

STRUCTURAL NOTES  
GENERAL:

1. Dimensions: The structural drawings shall be considered as a part of the complete set of Contract drawings, including the drawings of all disciplines. It is intended that the Structural drawings will provide sufficient dimensions to locate the primary structural elements and members. Location of secondary members which are affected by systems detailed by others may require reference to the drawings of other disciplines and layout and coordination by the contractor. If direct conflict between dimensions of two or more disciplines is encountered, such conflicts shall be resolved by the Architect. Do not use scaled dimensions. Use written dimensions or where dimensions are not provided, consult the architect for clarifications before proceeding with the work in question.
2. Omissions or conflicts between various elements of the drawings, specifications, notes, and details shall be brought to the attention of the structural engineer and resolved before proceeding with the work. The contractor must submit in writing any requests for modifications to the plans and specifications. Shop drawings submitted to the structural engineer for review do not constitute "in writing" unless it is clearly noted that specific changes are being requested.
3. Deferred Submittals: Where Structural components are fully or partially designed and detailed by the supplier or fabricator, complete shop drawings and calculations, signed and sealed by a professional engineer registered in the state where the project is located, shall be submitted to the structural engineer for review. In addition, a copy of these documents shall be submitted to the Building Official for approval in accordance with IBC Section 107.3.4.1.
4. The Contract drawings and specifications represent the finished structure. They do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure during construction. Such measures shall include but not be limited to bracing and shoring for loads due to construction equipment and materials.

DESIGN CRITERIA:

1. Used 2021 International Building Code.
  - 1.1. Risk Category = II
2. ASD Design Loads:
  - 2.1. Roof: D.L. = 12#/SF., S.L. = 48#/SF. Pf
  - 2.2. Loft: D.L. = 15#/SF., D.L. = 40#/SF. Accessible Access
3. Wind load = 100 MPH (nominal), 130 MPH (ultimate), exposure C, Iw = 1.0.
4. Seismic: Equivalent Static Force Design Procedure.  
Seismic Design Category B, Site Class D.  
Ss = 0.194 SDS = 0.207  
S1 = 0.056 SD1 = 0.090  
R = 2.5 - Light framed walls w/shear panels all other materials.  
Ie = 1.0.
5. Roof Snow Load Factors:  $P_f = 0.7(C_e)(C_t)(I)P_g$ 

|                     |                   |
|---------------------|-------------------|
| Elevation           | = 7339 ft.        |
| Importance (I)      | = 1.0             |
| Thermal (Ct)        | = 1.2             |
| Exposure (Ce)       | = 1.0             |
| Ground Snow (Pg)    | = 57 psf.         |
| Flat Roof Snow (Pf) | = 48 psf.         |
| Slippery Roof       | = Yes (Cs = 0.94) |

QUALITY ASSURANCE:

FOUNDATIONS:

1. Followed recommendations from soils investigation report No. CS20082-125 prepared by CTL Thompson dated July 1, 2026.
2. Maximum foundation soil bearing pressure used = 3,000#/SF.

MATERIALS:

CONCRETE:

1. Mix design shall be established in accordance to Chapter 5 of ACI 318.
2. Minimum foundation cement content = 376#/YD.
3. Maximum slump = 4".
4. Foundation 28 day strength  $f'_c$  = 2,500 PSI. Special inspection not required per IBC 1705.3, exception 2.
5. Pre-Engineered Perma-Column 28 day strength  $f'_c$  = 10,000 PSI.  
\*(If using Perma Column's)\*

STRUCTURAL AND MISCELLANEOUS STEEL:

1. All steel work shall conform with AISC specifications.
2. Bolts ASTM A307 for connections to concrete. Bolts ASTM A325 for steel to steel connections. Bolts to be snug tight except bolts indicated as S.C. to be fully tightened.
3. Roof Steel shall be painted 29 Ga, ribbed steel and shall be attached to framing with 1 ½" x #9 screws with neoprene washers at 9" o.c.
4. Wall Steel shall be painted 26 Ga, Tuff Rib steel and shall be attached to framing with 1 ½" x #9 screws with neoprene washers at 9" o.c.
5. Soffit Steel shall be painted 26 Ga, Tuff Rib steel and shall be attached to framing with 1 ½" x #9 screws with neoprene washers at 9" o.c.
6. Fascia Steel shall be painted 29 Ga, ribbed steel and shall be attached to framing with 1 ½" x #9 screws with neoprene washers
7. All sheet steel shall be UL 790 Class A Fire Rated.

LUMBER:

1. Sawn lumber for studs, joists, etc.(2x6 or larger) = No.2 Doug Fir larch or SPF 1250 psi. or better.
2. 2x4's = Standard Doug Fir larch.
3. Posts (Interior) = Triad Building Components MSR SPF.
  - 3.1. 2x8 Post Lam Uppers = 1,950Fb-1.7E
  - 3.2. 2x6 Post Lam Uppers = 1,650Fb-1.5E
  - 3.3. (Loft Exception: Uppers to be Hem-Fir or Doug-Fir per Schedule.)
  - 3.4. #1 Treated So. Pine Lam when embedment is in contact with ground. (Excludes Perma Columns)
4. Roof Sheathing = ¾", minimum.
  - 4.1. Nailing = 8d(.131 Ø shank x 2 ½" long) or 1 ½" x 16Ga. staples at 6" o.c. all supported edges, 12" o.c. in field.
5. Wall Sheathing = ¾", minimum.
  - 5.1. Nailing = 8d(.131 Ø shank x 2 ½" long) or 1 ½" x 16Ga. staples at 6" o.c. all supported edges, 12" o.c. in field. (Non-Structural, Unblocked U.N.O.)
6. Loft Sheathing = ¾", minimum.
  - 6.1. Nailing = 10d(.148 Ø shank x 3" long) at 6" o.c. all supported edges, 12" o.c. in field.
7. All nails are to be common nails unless noted otherwise.
8. For connections of "SIMPSON/MITEK" hardware or equivalent follow manufacturers recommendations.
9. Engineered lumber products:
  - 9.1. Roof or floor joists shown as BCI etc. shall be designed for the loads specified and shall conform to Boise Cascade specification.
  - 9.2. Joists exceeding 24' in length shall be cambered to a standard radius of R = 2250.
  - 9.3. Any alternate joist system(s) shall be the same depth and load Carrying capacity as the Boise Cascade system show on the drawings.
  - 9.4. Techlam (LVL) E-2,000,000 psi.
10. Structural Glued Laminated Timber (GLB) = 24f-V4 (DF/DF) with standard camber.

11. Premanufactured Trusses

- 11.1. Truss Loading:

|                   |                                                                |
|-------------------|----------------------------------------------------------------|
| Top Chord D.L.    | = 5 PSF.                                                       |
| Bottom Chord D.L. | = 7 PSF.                                                       |
| Flat Roof S.L.    | = 48 PSF.                                                      |
| Bottom Chord L.L. | = 10 PSF. <i>Does not act concurrently with top chord L.L.</i> |
- 11.2. Member Properties:  
Chords shall be #2 Douglas Fir or better.  
Webs shall have minimum Modulus of Elasticity of 1,500,000 psi.
- 11.3. All truss blocking shall be provided by the truss manufacturer and constructed with approved plates.
- 11.4. Truss Manufacturer shall verify all truss dimensions, accounting for tolerances, connections, and splice requirements.
- 11.5. Truss profiles shown are representations of possible configurations of Web locations and member sizes. Truss manufacturer shall submit shop drawings for approval. All trusses shall be designed by a registered professional engineer and all shop drawings shall be stamped and signed by a registered professional engineer.
- 11.6. Truss manufacturer shall provide proof of approved third party I inspection as required by IBC chapter 2303.4.
- 11.7. Truss manufacturer shall design all truss to truss connections and shall indicate said connections on the shop drawings.
- 11.8. Each truss shall be marked with the following information:
  - 11.8.1. Manufacturers identity.
  - 11.8.2. Design Load.
  - 11.8.3. Truss spacing.
12. All lumber in contact with concrete, masonry, or ground shall be preservative treated wood in accordance with AWPA standards. Posts embedded in the ground shall comply with UC4B .60 treatment or greater.



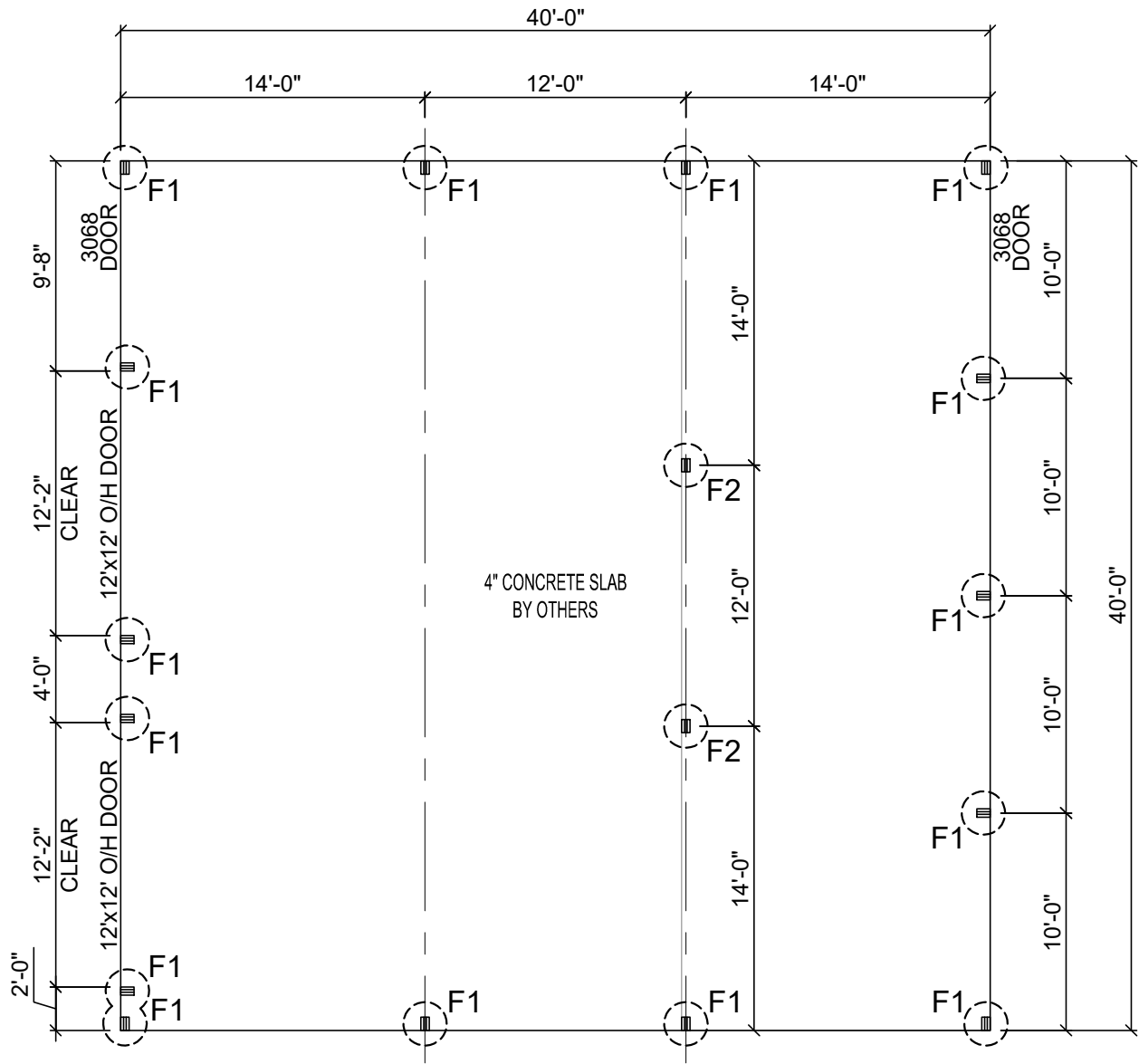
STRUCTURAL NOTES

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POST FRAME BLOG FOR:  
CHRISTOPHER LEACH  
11005 HUNTSMAN RD  
COLORADO SPRINGS, CO

|           |            |
|-----------|------------|
| PROJECT # | 14889      |
| DRAWN BY  | AG         |
| DRAW DATE | 06/17/2026 |
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| FOOTING SCHEDULE |       |       |             |           |        |
|------------------|-------|-------|-------------|-----------|--------|
| MARK             | "Ø"   | DEPTH | POST        | PUNCH PAD | DETAIL |
| F1               | 2'-0" | 3'-6" | 3 - 2x8 SPF | ----      | 2/S3.1 |
| F2               | 2'-0" | 2'-6" | 3 - 2x8 SPF | ----      | 2/S3.1 |
| F3               | ----  | ----  | ----        | ----      | ----   |
| F4               | ----  | ----  | ----        | ----      | ----   |



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CHRISTOPHER LEACH

11005 HUNTSMAN RD  
COLORADO SPRINGS, CO

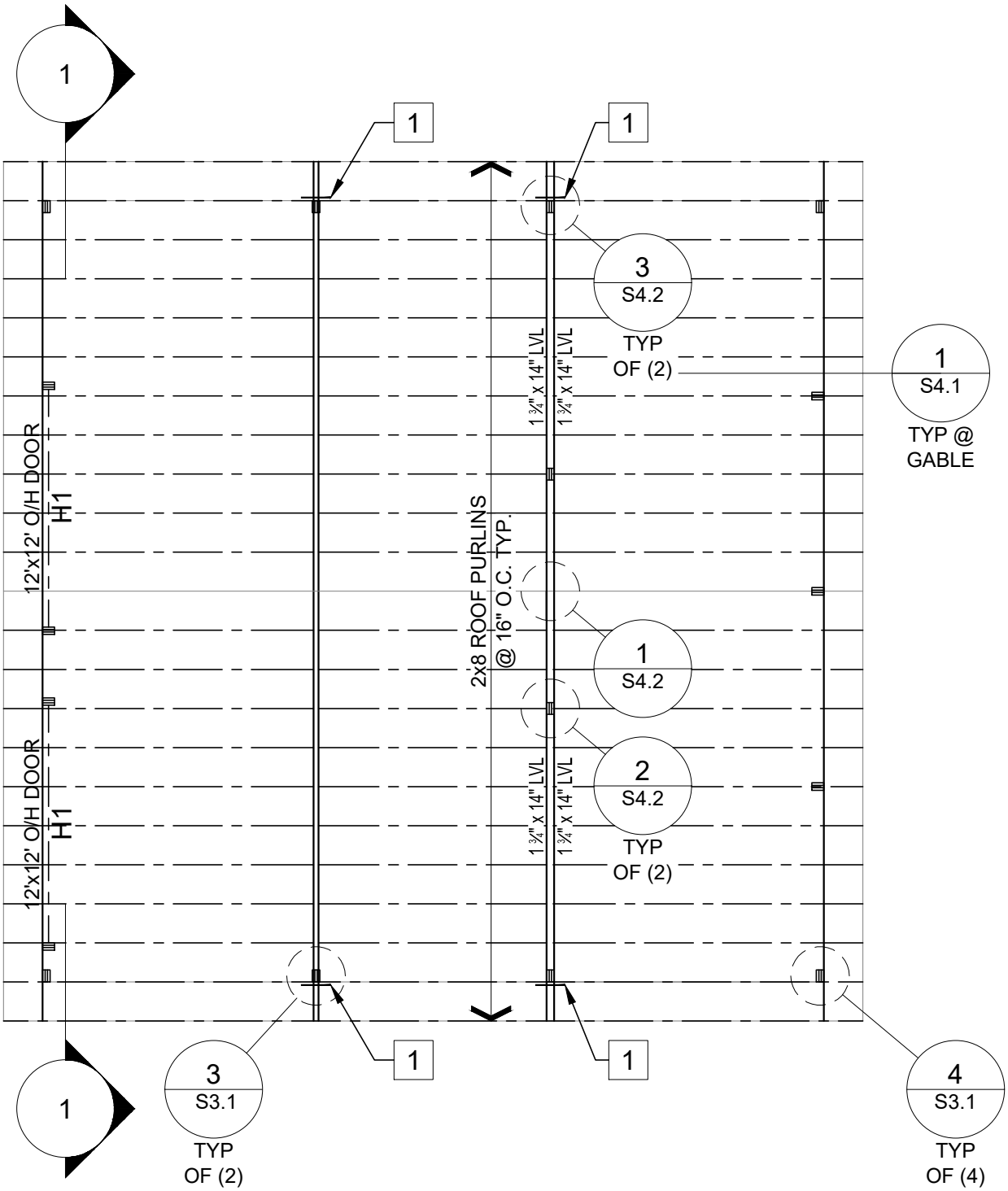
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ROOF FRAMING PLAN

SCALE: 1/8" = 1'



- ROOF FRAMING NOTES:**
- 1 LSTA18 STRAP- PLACE AT INSIDE OF FASCIA BOARD IF NO OVERHANGS. PLACE AT INSIDE FACE OF TOP WALL GIRT IF OVERHANGS
  - 2 CONFIRM WINDOW & MAN DOOR LOCATIONS w/OWNER.

| HEADER SCHEDULE |                                 |               |
|-----------------|---------------------------------|---------------|
| MARK            | MEMBERS                         | SUPPORT       |
| H1              | V = 1-2x10 DF#2; H = 1-2x6 DF#2 | 1-2x8 TRIMMER |
| H2              | V = 1-2x6 DF#2; H = 1-2x8 DF#2  | ----          |
| H3              | H = 1-2x8 DF#2;                 | ----          |
| H4              | ----                            | ----          |

NOTE: REFER TO DETAILS 2, 3 & 4/S4.1 FOR HEADER FRAMING CONDITIONS.



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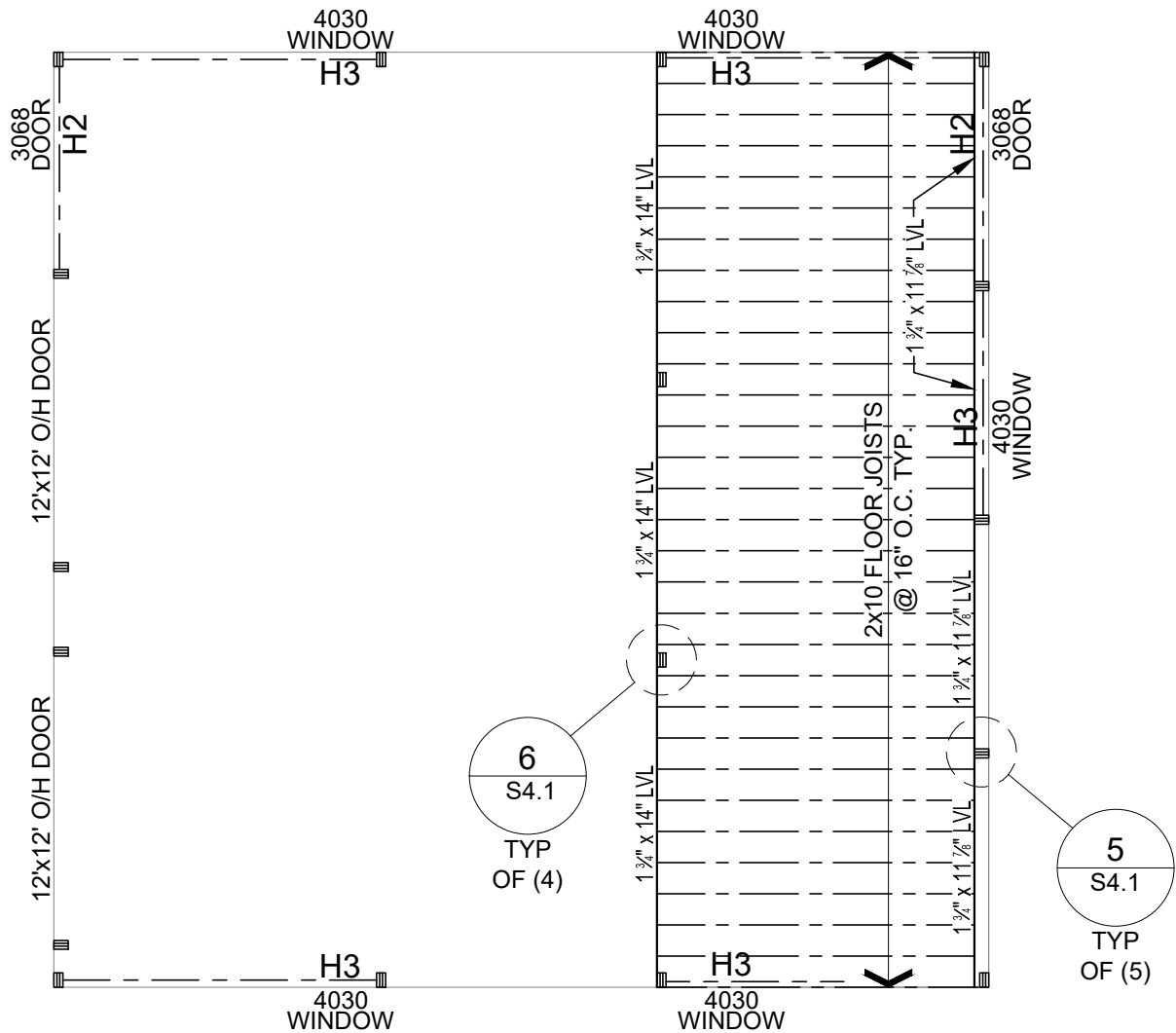
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ROOF FRAMING  
PLAN

LOFT FRAMING PLAN

SCALE: 1/8" = 1'



LOFT FRAMING NOTES:

- 1 ATTACH 2x10 JOISTS TO GLB/RIM JOIST w/HU210 HANGER TYP.

| HEADER SCHEDULE |                                 |               |
|-----------------|---------------------------------|---------------|
| MARK            | MEMBERS                         | SUPPORT       |
| H1              | V = 1-2x10 DF#2; H = 1-2x6 DF#2 | 1-2x8 TRIMMER |
| H2              | V = 1-2x6 DF#2; H = 1-2x8 DF#2  | ----          |
| H3              | H = 1-2x8 DF#2;                 | ----          |
| H4              | ----                            | ----          |

NOTE: REFER TO DETAILS 2, 3 & 4/S4.1 FOR HEADER FRAMING CONDITIONS.



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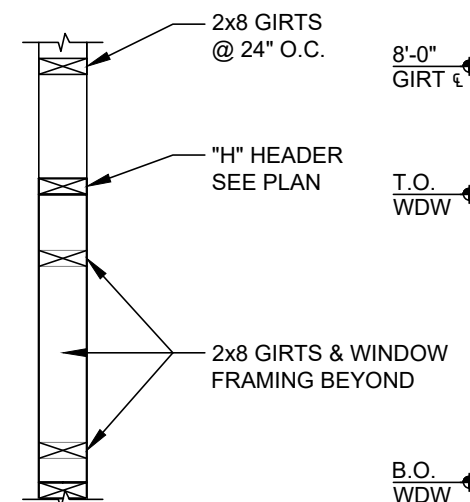
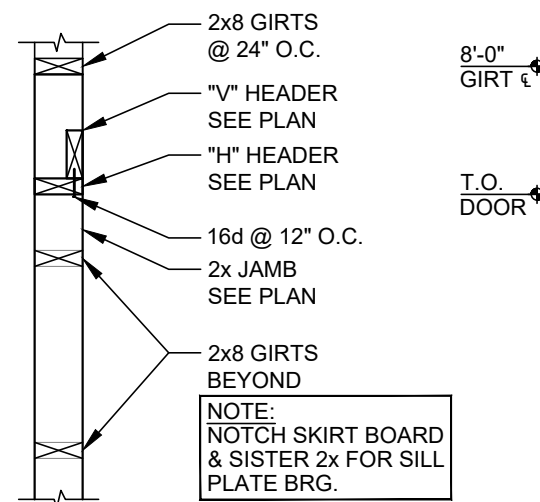
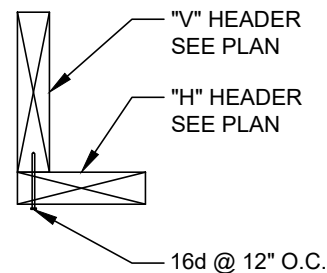
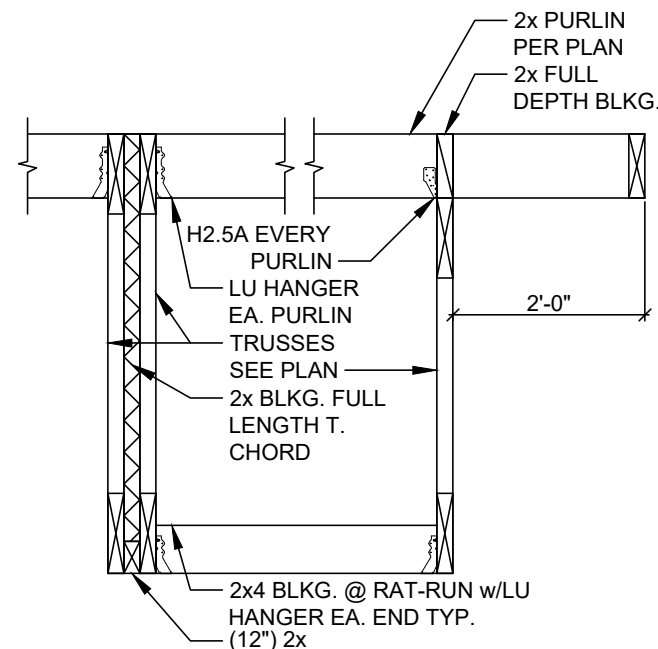
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LOFT FRAMING PLAN



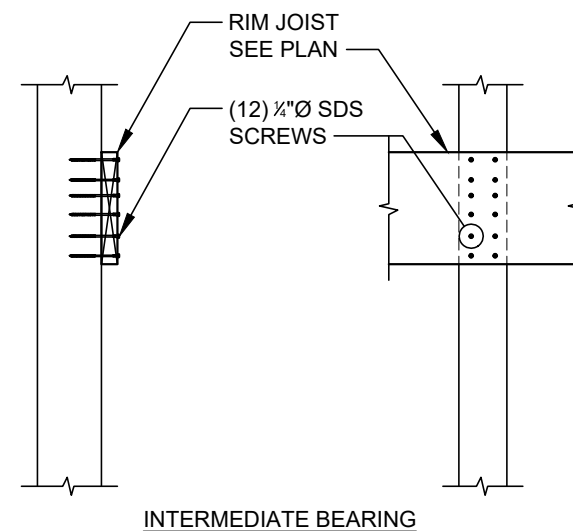
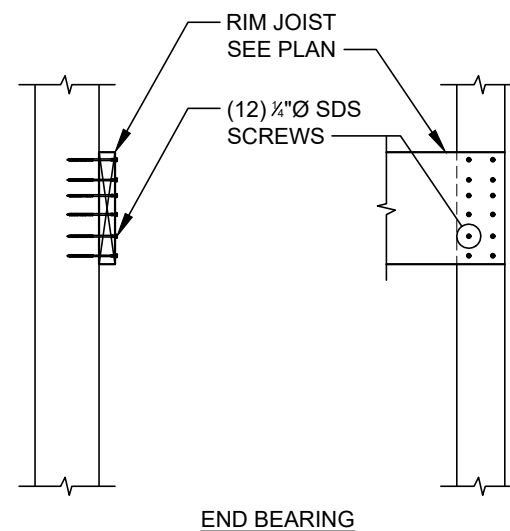
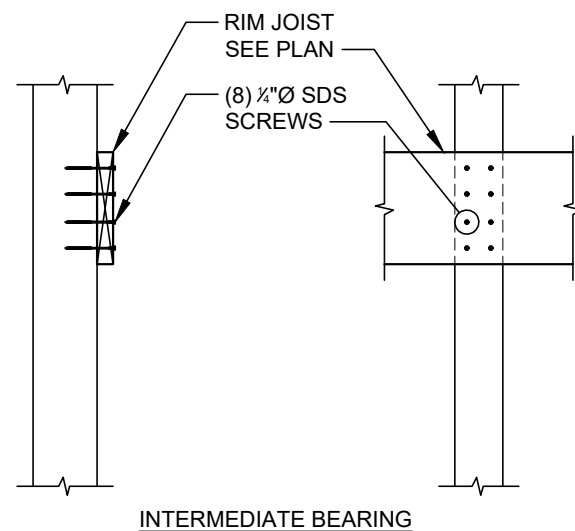
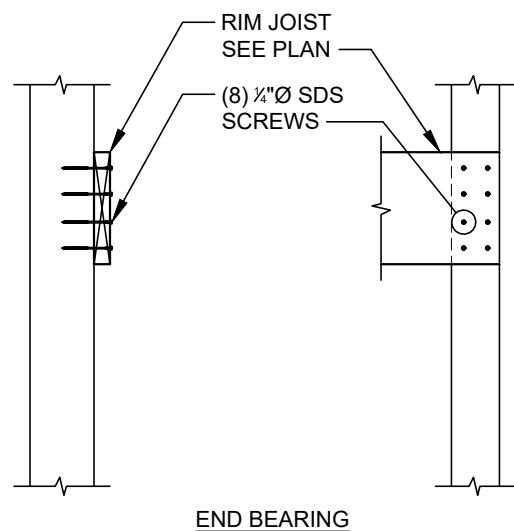


1 GABLE TRUSS FRAMING DETAIL SCALE: 1/2"=1'-0"

2 HEADER DETAIL SCALE: 1"=1'-0"

3 MAN DOOR HEADER DETAIL SCALE: 1/2"=1'-0"

4 WINDOW HEADER DETAIL SCALE: 1/2"=1'-0"

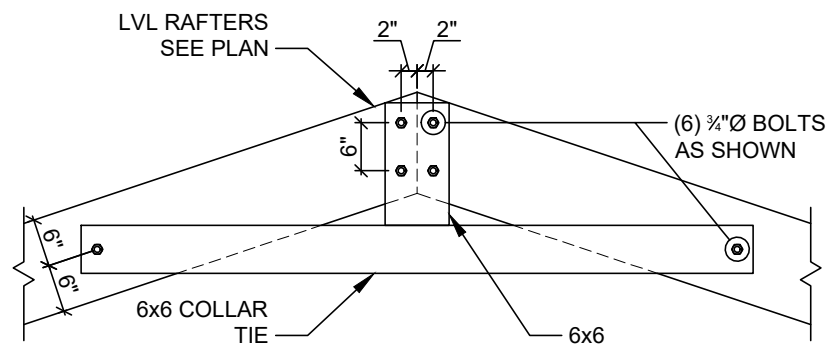


5 LOFT RIM CONNECTION DETAIL SCALE: 1/2"=1'-0"

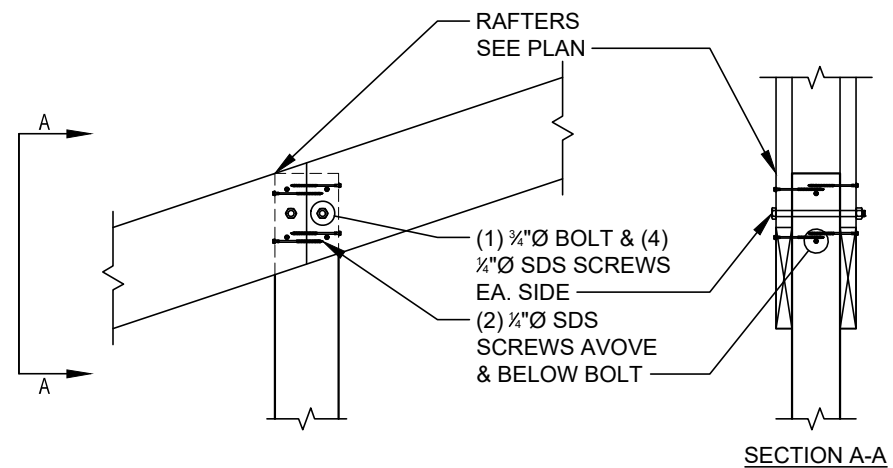
6 LOFT RIM CONNECTION DETAIL SCALE: 1/2"=1'-0"



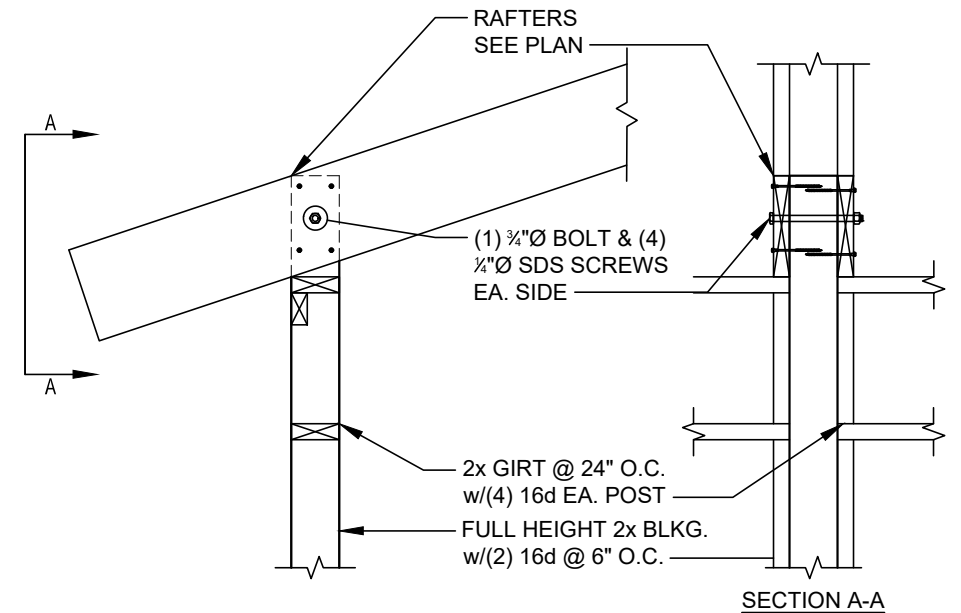
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1 RAFTER CONNECTION DETAIL  
SCALE: 1/2"=1'-0"



2 ROOF FRAMING CONNECTION DETAIL  
SCALE: 1/2"=1'-0"



3 ROOF FRAMING CONNECTION DETAIL  
SCALE: 1/2"=1'-0"



FRAMING  
DETAILS

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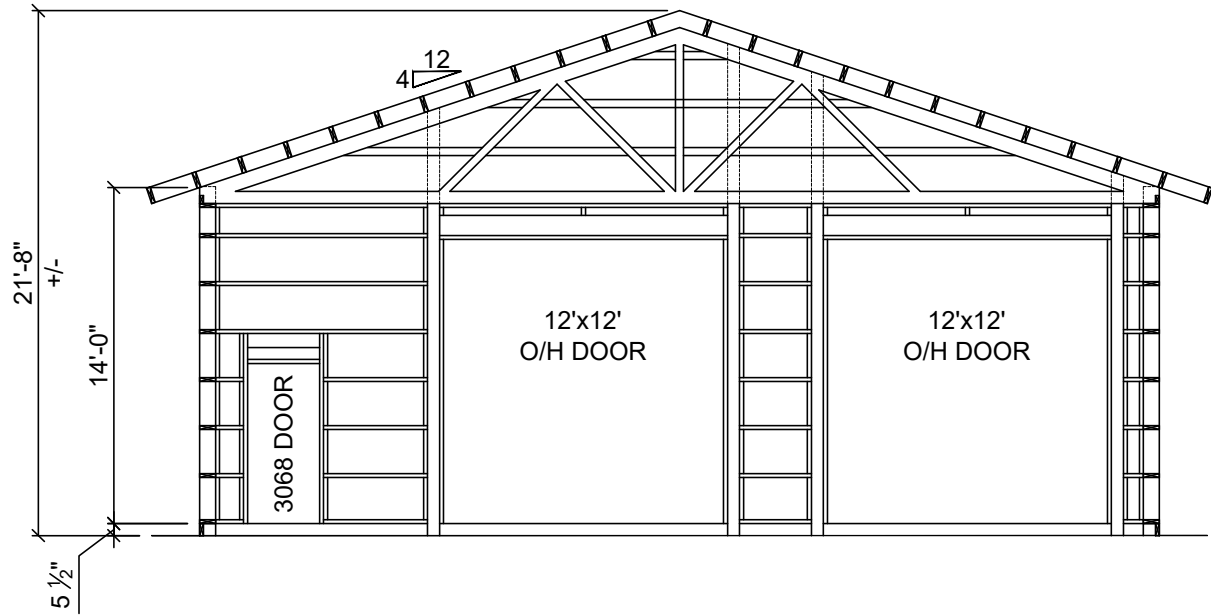
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COLORADO SPRINGS, CO

11005 HUNTSMAN RD

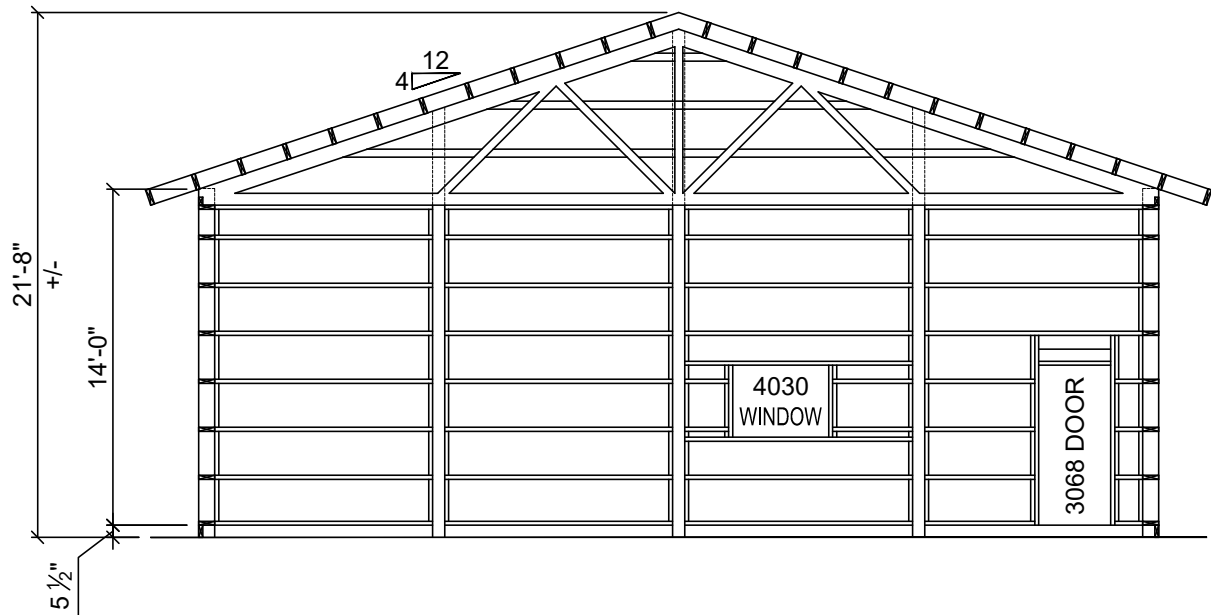
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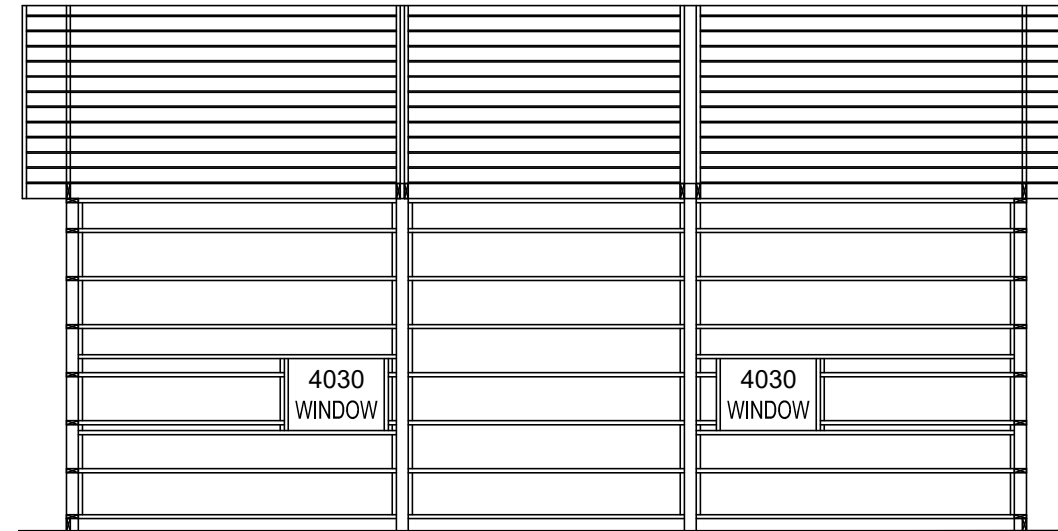
FRONT ELEVATION

SCALE: 1/8" = 1'



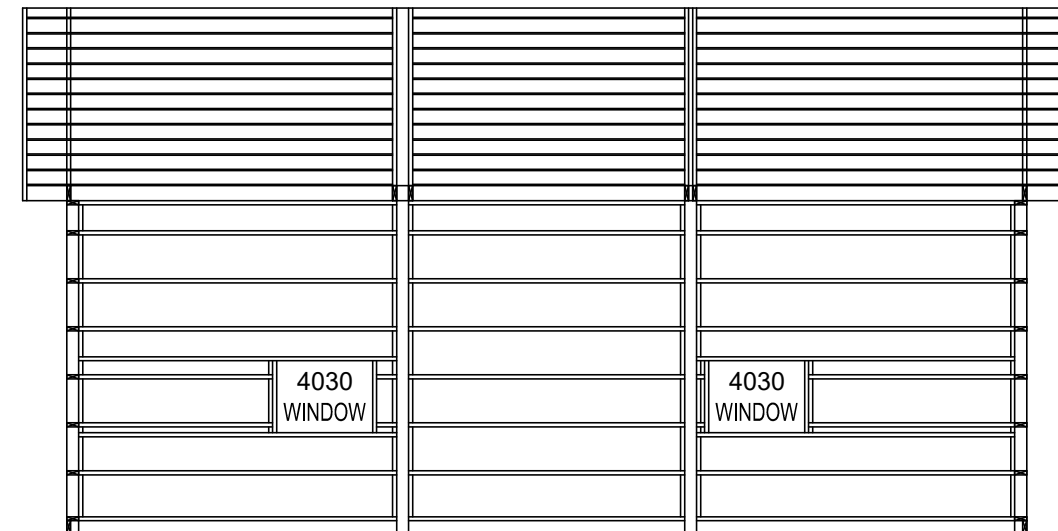
REAR ELEVATION

SCALE: 1/8" = 1'



RIGHT ELEVATION

SCALE: 1/8" = 1'



LEFT ELEVATION

SCALE: 1/8" = 1'



ELEVATIONS

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