLn1
					316
		_	-	-	0.326
.)		-	-		21.8
1			-	-	21.0 C
	^				١.
۱)	0.1	-	_		1.4
	EBL 21 21 0 Free - 150 e, # - 95 2 22 Major1 795 - 4.12 - 2.218 826 - 826 - 826	EBL EBT 21 336 21 336 0 0 0 Free Free - None 150 - e, # - 0 95 95 2 2 2 22 354 Major1 N 795 0 4.12 2.218 - 826 826 EB 0.6	EBL EBT WBT 21 336 744 21 336 744 0 0 0 0 Free Free Free - None 150 e, # - 0 0 95 95 95 2 2 2 2 22 354 783 Major1 Major2 795 0 2.218 2.218 826 826	EBL EBT WBT WBR 21 336 744 11 21 336 744 11 0 0 0 0 0 0 Free Free Free Free - None - None 150 0 0 - 95 95 95 95 2 2 2 2 2 22 354 783 12 Major1 Major2 7 795 0 - 0 4.12 2.218 2.218 826 826 EB WB 0.6 0 mt EBL EBT WBT 826 0.027	EBL EBT WBT WBR SBL 1 1 1 1 1 1 1 25 21 336 744 11 25 0 95

	۶	→	•	•	←	*	4	†	/	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	7	^	7	7	^	7
Traffic Volume (vph)	142	949	96	191	718	154	171	728	191	132	342	199
Future Volume (vph)	142	949	96	191	718	154	171	728	191	132	342	199
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8	4		4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0	10.0	23.0	23.0
Total Split (s)	20.0	50.0	50.0	19.0	49.0	49.0	15.0	36.0	36.0	15.0	36.0	36.0
Total Split (%)	16.7%	41.7%	41.7%	15.8%	40.8%	40.8%	12.5%	30.0%	30.0%	12.5%	30.0%	30.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	Max	None	Max	Max
Act Effct Green (s)	57.4	46.7	46.7	60.6	48.3	48.3	41.2	31.4	31.4	40.8	31.2	31.2
Actuated g/C Ratio	0.48	0.39	0.39	0.50	0.40	0.40	0.34	0.26	0.26	0.34	0.26	0.26
v/c Ratio	0.43	0.73	0.15	0.73	0.53	0.23	0.50	0.83	0.39	0.68	0.39	0.37
Control Delay	18.7	35.3	4.5	53.2	22.5	5.5	31.4	50.7	13.7	43.5	38.2	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.7	35.3	4.5	53.2	22.5	5.5	31.4	50.7	13.7	43.5	38.2	6.7
LOS	В	D	Α	D	С	Α	С	D	В	D	D	Α
Approach Delay		30.8			25.5			41.2			29.9	
Approach LOS		С			С			D			С	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 32.1 Intersection LOS: C
Intersection Capacity Utilization 80.9% ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 12: Vollmer Rd & Marksheffel Rd



2042 Total Traffic Synchro 11 Report PM Peak Hour Page 1

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	7	^	7	77	†	7
Traffic Volume (vph)	310	946	22	130	841	581	27	7	37	348	24	194
Future Volume (vph)	310	946	22	130	841	581	27	7	37	348	24	194
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases	2		2	6		6	8		8			4
Detector Phase	5	2	2	1	6	6	3	8	8	7	4	4
Switch Phase												
Minimum Initial (s)	5.0	15.0	15.0	5.0	15.0	15.0	5.0	10.0	10.0	20.0	10.0	10.0
Minimum Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	10.0	15.0	15.0	25.0	20.0	20.0
Total Split (s)	20.0	68.0	68.0	12.0	60.0	60.0	10.0	15.0	15.0	25.0	30.0	30.0
Total Split (%)	16.7%	56.7%	56.7%	10.0%	50.0%	50.0%	8.3%	12.5%	12.5%	20.8%	25.0%	25.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	80.7	69.1	69.1	69.5	62.6	62.6	11.0	10.0	10.0	20.0	23.0	23.0
Actuated g/C Ratio	0.67	0.58	0.58	0.58	0.52	0.52	0.09	0.08	0.08	0.17	0.19	0.19
v/c Ratio	0.76	0.49	0.02	0.41	0.48	0.55	0.20	0.05	0.12	0.64	0.07	0.44
Control Delay	40.3	10.5	0.0	12.9	20.9	3.6	37.8	51.4	0.8	52.4	39.2	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.3	10.5	0.0	12.9	20.9	3.6	37.8	51.4	0.8	52.4	39.2	8.5
LOS	D	В	Α	В	С	Α	D	D	Α	D	D	Α
Approach Delay		17.6			13.8			19.6			36.8	_
Approach LOS		В			В			В			D	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

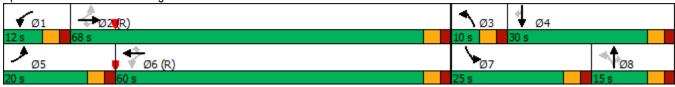
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 19.0 Intersection LOS: B
Intersection Capacity Utilization 74.0% ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 13: Sterling Ranch Rd & Marksheffel Rd



2042 Total Traffic Synchro 11 Report PM Peak Hour Page 2

Intersection Int Delay, s/veh 0 Movement WBL Lane Configurations					
Movement WBL					
	WDD	NDT	NDD	CDI	CDT
	WBR	NBT	NBR	SBL	SBT
	7	^	17	0	^
Traffic Vol, veh/h 0	3	1007	17	0	674
Future Vol, veh/h 0	3	1007	17	0	674
Conflicting Peds, #/hr 0		0	0	0	0
Sign Control Stop	Stop	Free	Free	Free	Free
RT Channelized -	None	-	None	-	None
Storage Length -	0	-	200	-	-
Veh in Median Storage, # 0	-	0	-	-	0
Grade, % 0	-	0	-	-	0
Peak Hour Factor 95	95	95	95	95	95
Heavy Vehicles, % 2	2	2	2	2	2
Mvmt Flow 0	3	1060	18	0	709
Major/Minor Minor1	N	Major1	Λ	//ajor2	
Conflicting Flow All -	530	0	0		-
Stage 1 -	-	_	-	_	_
Stage 2 -	_	_	_	_	_
Critical Hdwy -	6.94	_	_	_	_
Critical Hdwy Stg 1 -	-	_	_	_	_
Critical Hdwy Stg 2 -	_	_	_	_	_
Follow-up Hdwy -	3.32	_	_	_	_
Pot Cap-1 Maneuver 0	493	_	_	0	_
Stage 1 0	-	_	_	0	_
Stage 2 0	_	_	_	0	_
Platoon blocked, %	_	_	_	U	_
Mov Cap-1 Maneuver -	493	-	-	_	-
	495	_	_	_	_
			-		
Stage 1 -	-	-	-	-	-
Stage 2 -	-	-	-	-	-
Approach WB		NB		SB	
		0		0	
HCM Control Delay, s 12.3					
HCM Control Delay, s 12.3 HCM LOS B					
HCM Control Delay, s 12.3 HCM LOS B					
HCM LOS B					
HCM LOS B Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBT	
HCM LOS B Minor Lane/Major Mvmt Capacity (veh/h)	NBT -	-	493	SBT -	
HCM LOS B Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio		-	493 0.006		
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	-	-	493 0.006 12.3	-	
HCM LOS B Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	-	-	493 0.006	-	

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	<u> </u>	1		¥	
Traffic Vol, veh/h	90	807	505	12	6	61
Future Vol, veh/h	90	807	505	12	6	61
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	_	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %	,	0	0	_	0	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	95	849	532	13	6	64
IVIVIIIL FIOW	90	049	JJZ	13	U	04
Major/Minor	Major1	N	Major2	ı	Minor2	
Conflicting Flow All	545	0	-	0	1578	539
Stage 1	-	-	-	-	539	-
Stage 2	_	-	-	-	1039	-
Critical Hdwy	4.12	-	_	-	6.42	6.22
Critical Hdwy Stg 1	_	_	-	_	5.42	_
Critical Hdwy Stg 2	_	_	-	-	5.42	_
Follow-up Hdwy	2.218	_	_	-	3.518	3 318
Pot Cap-1 Maneuver	1024	_	_	_	120	542
Stage 1	-	_	_	_	585	-
Stage 2	_	_	_	_	341	_
Platoon blocked, %		_	_	_	J + 1	
Mov Cap-1 Maneuver	1024	_	_	_	109	542
Mov Cap-1 Maneuver		_		_	109	- 542
	-	_	-	-	531	
Stage 1	-	-	-			-
Stage 2	-	-	-	-	341	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.9		0		15.9	
HCM LOS	0.0		•		C	
					<u> </u>	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		1024	-	-	-	400
HCM Lane V/C Ratio		0.093	-	-	-	0.176
HCM Control Delay (s)		8.9	-	-	-	15.9
HCM Lane LOS		Α	-	-	-	С
HCM 95th %tile Q(veh)	0.3	_	-	-	0.6

Appendix Table 1



Appendix Table 1
Area Traffic Impact Studies
Copper Chase Sterling Ranch

tudy	PCD File No ⁽¹⁾	Consultant	Date
terling Ranch Reports			
Sterling Ranch Updated Traffic Impact Analysis	<u>SKP07007</u>	LSC Transportation Consultants, Inc	June 5, 2008
Sterling Ranch Phase 1 Traffic Impact Study	<u>P151</u>	LSC Transportation Consultants, Inc	March 16, 2015
Sterling Ranch Phases 1-3 Transportation Memorandum	<u>SP1415</u>	LSC Transportation Consultants, Inc	October 2, 2017
Branding Iron at Sterling Ranch Filing No. 1 and Homestead at Sterling Ranch Filing No. 1 Transportation	<u>SF1724</u> <u>SF1725</u>	LSC Transportation Consultants, Inc	December 19, 20
Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF1820</u>	LSC Transportation Consultants, Inc	April 3, 2018
Sterling Ranch Phase 2 Preliminary Plan Traffic Impact Study	<u>SP203</u>	LSC Transportation Consultants, Inc	December 20, 20
Homestead at Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF194</u>	LSC Transportation Consultants, Inc	March 3, 2020
Branding Iron at Sterling Ranch Filing No. 2 Transportation Memorandum	<u>SF1918</u>	LSC Transportation Consultants, Inc	May 6, 2020
Sterling Ranch Filing No. 2 and Phase 2 Traffic Impact Study	<u>SF2015</u> <u>SP191</u>	LSC Transportation Consultants, Inc	June 23, 2021
Sterling Ranch Filing No. 3 Transportation Memorandum	<u>SF2132</u>	LSC Transportation Consultants, Inc	April 19, 2022
Homestead North Phase 1 Updated Transportation Memorandum	<u>SP208</u>	LSC Transportation Consultants, Inc	January 11, 2022
Homestead North Filing No. 1 Traffic Technical Memorandum	<u>SF2213</u>	LSC Transportation Consultants, Inc	February 2, 2022
Homestead North Filing No. 2 Traffic Technical Memorandum	<u>SF2218</u>	LSC Transportation Consultants, Inc	April 15, 2022
Homestead North Filing 3 Traffic Impact Study	<u>SF2229</u>	LSC Transportation Consultants, Inc	June 17, 2022
The Villages at Sterling Ranch East Preliminary Plan/Traffic Generation Analysis	PUDSP226	SM Rocha, LLC	July 1, 2022
Sterling Ranch Sketch Plan Amendment Master Traffic Impact Study	<u>SKP224</u>	LSC Transportation Consultants, Inc	March 17, 2023
Sterling Ranch East - Rezoning & Preliminary Plan Traffic Impact Study	SP-22-004, P-22-012, P-22-013	LSC Transportation Consultants, Inc	March 17, 2023 ⁽²
Sterling Ranch East Filing Nos 1 & 2 Traffic Technical Memorandum	<u>SF2235</u> <u>SF2237</u>	LSC Transportation Consultants, Inc	February 10, 202
Sterling Ranch Filing No. 4 Transportation Memorandum	<u>SF2230</u>	LSC Transportation Consultants, Inc	February 21, 202
Foursquare at Sterling Ranch East Transportation Memorandum	<u>SF2236</u>	LSC Transportation Consultants, Inc	April 20, 2023
etreat at TimberRidge Reports			
The Retreat at TimberRidge Traffic Impact Analysis	<u>PUD173</u>	LSC Transportation Consultants, Inc	January 25, 2018
The Retreat at TimberRidge Preliminary Plan Traffic Technical Memorandum	<u>SP182</u>	LSC Transportation Consultants, Inc	June 29, 2018
The Retreat at TimberRidge Filing No. 1 Traffic Technical Memorandum	<u>SF199</u>	LSC Transportation Consultants, Inc	April 3, 2020
The Retreat at TimberRidge Filing No. 2 Updated Traffic Technical Memorandum	SF2121	LSC Transportation Consultants, Inc	October 4, 2021
The Retreat at TimberRidge Filing No. 3 Traffic Technical Memorandum	<u>SF2241</u>	LSC Transportation Consultants, Inc	July 1, 2022
ther Area Reports			
Wolf Ranch School Site Traffic Impact Study	OAR1720	Matrix Design Group, Inc.	5-May-17
The Ranch Sketch Plan Traffic Impact Analysis	SKP186	LSC Transportation Consultants, Inc	July 9, 2019
Lodge III Traffic Impact Study	OAR	LSC Transportation Consultants, Inc	December 13, 20
Continental 613 Traffic Impact Study	<u>OAR2177</u>	LSC Transportation Consultants, Inc	July 16, 2021
	OAR2134	LSC Transportation Consultants, Inc	August 13, 2021
Solace at Black Forest Traffic Impact and Access Analysis		SM Rocha, LLC	October, 2021
Solace at Black Forest Traffic Impact and Access Analysis Traffic Impact Study Addendum for Percheron	OAR2173	SIVI NOCIIa, LLC	
	OAR2173 OAR2191	LSC Transportation Consultants, Inc	
Traffic Impact Study Addendum for Percheron		·	December 8, 202 May, 2022
Traffic Impact Study Addendum for Percheron Woodmen East Commercial Center Traffic Impact Analysis	<u>OAR2191</u>	LSC Transportation Consultants, Inc	December 8, 202
Traffic Impact Study Addendum for Percheron Woodmen East Commercial Center Traffic Impact Analysis Traffic Impact Study for Jaynes Property	OAR2191 SKP225	LSC Transportation Consultants, Inc SM Rocha, LLC	December 8, 202 May, 2022
Traffic Impact Study Addendum for Percheron Woodmen East Commercial Center Traffic Impact Analysis Traffic Impact Study for Jaynes Property Traffic Impact Study for Rhetoric Site	OAR2191 <u>SKP225</u> <u>P2216</u>	LSC Transportation Consultants, Inc SM Rocha, LLC SM Rocha, LLC	December 8, 202 May, 2022 June, 2022

		Appendix Table 2 ⁽¹⁾									
		(Page 1 of 2)									
		Sterling Ranch									
Roadway Segment Improvements											
Segment ID ⁽²⁾ (See Figure 14 for map)	Improvement Description	Timing	Design ADT (vpd)	Projected 2042 ADT (vpd)	Responsibility						
V1 northbound V1 southbound	Per the City of Colorado Springs, an outside paved shoulder will need to be added along the east side of Vollmer Road from Dry Needle Place up to the south end of segment V2 improvements.	With Sterling Ranch Filing No. 4 but potentially complete concurrently with the construction of the right-turn lane at Pioneer Landscape Center access for the Sterling Ranch Recycling Facility (PCD No. PPR2241)	5,500 (Directional northbound) 10,000 (Directional southbound)	16,275	Sterling Ranch						
V1	Improve Vollmer Road between Dry Needle Place and the Sterling Ranch south boundary to a standard 4-Lane Urban Minor Arterial Cross Section (add a second northbound through lane and painted center median). (3)	The need driven by anticipated traffic from each development impacting this section of Vollmer Road.	20,000		Sterling Ranch, if necessary, prior to construction by others.						
V2	Improve Vollmer Road between the Sterling Ranch south boundary to Lochwinnoch Lane/Sterling property boundary to a standard 4-Lane Urban Minor Arterial Cross Section. (3)	Short-Term Future (With Sterling Ranch Fil No. 2 Or Sterling Ranch Phase 2)	20,000 (Note: Existing Capacity 8,000 ⁽³⁾)	17,475	Sterling Ranch						
V3	Short Term: Improve Vollmer Road from Lochwinnoch Lane to Sterling Ranch boundary (northeast of Glider Loop) to provide 36' of pavement (existing pavement 1 approx. 23.38') and stripe for one through lane plus a 6' paved, striped outside shoulder in each direction. (3)	Short-Term Future (With Homestead North)	11,000 (Note: Existing Capacity 8,000)		Sterling Ranch						
	Long Term: Improve Vollmer Road from Lochwinnoch Lane to Sterling Ranch boundary (northeast of Glider Loop) to a standard 4-Lane Urban Minor Arterial Cross Section. (3)	Long-Term Future	20,000	17,380	Sterling Ranch with potential County assistance with ROW acquisition - pursuant to the recent development agreement between Sterling Ranch and EPC.						
V4	Improve Vollmer Road from Sterling Ranch boundary (northeast of Glider Loop) to Briargate Parkway to a standard 4- Lane Urban Minor Arterial Cross Section. ⁽³⁾	Short-Term Future— May 2024Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: With Homestead North Filing 1)	20,000	16,445	Sterling Ranch						
V5	Improve Vollmer Road from Briargate Parkway to Jane Kirkham Drive to a standard 4-Lane Urban Minor Arterial Cross Section. (3)	Short-Term Future— May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: prior note: With Homestead North Filing 1)	20,000	11,690	Sterling Ranch						
V6	Improve Vollmer Road from Jane Kirkham Drive to Sam Bass Drive to a standard 4-Lane Urban Minor Arterial Cross Section. (3)	Short-Term Future— May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: prior note: With Homestead North Filing 2)	20,000	11,425	Sterling Ranch						
V7	Improve Vollmer Road between Sam Bass Drive and Poco Road to a 4-lane Urban Minor Arterial but with necessary lane transitions, redirect tapers, etc. south of Poco to adequately transition between the 4-Lane Urban Minor Arterial Cross Section and the 2-Lane Rural Arterial Cross Section north of Poco Road.	Short-Term Future – May 2024 Updated 10/15/2022 - Sections V4, V5, V6 to be constructed by May 2024 (prior note: With Homestead North Filing 3)	20,000	10,030	Sterling Ranch						
V8	Improve Vollmer Road from Poco Road to Shoup Road to a Rural 2-Lane Arterial Cross Section. (3)	Long-Term Future	10,000	11,790	El Paso County Project ID U-12						
Part 1/2 of this	table (see Part 2 on next page)		1	L							

Notes:

- (1) Source: This Appendix Table 2 is a copy of Table 6 from the Sterling Ranch East Phase 1 Rezoning and Preliminary Plan TIS, March 17, 2023 (with minor revisions April 3, 2023) with LSC notes regarding this Copper Chase project. Improvements needed prior to Copper Chase at Sterling Ranch have been highlighted in green.
- (2) See Figure 14
- (3) Adequate transition/redirect tapers would be needed between the various cross sections on Vollmer Road. Based on the criteria contained in Table 2-29 of the *El Paso Engineering Criteria Manual*, an appropriate taper ratio for a roadway with a design speed of 40 miles per hour is 20:1
- (4) Source: Table 20 Road Impact Fee Study Updated November 16, 2016

Source: LSC Transportation Consultants, Inc. (April 26, 2023)

	A	ppendix Table 2 ⁽¹⁾										
	(Page 2 of 2)											
	Sterling Ranch Roadway Segment Improvements											
Segment ID ⁽²⁾ (See Figure 14 for map)	Improvement Description	Timing	Design ADT (vpd)	Projected 2042 ADT (vpd)	Responsibility							
SR1	Construct Sterling Ranch Road as an Urban Non-Residential Collector from Marksheffel Road to Dines Boulevard.	Short Term - with Sterling Ranch Fil No. 2	20,000	14,840	Sterling Ranch							
SR2	Construct Sterling Ranch Road as an Urban Non-Residential Collector from Dines Boulevard to Briargate Parkway.	Short-Term	20,000	10,275	Sterling Ranch							
SR3	Construct Sterling Ranch Road as an Urban Collector from Briargate Parkway to Vancouver Street.	Short Term	10,000	9,300	Sterling Ranch							
SR4	Construct Sterling Ranch Road from Vancouver Street north to Arroya (or ultimate north terminus).	Long-Term Future	10,000	4,260	Sterling Ranch							
M1	Construct Marksheffel Road as an Urban Principal Arterial to City of Colorado Springs standards in 107' of right-of-way between Vollmer Road and Sterling Ranch Road.	Updated 10/15/2022: to be completed by the end of 2022 (prior note: With Sterling Ranch Fil No. 2	40,000	23,370	Sterling Ranch							
M2	Construct Marksheffel Road as an Urban Principal Arterial to City of Colorado Springs standards in 107' of right-of-way between Sterling Ranch Road and the south boundary of the Sterling Ranch Master Plan Area. 10/16/2022 NOTE: With the completion of M2 in 2023, the connection between Vollmer and Woodmen Road (via M3) will be completed. Construct Marksheffel Road between the south boundary of the Sterling Ranch Master Plan Area and Woodmen Road.	Short Term Updated 10/15/2022: to be completed in 2023 (prior note: With Sterling Ranch Phase 2)	40,000	29,600	Sterling Ranch							
M3	(Note this segment is located within the City of Colorado Springs). 10/16/2022 NOTE: With the completion of M2 in 2023, the connection between Vollmer and Woodmen Road (via M3) will be completed.	Updated 10/15/2022: <u>Completed</u> (by Others)	40,000	24,525	Others (Completed)							
M4	Construct Marksheffel Road between Black Forest Road and Vollmer Road.	Long-Term Future	40,000	27,910	Others							
B1	Construct the south half section of Briargate Pkwy (4-Lane Principal Arterial) between Vollmer Road and Wheatland Drive [now full section by 2023].	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 with Homestead at Sterling Ranch Filing No. 1 (prior note: With Homestead at Sterling Ranch Fil 2)	20,000	- 24,745	Sterling Ranch							
B1	Construct the north half section of Briargate Pkwy (4-Lane Principal Arterial) between Vollmer Road and Wheatland Drive [now full section by 2023].	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 with Homestead at Sterling Ranch Filing No. 1 (prior note: Long-Term Future)	40,000	24,743	Sterling Ranch							
B2	Construct Briargate Pkwy (full section) as a 4-Lane Principal Arterial between Wheatland Dr and Sterling Ranch Road.	Short-Term Future Updated 10/15/2022: Full section to be completed in 2023 or Spring 2024 (prior note: Long-Term Future)	40,000	26,375	Sterling Ranch							
В3	Construct Briargate Pkwy as a 4-Lane Principal Arterial between Sterling Ranch Road and Banning Lewis Parkway.	Intermediate Term	40,000	22.365	Sterling Ranch							
В4	Construct Stapleton Road as a 4-Lane Principal Arterial between Banning Lewis Parkway and Meridian Road (including upgrade of existing rural two-lane segment between Towner and Meridian).	Long-Term Future	40,000	17,945	Others							
B5	Construct Briargate Pkwy as a 4-Lane Principal Arterial between Black Forest Road and Vollmer Road.	Long-Term Future	40,000	24,340	Others; PPRTA A List Project							
BL1	Construct Banning Lewis Parkway as a 4-Lane Principal Arterial between the south Sterling Ranch boundary and Briargate Pkwy.	Long-Term Future	40,000	20,320	Financial assurances for half-section, Sterling Ranch half-section or full-section w/ cost recovery							
BL2	Construct Banning Lewis Parkway as a 4-Lane Principal Arterial between Woodmen Road and the south Sterling Ranch boundary. (Note this segment will be located within the City of Colorado Springs)	Long-Term Future	40,000	28,480	Others							
W1	Widen Woodmen Road from 4-lane to 6-lane section from Powers Boulevard to US 24.	Long-Term Future	72,000	66,690	PPRTA A-List Project; City of Colorado Springs ConnectCOS Index No.476							
B1	Widen Black Forest Road between Woodmen Road to just north of Research Road to two northbound and southbound through lanes.	Black Forest Widening Project	40,000	28,420	City of Colorado Springs							
B2	Widen Black Forest Road from just north of Research Road to Briargate Parkway.	Long-Term Future	40,000	25,145	Others/City of Colorado Springs							

Part 2/2 of this table

В3

Notes:

(1) Source: This Appendix Table 2 is a copy of Table 6 from the Sterling Ranch East Phase 1 Rezoning and Preliminary Plan TIS, March 17, 2023 (with minor revisions April 3, 2023) with LSC notes regarding this Copper Chase project. Improvements needed prior to Copper Chase at Sterling Ranch have been highlighted in green.

Long-Term Future

PPRTA B List Project

ConnectCOS Index No. 479

40,000

19,135

(2) See Figure 14

Source: LSC Transportation Consultants, Inc. (April 26, 2023)

Widen Black Forest Rd from Briargate Pkwy to Old Ranch Rd as a 4-lane Principal Arterial with bike and pedestrian