

Les Schwab Tire Center Storm Report



**7105 Old Meridian RD.
Falcon Colorado**

Prepared For:

SFP-E, LLC
PO Box 5350
20900 Cooley Road
Bend, OR 97701

Prepared By:

Cushing Terrell

Cushing Terrell

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Cushing Terrell Project No. LSCO_21WIN
PCD FILLING NO. : PPR-21-023
JULY, 21, 2021



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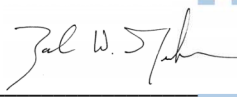
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Design Engineer’s Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

 Digitally signed by
Zack Graham
Date: 2021.10.11
12:56:02 -06'00'


10/11/21

Date

[Zack Graham, PE #39039]

Owner/Developer’s Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

 DocuSigned by:
George Bunting
603648E0C89B42D...

10/11/21

Date

George Bunting,
SFP-E, LLC
P.O. BOX 5350 Bend OR 97708

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Jennifer Irvine, P.E.
County Engineer / ECM Administrator

APPROVED
Engineering Department —
10/18/2021 4:23:50 PM

EPC Planning & Community
Development Department

Conditions:

2.0 GENERAL LOCATION AND DESCRIPTION

2.1 Location

The project site is located at 7105 N Meridian Rd, Falcon, Colorado and falls within El Paso County. The parcel is part of the larger Meridian Crossing Development, which includes the existing stormwater system infrastructure, including the treatment pond to the south. The site is located on the northeast side of the intersection of Meridian Rd and N Meridian Rd. The site is located to north of the existing storm water treatment facilities maintained by the Meridian Crossing Development and an existing storm line runs along the south west property line of the site. The property lies within the NE ¼ of Section 12, Township 13 S, Range 65 West of the Sixth Principal Meridian.

2.2 Description of Property

The existing site consists of an undeveloped 2.48 acre lot covered with native grasses and shrubs. In areas taken from the ALTA Survey the site consists of roughly 12% impervious road and sidewalk area with the remaining 88% being the native vegetation. There are no stream crossings or significant waterways located within the area being developed by this project. The site is accessed via the existing private roads that are centered on the north east and south east property lines of the site. These roads will provide means of vehicular ingress and egress. The site falls entirely with the Falcon Major Drainage Basin as identified by the Falcon Drainage Basin Planning Study dated September 2015.

The topography of the existing site consists of a roughly consistent grade which directs flow from the north of the site towards the south at slopes ranging from 2-5%. There is an existing storm PLD pond located to the south of the neighboring lot that ultimately then out falls to the existing detention pond WU. The site is not located in a floodway or flood plain and is designated as area of minimal flood hazard (Zone X) per FEMA FIRM panel 08041C0561G.

2.3 Existing PLD Condition Assessment

Runoff from this site will be collected via proposed curb and gutter and be routed overland to the existing Porous Landscape Detention pond. In field observations on 1/5/2021 and 06/18/2021 this pond has been determined to need maintenance as prescribed in the operation and maintenance manual that is included as appendix H of the Meridian Crossing Final Drainage Report (MCFDR). This full report is located as appendix E of this report. Specifically, we recommend the pond should be cleared of Debris and Litter and that landscaping should be removed and replaced for portions of the pond where the existing landscaping is failing. Photos showing the condition for this PLD are included in Appendix F of this report.

3.0 DRAINAGE BASINS

3.1 Reference Reports and Manuals

- Meridian Crossing Final Drainage Report (MCFDR)
- Colorado Springs Drainage Criteria (CSDC)
- El Paso County Drainage Criteria Manual (EPCSCM)
- Mile High Flood Control District Criteria Manual (MHFC)

3.2 Existing Drainage Basins

See appendix B for drainage maps showing basin locations, and appendix A for full drainage calculation sheets.

3.2.1 BASIN X

Basin X is the sole existing basin that consists of the entire 2.48 acres site. The flow path of this basin is from north to south into the existing private drive at design points 1 and 2. Once leaving the property across the existing private drive runoff enters the adjacent lot. The groundcover of this basin is primarily native grasses but also contains a portion of the private drive. After flowing across the southern property, the runoff enters the PLD (porous landscape detention) before being captured by the existing outlet structure and being routed to detention pond WU. The rational calculation for the basin is shown below in table 3.1

Table 3.1

RATIONAL CALC EXISTING									
Basin	% Impervious	C5	C100	Area	TC	I 5 year	I 100 year	Q 5	Q 100
X	12	0.28	0.42	2.48	25	2.6	4.5	1.81	4.69
TOTAL	12	0.28	0.42	2.48				1.81	4.69

3.3 Proposed Drainage Basins

See appendix B for drainage maps showing basin locations, and appendix A for full drainage calculation sheets. In general, all basins flow to the south and outlet on the existing private road. These basins then flow through the neighboring property to the south and outfall at the existing PLD pond following the historic route. The exception to this being basin A which follows the historic drainage pattern for this area of the site and flows to Old Meridian Road where it is captured by existing storm infrastructure.

3.3.1 BASIN A

Basin A consists of the proposed sidewalk connection and the landscaping along old meridian road. The runoff for this basin is captured by the curb and gutter and directed to the existing curb inlets in Old Meridian Road following the historic route.

3.3.2 BASIN B

Basin B consists of a portion of the building roof flows which are released at grade via downspouts along the rear of the structure and the south west portion of the side parking and drive aisles. These flows are then conveyed to the private road on the south of the site at design point 1. The flows then follow the historic path across the private drive into the neighboring site and ultimately the PLD pond.

3.3.3 BASIN C

Basin C consists of the parking, and drive aisles located along a portion of the front and east sides of the building. The flows are collected by curb and gutter and directed via a curb cut and concrete channel to the sidewalk chase at design point 3. The flows then follow the historic path across the private drive into the neighboring site and ultimately the PLD pond.

3.3.4 BASIN D

Basin D consists of the remainder of the roof flows which are released at grade via downspouts along the rear of the structure and the drive aisle and parking off the rear of the building. These flows are captured by curb and gutter and directed to the sidewalk chase at design point 2. The flows then follow the historic path across the private drive into the neighboring site and ultimately the PLD pond.

3.3.5 BASIN E

Basin E consists of the existing private drive on the north east edge of the site and a small portion on the landscaping adjacent to it. These flows are captured in the existing curb and gutter and are routed to the PLD through this existing curb.

Table 3.2

RATIONAL CALC PROPOSED									
Basin	% Impervious	C5	C100	Area	TC*	I 5 year	I 100 year	Q 5	Q 100
A	8	0.15	0.4	0.20	5	5.2	9	0.16	0.72
B	88	0.80	0.89	0.54	6	5	8.8	2.16	4.23
C	54	0.52	0.68	1.01	11	3.7	7	1.94	4.81
D	90	0.82	0.9	0.56	5	5.2	9	2.39	4.54
E	87	0.79	0.88	0.17	5	5.2	9	0.70	1.35
TOTAL	68			2.48				7.35	15.64

*Minimum value of 5 min used

4.0 DRAINAGE DESIGN CRITERIA

4.1 Development Criteria Reference

This report was prepared using the City of Colorado Springs Drainage Criteria (CSDC) chapter 6 and the Mile High Flood District Criteria Manual. In creating this report reference was made to the "Meridian Crossing Final Drainage Report" which is included in Appendix E and describes the existing stormwater quality treatment and detention facilities that will be used by this project.

4.2 Hydrologic Criteria

Because the site in question is under 100 acres, the rational method was used to determine the peak stormwater runoff for all basins. For the purposes of sizing proposed stormwater structures, the major 100-year storm as described in the CSDC was used. The rational method coefficients for these calculations were selected from Table 6-6 of the CSDC. Time of concentration was calculated per CSDC section 3.2 (See appendix A for calculation). All rainfall values were taken from Figure 6-6 of the CSDC.

5.0 DRAINAGE FACILITY DESIGN

The drainage facilities proposed for this project consist of curb and gutter, concrete channels, and sidewalk chases designed to collect the additional flows generated by development of the site. These facilities flow south following the historic route to the existing western PLD treatment facility. This PLD consists of a grassy swale and contains an outlet structure which outlets into a storm network under old meridian road that then discharges into a swale located to the west. This swale conveys water to the detention pond known as pond WU (page 11 of MCFDR).

The design and calculations of this existing stormwater treatment and detention facility are not within the scope of this report and can be found in the "Meridian Crossing Final Drainage Report" which is included in Appendix E.

The treatment facility is described as a "Porous Landscape Detention" (PLD) and is described in detail on page 16 of the referenced report. This facility was sized to include flows created by the future development we are now proposing. The proposed Les Schwab site can be described as the northern half of basin D-2 using the terminology of the referenced report. The assumed imperviousness for the tributary area to the PLD used in the referred report was 82% (see appendix E of MCFDR) our proposed design has a percent imperviousness of 68%. For the entire D-2 basin as described in the referenced report the assumed post development flows generated are 23.4 CFS for the minor event and 43.9 CFS for the major event (page 11). The Les Schwab site makes up roughly half of this basin (49%) and generates 7.35 CFS in the minor event and 15.64 CFS in the major event. These flows are well within the expected values for 49% of the basin which would be 11.5 CFS for minor and 21.5 CFS for major storm. No improvements are proposed for this PLD or detention pond WU which are both owned and maintained by Park Place Enterprises, LLC.

6.0 FOUR STEP PROCESS

6.1 Step 1 Employ Runoff Reduction Methods

To reduce runoff peak flows and volume our site discharges to the preexisting grass lined PLD pond that infiltrates / treats the stormwater. Our site also uses a compact design for impervious areas with a total percent imperviousness of 68% which is 14% lower than design imperviousness used by the "Meridian Crossing Final Drainage Report" which sized the facilities for this project.

6.2 Step 2 Stabilize Drainageways

The internal drainageways to the Les Schwab site are stabilized via the use of concrete curb and gutter, concrete channels, and sidewalk chases. Once water leaves the site it flows overland to the existing PLD channel and ultimately to the existing pond WU through storm sewer and a grass lined swale.

Because the design for the overall development calls for overland flow from the Les Schwab site across the adjacent property to the southeast and ultimately to the water quality treatment pond on that property, we evaluated the erosion potential due to runoff across the adjacent property. The adjacent property is characterized by fine to course sandy loam soils with sparse native prairie grasses as indicated in the photo below:



In the existing condition, the existing road separating the two lots tends to concentrate runoff from the Les Schwab site at the approaches to the adjacent property such that the existing condition for runoff from the Les Schwab site should be considered shallow/concentrated flow as it flows through the adjacent property. Signs of light erosion are evident where runoff transitions from shallow/concentrated flow into the existing pond as indicated in the photo below:



This also shows that the vegetation in this pond could use maintenance to increase the overall density and efficacy of the growth.

While the proposed development will increase peak runoff flowrates and overall runoff volume, based on TR-55, the velocity of the shallow/concentrated condition does not increase, rather the flow widens over a larger area. The average slope across the adjacent lot is 1.5%. Based on Figure 3-1 of TR-55, this correlates to a velocity of 2 ft/sec. The maximum permissible velocity, for non-colloidal (assumed conservative condition) sandy loam, is 1.75 ft/s per Table 7-3 of Chow. However, because of the vegetative condition, the direction of flow is not direct across the gradually sloped surface resulting in a slower than 2 ft/s velocity. This is evidenced by lack of erosion in the existing condition. Based on the above analysis, I do not expect a general increase in erosion on the adjacent property beyond what might occur in the existing condition.

6.3 Step 3 Provide Water Quality Capture Volume

Water quality Capture Volume is provided by the existing southern Porous Landscape Detention. The design of this facility is not within the scope of this report but in summary the pond has a design capacity of 4,568 CF and is sized to provide water quality treatment for a post development basin D-2 with an 82% imperviousness. Please see the "Meridian Crossing Final Drainage Report" located in appendix E for more information regarding this pond.

6.4 Step 4 Consider Need for Industrial and Commercial BMPs

No industrial or commercial BMPs are proposed for this site. Potential pollutants such as oil fluids etc. are stored and handled inside the building and captured by floor drains directed to a sand oil separator before being outlet to the sanitary system. The outdoor storage of the site contains only used car tires which do not present the need for spill prevention or a roofed enclosure any more than a typical parking lot. The proposed CMU walls serve solely for visual screening / theft prevention.

7.0 CONCLUSIONS

The proposed design meets the design assumptions utilized in the "Meridian Crossing Final Drainage Report" that is included in Appendix E. The "West PLD", which this site is drains to, assumed a developed condition with a percent impervious area of 82%. The proposed design has a percent impervious area of 68%. Therefore, the anticipated peak runoff and water quality volume is less than what the PLD "West PLD" was originally designed to detain and treat from this development and no additional stormwater quality treatment or detention volume is required for this project.

APPENDIX A: HYDROLOGIC AND HYDRAULIC CALCULATIONS

Channel Report

5 YEAR CONC CHANNEL

Rectangular

Bottom Width (ft) = 5.00

Total Depth (ft) = 0.50

Invert Elev (ft) = 1.00

Slope (%) = 0.50

N-Value = 0.016

Calculations

Compute by: Known Q

Known Q (cfs) = 1.94

Highlighted

Depth (ft) = 0.19

Q (cfs) = 1.940

Area (sqft) = 0.95

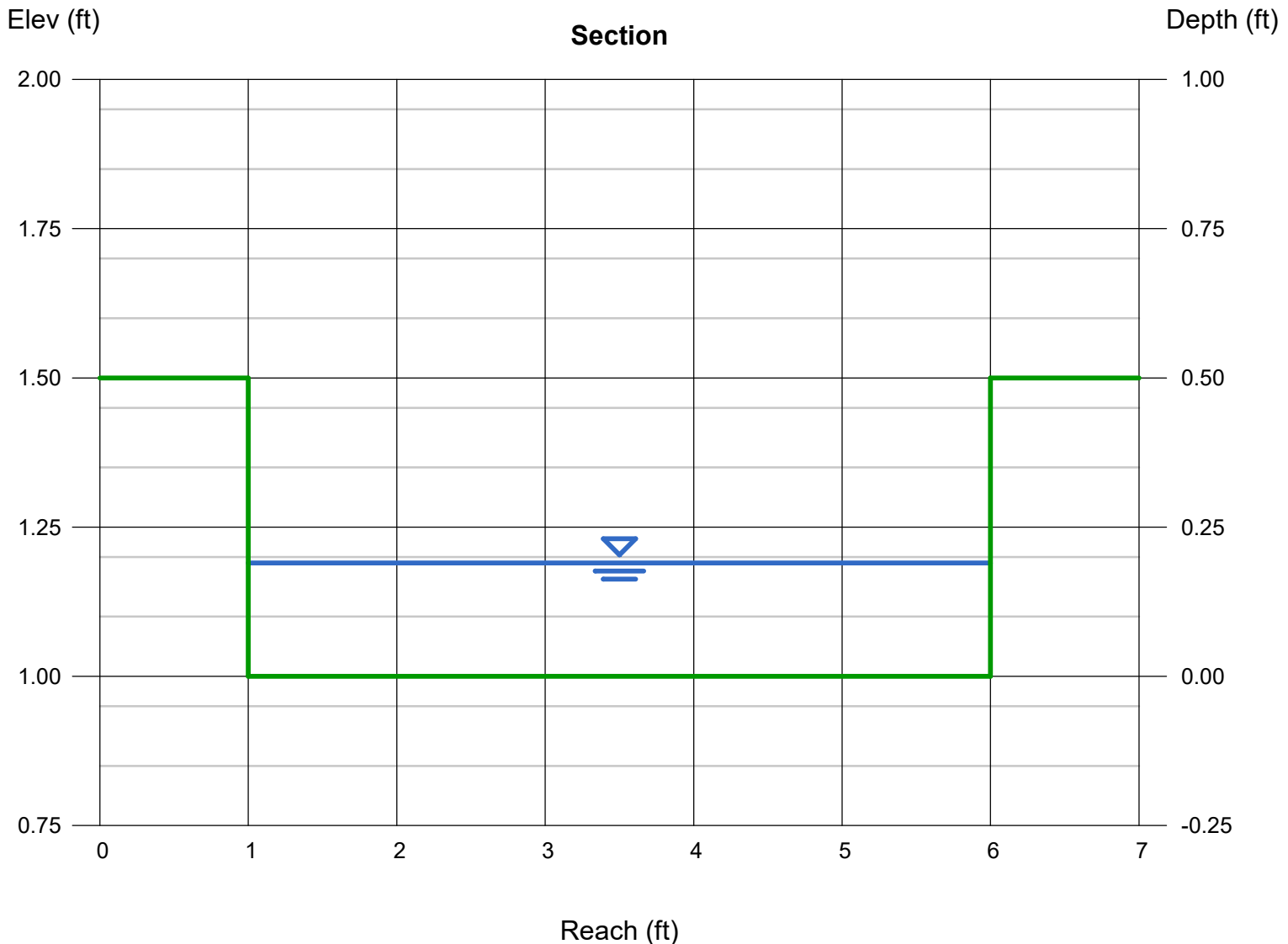
Velocity (ft/s) = 2.04

Wetted Perim (ft) = 5.38

Crit Depth, Yc (ft) = 0.17

Top Width (ft) = 5.00

EGL (ft) = 0.25



Channel Report

100 YEAR CONC CHANNEL

Rectangular

Bottom Width (ft) = 5.00
Total Depth (ft) = 0.50

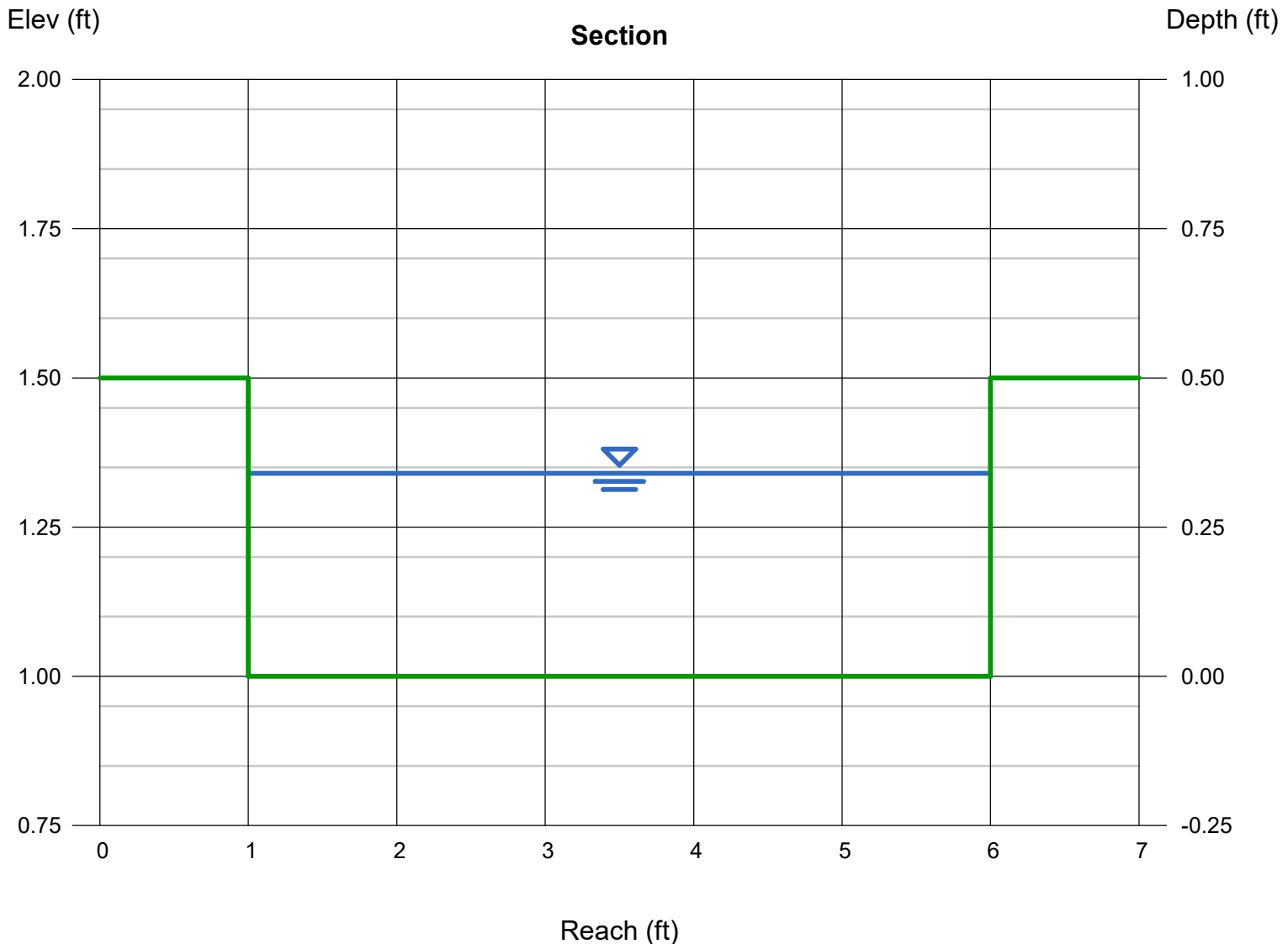
Invert Elev (ft) = 1.00
Slope (%) = 0.50
N-Value = 0.016

Calculations

Compute by: Known Q
Known Q (cfs) = 4.81

Highlighted

Depth (ft) = 0.34
Q (cfs) = 4.810
Area (sqft) = 1.70
Velocity (ft/s) = 2.83
Wetted Perim (ft) = 5.68
Crit Depth, Yc (ft) = 0.31
Top Width (ft) = 5.00
EGL (ft) = 0.46



Channel Report

CROSS PAN 5 YEAR

Trapezoidal

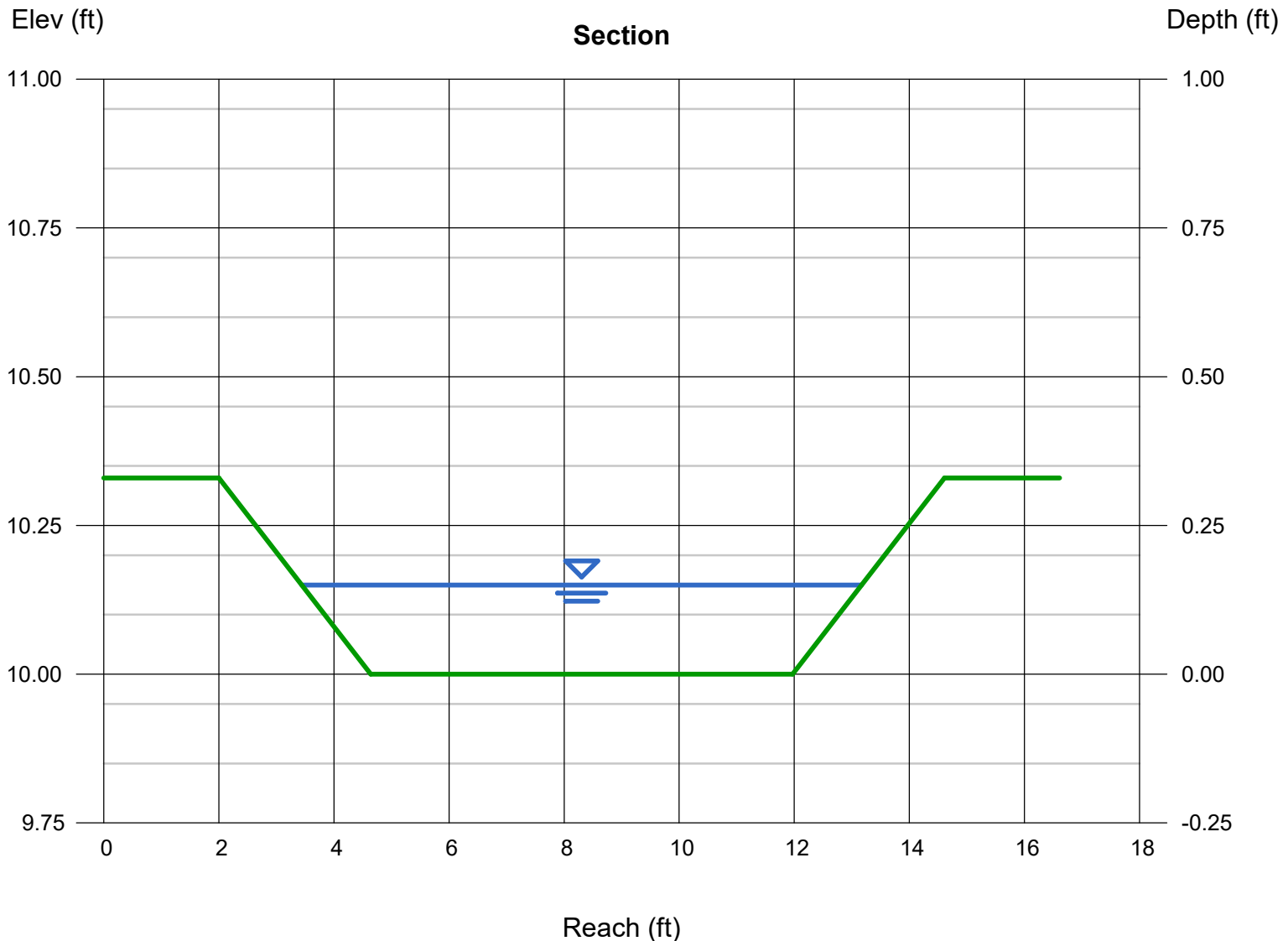
Bottom Width (ft) = 7.33
Side Slopes (z:1) = 8.00, 8.00
Total Depth (ft) = 0.33
Invert Elev (ft) = 10.00
Slope (%) = 0.50
N-Value = 0.016

Highlighted

Depth (ft) = 0.15
Q (cfs) = 1.940
Area (sqft) = 1.28
Velocity (ft/s) = 1.52
Wetted Perim (ft) = 9.75
Crit Depth, Yc (ft) = 0.13
Top Width (ft) = 9.73
EGL (ft) = 0.19

Calculations

Compute by: Known Q
Known Q (cfs) = 1.94



Channel Report

CROSS PAN 100 YEAR

Trapezoidal

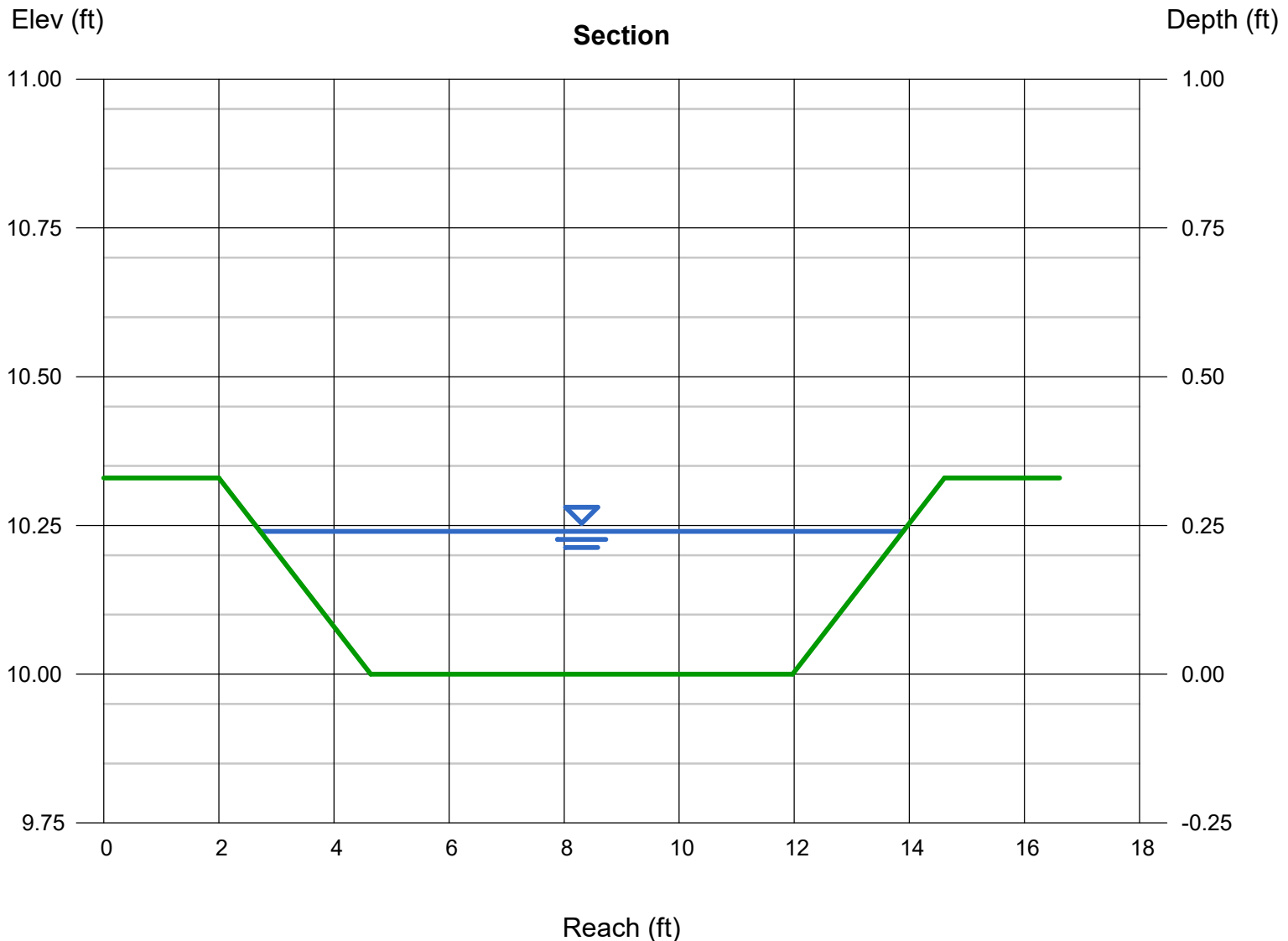
Bottom Width (ft) = 7.33
Side Slopes (z:1) = 8.00, 8.00
Total Depth (ft) = 0.33
Invert Elev (ft) = 10.00
Slope (%) = 0.50
N-Value = 0.016

Highlighted

Depth (ft) = 0.24
Q (cfs) = 4.810
Area (sqft) = 2.22
Velocity (ft/s) = 2.17
Wetted Perim (ft) = 11.20
Crit Depth, Yc (ft) = 0.22
Top Width (ft) = 11.17
EGL (ft) = 0.31

Calculations

Compute by: Known Q
Known Q (cfs) = 4.81



Channel Report

SINGLE SIDEWALK CHASE CAPACITY

Rectangular

Bottom Width (ft) = 2.00
Total Depth (ft) = 0.33

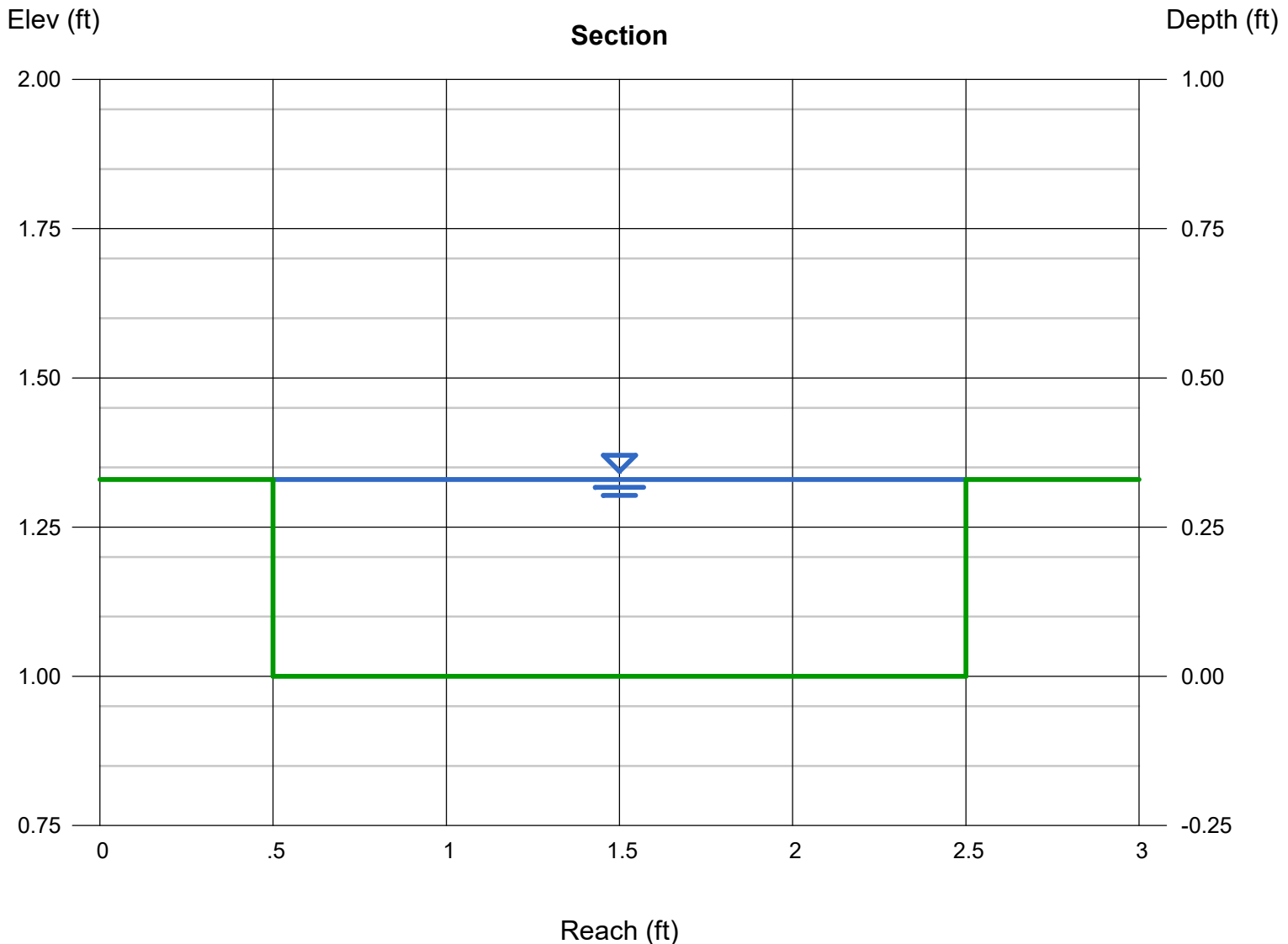
Invert Elev (ft) = 1.00
Slope (%) = 0.50
N-Value = 0.016

Calculations

Compute by: Q vs Depth
No. Increments = 1

Highlighted

Depth (ft) = 0.33
Q (cfs) = 1.711
Area (sqft) = 0.66
Velocity (ft/s) = 2.59
Wetted Perim (ft) = 2.66
Crit Depth, Yc (ft) = 0.29
Top Width (ft) = 2.00
EGL (ft) = 0.43



Overland Velocity Calc	
SLOPE	VELOCITY
0.015	1.98

Figure 3-1 (average velocities for estimating travel time for shallow concentrated flow):

Unpaved $V = 16.1345 (s)^{0.5}$

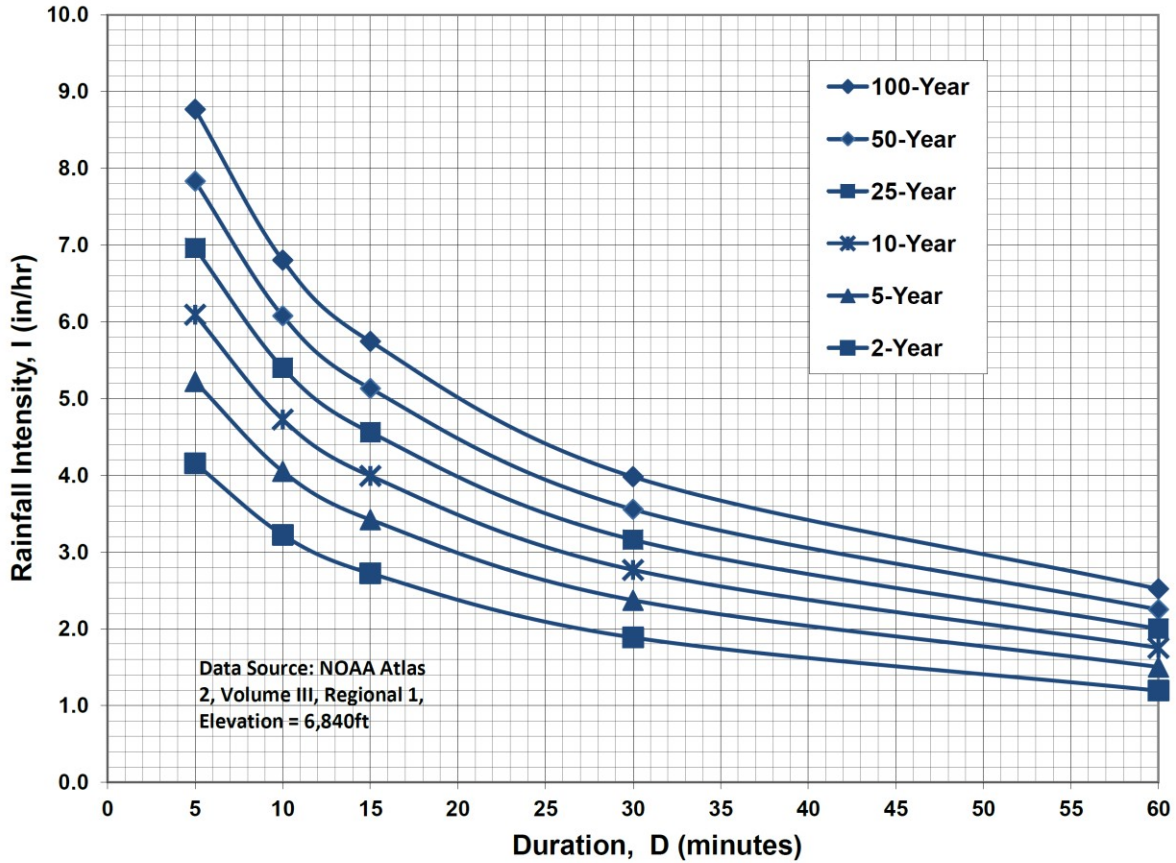
Paved $V = 20.3282 (s)^{0.5}$

where

V = average velocity (ft/s)

s = slope of hydraulic grade line
(watercourse slope, ft/ft)

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

RATIONAL CALC EXISTING									
Basin	% Impervious	C5	C100	Area	TC	I 5 year	I 100 year	Q 5	Q 100
X	12	0.28	0.42	2.48	25	2.6	4.5	1.81	4.69
TOTAL	12	0.28	0.42	2.48				1.81	4.69

RATIONAL CALC PROPOSED									
Basin	% Impervious	C5	C100	Area	TC*	I 5 year	I 100 year	Q 5	Q 100
A	8	0.15	0.4	0.20	5	5.2	9	0.16	0.72
B	88	0.80	0.89	0.54	6	5	8.8	2.16	4.23
C	54	0.52	0.68	1.01	11	3.7	7	1.94	4.81
D	90	0.82	0.9	0.56	5	5.2	9	2.39	4.54
E	87	0.79	0.88	0.17	5	5.2	9	0.70	1.35
TOTAL	68		0.77	2.48				7.35	15.64

*Minimum value of 5 min used

Proposed Time of Concentration											
BASIN	OVERLAND				Concentrated Flow						
	C5	L	S	ti	N	S	R	V	L	tc	Tc
A	0.15	33	9	4.9	0	0	0	0	0	0	5
B	0.80	51	0.5	5.0	0.016	0.005	0.5	4.15	297	1.2	6
C	0.52	51	0.5	9.7	0.016	0.005	0.5	4.15	418	1.7	11
D	0.82	161	3.5	4.4	0.016	0.005	0.5	4.15	101	0.4	5
E	0.79	30	4	2.0	0.016	0.025	0.5	9.28	100	0.2	2

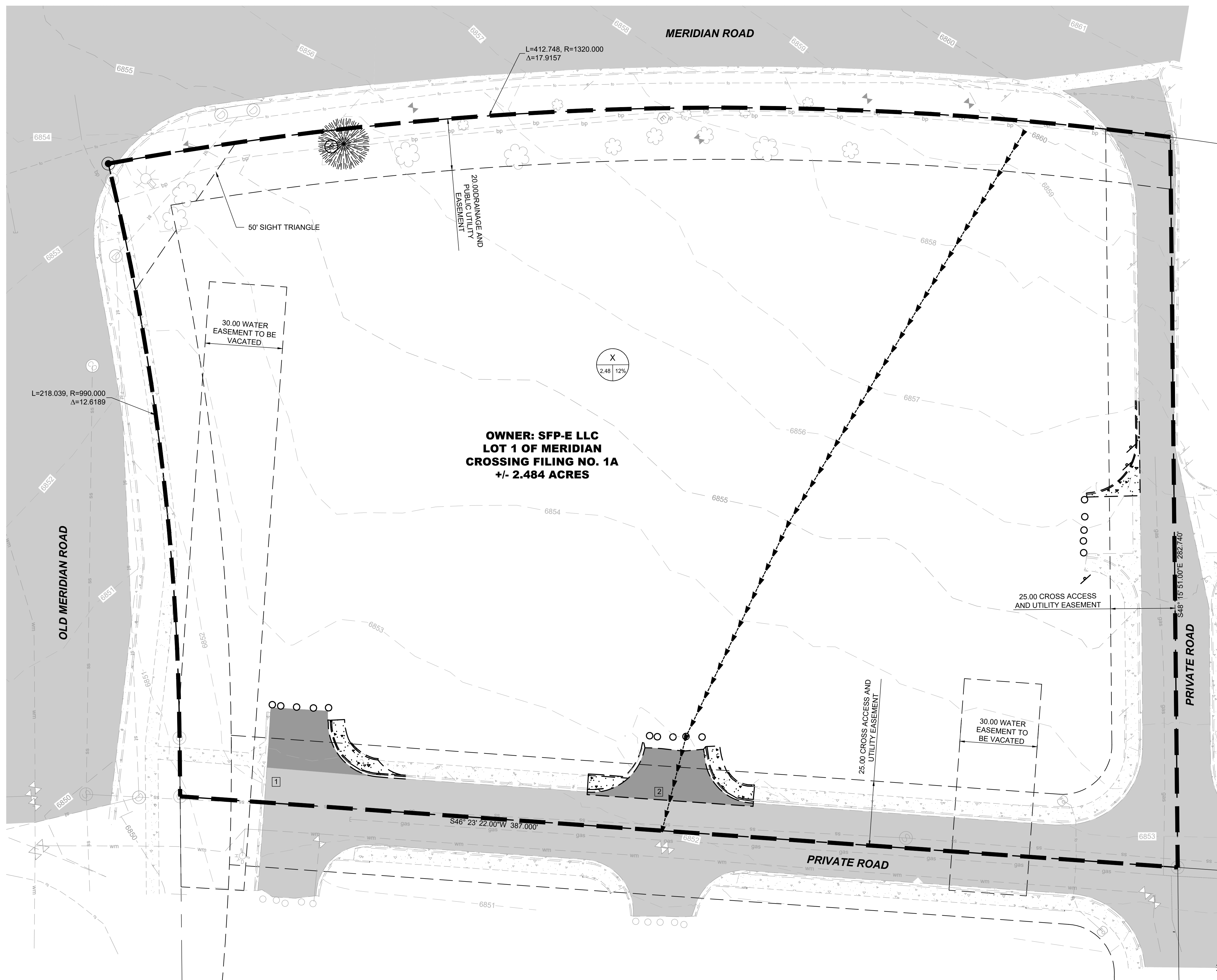
Existing Time of Concentration											
BASIN	OVERLAND				Concentrated Flow						
	C5	L	S	ti	N	S	R	V	L	tc	Tc
X	0.28	307	3	25.1	0	0	0	0	0	0	25

APPENDIX B: BASIN MAPS

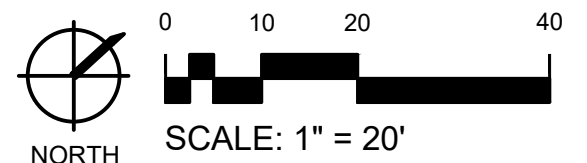


LEGEND

- TIME OF CONCENTRATION FLOW PATH
- BASIN LIMITS



1 EXISTING CONDITIONS PLAN
C001



- BASIN DESIGNATION
- % IMPERVIOUS
- BASIN AREA IN ACRES
- DESIGN POINT

NOT FOR CONSTRUCTION - PRELIMINARY DESIGN

7105 N MERIDIAN RD.
FALCON, CO

LES SCHWAB TIRE CENTER

© 2021 | ALL RIGHTS RESERVED
SITE DEVELOPMENT
PLANS

07.21.2021
DRAWN BY | WALKER
CHECKED BY | GRAHAM
REVISIONS

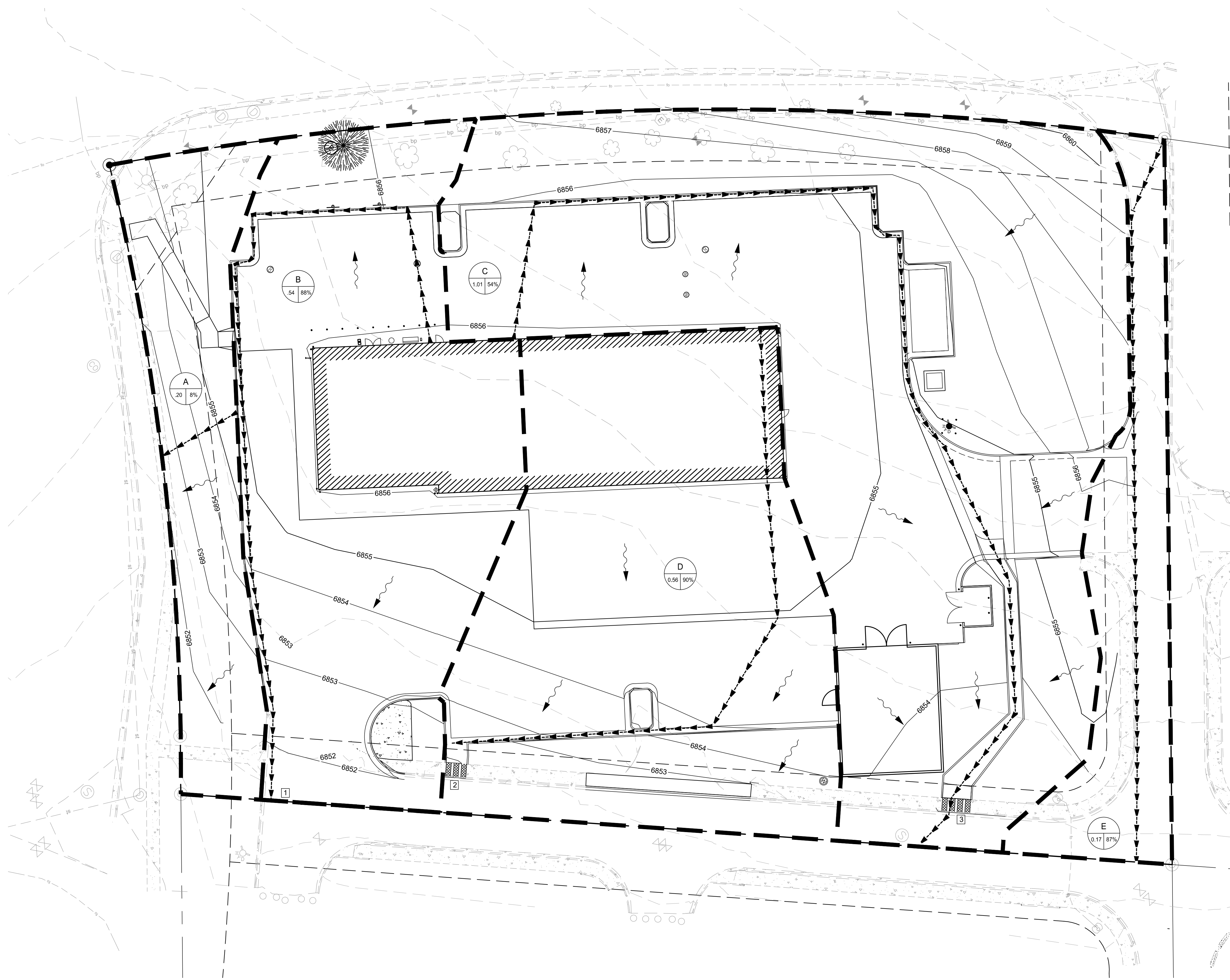
EXISTING
BASIN
MAP

D.1



LEGEND

- TIME OF CONCENTRATION FLOW PATH
- BASIN LIMITS



1 GRADING PLAN
 NORTH
 SCALE: 1" = 20'
 BASIN DESIGNATION
 % IMPERVIOUS
 BASIN AREA IN ACRES
 DESIGN POINT

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SITE DEVELOPMENT PLANS

09.01.2021
DRAWN BY | WALKER
CHECKED BY | GRAHAM
REVISIONS

EXISTING
BASIN
MAP

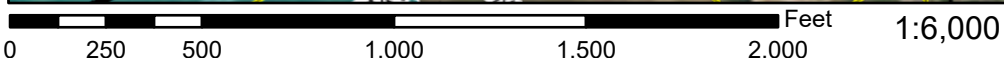
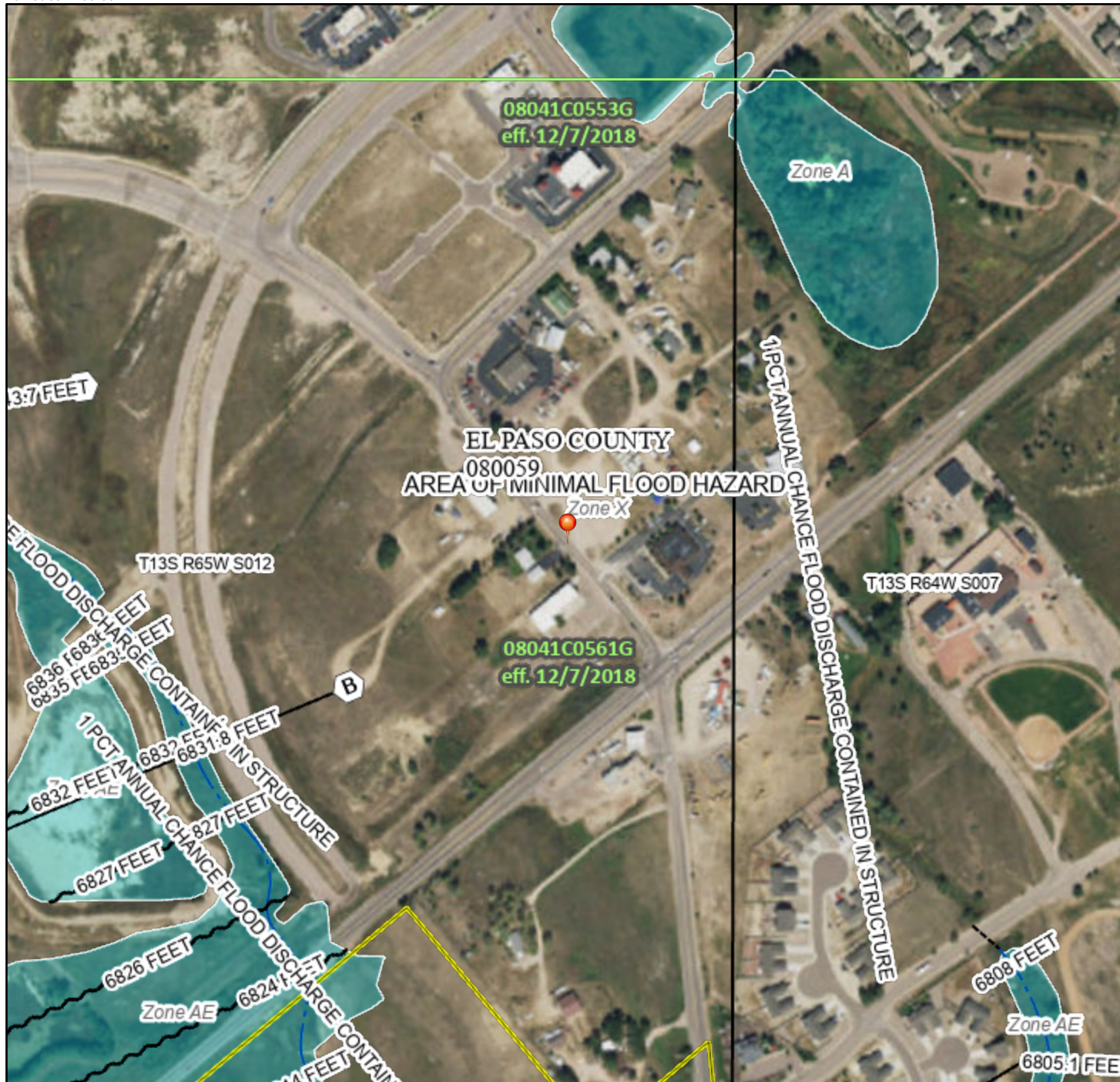
D.2

APPENDIX C: FEMA FIRMETTE

National Flood Hazard Layer FIRMMette



104°36'53"W 38°56'17"N



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS	Without Base Flood Elevation (BFE) Zone A, V, A99	With BFE or Depth Zone AE, AO, AH, VE, AR	Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD	0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X	Future Conditions 1% Annual Chance Flood Hazard Zone X	Area with Reduced Flood Risk due to Levee. See Notes. Zone X	Area with Flood Risk due to Levee Zone D

OTHER AREAS	NO SCREEN Area of Minimal Flood Hazard Zone X	Effective LOMRs	Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES	Channel, Culvert, or Storm Sewer	Levee, Dike, or Floodwall

OTHER FEATURES	20.2 Cross Sections with 1% Annual Chance Water Surface Elevation	17.5 Coastal Transect	Base Flood Elevation Line (BFE)	Limit of Study	Jurisdiction Boundary	Coastal Transect Baseline	Profile Baseline	Hydrographic Feature

MAP PANELS	Digital Data Available	No Digital Data Available	Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

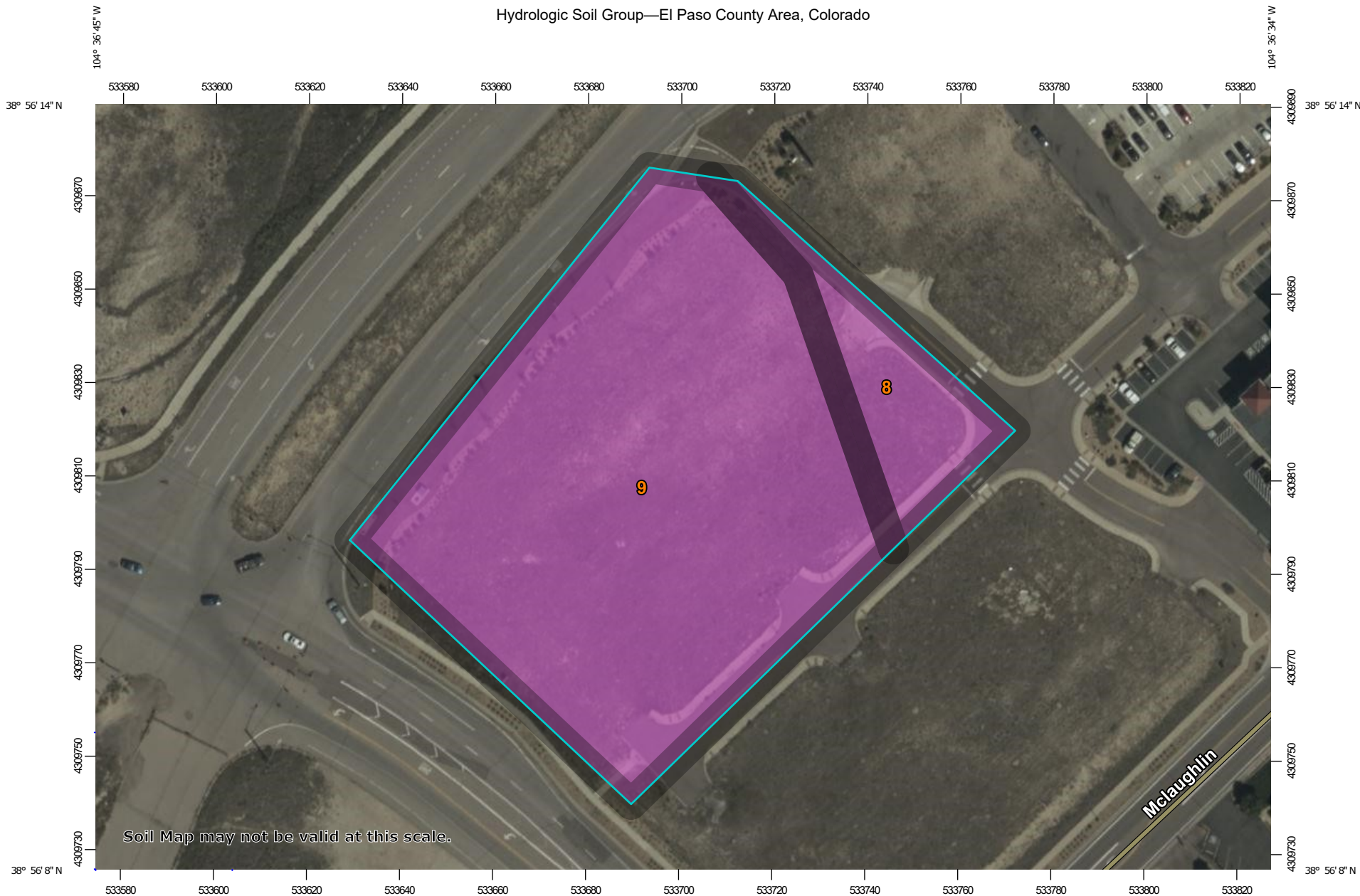
This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **4/14/2021 at 9:15 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

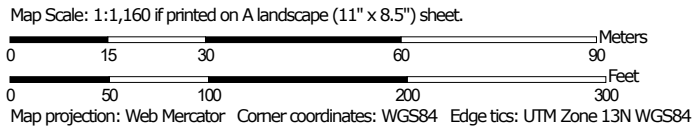
This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

APPENDIX D: WEB SOIL SURVEY

































Hydrologic Soil Group—El Paso County Area, Colorado



Soil Map may not be valid at this scale.



MAP LEGEND

- Area of Interest (AOI)**
 -  Area of Interest (AOI)
- Soils**
 - Soil Rating Polygons**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Lines**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Points**
 -  A
 -  A/D
 -  B
 -  B/D
- Water Features**
 -  Streams and Canals
- Transportation**
 -  Rails
 -  Interstate Highways
 -  US Routes
 -  Major Roads
 -  Local Roads
- Background**
 -  Aerial Photography
- Other**
 -  C
 -  C/D
 -  D
 -  Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
 Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 11, 2018—Oct 20, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	0.3	13.1%
9	Blakeland-Fluvaquentic Haplaquolls	A	2.2	86.9%
Totals for Area of Interest			2.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

APPENDIX E: MERIDIAN CROSSING STORM REPORT

MERIDIAN CROSSING
FINAL DRAINAGE REPORT
EL PASO COUNTY, COLORADO

July 2008

PREPARED FOR:

Park Place Enterprises

15 Miranda Road
Colorado Springs, CO 80906

PREPARED BY:

Springs Engineering

31 N. Tejon, Suite 315
Colorado Springs, CO 80903
719.227.7388

PROJECT NO. 057-07-032

CERTIFICATIONS

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City/County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for liability caused by negligent acts, errors or omissions on my part in preparing this report.

Charlene M. Sammons
Charlene M. Sammons, P.E. #36727



Developer's Statement:

I, the developer, have read and will comply with all the requirements specified in this drainage report.

By (signature): [Handwritten Signature]
Title: PARK PLACE ENTERPRISES, LLC
MANAGING MEMBER
Address: 15 MISADA RD.
COLORADO SPRINGS, CO 80906

El Paso County's Statement:

Filed in accordance with Section 51.1 of the El Paso Land Development Code, as amended.

[Handwritten Signature]
John McCarty, County Engineer/Director

8-19-08
Date

Conditions:

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Appendix

Appendix A: Existing HEC-1 Calculations
Appendix B: Proposed HEC-1 Calculations
Appendix C: Existing Rational Calculations
Appendix D: Proposed Rational Calculations
Appendix E: StormCAD Calculations
Appendix F: Channel and Culvert Calculations
Appendix G: Water Quality Pond Calculations
Appendix H: Operations and Maintenance Manual
Appendix I: Ultimate Design StormCAD Calculations

EXECUTIVE SUMMARY

The purpose of this Preliminary Drainage Report (PDR) and Final Drainage Report (FDR) is to present final drainage design and improvements for Meridian Crossing, located at the northeast corner of Meridian Road and Old Meridian Road, in the Falcon Highlands development. Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM). Existing facilities have been analyzed to ensure they are able to function as designed with the new facilities and construction.

This report encompasses approximately 9.5 acres of proposed commercial development in the southeast corner of the Falcon Highlands development. A proposed collector (Flower Road) will traverse the site, connecting Meridian Road to McLaughlin Road. This development will also include improvements to McLaughlin Road.

Flower Road and McLaughlin Road are to be designed as Non-Residential Collectors, per the El Paso County Criteria Manual, with a design speed of 40 miles per hour (mph) and a posted speed of 35 mps. Curb and gutter will be installed along both of these roads.

INTRODUCTION

The Meridian Crossing subdivision is a 9.5 acre commercial development located on the northwest side of the Town of Falcon. Meridian Crossing is located east and south of Falcon Highlands Market Place Filing No. 1 and adjacent to the southeast side of the "New" Meridian Road alignment. Existing development occurring in the area includes the Falcon Highlands subdivision to the west, the Beckett at Woodmen Hills development to the east and the Falcon Highlands Market Place to the west.

The area containing Meridian Crossing has been studied as part of the Falcon Area Drainage Basin Planning Study (DBPS)-Preliminary Design Report by URS, dated December 15, 2000 and Falcon Highlands Master Drainage and Development Plan (MDDP) by URS, dated October 2004.

Purpose

The purpose of the following Final Drainage Report (FDR) is to present the final design drainage improvements for the Meridian Crossing commercial development. Drainage improvements will include curb inlets, roadside ditches, and Water Quality Capture Ponds.

Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM) Volumes I and II.

Limits of Study

The Meridian Crossing FDR details the hydrology and hydraulics for the West Tributary of the Falcon Basin. Storm flow is routed by and from the proposed site and then directed through the proposed and future developments to US Highway 24. This includes an analysis of the storm systems, which includes the culverts and inlets along Meridian Road and Rolling Thunder Way. The area of study is bounded by Flower Road to the north and east, Meridian Road to the north and west, Old Meridian Road to the west and McLaughlin Road on the south

EXISTING CONDITIONS

General Location

The proposed Meridian Crossing is approximately 9.5 acres and is located at the southeast corner of Meridian Road and Old Meridian Road in Falcon, Colorado, Section 12, Township 13 South, Range 65 West of the 6th Principal Meridian. Currently, the site is zoned CR.

Falcon Highlands, Woodmen Hills, Falcon Vista, Meridian Ranch, Elkhorn Estates and Falcon Hills are all developments within a 5-mile radius of the site.

Land Use

The proposed site has just recently been rezoned to a Commercial Regional (CR) zone.

Topography and Floodplains

The topography of the surrounding area is typical of a high desert, short prairie grass with relatively flat slopes generally ranging from 2% to 4%. The area generally drains to the south. The site combines with the outlet flow of Detention Pond WU prior to crossing through the existing box culverts at Highway 24. Existing drainage swales convey these flows.

The Flood Insurance Rate Map (FIRM No. 08041C0575-F dated 3/17/99) indicates that there is a floodplain north and east of the proposed site (Falcon Basin Middle Tributary). FEMA has approved a LOMR for the Middle Tributary Floodplain (Case No. 06-08-B427P, with an effective date of November 3, 2006). This flow will now be contained within a storm drainage system and detention pond, which realigns the floodplain to the east of the site. (See Figure 3: Floodplain Map) The floodplain ties in with the FIRM after the detention pond at McLaughlin Road.

Geology

Soil Conservation Service soil survey records indicate the project area is covered by soils classified in the Blakeland Series, which are categorized in the Hydrological Group B.

The Blakeland (8) loamy sand is a deep, excessively drained soil that can exceed depths of 60 inches. Permeability of this soil is rapid with an effective rooting depth of 60 inches. This soil has good potential for urban development. The available water capacity is moderate to low. Surface runoff is slow, and the hazard of erosion is moderate.

The Blakeland (9) complex soil is comprised of approximately 60 percent Blakeland loamy sand, 30 percent Fluvaquentic Haplaquolls and 10 percent other soils. This soil is found more in sloping areas. It is deep and somewhat excessively drained. It formed in sandy alluvium and eolian material derived from arkosic sedimentary rock. Permeability of Blakeland soils is rapid, with an effective rooting depth of 60 inches. The available water capacity is moderate to low. Surface runoff is slow, and the hazard of erosion is moderate. The Fluvaquentic Haplaquolls are generally located in swale areas, and are deep, poorly drained soils. They formed in alluvium derived from arkosic sedimentary rock.

The Blakeland soil is well suited to wildlife habitat, home sites, streets and roads. This soil needs to be protected from erosion when vegetation has been removed from building sites. The Fluvaquentic Haplaquolls soil is good for wetlands. This soil has poor potential for home sites. The main limitation of this soil is the high water table and potential for flooding.

Note: (#) indicated Soil Conservation Survey soil classification number. See Figure 2: SCS Soils Map.

Climate

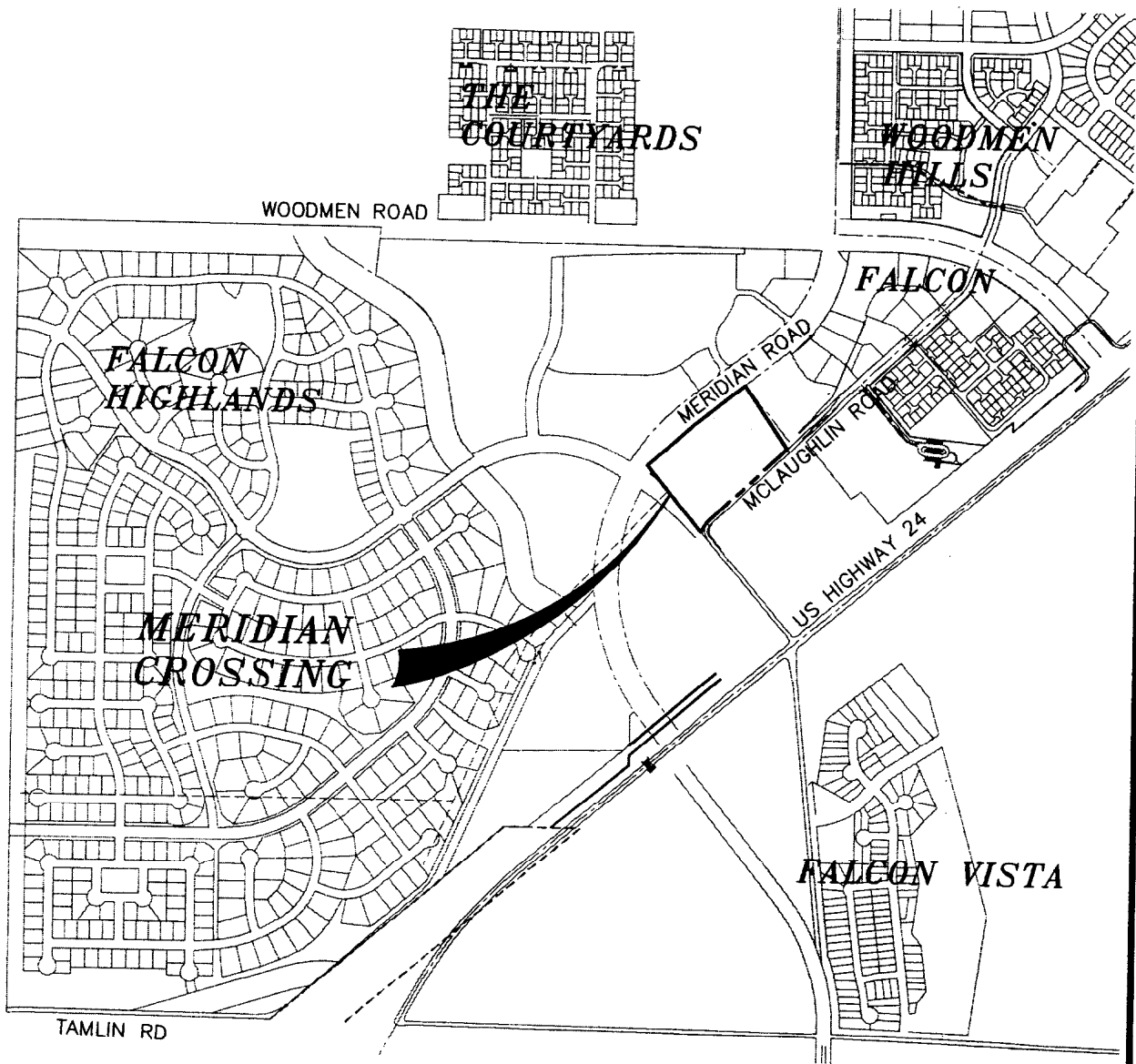
Mild summers and winters, light precipitation; high evaporation and moderately high wind velocities characterize the climate of the study area.

The average annual monthly temperature is 48.4 F with an average monthly low of 30.3 F in the winter and an average monthly high of 68.1 F in the summer. Two years in ten will have a maximum temperature higher than 98 F and a minimum temperature lower than -16 F. Precipitation averages 15.73 inches annually, with 80% of this occurring during the months of April through September. The average annual Class A pan evaporation is 45 inches.

Natural Hazards Analysis

Natural hazards analysis indicates that there is high ground water, potentially expansive claystone bedrock and wetlands located on or near the proposed site. Refer to the Geologic Hazards Evaluation Retail Site Woodmen Road and Meridian Road report by Entech Engineering. Usually, in areas where high ground water is an issue, underdrains are built to help alleviate the problem. However, since the proposed site is a commercial development, construction will be done as slab-on-grade and no basements. If lower levels are built, an underdrain system would be required. Wetland areas in the site have been identified and approved by Corps of Engineers. A mitigation plan for the site has been approved and implementation began in the fall of 2005. A copy of this plan is on file with the Falcon Highlands Metropolitan District.

Soils in this area are cohesionless, sloughing of steep banks during drilling and/or excavation could occur. By siting improvements in a manner that provides an opportunity to lay the banks of excavations back at a 1:1 slope during construction, the problems associated with sloughing soils can be minimized.



VICINITY MAP

N.T.S.

MERIDIAN CROSSING

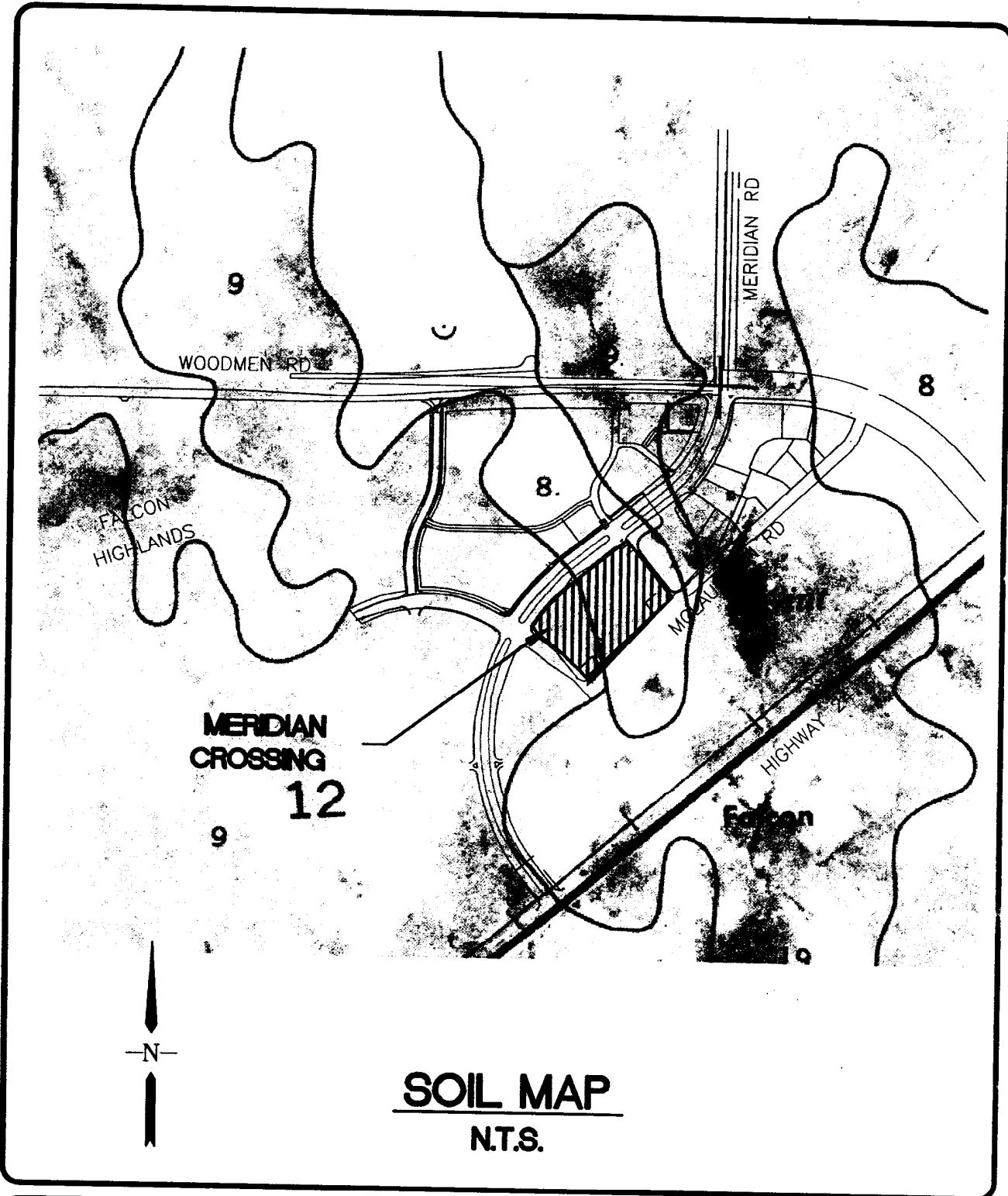
VICINITY MAP

SE Springs Engineering

31 NORTH TEJON, SUITE 315
 COLORADO SPRINGS, CO 80903
 TEL: (719) 227-7388
 FAX: (719) 227-7392

FIGURE 1

PROJECT NO. 057-07-032



MERIDIAN CROSSING

SCS Soils Map

SE *Springs*
Engineering

31 NORTH TEJON, SUITE 315
COLORADO SPRINGS, CO 80903
TEL: (719) 227-7388
FAX: (719) 227-7392

FIGURE 2

PROJECT NO. 057-07-032

DRAINAGE DESIGN CRITERIA

SCS Hydrograph Method

Hydrologic modeling was used to the West Tributary of the Falcon Basin, which routes through Meridian Crossing. Modeling was completed using The United States Army Corps of Engineers Hydrologic Engineering Center-HEC-1 version 4.1. The Soil Conservation Service (SCS) (since renamed National Resources Conservation Service (NRCS)) curve number method was selected for calculating the runoff volume from the drainage basins per the Drainage Criteria Manual (DCM). The precipitation data, basin delineation, CN runoff coefficients, and time of concentrations were taken from the Falcon Basin DBPS. Modifications have been performed on the original data, as new developments have been built and boundary lines have changed. The model has been updated to reflect the most current changes occurring in the Falcon area. The existing models in the appendix are those which were included in the FDR for Pond WU, as this report updated and modified the existing conditions of the West Tributary as originally analyzed in the Falcon Basin DBPS. Below is a summary of major design points entering the Falcon Highlands development, through the site, and where the flows exit the Falcon Highlands site. The Falcon Highlands Master Drainage Development Plan (MDDP) corrected an area in the volume of the Woodmen Hills detention pond (Pond W).

The West Tributary was analyzed in the MDDP/PDR/FDR for Falcon Highlands Filing No. 1. This report made the assumption that Pond WU would capture flows from Basins W-39, W-40, W-41 and W-42. With the construction of Meridian Road, flows from Basin W-42 do not release into the detention pond and instead combine with the outflow from the pond. With this change in routing, actual design point locations have changed slightly. The existing DBPS analysis and this report show design point WU as the flow at Highway 24 as it passes through the existing box culverts. The proposed DBPS analysis and the Market Place report have Design Point WU as the flows entering the pond with the pond release flows as the Highway 24 flows. This report made the change, as stated previously, Basin W-42 no longer enters the pond, but combines at Highway 24 and a design point was needed to evaluate flows at this location. See the table below for a summary of major design points through the West Tributary in the Falcon Highlands development. Pond WU will release at less than historic rates for both, the 5-year and 100-year events.

Design Point	Existing*		Proposed		Location
	5-Yr	100-Yr	5-Yr	100-Yr	
WS	137	1575	145	1705	Woodmen
WT	143	1621	244	1867	Tamlin Road (Removed)
Pond WU			118	1313	Highway 24/Pond Outlet
WU	148	1648	135	1339	Highway 24 (Pr Condition)
WV	149	1640	135	1338	Falcon Highway

*Existing flows have been modified per the approved Falcon Highlands Final Drainage Report for Regional Detention Pond WU. This report adjusted routing for Basin W-42 and has modified the existing flows at Design Point WU. The "existing" flows which will be the target in this report are those existing flows which have been identified in the previous mentioned report, not those which were stated in the DBPS.

Rational Method

Because the Meridian Crossing is less than 100 acres, the rational method was used to estimate stormwater runoff for basins, and to size inlets, culverts and ditches, as required by the current

City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM). The rational method coefficients "C" were selected from Table 5-1 of the DCM, the time of concentration was calculated per DCM requirements and intensities for each basin were calculated from storm intensity curve formulas provided by the City of Colorado Springs. The rational method was used to determine onsite flows. Rational Method results are shown in Appendix B and C.

Water Quality Criteria

The water quality capture volume (WQCV) was calculated based on equations found in the Drainage Criteria Manual Volume 2, Stormwater quality Policies, Procedures and Best Management Practices (BMP's). The WQCV allows suspended sediment and absorbed pollutants to settle out of the water and improve the overall quality of runoff leaving the facility and reduce the potential for erosion. The positive impact on water quality is significant see appendix for proposed pond calculations.

Street Capacity

Street capacity is based on the DCM criteria, as stated in Chapter 6. Capacity of the streets (Flower Road and McLaughlin Road) will be based on the minor and major storms. Minor storm criteria is based on pavement encroachment and the major storm criteria is based on allowable depth and pavement encroachment. In all cases, flow encroachment shall not extend past the right-of-way (R.O.W.). Mannings equation will be used to determine the street capacity based on the following criteria from the DCM. Flower and McLaughlin Roads are both collector roads. Both streets shall meet the following criteria from the Table 6-1 in the DCM:

Roadway Classification	Use of street in storm		Cross flow in streets for storm	
	Initial	Major	Initial	Major
Collector	No curb overtopping. Flow spread must be limited to a max. 20 foot spread from each curb face	Residential dwellings, public, commercial and industrial buildings shall not be inundated at the ground line. The depth of water at the gutter flow line shall not exceed 12 inches.	Where cross pans are allowed, depth of flow shall not exceed 6 inches at flow line.	12 inches of depth at gutter flow line.

DRAINAGE BASINS

Existing Drainage Analysis

Since the site is currently undeveloped, the existing drainage analysis was determined by analyzing existing runoff quantities and patterns. The site is covered predominantly with grasses. Existing storm runoff is generally from the north to the south through natural drainage swales, as well as gutter flow in a previously constructed roadway (Meridian Road). (See Figure 5: Existing Drainage Plan) On-site basins, being smaller than 100 acres, were analyzed using the rational method. See below for a brief discussion of each of these basins.

- Basin E-1 (2.18 acres) consists of the southeast half of Meridian Road, at a high point in the road adjacent to Pond MN. Basin E-1 slopes to the south to design point 1. An existing at grade inlet intercepts this flow. Any flow-by from this inlet will be directed onto Old Meridian Road. The remainder of the flow will continue east along Old Meridian Road, which allows street flow to runoff into an existing roadside ditch. This flow then enters a temporary culvert under Old Meridian Road. Basin E-1 produces runoff quantities of 9.4 cfs and 17.6 cfs for the 5-year and 100-year storms.
- Basin E-2 (8.76 acres) consists of an area just east of Old Meridian Road and between Meridian Road and McLaughlin Road. This area is currently undeveloped. Basin E-2 slopes to the southwest to design point 2. Currently, a temporary culvert exists to transfer flow from the east side of Old Meridian Road to the south side. An estimated 13.7 cfs and 31.4 cfs are produced for the 5-year and 100-year storms.
- Basin E-3 (13.96 acres) consists of an area northeast of Old Meridian and McLaughlin Roads in the "Town of Falcon". The flow from this basin will be directed towards the intersection of State Highway 24 and Meridian Road, where it is conveyed under an existing culvert under Old Meridian Road. This flow is directed towards the existing structure at Design Point 6. The basin generates 16.4 cfs and 37.5 cfs for the 5-year and 100-year storms.
- Basin E-4 (2.15 acres) consists of the east half of Meridian Road from Old Meridian Road to the right-in access point to the south. Basin E-4 slopes to the south. Flow from this basin will be conveyed through curb and gutter to the right in access drive, where a sump inlet intercepts the flow. An 18-inch rcp will then release into a temporary channel along Meridian Road, which conveys the flow to design point 6. An estimated 9.3 cfs and 17.5 cfs are produced for the 5-year and 100-year storms.
- Basin E-5 (9.41 acres) consists of an undeveloped native area just east and south of the Falcon Highlands detention pond (Pond WU). Runoff from this basin combines with flows from design point 7 and the detention pond outlet and crosses under US Highway 24 through existing culverts at design point 8. Basin E-5 generates 12.1 cfs and 27.8 cfs for the 5-year and 100-year storms.
- Basin E-6 (2.62 acres) consists of the west half of Meridian Road from the right-in access point to Highway 24. Basin E-6 slopes to the south to design point 4. Runoff will flow south via curb and gutter along this section of Meridian Road. A sump inlet at

the low point, just before Highway 24, intercepts the runoff. This inlet connects to the box culvert under Meridian Road. This flow will continue to the existing box culverts at Highway 24. The Falcon Highlands detention pond also discharges to the existing culvert at Highway 24. However, the detention pond was designed to release flows at 80% of historic flow. The existing culvert under Highway 24 will have enough capacity, because the peak discharge from design point 7 will occur long before the peak discharge of the pond. An estimated 11.6 cfs and 21.7 cfs are produced for the 5-year and 100-year storms.

- Basin E-7 (2.32 acres) consists of the east half of Meridian Road from the right-in access point south to Highway 24. Basin E-7 slopes to the south to design point 15. Runoff will flow south through curb and gutter along this section of Meridian Road. A sump inlet intercepts this flow and connects to the box culvert under Meridian Road. An estimated 10.5 cfs and 19.7 cfs are produced from Basin E-7 for the 5-year and 100-year storms.
- Basin E-8 (23.89 acres) consists of the area south of Old Meridian Road and east of Meridian Road. This area is currently undeveloped. This flow sheetflows across the basin to design point 6. A concrete box culvert conveys the flow under Meridian Road to the existing culvert under Highway 24. An estimated 32.2 cfs and 73.7 cfs are produced for the 5-year and 100-year storms.

Existing Design Points

- Design Point 1 is an existing 15' on-grade inlet in Meridian Road, north of Old Meridian Road. This inlet intercepts flow from Basin E-1, street flow from Meridian Road, south of Pond MN to Old Meridian Road. The inlet releases the flow into a temporary ditch along Old Meridian Road, which conveys the flow to Design Point 2. Flows at this location are 9.4 and 17.6 cfs.
- Design Point 2 collects flow from Basin E-2 and combines it with the flow in the temporary channel from DP-1. A temporary 24-inch culvert under Old Meridian Road conveys the flow to the south towards Design Point 6. This design point has flows of 22.9 and 48.7 cfs.
- Design Point 3 is an existing 20' sump inlet that intercepts the street flow in Meridian Road from Basin D-4. The inlet releases flows into an existing temporary roadside ditch along Meridian Road to Design Point 6. Flows at this location are 9.3 and 17.5 cfs.
- Design Point 4 is an existing 25' sump inlet used to intercept the west side of Meridian Road, north of Highway 24 (Basin E-6). This flow enters the existing storm system and is conveyed to Design Point 7. This design point has flows of 11.6 and 21.7 cfs.
- Design Point 5 is an existing 20' sump inlet in Meridian Road opposite Design Point 4. This inlet intercepts street flow from Basin E-7 and combines with flows in the storm system under Meridian Road. Flows are released at Design Point 7. The design point generates flows of 10.5 and 19.7 cfs.

- Design Point 6 combines flows from Basins E-3 and E-8 with flows from Design Points 2 and 3. Two 12' (W) x 3' (H) reinforced concrete box culverts (RCBC's) convey the flow under Meridian Road to Design Point 7. Flows generated at this design point are 67.8 and 149.7 cfs.
- Design Point 7 is the location where the storm system releases flows. It is the combined flow from Design Points 4, 5 and 6. Once released, flows will continue through an existing ditch to Design Point 8 at Highway 24. Flows at this location are 67.0 and 148.5 cfs.
- Design Point 8 combines the flow from Basin E-5 with flows from Design Point 7 and Detention Pond WU. There are three 12' (W) x 6' (H) RCBC's under Highway 24 to convey flows. These flows will continue towards the south, in a FEMA floodplain, along their existing paths. Flows generated at this location are 147.7 and 1286.1 cfs. This design point corresponds to Design Point WU in the HEC models.

Developed Drainage Analysis

The proposed site was studied in the Falcon Basin DBPS. Efforts have been made to comply with the recommendations set forth in the approved DBPS. The flows in the commercial development will combine with the outlet flows of Detention Pond WU and continue under Highway 24 through the existing box culverts. Figure 6: Developed Drainage Plan illustrates the basin boundaries used for the rational hydrologic model.

Basins D-1 and D-4 through D-7 do not have any changes from the corresponding, existing basins (E-1 and E-4 through E-7), as they have already been developed and the drainage structures have been designed. Basin E-2, a proposed commercial site (Meridian Crossing, whose preliminary plan has been approved by the Board of County Commissioners (BOCC)), the proposed site and adjacent roadways, has been divided into five new developed basins. Changes to Basins D-3 and D-8, is the assumption that these basins will be developed in the future as commercial use. The description of these basins follows.

- Basin D-2 (5.14 acres) consists of approximately the south half of the Meridian Crossing commercial development. It is anticipated for this basin to drain towards the south, where it will be intercepted by a proposed water quality facility (Porous Landscape Detention – PLD). This storm system will release flows into a temporary drainage swale through Basin D-8. This drainage pattern is consistent with the approved Master Drainage Development Plan Amendment to Falcon Highlands, which shows these flows reaching the existing box culvert under Highway 24 at design point 8. This basin generates 23.4 cfs and 43.9 cfs for the 5-year and 100-year storm events.
- Basin D-3 (11.49 acres) consists of an area northeast of Old Meridian Road and McLaughlin Road in the "Town of Falcon". It is assumed that this area will be developed as commercial use in the future. The flow from this basin will be directed towards the intersection of Highway 24 and Meridian Road, where it is conveyed under a proposed culvert under Old Meridian Road. The flow is directed towards the existing structure at DP-6. The basin generates 34.7 and 65.1 cfs for the 5 and 100-year events.

intercepted by the temporary grated lid on the proposed manhole at DP-Z This flow will ultimately reach Highway 24. Flows at this location are 4.7 and 8.9 cfs.

- Design Point X is the released flow from the East PLD. Flow from this area combines with a storm system in Old Meridian Road via a 30" rcp. Flows generated in this location are 16.9 cfs and 31.8 cfs.
- Design Point Z is the flow from DP-B, DP-X intercepted flow from inlet 1 and the flow-by from inlet 1 combined with the released flow from the West PLD (Basin D-2). A 36" rcp connects and conveys this flow to a temporary culvert under Old Meridian Road which will release flows into a temporary ditch. Flows at this junction are 44.6 cfs and 83.8 cfs.
- Design Point Y is a proposed 54" rcp which releases flows across Old Meridian Road. The proposed pipe will replace an existing culvert, which is currently undersized, and intercepts flows from D-3. This design point has a flow of 34.7 cfs and 65.1 cfs. There is no corresponding design point in the Market Place Filing No. 1 drainage report.
- Design Point 3 is an existing 20' sump inlet that intercepts the street flow in Meridian Road from Basin D-4. The inlet currently combines with DP-Z and flows into an existing temporary roadside ditch along Meridian Road to Design Point 6. Flows at this location are 9.3 and 17.5 cfs. This design point corresponds to Design Point 13 in the Market Place FDR.
- Design Point E is the combination of D-3 and DP-Z. Flows are released into a temporary channel which conveys flows to a roadside ditch along Meridian Road. This flow is conveyed all the way to Design Point DP-8 at Highway 24 where flows are released from the Falcon Highlands site. Flows intercepted at this location are 46.938.3 cfs and 58.2 cfs. The Market Place Filing No. 1 report does not have any corresponding design points.
- Design Point 4 is an existing 25' sump inlet used to intercept the west side of Meridian Road, north of Highway 24 (Basin D-6). This flow enters the existing storm system and is conveyed to Design Point 7. This design point has flows of 11.6 and 21.7 cfs. This design point corresponds to Design Point 16 in the Market Place FDR. There is no difference in the drainage flows.
- Design Point 5 is an existing 20' sump inlet in Meridian Road opposite Design Point 4. This inlet intercepts street flow from Basin D-7 and combines with flows in the storm system under Meridian Road. Flows are released at Design Point 7. The design point generates flows of 10.5 and 19.7 cfs. This location corresponds to Design Point 15 in the Market Place FDR. There are no changes in the flows.
- Design Point 6 combines flow from Basin D-8 with DP-E and DP-Y. Two 12' (W) x 3' (H) reinforced concrete box culverts (RCBC's) convey the flow under Meridian Road to Design Point 7. Flows generated at this design point are 127.7 and 239.9 cfs. This location corresponds to Design Point 17 in the Market Place FDR. This report calculated flows to be 157.9 and 300.6 cfs.

DRAINAGE FACILITY DESIGN

General Concept

Meridian Crossing is located completely within the West Tributary of the Falcon Drainage Basin. The site drains towards the southwest where it is directed towards an existing drainage structure under Highway 24. This structure has been analyzed to ensure it still properly functions with the developed flow released here. The flow from these structures will continue along an existing swale to the south. There are two water quality facilities proposed for the site, prior to flows exiting.

Storm Systems

There has been one storm system previously installed with the construction of Meridian Road. This system was designed in the Market Place Filing No. 1 FDR. One new culvert and a temporary culvert have been proposed with the development of this site. StormCAD and CulvertMaster calculations have been included at the end of the report analyzing all of these facilities to ensure they are still adequate for the developed flow associated with this development.

The first system is an existing system located at the intersection of Meridian Road and Highway 24. This system was initially designed in the Market Place Filing No. 1 FDR. The system has been analyzed to ensure it will still function properly with the development of The Shoppes at Falcon. A 20-foot sump inlet is located at DP-5 in Meridian Road. This inlet intercepts 10 cfs and 19 cfs. A 25-foot sump inlet is located on the other side of Meridian Road at DP-4. This inlet intercepts 12 and 22 cfs. Both of these inlets connect to an existing 12'(W) x 3' (H) box culvert under Meridian Road via 24-inch rcp's. This structure intercepts a total flow of 127.7 and 239.9 cfs. This system releases flows into an existing channel parallel to Highway 24 at DP-7. Flows at this location are 140.2 cfs and 264.8 cfs. The channel conveys this flow to DP-8 at Highway 24, where the flow exits Falcon Highlands and continues on its existing path to the south.

Channel Improvements

The temporary channel from DP-Z has a 100-year flow of 73.3 cfs from the proposed storm system. The channel will be shaped similarly to the roadside ditch along the southeast side of Meridian Road, which it connects to. Velocity in this channel is 3.2 ft/s with a flow depth of 1.8 feet. A temporary drainage easement will be recorded on the final plat to accommodate this channel.

The existing roadside ditch along Basin D-7 is located east of Meridian Road from the right-in access point south to Highway 24. The ditch will carry the 100-year storm (88.2 cfs) at a depth of 1.8 feet to DP-6. The velocity in this channel is 5.5 fps. This channel will also be removed upon development of Basin D-8 and the construction of an internal storm drain system.

There is a series of onsite temporary swales in lots 3, 4 and 5. These swales will be utilized to ensure flows are conveyed to the west PLD. Once these lots develop, the swales will no longer be necessary and will be removed. Also, located outside of the Meridian Crossing right-of-way

along Old Meridian Road are 3 temporary swales, which convey flows to storm inlets. These swales will be removed upon the construction of Old Meridian Road.

Detention Pond WU

Based on the current configuration of the basins, Pond WU does work as intended. The 5-year release rate is less than the existing flow rate. Refer to the table earlier in the report for flow rates at major design points. Based on the current analysis of the hydrology for the area no modifications will be necessary to the outlet structure of the detention pond as previously assumed. The 100-year storm also functions properly and has a release rate lower than the existing flows.

Proposed Water Quality Pond

Based on the City of Colorado Springs/El Paso County DCM Volume 2, a water quality pond is needed, as the development area is greater than 1.0 acre. There will be two water quality capture ponds (WQCP), which will be porous landscape detention (PLD). Both ponds structures will be located between the northern right of way of McLaughlin road and the proposed curb and gutter for lots 5 and 6. This will enable the ponds to be used for final construction of each of the building sites. The east and west pond combine for a total 8200 square feet.

Ultimate Design

Currently, there is evidence that Old Meridian Road will be improved. If this situation does not happen, there is an "ultimate design" scenario to account for this. Meridian Crossing will be responsible for installing curb and gutter and sidewalk for the portion of Old Meridian Road which fronts their property. The rcp stub behind inlet DP-1 will be extended via 18"rcp to the existing culvert under Old Meridian Road. A 5' type R inlet will be installed to catch the flows of Old Meridian Road (see appendix item I for StormCAD calculations). A 5' manhole will be installed to connect the new and existing pipes. The existing culvert will continue to convey the flow through an existing swale, which releases flows into the roadside swale along Meridian Road. This flow still reaches an ultimate location of DP-8, where all flows leave the Falcon Highlands development.

Downstream Facilities

Falcon West Tributary

Detention Pond WU discharges below the historical rate as described in the Falcon Highlands Filing No. 1 PDR and the Falcon Basin DBPS. Just downstream of Pond WU outlet works is an existing bridge at SH 24. At Highway 24 near Pond WU, triple 12' x 6' RCBC's were installed in 1999. This facility conveys the 1239 cfs 100-year design flow. An analysis of these structures is included in the appendix. The DBPS recommended installing a lined channel with geotextile fabric and grade control drop structures. Currently, this area has no real definable channels, but flows are allowed to spread once they are released through the structure at SH 24. This area is within a FEMA designated floodplain. Historic flows as stated in the DBPS are 1518 cfs.

Downstream of SH 24, flows follow a FEMA floodplain to Falcon Highway. At Falcon Highway there is a 36-inch cmp culvert that is inadequate to carry the 100-year design flow.

DRAINAGE FEES, COST ESTIMATE & MAINTENANCE

Maintenance

The streets and major improvements within this site will be maintained by the Meridian Crossing Property Owners Association (POA) for ownership and maintenance. This includes the roads, drainage facilities, and water quality ponds. The Falcon Highlands Metropolitan District will own and operate water and wastewater systems. The remaining utilities (gas, phone, electric, cable, etc) will be owned and maintained by their respective companies. Easements will be issued to ensure each entity is able to access and maintain their facilities.

Drainage Fees

The proposed development is located within the Falcon Basin. The proposed commercial site encompasses approximately 9.5 acres. Fees will be based on 9.0 acres (95% imperviousness due to commercial development).

Drainage fees in the Falcon Basin are \$6,925 and bridge fees are \$2,659. Based on these numbers and an impervious area of 9.0 acres fees for this development are \$62,325 for drainage and \$25,261 for bridge fees. This gives a total fee of \$87,586.

Proposed Facilities Estimate

ITEM	UNITS	UNIT COST	QUANTITY	ITEM COST
DRAINAGE				
5' STORM MANHOLE	EA	2,800	1	2,800
30" RCP	LF	55	435	23,925
36" RCP	LF	65	60	3,900
42" RCP	LF	80	105	8,400
RIPRAP	CY	45	30	1,350
SUBTOTAL DRAINAGE				\$40,375
GRADING AND EROSION CONTROL				
CLEARING AND GRUBBING	AC	\$800	9.5	\$7,600
EARTHWORK	CY	3.50	13300	46,550
WATER QUALITY PONDS	EA	3,000	2	6,000
CURB BACKFILL	LF	2.50	3200	8,000
MISC SEEDING AND MULCH	AC	3,500	6	21,000
HAY BALE CHECKS	EA	10	37	370
INLET PROTECTORS	EA	200	4	800
VEHICLE TRACKING CONTROL	EA	1,500	1	1,500
SILT FENCING	LF	5	1800	9,000
SUBTOTAL GRADING & EROSION CONTROL				\$100,770
ULTIMATE DRAINAGE				
5' STORM MANHOLE	EA	2,800	1	2,800
30" RCP	LF	40	259	10,360
5' TYPE R INLET	EA	3,500	1	3,500
SUBTOTAL ULTIMATE DRAINAGE				\$16,660
SUBTOTAL DRAINAGE & EROSION CONTROL				\$157,805
Engineering (10%)				\$15,781

Contingency (10%)				\$15,781
TOTAL				\$189,367

EROSION CONTROL

General Concept

During construction, best management practices for erosion control will be employed based on El Paso County criteria and the erosion control plan.

Ditches will be designed to meet El Paso County criteria for slope and velocity, keeping velocities below scouring levels.

During construction, best management practices (BMP) for erosion control will be employed based on El Paso County Criteria. BMP's will be utilized as deemed necessary by the contractor and/or engineer and are not limited to measures shown on the construction drawing set. The contractor shall minimize the amount of area disturbed during all construction activities.

In general the following shall be applied in developing the sequence of major activities:

- Install downslope and sideslope perimeter BMP's before land disturbing activity occurs.
- Do not disturb an area until it is necessary for construction activity to proceed.
- Cover or stabilize as soon as possible.
- Time the construction activities to reduce the impacts from seasonal climatic changes or weather events.
- The construction of filtration BMP's should wait until the end of the construction project when upstream drainage areas have been stabilized.
- Do not remove temporary perimeter controls until after all upstream areas are stabilized.

Silt Fence

Silt fence will be placed along downstream limits of disturbed areas. This will prevent suspended sediment from leaving the site during infrastructure construction. Silt fencing is to remain in place until vegetation is reestablished.

Erosion Bales

Erosion bales will be placed ten (10) feet from the inlet of all culverts and inlets during construction to prevent culverts from filling with sediment. Erosion bales will remain in place until vegetation is reestablished in graded roadside ditches and channels. Erosion bale ditch checks will be used on slopes greater than 1% to reduce flow velocities until vegetation is reestablished.

Vehicle Tracking Control

This BMP is used to stabilize construction entrances, roads, parking areas and staging areas to prevent the tracking of sediment from the construction site. A vehicle tracking control (VIC) is to be used at all locations where vehicles exit the construction site onto public roads, loading and unloading areas, storage and staging areas, where construction trailers are to be located, any construction area that receives high vehicular traffic, construction roads and parking areas. VTC's should not be installed in areas where soils erode easily or are wet.

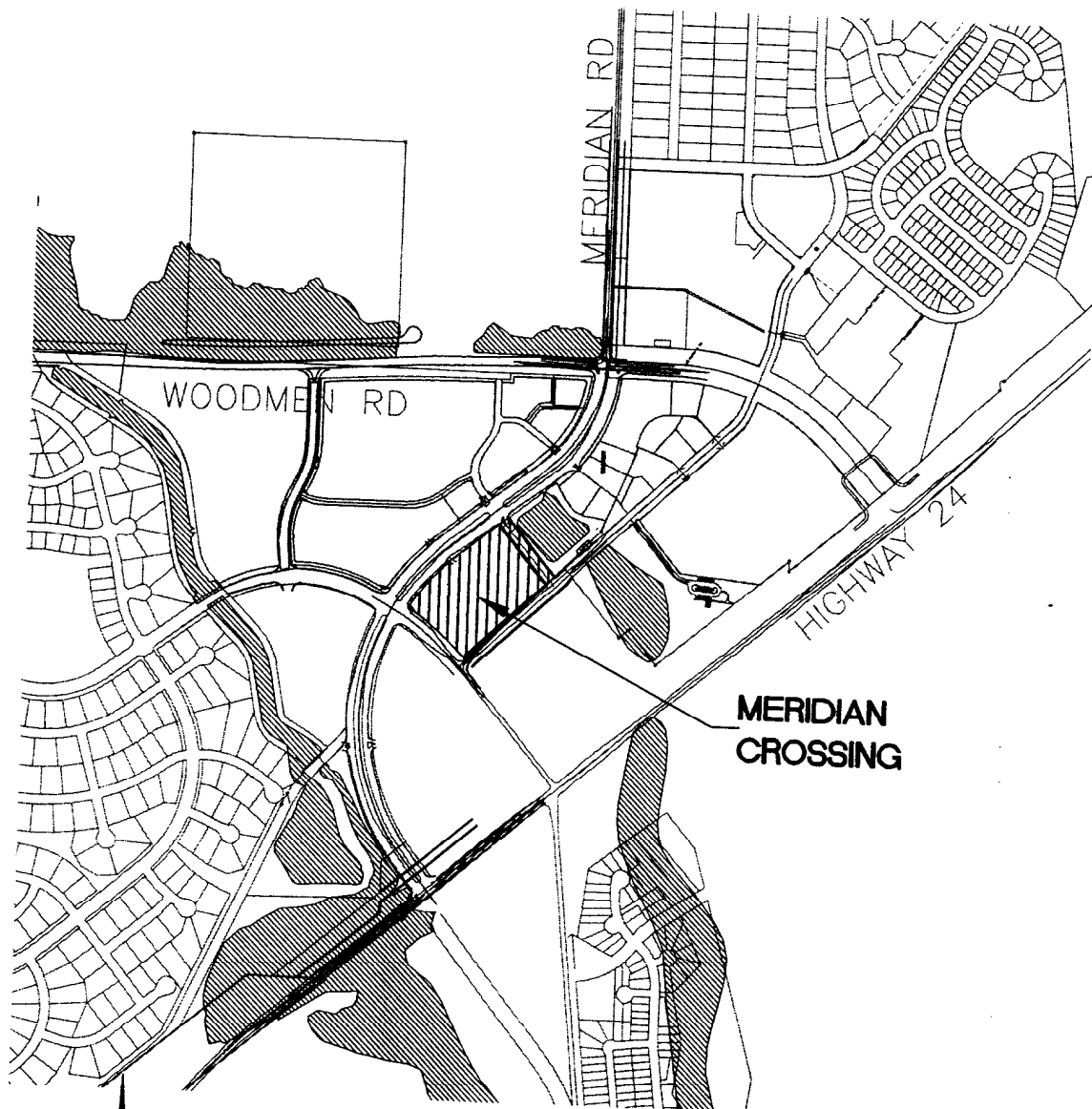
Sedimentation Pond

This BMP is used to detain runoff which has become laden with sediment long enough to allow the sediment to settle out. As the construction area is larger than 1 acre, a temporary sediment basin is required per Volume 2 of the Drainage Criteria Manual. The basin will be located in the area of the proposed water quality pond, as this area will need to be excavated and an embankment built. A temporary 8" pvc underdrain will be installed to drain this basin during construction.

REFERENCE MATERIALS

1. "City of Colorado Springs/El Paso County Drainage Criteria Manual" September 1987, Revised November 1991, Revised October 1994.
2. "City of Colorado Springs/El Paso County Drainage Criteria Manual, Volume 2: Stormwater Quality Policies, Procedures and Best Management Practices" November 1, 2002.
3. Soils Survey of El Paso County Area, Natural Resources Conservation Services of Colorado.
4. Flood Insurance Rate Study for El Paso County, Colorado and Incorporated Areas. Federal Emergency Management Agency, Revised March 17, 1997.
5. Falcon Area Drainage Basin Planning Study Preliminary Design Report, December 2000. Prepared by URS Corp.
6. Master Development Drainage Plan, and Preliminary Drainage Report and Final Drainage Report for Falcon Highland Filing No. 1, October 2004. Prepared by URS Corp.
7. Floodplain Modification Study and Application for Conditional Letter of Map Revision for the Middle Tributary of the Falcon Basin-Regency Center, January 2005. Prepared by URS Corp.
8. Amendment to Falcon Highlands Master Drainage Development Plan, September 2005. Prepared by URS Corp.
9. Falcon Highlands Market Place Filing No. 1 Preliminary and Final Drainage Report, December 22, 2005. Prepared by URS Corp.

Figure 3: FEMA Floodplain Map



MERIDIAN CROSSING

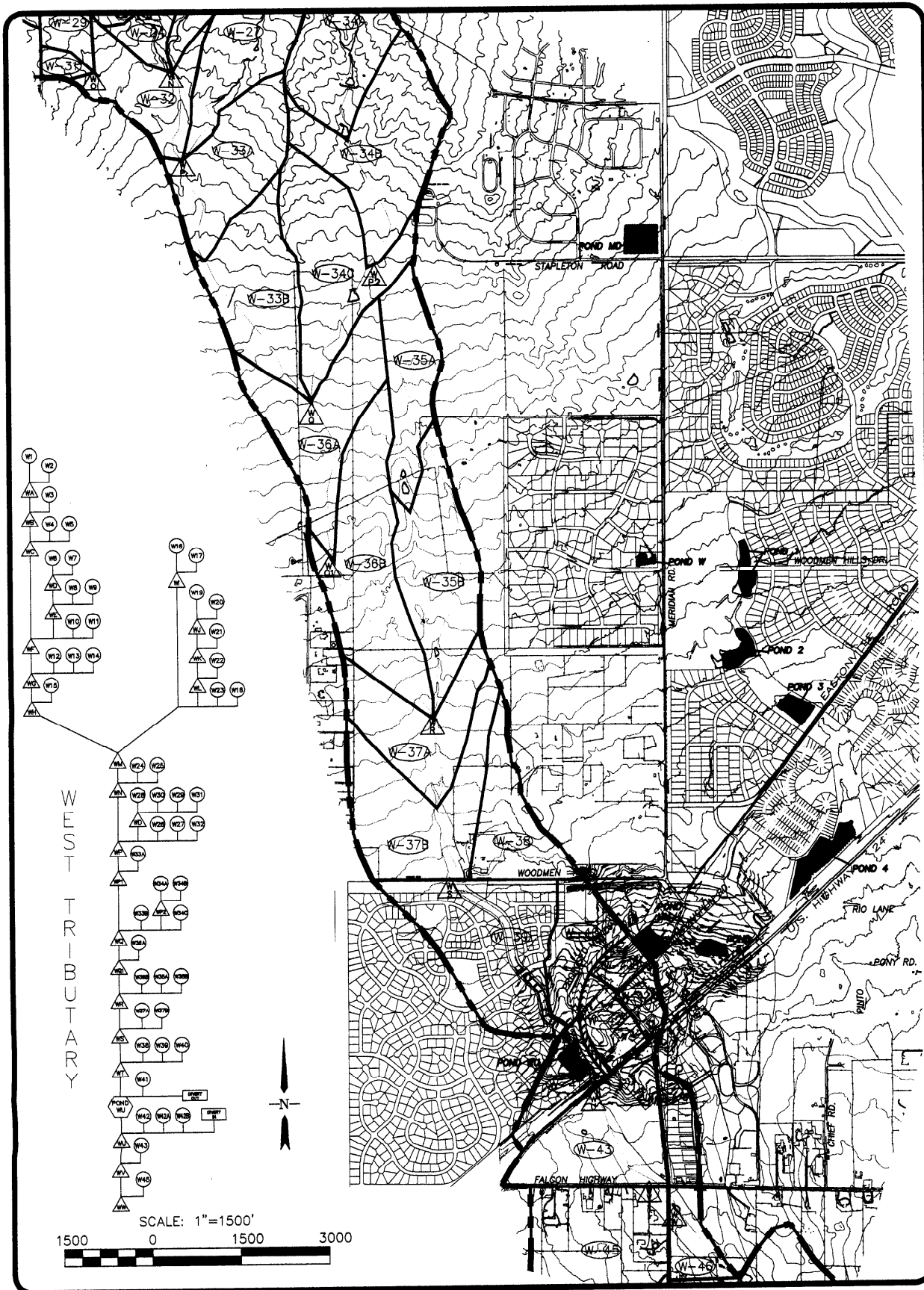
FIRM MAP 08041CO575 REV 11/26/03

SE *Springs*
Engineering

31 NORTH TEJON, SUITE 315
COLORADO SPRINGS, CO 80903
TEL: (719) 227-7388
FAX: (719) 227-7392

FIGURE 3

PROJECT NO. 057-07-032



MERIDIAN CROSSING

HEC-1 HYDROLOGIC MODEL

SE Springs
Engineering

31 NORTH TEJON, SUITE 315
COLORADO SPRINGS, COLORADO 80903
TEL: (719) 227-7388 FAX: (719) 227-7392

FIGURE 4

PROJECT NO. 06-0033

Appendix A: Existing HEC-1 Calculations

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                       *
*   VERSION 4.1                     *
*
* RUN DATE 28SEP07 TIME 11:57:06   *
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*
* U.S. ARMY CORPS OF ENGINEERS     *
* HYDROLOGIC ENGINEERING CENTER    *
* 609 SECOND STREET                 *
* DAVIS, CALIFORNIA 95616          *
* (916) 756-1104                   *
*
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	FALCON BASIN 5-YR/ 24-HOUR FLOOD/ EXISTING CONDITIONS									
2	ID	UPPER EAST TRIBUTARY (WOODMEN HILLS) BASED ON CLOMR APPROVED 2/2/99									
3	ID	INCLUDING 2 EXISTING SCS STOCK PONDS, WEST WOODMEN HILLS POND									
4	ID	NOTE: M1-M4 (PAINT BRUSH HILLS) MODELED AS HISTORIC TO ACCOUNT FOR									
5	ID	DETENTION POND AT MC									
6	ID	NOTE: NO CULVERT AT STAPLETON & MERIDIAN, TEMP CULVERTS AT MERIDIAN									
7	ID	DOWNSTREAM OF WOODMEN HILLS DRIVE (DIVERSION)									
8	IT	5	14JUL99	800	300						
9	IO	5									
10	KK	W1									
11	KM										
12	BA	.0479									
13	PB	2.6									
14	IN	15									
15	PC	.0005	.0015	.0030	.0045	.0060	.0080	.0100	.0120	.0143	.0165
16	PC	.0188	.0210	.0233	.0255	.0278	.0320	.0390	.0460	.0530	.0600
17	PC	.0750	.1000	.4000	.7000	.7250	.7500	.7650	.7800	.7900	.8000
18	PC	.8100	.8200	.8250	.8300	.8350	.8400	.8450	.8500	.8550	.8600
19	PC	.8638	.8675	.8713	.8750	.8788	.8825	.8863	.8900	.8938	.8975
20	PC	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270	.9300
21	PC	.9325	.9350	.9375	.9400	.9425	.9450	.9475	.9500	.9525	.9550
22	PC	.9575	.9600	.9625	.9650	.9675	.9700	.9725	.9750	.9775	.9800
23	PC	.9813	.9825	.9838	.9850	.9863	.9875	.9888	.9900	.9913	.9925
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31	KM										
32	BA	.0278									
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38	KK										
39	KM										
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50	KM										
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77	KM						
78	RK	592	.0372	.035	TRAP	5	4
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229	KK							
230	KM							
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246	KM							
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256	LS		61					
257	UD	.197						
258	KK	WN						
259	KM							
260	HC	3						
261	KK	N-P						
262	KM							
263	RK	1589	.017	.035	TRAP	20	4	
264	KK	W28						
265	KM							
266	BA	.0397						
267	LS		63					
268	UD	.128						
269	KK							
270	KM							
271	RK	1345	.0208	.035	TRAP	5	4	
272	KK	W30						
273	KM							
274	BA	.0509						
275	LS		63					
276	UD	.123						
277	KK							
278	KM							
279	RK	1078	.0074	.035	TRAP	5	4	
280	KK	W29						
281	KM							
282	BA	.0409						
283	LS		63					
284	UD	.145						
285	KK	W31						
286	KM							
287	BA	.0123						
288	LS		63					
289	UD	.073						
290	KK	W0						
291	KM							
292	HC	4						
293	KK	O-P						
294	KM							
295	RK	2169	.0226	.035	TRAP	5	4	
296	KK	W26						
297	KM							
298	BA	.0301						
299	LS		63					
300	UD	.062						

301	KK							
302	KM							
303	RK	4662	.0225	.035	TRAP	5	4	
304	KK	W27						
305	KM							
306	BA	.1633						
307	LS		60					
308	UD	.253						
309	KK	W32						
310	KM							
311	BA	.0890						
312	LS		60					
313	UD	.170						
314	KK	WP						
315	KM							
316	HC	5						
317	KK	P-Q						
318	KM							
319	RK	1925	.0182	.035	TRAP	25	4	
320	KK	W33A						
321	KM							
322	BA	.1261						
323	LS		60					
324	UD	.186						
325	KK	WP1						
326	KM							
327	HC	2						
328	KK	P1-Q						
329	KM							
330	RK	3000	.020	.035	TRAP	25	4	
331	KK	W33B						
332	KM							
333	BA	.1360						
334	LS		60					
335	UD	.225						
336	KK	W34A						
337	KM							
338	BA	.1261						
339	LS		60					
340	UD	.173						
341	KK	34A-P2						
342	KM							
343	RK	2550	.0176	.035	TRAP	25	4	
344	KK	W34B						
345	KM							
346	BA	.1766						
347	LS		60					
348	UD	.224						
349	KK	WP2						
350	KM							
351	HC	2						
352	KK	P2-Q						
353	KM							
354	RK	2640	.021	.035	TRAP	25	4	
355	KK	W34C						
356	KM							
357	BA	.1625						
358	LS		60					
359	UD	.244						
360	KK	WQ						
361	KM							
362	HC	4						
363	KK	Q-Q1						
364	KM							
365	RK	2940	.022	.035	TRAP	25	4	
366	KK	W36A						
367	KM							
368	BA	.1429						
369	LS		60					
370	UD	.234						
371	KK	WQ1						
372	KM							
373	HC	2						
374	KK	Q1-R						
375	KM							
376	RK	3400	.022	.035	TRAP	25	4	

377	KK	W36B						
378	KM							
379	BA	.1918						
380	LS		60					
381	UD	.306						
382	KK	W35A						
383	KM							
384	BA	.0958						
385	LS		60					
386	UD	.187						
387	KK	35A-WR						
388	KM							
389	RK	3715	.023	.035	TRAP	25	4	
390	KK	W35B						
391	KM							
392	BA	.1507						
393	LS	0	60					
394	UD	.259						
395	KK	WR						
396	KM							
397	HC	4						
398	KK	WR-S						
399	KM							
400	RK	2922	.0168	.035	TRAP	25	4	
401	KK	W37A						
402	KM							
403	BA	.1138						
404	LS		60					
405	UD	.185						
406	KK	37A-S						
407	KM							
408	RK	1430	.014	.035	TRAP	25	4	
409	KK	W37B						
410	KM							
411	BA	.1636						
412	LS		61					
413	UD	.218						
414	KK	WS						
415	KM							
416	HC	3						
417	KK	S-T						
418	KM							
419	RK	3653	.0164	.035	TRAP	25	4	
420	KK	W38						
421	KM							
422	BA	.0907						
423	LS		62					
424	UD	.190						
425	KK							
426	KM							
427	RK	2922	.0171	.035	TRAP	5	4	
428	KK	W39						
429	KM							
430	BA	.1833						
431	LS		60					
432	UD	.251						
433	KK	W40						
434	KM							
435	BA	.0964						
436	LS		60					
437	UD	.165						
438	KK	WT						
439	KM							
440	HC	4						
441	KK	T-U						
442	KM							
443	RK	1125	.0098	.035	TRAP	25	4	
444	KK	W41						
445	KM							
446	BA	.0601						
447	LS		60					
448	UD	.117						
449	KK	W42						
450	KM							
451	BA	.0581						
452	LS		81					

453	UD	.127						
454	KK	U-V						
455	KM							
456	RK	2656	.0184	.035	TRAP	5	4	
457	KK	WU						
458	KM							
459	HC	3						
460	KK							
461	KM							
462	RK	2215	.0181	.035	TRAP	25	4	
463	KK	W43						
464	KM							
465	BA	.1457						
466	LS		61					
467	UD	.169						
468	KK	WV						
469	KM							
470	HC	2						
471	KK	V-W						
472	KM							
473	RK	487	.0103	.035	TRAP	25	4	
474	KK	W45						
475	KM							
476	BA	.1931						
477	LS		61					
478	UD	.189						
479	KK	WW						
480	KM							
481	HC	2						
482	KK	W-X						
483	KM							
484	RK	1542	.0149	.035	TRAP	5	4	
485	KK	M1						
486	KM							
487	BA	.0665						
488	LS		60					
489	UD	.108						
490	KK							
491	KM							
492	RK	650	.0308	.035	TRAP	5	4	
493	KK	M2						
494	KM							
495	BA	.0273						
496	LS		60					
497	UD	.114						
498	KK	MB						
499	KM							
500	HC	2						
501	KK							
502	KM							
503	RK	928	.0302	.035	TRAP	5	4	
504	KK	M4						
505	KM							
506	BA	.0346						
507	LS		60					
508	UD	.121						
509	KK							
510	KM							
511	RK	406	.0197	.02	TRAP	40	0	
512	KK	M3						
513	KM							
514	BA	.0149						
515	LS		60					
516	UD	.076						
517	KK	MC						
518	KM							
519	HC	3						
520	KK							
521	KM							
522	RK	1902	.0231	.035	TRAP	5	4	
523	KK	M5						
524	KM							
525	BA	.0176						
526	LS		69					
527	UD	.108						

528	KK						
529	KM						
530	RK	1717	.0186	.02	TRAP	40	0
531	KK	M6					
532	KM						
533	BA	.0637					
534	LS		65				
535	UD	.233					
536	KK	MD					
537	KM						
538	HC	3					
539	KK						
540	KM						
541	RK	2841	.019	.035	TRAP	5	4
542	KK	M7					
543	KM						
544	BA	.0524					
545	LS		69				
546	UD	.170					
547	KK						
548	KM						
549	RK	1044	.0268	.02	TRAP	40	0
550	KK	M8					
551	KM						
552	BA	.0370					
553	LS		61				
554	UD	.126					
555	KK	ME					
556	KM						
557	HC	2					
558	KK						
559	KM						
560	RK	2992	.0187	.035	TRAP	5	4
561	KK	M9					
562	KM						
563	BA	.0169					
564	LS		69				
565	UD	.087					
566	KK						
567	KM						
568	RK	3433	.0253	.03	TRAP	5	4
569	KK	M12A					
570	KM						
571	BA	.0658					
572	LS		60				
573	UD	.159					
574	KK	M12B					
575	KM						
576	BA	.1481					
577	LS		60				
578	UD	.219					
579	KK	MF					
580	KM						
581	HC	5					
582	KK						
583	KM						
584	RK	2586	.0224	.035	TRAP	10	4
585	KK	M13					
586	KM						
587	BA	.0614					
588	LS		64				
589	UD	.165					
590	KK						
591	KM						
592	RK	1700	.01	.035	TRAP	6	4
593	KK	M14					
594	KM						
595	BA	.1624					
596	LS		64				
597	UD	.228					
598	KK	MG					
599	KM						
600	HC	2					
601	KK	PONDW					
602	KM	WOODMEN HILLS DETENTION POND WEST (FROM FDR WH FLG F4)					

603	SV	0	.68	1.5	235	3.6	4.9	6.3	7.34	7.34
604	SE	968	969	970	971	972	973	974	975	976
605	SQ	0	8	15.5	41	84.4	110	138	152	205
606	RS	1	ELEV	968						
607	KK	MH								
608	KM									
609	HC	2								
610	KK									
611	KM									
612	RK	1276	.0212	.035		TRAP	15		4	
613	KK	MH-P2								
614	KM	DIVERT FLOW TO POND 2 VIA TWIN 23x47 ARCH CMPS UNDER MERIDIAN								
615	DT	DIVRT1	90							
616	DI	0	39	72	152	263	318	377	442	591
617	DQ	0	39	70	80	80	80	85	85	90
618	KK	M15								
619	KM									
620	BA	.1242								
621	LS		64							
622	UD	.203								
623	KK	MI								
624	KM									
625	HC	2								
626	KK									
627	KM									
628	RK	1995	.0165	.035		TRAP	15		4	
629	KK	M19								
630	KM									
631	BA	.0499								
632	LS		61							
633	UD	.159								
634	KK	MJ								
635	KM									
636	HC	2								
637	KK									
638	KM									
639	RK	2215	.0158	.035		TRAP	15		4	
640	KK	M10								
641	KM									
642	BA	.0581								
643	LS		62							
644	UD	.102								
645	KK	M10-K								
646	KM									
647	RK	3150	.0255	.03		TRAP	5		4	
648	KK	M11A								
649	KM									
650	BA	.1067								
651	LS		61							
652	UD	.231								
653	KK	MK								
654	KM									
655	HC	2								
656	KK	MK-K1								
657	KM									
658	RK	2300	.260	.03		TRAP	5		4	
659	KK	M11B								
660	KM									
661	BA	.0879								
662	LS		60							
663	UD	.150								
664	KK	11B-K1								
665	KM									
666	RK	2400	.025	.03		TRAP	5		4	
667	KK	M11C								
668	KM									
669	BA	.0933								
670	LS		60							
671	UD	.160								
672	KK	MK1								
673	KM									
674	HC	3								
675	KK	K1-ML								
676	KM									
677	RK	1821	.028	.035		TRAP	5		4	

678	KK	M16						
679	KM							
680	BA	.042						
681	LS		60					
682	UD	.139						
683	KK	ML						
684	KM							
685	HC	2						
686	KK							
687	KM							
688	RK	2099	.02	.035	TRAP	5	4	
689	KK	M17						
690	KM							
691	BA	.0765						
692	LS		61					
693	UD	.133						
694	KK	MM						
695	KM							
696	HC	2						
697	KK							
698	KM							
699	RK	2320	.0121	.035	TRAP	10	4	
700	KK	M18						
701	KM							
702	BA	.061						
703	LS		61					
704	UD	.142						
705	KK							
706	KM							
707	RK	2122	.017	.035	TRAP	5	4	
708	KK	M20						
709	KM							
710	BA	.1341						
711	LS		61					
712	UD	.211						
713	KK	MN						
714	KM							
715	HC	4						
716	KK							
717	KM							
718	RK	1531	.0202	.035	TRAP	25	4	
719	KK	M21						
720	KM							
721	BA	.0241						
722	LS		61					
723	UD	.125						
724	KK							
725	KM							
726	RK	1322	.0212	.035	TRAP	5	4	
727	KK	M23						
728	KM							
729	BA	.0461						
730	LS		60					
731	UD	.120						
732	KK	MO						
733	KM							
734	HC	3						
735	KK							
736	KM							
737	RK	974	.0133	.035	TRAP	25	4	
738	KK	M24						
739	KM							
740	BA	.0776						
741	LS		60					
742	UD	.125						
743	KK	MP						
744	KM							
745	HC	2						
746	KK							
747	KM							
748	RK	290	.0138	.035	TRAP	25	4	
749	KK	M25						
750	KM							
751	BA	.0105						
752	LS		60					
753	UD	.130						

754	KK	MQ						
755	KM							
756	HC	2						
757	KK							
758	KM							
759	RK	3305	.0136	.035	TRAP	25	4	
760	KK	M26						
761	KM							
762	BA	.1779						
763	LS			65				
764	UD	.250						
765	KK	MR						
766	KM							
767	HC	2						
768	KK	W44						
769	KM							
770	BA	.0384						
771	LS			60				
772	UD	.141						
773	KK							
774	KM							
775	RK	2029	.0148	.035	TRAP	5	4	
776	KK	W47						
777	KM							
778	BA	.0541						
779	LS			60				
780	UD	.148						
781	KK							
782	KM							
783	RK	1438	.0223	.035	TRAP	5	4	
784	KK	W46						
785	KM							
786	BA	.0418						
787	LS			61				
788	UD	.154						
789	KK	M27						
790	KM							
791	BA	.0528						
792	LS			60				
793	UD	.132						
794	KK	WX						
795	KM							
796	HC	6						
797	KK							
798	KM							
799	RK	2563	.0125	.035	TRAP	40	4	
800	KK	W48						
801	KM							
802	BA	.1179						
803	LS			61				
804	UD	.091						
805	KK							
806	KM							
807	RK	2400	.0188	.035	TRAP	5	4	
808	KK	W49						
809	KM							
810	BA	.2651						
811	LS			61				
812	UD	.181						
813	KK	WZ						
814	KM							
815	HC	3						
816	KK							
817	KM							
818	RK	800	.0125	.035	TRAP	40	4	
819	KK	W50						
820	KM							
821	BA	.1061						
822	LS			61				
823	UD	.145						
824	KK	WAB						
825	KM							
826	HC	2						
827	KK							
828	KM							

829	RK	742	.0108	.035	TRAP	40	4
830	KK	W51					
831	KM						
832	BA	.0546					
833	LS		63				
834	UD	.172					
835	KK	WAC					
836	KM						
837	HC	2					
838	KK						
839	KM						
840	RK	638	.0345	.035	TRAP	40	4
841	KK	W52					
842	KM						
843	BA	.0499					
844	LS		63				
845	UD	.109					
846	KK						
847	KM						
848	RK	1171	.0205	.035	TRAP	5	4
849	KK	W53					
850	KM						
851	BA	.0531					
852	LS		63				
853	UD	.156					
854	KK	WAD					
855	KM						
856	HC	2					
857	KK						
858	KM						
859	RK	290	.0310	.035	TRAP	10	4
860	KK	W54					
861	KM						
862	BA	.0078					
863	LS		60				
864	UD	.050					
865	KK	WAE					
866	KM						
867	HC	3					
868	KK						
869	KM						
870	RK	1925	.0052	.035	TRAP	40	4
871	KK	W56					
872	KM						
873	BA	.1831					
874	LS		60				
875	UD	.191					
876	KK	WAF					
877	KM						
878	HC	2					
879	KK						
880	KM						
881	RK	1032	.0155	.035	TRAP	40	4
882	KK	W62					
883	KM						
884	BA	.0750					
885	LS		60				
886	UD	.090					
887	KK						
888	KM						
889	RK	2169	.0203	.035	TRAP	5	4
890	KK	W63					
891	KM						
892	BA	.047					
893	LS		60				
894	UD	.109					
895	KK						
896	KM						
897	RK	1450	.0131	.035	TRAP	5	4
898	KK	W61					
899	KM						
900	BA	.192					
901	LS		60				
902	UD	.251					
903	KK	WAH					

904	KM																			
905	HC	3																		
906	KK																			
907	KM																			
908	RK	1241	.0153	.035		TRAP		5		4										
909	KK	W57																		
910	KM																			
911	BA	.0732																		
912	LS		60																	
913	UD	.140																		
914	KK																			
915	KM																			
916	RK	5903	.0254	.035		TRAP		5		4										
917	KK	W58																		
918	KM																			
919	BA	.2296																		
920	LS		60																	
921	UD	.251																		
922	KK	WAI																		
923	KM																			
924	HC	3																		
925	KK																			
926	KM																			
927	RK	232	.0086	.035		TRAP		15		4										
928	KK	E1A																		
929	KM																			
930	BA	.1151																		
931	LS	0	60																	
932	UD	.234																		
933	KK	E1A-EA																		
934	KM																			
935	RK	4000	.022	.035		TRAP		5		4										
936	KK	E1B																		
937	KM																			
938	BA	.1665																		
939	LS	0	60																	
940	UD	.233																		
941	KK	EA																		
942	KM																			
943	HC	2																		
944	KK	EA-EB																		
945	KM																			
946	RK	1900	.022	.035		TRAP		5		4										
947	KK	E2																		
948	KM																			
949	BA	.104																		
950	LS	0	60																	
951	UD	.149																		
952	KK	EB																		
953	KM																			
954	HC	2																		
955	KK	POND1																		
956	KM																			
957	SV	0	.01	.28	1.12	2.70	5.18	6.00	6.94											
958	SE	945.5	946	948	950	952	954	954.5	955											
959	SQ	0	0	0	0	0	48.5	176.4	351.4											
960	RS	1	ELEV	945.5																
961	KK																			
962	KM																			
963	RK	1300	.0192	.035		TRAP		5		4										
964	KK	E3																		
965	KM																			
966	BA	.090																		
967	LS		60																	
968	UD	.128																		
969	KK	MH-P2																		
970	KM	RETRIEVE DIVERSION FROM W. MERIDIAN RD DITCH																		
971	DR	DIVRT1																		
972	KK	EC																		
973	KM																			
974	HC	3																		
975	KK	POND2																		
976	KM																			
977	SV	0	.21	1.11	3.19	6.89	9.52	11.08	12.82	14.72	16.70									
978	SE	920	922	924	926	928	929	929.5	930	930.5	931									
979	SQ	0	0	0	0	0	0	25	86.5	186.2	308.4									

980	RS	1	ELEV	920				
981	KK							
982	KM							
983	RK	1700	.0141	.035	TRAP	5	4	
984	KK	E1C						
985	KM							
986	BA	.0845						
987	LS		60					
988	UD	.200						
989	KK	1C-ED1						
990	KM							
991	RK	3450	.022	.035	TRAP	5	4	
992	KK	E4						
993	KM							
994	BA	.127						
995	LS		60					
996	UD	.200						
997	KK	ED1						
998	KM							
999	HC	2						
1000	KK	ED1-ED						
1001	KM							
1002	RK	450	.0178	.03	TRAP	5	4	
1003	KK	E5						
1004	KM							
1005	BA	.094						
1006	LS		60					
1007	UD	.160						
1008	KK	ED						
1009	KM							
1010	HC	3						
1011	KK							
1012	KM							
1013	RK	950	.0211	.035	TRAP	10	4	
1014	KK	E8						
1015	KM							
1016	BA	.0446						
1017	LS		60					
1018	UD	.139						
1019	KK	EE						
1020	KM							
1021	HC	2						
1022	KK							
1023	KM							
1024	RK	1500	.0127	.035	TRAP	10	4	
1025	KK	E10						
1026	KM							
1027	BA	.029						
1028	LS		60					
1029	UD	.158						
1030	KK	EF						
1031	KM							
1032	HC	2						
1033	KK	F-G						
1034	KM							
1035	RK	950	.0074	.035	TRAP	15	4	
1036	KK	E6						
1037	KM							
1038	BA	.119						
1039	LS		60					
1040	UD	.228						
1041	KK	E7						
1042	KM							
1043	BA	.031						
1044	LS		60					
1045	UD	.082						
1046	KK							
1047	KM							
1048	RK	1100	.0100	.035	TRAP	5	4	
1049	KK	EG1						
1050	KM							
1051	HC	2						
1052	KK	G1-G						
1053	KM							
1054	RK	1650	.0176	.035	TRAP	5	4	

1055	KK	E9						
1056	KM							
1057	BA	.077						
1058	LS		60					
1059	UD	.207						
1060	KK							
1061	KM							
1062	RK	1500	.0080	.03	TRAP	5	4	
1063	KK	E11						
1064	KM							
1065	BA	.045						
1066	LS		60					
1067	UD	.195						
1068	KK	E12						
1069	KM							
1070	BA	.092						
1071	LS		60					
1072	UD	.156						
1073	KK	EG						
1074	KM							
1075	HC	5						
1076	KK	E13						
1077	KM							
1078	BA	.0165						
1079	LS		60					
1080	UD	.252						
1081	KK	E14						
1082	KM							
1083	BA	.0051						
1084	LS		60					
1085	UD	.153						
1086	KK							
1087	KM							
1088	RK	279	.0108	.03	TRAP	5	4	
1089	KK	EH						
1090	KM							
1091	HC	3						
1092	KK							
1093	KM							
1094	RK	2400	.0204	.035	TRAP	10	4	
1095	KK	E19						
1096	KM							
1097	BA	.0406						
1098	LS		62					
1099	UD	.127						
1100	KK	EJ1						
1101	KM							
1102	HC	2						
1103	KK	J1-K						
1104	KM							
1105	RK	4013	.013	.035	TRAP	10	4	
1106	KK	E15						
1107	KM							
1108	BA	.0355						
1109	LS		63					
1110	UD	.097						
1111	KK							
1112	KM							
1113	RK	951	.0189	.035	TRAP	5	4	
1114	KK	E16						
1115	KM							
1116	BA	.0307						
1117	LS		63					
1118	UD	.100						
1119	KK	E1						
1120	KM							
1121	HC	2						
1122	KK							
1123	KM							
1124	RK	1334	.0105	.035	TRAP	5	4	
1125	KK	E17						
1126	KM							
1127	BA	.0312						
1128	LS		63					
1129	UD	.097						

1130	KK						
1131	KM						
1132	RK	1728	.0145	.035	TRAP	5	4
1133	KK	E18					
1134	KM						
1135	BA	.0488					
1136	LS		63				
1137	UD	.180					
1138	KK	EJ2					
1139	KM						
1140	HC	3					
1141	KK						
1142	KM						
1143	RK	4221	.0123	.035	TRAP	20	4
1144	KK	E23					
1145	KM						
1146	BA	.1683					
1147	LS		62				
1148	UD	.250					
1149	KK	E24					
1150	KM						
1151	BA	.140					
1152	LS		63				
1153	UD	.371					
1154	KK	EK					
1155	KM						
1156	HC	4					
1157	KK						
1158	KM						
1159	RK	2817	.0149	.035	TRAP	25	4
1160	KK	E21					
1161	KM						
1162	BA	.0873					
1163	LS		60				
1164	UD	.183					
1165	KK						
1166	KM						
1167	RK	1647	.0121	.035	TRAP	5	4
1168	KK	E20					
1169	KM						
1170	BA	.0771					
1171	LS		62				
1172	UD	.219					
1173	KK						
1174	KM						
1175	RK	569	.0141	.035	TRAP	5	4
1176	KK	E22					
1177	KM						
1178	BA	.0677					
1179	LS		61				
1180	UD	.240					
1181	KK	EL					
1182	KM						
1183	HC	3					
1184	KK						
1185	KM						
1186	RK	2041	.0162	.035	TRAP	25	4
1187	KK	E25					
1188	KM						
1189	BA	.1665					
1190	LS		61				
1191	UD	.176					
1192	KK	EM					
1193	KM						
1194	HC	3					
1195	KK						
1196	KM						
1197	RK	928	.0108	.035	TRAP	40	4
1198	KK	E26					
1199	KM						
1200	BA	.0361					
1201	LS		63				
1202	UD	.096					
1203	KK	EN					
1204	KM						
1205	HC	2					

1206	KK						
1207	KM						
1208	RK	1832	.0126	.035	TRAP	40	4
1209	KK	E27					
1210	KM						
1211	BA	.1236					
1212	LS		63				
1213	UD	.172					
1214	KK	EO					
1215	KM						
1216	HC	2					
1217	KK						
1218	KM						
1219	RK	1625	.0133	.035	TRAP	5	3
1220	KK	W55					
1221	KM						
1222	BA	.0452					
1223	LS		60				
1224	UD	.093					
1225	KK	WAG					
1226	KM						
1227	HC	2					
1228	KK						
1229	KM						
1230	RK	2025	.0109	.035	TRAP	5	4
1231	KK	W59					
1232	KM						
1233	BA	.0705					
1234	LS		60				
1235	UD	.200					
1236	KK	WAJ					
1237	KM						
1238	HC	4					
1239	KK						
1240	KM						
1241	RK	1450	.0124	.035	TRAP	40	4
1242	KK	E28					
1243	KM						
1244	BA	.0718					
1245	LS		61				
1246	UD	.223					
1247	KK						
1248	KM						
1249	RK	2064	.0165	.035	TRAP	40	4
1250	KK	E29					
1251	KM						
1252	BA	.0465					
1253	LS		61				
1254	UD	.166					
1255	KK	EZZ					
1256	KM	COMBINE E29 & E30 AT DP ZZ					
1257	HC	2					
1258	KK	W60					
1259	KM						
1260	BA	.0711					
1261	LS		60				
1262	UD	.182					
1263	KK	ZZ					
1264	KM	COMBINE ALL AT DP ZZ					
1265	HC	3					
1266	ZZ						


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      .           v
      .           V
136  .
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139  .           .           W14
      .           .           V
      .           .           V
144  .
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147  .           .           .           W13
      .           .           .           .
152  .           .           .           .
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155  .           .           .           .
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      .           .           .           .
158  .
      .
161  .           .           W15
      .           .           V
      .           .           V
166  .
      .
169  .           .           .           .
      .           .           .           .
      .           .           .           .
172  .           .           W16
      .           .           V
      .           .           V
177  .
      .
180  .           .           .           W17
      .           .           .           .
185  .           .           .           .
      .           .           .           .
      .           .           .           .
188  .           .           .           .
      .           .           .           .
      .           .           .           .
191  .           .           .           W19
      .           .           .           V
      .           .           .           V
196  .
      .
199  .           .           .           .           W20
      .           .           .           .           .
204  .           .           .           .           .
      .           .           .           .           .
      .           .           .           .           .
207  .           .           .           .           .
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210  .           .           .           .           .           W21
      .           .           .           .           .           .
215  .           .           .           .           .           .
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218  .           .           .           .           .           .
      .           .           .           .           .           .
221  .           .           .           .           .           .           W22
      .           .           .           .           .           .           .
226  .           .           .           .           .           .           .
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229  .           .           .           .           .           .           .
      .           .           .           .           .           .           .
232  .           .           .           .           .           .           .           W23
      .           .           .           .           .           .           .           .
237  .           .           .           .           .           .           .           .           W18
      .           .           .           .           .           .           .           .           .
242  .           .           .           .           .           .           .           .           .
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245  .           .           .           .           .           .           .           .           .
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248  .           .           .           .           .           .           .           .           .
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377 . . . W36B
382 . . . W35A
. . . V
387 . . . 35A-WR
. . . V
390 . . . W35B
. . .
395 WR-----
. V
398 WR-S
. V
401 . . . W37A
. . . V
406 . . . 37A-S
. . . V
409 . . . W37B
. . .
414 WS-----
. V
417 S-T
. V
420 . . . W38
. . . V
425 . . . V
. . .
428 . . . W39
. . .
433 . . . W40
. . .
438 WT-----
. V
441 T-U
. V
444 . . . W41
. . .
449 . . . W42
. . . V
454 . . . U-V
. . . V
457 WU-----
. V
460 . . .
. . .
463 . . . W43
. . .
468 WV-----
. V
471 V-W
. . .
474 . . . W45
. . .
479 WW-----
. V
482 W-X
. . .
485 . . . M1
. . . V
490 . . . V
. . .
493 . . . M2
. . .
498 . . . MB-----

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V
V
501 .
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504 .          M4
      .          V
      .          V
509 .
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512 .          M3
      .
.
517 .-----MC
      .          V
      .          V
520 .
.
523 .          M5
      .          V
      .          V
528 .
.
531 .          M6
      .
.
536 .-----MD
      .          V
      .          V
539 .
.
542 .          M7
      .          V
      .          V
547 .
.
550 .          M8
      .
.
555 .-----ME
      .          V
      .          V
558 .
.
561 .          M9
      .          V
      .          V
566 .
.
569 .          M12A
      .
      .
574 .          M12B
      .
      .
579 .-----MF
      .          V
      .          V
582 .
.
585 .          M13
      .          V
      .          V
590 .
.
593 .          M14
      .
      .
598 .-----MG
      .          V
      .          V
601 .          PONDW
      .
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607 .-----MH
      .          V
      .          V
610 .
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615 .-----> DIVRT1
613 .-----MH-P2
.
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618 .          M15
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623	MI			
	V				
626	V				
	.				
629		M19			
	.				
634	MJ			
	V				
637	V				
	.				
640		M10			
	.	V			
	.	V			
645		M10-K			
	.				
648			M11A		
	.				
653		MK		
	.	V			
656		MK-K1			
	.				
659			M11B		
	.		V		
	.		V		
664			11B-K1		
	.				
667				M11C	
	.				
672		MK1		
	.	V			
	.	V			
675		K1-ML			
	.				
678				M16	
	.				
683		ML		
	.	V			
686		V			
	.				
689				M17	
	.				
694		MM		
	.	V			
697		V			
	.				
700				M18	
	.			V	
705				V	
	.				
708					M20
	.				
713		MN		
	.	V			
716		V			
	.				
719				M21	
	.	V			
724		V			
	.				
727					M23
	.				
732		MO		
	.	V			
735		V			
	.				
738					M24

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860      .           .           W54
      .           .           .
865      WAE .....
      V
      V
868      .
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871      .           W56
      .           .
876      WAF .....
      V
      V
879      .
      .
882      .           W62
      .           V
      .           V
887      .
      .
890      .           W63
      .           V
      .           V
895      .
      .
898      .           .           W61
      .           .           .
903      WAH .....
      V
      V
906      .
      .
909      .           W57
      .           V
      .           V
914      .
      .
917      .           .           W58
      .           .           .
922      WAI .....
      V
      V
925      .
      .
928      .           E1A
      .           V
      .           V
933      .           E1A-EA
      .           .
      .           .           E1B
936      .           .           .
      .           .           .
941      .           EA .....
      .           V
      .           V
944      .           EA-EB
      .           .
      .           .           E2
947      .           .           .
      .           .           .
952      .           EB .....
      .           V
      .           V
955      .           POND1
      .           V
      .           V
961      .
      .
964      .           .           E3
      .           .
      .           .
971      .           .           .           <----- DIVRT1
969      .           .           .           MH-P2
      .           .           .
972      .           EC .....
      .           V
      .           V
975      .           POND2
      .           V
      .           V
981      .

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984	.	.	.	E1C	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
989	.	.	.	1C-ED1	.	.	.

992	E4	.

997	.	.	.	ED1
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
1000	.	.	.	ED1-ED	.	.	.

1003	E5	.

1008	.	.	.	ED
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
1011

1014	E8	.

1019	.	.	.	EE
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
1022

1025	E10	.

1030	.	.	.	EF
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
1033	.	.	.	F-G	.	.	.

1036	E6	.

1041	E7	.
	V	.
	V	.
1046

1049	EG1
	V	.
	V	.
1052	.	.	.	G1-G	.	.	.

1055	E9	.
	V	.
	V	.
1060

1063	E11

1068	E12

1073	.	.	.	EG

1076	E13	.

1081	E14	.
	V	.
	V	.
1086

1089	.	.	.	EH
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
1092

1095	E19	.

1100	.	.	.	EJ1
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.

1103	J1-K
1106	.	E15	.	.	.
	.	V	.	.	.
1111	.	V	.	.	.
1114	.	.	E16	.	.

1119	.	E1	.	.	.
	.	V	.	.	.
1122	.	V	.	.	.
1125	.	.	E17	.	.
	.	.	V	.	.
1130	.	.	V	.	.
1133	.	.	.	E18	.

1138	.	EJ2	.	.	.
	.	V	.	.	.
1141	.	V	.	.	.
1144	.	.	E23	.	.
1149	.	.	.	E24	.

1154	EK
	V
1157	V
1160	.	E21	.	.	.
	.	V	.	.	.
1165	.	V	.	.	.
1168	.	.	E20	.	.
	.	.	V	.	.
1173	.	.	V	.	.
1176	.	.	.	E22	.

1181	.	EL	.	.	.
	.	V	.	.	.
1184	.	V	.	.	.
1187	.	.	E25	.	.

1192	EM
	V
1195	V
1198	.	E26	.	.	.

1203	EN
	V
1206	V
1209	.	E27	.	.	.

1214	EO
	V
1217	V
1220	.	W55	.	.	.

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1225      .           .           .           .           .           .           .           .           .           .
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1236      .           .           .           .           .           .           .           .           .           .
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1239      .           .           .           .           .           .           .           .           .           .
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1247      .           .           .           .           .           .           .           .           .           .
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1258      .           .           .           .           .           .           .           .           .           .
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1263      .           .           .           .           .           .           .           .           .           .
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(***) RUNOFF ALSO COMPUTED AT THIS LOCATION


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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 28SEP07 TIME 11:57:06 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

FALCON BASIN 5-YR/ 24-HOUR FLOOD/ EXISTING CONDITIONS
 UPPER EAST TRIBUTARY (WOODMEN HILLS) BASED ON CLOMR APPROVED 2/2/99
 INCLUDING 2 EXISTING SCS STOCK PONDS, WEST WOODMEN HILLS POND
 NOTE: M1-M4 (PAINT BRUSH HILLS) MODELED AS HISTORIC TO ACCOUNT FOR
 DETENTION POND AT MC
 NOTE: NO CULVERT AT STAPLETON & MERIDIAN, TEMP CULVERTS AT MERIDIAN
 DOWNSTREAM OF WOODMEN HILLS DRIVE (DIVERSION)

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9 IO OUTPUT CONTROL VARIABLES
    IPRNT 5 PRINT CONTROL
    IPLOT 0 PLOT CONTROL
    QSCAL 0. HYDROGRAPH PLOT SCALE

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IT HYDROGRAPH TIME DATA
    NMIN 5 MINUTES IN COMPUTATION INTERVAL
    IDATE 14JUL99 STARTING DATE
    ITIME 0800 STARTING TIME
    NQ 300 NUMBER OF HYDROGRAPH ORDINATES
    NDDATE 15JUL99 ENDING DATE
    NDTIME 0855 ENDING TIME
    ICENT 19 CENTURY MARK

    COMPUTATION INTERVAL .08 HOURS
    TOTAL TIME BASE 24.92 HOURS

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ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

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1
 RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	W1	5.	5.83	1.	0.	0.	.05		
ROUTED TO		4.	5.92	1.	0.	0.	.05		
HYDROGRAPH AT	W2	2.	5.83	0.	0.	0.	.03		
2 COMBINED AT	WA	6.	5.92	1.	0.	0.	.08		
ROUTED TO		6.	5.92	1.	0.	0.	.08		
HYDROGRAPH AT	W3	5.	5.83	1.	0.	0.	.05		
2 COMBINED AT	WB	11.	5.92	2.	1.	1.	.13		
ROUTED TO		10.	5.92	2.	1.	1.	.13		
HYDROGRAPH AT	W4	1.	5.75	0.	0.	0.	.01		
ROUTED TO		1.	5.83	0.	0.	0.	.01		
HYDROGRAPH AT	W5	2.	5.75	0.	0.	0.	.02		
3 COMBINED AT	WC	11.	5.92	2.	1.	1.	.15		
ROUTED TO									

+			11.	5.92	2.	1.	1.	.15
+	HYDROGRAPH AT							
+		W6	5.	5.83	1.	0.	0.	.05
+	ROUTED TO		5.	5.83	1.	0.	0.	.05
+	HYDROGRAPH AT							
+		W7	2.	5.75	0.	0.	0.	.02
+	ROUTED TO		2.	5.83	0.	0.	0.	.02
+	2 COMBINED AT							
+		WD	7.	5.83	1.	0.	0.	.07
+	ROUTED TO							
+		D-E	6.	5.83	1.	0.	0.	.07
+	HYDROGRAPH AT							
+		W8	3.	5.75	0.	0.	0.	.03
+	ROUTED TO		3.	5.83	0.	0.	0.	.03
+	HYDROGRAPH AT							
+		W9	5.	5.83	1.	0.	0.	.04
+	3 COMBINED AT							
+		WE	14.	5.83	2.	1.	1.	.14
+	ROUTED TO							
+		E-F	13.	5.92	2.	1.	1.	.14
+	HYDROGRAPH AT							
+		W10	5.	5.83	1.	0.	0.	.04
+	ROUTED TO		5.	5.83	1.	0.	0.	.04
+	HYDROGRAPH AT							
+		W11	3.	5.75	0.	0.	0.	.03
+	4 COMBINED AT							
+		WF	29.	5.92	5.	2.	2.	.36
+	ROUTED TO							
+		F-G	28.	6.00	5.	2.	2.	.36
+	HYDROGRAPH AT							
+		W12	4.	5.83	1.	0.	0.	.04
+	ROUTED TO		4.	5.92	1.	0.	0.	.04
+	HYDROGRAPH AT							
+		W14	5.	5.83	1.	0.	0.	.05
+	ROUTED TO		5.	5.92	1.	0.	0.	.05
+	HYDROGRAPH AT							
+		W13	10.	5.92	2.	1.	1.	.11
+	4 COMBINED AT							
+		WG	43.	6.00	8.	3.	3.	.56
+	ROUTED TO							
+		G-H	41.	6.08	8.	3.	3.	.56
+	ROUTED TO		39.	6.17	8.	3.	3.	.56
+	HYDROGRAPH AT							
+		W15	10.	5.83	1.	1.	1.	.09
+	ROUTED TO		9.	5.92	1.	1.	1.	.09
+	2 COMBINED AT							
+		WH	43.	6.17	9.	4.	4.	.65
+	HYDROGRAPH AT							
+		W16	4.	5.83	0.	0.	0.	.03
+	ROUTED TO		3.	5.92	0.	0.	0.	.03
+	HYDROGRAPH AT							
+		W17	2.	5.83	0.	0.	0.	.02
+	2 COMBINED AT							
+		WI	5.	5.83	1.	0.	0.	.05

+	ROUTED TO	I-M	5.	6.00	1.	0.	0.	.05
+	HYDROGRAPH AT	W19	5.	5.75	1.	0.	0.	.04
+	ROUTED TO		5.	5.83	1.	0.	0.	.04
+	HYDROGRAPH AT	W20	5.	5.75	0.	0.	0.	.03
+	2 COMBINED AT	WJ	9.	5.83	1.	0.	0.	.07
+	ROUTED TO		9.	6.00	1.	0.	0.	.07
+	HYDROGRAPH AT	W21	11.	5.83	2.	1.	1.	.13
+	2 COMBINED AT	WK	18.	5.92	3.	1.	1.	.21
+	ROUTED TO		16.	5.92	3.	1.	1.	.21
+	HYDROGRAPH AT	W22	2.	5.75	0.	0.	0.	.01
+	2 COMBINED AT	WL	17.	5.92	3.	1.	1.	.22
+	ROUTED TO		17.	6.00	3.	1.	1.	.22
+	HYDROGRAPH AT	W23	2.	5.83	0.	0.	0.	.02
+	HYDROGRAPH AT	W18	9.	5.92	2.	1.	1.	.13
+	5 COMBINED AT	WM	66.	6.08	15.	6.	6.	1.06
+	ROUTED TO	M-N	65.	6.17	15.	6.	6.	1.06
+	HYDROGRAPH AT	W24	4.	5.83	1.	0.	0.	.04
+	HYDROGRAPH AT	W25	8.	5.92	1.	1.	1.	.10
+	3 COMBINED AT	WN	70.	6.17	17.	7.	7.	1.20
+	ROUTED TO	N-P	67.	6.25	16.	7.	7.	1.20
+	HYDROGRAPH AT	W28	7.	5.83	1.	0.	0.	.04
+	ROUTED TO		6.	5.92	1.	0.	0.	.04
+	HYDROGRAPH AT	W30	9.	5.83	1.	0.	0.	.05
+	ROUTED TO		8.	5.92	1.	0.	0.	.05
+	HYDROGRAPH AT	W29	6.	5.83	1.	0.	0.	.04
+	HYDROGRAPH AT	W31	3.	5.75	0.	0.	0.	.01
+	4 COMBINED AT	WO	20.	5.92	3.	1.	1.	.14
+	ROUTED TO	O-P	20.	6.00	3.	1.	1.	.14
+	HYDROGRAPH AT	W26	7.	5.75	1.	0.	0.	.03
+	ROUTED TO		6.	6.08	1.	0.	0.	.03
+	HYDROGRAPH AT	W27	10.	6.00	2.	1.	1.	.16
+	HYDROGRAPH AT	W32	7.	5.92	1.	0.	0.	.09

+	5 COMBINED AT	WP	88.	6.17	23.	10.	9.	1.63
	ROUTED TO	P-Q	87.	6.25	23.	10.	9.	1.63
+	HYDROGRAPH AT	W33A	9.	5.92	2.	1.	1.	.13
+	2 COMBINED AT	WP1	91.	6.25	24.	10.	10.	1.75
+	ROUTED TO	P1-Q	90.	6.33	24.	10.	10.	1.75
+	HYDROGRAPH AT	W33B	9.	5.92	2.	1.	1.	.14
+	HYDROGRAPH AT	W34A	9.	5.92	2.	1.	1.	.13
+	ROUTED TO	34A-P2	9.	6.08	2.	1.	1.	.13
+	HYDROGRAPH AT	W34B	11.	5.92	2.	1.	1.	.18
+	2 COMBINED AT	WP2	18.	6.00	4.	2.	2.	.30
+	ROUTED TO	P2-Q	18.	6.17	4.	2.	2.	.30
+	HYDROGRAPH AT	W34C	10.	6.00	2.	1.	1.	.16
+	4 COMBINED AT	WQ	114.	6.25	32.	13.	13.	2.36
+	ROUTED TO	Q-Q1	113.	6.33	32.	13.	13.	2.36
+	HYDROGRAPH AT	W36A	9.	5.92	2.	1.	1.	.14
+	2 COMBINED AT	WQ1	118.	6.33	33.	14.	14.	2.50
+	ROUTED TO	Q1-R	117.	6.42	33.	14.	14.	2.50
+	HYDROGRAPH AT	W36B	10.	6.00	2.	1.	1.	.19
+	HYDROGRAPH AT	W35A	7.	5.92	1.	1.	1.	.10
+	ROUTED TO	35A-WR	6.	6.25	1.	1.	1.	.10
+	HYDROGRAPH AT	W35B	9.	6.00	2.	1.	1.	.15
+	4 COMBINED AT	WR	131.	6.42	38.	17.	16.	2.94
+	ROUTED TO	WR-S	130.	6.50	38.	16.	16.	2.94
+	HYDROGRAPH AT	W37A	8.	5.92	1.	1.	1.	.11
+	ROUTED TO	37A-S	8.	6.00	1.	1.	1.	.11
+	HYDROGRAPH AT	W37B	13.	5.92	2.	1.	1.	.16
+	3 COMBINED AT	WS	137.	6.50	42.	18.	17.	3.21
+	ROUTED TO	S-T	134.	6.67	42.	18.	17.	3.21
+	HYDROGRAPH AT	W38	10.	5.92	2.	1.	1.	.09
+	ROUTED TO		9.	6.08	2.	1.	1.	.09
+	HYDROGRAPH AT	W39	11.	6.00	2.	1.	1.	.18
	HYDROGRAPH AT							

+		W40	7.	5.83	1.	1.	1.	.10
+	4 COMBINED AT	WT	143.	6.58	46.	20.	19.	3.59
+	ROUTED TO	T-U	142.	6.67	46.	20.	19.	3.59
+	HYDROGRAPH AT	W41	6.	5.83	1.	0.	0.	.06
+	HYDROGRAPH AT	W42	50.	5.75	5.	2.	2.	.06
+	ROUTED TO	U-V	49.	5.83	5.	2.	2.	.06
+	3 COMBINED AT	WU	148.	6.67	51.	22.	21.	3.70
+	ROUTED TO		146.	6.75	51.	22.	21.	3.70
+	HYDROGRAPH AT	W43	13.	5.83	2.	1.	1.	.15
+	2 COMBINED AT	WV	149.	6.75	53.	23.	22.	3.85
+	ROUTED TO	V-W	148.	6.75	53.	23.	22.	3.85
+	HYDROGRAPH AT	W45	17.	5.92	3.	1.	1.	.19
+	2 COMBINED AT	WW	152.	6.75	56.	24.	23.	4.04
+	ROUTED TO	W-X	149.	6.83	55.	24.	23.	4.04
+	HYDROGRAPH AT	M1	7.	5.83	1.	0.	0.	.07
+	ROUTED TO		6.	5.83	1.	0.	0.	.07
+	HYDROGRAPH AT	M2	3.	5.83	0.	0.	0.	.03
+	2 COMBINED AT	MB	9.	5.83	1.	1.	0.	.09
+	ROUTED TO		8.	5.92	1.	1.	0.	.09
+	HYDROGRAPH AT	M4	3.	5.83	0.	0.	0.	.03
+	ROUTED TO		3.	5.83	0.	0.	0.	.03
+	HYDROGRAPH AT	M3	2.	5.75	0.	0.	0.	.01
+	3 COMBINED AT	MC	11.	5.83	2.	1.	1.	.14
+	ROUTED TO		11.	6.00	2.	1.	1.	.14
+	HYDROGRAPH AT	M5	6.	5.75	1.	0.	0.	.02
+	ROUTED TO		6.	5.92	1.	0.	0.	.02
+	HYDROGRAPH AT	M6	10.	5.92	2.	1.	1.	.06
+	3 COMBINED AT	MD	26.	5.92	4.	2.	2.	.22
+	ROUTED TO		25.	6.08	4.	2.	2.	.22
+	HYDROGRAPH AT	M7	16.	5.83	2.	1.	1.	.05
+	ROUTED TO		15.	5.92	2.	1.	1.	.05
+	HYDROGRAPH AT	M8	4.	5.83	1.	0.	0.	.04

+	2 COMBINED AT	ME	18.	5.83	2.	1.	1.	.09		
	ROUTED TO		18.	6.00	2.	1.	1.	.09		
+	HYDROGRAPH AT	M9	7.	5.75	1.	0.	0.	.02		
	ROUTED TO		6.	5.92	1.	0.	0.	.02		
+	HYDROGRAPH AT	M12A	5.	5.83	1.	0.	0.	.07		
+	HYDROGRAPH AT	M12B	10.	5.92	2.	1.	1.	.15		
+	5 COMBINED AT	MF	59.	6.00	10.	4.	4.	.54		
	ROUTED TO		57.	6.08	10.	4.	4.	.54		
+	HYDROGRAPH AT	M13	10.	5.83	1.	1.	0.	.06		
	ROUTED TO		9.	6.00	1.	1.	0.	.06		
+	HYDROGRAPH AT	M14	22.	5.92	4.	1.	1.	.16		
+	2 COMBINED AT	MG	30.	5.92	5.	2.	2.	.22		
	ROUTED TO	PONDW	9.	6.50	5.	2.	2.	.22	969.11	6.50
+	2 COMBINED AT	MH	64.	6.08	14.	6.	5.	.77		
	ROUTED TO		62.	6.17	14.	6.	5.	.77		
+	DIVERSION TO	DIVRT1	61.	6.17	14.	6.	5.	.77		
+	HYDROGRAPH AT	MH-P2	1.	6.17	0.	0.	0.	.77		
+	HYDROGRAPH AT	M15	18.	5.92	3.	1.	1.	.12		
+	2 COMBINED AT	MI	18.	5.92	3.	1.	1.	.89		
	ROUTED TO		17.	6.00	3.	1.	1.	.89		
+	HYDROGRAPH AT	M19	5.	5.83	1.	0.	0.	.05		
+	2 COMBINED AT	MJ	20.	6.00	3.	1.	1.	.94		
	ROUTED TO		20.	6.08	3.	1.	1.	.94		
+	HYDROGRAPH AT	M10	8.	5.83	1.	0.	0.	.06		
	ROUTED TO	M10-K	8.	5.92	1.	0.	0.	.06		
+	HYDROGRAPH AT	M11A	8.	5.92	2.	1.	1.	.11		
+	2 COMBINED AT	MK	16.	5.92	3.	1.	1.	.16		
	ROUTED TO	MK-K1	16.	6.00	3.	1.	1.	.16		
+	HYDROGRAPH AT	M11B	7.	5.83	1.	0.	0.	.09		
	ROUTED TO	11B-K1	7.	6.00	1.	0.	0.	.09		
+	HYDROGRAPH AT	M11C	7.	5.83	1.	1.	0.	.09		
	3 COMBINED AT									

+		MK1	27.	6.00	5.	2.	2.	.35
	ROUTED TO							
+		K1-ML	26.	6.00	5.	2.	2.	.35
	HYDROGRAPH AT							
+		M16	4.	5.83	1.	0.	0.	.04
	2 COMBINED AT							
+		ML	28.	6.00	5.	2.	2.	.39
	ROUTED TO							
+			28.	6.08	5.	2.	2.	.39
	HYDROGRAPH AT							
+		M17	9.	5.83	1.	0.	0.	.08
	2 COMBINED AT							
+		MM	31.	6.08	7.	3.	3.	.46
	ROUTED TO							
+			29.	6.17	7.	3.	3.	.46
	HYDROGRAPH AT							
+		M18	7.	5.83	1.	0.	0.	.06
	ROUTED TO							
+			6.	6.00	1.	0.	0.	.06
	HYDROGRAPH AT							
+		M20	11.	5.92	2.	1.	1.	.13
	4 COMBINED AT							
+		MN	57.	6.17	13.	5.	5.	1.60
	ROUTED TO							
+			55.	6.25	13.	5.	5.	1.60
	HYDROGRAPH AT							
+		M21	3.	5.83	0.	0.	0.	.02
	ROUTED TO							
+			3.	5.92	0.	0.	0.	.02
	HYDROGRAPH AT							
+		M23	4.	5.83	1.	0.	0.	.05
	3 COMBINED AT							
+		MO	57.	6.25	14.	6.	5.	1.67
	ROUTED TO							
+			56.	6.25	14.	6.	5.	1.67
	HYDROGRAPH AT							
+		M24	7.	5.83	1.	0.	0.	.08
	2 COMBINED AT							
+		MP	58.	6.25	15.	6.	6.	1.75
	ROUTED TO							
+			57.	6.25	15.	6.	6.	1.75
	HYDROGRAPH AT							
+		M25	1.	5.83	0.	0.	0.	.01
	2 COMBINED AT							
+		MQ	58.	6.25	15.	6.	6.	1.76
	ROUTED TO							
+			57.	6.42	15.	6.	6.	1.76
	HYDROGRAPH AT							
+		M26	26.	5.92	4.	2.	2.	.18
	2 COMBINED AT							
+		MR	66.	6.42	18.	8.	7.	1.94
	HYDROGRAPH AT							
+		W44	3.	5.83	1.	0.	0.	.04
	ROUTED TO							
+			3.	6.00	1.	0.	0.	.04
	HYDROGRAPH AT							
+		W47	4.	5.83	1.	0.	0.	.05
	ROUTED TO							
+			4.	5.92	1.	0.	0.	.05
	HYDROGRAPH AT							
+		W46	4.	5.83	1.	0.	0.	.04
	HYDROGRAPH AT							
+		M27	5.	5.83	1.	0.	0.	.05

+	6 COMBINED AT	WX	192.	6.75	76.	33.	31.	6.17
	ROUTED TO		191.	6.83	76.	32.	31.	6.17
+	HYDROGRAPH AT	W48	14.	5.83	2.	1.	1.	.12
+	ROUTED TO		14.	5.92	2.	1.	1.	.12
+	HYDROGRAPH AT	W49	24.	5.92	4.	2.	2.	.27
+	3 COMBINED AT	WZ	199.	6.83	81.	35.	34.	6.55
+	ROUTED TO		196.	6.92	81.	35.	34.	6.55
+	HYDROGRAPH AT	W50	11.	5.83	2.	1.	1.	.11
+	2 COMBINED AT	WAB	198.	6.92	83.	35.	34.	6.66
+	ROUTED TO		198.	6.92	83.	35.	34.	6.66
+	HYDROGRAPH AT	W51	7.	5.83	1.	0.	0.	.05
+	2 COMBINED AT	WAC	199.	6.92	83.	36.	34.	6.71
+	ROUTED TO		198.	6.92	83.	36.	34.	6.71
+	HYDROGRAPH AT	W52	9.	5.83	1.	0.	0.	.05
+	ROUTED TO		8.	5.83	1.	0.	0.	.05
+	HYDROGRAPH AT	W53	8.	5.83	1.	0.	0.	.05
+	2 COMBINED AT	WAD	16.	5.83	2.	1.	1.	.10
+	ROUTED TO		14.	5.83	2.	1.	1.	.10
+	HYDROGRAPH AT	W54	1.	5.75	0.	0.	0.	.01
+	3 COMBINED AT	WAE	200.	6.92	85.	37.	35.	6.82
+	ROUTED TO		199.	7.00	85.	36.	35.	6.82
+	HYDROGRAPH AT	W56	13.	5.92	2.	1.	1.	.18
+	2 COMBINED AT	WAF	202.	7.00	87.	37.	36.	7.01
+	ROUTED TO		200.	7.00	87.	37.	36.	7.01
+	HYDROGRAPH AT	W62	7.	5.83	1.	0.	0.	.08
+	ROUTED TO		7.	5.92	1.	0.	0.	.08
+	HYDROGRAPH AT	W63	5.	5.83	1.	0.	0.	.05
+	ROUTED TO		4.	5.92	1.	0.	0.	.05
+	HYDROGRAPH AT	W61	11.	6.00	3.	1.	1.	.19
+	3 COMBINED AT	WAH	22.	5.92	4.	2.	2.	.31
+	ROUTED TO		21.	6.00	4.	2.	2.	.31
+	HYDROGRAPH AT	W57	6.	5.83	1.	0.	0.	.07

+	ROUTED TO		6.	6.25	1.	0.	0.	.07		
+	HYDROGRAPH AT	W58	13.	6.00	3.	1.	1.	.23		
+	3 COMBINED AT	WAI	35.	6.00	8.	3.	3.	.62		
+	ROUTED TO		34.	6.00	8.	3.	3.	.62		
+	HYDROGRAPH AT	E1A	7.	5.92	2.	1.	1.	.12		
+	ROUTED TO	E1A-EA	7.	6.17	1.	1.	1.	.12		
+	HYDROGRAPH AT	E1B	10.	5.92	2.	1.	1.	.17		
+	2 COMBINED AT	EA	13.	6.17	4.	2.	1.	.28		
+	ROUTED TO	EA-EB	13.	6.25	4.	2.	1.	.28		
+	HYDROGRAPH AT	E2	9.	5.83	1.	1.	1.	.10		
+	2 COMBINED AT	EB	16.	6.25	5.	2.	2.	.39		
+	ROUTED TO	POND1	2.	14.92	2.	1.	1.	.39	952.08	14.92
+	ROUTED TO		2.	15.00	2.	1.	1.	.39		
+	HYDROGRAPH AT	E3	8.	5.83	1.	0.	0.	.09		
+	HYDROGRAPH AT	MH-P2	61.	6.17	14.	6.	5.	.00		
+	3 COMBINED AT	EC	64.	6.17	15.	7.	7.	.48		
+	ROUTED TO	POND2	6.	19.83	6.	2.	2.	.48	929.13	19.83
+	ROUTED TO		6.	20.00	6.	2.	2.	.48		
+	HYDROGRAPH AT	E1C	6.	5.92	1.	0.	0.	.08		
+	ROUTED TO	1C-ED1	5.	6.17	1.	0.	0.	.08		
+	HYDROGRAPH AT	E4	9.	5.92	2.	1.	1.	.13		
+	2 COMBINED AT	ED1	10.	6.17	3.	1.	1.	.21		
+	ROUTED TO	ED1-ED	10.	6.17	3.	1.	1.	.21		
+	HYDROGRAPH AT	E5	7.	5.83	1.	1.	0.	.09		
+	3 COMBINED AT	ED	15.	5.92	7.	4.	3.	.78		
+	ROUTED TO		14.	5.92	7.	4.	3.	.78		
+	HYDROGRAPH AT	E8	4.	5.83	1.	0.	0.	.04		
+	2 COMBINED AT	EE	18.	5.92	7.	4.	4.	.83		
+	ROUTED TO		17.	6.00	7.	4.	4.	.83		
+	HYDROGRAPH AT	E10	2.	5.83	0.	0.	0.	.03		
+	2 COMBINED AT	EF	19.	6.00	7.	4.	4.	.85		

+	ROUTED TO	F-G	18.	6.08	7.	4.	4.	.85
+	HYDROGRAPH AT	E6	7.	5.92	2.	1.	1.	.12
+	HYDROGRAPH AT	E7	3.	5.75	0.	0.	0.	.03
+	ROUTED TO		3.	5.92	0.	0.	0.	.03
+	2 COMBINED AT	EG1	10.	5.92	2.	1.	1.	.15
+	ROUTED TO	G1-G	10.	6.00	2.	1.	1.	.15
+	HYDROGRAPH AT	E9	5.	5.92	1.	0.	0.	.08
+	ROUTED TO		5.	6.00	1.	0.	0.	.08
+	HYDROGRAPH AT	E11	3.	5.92	1.	0.	0.	.05
+	HYDROGRAPH AT	E12	7.	5.83	1.	1.	0.	.09
+	5 COMBINED AT	EG	37.	6.08	10.	6.	6.	1.22
+	HYDROGRAPH AT	E13	1.	6.00	0.	0.	0.	.02
+	HYDROGRAPH AT	E14	0.	5.83	0.	0.	0.	.01
+	ROUTED TO		0.	5.92	0.	0.	0.	.01
+	3 COMBINED AT	EH	39.	6.00	10.	6.	6.	1.24
+	ROUTED TO		39.	6.17	10.	6.	6.	1.24
+	HYDROGRAPH AT	E19	6.	5.83	1.	0.	0.	.04
+	2 COMBINED AT	EJ1	39.	6.17	10.	6.	6.	1.28
+	ROUTED TO	J1-K	39.	6.25	10.	6.	6.	1.28
+	HYDROGRAPH AT	E15	6.	5.83	1.	0.	0.	.04
+	ROUTED TO		6.	5.83	1.	0.	0.	.04
+	HYDROGRAPH AT	E16	5.	5.83	1.	0.	0.	.03
+	2 COMBINED AT	EI	11.	5.83	1.	0.	0.	.07
+	ROUTED TO		10.	5.92	1.	0.	0.	.07
+	HYDROGRAPH AT	E17	5.	5.83	1.	0.	0.	.03
+	ROUTED TO		5.	5.92	1.	0.	0.	.03
+	HYDROGRAPH AT	E18	6.	5.92	1.	0.	0.	.05
+	3 COMBINED AT	EJ2	22.	5.92	3.	1.	1.	.15
+	ROUTED TO		19.	6.25	3.	1.	1.	.15
+	HYDROGRAPH AT	E23	15.	5.92	3.	1.	1.	.17
+	HYDROGRAPH AT	E24	11.	6.08	3.	1.	1.	.14
	4 COMBINED AT							

+		EK	76.	6.25	18.	10.	9.	1.74
	ROUTED TO							
+			69.	6.42	18.	9.	9.	1.74
	HYDROGRAPH AT							
+		E21	6.	5.92	1.	0.	0.	.09
	ROUTED TO							
+			6.	6.00	1.	0.	0.	.09
	HYDROGRAPH AT							
+		E20	8.	5.92	1.	1.	1.	.08
	ROUTED TO							
+			7.	6.00	1.	1.	1.	.08
	HYDROGRAPH AT							
+		E22	5.	5.92	1.	0.	0.	.07
	3 COMBINED AT							
+		EL	18.	6.00	3.	1.	1.	.23
	ROUTED TO							
+			17.	6.08	3.	1.	1.	.23
	HYDROGRAPH AT							
+		E25	15.	5.92	3.	1.	1.	.17
	3 COMBINED AT							
+		EM	85.	6.33	24.	12.	11.	2.13
	ROUTED TO							
+			84.	6.42	24.	12.	11.	2.13
	HYDROGRAPH AT							
+		E26	6.	5.83	1.	0.	0.	.04
	2 COMBINED AT							
+		EN	85.	6.42	25.	12.	12.	2.17
	ROUTED TO							
+			84.	6.50	24.	12.	12.	2.17
	HYDROGRAPH AT							
+		E27	17.	5.83	2.	1.	1.	.12
	2 COMBINED AT							
+		EO	87.	6.50	26.	13.	13.	2.29
	ROUTED TO							
+			84.	6.58	26.	13.	12.	2.29
	HYDROGRAPH AT							
+		W55	5.	5.83	1.	0.	0.	.05
	2 COMBINED AT							
+		WAG	85.	6.58	27.	13.	13.	2.34
	ROUTED TO							
+			84.	6.58	27.	13.	13.	2.34
	HYDROGRAPH AT							
+		W59	5.	5.92	1.	0.	0.	.07
	4 COMBINED AT							
+		WAJ	269.	7.00	122.	54.	52.	10.03
	ROUTED TO							
+			267.	7.00	122.	54.	52.	10.03
	HYDROGRAPH AT							
+		E28	6.	5.92	1.	0.	0.	.07
	ROUTED TO							
+			6.	6.17	1.	0.	0.	.07
	HYDROGRAPH AT							
+		E29	4.	5.83	1.	0.	0.	.05
	2 COMBINED AT							
+		EZZ	7.	6.17	2.	1.	1.	.12
	HYDROGRAPH AT							
+		W60	5.	5.92	1.	0.	0.	.07
	3 COMBINED AT							
+		ZZ	271.	7.00	125.	55.	53.	10.22

*** NORMAL END OF HEC-1 ***

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 28SEP07 TIME 11:49:21
*
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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1      ID      FALCON BASIN 100-YR/ 24-HOUR FLOOD/ EXISTING CONDITIONS
2      ID      UPPER EAST TRIBUTARY (WOODMEN HILLS) BASED ON CLOMR APPROVED 2/2/99
3      ID      INCLUDING 2 EXISTING SCS STOCK PONDS, WEST WOODMEN HILLS POND
4      ID      NOTE: M1-M4 (PAINT BRUSH HILLS) MODELED AS HISTORIC TO ACCOUNT FOR
5      ID      DETENTION POND AT MC
6      ID      NOTE: NO CULVERT AT STAPLETON & MERIDIAN, TEMP CULVERTS AT MERIDIAN
7      ID      DOWNSTREAM OF WOODMEN HILLS DRIVE (DIVERSION)
8      *DIAGRAM
9      IT      5 14JUL99      800      300
10     IO      5
11
12     KK      W1
13     KM
14     BA      .0479
15     PB      4.4
16     IN      15
17     PC      .0005 .0015 .0030 .0045 .0060 .0080 .0100 .0120 .0143 .0165
18     PC      .0188 .0210 .0233 .0255 .0278 .0320 .0390 .0460 .0530 .0600
19     PC      .0750 .1000 .4000 .7000 .7250 .7500 .7650 .7800 .7900 .8000
20     PC      .8100 .8200 .8250 .8300 .8350 .8400 .8450 .8500 .8550 .8600
21     PC      .8638 .8675 .8713 .8750 .8788 .8825 .8863 .8900 .8938 .8975
22     PC      .9013 .9050 .9083 .9115 .9148 .9180 .9210 .9240 .9270 .9300
23     PC      .9325 .9350 .9375 .9400 .9425 .9450 .9475 .9500 .9525 .9550
24     PC      .9575 .9600 .9625 .9650 .9675 .9700 .9725 .9750 .9775 .9800
25     PC      .9813 .9825 .9838 .9850 .9863 .9875 .9888 .9900 .9913 .9925
26     PC      .9938 .9950 .9963 .9975 .9988 1.000
27     LS      60
28     UD      .097
29
30     KK      W2
31     KM
32     BA      .0278
33     LS      60
34     UD      .160
35
36     KK      WA
37     KM
38     HC      2
39
40     KK
41     KM
42     RK      464 .0151 .035      TRAP      5      4
43
44     KK      W3
45     KM
46     BA      .0498
47     LS      61
48     UD      .139
49
50     KK      WB
51     KM
52     HC      2
53
54     KK
55     KM
56     RK      823 .0279 .035      TRAP      5      4      54
57
58     KK      W4
59     KM
60     BA      .0054
61     LS      62
62     UD      .044
63
64     KK
65     KM
66     RK      1078 .0482 .035      TRAP      5      4
67
68     KK      W5
69     KM
70     BA      .0159
71     LS      60
72     UD      .075
73
74     KK      WC
75     KM
76     HC      3
77
78     KK
79     KM
80     RK      557 .0449 .035      TRAP      10     4
81
82     KK      W6
83     KM
84     BA      .0486
85     LS      60
86     UD      .085
87
88     KK
89     KM
90     RK      592 .0372 .035      TRAP      5      4

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79	KK	W7						
80	KM							
81	BA	.0217						
82	LS		60					
83	UD	.074						
84	KK							
85	KM							
86	RK	464	.1466	.035	TRAP	5	4	
87	KK	WD						
88	KM							
89	HC	2						
90	KK	D-E						
91	KM							
92	RK	1044	.0479	.035	TRAP	5	4	
93	KK	W8						
94	KM							
95	BA	.0286						
96	LS		60					
97	UD	.069						
98	KK							
99	KM							
100	RK	1449	.0504	.035	TRAP	5	4	
101	KK	W9						
102	KM							
103	BA	.0402						
104	LS		61					
105	UD	.097						
106	KK	WE						
107	KM							
108	HC	3						
109	KK	E-F						
110	KM							
111	RK	789	.0038	.035	TRAP	5	4	
112	KK	W10						
113	KM							
114	BA	.0431						
115	LS		61					
116	UD	.096						
117	KK							
118	KM							
119	RK	824	.0388	.035	TRAP	5	4	
120	KK	W11						
121	KM							
122	BA	.0314						
123	LS		60					
124	UD	.077						
125	KK	WF						
126	KM							
127	HC	4						
128	KK	F-G						
129	KM							
130	RK	2319	.0211	.035	TRAP	10	4	
131	KK	W12						
132	KM							
133	BA	.0398						
134	LS		60					
135	UD	.095						
136	KK							
137	KM							
138	RK	2478	.0307	.035	TRAP	5	4	
139	KK	W14						
140	KM							
141	BA	.0473						
142	LS		61					
143	UD	.135						
144	KK							
145	KM							
146	RK	81	0.0001	.035	TRAP	5	4	
147	KK	W13						
148	KM							
149	BA	.1123						
150	LS		61					
151	UD	.182						
152	KK	WG						
153	KM							
154	HC	4						

155	KK	G-H					
156	KM						
157	RK	2632	.0217	.035	TRAP	15	4
158	KK						
159	KM						
160	RK	2447	.0372	.035	TRAP	5	4
161	KK	W15					
162	KM						
163	BA	.0881					
164	LS		61				
165	UD	.141					
166	KK						
167	KM						
168	RK	1763	.0289	.035	TRAP	5	4
169	KK	WH					
170	KM						
171	HC	2					
172	KK	W16					
173	KM						
174	BA	.0292					
175	LS		61				
176	UD	.092					
177	KK						
178	KM						
179	RK	1345	.0260	.035	TRAP	5	4
180	KK	W17					
181	KM						
182	BA	.0184					
183	LS		60				
184	UD	.085					
185	KK	WI					
186	KM						
187	HC	2					
188	KK	I-M					
189	KM						
190	RK	2650	.0370	.035	TRAP	15	4
191	KK	W19					
192	KM						
193	BA	.0428					
194	LS		61				
195	UD	.083					
196	KK						
197	KM						
198	RK	881	.0329	.035	TRAP	5	4
199	KK	W20					
200	KM						
201	BA	.0315					
202	LS		61				
203	UD	.071					
204	KK	WJ					
205	KM						
206	HC	2					
207	KK						
208	KM						
209	RK	3061	.0235	.035	TRAP	5	4
210	KK	W21					
211	KM						
212	BA	.1347					
213	LS		60				
214	UD	.156					
215	KK	WK					
216	KM						
217	HC	2					
218	KK						
219	KM						
220	RK	487	.0246	.035	TRAP	5	4
221	KK	W22					
222	KM						
223	BA	.0086					
224	LS		63				
225	UD	.055					
226	KK	WL					
227	KM						
228	HC	2					

229	KK						
230	KM						
231	RK	1786	.0297	.035	TRAP	5	4
232	KK	W23					
233	KM						
234	BA	.0244					
235	LS		60				
236	UD	.112					
237	KK	W18					
238	KM						
239	BA	.1251					
240	LS		60				
241	UD	.189					
242	KK	WM					
243	KM						
244	HC	5					
245	KK	M-N					
246	KM						
247	RK	1345	.0149	.035	TRAP	20	4
248	KK	W24					
249	KM						
250	BA	.0442					
251	LS		60				
252	UD	.140					
253	KK	W25					
254	KM						
255	BA	.0957					
256	LS		61				
257	UD	.197					
258	KK	WN					
259	KM						
260	HC	3					
261	KK	N-P					
262	KM						
263	RK	1589	.017	.035	TRAP	20	4
264	KK	W28					
265	KM						
266	BA	.0397					
267	LS		63				
268	UD	.128					
269	KK						
270	KM						
271	RK	1345	.0208	.035	TRAP	5	4
272	KK	W30					
273	KM						
274	BA	.0509					
275	LS		63				
276	UD	.123					
277	KK						
278	KM						
279	RK	1078	.0074	.035	TRAP	5	4
280	KK	W29					
281	KM						
282	BA	.0409					
283	LS		63				
284	UD	.145					
285	KK	W31					
286	KM						
287	BA	.0123					
288	LS		63				
289	UD	.073					
290	KK	W0					
291	KM						
292	HC	4					
293	KK	O-P					
294	KM						
295	RK	2169	.0226	.035	TRAP	5	4
296	KK	W26					
297	KM						
298	BA	.0301					
299	LS		63				
300	UD	.062					
301	KK						
302	KM						
303	RK	4662	.0225	.035	TRAP	5	4
304	KK	W27					

305	KM						
306	BA	.1633					
307	LS		60				
308	UD	.253					
309	KK	W32					
310	KM						
311	BA	.0890					
312	LS		60				
313	UD	.170					
314	KK	WP					
315	KM						
316	HC	5					
317	KK	P-Q					
318	KM						
319	RK	1925	.0182	.035	TRAP	25	4
320	KK	W33A					
321	KM						
322	BA	.1261					
323	LS		60				
324	UD	.186					
325	KK	WP1					
326	KM						
327	HC	2					
328	KK	P1-Q					
329	KM						
330	RK	3000	.020	.035	TRAP	25	4
331	KK	W33B					
332	KM						
333	BA	.1360					
334	LS		60				
335	UD	.225					
336	KK	W34A					
337	KM						
338	BA	.1261					
339	LS		60				
340	UD	.173					
341	KK	34A-P2					
342	KM						
343	RK	2550	.0176	.035	TRAP	25	4
344	KK	W34B					
345	KM						
346	BA	.1766					
347	LS		60				
348	UD	.224					
349	KK	WP2					
350	KM						
351	HC	2					
352	KK	P2-Q					
353	KM						
354	RK	2640	.021	.035	TRAP	25	4
355	KK	W34C					
356	KM						
357	BA	.1625					
358	LS		60				
359	UD	.244					
360	KK	WQ					
361	KM						
362	HC	4					
363	KK	Q-Q1					
364	KM						
365	RK	2940	.022	.035	TRAP	25	4
366	KK	W36A					
367	KM						
368	BA	.1429					
369	LS		60				
370	UD	.234					
371	KK	WQ1					
372	KM						
373	HC	2					
374	KK	Q1-R					
375	KM						
376	RK	3400	.022	.035	TRAP	25	4
377	KK	W36B					
378	KM						
379	BA	.1918					
380	LS		60				

381	UD	.306					
382	KK	W35A					
383	KM						
384	BA	.0958					
385	LS		60				
386	UD	.187					
387	KK	35A-WR					
388	KM						
389	RK	3715	.023	.035	TRAP	25	4
390	KK	W35B					
391	KM						
392	BA	.1507					
393	LS	0	60				
394	UD	.259					
395	KK	WR					
396	KM						
397	HC	4					
398	KK	WR-S					
399	KM						
400	RK	2922	.0168	.035	TRAP	25	4
401	KK	W37A					
402	KM						
403	BA	.1138					
404	LS		60				
405	UD	.185					
406	KK	37A-S					
407	KM						
408	RK	1430	.014	.035	TRAP	25	4
409	KK	W37B					
410	KM						
411	BA	.1636					
412	LS		61				
413	UD	.218					
414	KK	WS					
415	KM						
416	HC	3					
417	KK	S-T					
418	KM						
419	RK	3653	.0164	.035	TRAP	25	4
420	KK	W38					
421	KM						
422	BA	.0907					
423	LS		62				
424	UD	.190					
425	KK						
426	KM						
427	RK	2922	.0171	.035	TRAP	5	4
428	KK	W39					
429	KM						
430	BA	.1833					
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432	UD	.251					
433	KK	W40					
434	KM						
435	BA	.0964					
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437	UD	.165					
438	KK	WT					
439	KM						
440	HC	4					
441	KK	T-U					
442	KM						
443	RK	1125	.0098	.035	TRAP	25	4
444	KK	W41					
445	KM						
446	BA	.0601					
447	LS		60				
448	UD	.117					
449	KK	W42					
450	KM						
451	BA	.0581					
452	LS		81				
453	UD	.127					
454	KK	U-V					
455	KM						
456	RK	2656	.0184	.035	TRAP	5	4

457	KK	WU							
458	KM								
459	HC	3							
460	KK								
461	KM								
462	RK	2215	.0181	.035	TRAP	25	4		
463	KK	W43							
464	KM								
465	BA	.1457							
466	LS		61						
467	UD	.169							
468	KK	WV							
469	KM								
470	HC	2							
471	KK	V-W							
472	KM								
473	RK	487	.0103	.035	TRAP	25	4		
474	KK	W45							
475	KM								
476	BA	.1931							
477	LS		61						
478	UD	.189							
479	KK	WW							
480	KM								
481	HC	2							
482	KK	W-X							
483	KM								
484	RK	1542	.0149	.035	TRAP	5	4		
485	KK	M1							
486	KM								
487	BA	.0665							
488	LS		60						
489	UD	.108							
490	KK								
491	KM								
492	RK	650	.0308	.035	TRAP	5	4		
493	KK	M2							
494	KM								
495	BA	.0273							
496	LS		60						
497	UD	.114							

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HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

498	KK	MB							
499	KM								
500	HC	2							
501	KK								
502	KM								
503	RK	928	.0302	.035	TRAP	5	4		
504	KK	M4							
505	KM								
506	BA	.0346							
507	LS		60						
508	UD	.121							
509	KK								
510	KM								
511	RK	406	.0197	.02	TRAP	40	0		
512	KK	M3							
513	KM								
514	BA	.0149							
515	LS		60						
516	UD	.076							
517	KK	MC							
518	KM								
519	HC	3							
520	KK								
521	KM								
522	RK	1902	.0231	.035	TRAP	5	4		
523	KK	M5							
524	KM								
525	BA	.0176							
526	LS		69						
527	UD	.108							

528	KK									
529	KM									
530	RK	1717	.0186	.02	TRAP	40	0			
531	KK	M6								
532	KM									
533	BA	.0637								
534	LS		65							
535	UD	.233								
536	KK	MD								
537	KM									
538	HC	3								
539	KK									
540	KM									
541	RK	2841	.019	.035	TRAP	5	4			
542	KK	M7								
543	KM									
544	BA	.0524								
545	LS		69							
546	UD	.170								
547	KK									
548	KM									
549	RK	1044	.0268	.02	TRAP	40	0			
550	KK	M8								
551	KM									
552	BA	.0370								
553	LS		61							
554	UD	.126								
555	KK	ME								
556	KM									
557	HC	2								
558	KK									
559	KM									
560	RK	2992	.0187	.035	TRAP	5	4			
561	KK	M9								
562	KM									
563	BA	.0169								
564	LS		69							
565	UD	.087								
566	KK									
567	KM									
568	RK	3433	.0253	.03	TRAP	5	4			
569	KK	M12A								
570	KM									
571	BA	.0658								
572	LS		60							
573	UD	.159								
574	KK	M12B								
575	KM									
576	BA	.1481								
577	LS		60							
579	KK	MF								
580	KM									
581	HC	5								
582	KK									
583	KM									
584	RK	2586	.0224	.035	TRAP	10	4			
585	KK	M13								
586	KM									
587	BA	.0614								
588	LS		64							
589	UD	.165								
590	KK									
591	KM									
592	RK	1700	.01	.035	TRAP	6	4			
593	KK	M14								
594	KM									
595	BA	.1624								
596	LS		64							
597	UD	.228								
598	KK	MG								
599	KM									
600	HC	2								
601	KK	PONDW								
602	KM	WOODMEN HILLS DETENTION POND WEST (FROM FDR WH FLG F4)								
603	SV	0	.68	1.5	235	3.6	4.9	6.3	7.34	7.34
604	SE	968	969	970	971	972	973	974	975	976

680	BA	.042					
681	LS		60				
682	UD	.139					
683	KK	ML					
684	KM						
685	HC	2					
686	KK						
687	KM						
688	RK	2099	.02	.035	TRAP	5	4
689	KK	M17					
690	KM						
691	BA	.0765					
692	LS		61				
693	UD	.133					
694	KK	MM					
695	KM						
696	HC	2					
697	KK						
698	KM						
699	RK	2320	.0121	.035	TRAP	10	4
700	KK	M18					
701	KM						
702	BA	.061					
703	LS		61				
704	UD	.142					
705	KK						
706	KM						
707	RK	2122	.017	.035	TRAP	5	4
708	KK	M20					
709	KM						
710	BA	.1341					
711	LS		61				
712	UD	.211					
713	KK	MN					
714	KM						
715	HC	4					
716	KK						
717	KM						
718	RK	1531	.0202	.035	TRAP	25	4
719	KK	M21					
720	KM						
721	BA	.0241					
722	LS		61				
723	UD	.125					
724	KK						
725	KM						
726	RK	1322	.0212	.035	TRAP	5	4
727	KK	M23					
728	KM						
729	BA	.0461					
730	LS		60				
731	UD	.120					
732	KK	MO					
733	KM						
734	HC	3					
735	KK						
736	KM						
737	RK	974	.0133	.035	TRAP	25	4
738	KK	M24					
739	KM						
740	BA	.0776					
741	LS		60				
742	UD	.125					
743	KK	MP					
744	KM						
745	HC	2					
746	KK						
747	KM						
748	RK	290	.0138	.035	TRAP	25	4
749	KK	M25					
750	KM						
751	BA	.0105					
752	LS		60				
753	UD	.130					
754	KK	MQ					

755	KM						
756	HC	2					
757	KK						
758	KM						
759	RK	3305	.0136	.035	TRAP	25	4
760	KK	M26					
761	KM						
762	BA	.1779					
763	LS		65				
764	UD	.250					
765	KK	MR					
766	KM						
767	HC	2					
768	KK	W44					
769	KM						
770	BA	.0384					
771	LS		60				
772	UD	.141					
773	KK						
774	KM						
775	RK	2029	.0148	.035	TRAP	5	4
776	KK	W47					
777	KM						
778	BA	.0541					
779	LS		60				
780	UD	.148					
781	KK						
782	KM						
783	RK	1438	.0223	.035	TRAP	5	4
784	KK	W46					
785	KM						
786	BA	.0418					
787	LS		61				
788	UD	.154					
789	KK	M27					
790	KM						
791	BA	.0528					
792	LS		60				
793	UD	.132					
794	KK	WX					
795	KM						
796	HC	6					
797	KK						
798	KM						
799	RK	2563	.0125	.035	TRAP	40	4
800	KK	W48					
801	KM						
802	BA	.1179					
803	LS		61				
804	UD	.091					
805	KK						
806	KM						
807	RK	2400	.0188	.035	TRAP	5	4
808	KK	W49					
809	KM						
810	BA	.2651					
811	LS		61				
812	UD	.181					
813	KK	WZ					
814	KM						
815	HC	3					
816	KK						
817	KM						
818	RK	800	.0125	.035	TRAP	40	4
819	KK	W50					
820	KM						
821	BA	.1061					
822	LS		61				
823	UD	.145					
824	KK	WAB					
825	KM						
826	HC	2					
827	KK						
828	KM						
829	RK	742	.0108	.035	TRAP	40	4

830	KK	W51						
831	KM							
832	BA	.0546						
833	LS		63					
834	UD	.172						
835	KK	WAC						
836	KM							
837	HC	2						
838	KK							
839	KM							
840	RK	638	.0345	.035	TRAP	40	4	
841	KK	W52						
842	KM							
843	BA	.0499						
844	LS		63					
845	UD	.109						
846	KK							
847	KM							
848	RK	1171	.0205	.035	TRAP	5	4	
849	KK	W53						
850	KM							
851	BA	.0531						
852	LS		63					
853	UD	.156						
854	KK	WAD						
855	KM							
856	HC	2						
857	KK							
858	KM							
859	RK	290	.0310	.035	TRAP	10	4	
860	KK	W54						
861	KM							
862	BA	.0078						
863	LS		60					
864	UD	.050						
865	KK	WAE						
866	KM							
867	HC	3						
868	KK							
869	KM							
870	RK	1925	.0052	.035	TRAP	40	4	
871	KK	W56						
872	KM							
873	BA	.1831						
874	LS		60					
875	UD	.191						
876	KK	WAF						
877	KM							
878	HC	2						
879	KK							
880	KM							
881	RK	1032	.0155	.035	TRAP	40	4	
882	KK	W62						
883	KM							
884	BA	.0750						
885	LS		60					
886	UD	.090						
887	KK							
888	KM							
889	RK	2169	.0203	.035	TRAP	5	4	
890	KK	W63						
891	KM							
892	BA	.047						
893	LS		60					
894	UD	.109						
895	KK							
896	KM							
897	RK	1450	.0131	.035	TRAP	5	4	
898	KK	W61						
899	KM							
900	BA	.192						
901	LS		60					
902	UD	.251						
903	KK	WAH						
904	KM							
905	HC	3						

906	KK								
907	KM								
908	RK	1241	.0153	.035	TRAP	5	4		
909	KK	W57							
910	KM								
911	BA	.0732							
912	LS		60						
913	UD	.140							
914	KK								
915	KM								
916	RK	5903	.0254	.035	TRAP	5	4		
917	KK	W58							
918	KM								
919	BA	.2296							
920	LS		60						
921	UD	.251							
922	KK	WAI							
923	KM								
924	HC	3							
925	KK								
926	KM								
927	RK	232	.0086	.035	TRAP	15	4		
928	KK	E1A							
929	KM								
930	BA	.1151							
931	LS	0	60						
932	UD	.234							
933	KK	E1A-EA							
934	KM								
935	RK	4000	.022	.035	TRAP	5	4		
936	KK	E1B							
937	KM								
938	BA	.1665							
939	LS	0	60						
940	UD	.233							
941	KK	EA							
942	KM								
943	HC	2							
944	KK	EA-EB							
945	KM								
946	RK	1900	.022	.035	TRAP	5	4		

1

HEC-1 INPUT

PAGE. 24

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

947	KK	E2							
948	KM								
949	BA	.104							
950	LS	0	60						
951	UD	.149							
952	KK	EB							
953	KM								
954	HC	2							
955	KK	POND1							
956	KM	ROUTE FLOW THROUGH SCS POND 1							
957	SV	0 .01 .28 1.12 2.70 5.18 6.00 6.94							
958	SE	945.5 946 948 950 952 954 954.5 955							
959	SQ	0 0 0 0 0 48.5 176.4 351.4							
960	RS	1 ELEV 945.5							
961	KK								
962	KM								
963	RK	1300	.0192	.035	TRAP	5	4		
964	KK	E3							
965	KM								
966	BA	.090							
967	LS		60						
968	UD	.128							
969	KK	MH-P2							
970	KM	RETRIEVE DIVERSION FROM W. MERIDIAN RD DITCH							
971	DR	DIVRT1							
972	KK	EC							
973	KM								
974	HC	3							
975	KK	POND2							
976	KM	ROUTE FLOW THROUGH SCS POND 2							
977	SV	0 .21 1.11 3.19 6.89 9.52 11.08 12.82 14.72 16.70							

978	SE	920	922	924	926	928	929	929.5	930	930.5	931
979	SQ	0	0	0	0	0	0	25	86.5	186.2	308.4
980	RS	1	ELEV	920							
981	KK										
982	KM										
983	RK	1700	.0141	.035		TRAP	5	4			
984	KK	E1C									
985	KM										
986	BA	.0845									
987	LS		60								
988	UD	.200									
989	KK	1C-ED1									
990	KM										
991	RK	3450	.022	.035		TRAP	5	4			
992	KK	E4									
993	KM										
994	BA	.127									
995	LS		60								
996	UD	.200									
997	KK	ED1									
998	KM										
999	HC	2									
1000	KK	ED1-ED									
1001	KM										
1002	RK	450	.0178	.03		TRAP	5	4			
1003	KK	E5									
1004	KM										
1005	BA	.094									
1006	LS		60								
1007	UD	.160									
1008	KK	ED									
1009	KM										
1010	HC	3									
1011	KK										
1012	KM										
1013	RK	950	.0211	.035		TRAP	10	4			
1014	KK	E8									
1015	KM										
1016	BA	.0446									
1017	LS		60								
1018	UD	.139									
1019	KK	EE									
1020	KM										
1021	HC	2									
1022	KK										
1023	KM										
1024	RK	1500	.0127	.035		TRAP	10	4			
1025	KK	E10									
1026	KM										
1027	BA	.029									
1028	LS		60								
1029	UD	.158									
1030	KK	EF									
1031	KM										
1032	HC	2									
1033	KK	F-G									
1034	KM										
1035	RK	950	.0074	.035		TRAP	15	4			
1036	KK	E6									
1037	KM										
1038	BA	.119									
1039	LS		60								
1040	UD	.228									
1041	KK	E7									
1042	KM										
1043	BA	.031									
1044	LS		60								
1045	UD	.082									
1046	KK										
1047	KM										
1048	RK	1100	.0100	.035		TRAP	5	4			
1049	KK	EG1									
1050	KM										
1051	HC	2									
1052	KK	G1-G									

1204	KM						
1205	HC	2					
1206	KK						
1207	KM						
1208	RK	1832	.0126	.035	TRAP	40	4
1209	KK	E27					
1210	KM						
1211	BA	.1236					
1212	LS		63				
1213	UD	.172					
1214	KK	E0					
1215	KM						
1216	HC	2					
1217	KK						
1218	KM						
1219	RK	1625	.0133	.035	TRAP	5	3
1220	KK	W55					
1221	KM						
1222	BA	.0452					
1223	LS		60				
1224	UD	.093					
1225	KK	WAG					
1226	KM						
1227	HC	2					
1228	KK						
1229	KM						
1230	RK	2025	.0109	.035	TRAP	5	4
1231	KK	W59					
1232	KM						
1233	BA	.0705					
1234	LS		60				
1235	UD	.200					
1236	KK	WAJ					
1237	KM						
1238	HC	4					
1239	KK						
1240	KM						
1241	RK	1450	.0124	.035	TRAP	40	4
1242	KK	E28					
1243	KM						
1244	BA	.0718					
1245	LS		61				
1246	UD	.223					
1247	KK						
1248	KM						
1249	RK	2064	.0165	.035	TRAP	40	4
1250	KK	E29					
1251	KM						
1252	BA	.0465					
1253	LS		61				
1254	UD	.166					
1255	KK	EZZ					
1256	KM	COMBINE E29 & E30 AT DP ZZ					
1257	HC	2					
1258	KK	W60					
1259	KM						
1260	BA	.0711					
1261	LS		60				
1262	UD	.182					
1263	KK	ZZ					
1264	KM	COMBINE ALL AT DP ZZ					
1265	HC	3					
1266	ZZ						

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	[<---] RETURN OF DIVERTED OR PUMPED FLOW
10	W1	
	V	
27	V	
	.	
30	.	W2
	.	
35	WA	
	V	
38	V	
	.	
41	.	W3
	.	
46	WB	
	V	
49	V	
	.	
52	.	W4
	.	V
57	.	V
	.	
60	.	W5
	.	
65	WC	
	V	
68	V	
	.	
71	.	W6
	.	V
76	.	V
	.	
79	.	W7
	.	V
84	.	V
	.	
87	WD	
	V	
90	V	
	D-E	
93	.	W8
	.	V
98	.	V
	.	
101	.	W9
	.	
106	WE	
	V	
109	V	
	E-F	
112	.	W10
	.	V
117	.	V
	.	
120	.	W11
	.	
125	WF	
	V	
128	V	
	F-G	
	.	
131	.	W12

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V
V
501 .
.
504 . M4
. V
. V
509 .
.
512 . M3
.
.
517 MC-----
. V
. V
520 .
.
523 . M5
. V
. V
528 .
.
531 . M6
.
.
536 MD-----
. V
. V
539 .
.
542 . M7
. V
. V
547 .
.
550 . M8
.
.
555 . ME-----
. V
. V
558 .
.
561 . M9
. V
. V
566 .
.
569 . M12A
.
.
574 . M12B
.
.
579 MF-----
. V
. V
582 .
.
585 . M13
. V
. V
590 .
.
593 . M14
.
.
598 . MG-----
. V
. V
601 . PONDW
.
.
607 . MH-----
. V
. V
610 .
.
615 -----> DIVRT1
613 MH-P2
.
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618 . M15
.

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623	MI		
	V			
	V			
626	.			
	.			
629	.	M19		
	.			
634	MJ		
	V			
	V			
637	.			
	.			
640	.	M10		
	.	V		
	.	V		
645	.	M10-K		
	.			
	.			
648	.		M11A	
	.			
	.			
653	.	MK	
	.	V		
	.	V		
656	.	MK-K1		
	.			
	.			
659	.		M11B	
	.		V	
	.		V	
664	.		11B-K1	
	.			
	.			
667	.			M11C
	.			
	.			
672	.	MK1	
	.	V		
	.	V		
675	.	K1-ML		
	.			
	.			
678	.		M16	
	.			
	.			
683	.	ML	
	.	V		
	.	V		
686	.			
	.			
689	.		M17	
	.			
	.			
694	.	MM	
	.	V		
	.	V		
697	.			
	.			
	.			
700	.		M18	
	.		V	
	.		V	
705	.			
	.			
	.			
708	.			M20
	.			
	.			
713	MN		
	V			
	V			
716	.			
	.			
	.			
719	.	M21		
	.	V		
	.	V		
724	.			
	.			
	.			
727	.		M23	
	.			
	.			
732	MO		
	V			
	V			
735	.			
	.			
	.			
738	.	M24		

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860      .           .           W54
      .           .           .
865      WAE.....
      V
      V
868      .
      .
871      .           W56
      .           .
876      WAF.....
      V
      V
879      .
      .
882      .           W62
      .           V
      .           V
887      .
      .
890      .           W63
      .           V
      .           V
895      .
      .
898      .           W61
      .           .
      .           .
903      WAH.....
      V
      V
906      .
      .
909      .           W57
      .           V
      .           V
914      .
      .
917      .           W58
      .           .
      .           .
922      WAI.....
      V
      V
925      .
      .
928      .           E1A
      .           V
      .           V
933      .           E1A-EA
      .           .
      .           .
936      .           E1B
      .           .
      .           .
941      .           EA.....
      .           V
      .           V
944      .           EA-EB
      .           .
      .           .
947      .           E2
      .           .
      .           .
952      .           EB.....
      .           V
      .           V
955      .           POND1
      .           V
      .           V
961      .
      .
964      .           E3
      .           .
      .           .
971      .           <----- DIVRT1
969      .           MH-P2
      .           .
      .           .
972      .           EC.....
      .           V
      .           V
975      .           POND2
      .           V
      .           V
981      .
      .

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1103	.	J1-K	.	.	.
1106	.	.	E15	.	.
	.	.	V	.	.
	.	.	V	.	.
1111
1114	.	.	.	E16	.

1119	.	.	EI
	.	.	V	.	.
	.	.	V	.	.
1122
1125	.	.	.	E17	.
	.	.	.	V	.
	.	.	.	V	.
1130
1133	E18

1138	.	.	EJ2
	.	.	V	.	.
	.	.	V	.	.
1141
1144	.	.	.	E23	.

1149	E24

1154	.	.	EK
	.	.	V	.	.
	.	.	V	.	.
1157
1160	.	.	E21	.	.
	.	.	V	.	.
	.	.	V	.	.
1165
1168	.	.	.	E20	.
	.	.	.	V	.
	.	.	.	V	.
1173
1176	E22

1181	.	.	EL
	.	.	V	.	.
	.	.	V	.	.
1184
1187	.	.	.	E25	.

1192	.	.	EM
	.	.	V	.	.
	.	.	V	.	.
1195
1198	.	.	.	E26	.

1203	.	.	EN
	.	.	V	.	.
	.	.	V	.	.
1206
1209	.	.	.	E27	.

1214	.	.	EO
	.	.	V	.	.
	.	.	V	.	.
1217
1220	.	.	.	W55	.

1225
	.	.	WAG.....	.
	.	.	V	.
1228	.	.	V	.

1231	.	.	.	W59

1236	WAJ.....	.	.	.
	V	.	.	.
	V	.	.	.
1239

1242	.	E28	.	.
	.	V	.	.
	.	V	.	.
1247

1250	.	.	E29	.

1255	.	EZZ.....	.	.

1258	.	.	W60	.

1263	ZZ.....	.	.	.

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION


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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 28SEP07 TIME 11:49:21
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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FALCON BASIN 100-YR/ 24-HOUR FLOOD/ EXISTING CONDITIONS
UPPER EAST TRIBUTARY (WOODMEN HILLS) BASED ON CLOMR APPROVED 2/2/99
INCLUDING 2 EXISTING SCS STOCK PONDS, WEST WOODMEN HILLS POND
NOTE: M1-M4 (PAINT BRUSH HILLS) MODELED AS HISTORIC TO ACCOUNT FOR
DETENTION POND AT MC
NOTE: NO CULVERT AT STAPLETON & MERIDIAN, TEMP CULVERTS AT MERIDIAN
DOWNSTREAM OF WOODMEN HILLS DRIVE (DIVERSION)

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9 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

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IT        HYDROGRAPH TIME DATA
          NMIN       5  MINUTES IN COMPUTATION INTERVAL
          IDATE      14JUL99 STARTING DATE
          ITIME      0800  STARTING TIME
          NQ         300  NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     15JUL99 ENDING DATE
          NDTIME     0855  ENDING TIME
          ICENT      19   CENTURY MARK

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          COMPUTATION INTERVAL .08 HOURS
          TOTAL TIME BASE     24.92 HOURS

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ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

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1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	W1	40.	5.75	4.	1.	1.	.05		
ROUTED TO		37.	5.83	4.	1.	1.			
HYDROGRAPH AT	W2	20.	5.83	2.	1.	1.	.03		
2 COMBINED AT		57.	5.83	6.	2.	2.			
ROUTED TO	WA	55.	5.83	6.	2.	2.	.08		
HYDROGRAPH AT		39.	5.83	4.	1.	1.			
2 COMBINED AT	WB	95.	5.83	10.	3.	3.	.13		
ROUTED TO		91.	5.83	10.	3.	3.			
HYDROGRAPH AT	W4	6.	5.75	0.	0.	0.	.01		
ROUTED TO		6.	5.75	0.	0.	0.			
HYDROGRAPH AT	W5	15.	5.75	1.	0.	0.	.02		
3 COMBINED AT		105.	5.83	11.	4.	4.			
ROUTED TO	WC	103.	5.83	11.	4.	4.	.15		
HYDROGRAPH AT		43.	5.75	4.	1.	1.			
ROUTED TO	W6	40.	5.75	4.	1.	1.	.05		
HYDROGRAPH AT		20.	5.75	2.	1.	1.			
ROUTED TO	W7	20.	5.75	2.	1.	1.	.02		
2 COMBINED AT		60.	5.75	5.	2.	2.			
ROUTED TO	WD	55.	5.75	5.	2.	2.	.07		
HYDROGRAPH AT		27.	5.75	2.	1.	1.			
ROUTED TO	W8	24.	5.75	2.	1.	1.	.03		
HYDROGRAPH AT		36.	5.75	3.	1.	1.			
3 COMBINED AT	WE	114.	5.75	11.	4.	4.	.14		
ROUTED TO		108.	5.83	11.	4.	4.			
HYDROGRAPH AT	W10	39.	5.75	3.	1.	1.	.04		
ROUTED TO		36.	5.75	4.	1.	1.			
HYDROGRAPH AT	W11	29.	5.75	2.	1.	1.	.03		
4 COMBINED AT		265.	5.83	28.	10.	9.			
ROUTED TO	WF								

+		F-G	255.	5.83	28.	10.	9.	.36
	HYDROGRAPH AT							
+		W12	33.	5.75	3.	1.	1.	.04
	ROUTED TO							
+			32.	5.83	3.	1.	1.	.04
	HYDROGRAPH AT							
+		W14	38.	5.83	4.	1.	1.	.05
	ROUTED TO							
+			37.	5.83	4.	1.	1.	.05
	HYDROGRAPH AT							
+		W13	80.	5.83	9.	3.	3.	.11
	4 COMBINED AT							
+		WG	403.	5.83	44.	15.	14.	.56
	ROUTED TO							
+		G-H	382.	5.92	44.	15.	14.	.56
	ROUTED TO							
+			368.	5.92	44.	15.	14.	.56
	HYDROGRAPH AT							
+		W15	70.	5.83	7.	2.	2.	.09
	ROUTED TO							
+			66.	5.83	7.	2.	2.	.09
	2 COMBINED AT							
+		WH	428.	5.92	51.	17.	17.	.65
	HYDROGRAPH AT							
+		W16	27.	5.75	2.	1.	1.	.03
	ROUTED TO							
+			24.	5.83	2.	1.	1.	.03
	HYDROGRAPH AT							
+		W17	16.	5.75	1.	0.	0.	.02
	2 COMBINED AT							
+		WI	38.	5.75	4.	1.	1.	.05
	ROUTED TO							
+		I-M	37.	5.83	4.	1.	1.	.05
	HYDROGRAPH AT							
+		W19	41.	5.75	3.	1.	1.	.04
	ROUTED TO							
+			37.	5.75	3.	1.	1.	.04
	HYDROGRAPH AT							
+		W20	32.	5.75	3.	1.	1.	.03
	2 COMBINED AT							
+		WJ	69.	5.75	6.	2.	2.	.07
	ROUTED TO							
+			65.	5.83	6.	2.	2.	.07
	HYDROGRAPH AT							
+		W21	96.	5.83	10.	3.	3.	.13
	2 COMBINED AT							
+		WK	161.	5.83	16.	6.	5.	.21
	ROUTED TO							
+			156.	5.83	16.	6.	5.	.21
	HYDROGRAPH AT							
+		W22	10.	5.75	1.	0.	0.	.01
	2 COMBINED AT							
+		WL	161.	5.83	17.	6.	6.	.22
	ROUTED TO							
+			147.	5.83	17.	6.	6.	.22
	HYDROGRAPH AT							
+		W23	19.	5.75	2.	1.	1.	.02
	HYDROGRAPH AT							
+		W18	80.	5.83	9.	3.	3.	.13
	5 COMBINED AT							
+		WM	690.	5.92	83.	28.	27.	1.06
	ROUTED TO							
+		M-N	673.	5.92	83.	28.	27.	1.06

+	HYDROGRAPH AT	W24	33.	5.83	3.	1.	1.	.04
+	HYDROGRAPH AT	W25	64.	5.83	8.	3.	3.	.10
+	3 COMBINED AT	WN	756.	5.92	94.	32.	31.	1.20
+	ROUTED TO	N-P	722.	5.92	94.	32.	31.	1.20
+	HYDROGRAPH AT	W28	36.	5.83	4.	1.	1.	.04
+	ROUTED TO		35.	5.83	4.	1.	1.	.04
+	HYDROGRAPH AT	W30	46.	5.83	5.	2.	2.	.05
+	ROUTED TO		46.	5.83	5.	2.	2.	.05
+	HYDROGRAPH AT	W29	37.	5.83	4.	1.	1.	.04
+	HYDROGRAPH AT	W31	14.	5.75	1.	0.	0.	.01
+	4 COMBINED AT	WO	127.	5.83	13.	4.	4.	.14
+	ROUTED TO	O-P	118.	5.83	13.	4.	4.	.14
+	HYDROGRAPH AT	W26	36.	5.75	3.	1.	1.	.03
+	ROUTED TO		35.	5.92	3.	1.	1.	.03
+	HYDROGRAPH AT	W27	89.	5.92	12.	4.	4.	.16
+	HYDROGRAPH AT	W32	61.	5.83	7.	2.	2.	.09
+	5 COMBINED AT	WP	1012.	5.92	128.	44.	43.	1.63
+	ROUTED TO	P-Q	950.	5.92	128.	44.	43.	1.63
+	HYDROGRAPH AT	W33A	82.	5.83	10.	3.	3.	.13
+	2 COMBINED AT	WP1	1023.	5.92	138.	47.	46.	1.75
+	ROUTED TO	P1-Q	1007.	6.00	137.	47.	46.	1.75
+	HYDROGRAPH AT	W33B	78.	5.92	10.	4.	3.	.14
+	HYDROGRAPH AT	W34A	86.	5.83	10.	3.	3.	.13
+	ROUTED TO	34A-P2	82.	5.92	9.	3.	3.	.13
+	HYDROGRAPH AT	W34B	101.	5.92	13.	5.	4.	.18
+	2 COMBINED AT	WP2	183.	5.92	23.	8.	8.	.30
+	ROUTED TO	P2-Q	175.	6.00	23.	8.	8.	.30
+	HYDROGRAPH AT	W34C	90.	5.92	12.	4.	4.	.16
+	4 COMBINED AT	WQ	1325.	6.00	181.	63.	61.	2.36
+	ROUTED TO	Q-Q1	1284.	6.00	181.	63.	60.	2.36
+	HYDROGRAPH AT	W36A	81.	5.92	11.	4.	4.	.14
+	2 COMBINED AT	WQ1	1352.	6.00	191.	66.	64.	2.50

+	ROUTED TO	Q1-R	1311.	6.08	190.	66.	64.	2.50
+	HYDROGRAPH AT	W36B	91.	6.00	14.	5.	5.	.19
+	HYDROGRAPH AT	W35A	62.	5.83	7.	2.	2.	.10
+	ROUTED TO	35A-WR	58.	6.00	7.	2.	2.	.10
+	HYDROGRAPH AT	W35B	81.	5.92	11.	4.	4.	.15
+	4 COMBINED AT	WR	1497.	6.08	222.	78.	75.	2.94
+	ROUTED TO	WR-S	1475.	6.08	222.	77.	75.	2.94
+	HYDROGRAPH AT	W37A	74.	5.83	9.	3.	3.	.11
+	ROUTED TO	37A-S	71.	5.92	9.	3.	3.	.11
+	HYDROGRAPH AT	W37B	102.	5.92	13.	5.	4.	.16
+	3 COMBINED AT	WS	1575.	6.08	242.	85.	82.	3.21
+	ROUTED TO	S-T	1522.	6.17	242.	85.	81.	3.21
+	HYDROGRAPH AT	W38	67.	5.83	8.	3.	3.	.09
+	ROUTED TO		66.	5.92	8.	3.	3.	.09
+	HYDROGRAPH AT	W39	100.	5.92	14.	5.	5.	.18
+	HYDROGRAPH AT	W40	67.	5.83	7.	3.	2.	.10
+	4 COMBINED AT	WT	1621.	6.17	269.	95.	91.	3.59
+	ROUTED TO	T-U	1613.	6.17	269.	95.	91.	3.59
+	HYDROGRAPH AT	W41	45.	5.83	5.	2.	2.	.06
+	HYDROGRAPH AT	W42	125.	5.75	13.	4.	4.	.06
+	ROUTED TO	U-V	122.	5.83	13.	4.	4.	.06
+	3 COMBINED AT	WU	1648.	6.17	285.	100.	96.	3.70
+	ROUTED TO		1612.	6.17	285.	100.	96.	3.70
+	HYDROGRAPH AT	W43	108.	5.83	12.	4.	4.	.15
+	2 COMBINED AT	WV	1640.	6.17	296.	104.	100.	3.85
+	ROUTED TO	V-W	1621.	6.17	296.	104.	100.	3.85
+	HYDROGRAPH AT	W45	134.	5.83	16.	5.	5.	.19
+	2 COMBINED AT	WW	1663.	6.17	311.	109.	105.	4.04
+	ROUTED TO	W-X	1621.	6.25	311.	109.	105.	4.04
+	HYDROGRAPH AT	M1	52.	5.75	5.	2.	2.	.07
+	ROUTED TO		50.	5.83	5.	2.	2.	.07
	HYDROGRAPH AT							

+		M2	21.	5.75	2.	1.	1.	.03		
	2 COMBINED AT									
+		MB	70.	5.83	7.	2.	2.	.09		
	ROUTED TO									
+			70.	5.83	7.	2.	2.	.09		
	HYDROGRAPH AT									
+		M4	26.	5.83	3.	1.	1.	.03		
	ROUTED TO									
+			26.	5.83	3.	1.	1.	.03		
	HYDROGRAPH AT									
+		M3	14.	5.75	1.	0.	0.	.01		
	3 COMBINED AT									
+		MC	105.	5.83	11.	4.	4.	.14		
	ROUTED TO									
+			102.	5.83	11.	4.	4.	.14		
	HYDROGRAPH AT									
+		M5	24.	5.75	2.	1.	1.	.02		
	ROUTED TO									
+			23.	5.83	2.	1.	1.	.02		
	HYDROGRAPH AT									
+		M6	51.	5.92	7.	2.	2.	.06		
	3 COMBINED AT									
+		MD	175.	5.83	20.	7.	6.	.22		
	ROUTED TO									
+			170.	5.92	20.	7.	6.	.22		
	HYDROGRAPH AT									
+		M7	63.	5.83	7.	2.	2.	.05		
	ROUTED TO									
+			61.	5.83	7.	2.	2.	.05		
	HYDROGRAPH AT									
+		M8	30.	5.83	3.	1.	1.	.04		
	2 COMBINED AT									
+		ME	91.	5.83	10.	3.	3.	.09		
	ROUTED TO									
+			89.	5.92	10.	3.	3.	.09		
	HYDROGRAPH AT									
+		M9	25.	5.75	2.	1.	1.	.02		
	ROUTED TO									
+			23.	5.83	2.	1.	1.	.02		
	HYDROGRAPH AT									
+		M12A	47.	5.83	5.	2.	2.	.07		
	HYDROGRAPH AT									
+		M12B	86.	5.92	11.	4.	4.	.15		
	5 COMBINED AT									
+		MF	401.	5.92	47.	16.	15.	.54		
	ROUTED TO									
+			300.	5.92	47.	16.	15.	.54		
	HYDROGRAPH AT									
+		M13	56.	5.83	6.	2.	2.	.06		
	ROUTED TO									
+			53.	5.92	6.	2.	2.	.06		
	HYDROGRAPH AT									
+		M14	122.	5.92	16.	5.	5.	.16		
	2 COMBINED AT									
+		MG	176.	5.92	22.	7.	7.	.22		
	ROUTED TO									
+		PONDW	16.	7.00	16.	7.	7.	.22	970.02	7.00
	2 COMBINED AT									
+		MH	403.	5.92	63.	23.	22.	.77		
	ROUTED TO									
+			385.	6.00	63.	23.	22.	.77		
	DIVERSION TO									
+		DIVRT1	85.	6.00	43.	18.	18.	.77		

+	HYDROGRAPH AT	MH-P2	300.	6.00	20.	5.	5.	.77
+	HYDROGRAPH AT	M15	101.	5.83	12.	4.	4.	.12
+	2 COMBINED AT	MI	388.	5.92	32.	9.	9.	.89
+	ROUTED TO		378.	6.00	32.	9.	9.	.89
+	HYDROGRAPH AT	M19	38.	5.83	4.	1.	1.	.05
+	2 COMBINED AT	MJ	397.	6.00	36.	10.	10.	.94
+	ROUTED TO		381.	6.00	36.	10.	10.	.94
+	HYDROGRAPH AT	M10	54.	5.75	5.	2.	2.	.06
+	ROUTED TO	M10-K	53.	5.83	5.	2.	2.	.06
+	HYDROGRAPH AT	M11A	65.	5.92	9.	3.	3.	.11
+	2 COMBINED AT	MK	115.	5.83	14.	5.	4.	.16
+	ROUTED TO	MK-K1	111.	5.92	14.	5.	4.	.16
+	HYDROGRAPH AT	M11B	64.	5.83	7.	2.	2.	.09
+	ROUTED TO	11B-K1	58.	5.92	7.	2.	2.	.09
+	HYDROGRAPH AT	M11C	66.	5.83	7.	2.	2.	.09
+	3 COMBINED AT	MK1	230.	5.83	27.	9.	9.	.35
+	ROUTED TO	K1-ML	225.	5.92	27.	9.	9.	.35
+	HYDROGRAPH AT	M16	31.	5.83	3.	1.	1.	.04
+	2 COMBINED AT	ML	247.	5.92	30.	10.	10.	.39
+	ROUTED TO		240.	5.92	30.	10.	10.	.39
+	HYDROGRAPH AT	M17	61.	5.83	6.	2.	2.	.08
+	2 COMBINED AT	MM	282.	5.92	36.	13.	12.	.46
+	ROUTED TO		268.	6.00	36.	12.	12.	.46
+	HYDROGRAPH AT	M18	48.	5.83	5.	2.	2.	.06
+	ROUTED TO		45.	5.92	5.	2.	2.	.06
+	HYDROGRAPH AT	M20	85.	5.83	11.	4.	4.	.13
+	4 COMBINED AT	MN	747.	6.00	88.	28.	27.	1.60
+	ROUTED TO		724.	6.00	88.	28.	27.	1.60
+	HYDROGRAPH AT	M21	19.	5.83	2.	1.	1.	.02
+	ROUTED TO		19.	5.83	2.	1.	1.	.02
+	HYDROGRAPH AT	M23	34.	5.83	3.	1.	1.	.05
+	3 COMBINED AT							

+		MO	747.	6.00	93.	30.	29.	1.67
	ROUTED TO		724.	6.00	93.	30.	29.	1.67
+	HYDROGRAPH AT	M24	58.	5.83	6.	2.	2.	.08
+	2 COMBINED AT	MP	744.	6.00	99.	32.	31.	1.75
+	ROUTED TO		736.	6.00	99.	32.	31.	1.75
+	HYDROGRAPH AT	M25	8.	5.83	1.	0.	0.	.01
+	2 COMBINED AT	MQ	739.	6.00	100.	32.	31.	1.76
+	ROUTED TO		734.	6.08	99.	32.	31.	1.76
+	HYDROGRAPH AT	M26	139.	5.92	18.	6.	6.	.18
+	2 COMBINED AT	MR	825.	6.08	117.	38.	37.	1.94
+	HYDROGRAPH AT	W44	28.	5.83	3.	1.	1.	.04
+	ROUTED TO		27.	5.92	3.	1.	1.	.04
+	HYDROGRAPH AT	W47	39.	5.83	4.	1.	1.	.05
+	ROUTED TO		36.	5.83	4.	1.	1.	.05
+	HYDROGRAPH AT	W46	32.	5.83	3.	1.	1.	.04
+	HYDROGRAPH AT	M27	39.	5.83	4.	1.	1.	.05
+	6 COMBINED AT	WX	2398.	6.17	442.	153.	147.	6.17
+	ROUTED TO		2323.	6.25	441.	152.	147.	6.17
+	HYDROGRAPH AT	W48	108.	5.75	10.	3.	3.	.12
+	ROUTED TO		102.	5.83	10.	3.	3.	.12
+	HYDROGRAPH AT	W49	189.	5.83	21.	7.	7.	.27
+	3 COMBINED AT	WZ	2392.	6.17	471.	163.	157.	6.55
+	ROUTED TO		2383.	6.25	471.	163.	157.	6.55
+	HYDROGRAPH AT	W50	83.	5.83	9.	3.	3.	.11
+	2 COMBINED AT	WAB	2399.	6.25	480.	166.	160.	6.66
+	ROUTED TO		2396.	6.25	480.	166.	160.	6.66
+	HYDROGRAPH AT	W51	46.	5.83	5.	2.	2.	.05
+	2 COMBINED AT	WAC	2406.	6.25	484.	168.	161.	6.71
+	ROUTED TO		2402.	6.25	485.	168.	161.	6.71
+	HYDROGRAPH AT	W52	48.	5.75	5.	2.	1.	.05
+	ROUTED TO		46.	5.83	5.	2.	1.	.05
+	HYDROGRAPH AT	W53	47.	5.83	5.	2.	2.	.05

+	2 COMBINED AT	WAD	92.	5.83	9.	3.	3.	.10		
	ROUTED TO		92.	5.83	9.	3.	3.	.10		
+	HYDROGRAPH AT	W54	8.	5.75	1.	0.	0.	.01		
+	3 COMBINED AT	WAE	2419.	6.25	494.	171.	165.	6.82		
+	ROUTED TO		2396.	6.25	493.	171.	164.	6.82		
+	HYDROGRAPH AT	W56	116.	5.83	14.	5.	5.	.18		
+	2 COMBINED AT	WAF	2426.	6.25	506.	175.	169.	7.01		
+	ROUTED TO		2405.	6.25	506.	175.	169.	7.01		
+	HYDROGRAPH AT	W62	64.	5.75	6.	2.	2.	.08		
+	ROUTED TO		61.	5.83	6.	2.	2.	.08		
+	HYDROGRAPH AT	W63	36.	5.75	4.	1.	1.	.05		
+	ROUTED TO		36.	5.83	4.	1.	1.	.05		
+	HYDROGRAPH AT	W61	105.	5.92	14.	5.	5.	.19		
+	3 COMBINED AT	WAH	190.	5.83	24.	8.	8.	.31		
+	ROUTED TO		183.	5.92	24.	8.	8.	.31		
+	HYDROGRAPH AT	W57	54.	5.83	6.	2.	2.	.07		
+	ROUTED TO		51.	6.00	6.	2.	2.	.07		
+	HYDROGRAPH AT	W58	126.	5.92	17.	6.	6.	.23		
+	3 COMBINED AT	WAI	345.	5.92	46.	16.	15.	.62		
+	ROUTED TO		341.	5.92	47.	16.	15.	.62		
+	HYDROGRAPH AT	E1A	65.	5.92	9.	3.	3.	.12		
+	ROUTED TO	E1A-EA	64.	6.00	9.	3.	3.	.12		
+	HYDROGRAPH AT	E1B	94.	5.92	13.	4.	4.	.17		
+	2 COMBINED AT	EA	147.	5.92	21.	7.	7.	.28		
+	ROUTED TO	EA-EB	145.	6.00	21.	7.	7.	.28		
+	HYDROGRAPH AT	E2	75.	5.83	8.	3.	3.	.10		
+	2 COMBINED AT	EB	187.	5.92	29.	10.	10.	.39		
+	ROUTED TO	POND1	101.	6.25	23.	9.	8.	.39	954.20	6.25
+	ROUTED TO		95.	6.25	23.	9.	8.	.39		
+	HYDROGRAPH AT	E3	67.	5.83	7.	2.	2.	.09		
+	HYDROGRAPH AT	MH-P2	85.	5.92	43.	18.	18.	.00		
+	3 COMBINED AT									

+		EC	187.	6.25	72.	29.	28.	.48		
	ROUTED TO									
+		POND2	95.	7.17	55.	24.	23.	.48	930.04	7.17
+	ROUTED TO									
			94.	7.25	55.	24.	23.	.48		
	HYDROGRAPH AT									
+		E1C	52.	5.83	6.	2.	2.	.08		
	ROUTED TO									
+		1C-ED1	50.	6.00	6.	2.	2.	.08		
	HYDROGRAPH AT									
+		E4	78.	5.83	10.	3.	3.	.13		
	2 COMBINED AT									
+		ED1	123.	5.92	16.	5.	5.	.21		
	ROUTED TO									
+		ED1-ED	121.	5.92	16.	6.	5.	.21		
	HYDROGRAPH AT									
+		E5	66.	5.83	7.	2.	2.	.09		
	3 COMBINED AT									
+		ED	174.	5.92	72.	32.	31.	.78		
	ROUTED TO									
+			172.	5.92	72.	32.	31.	.78		
	HYDROGRAPH AT									
+		E8	33.	5.83	3.	1.	1.	.04		
	2 COMBINED AT									
+		EE	196.	5.92	75.	33.	32.	.83		
	ROUTED TO									
+			192.	5.92	75.	33.	32.	.83		
	HYDROGRAPH AT									
+		E10	21.	5.83	2.	1.	1.	.03		
	2 COMBINED AT									
+		EF	208.	5.92	77.	34.	33.	.85		
	ROUTED TO									
+		F-G	199.	5.92	77.	34.	33.	.85		
	HYDROGRAPH AT									
+		E6	68.	5.92	9.	3.	3.	.12		
	HYDROGRAPH AT									
+		E7	28.	5.75	2.	1.	1.	.03		
	ROUTED TO									
+			25.	5.83	2.	1.	1.	.03		
	2 COMBINED AT									
+		EG1	89.	5.83	11.	4.	4.	.15		
	ROUTED TO									
+		G1-G	87.	5.92	11.	4.	4.	.15		
	HYDROGRAPH AT									
+		E9	46.	5.83	6.	2.	2.	.08		
	ROUTED TO									
+			46.	5.92	6.	2.	2.	.08		
	HYDROGRAPH AT									
+		E11	28.	5.83	3.	1.	1.	.05		
	HYDROGRAPH AT									
+		E12	66.	5.83	7.	2.	2.	.09		
	5 COMBINED AT									
+		EG	409.	5.92	103.	43.	42.	1.22		
	HYDROGRAPH AT									
+		E13	9.	5.92	1.	0.	0.	.02		
	HYDROGRAPH AT									
+		E14	4.	5.83	0.	0.	0.	.01		
	ROUTED TO									
+			4.	5.83	0.	0.	0.	.01		
	3 COMBINED AT									
+		EH	421.	5.92	105.	44.	42.	1.24		
	ROUTED TO									
+			398.	5.92	105.	44.	42.	1.24		

+	HYDROGRAPH AT	E19	35.	5.83	4.	1.	1.	.04
+	2 COMBINED AT	EJ1	421.	5.92	108.	45.	43.	1.28
+	ROUTED TO	J1-K	411.	6.00	107.	45.	43.	1.28
+	HYDROGRAPH AT	E15	36.	5.75	3.	1.	1.	.04
+	ROUTED TO		33.	5.83	3.	1.	1.	.04
+	HYDROGRAPH AT	E16	31.	5.75	3.	1.	1.	.03
+	2 COMBINED AT	EI	63.	5.75	6.	2.	2.	.07
+	ROUTED TO		61.	5.83	6.	2.	2.	.07
+	HYDROGRAPH AT	E17	32.	5.75	3.	1.	1.	.03
+	ROUTED TO		31.	5.83	3.	1.	1.	.03
+	HYDROGRAPH AT	E18	40.	5.83	4.	2.	1.	.05
+	3 COMBINED AT	EJ2	132.	5.83	13.	4.	4.	.15
+	ROUTED TO		119.	6.00	13.	4.	4.	.15
+	HYDROGRAPH AT	E23	108.	5.92	14.	5.	5.	.17
+	HYDROGRAPH AT	E24	74.	6.00	13.	4.	4.	.14
+	4 COMBINED AT	EK	698.	6.00	147.	59.	56.	1.74
+	ROUTED TO		671.	6.08	146.	58.	56.	1.74
+	HYDROGRAPH AT	E21	57.	5.83	7.	2.	2.	.09
+	ROUTED TO		55.	5.92	7.	2.	2.	.09
+	HYDROGRAPH AT	E20	51.	5.92	7.	2.	2.	.08
+	ROUTED TO		51.	5.92	7.	2.	2.	.08
+	HYDROGRAPH AT	E22	41.	5.92	5.	2.	2.	.07
+	3 COMBINED AT	EL	147.	5.92	19.	6.	6.	.23
+	ROUTED TO		140.	6.00	19.	6.	6.	.23
+	HYDROGRAPH AT	E25	121.	5.83	13.	5.	4.	.17
+	3 COMBINED AT	EM	845.	6.00	177.	69.	67.	2.13
+	ROUTED TO		834.	6.08	177.	69.	67.	2.13
+	HYDROGRAPH AT	E26	37.	5.75	3.	1.	1.	.04
+	2 COMBINED AT	EN	840.	6.08	180.	70.	68.	2.17
+	ROUTED TO		826.	6.08	179.	70.	68.	2.17
+	HYDROGRAPH AT	E27	104.	5.83	11.	4.	4.	.12
+	2 COMBINED AT							

+		EO	863.	6.08	189.	74.	71.	2.29
	ROUTED TO							
+			835.	6.08	189.	74.	71.	2.29
	HYDROGRAPH AT							
+		W55	38.	5.75	3.	1.	1.	.05
	2 COMBINED AT							
+		WAG	841.	6.08	192.	75.	72.	2.34
	ROUTED TO							
+			833.	6.17	192.	75.	72.	2.34
	HYDROGRAPH AT							
+		W59	43.	5.83	5.	2.	2.	.07
	4 COMBINED AT							
+		WAJ	3321.	6.25	749.	268.	259.	10.03
	ROUTED TO							
+			3310.	6.25	749.	268.	259.	10.03
	HYDROGRAPH AT							
+		E28	44.	5.92	6.	2.	2.	.07
	ROUTED TO							
+			44.	6.00	6.	2.	2.	.07
	HYDROGRAPH AT							
+		E29	35.	5.83	4.	1.	1.	.05
	2 COMBINED AT							
+		EZZ	69.	5.92	9.	3.	3.	.12
	HYDROGRAPH AT							
+		W60	47.	5.83	5.	2.	2.	.07
	3 COMBINED AT							
+	1	ZZ	3350.	6.25	763.	274.	263.	10.22

*** NORMAL END OF HEC-1 ***

Appendix B: Proposed HEC-1 Calculations

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 20MAY08 TIME 08:48:57 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G5, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID FALCON HIGHLANDS PRELIMINARY DRAINAGE PLAN - BASED ON DBPS MODEL F100FNLQ
2 ID ** DETENTION POND AT WU (BETWEEN SH 24 AND TAMLIN RD)
3 ID ** 2 Year, 5 Year and 100 Year Storm Events (24hr Storm)
4 ID **Basins W40 & W42 revised due to Meridian Crossing development
*DIAGRAM
5 IT 5 25MAY05 800 300
6 IO 5
7 JR PREC 1 1.2381 2.0952
8 KK W1
9 KM
10 BA .0479
11 PB 2.1
12 IN 15
13 PC .0005 .0015 .0030 .0045 .0060 .0080 .0100 .0120 .0143 .0165
14 PC .0188 .0210 .0233 .0255 .0278 .0320 .0390 .0460 .0530 .0600
15 PC .0750 .1000 .4000 .7000 .7250 .7500 .7650 .7800 .7900 .8000
16 PC .8100 .8200 .8250 .8300 .8350 .8400 .8450 .8500 .8550 .8600
17 PC .8638 .8675 .8713 .8750 .8788 .8825 .8863 .8900 .8938 .8975
18 PC .9013 .9050 .9083 .9115 .9148 .9180 .9210 .9240 .9270 .9300
19 PC .9325 .9350 .9375 .9400 .9425 .9450 .9475 .9500 .9525 .9550
20 PC .9575 .9600 .9625 .9650 .9675 .9700 .9725 .9750 .9775 .9800
21 PC .9813 .9825 .9838 .9850 .9863 .9875 .9888 .9900 .9913 .9925
22 PC .9938 .9950 .9963 .9975 .9988 1.000
23 LS 60
24 UD .097
25 KK W2
26 KM
27 RK 1519 .0263 .035 TRAP 5 4
28 KK W3
29 KM
30 BA .0278
31 LS 60
32 UD .160
33 KK WA
34 KM
35 HC 2
36 KK W4
37 KM
38 RK 464 .0151 .035 TRAP 5 4
39 KK W5
40 KM
41 BA .0498
42 LS 61
43 UD .139

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1 HEC-1 INPUT PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
44 KK WB
45 KM

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46	HC	2							
47	KK								
48	KM								
49	RK	823	.0279	.035	TRAP	5	4		54
50	KK	W4							
51	KM								
52	BA	.0054							
53	LS		62						
54	UD	.044							
55	KK								
56	KM								
57	RK	1078	.0482	.035	TRAP	5	4		
58	KK	W5							
59	KM								
60	BA	.0159							
61	LS		60						
62	UD	.075							
63	KK	WC							
64	KM								
65	HC	3							
66	KK								
67	KM								
68	RK	557	.0449	.035	TRAP	10	4		
69	KK	W6							
70	KM								
71	BA	.0486							
72	LS		60						
73	UD	.085							
74	KK								
75	KM								
76	RK	592	.0372	.035	TRAP	5	4		
77	KK	W7							
78	KM								
79	BA	.0217							
80	LS		60						
81	UD	.074							
82	KK								
83	KM								
84	RK	464	.1466	.035	TRAP	5	4		

1

HEC-1 INPUT

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
85	KK	WD									
86	KM										
87	HC	2									
88	KK	D-E									
89	KM										
90	RK	1044	.0479	.035	TRAP	5	4				
91	KK	W8									
92	KM										
93	BA	.0286									
94	LS		60								
95	UD	.069									
96	KK										
97	KM										
98	RK	1449	.0504	.035	TRAP	5	4				
99	KK	W9									
100	KM										
101	BA	.0402									
102	LS		61								
103	UD	.097									
104	KK	WE									
105	KM										
106	HC	3									
107	KK	E-F									
108	KM										
109	RK	789	.0038	.035	TRAP	5	4				
110	KK	W10									
111	KM										
112	BA	.0431									
113	LS		61								
114	UD	.096									
115	KK										
116	KM										
117	RK	824	.0388	.035	TRAP	5	4				

118 KK W11
 119 KM
 120 BA .0314
 121 LS 60
 122 UD .077

123 KK WF
 124 KM
 125 HC 4

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

126 KK F-G
 127 KM
 128 RK 2319 .0211 .035 TRAP 10 4

129 KK W12
 130 KM
 131 BA .0398
 132 LS 60
 133 UD .095

134 KK
 135 KM
 136 RK 2478 .0307 .035 TRAP 5 4

137 KK W14
 138 KM
 139 BA .0473
 140 LS 61
 141 UD .135

142 KK
 143 KM
 144 RK 81 0.0001 .035 TRAP 5 4

145 KK W13
 146 KM
 147 BA .1123
 148 LS 61
 149 UD .182

150 KK WG
 151 KM
 152 HC 4

153 KK G-H
 154 KM
 155 RK 2632 .0217 .035 TRAP 15 4

156 KK
 157 KM
 158 RK 2447 .0372 .035 TRAP 5 4

159 KK W15
 160 KM
 161 BA .0881
 162 LS 61
 163 UD .141

164 KK
 165 KM
 166 RK 1763 .0289 .035 TRAP 5 4

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

167 KK WH
 168 KM
 169 HC 2

170 KK W16
 171 KM
 172 BA .0292
 173 LS 61
 174 UD .092

175 KK
 176 KM
 177 RK 1345 .0260 .035 TRAP 5 4

178 KK W17
 179 KM
 180 BA .0184
 181 LS 60
 182 UD .085

183 KK WI
 184 KM
 185 HC 2

186	KK	I-M							
187	KM								
188	RK	2650	.0370	.035	TRAP	15	4		
189	KK	W19							
190	KM								
191	BA	.0428							
192	LS		61						
193	UD	.083							
194	KK								
195	KM								
196	RK	881	.0329	.035	TRAP	5	4		
197	KK	W20							
198	KM								
199	BA	.0315							
200	LS		61						
201	UD	.071							
202	KK	WJ							
203	KM								
204	HC	2							
205	KK								
206	KM								
207	RK	3061	.0235	.035	TRAP	5	4		

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

208	KK	W21							
209	KM								
210	BA	.1347							
211	LS		60						
212	UD	.156							
213	KK	WK							
214	KM								
215	HC	2							
216	KK								
217	KM								
218	RK	487	.0246	.035	TRAP	5	4		
219	KK	W22							
220	KM								
221	BA	.0086							
222	LS		63						
223	UD	.055							
224	KK	WL							
225	KM								
226	HC	2							
227	KK								
228	KM								
229	RK	1786	.0297	.035	TRAP	5	4		
230	KK	W23							
231	KM								
232	BA	.0244							
233	LS		60						
234	UD	.112							
235	KK	W18							
236	KM								
237	BA	.1251							
238	LS		60						
239	UD	.189							
240	KK	WM							
241	KM								
242	HC	5							
243	KK	M-N							
244	KM								
245	RK	1345	.0149	.035	TRAP	20	4		
246	KK	W24							
247	KM								
248	BA	.0442							
249	LS		60						
250	UD	.140							

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

251	KK	W25							
252	KM								
253	BA	.0957							
254	LS		61						
255	UD	.197							

256	KK	WN							
257	KM								
258	HC	3							
259	KK	N-P							
260	KM								
261	RK	1589	.017	.035	TRAP	20	4		
262	KK	W28							
263	KM								
264	BA	.0397							
265	LS		63						
266	UD	.128							
267	KK								
268	KM								
269	RK	1345	.0208	.035	TRAP	5	4		
270	KK	W30							
271	KM								
272	BA	.0509							
273	LS		63						
274	UD	.123							
275	KK								
276	KM								
277	RK	1078	.0074	.035	TRAP	5	4		
278	KK	W29							
279	KM								
280	BA	.0409							
281	LS		63						
282	UD	.145							
283	KK	W31							
284	KM								
285	BA	.0123							
286	LS		63						
287	UD	.073							
288	KK	WO							
289	KM								
290	HC	4							

1

HEC-1 INPUT

PAGE 8

LINE	ID	1	2	3	4	5	6	7	8	9	10
291	KK	O-P									
292	KM										
293	RK	2169	.0226	.035	TRAP	5	4				
294	KK	W26									
295	KM										
296	BA	.0301									
297	LS		63								
298	UD	.062									
299	KK										
300	KM										
301	RK	4662	.0225	.035	TRAP	5	4				
302	KK	W27									
303	KM										
304	BA	.1633									
305	LS		60								
306	UD	.253									
307	KK	W32									
308	KM										
309	BA	.0890									
310	LS		60								
311	UD	.170									
312	KK	WP									
313	KM										
314	HC	5									
315	KK	P-Q									
316	KM										
317	RK	1925	.0182	.035	TRAP	25	4				
318	KK	W33A									
319	KM										
320	BA	.1261									
321	LS		60								
322	UD	.186									
323	KK	WP1									
324	KM										
325	HC	2									
326	KK	P1-Q									
327	KM										

328	RK	3000	.020	.035	TRAP	25	4
329	KK	W33B					
330	KM						
331	BA	.1360					
332	LS		60				
333	UD	.225					

1

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

334	KK	W34A					
335	KM						
336	BA	.1418					
337	LS		60				
338	UD	.173					
339	KK	34A-P2					
340	KM						
341	RK	2550	.0176	.035	TRAP	25	4
342	KK	W34B					
343	KM						
344	BA	.1766					
345	LS		60				
346	UD	.224					
347	KK	WP2					
348	KM						
349	HC	2					
350	KK	P2-Q					
351	KM						
352	RK	2640	.021	.035	TRAP	25	4
353	KK	W34C					
354	KM						
355	BA	.1625					
356	LS		60				
357	UD	.244					
358	KK	WQ					
359	KM						
360	HC	4					
361	KK	Q-Q1					
362	KM						
363	RK	2940	.022	.035	TRAP	25	4
364	KK	W36A					
365	KM						
366	BA	.1429					
367	LS		60				
368	UD	.234					
369	KK	WQ1					
370	KM						
371	HC	2					
372	KK	Q1-R					
373	KM						
374	RK	3400	.022	.035	TRAP	25	4

1

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

375	KK	W36B					
376	KM						
377	BA	.1918					
378	LS		60				
379	UD	.306					
380	KK	W35A					
381	KM						
382	BA	.0958					
383	LS		60				
384	UD	.187					
385	KK	35A-WR					
386	KM						
387	RK	3715	.023	.035	TRAP	25	4
388	KK	W35B					
389	KM						
390	BA	.1507					
391	LS	0	60				
392	UD	.259					
393	KK	WR					
394	KM						
395	HC	4					
396	KK	WR-S					

397	KM								
398	RK	2922	.0168	.035	TRAP	25	4		
399	KK	W37A							
400	KM								
401	BA	.1138							
402	LS		60						
403	UD	.185							
404	KK	37A-S							
405	KM								
406	RK	1430	.014	.035	TRAP	25	4		
407	KK	W37B							
408	KM								
409	BA	.1636							
410	LS		90						
411	UD	.218							
412	KK	WS							
413	KM								
414	HC	3							

1

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

415	KK	S-T							
416	KM								
417	RK	3653	.0164	.035	TRAP	25	4		
418	KK	W38							
419	KM								
420	BA	.0907							
421	LS		62						
422	UD	.190							
423	KK								
424	KM								
425	RK	2922	.0171	.035	TRAP	5	4		
426	KK	W39							
427	KM								
428	BA	.1833							
429	LS		76						
430	UD	.251							
431	KK	W40							
432	KM								
433	BA	.0706							
434	LS		92						
435	UD	.165							
436	KK	WT							
437	KM								
438	HC	4							
439	KK	T-U							
440	KM								
441	RK	1125	.0098	.035	TRAP	25	4		
442	KK	W41							
443	KM								
444	BA	.0601							
445	LS		83						
446	UD	.117							
447	KK	POND							
448	KM								
449	HC	2							
450	KK	DIVOUT							
451	KM	DIVERT 35.8 CFS (2-YR HISTORIC FLOW) FROM DETENTION POND							
452	DT	DIVERT 0 35.8							
453	DI	0 35.8							
454	DQ	0 35.8							

1

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

455	KK	PONDWU							
456	KM	REGIONAL DETENTION POND Q5=94, Q100=1214							
457	SV	0.0 0.006 .09 .40 1.19 3.31 7.12 11.50 16.05							
458	SV	20.76 25.61 30.61 35.77 40.99							
459	SE	16.5 17 18 19 20 21 22 23 24							
460	SE	25 26 27 28 29							
461	SQ	0 0 27.40 46.66 67.77 81.84 249.51 544.89 923.33							
462	SQ	1090.5 1212.93 1323.91 1426.18 1521.53							
463	RS	1 ELEV 16.5							
464	KK	B-WU							
465	KM								
466	RK	2215	.0181	.035	TRAP	25	4		

467	KK	DIVIN								
468	KM	RETRIEVE	35.8	CFS	DIVERTED	IN	CHANNEL			
469	DR	DIVERT								
470	KK	W42A								
471	KM									
472	BA	.0127								
473	LS		92							
474	UD	.180								
475	KK	42A-WU								
476	KM									
477	RK	1970	.0090	.013		TRAP		4		4
478	KK	W42								
479	KM									
480	BA	.0519								
481	LS		92							
482	UD	.127								
483	KK	W42B								
484	KM									
485	BA	.0195								
486	LS		92							
487	UD	.180								
488	KK	42B-WU								
489	KM									
490	RK	2770	.0100	.013		TRAP		4		4
491	KK	WU								
492	KM									
493	HC	5								
494	KK	U-V								
495	KM									
496	RK	2200	.0145	.035		TRAP		25		4

1

HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

497	KK	W43								
498	KM									
499	BA	.1457								
500	LS		64							
501	UD	.169								
502	KK	WV								
503	KM									
504	HC	2								
505	KK	V-W								
506	KM									
507	RK	487	.0103	.035		TRAP		25		4
508	KK	W45								
509	KM									
510	BA	.1931								
511	LS		61							
512	UD	.189								
513	KK	WW								
514	KM									
515	HC	2								
516	ZZ									

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

8      W1
      V
      V
25     .
      .
28     .      W2
      .
      .
33     WA .....
      V
      V
36     .
      .
39     .      W3
      .
      .
44     WB .....
      V
      V
47
  
```

50	.			
	.	W4		
	.	V		
55	.	V		
	.	.		
58	.		W5	
	.	.	.	
63	.	WC	.	
	.	V	.	
	.	V	.	
66	.	.	.	
	.			
69	.	W6		
	.	V		
	.	V		
74	.	.		
	.	.		
77	.		W7	
	.	.	V	
	.	.	V	
82	.	.	.	
	.	.	.	
85	.	WD	.	
	.	V	.	
	.	V	.	
88	.	D-E	.	
	.	.	.	
91	.		W8	
	.	.	V	
	.	.	V	
96	.	.	.	
	.	.	.	
99	.	.	.	W9

104	.	WE	.	
	.	V	.	
	.	V	.	
107	.	E-F	.	
	.	.	.	
110	.		W10	
	.	.	V	
	.	.	V	
115	.	.	.	
	.	.	.	
118	.	.	.	W11

123	.	WF	.	
	.	V	.	
	.	V	.	
126	.	F-G	.	
	.	.	.	
129	.	W12		
	.	V		
	.	V		
134	.	.		
	.	.		
137	.		W14	
	.	.	V	
	.	.	V	
142	.	.	.	
	.	.	.	
145	.	.	.	W13

150	.	WG	.	
	.	V	.	
	.	V	.	
153	.	G-H	.	
	.	V	.	
	.	V	.	
156	.	.	.	
	.	.	.	
159	.	W15		
	.	V		
	.	V		
164	.	.	.	
	.	.	.	
	.	.	.	

```

167 WH .....
170 . W16
170 . V
170 . V
175 .
178 . W17
183 . WI .....
183 . V
183 . V
186 . I-M
189 . W19
189 . V
189 . V
194 .
197 . W20
202 . WJ .....
202 . V
202 . V
205 .
208 . W21
213 . WK .....
213 . V
213 . V
216 .
219 . W22
224 . WL .....
224 . V
224 . V
227 .
230 . W23
235 . W18
240 . WM .....
240 . V
240 . V
243 . M-N
246 . W24
251 . W25
256 . WN .....
256 . V
256 . V
259 . N-P
262 . W28
262 . V
262 . V
267 .
270 . W30
270 . V
270 . V
275 .
278 . W29
283 . W31
288 . WO .....
288 . V

```

291	.	.	V	.	.
	.	.	O-P	.	.
294	.	.	.	W26	.
	.	.	.	V	.
299	.	.	.	V	.

302	W27

307	W32

312	WP
	V
	V
315	P-Q

318	.	.	W33A	.	.

323	WP1
	V
	V
326	P1-Q

329	.	.	W33B	.	.

334	.	.	.	W34A	.
	.	.	.	V	.
	.	.	.	V	.
339	.	.	34A-P2	.	.

342	W34B

347	.	.	.	WP2
	.	.	.	V	.
	.	.	.	V	.
350	.	.	P2-Q	.	.

353	W34C

358	WQ
	V
	V
361	Q-Q1

364	.	.	W36A	.	.

369	WQ1
	V
	V
372	Q1-R

375	.	.	W36B	.	.

380	.	.	.	W35A	.
	.	.	.	V	.
	.	.	.	V	.
385	.	.	35A-WR	.	.

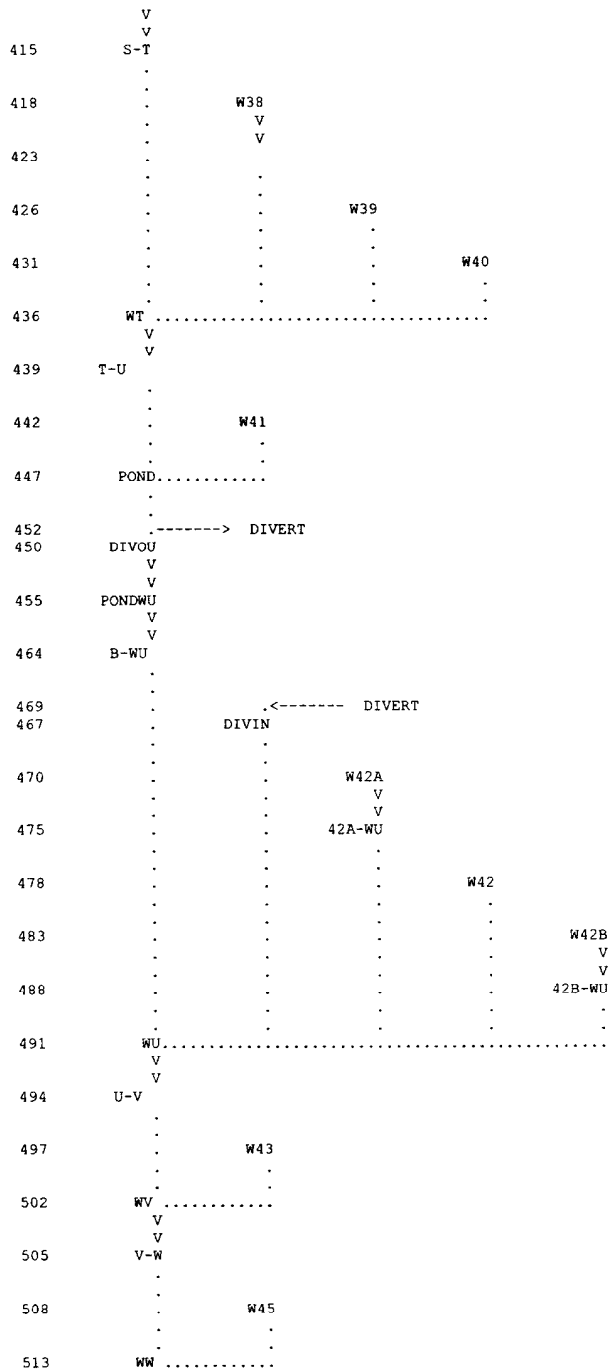
388	W35B

393	WR
	V
	V
396	WR-S

399	.	.	W37A	.	.
	.	.	V	.	.
	.	.	V	.	.
404	.	.	37A-S	.	.

407	.	.	.	W37B	.

412	WS



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 20MAY08 TIME 08:48:57 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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FALCON HIGHLANDS PRELIMINARY DRAINAGE PLAN - BASED ON DBPS MODEL F100FNLQ
 ** DETENTION POND AT WU (BETWEEN SH 24 AND TAMLIN RD)

** 2 Year, 5 Year and 100 Year Storm Events (24hr Storm)
 **Basins W40 & W42 revised due to Meridian Crossing development

6 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 25MAY 5 STARTING DATE
 ITIME 0800 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 26MAY 5 ENDING DATE
 NDTIME 0855 ENDING TIME
 ICENT 19 CENTURY MARK

 COMPUTATION INTERVAL .08 HOURS
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 1.00 1.24 2.10

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	FLOW	RATIOS APPLIED TO PRECIPITATION		
					RATIO 1 1.00	RATIO 2 1.24	RATIO 3 2.10
HYDROGRAPH AT							
+	W1	.05	1	FLOW TIME	1. 5.83	5. 5.83	40. 5.75
ROUTED TO							
+		.05	1	FLOW TIME	0. 6.00	4. 5.92	37. 5.83
HYDROGRAPH AT							
+	W2	.03	1	FLOW TIME	0. 5.92	2. 5.83	20. 5.83
2 COMBINED AT							
+	WA	.08	1	FLOW TIME	1. 6.00	6. 5.92	57. 5.83
ROUTED TO							
+		.08	1	FLOW TIME	1. 6.08	6. 5.92	55. 5.83
HYDROGRAPH AT							
+	W3	.05	1	FLOW TIME	1. 5.83	5. 5.83	39. 5.83
2 COMBINED AT							
+	WB	.13	1	FLOW TIME	1. 6.08	11. 5.92	95. 5.83
ROUTED TO							
+		.13	1	FLOW TIME	1. 6.17	10. 5.92	91. 5.83
HYDROGRAPH AT							
+	W4	.01	1	FLOW TIME	0. 5.75	1. 5.75	6. 5.75
ROUTED TO							
+		.01	1	FLOW TIME	0. 5.92	1. 5.83	6. 5.75

HYDROGRAPH AT								
+	W5	.02	1	FLOW	0.	2.	15.	
				TIME	5.83	5.75	5.75	
3 COMBINED AT								
+	WC	.15	1	FLOW	1.	11.	105.	
				TIME	6.17	5.92	5.83	
ROUTED TO								
+		.15	1	FLOW	1.	11.	103.	
				TIME	6.25	5.92	5.83	
HYDROGRAPH AT								
+	W6	.05	1	FLOW	1.	5.	43.	
				TIME	5.83	5.83	5.75	
ROUTED TO								
+		.05	1	FLOW	0.	5.	40.	
				TIME	5.92	5.83	5.75	
HYDROGRAPH AT								
+	W7	.02	1	FLOW	0.	2.	20.	
				TIME	5.83	5.75	5.75	
ROUTED TO								
+		.02	1	FLOW	0.	2.	20.	
				TIME	5.83	5.83	5.75	
2 COMBINED AT								
+	WD	.07	1	FLOW	1.	7.	60.	
				TIME	5.83	5.83	5.75	
ROUTED TO								
+	D-E	.07	1	FLOW	1.	6.	55.	
				TIME	5.92	5.83	5.75	
HYDROGRAPH AT								
+	W8	.03	1	FLOW	0.	3.	27.	
				TIME	5.83	5.75	5.75	
ROUTED TO								
+		.03	1	FLOW	0.	3.	24.	
				TIME	5.92	5.83	5.75	
HYDROGRAPH AT								
+	W9	.04	1	FLOW	1.	5.	36.	
				TIME	5.83	5.83	5.75	
3 COMBINED AT								
+	WE	.14	1	FLOW	1.	14.	114.	
				TIME	5.92	5.83	5.75	
ROUTED TO								
+	E-F	.14	1	FLOW	1.	13.	108.	
				TIME	6.08	5.92	5.83	
HYDROGRAPH AT								
+	W10	.04	1	FLOW	1.	5.	39.	
				TIME	5.83	5.83	5.75	
ROUTED TO								
+		.04	1	FLOW	1.	5.	36.	
				TIME	5.92	5.83	5.75	
HYDROGRAPH AT								
+	W11	.03	1	FLOW	0.	3.	29.	
				TIME	5.83	5.75	5.75	
4 COMBINED AT								
+	WF	.36	1	FLOW	3.	29.	265.	
				TIME	6.08	5.92	5.83	
ROUTED TO								
+	F-G	.36	1	FLOW	3.	28.	255.	
				TIME	6.25	6.00	5.83	
HYDROGRAPH AT								
+	W12	.04	1	FLOW	0.	4.	33.	
				TIME	5.83	5.83	5.75	
ROUTED TO								
+		.04	1	FLOW	0.	4.	32.	
				TIME	6.17	5.92	5.83	
HYDROGRAPH AT								
+	W14	.05	1	FLOW	1.	5.	38.	
				TIME	5.83	5.83	5.83	
ROUTED TO								
+		.05	1	FLOW	1.	5.	37.	
				TIME	5.92	5.92	5.83	
HYDROGRAPH AT								
+	W13	.11	1	FLOW	2.	10.	80.	

HYDROGRAPH AT								
+	W33B	.14	1	FLOW TIME	1. 6.33	9. 5.92	78. 5.92	
HYDROGRAPH AT								
+	W34A	.14	1	FLOW TIME	1. 6.33	10. 5.92	96. 5.83	
ROUTED TO								
+	34A-P2	.14	1	FLOW TIME	1. 6.33	10. 6.08	93. 5.92	
HYDROGRAPH AT								
+	W34B	.18	1	FLOW TIME	1. 6.33	11. 5.92	101. 5.92	
2 COMBINED AT								
+	WP2	.32	1	FLOW TIME	2. 6.33	20. 6.00	195. 5.92	
ROUTED TO								
+	P2-Q	.32	1	FLOW TIME	2. 6.75	19. 6.17	186. 6.00	
HYDROGRAPH AT								
+	W34C	.16	1	FLOW TIME	1. 6.33	10. 6.00	90. 5.92	
4 COMBINED AT								
+	WQ	2.37	1	FLOW TIME	19. 6.92	115. 6.25	1336. 6.00	
ROUTED TO								
+	Q-Q1	2.37	1	FLOW TIME	19. 7.08	114. 6.33	1294. 6.00	
HYDROGRAPH AT								
+	W36A	.14	1	FLOW TIME	1. 6.33	9. 5.92	81. 5.92	
2 COMBINED AT								
+	WQ1	2.51	1	FLOW TIME	20. 7.08	118. 6.33	1363. 6.00	
ROUTED TO								
+	Q1-R	2.51	1	FLOW TIME	20. 7.25	117. 6.42	1320. 6.08	
HYDROGRAPH AT								
+	W36B	.19	1	FLOW TIME	1. 6.42	10. 6.00	91. 6.00	
HYDROGRAPH AT								
+	W35A	.10	1	FLOW TIME	1. 6.33	7. 5.92	62. 5.83	
ROUTED TO								
+	35A-WR	.10	1	FLOW TIME	1. 6.92	6. 6.25	58. 6.00	
HYDROGRAPH AT								
+	W35B	.15	1	FLOW TIME	1. 6.42	9. 6.00	81. 5.92	
4 COMBINED AT								
+	WR	2.95	1	FLOW TIME	22. 7.17	132. 6.42	1506. 6.08	
ROUTED TO								
+	WR-S	2.95	1	FLOW TIME	22. 7.33	131. 6.50	1485. 6.08	
HYDROGRAPH AT								
+	W37A	.11	1	FLOW TIME	1. 6.33	8. 5.92	74. 5.83	
ROUTED TO								
+	37A-S	.11	1	FLOW TIME	1. 6.58	8. 6.00	71. 5.92	
HYDROGRAPH AT								
+	W37B	.16	1	FLOW TIME	145. 5.83	201. 5.83	410. 5.83	
3 COMBINED AT								
+	WS	3.23	1	FLOW TIME	145. 5.83	201. 5.83	1705. 6.08	
ROUTED TO								
+	S-T	3.23	1	FLOW TIME	140. 5.92	197. 5.92	1632. 6.17	
HYDROGRAPH AT								
+	W38	.09	1	FLOW TIME	2. 5.92	10. 5.92	67. 5.83	

ROUTED TO							
+		.09	1	FLOW TIME	2. 6.25	9. 6.08	66. 5.92
HYDROGRAPH AT							
+	W39	.18	1	FLOW TIME	49. 5.92	85. 5.92	251. 5.92
HYDROGRAPH AT							
+	W40	.07	1	FLOW TIME	76. 5.75	102. 5.75	201. 5.75
4 COMBINED AT							
+	WT	3.57	1	FLOW TIME	244. 5.92	359. 5.83	1867. 6.08
ROUTED TO							
+	T-U	3.57	1	FLOW TIME	243. 5.92	358. 5.92	1821. 6.17
HYDROGRAPH AT							
+	W41	.06	1	FLOW TIME	40. 5.75	60. 5.75	142. 5.75
2 COMBINED AT							
+	POND	3.63	1	FLOW TIME	264. 5.92	392. 5.83	1839. 6.17
DIVERSION TO							
+	DIVERT	3.63	1	FLOW TIME	36. 5.58	36. 5.50	36. 5.42
HYDROGRAPH AT							
+	DIVOU	3.63	1	FLOW TIME	229. 5.92	356. 5.83	1804. 6.17
ROUTED TO							
+	PONDWU	3.63	1	FLOW TIME	118. 6.17	223. 6.08	1313. 6.33
				** PEAK STAGES IN FEET **			
			1	STAGE TIME	20.68 5.92	7.42 6.08	26.90 6.33
ROUTED TO							
+	B-WU	3.63	1	FLOW TIME	116. 10.58	219. 9.92	1298. 8.75
HYDROGRAPH AT							
+	DIVIN	.00	1	FLOW TIME	36. 5.58	36. 5.50	36. 5.42
HYDROGRAPH AT							
+	W42A	.01	1	FLOW TIME	13. 5.83	18. 5.83	35. 5.75
ROUTED TO							
+	42A-WU	.01	1	FLOW TIME	13. 6.17	18. 6.17	35. 6.08
HYDROGRAPH AT							
+	W42	.05	1	FLOW TIME	60. 5.75	81. 5.75	156. 5.75
HYDROGRAPH AT							
+	W42B	.02	1	FLOW TIME	21. 5.83	28. 5.83	54. 5.75
ROUTED TO							
+	42B-WU	.02	1	FLOW TIME	20. 5.92	27. 5.92	54. 5.83
5 COMBINED AT							
+	WU	3.72	1	FLOW TIME	135. 10.58	256. 9.92	1339. 8.75
ROUTED TO							
+	U-V	3.72	1	FLOW TIME	135. 10.67	253. 10.00	1333. 8.75
HYDROGRAPH AT							
+	W43	.15	1	FLOW TIME	7. 5.92	24. 5.83	132. 5.83
2 COMBINED AT							
+	WV	3.86	1	FLOW TIME	135. 10.67	254. 10.00	1338. 8.75
ROUTED TO							
+	V-W	3.86	1	FLOW TIME	133. 10.75	249. 10.00	1334. 8.75
HYDROGRAPH AT							
+	W45	.19	1	FLOW	3.	17.	134.

MERIDIAN CROSSING FILING NO. 1 - FDR - EXISTING CONDITIONS SURFACE ROUTING

DESIGN POINT	CONTRIBUTING BASINS	CA (equivalent)		Tc (min.)	INTENSITY		TOTAL FLOWS		
		CA(5)	CA(100)		I(5) (in/hr)	I(100) (in/hr)	Q(5) (cfs)	Q(100) (cfs)	
1	E-1	1.96	2.07	6.4	4.8	8.5	9.4	17.6	
		TRAVEL TIME							
		1.96	2.07	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
				265	4.2	1.1	7.4		
2	DP-1 E-2	1.96	2.07	7.4	4.6	8.1	22.9	48.7	
		TRAVEL TIME							
		5.03	6.01	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			1500	4.7	5.3	12.7			
3	E-4	1.94	2.04	6.3	4.8	8.5	9.3	17.5	
		TRAVEL TIME							
		1.94	2.04	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			900	4.1	3.7	9.9			
5	E-7	2.09	2.20	5.4	5.0	8.9	10.5	19.7	
		TRAVEL TIME							
		2.09	2.20	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			347	2.0	2.9	8.3			
4	E-6	2.36	2.49	5.8	4.9	8.7	11.6	21.7	
		TRAVEL TIME							
		2.36	2.49	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			62	9.0	0.1	5.9			
6	DP-2 DP-3 E-8 E-3	5.03	6.01	16.1	3.4	6.0	67.8	149.7	
		1.94	2.04	TRAVEL TIME					
		8.36	10.75	TRAVEL TIME					
		4.89	6.28	TRAVEL TIME					
		20.21	25.09	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			36	1.5	0.4	16.5			
7	DP-6 DP-5 (INLET) DP-4 (INLET)	20.21	25.09	16.5	3.3	5.9	67.0	148.5	
		0.00	0.08	TRAVEL TIME					
		0.00	0.00	TRAVEL TIME					
		20.21	25.17	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			139	8.4	0.3	16.8			
8	DP-7 POND WU* WEST TRIB CHAN.* E-5	20.21	25.17	16.8	3.3	5.9	147.7	1286.1	
		19.44	188.25	TRAVEL TIME					
		1.96	2.07	TRAVEL TIME					
		3.29	4.23	TRAVEL TIME					
		44.91	219.72	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			83	5.8	0.2	17.0			

* VALUES WERE OBTAINED FROM THE APPROVED MARKET PLACE FILING NO. 1 DRAINAGE REPORT

**MERIDIAN CROSSING FILING NO. 1 - FDR - EXISTING CONDITIONS
INLET CALCULATIONS**

DP	Inlet size L(i)	INLET TYPE	CROSS SLOPE	STREET SLOPE	Q(5)	Q(100)	Q _s						Q ₁₀₀					
							Qi	CA(eqv.)	FB	CA(eqv.)	DEPTH (max)	SPREAD	Qi	CA(eqv.)	FB	CA(eqv.)	DEPTH (max)	SPREAD
1	15	FLOW-BY	2.0%	1.0%	9	18	6.4	1.34	3	0.62	0.42	16.7	10.8	1.27	7	0.80	0.51	21.1
3	20	SUMP	2.0%	SAG	9	17	9.3	1.94	0	0.00	0.50		17.5	2.04	0	0.00	0.50	
4	25	SUMP	2.0%	SAG	12	22	11.6	2.36	0	0.00	0.50		21.7	2.49	0	0.00	0.50	
5	20	SUMP	2.0%	SAG	10	20	10.5	2.09	0	0.00	0.50		18.9	2.12	1	0.08	0.50	

Appendix D: Proposed Rational Calculations

MERIDIAN CROSSING - FDR - PROPOSED CONDITIONS (RATIONAL METHOD Q=CIA)

BASIN	TOTAL FLOWS			AREA TOTAL (Ac)	WEIGHTED			OVERLAND				CHANNEL			Tc TOTAL (min)		INTENSITY		COMMENTS
	Q5 (c.f.s.)	Q100 (c.f.s.)	CA(equiv.) 5 YR 100 YR		C5	C100	C5	Length (ft)	Slope (ft)	Tco (min)	Length (ft)	Slope (%)	Velocity (fps)	Tcc (min)	Is (in/hr)	I100 (in/hr)			
D-1	9.4	17.6	1.96	2.07	0.90	0.95	5	2.0%	0.7	867	1.6%	2.5	5.7	6.4	4.8	8.5			
D-2	23.4	43.9	4.63	4.88	0.90	0.95	10	2.0%	0.9	675	1.8%	2.6	4.3	5.2	5.1	9.0			
D-3	34.7	65.1	10.34	10.92	0.90	0.95	20	2.0%	0.7	1,370	1.2%	2.2	10.4	16.1	3.4	6.0			
D-4	9.3	17.5	1.94	2.04	0.90	0.95	5	2.0%	0.7	848	1.6%	2.5	5.6	6.3	4.8	8.5			
D-5	12.1	27.8	3.29	4.23	0.90	0.95	5	2.0%	0.7	873	1.4%	2.3	7.3	13.0	3.7	6.6			
D-6	11.6	21.7	2.36	2.49	0.90	0.95	5	2.0%	0.7	873	2.0%	2.8	5.1	5.8	4.9	8.7			
D-7	10.5	19.7	2.09	2.20	0.90	0.95	5	2.0%	0.7	797	2.0%	2.8	4.7	5.4	5.0	8.9			
D-8	82.8	155.5	21.50	22.70	0.90	0.95	10	2.0%	4.0	1,315	2.0%	2.8	7.7	11.7	3.8	6.9			
D-9	16.9	31.8	3.31	3.50	0.90	0.95	5	2.0%	0.7	525	1.2%	2.2	4.0	5.0	5.1	9.1			
D-10	1.9	3.6	0.38	0.40	0.90	0.95	0.90	2.0%	0.7	400	4.0%	4.0	1.7	5.0	5.1	9.1			
D-11	1.8	3.4	0.35	0.37	0.90	0.95	0.90	2.0%	0.7	400	4.0%	4.0	1.7	5.0	5.1	9.1			
D-12	1.8	3.3	0.43	0.45	0.90	0.95	5	2.0%	0.7	800	0.5%	1.4	9.4	10.1	4.1	7.3			
D-20	5.9	11.2	1.47	1.55	0.90	0.95	10	2.0%	0.9	800	0.5%	1.4	9.4	10.4	4.0	7.2			
Formula:	C*I*A	C*I*A	Q/I	Q/I					*1			*2	*3	Tco+Tcc	*4	*6			
				65.80								20			1.5	2.67			

- 1* Tco = 1.87*(1.1-C5)*(L^0.5)*((S*100)^-0.33) (DCM page 5-11)
- 2* Vc = 20*S^0.5 (USDCM RO-4)
- 3* Tcc = 1/V*60
- 4* Is = (26.65*1.50)/(10+Tc)^0.76 (City Letter of 1/7/2003)
- 6* I100 = (26.65*2.67)/(10+Tc)^0.76 (City Letter of 1/7/2003)

MERIDIAN CROSSING - FDR - PROPOSED CONDITIONS

SURFACE ROUTING

DESIGN POINT	CONTRIBUTING BASINS	CA (equivalent)		Tc (min.)	INTENSITY		TOTAL FLOWS		
		CA(5)	CA(100)		I(5) (in/hr)	I(100) (in/hr)	Q(5) (cfs)	Q(100) (cfs)	
1	D-1	1.96	2.07	6.4	4.8	8.5	9.4	17.6	
		TRAVEL TIME							
		1.96	2.07	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
				250	4.2	1.0	7.4		
J	D-10 D-11	0.38	0.40	5.0	5.1	9.1	3.7	7.0	
		0.35	0.37	TRAVEL TIME					
		0.73	0.77	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			Gutter	820	5.0	2.7	7.7		
B	D-12 DP-J	0.43	0.45	10.1	4.1	7.3	4.7	8.9	
		0.73	0.77	TRAVEL TIME					
		1.16	1.22	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			Channel	40	1.8	0.4	10.5		
X	D-9	3.31	3.50	5.0	5.1	9.1	16.9	31.8	
		TRAVEL TIME							
		3.31	3.50	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			Pipe	420	7.9	0.9	5.9		
Z	D-2 DP-X Inlet 1 Flowby Inlet 1 DP-B	4.63	4.88	10.5	4.0	7.2	44.6	83.8	
		3.31	3.50	TRAVEL TIME					
		0.62	0.80	TRAVEL TIME					
		1.34	1.27	TRAVEL TIME					
		1.16	1.22	TRAVEL TIME					
		11.06	11.67	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
				Channel	725	3.8	3.2	13.6	
Y	D-3	10.34	10.92	16.1	3.4	6.0	34.7	65.1	
		TRAVEL TIME							
		10.34	10.92	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
			Channel	895	2.4	6.2	22.3		
3	D-4	1.94	2.04	6.3	4.8	8.5	9.3	17.5	
		TRAVEL TIME							
		1.94	2.04	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)	
					0.0	0.0	6.3		

DESIGN POINT	CONTRIBUTING BASINS	CA (equivalent)		Tc (min.)	INTENSITY		TOTAL FLOWS	
		CA(5)	CA(100)		I(5) (in/hr)	I(100) (in/hr)	Q(5) (cfs)	Q(100) (cfs)
E	DP-3 DP-Z	1.94	2.04	13.6	3.6	6.4	46.9	88.2
		11.06	11.67	TRAVEL TIME				
		12.99	13.71	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
				Channel	750	5.8	2.1	15.8
5	D-7	2.09	2.20	5.4	5.0	8.9	10.5	19.7
				TRAVEL TIME				
		2.09	2.20	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
					150	2.0	1.3	6.6
4	D-6	2.36	2.49	5.8	4.9	8.7	11.6	21.7
				TRAVEL TIME				
		2.36	2.49	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
					85	9.0	0.2	6.0
6	D-8 DP-E DP-Y	21.50	22.70	22.3	2.8	5.1	127.7	239.9
		12.99	13.71					
		10.34	10.92	TRAVEL TIME				
		44.83	47.33	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
				Pipe	36	10.0	0.1	22.4
7	DP-6 DP-5 (INLET) DP-4 (INLET)	44.83	47.33	22.4	2.8	5.1	140.2	264.8
		2.09	2.49					
		2.36	2.49	TRAVEL TIME				
		49.28	52.30	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
				Channel	110	8.4	0.2	22.6
8	DP-7 POND WU* WEST TRIB CHAN.* D-5	49.28	52.30	22.6	2.8	5.0	209.4	1243.5
		19.44	188.25					
		1.96	2.07					
		3.29	4.23	TRAVEL TIME				
		73.98	246.86	Type/flow	Length (ft)	Velocity (fps)	d. Time (min)	T. Time (min)
					83	5.8	0.2	22.8

* VALUES WERE OBTAINED FROM THE APPROVED MARKET PLACE FILING NO. 1 DRAINAGE REPORT

** VALUES WERE OBTAINED FROM LOWES DRAINAGE REPORT

**MERIDIAN CROSSING - FDR - PROPOSED CONDITIONS
INLET CALCULATIONS**

DP	Inlet size L(i)	INLET TYPE	CROSS SLOPE	STREET SLOPE	Q(5)	Q(100)	Q ₅					Q ₁₀₀						
							Qi	CA(eqv.)	FB	CA(eqv.)	DEPTH (max)	SPREAD	Qi	CA(eqv.)	FB	CA(eqv.)	DEPTH (max)	SPREAD
1	15	FLOW-BY	2.0%	1.0%	9	18	6.4	1.34	3	0.62	0.42	16.7	10.8	1.27	7	0.80	0.51	21.1
3	20	SUMP	2.0%	SAG	9	17	9.3	1.94	0	0.00	0.50		17.5	2.04	0	0.00	0.50	
4	25	SUMP	2.0%	SAG	12	22	11.6	2.36	0	0.00	0.50		21.7	2.49	0	0.00	0.50	
5	20	SUMP	2.0%	SAG	10	20	10.5	2.09	0	0.00	0.50		18.9	2.12	0.7	0.08	0.50	

MERIDIAN CROSSING - FDR - PROPOSED CONDITIONS PIPE ROUTING

DESIGN POINT	CONTRIBUTING BASINS	CA (equivalent)		Tc (min.)	INTENSITY		TOTAL FLOWS			
		CA(5)	CA(100)		I(5) (in/hr)	I(100) (in/hr)	Q(5) (cfs)	Q(100) (cfs)		
1	INLET DP-1	1.34	1.27	6.4	4.8	8.5	6	11		
		TRAVEL TIME								
		1.34	1.27	Type/flow 18" RCP	Length (ft) 270	Velocity (fps) 7.1	d. Time (min) 0.6	T. Time (min) 7.0		
X	DP-X	3.31	3.50	5.0	5.1	9.1	17	32		
		TRAVEL TIME								
		3.31	3.50	Type/flow 30" RCP	Length (ft) 400	Velocity (fps) 7.1	d. Time (min) 0.9	T. Time (min) 5.9		
Z	DP-B DP-2 DP-X	1.16	1.22	10.5	4.0	7.2	37	69		
		4.63	4.88							
		3.31	3.50	TRAVEL TIME						
		9.10	9.60	Type/flow 36" RCP	Length (ft) 400	Velocity (fps) 7.1	d. Time (min) 0.9	T. Time (min) 11.4		

**STREET CAPACITY
VERTICAL CURB
FOR 1/2 STREET SECTION**

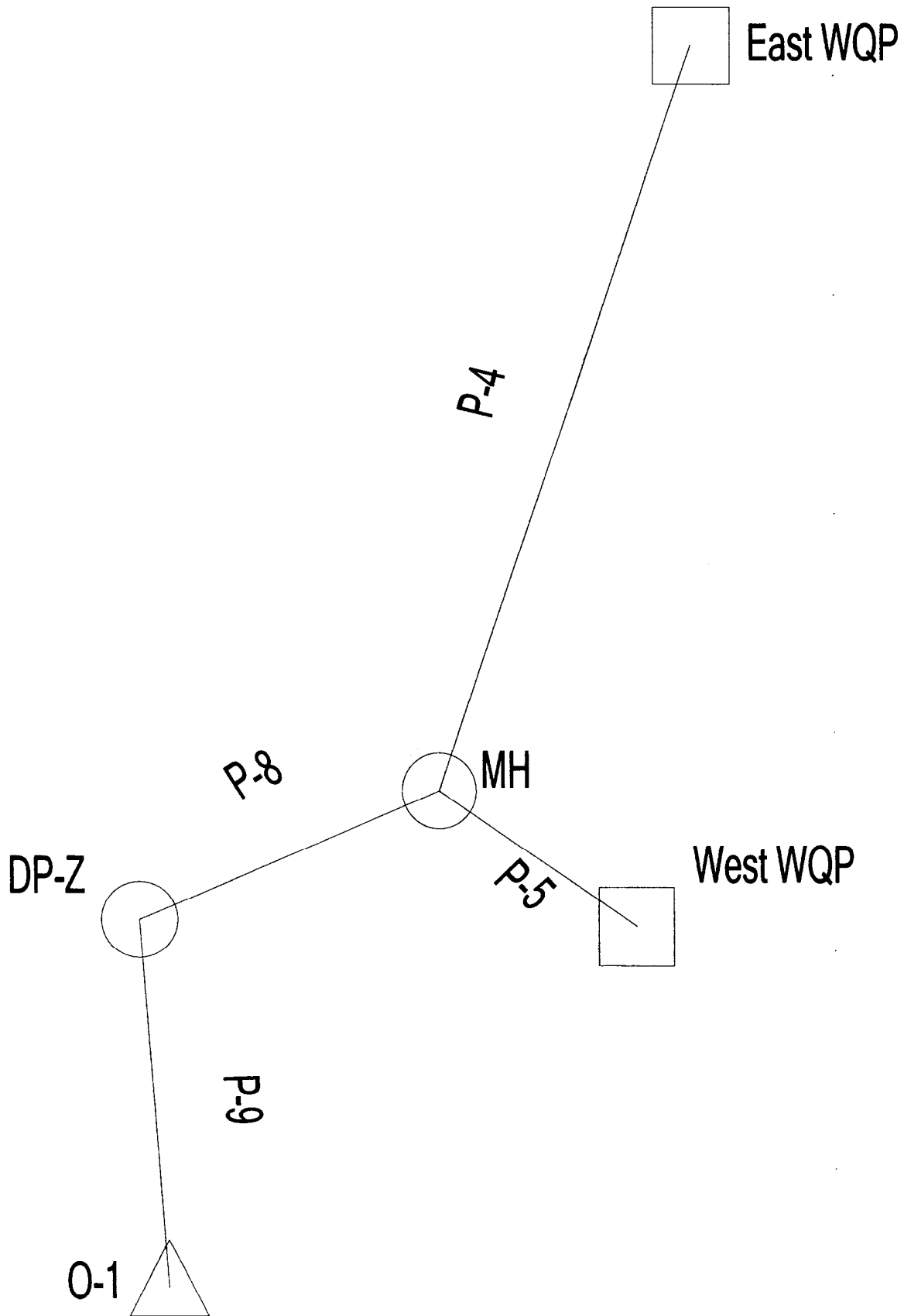
	Formula	Slope	Slope	n	Type	flow	Q _{max}	Q	Comments
Residential	Q=171.7 S ^{1/2}	0.5%	0.02	0.016	V	0.5	34	12.0	
		1.0%					34	17.0	
		1.5%					34	20.8	
		2.0%					34	24.1	
		2.5%					34	26.9	
		3.0%					34	29.5	
		3.5%					34	31.8	
		4.0%					34	34.0	
Collector/Arterial	Q=171.7 S ^{1/2}	0.5%	0.02	0.016	V	0.5	34	12.0	
		1.0%					34	17.0	
		1.5%					34	20.8	
		2.0%					34	24.1	
		2.5%					34	26.9	
		3.0%					34	29.5	
		3.5%					34	31.8	
		4.0%					34	34.0	

**STREET CAPACITY
RAMP CURB
FOR 1/2 STREET SECTION**

	Formula	Slope	Slope	n	Type	flow	Q _{max}	Q	Comments
Residential	Q=112.6 S ^{1/2}	0.5%	0.02	0.016	R	0.5	20	8.0	County ramp curb is 6"
		1.0%					20	11.3	
		1.5%					20	13.8	
		2.0%					20	15.9	
		2.5%					20	17.8	
		3.0%					20	19.5	
		3.15%					20	20.0	

Appendix E: StormCAD Calculations

Scenario: 100-year



Analysis Results Scenario: 100-year

Note:
The input data may have been modified since the last calculation was performed.
The calculated results may be outdated.

Title: Old Meridian Road
Project Engineer: Charlene Sammons
Project Date: 11/14/07
Comments: Storm in Old Meridian Road for Meridian Crossing Storm Sewer

Scenario Summary

Scenario	100-year
Physical Properties Alternat	Base-Physical Properties
Catchments Alternative	Catchments-100-year
System Flows Alternative	Base-System Flows
Structure Headlosses Altern	Base-Structure Headlosses
Boundary Conditions Altern	Base-Boundary Conditions
Design Constraints Alternat	Base-Design Constraints
Capital Cost Alternative	Base-Cost
User Data Alternative	Base-User Data

Network Inventory

Number of Pipes	4	Number of Inlets	2
- Circular Pipes:	4	- Grate Inlets:	0
- Box Pipes:	0	- Curb Inlets:	0
- Arch Pipes:	0	- Combination Inlets:	0
- Vertical Elliptical Pipes:	0	- Slot Inlets:	0
- Horizontal Elliptical Pipes:	0	- Grate Inlets in Ditch:	0
Number of Junctions	2	- Generic Inlets:	2
Number of Outlets	1		

Circular Pipes Inventory

30 inch	426.11 ft	42 inch	167.58 ft
36 inch	5.58 ft		
Total Length	599.27 ft		

Generic Inlet Inventory

Default 100%	2
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Junction elements for network with outlet: O-1

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Method	System Additional Flow (cfs)	System Known Flow (cfs)	System Rational Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
DP-Z	3,841.13	3,840.07	1.06	Standard	0.00	0.00	72.94	8.64	6.22	8.38
MH	3,842.25	3,841.42	0.83	Standard	0.00	0.00	73.34	8.68	6.09	8.38

Inlet elements for network with outlet: O-1

Label	Inlet	Total System Flow (cfs)	Total Intercepted Flow (cfs)	Total Bypassed Flow (cfs)	Capture Efficiency (%)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Method
East WQP	Generic Default	132.00	32.00	0.00	N/A	3,844.85	3,844.85	0.00	Absolut
West WQP	Generic Default	144.27	44.27	0.00	N/A	3,842.28	3,842.28	0.00	Absolut

Analysis Results

Scenario: 100-year

Outlet: O-1

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Additional Flow (cfs)	System Known Flow (cfs)	System Rational Flow (cfs)	System Intensity (in/hr)	System Flow (min)	System Time (min)	System CA (acres)
O-1	3,836.40	3,836.40	0.00	0.00	0.00	72.41	8.57	6.40	8.38	

Pipe elements for network with outlet: O-1

Label	Section Shape	Section Size	Length (ft)	Number of Sections	Constructe Slope (ft/ft)	Energy Slope (ft/ft)	Total System Flow (cfs)	Average Velocity (ft/s)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)
P-4	Circular	30 inch	26.11	1	0.008	0.006	32.00	6.52	3,842.25	6,839.05	3,844.85	3,842.25
P-5	Circular	36 inch	5.58	1	0.047	0.004	44.27	6.26	3,839.11	6,838.85	3,842.28	3,842.25
P-8	Circular	42 inch	60.40	1	0.011	0.005	73.34	7.67	3,838.05	6,837.39	3,841.42	3,841.13
P-9	Circular	42 inch	07.18	1	0.009	0.008	72.94	10.03	3,837.39	6,836.40	3,840.07	3,838.71

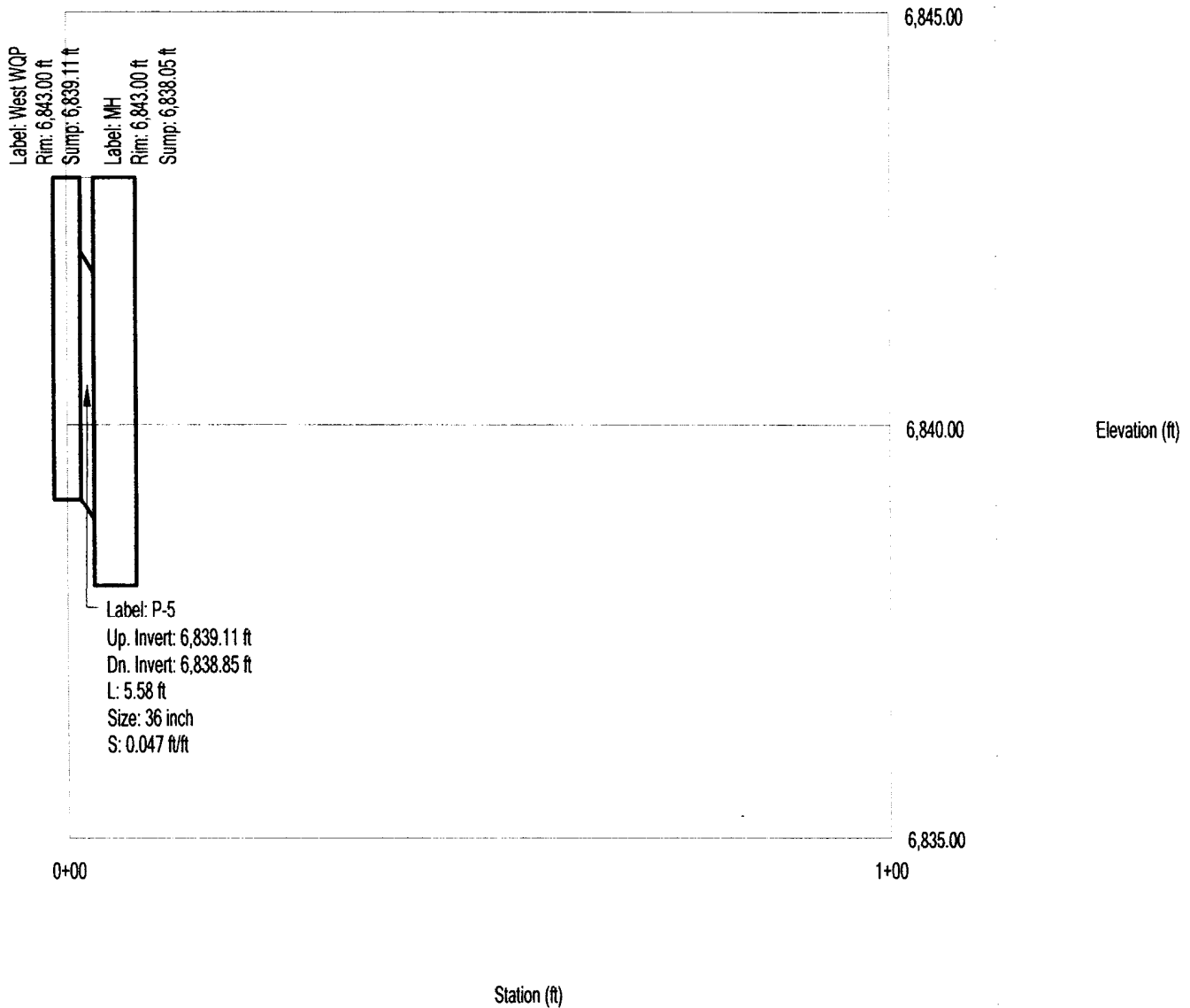
Scenario: 100-year

Combined PipeNode Report

Label	Up. Node	Dn. Node	L (ft)	Up. Inlet Area (acres)	Up. Inlet Rat. Coef.	Up. Inlet Area (acres)	Up. Calc. Sys. CA (acres)	Up. Inlet Rat. Q (cfs)	Size	Q Full (cfs)	Avg. v (ft/s)	Up. Gr Elev. (ft)	HGL In (ft)	Up. Invert (ft)	Dn. Gr. Elev. (ft)	HGL Out (ft)	Dn. Invert (ft)	S (ft/ft)
P-4	East WQF MH		426.11	3.50	1.00	3.50	3.50	32.00	30 inch	35.54	6.52	6,845.00	6,844.85	6,842.25	6,843.00	6,842.25	6,839.05	0.008
P-5	West WQF MH		5.58	4.88	1.00	4.88	4.88	44.27	36 inch	143.97	6.26	6,843.00	6,842.28	6,839.11	6,843.00	6,842.25	6,838.85	0.047
P-8	MH DP-Z		60.40	N/A	N/A	8.38	8.38	N/A	42 inch	105.16	7.57	6,843.00	6,841.42	6,838.05	6,841.75	6,841.13	6,837.39	0.011
P-9	DP-Z O-1		107.18	N/A	N/A	8.38	8.38	N/A	42 inch	96.69	10.03	6,841.75	6,840.07	6,837.39	6,841.00	6,838.71	6,836.40	0.009

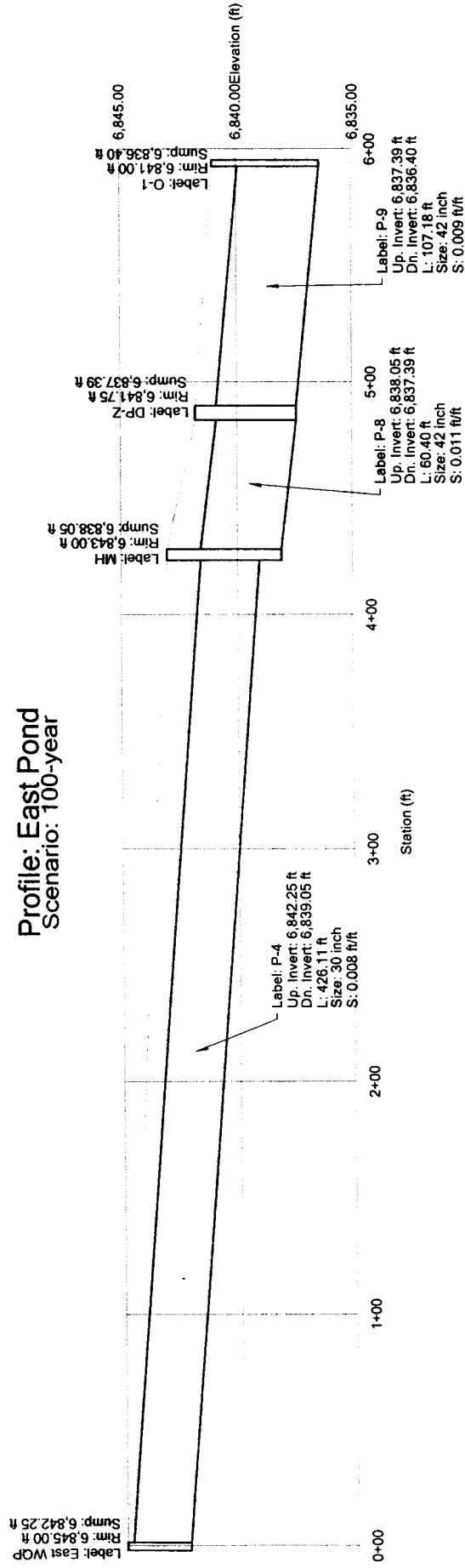
Profile
Scenario: 100-year

Profile: West Pond
Scenario: 100-year

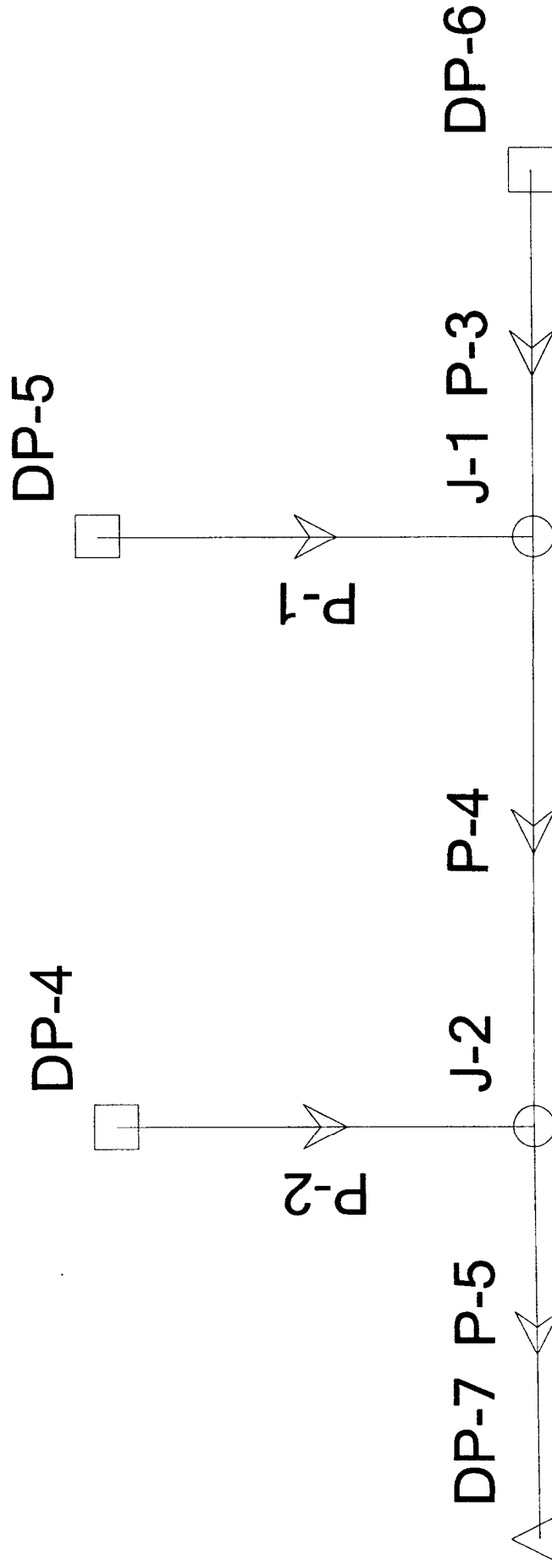


Profile
Scenario: 100-year

Profile: East Pond
Scenario: 100-year



Scenario: 100-year



Analysis Results Scenario: 100-year

Title: Meridian Crossing
 Project Engineer: Charlene Sammons
 Project Date: 01/31/05
 Comments: Analysis of existing storm system downstream of Meridian Crossing Project in Falcon, CO

Scenario Summary

Scenario	100-year
Physical Properties Alternat	Base-Physical Properties
Catchments Alternative	Catchments-100-year
System Flows Alternative	Base-System Flows
Structure Headlosses Alterr	Base-Structure Headlosses
Boundary Conditions Altern	Base-Boundary Conditions
Design Constraints Alternat	Base-Design Constraints
Capital Cost Alternative	Base-Cost
User Data Alternative	Base-User Data

Network Inventory

Number of Pipes	5	Number of Inlets	3
- Circular Pipes:	2	- Grate Inlets:	0
- Box Pipes:	3	- Curb Inlets:	2
- Arch Pipes:	0	- Combination Inlets:	0
- Vertical Elliptical Pipes:	0	- Slot Inlets:	0
- Horizontal Elliptical Pipes:	0	- Grate Inlets in Ditch:	0
Number of Junctions	2	- Generic Inlets:	1
Number of Outlets	1		

Circular Pipes Inventory

24 inch	185.50 ft
Total Length	185.50 ft

Box Pipes Inventory

6 x 3 ft	366.02 ft
Total Length	366.02 ft

Curb Inlet Inventory

Type R 10'	2
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Generic Inlet Inventory

Default 100%	1
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Inlet elements for network with outlet: DP-7

Label	Inlet	Total Flow (cfs)	Total Flow (cfs)	Total Bypassed Flow (cfs)	Capture Target (%)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Headloss (ft)	Headloss Method
DP-4	Curb Type R 10'	22.05	22.05	0.00	N/A	3,818.78	3,818.78	0.00	Absolut
DP-5	Curb Type R 10'	19.80	19.80	0.00	N/A	3,818.70	3,818.70	0.00	Absolut
DP-6	Generic Default	150.69	250.69	0.00	N/A	3,817.78	3,817.78	0.00	Absolut

Analysis Results

Scenario: 100-year

Outlet: DP-7

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Flow (cfs)	System Flow (cfs)	System Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
DP-7	3,814.44	3,814.44	0.00	0.00	0.00	273.30	5.21	22.65	52.02

Junction elements for network with outlet: DP-7

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	Headloss Method	System Flow (cfs)	System Flow (cfs)	System Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
J-1	3,817.71	3,817.71	0.00	Absolut	0.00	0.00	262.00	5.25	22.36	49.53
J-2	3,817.08	3,817.08	0.00	Absolut	0.00	0.00	273.54	5.22	22.61	52.02

Pipe elements for network with outlet: DP-7

Label	Section Shape	Section Size	Length (ft)	Number of Sections	Constructed Slope (ft/ft)	Energy Slope (ft/ft)	Total System Flow (cfs)	Average Velocity (ft/s)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)
P-1	Circular	24 inch	94.00	1	0.009043	008925	19.80	7.56	3,817.10	6,816.25	3,818.70	3,817.76
P-2	Circular	24 inch	91.50	1	0.016940	014085	22.05	8.96	3,817.10	6,815.55	3,818.78	3,816.86
P-3	Box	6 x 3 ft	28.85	2	0.005199	005093	50.69	8.63	3,815.40	6,815.25	3,817.78	3,817.71
P-4	Box	6 x 3 ft	33.50	2	0.005243	005221	62.00	8.76	3,815.25	6,814.55	3,817.71	3,817.08
P-5	Box	6 x 3 ft	20.66	2	0.005324	004607	73.54	9.31	3,814.55	6,814.44	3,817.08	3,816.82

Scenario: 100-year

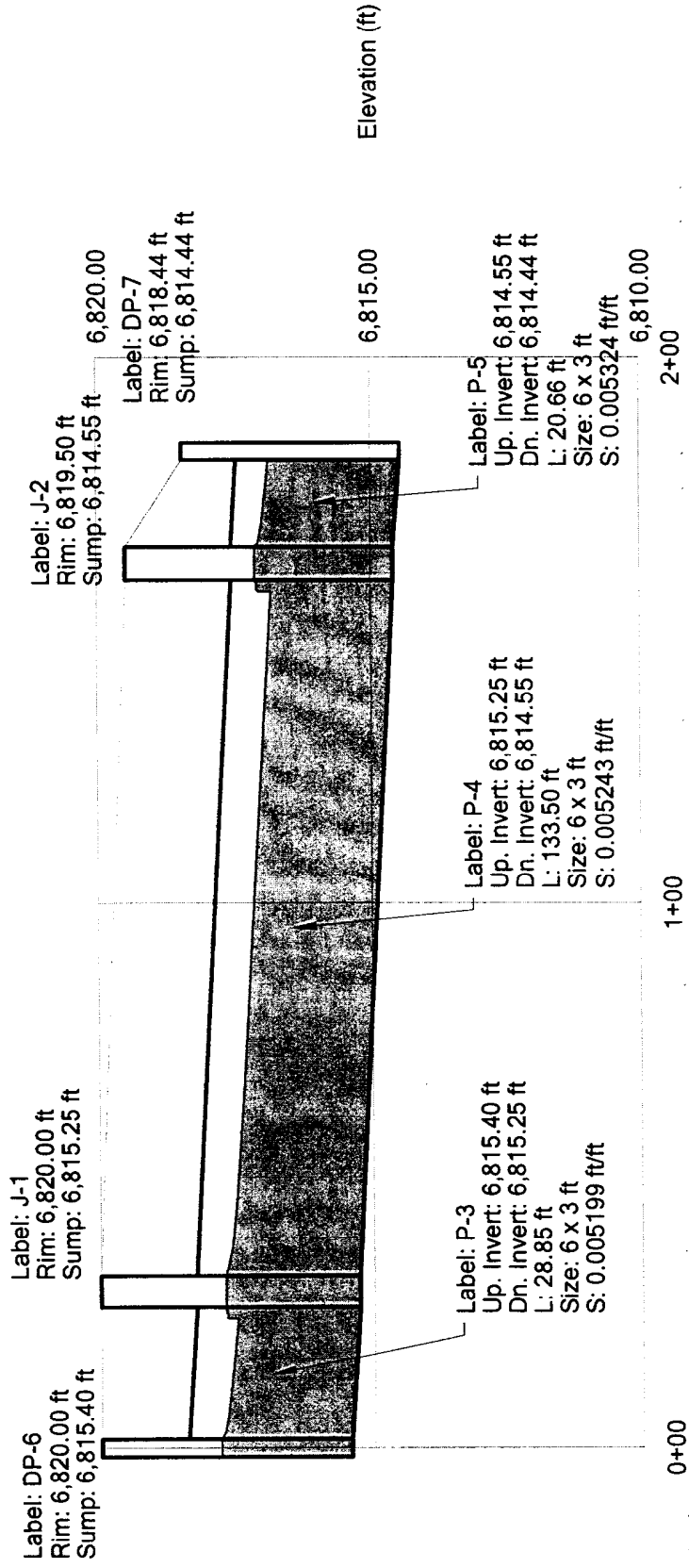
Combined Pipe\Node Report

Label	Up. Node	Dn. Node	L (ft)	Up. Inlet Area (acres)	Up. Inlet Rat. Coef.	Up. Inlet Area (acres)	Up. Calc. Sys. CA (acres)	Up. Inlet Rat. Q (cfs)	Size	Q Full (cfs)	Avg. V (ft/s)	Up. Gr. Elev. (ft)	HGL In (ft)	Up. Invert (ft)	Dn. Gr. Elev. (ft)	HGL Out (ft)	Dn. Invert (ft)	S (ft/ft)
P-2	DP-4	J-2	91.50	2.49	1.00	2.49	2.49	22.05	24 inch	29.44	8.96	6,821.16	6,818.78	6,817.10	6,819.50	6,816.86	6,815.55	0.016940
P-1	DP-5	J-1	94.00	2.20	1.00	2.20	2.20	19.80	24 inch	21.51	7.56	6,821.16	6,818.70	6,817.10	6,820.00	6,817.76	6,816.25	0.009043
P-3	DP-6	J-1	28.85	47.33	1.00	47.33	47.33	250.69	6 x 3 ft	296.71	8.63	6,820.00	6,817.78	6,815.40	6,820.00	6,817.71	6,815.25	0.005199
P-4	J-1	J-2	133.50	N/A	N/A	49.53	49.53	N/A	6 x 3 ft	297.96	8.76	6,820.00	6,817.71	6,815.25	6,819.50	6,817.08	6,814.55	0.005243
P-5	J-2	DP-7	20.66	N/A	N/A	52.02	52.02	N/A	6 x 3 ft	300.25	9.31	6,819.50	6,817.08	6,814.55	6,818.44	6,816.82	6,814.44	0.005324

Profile
Scenario: 100-year

Profile: RCB @ Highway 24

Scenario: 100-year

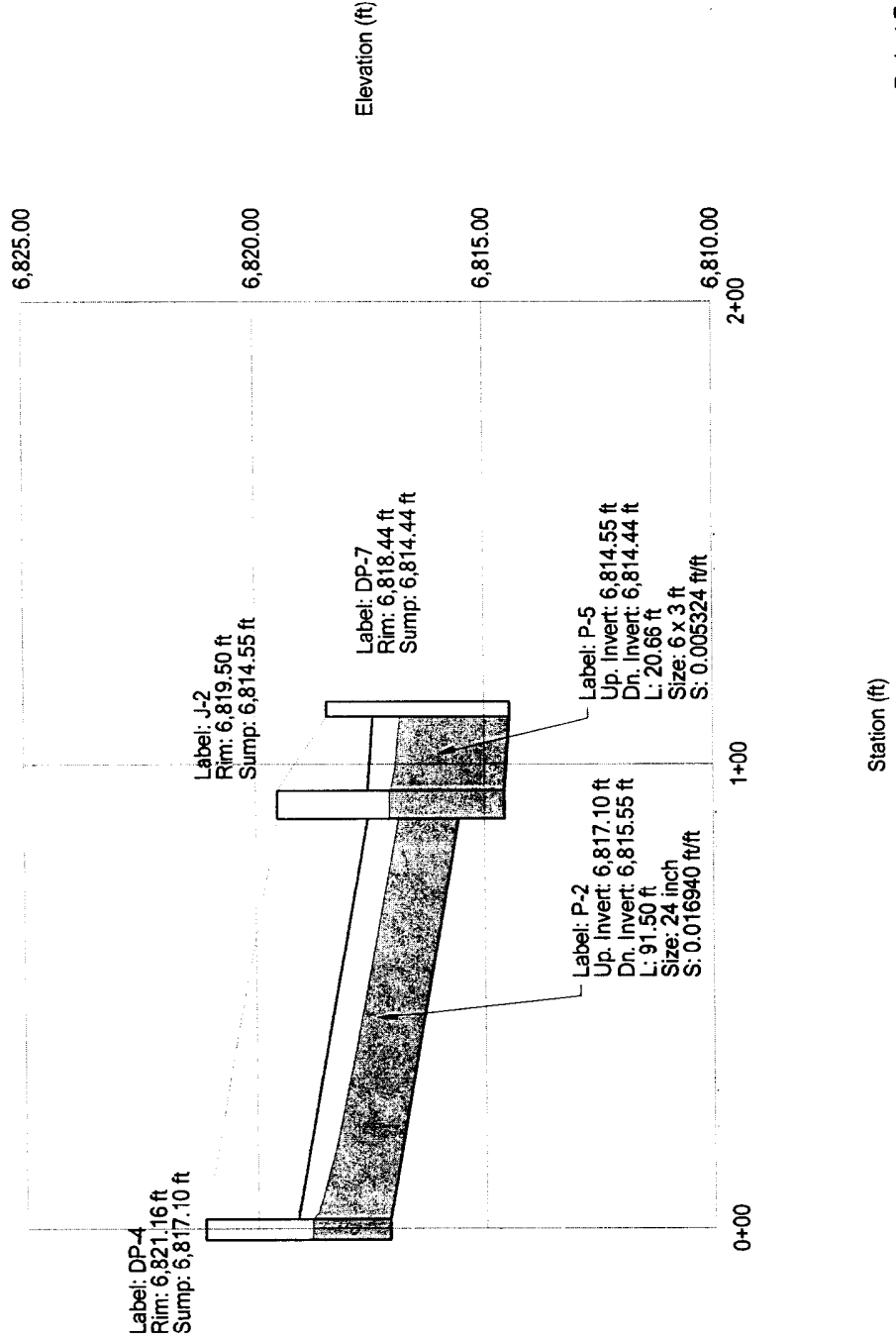


Station (ft)

Profile
Scenario: 100-year

Profile: Sump Inlet Left

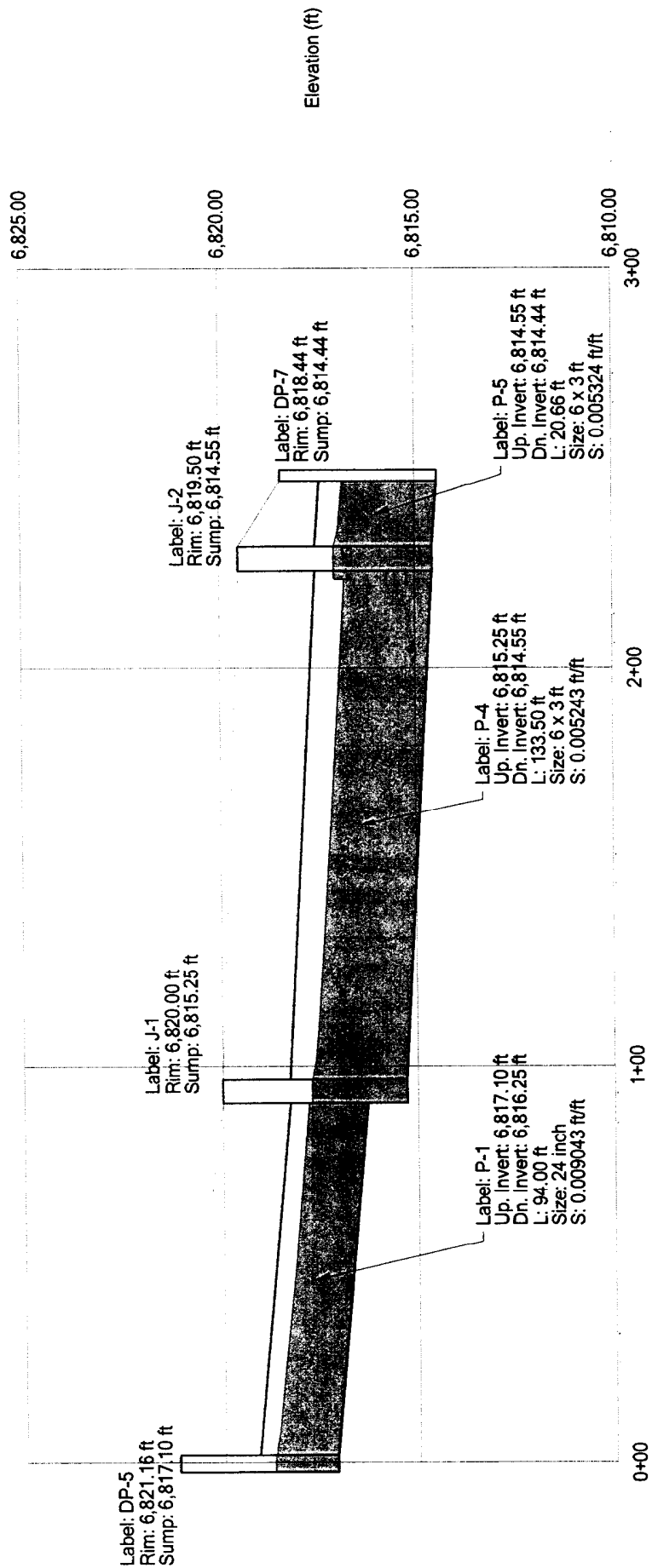
Scenario: 100-year



Profile
Scenario: 100-year

Profile: Sump Inlet Right

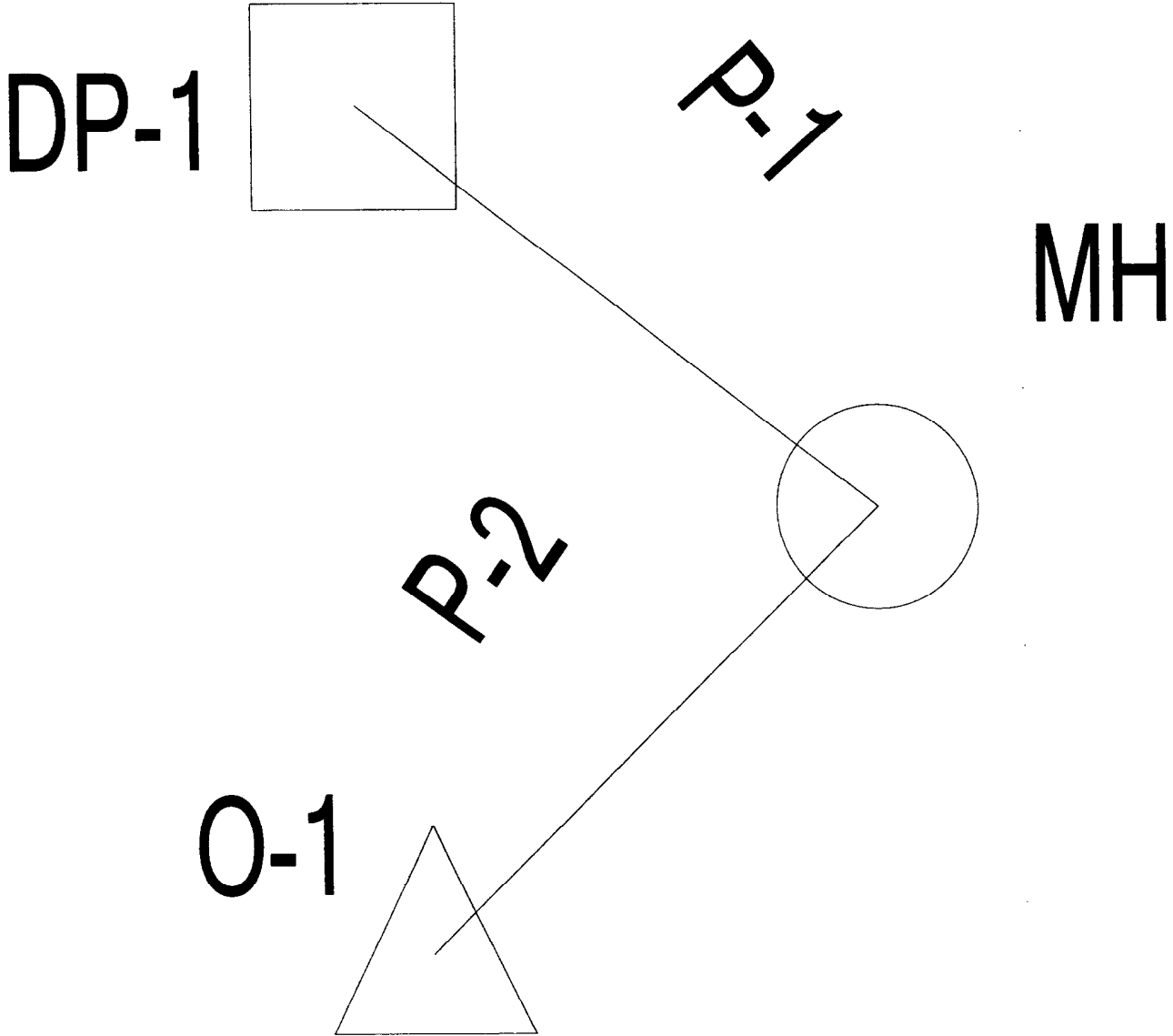
Scenario: 100-year



Station (ft)

Project Engineer: Charlene Sammons
StormCAD v5.6 [05.06.012.00]
Page 1 of 1

Scenario: 100-year



Analysis Results

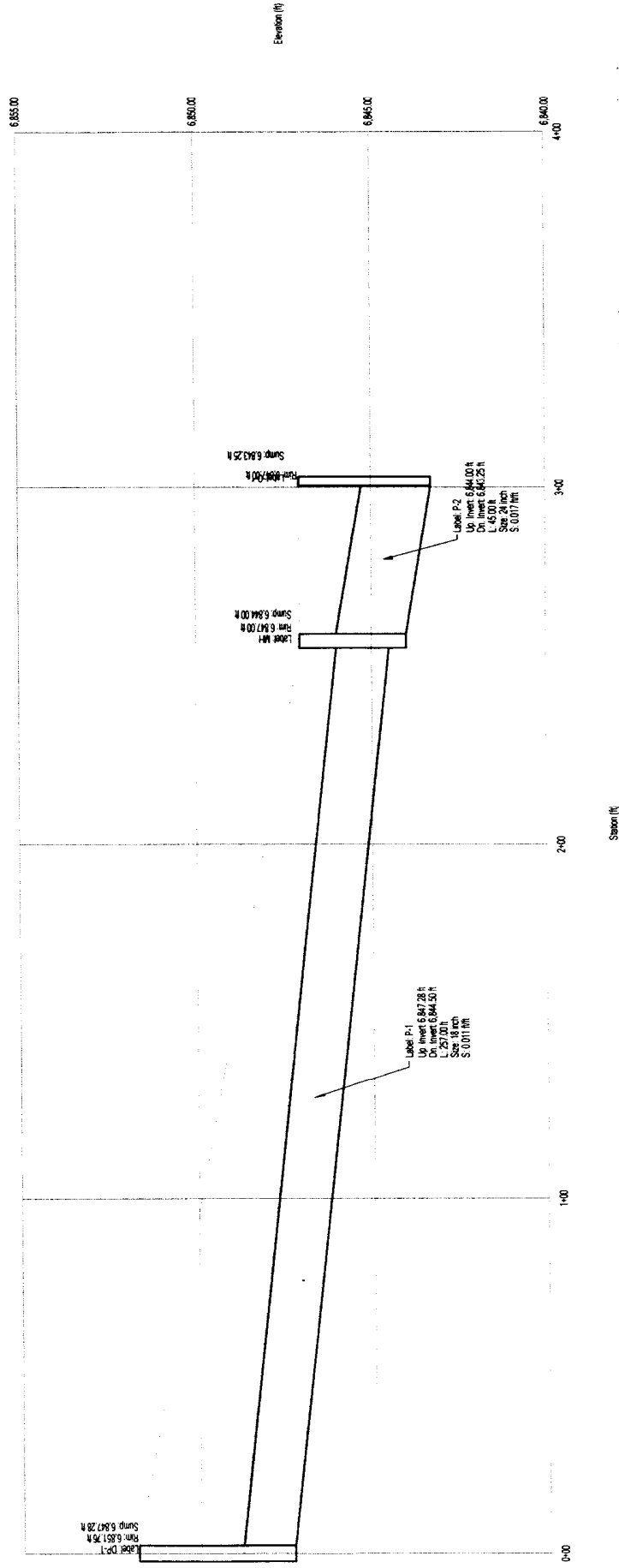
Scenario: 100-year

Outlet: O-1									
Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Additional Flow (cfs)	System Known Flow (cfs)	System Rational Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
O-1	3,843.25	3,843.25	0.00	0.00	0.00	7.35	8.28	7.22	0.88

Pipe elements for network with outlet: O-1												
Label	Section Shape	Section Size	Length (ft)	Number of Sections	Constructed Slope (ft/ft)	Energy Slope (ft/ft)	Total System Flow (cfs)	Average Velocity (ft/s)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)
P-1	Circular	18 inch	57.00	1	0.011	0.011	7.61	6.16	3,847.28	6,844.50	3,848.35	3,845.42
P-2	Circular	24 inch	45.00	1	0.017	0.012	7.39	6.17	3,844.00	6,843.25	3,844.97	3,843.96

Profile
Scenario: 100-year

Profile: Ultimate Design
Scenario: 100-year



Existing Channel along Highway 24 ROW
Channel Calculator

Given Input Data:

Shape	Trapezoidal
Solving for	Depth of Flow
Flowrate	65.1000 cfs
Slope	0.0090 ft/ft
Manning's n	0.0350
Height	36.0000 in
Bottom width	480.0000 in
Left slope	0.2500 ft/ft (V/H)
Right slope	0.2500 ft/ft (V/H)

Computed Results:

Depth	6.8903 in
Velocity	2.6805 fps
Full Flowrate	1129.3728 cfs
Flow area	24.2864 ft2
Flow perimeter	536.8189 in
Hydraulic radius	6.5148 in
Top width	535.1224 in
Area	156.0000 ft2
Perimeter	776.8636 in
Percent full	19.1397 %

Critical Information

Critical depth	5.1448 in
Critical slope	0.0241 ft/ft
Critical velocity	3.6400 fps
Critical area	17.8847 ft2
Critical perimeter	522.4254 in
Critical hydraulic radius	4.9297 in
Critical top width	521.1587 in
Specific energy	0.6859 ft
Minimum energy	0.6431 ft
Froude number	0.6404
Flow condition	Subcritical

Existing Temporary Channel along Meridian Road (DP-E)
Channel Calculator

Given Input Data:

Shape	Trapezoidal
Solving for	Depth of Flow
Flowrate	88.2000 cfs
Slope	0.0200 ft/ft
Manning's n	0.0350
Height	36.0000 in
Bottom width	0.0000 in
Left slope	0.2500 ft/ft (V/H)
Right slope	0.1667 ft/ft (V/H)

Computed Results:

Depth	21.4874 in
Velocity	5.5023 fps
Full Flowrate	349.2362 cfs
Flow area	16.0296 ft2
Flow perimeter	219.2720 in
Hydraulic radius	10.5269 in
Top width	214.8480 in
Area	44.9946 ft2
Perimeter	367.3687 in
Percent full	59.6872 %

Critical Information

Critical depth	21.7023 in
Critical slope	0.0190 ft/ft
Critical velocity	5.3939 fps
Critical area	16.3519 ft2
Critical perimeter	221.4655 in
Critical hydraulic radius	10.6322 in
Critical top width	216.9973 in
Specific energy	2.2611 ft
Minimum energy	2.7128 ft
Froude number	1.0252
Flow condition	Supercritical

Proposed Temporary Channel from Old Meridian Road (DP-Z)

Channel Calculator

Given Input Data:

Shape	Trapezoidal
Solving for	Depth of Flow
Flowrate	73.3000 cfs
Slope	0.0050 ft/ft
Manning's n	0.0350
Height	36.0000 in
Bottom width	60.0000 in
Left slope	0.2500 ft/ft (V/H)
Right slope	0.2500 ft/ft (V/H)

Computed Results:

Depth	21.9888 in
Velocity	3.2444 fps
Full Flowrate	219.3674 cfs
Flow area	22.5927 ft2
Flow perimeter	241.3242 in
Hydraulic radius	13.4813 in
Top width	235.9103 in
Area	51.0000 ft2
Perimeter	356.8636 in
Percent full	61.0800 %

Critical Information

Critical depth	16.0014 in
Critical slope	0.0192 ft/ft
Critical velocity	5.3195 fps
Critical area	13.7795 ft2
Critical perimeter	191.9505 in
Critical hydraulic radius	10.3373 in
Critical top width	188.0108 in
Specific energy	1.9960 ft
Minimum energy	2.0002 ft
Froude number	0.5336
Flow condition	Subcritical

Culvert Designer/Analyzer Report Highway 24 - DP WU

Analysis Component			
Storm Event	Check	Discharge	1,241.00 cfs

Peak Discharge Method: User-Specified			
Design Discharge	1,241.00 cfs	Check Discharge	1,241.00 cfs

Tailwater properties: Trapezoidal Channel

Tailwater conditions for Check Storm.			
Discharge	1,241.00 cfs	Bottom Elevation	6,813.00 ft
Depth	3.82 ft	Velocity	7.78 ft/s

Name	Description	Discharge	HW Elev.	Velocity
Culvert-1	3-12 x 6 ft Box	1,240.93 cfs	6,818.87 ft	8.71 ft/s
Weir	Roadway (Constant Elevation)	0.00 cfs	6,818.87 ft	N/A
Total	-----	1,240.93 cfs	6,818.87 ft	N/A

Culvert Designer/Analyzer Report Highway 24 - DP WU

Component: Culvert-1

Culvert Summary

Computed Headwater Elev:	6,818.87 ft	Discharge	1,240.93 cfs
Inlet Control HW Elev.	6,818.37 ft	Tailwater Elevation	6,816.82 ft
Outlet Control HW Elev.	6,818.87 ft	Control Type	Outlet Control
Headwater Depth/Height	0.96		

Grades

Upstream Invert	6,813.10 ft	Downstream Invert	6,812.87 ft
Length	47.00 ft	Constructed Slope	0.005000 ft/ft

Hydraulic Profile

Profile	S1	Depth, Downstream	3.96 ft
Slope Type	Steep	Normal Depth	2.78 ft
Flow Regime	Subcritical	Critical Depth	3.33 ft
Velocity Downstream	8.71 ft/s	Critical Slope	0.002971 ft/ft

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	12.00 ft
Section Size	12 x 6 ft	Rise	6.00 ft
Number Sections	3		

Outlet Control Properties

Outlet Control HW Elev.	6,818.87 ft	Upstream Velocity Head	1.54 ft
Ke	0.50	Entrance Loss	0.77 ft

Inlet Control Properties

Inlet Control HW Elev.	6,818.37 ft	Flow Control	N/A
Inlet Type	45° wingwall flares - offset	Area Full	216.0 ft ²
K	0.49700	HDS 5 Chart	13
M	0.66700	HDS 5 Scale	1
C	0.03020	Equation Form	2
Y	0.83500		

Culvert Designer/Analyzer Report McLaughlin Bridge

Component: Culvert-1

Culvert Summary

Computed Headwater Elev.	104.17 ft	Discharge	623.40 cfs
Inlet Control HW Elev.	104.17 ft	Tailwater Elevation	N/A ft
Outlet Control HW Elev.	103.98 ft	Control Type	Inlet Control
Headwater Depth/Height	1.39		

Grades

Upstream Invert	100.00 ft	Downstream Invert	100.00 ft
Length	50.00 ft	Constructed Slope	0.010000 ft/ft

Hydraulic Profile

Profile	S2	Depth, Downstream	1.99 ft
Slope Type	Steep	Normal Depth	1.76 ft
Flow Regime	Supercritical	Critical Depth	2.49 ft
Velocity Downstream	11.21 ft/s	Critical Slope	0.003719 ft/ft

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	7.00 ft
Section Size	7 x 3 ft	Rise	3.00 ft
Number Sections	4		

Outlet Control Properties

Outlet Control HW Elev.	103.98 ft	Upstream Velocity Head	1.24 ft
Ke	0.20	Entrance Loss	0.25 ft

Inlet Control Properties

Inlet Control HW Elev.	104.17 ft	Flow Control	N/A
Inlet Type	90° headwall w 45° bevels	Area Full	84.0 ft ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Culvert Designer/Analyzer Report Old Meridian Road - DP Y

Analysis Component			
Storm Event	Check	Discharge	65.10 cfs

Peak Discharge Method: User-Specified			
Design Discharge	34.70 cfs	Check Discharge	65.10 cfs

Tailwater properties: Trapezoidal Channel

Tailwater conditions for Check Storm.			
Discharge	65.10 cfs	Bottom Elevation	6,813.00 ft
Depth	0.89 ft	Velocity	3.45 ft/s

Name	Description	Discharge	HW Elev.	Velocity
Culvert-1	1-54 inch Circular	65.11 cfs	6,824.75 ft	9.95 ft/s
Weir	Roadway (Constant Elevation)	0.00 cfs	6,824.75 ft	N/A
Total	-----	65.11 cfs	6,824.75 ft	N/A

Culvert Designer/Analyzer Report Old Meridian Road - DP Y

Component: Culvert-1

Culvert Summary			
Computed Headwater Elev:	6,824.75 ft	Discharge	65.11 cfs
Inlet Control HW Elev.	6,824.43 ft	Tailwater Elevation	6,813.89 ft
Outlet Control HW Elev.	6,824.75 ft	Control Type	Entrance Control
Headwater Depth/Height	0.83		

Grades			
Upstream Invert	6,821.00 ft	Downstream Invert	6,820.50 ft
Length	50.00 ft	Constructed Slope	0.010000 ft/ft

Hydraulic Profile			
Profile	S2	Depth, Downstream	1.94 ft
Slope Type	Steep	Normal Depth	1.78 ft
Flow Regime	Supercritical	Critical Depth	2.35 ft
Velocity Downstream	9.95 ft/s	Critical Slope	0.003794 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	4.50 ft
Section Size	54 inch	Rise	4.50 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	6,824.75 ft	Upstream Velocity Head	0.93 ft
Ke	0.50	Entrance Loss	0.47 ft

Inlet Control Properties			
Inlet Control HW Elev.	6,824.43 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	15.9 ft ²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Designer/Analyzer Report

Old Meridian Road - DP Y

Component: Weir

Hydraulic Component(s): Roadway (Constant Elevation)

Discharge	0.00 cfs	Allowable HW Elevation	6,824.75 ft
Roadway Width	44.00 ft	Overtopping Coefficient	2.90 US
Length	150.00 ft	Crest Elevation	6,825.35 ft
Headwater Elevation	N/A ft	Discharge Coefficient (Cr)	2.90
Submergence Factor (Kt)	1.00		

Sta (ft)	Elev. (ft)
0.00	6,825.35
150.00	6,825.35

Culvert Outlet Protection

Meridian Crossing - Final Drainage Report

Culvert	Diameter (in)	No of Barrels	Slope (%)	Velocity ft/s	Riprap Width ft	Riprap Length ft	Riprap Size
DP-Y	54	1	1.20%	10.5	13.5	21.5	3.78 L
DP-Z	42	1	0.90%	10.0	10.5	18.5	3.44 L

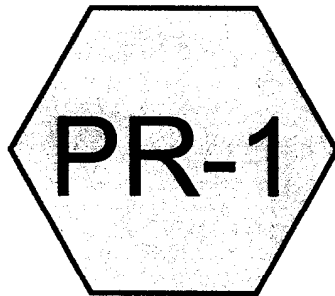
Appendix G: Water Quality Pond Calculations

Design Procedure Form: Porous Landscape Detention (PLD)

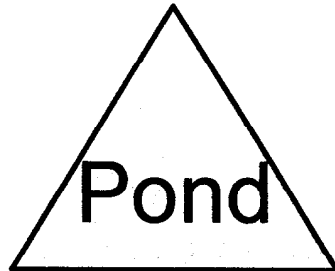
Designer: Thomas Roberts
Company: Springs Engineering
Date: July 23, 2008
Project: Meridian Crossing East Pond
Location: Falcon, CO

<p>1. Basin Storage Volume ($I_a = 100\%$ if all paved and roofed areas u/s of PLD) A) Tributary Area's Imperviousness Ratio ($i = I_a / 100$) B) Contributing Watershed Area Including the PLD (Area) C) Water Quality Capture Volume (WQCV) (WQCV = $0.8 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i)$) D) Design Volume: $Vol_{PLD} = (WQCV / 12) * Area$</p>	<p> $I_a = \underline{79.00} \%$ $i = \underline{0.79}$ Area = <u>167,616</u> square feet WQCV = <u>0.26</u> watershed inches Vol = <u>3,600</u> cubic feet </p>				
<p>2. PLD Surface Area (A_{PLD}) and Average Depth (d_{av}) (from 3600.24 square feet to 7200.48 square feet) ($d_{av} = (Vol / A_{PLD})$, Min=0.5', Max=1.0')</p>	<p> $A_{PLD} = \underline{3,600}$ square feet $d_{av} = \underline{1.00}$ feet </p>				
<p>3. Draining of PLD (Check A, or B, or C, answer D) Based on answers to 3A through 3D, check the appropriate method</p> <p>A) Check box if subgrade is heavy or expansive clay <input type="checkbox"/></p> <p>B) Check box if subgrade is silty or clayey sand <input type="checkbox"/></p> <p>C) Check box if subgrade is well-draining soil <input checked="" type="checkbox"/></p> <p>D) Check box if underdrains are not desirable or if underdrains are not feasible at this site. <input type="checkbox"/></p> <p>E) Does tributary catchment contain land uses that may have petroleum products, greases, or other chemicals present, such as gas station, hardware store, restaurant, etc.?</p> <table style="margin-left: 20px;"> <tr> <td style="padding: 0 10px;">yes</td> <td style="padding: 0 10px;">no</td> </tr> <tr> <td style="text-align: center;"><input checked="" type="checkbox"/></td> <td style="text-align: center;"><input type="checkbox"/></td> </tr> </table>	yes	no	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<p> <input type="checkbox"/> Infiltration to Subgrade with Permeable Membrane: 3(C) checked and 3(E) = no <input type="checkbox"/> Underdrain with Impermeable Liner: 3(A) checked or 3(E) = yes <input type="checkbox"/> Underdrain with Non-Woven Geotextile Fabric: 3(B) checked and 3(E) = no <input type="checkbox"/> 16-Mil. Impermeable Membrane with No Underdrain: 3(D) checked - Evapotranspiration only <input checked="" type="checkbox"/> Other: <u>Type D Inlet</u> </p>
yes	no				
<input checked="" type="checkbox"/>	<input type="checkbox"/>				
<p>4. Sand/Peat Mix and Gravel Subbase (See Figure PLD-1)</p> <p>A) Heavy or Expansive Clay (NRCS Group D Soils) Present; Perforated HDPE Underdrain Used.</p> <p>B) Silty or Clayey Sand (NRCS Group C Soils) Present; Perforated HDPE Underdrain Used.</p> <p>C) No Potential For Contamination And Well-Draining (NRCS Group A or B Soils) Are Present; Underdrains Eliminated.</p> <p>D) Underdrains Are Not Desirable Or Are Not Feasible At This Site.</p> <p>E) Other:</p>	<p> <input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with 8" Gravel Layer. 16-Mil. Impermeable Liner and a 3" to 4" Perforated HDPE Underdrain. <input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with 8" Gravel Layer and a 3" to 4" Perforated HDPE Underdrain w/ Non-Woven Pemeable Membrane. <input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with Non-Woven Pemeable Membrane and No Underdrain (Direct Infiltration). <input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with An Additional 18" Minimum Layer Sand-Peat Mix or Sand-Class 'A' Compost Bottom Layer (Total Sand-Peat Depth of 36"). 16-Mil. Impermeable Liner Used. <input checked="" type="checkbox"/> Other: <u>See Detail on Sheet 8</u> </p>				

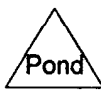
Notes: _____



Basin D-9



East PLD



Pond Pond: East PLD

Inflow Area = 3.680 ac, Inflow Depth = 0.56" for 2-Year event
 Inflow = 8.32 cfs @ 0.09 hrs, Volume= 0.172 af
 Outflow = 6.50 cfs @ 0.27 hrs, Volume= 0.134 af, Atten= 22%, Lag= 10.7 min
 Primary = 6.50 cfs @ 0.27 hrs, Volume= 0.134 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,844.47' @ 0.27 hrs Surf.Area= 3,394 sf Storage= 4,371 cf

Plug-Flow detention time= 12.8 min calculated for 0.134 af (78% of inflow)
 Center-of-Mass det. time= 10.8 min (20.9 - 10.0)

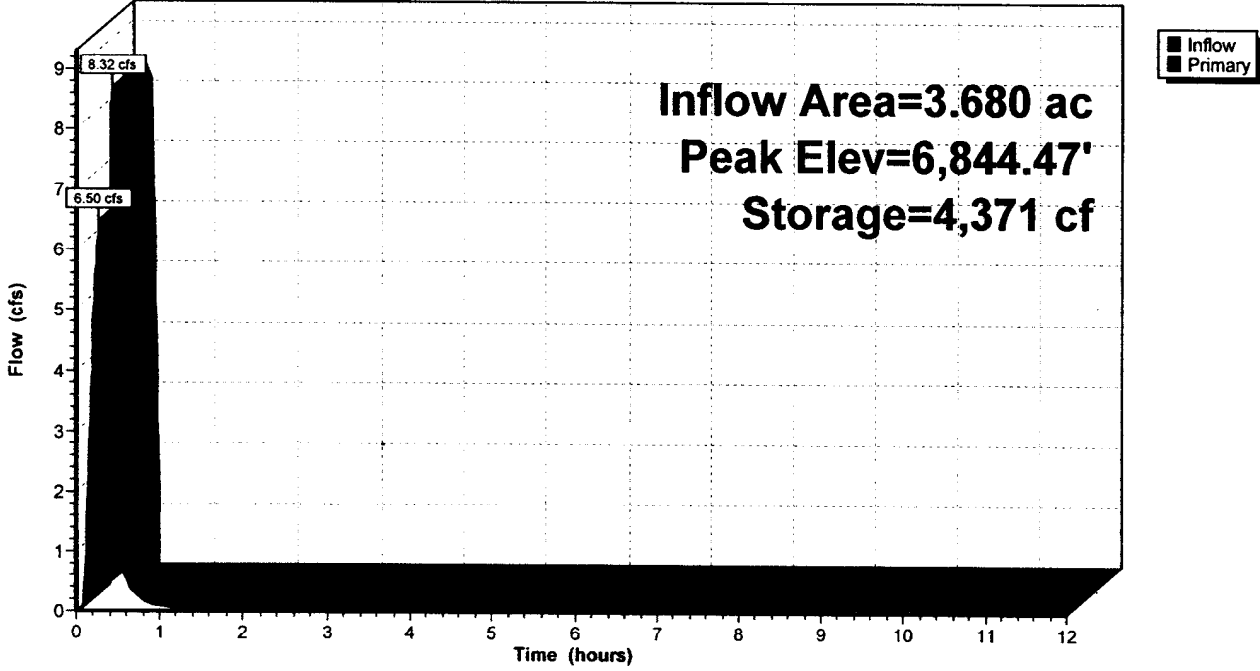
Volume	Invert	Avail.Storage	Storage Description		
#1	6,843.00'	9,728 cf	Custom Stage Data (Pyramidal) Listed below		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
6,843.00	2,200	0	0	2,200	
6,844.00	3,300	2,731	2,731	3,320	
6,846.00	3,700	6,996	9,728	3,939	

Device	Routing	Invert	Outlet Devices	
#1	Primary	6,845.50'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000	
#2	Primary	6,843.60'	2.50' W x 1.25' H Vert. Orifice/Grate C= 0.600	

Primary OutFlow Max=6.49 cfs @ 0.27 hrs HW=6,844.47' (Free Discharge)
 1=Orifice/Grate (Controls 0.00 cfs)
 2=Orifice/Grate (Orifice Controls 6.49 cfs @ 2.99 fps)

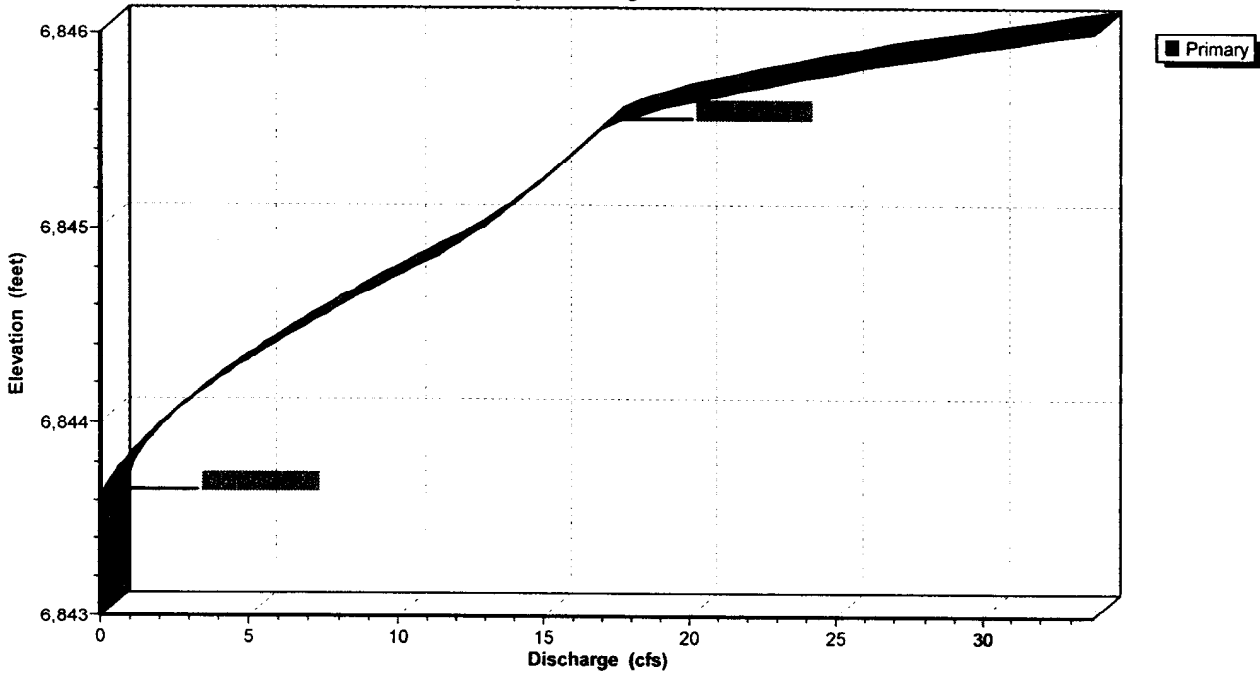
Pond Pond: East PLD

Hydrograph



Pond Pond: East PLD

Stage-Discharge



Pond Pond: East PLD

Inflow Area = 3.680 ac, Inflow Depth = 0.77" for 5-Year event
 Inflow = 11.42 cfs @ 0.09 hrs, Volume= 0.236 af
 Outflow = 9.65 cfs @ 0.26 hrs, Volume= 0.198 af, Atten= 16%, Lag= 10.4 min
 Primary = 9.65 cfs @ 0.26 hrs, Volume= 0.198 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,844.73' @ 0.26 hrs Surf.Area= 3,446 sf Storage= 5,288 cf

Plug-Flow detention time= 10.5 min calculated for 0.198 af (84% of inflow)
 Center-of-Mass det. time= 9.4 min (19.4 - 10.0)

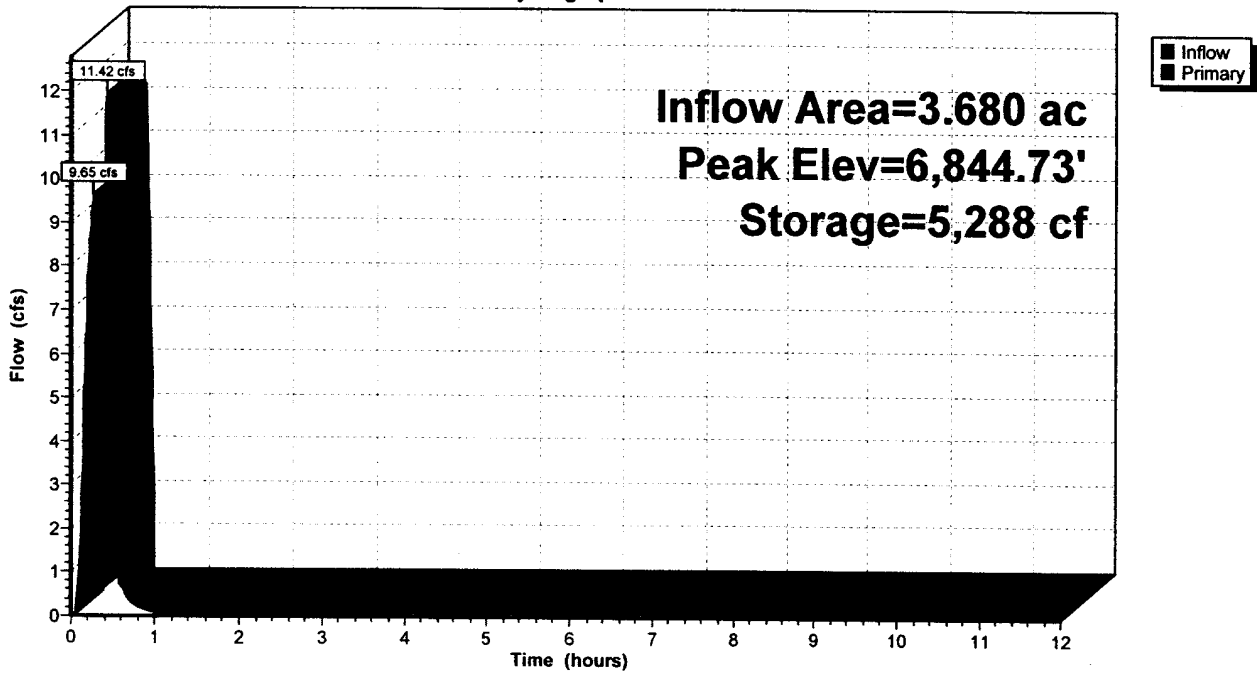
Volume	Invert	Avail.Storage	Storage Description		
#1	6,843.00'	9,728 cf	Custom Stage Data (Pyramidal) Listed below		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
6,843.00	2,200	0	0	2,200	
6,844.00	3,300	2,731	2,731	3,320	
6,846.00	3,700	6,996	9,728	3,939	

Device	Routing	Invert	Outlet Devices		
#1	Primary	6,845.50'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000		
#2	Primary	6,843.60'	2.50' W x 1.25' H Vert. Orifice/Grate C= 0.600		

Primary OutFlow Max=9.63 cfs @ 0.26 hrs HW=6,844.73' (Free Discharge)
 1=Orifice/Grate (Controls 0.00 cfs)
 2=Orifice/Grate (Orifice Controls 9.63 cfs @ 3.41 fps)

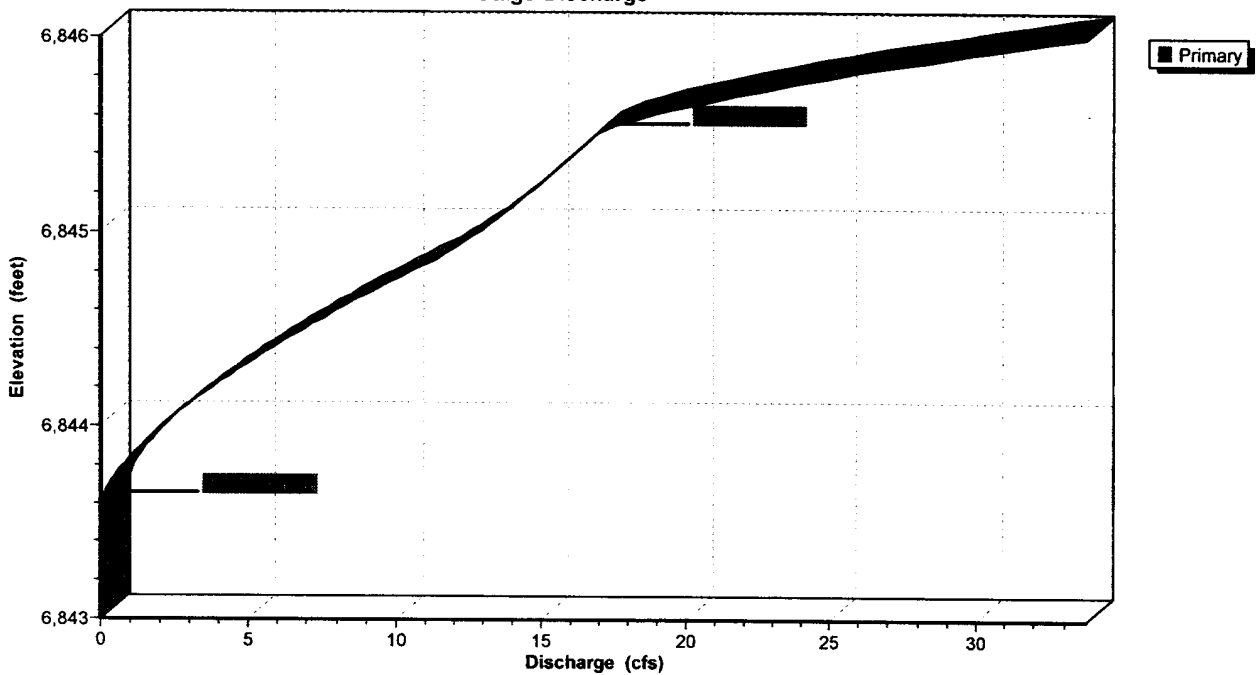
Pond Pond: East PLD

Hydrograph



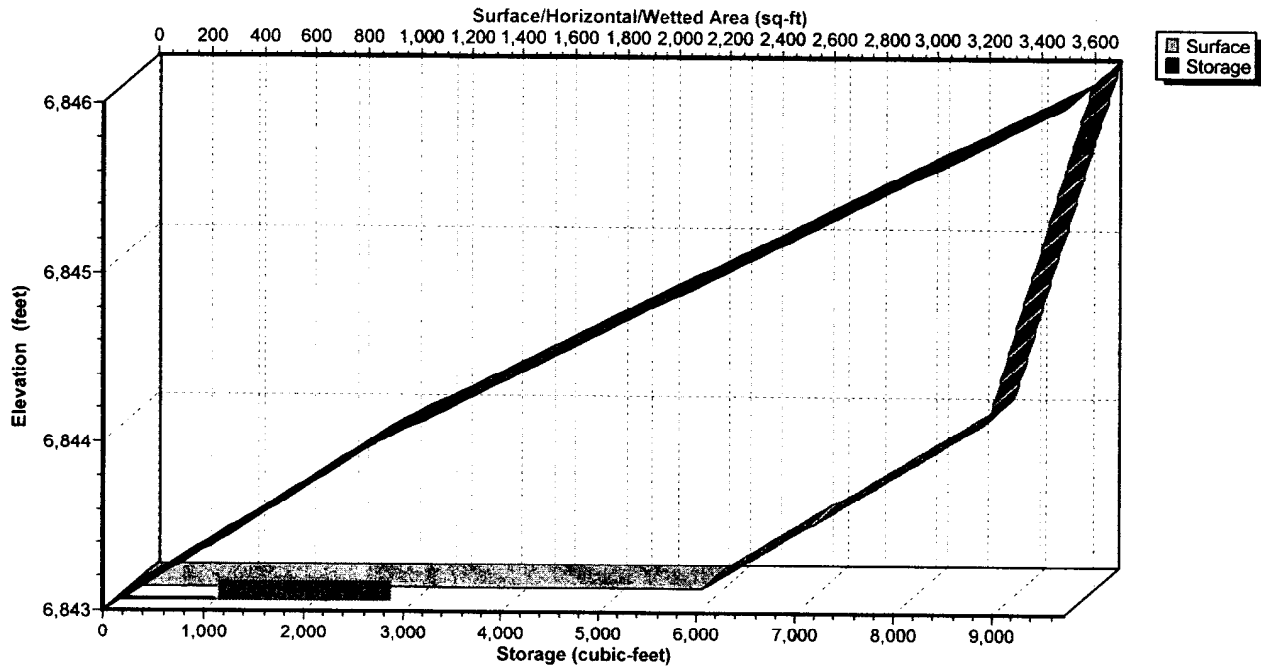
Pond Pond: East PLD

Stage-Discharge



Pond Pond: East PLD

Stage-Area-Storage



Pond Pond: East PLD

Inflow Area = 3.680 ac, Inflow Depth = 1.37" for 100-Year event
 Inflow = 20.30 cfs @ 0.09 hrs, Volume= 0.420 af
 Outflow = 16.97 cfs @ 0.26 hrs, Volume= 0.382 af, Atten= 16%, Lag= 10.4 min
 Primary = 16.97 cfs @ 0.26 hrs, Volume= 0.382 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,845.51' @ 0.26 hrs Surf.Area= 3,603 sf Storage= 8,024 cf

Plug-Flow detention time= 8.7 min calculated for 0.382 af (91% of inflow)
 Center-of-Mass det. time= 7.8 min (17.8 - 10.0)

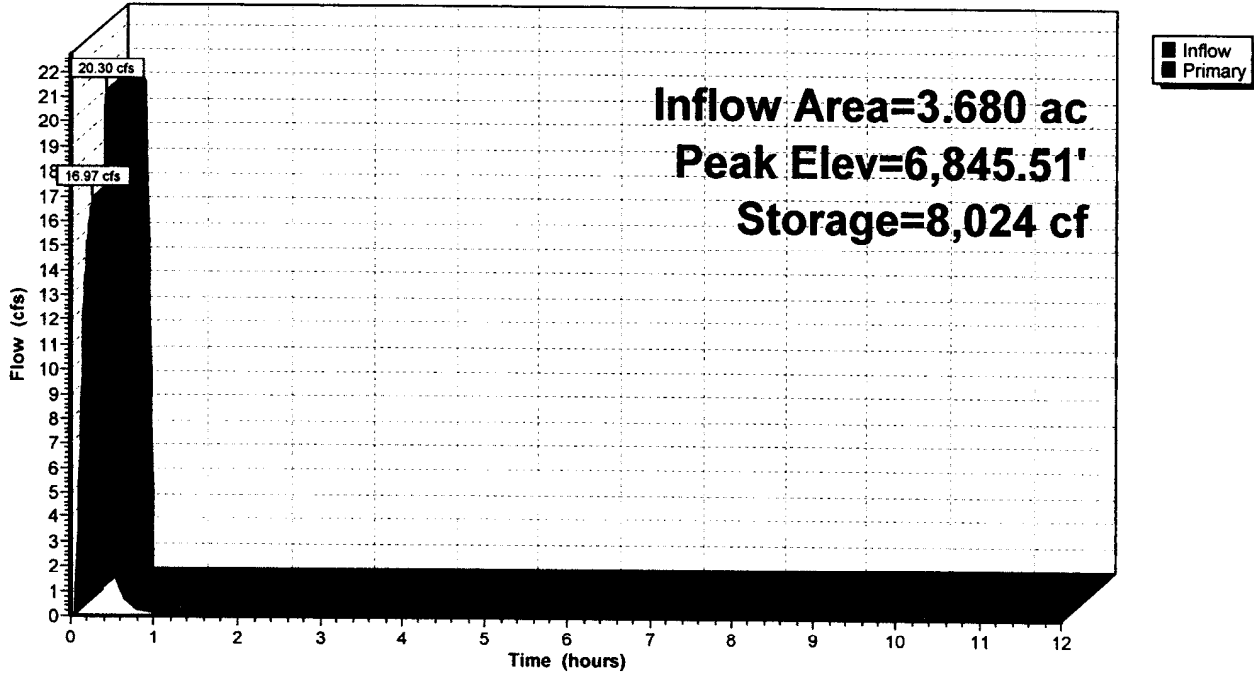
Volume	Invert	Avail.Storage	Storage Description		
#1	6,843.00'	9,728 cf	Custom Stage Data (Pyramidal) Listed below		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
6,843.00	2,200	0	0	2,200	
6,844.00	3,300	2,731	2,731	3,320	
6,846.00	3,700	6,996	9,728	3,939	

Device	Routing	Invert	Outlet Devices	
#1	Primary	6,845.50'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000	
#2	Primary	6,843.60'	2.50' W x 1.25' H Vert. Orifice/Grate C= 0.600	

Primary OutFlow Max=16.92 cfs @ 0.26 hrs HW=6,845.51' (Free Discharge)
 1=Orifice/Grate (Weir Controls 0.04 cfs @ 0.33 fps)
 2=Orifice/Grate (Orifice Controls 16.88 cfs @ 5.40 fps)

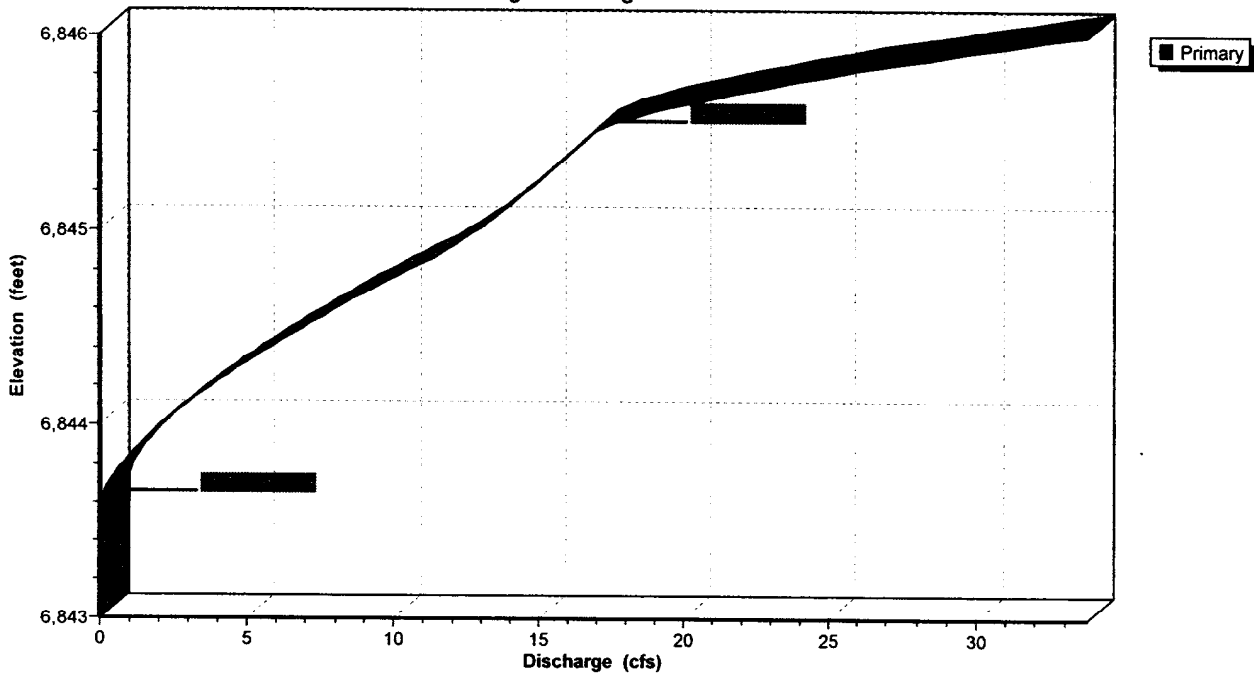
Pond Pond: East PLD

Hydrograph



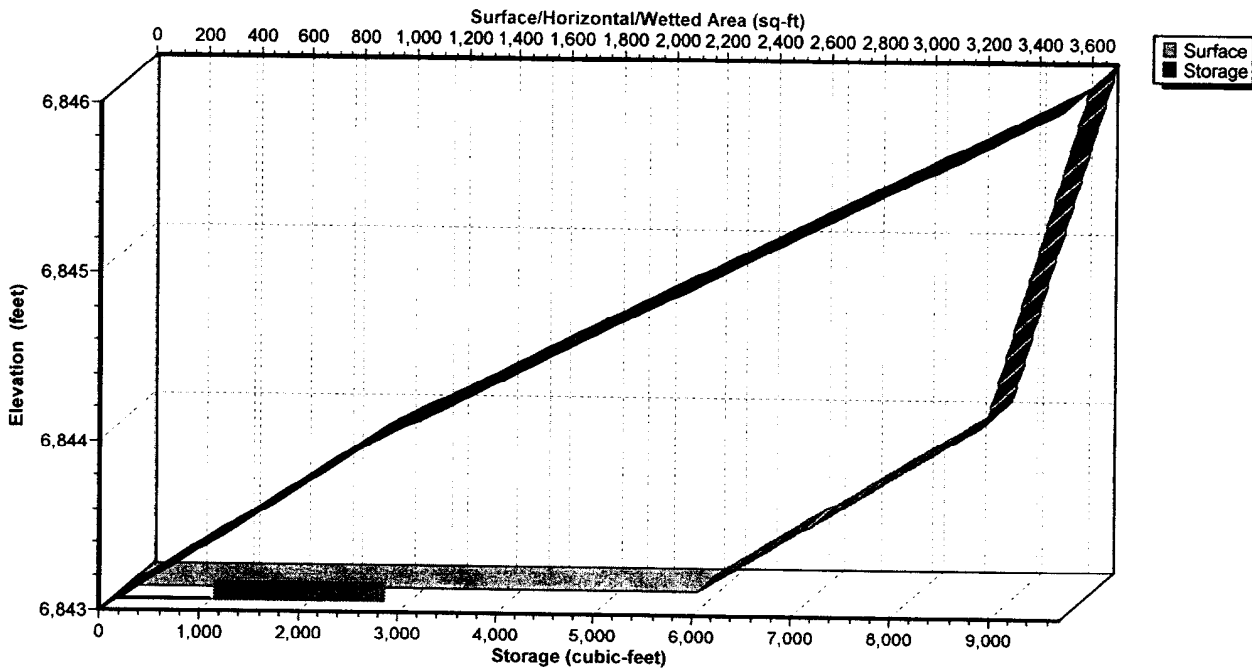
Pond Pond: East PLD

Stage-Discharge



Pond Pond: East PLD

Stage-Area-Storage

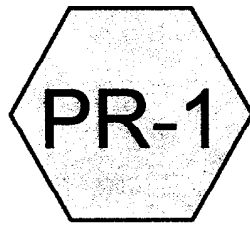


Design Procedure Form: Porous Landscape Detention (PLD)

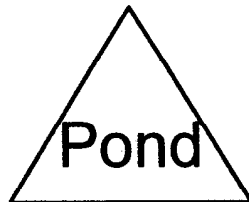
Designer: Thomas Roberts
Company: Springs Engineering
Date: July 23, 2008
Project: Meridian Crossing West Pond
Location: Falcon, CO

<p>1. Basin Storage Volume <i>(I_a = 100% if all paved and roofed areas u/s of PLD)</i></p> <p>A) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)</p> <p>B) Contributing Watershed Area Including the PLD (Area)</p> <p>C) Water Quality Capture Volume (WQCV) <i>(WQCV = 0.8 * (0.91 * I³ - 1.19 * I² + 0.78 * I))</i></p> <p>D) Design Volume: $Vol_{PLD} = (WQCV / 12) * Area$</p>	<p>$I_a = \frac{82.00}{100} \%$</p> <p>$i = \frac{0.82}{100}$</p> <p>Area = <u>200,821</u> square feet</p> <p>WQCV = <u>0.27</u> watershed inches</p> <p>Vol = <u>4,568</u> cubic feet</p>
<p>2. PLD Surface Area (A_{PLD}) and Average Depth (d_{av}) <i>(from 4567.86 square feet to 9135.72 square feet)</i></p> <p><i>($d_{av} = (Vol / A_{PLD})$, Min=0.5', Max=1.0')</i></p>	<p>$A_{PLD} = \underline{4,600}$ square feet</p> <p>$d_{av} = \underline{0.99}$ feet</p>
<p>3. Draining of PLD (Check A, or B, or C, answer D) Based on answers to 3A through 3D, check the appropriate method</p> <p>A) Check box if subgrade is heavy or expansive clay <input type="checkbox"/></p> <p>B) Check box if subgrade is silty or clayey sand <input type="checkbox"/></p> <p>C) Check box if subgrade is well-draining soil <input checked="" type="checkbox"/></p> <p>D) Check box if underdrains are not desirable or if underdrains are not feasible at this site. <input type="checkbox"/></p> <p>E) Does tributary catchment contain land uses that may have petroleum products, greases, or other chemicals present, such as gas station, hardware store, restaurant, etc.? yes no</p> <p style="margin-left: 100px;"><input checked="" type="checkbox"/> <input type="checkbox"/></p>	<p><input type="checkbox"/> Infiltration to Subgrade with Permeable Membrane: 3(C) checked and 3(E) = no</p> <p><input type="checkbox"/> Underdrain with Impermeable Liner: 3(A) checked or 3(E) = yes</p> <p><input type="checkbox"/> Underdrain with Non-Woven Geotextile Fabric: 3(B) checked and 3(E) = no</p> <p><input type="checkbox"/> 16-Mil. Impermeable Membrane with No Underdrain: 3(D) checked - Evapotranspiration only</p> <p><input checked="" type="checkbox"/> Other: <u>Type D Inlet</u></p>
<p>4. Sand/Peat Mix and Gravel Subbase (See Figure PLD-1)</p> <p>A) Heavy or Expansive Clay (NRCS Group D Soils) Present; Perforated HDPE Underdrain Used.</p> <p>B) Silty or Clayey Sand (NRCS Group C Soils) Present; Perforated HDPE Underdrain Used.</p> <p>C) No Potential For Contamination And Well-Draining (NRCS Group A or B Soils) Are Present; Underdrains Eliminated.</p> <p>D) Underdrains Are Not Desirable Or Are Not Feasible At This Site.</p> <p>E) Other:</p>	<p><input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with 8" Gravel Layer. 16-Mil. Impermeable Liner and a 3" to 4" Perforated HDPE Underdrain.</p> <p><input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with 8" Gravel Layer and a 3" to 4" Perforated HDPE Underdrain w/ Non-Woven Pemeable Membrane.</p> <p><input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with Non-Woven Pemeable Membrane and No Underdrain (Direct Infiltration).</p> <p><input type="checkbox"/> 18" Minimum Depth Sand-Peat Mix with An Additional 18" Minimum Layer Sand-Peat Mix or Sand-Class 'A' Compost Bottom Layer (Total Sand-Peat Depth of 36"). 16-Mil. Impermeable Liner Used.</p> <p><input checked="" type="checkbox"/> Other: <u>See Detail on Sheet 8</u></p>

Notes: _____



Basin D-9



West PLD



Pond Pond: West PLD

Inflow Area = 3.680 ac, Inflow Depth = 0.32" for 2-Year event
 Inflow = 11.93 cfs @ 0.10 hrs, Volume= 0.098 af
 Outflow = 11.81 cfs @ 0.10 hrs, Volume= 0.098 af, Atten= 1%, Lag= 0.2 min
 Primary = 11.81 cfs @ 0.10 hrs, Volume= 0.098 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,841.48' @ 0.10 hrs Surf.Area= 148 sf Storage= 133 cf

Plug-Flow detention time= 0.2 min calculated for 0.098 af (100% of inflow)
 Center-of-Mass det. time= 0.2 min (5.7 - 5.5)

Volume	Invert	Avail.Storage	Storage Description
#1	6,840.00'	6,085 cf	Custom Stage Data (Prismatic) Listed below

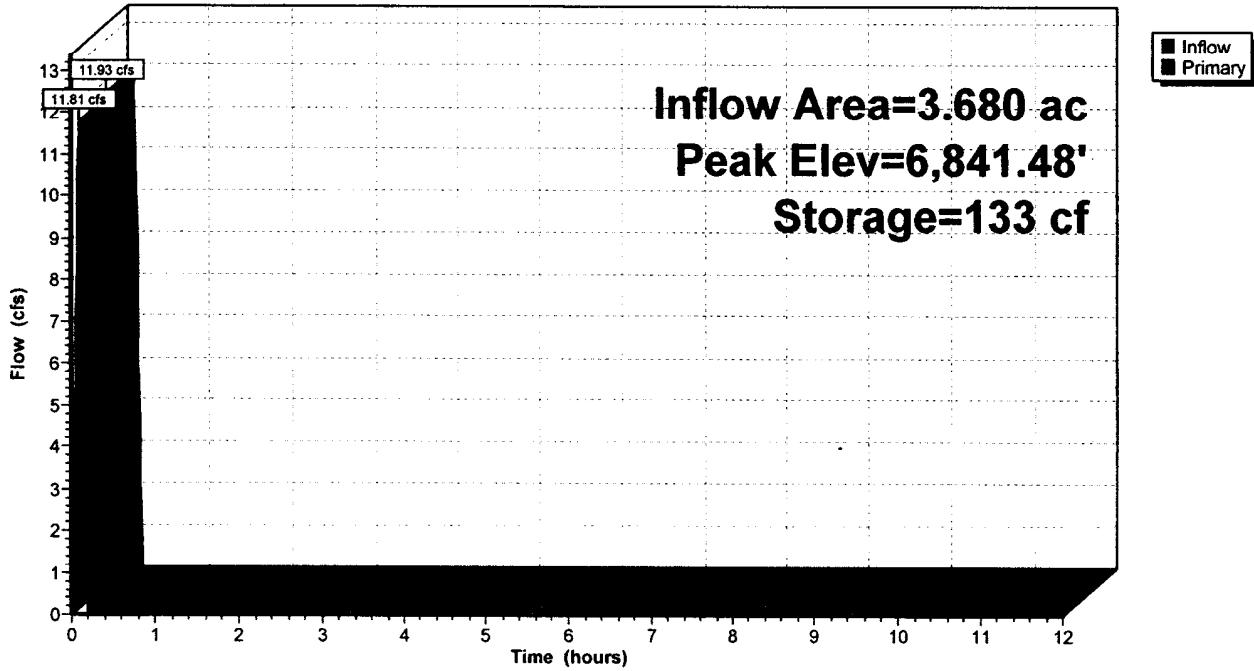
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6,840.00	20	0	0
6,841.00	100	60	60
6,842.00	200	150	210
6,843.00	1,500	850	1,060
6,844.50	5,200	5,025	6,085

Device	Routing	Invert	Outlet Devices
#1	Primary	6,842.85'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000
#2	Primary	6,840.00'	2.50' W x 1.00' H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=11.74 cfs @ 0.10 hrs HW=6,841.47' (Free Discharge)
 1=Orifice/Grate (Controls 0.00 cfs)
 2=Orifice/Grate (Orifice Controls 11.74 cfs @ 4.70 fps)

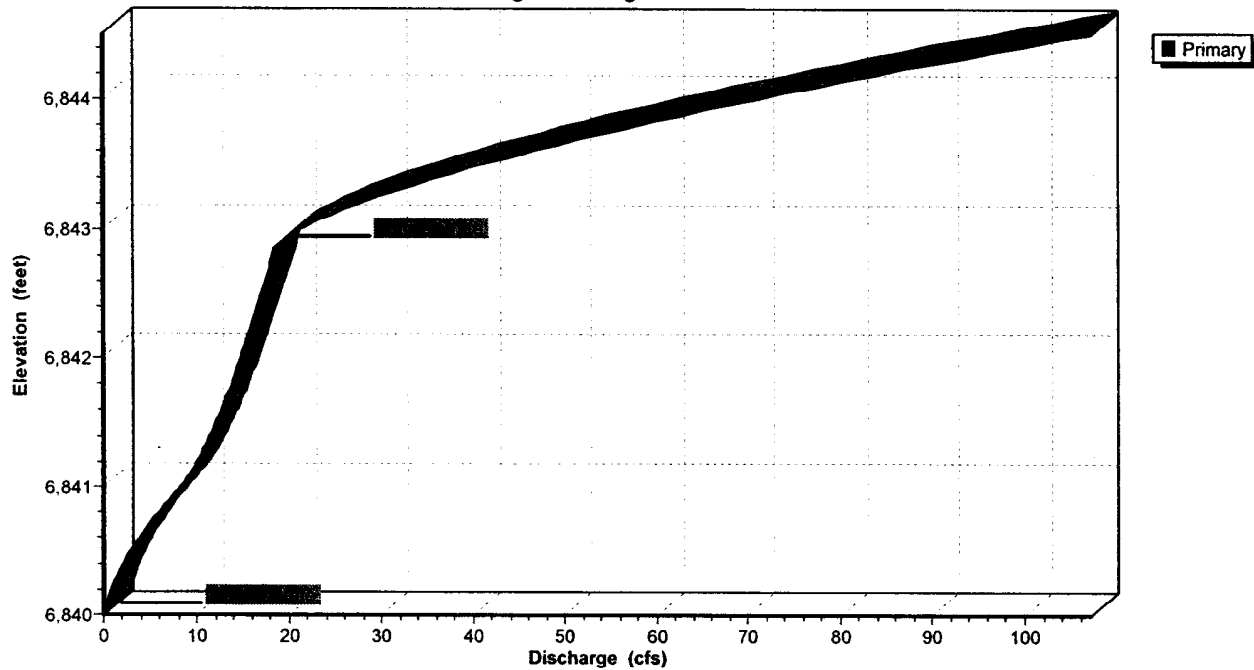
Pond Pond: West PLD

Hydrograph

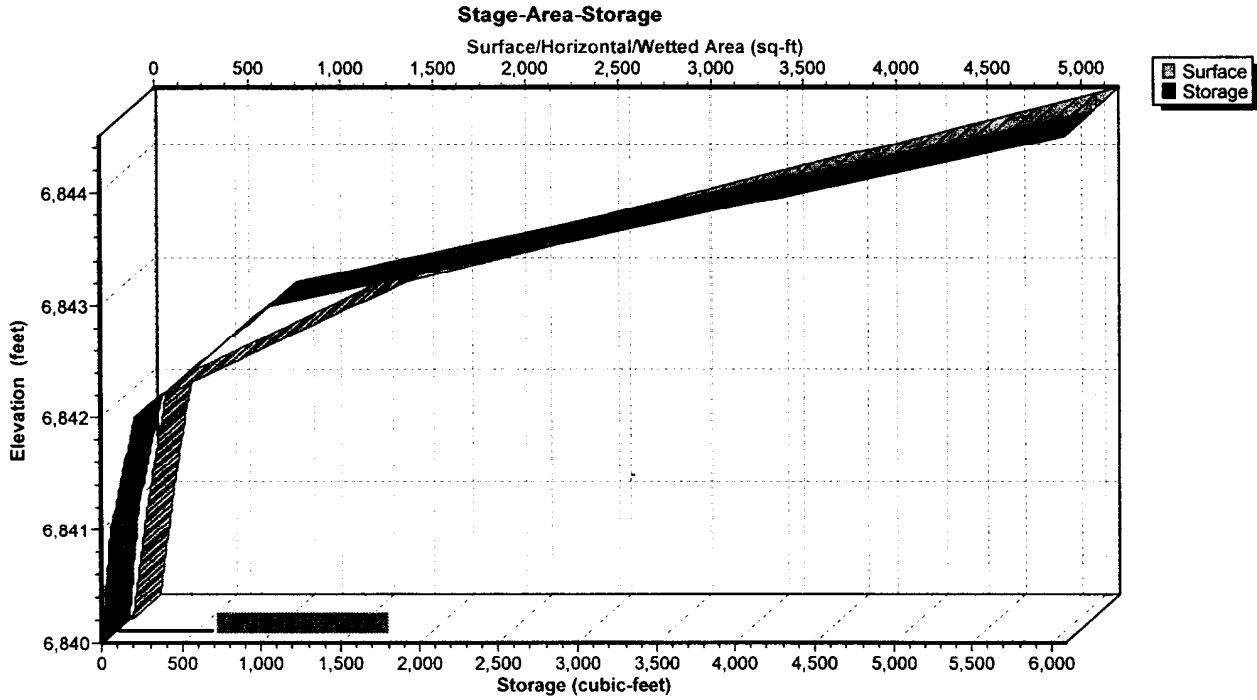


Pond Pond: West PLD

Stage-Discharge



Pond Pond: West PLD



Pond Pond: West PLD

Inflow Area = 3.680 ac, Inflow Depth = 0.44" for 5-Year event
 Inflow = 16.39 cfs @ 0.09 hrs, Volume= 0.135 af
 Outflow = 15.20 cfs @ 0.11 hrs, Volume= 0.135 af, Atten= 7%, Lag= 0.6 min
 Primary = 15.20 cfs @ 0.11 hrs, Volume= 0.135 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,842.11' @ 0.11 hrs Surf.Area= 340 sf Storage= 302 cf

Plug-Flow detention time= 0.2 min calculated for 0.134 af (100% of inflow)
 Center-of-Mass det. time= 0.2 min (5.7 - 5.5)

Volume	Invert	Avail.Storage	Storage Description
#1	6,840.00'	6,085 cf	Custom Stage Data (Prismatic) Listed below

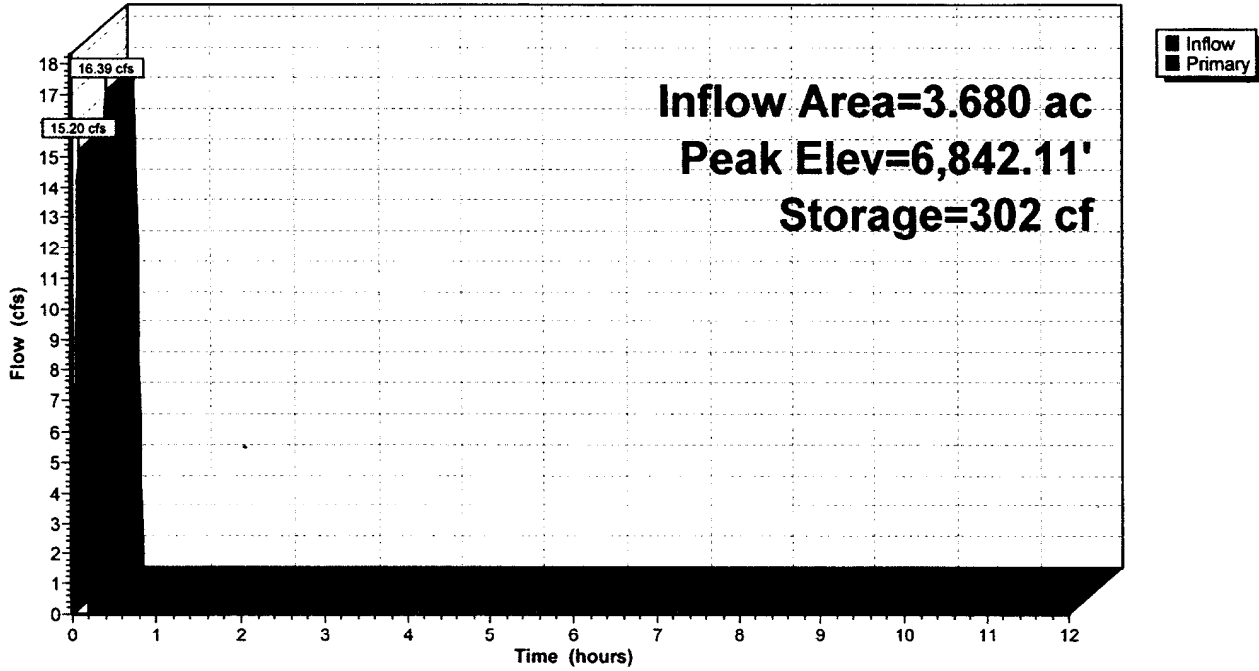
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6,840.00	20	0	0
6,841.00	100	60	60
6,842.00	200	150	210
6,843.00	1,500	850	1,060
6,844.50	5,200	5,025	6,085

Device	Routing	Invert	Outlet Devices
#1	Primary	6,842.85'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000
#2	Primary	6,840.00'	2.50' W x 1.00' H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=15.16 cfs @ 0.11 hrs HW=6,842.10' (Free Discharge)
 1=Orifice/Grate (Controls 0.00 cfs)
 2=Orifice/Grate (Orifice Controls 15.16 cfs @ 6.06 fps)

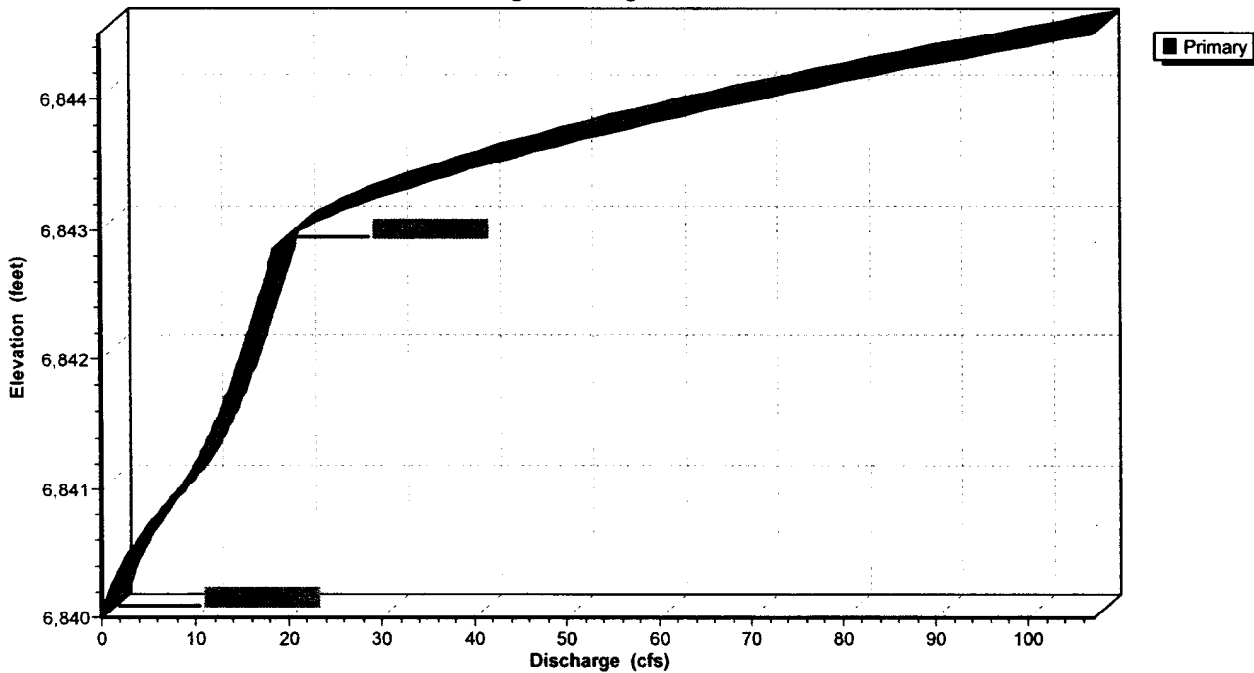
Pond Pond: West PLD

Hydrograph



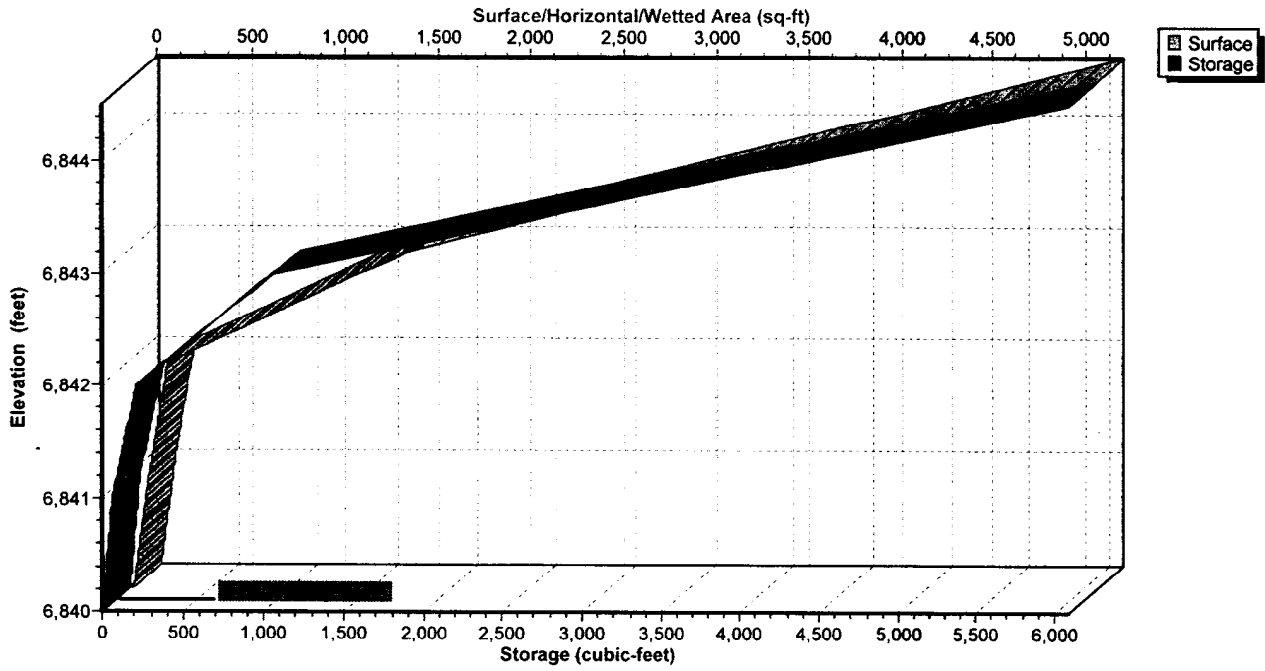
Pond Pond: West PLD

Stage-Discharge



Pond Pond: West PLD

Stage-Area-Storage



Pond Pond: West PLD

Inflow Area = 3.680 ac, Inflow Depth = 0.78" for 100-Year event
 Inflow = 29.14 cfs @ 0.09 hrs, Volume= 0.240 af
 Outflow = 25.15 cfs @ 0.11 hrs, Volume= 0.239 af, Atten= 14%, Lag= 1.0 min
 Primary = 25.15 cfs @ 0.11 hrs, Volume= 0.239 af

Routing by Stor-Ind method, Time Span= 0.00-12.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 6,843.13' @ 0.11 hrs Surf.Area= 1,809 sf Storage= 1,479 cf

Plug-Flow detention time= 0.7 min calculated for 0.239 af (100% of inflow)
 Center-of-Mass det. time= 0.7 min (6.2 - 5.5)

Volume	Invert	Avail.Storage	Storage Description
#1	6,840.00'	6,085 cf	Custom Stage Data (Prismatic) Listed below

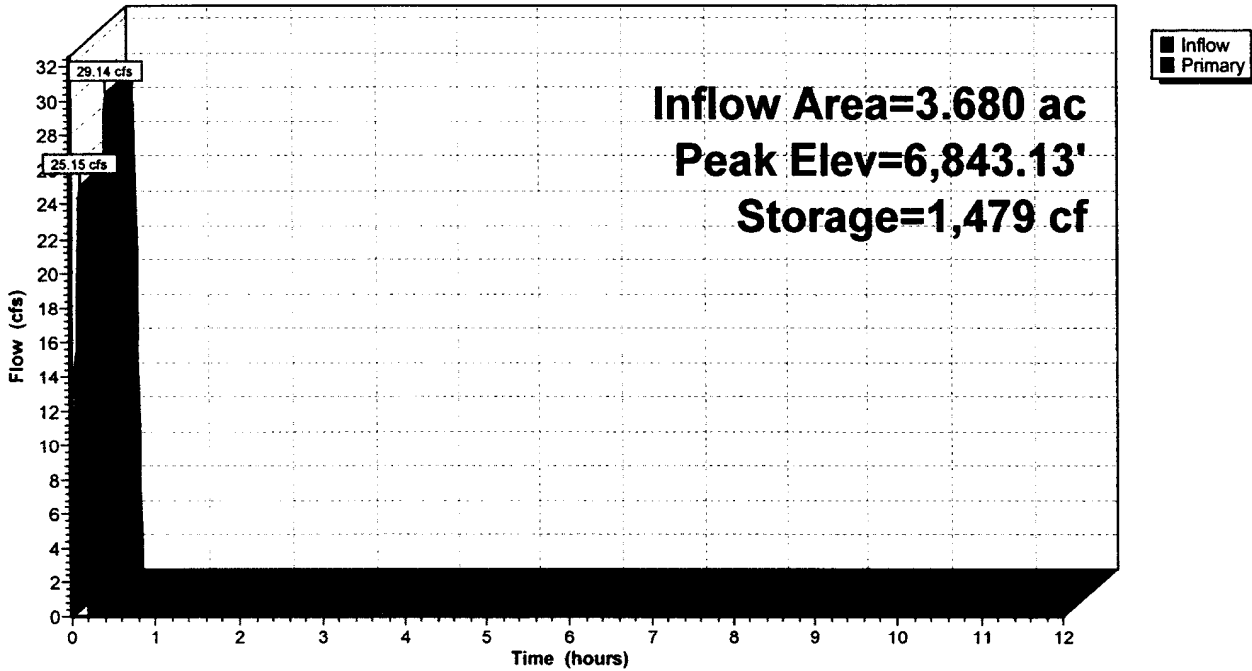
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
6,840.00	20	0	0
6,841.00	100	60	60
6,842.00	200	150	210
6,843.00	1,500	850	1,060
6,844.50	5,200	5,025	6,085

Device	Routing	Invert	Outlet Devices
#1	Primary	6,842.85'	3.00' x 3.00' Horiz. Orifice/Grate Limited to weir flow C= 1.000
#2	Primary	6,840.00'	2.50' W x 1.00' H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=25.05 cfs @ 0.11 hrs HW=6,843.12' (Free Discharge)
 1=Orifice/Grate (Weir Controls 5.58 cfs @ 1.71 fps)
 2=Orifice/Grate (Orifice Controls 19.46 cfs @ 7.79 fps)

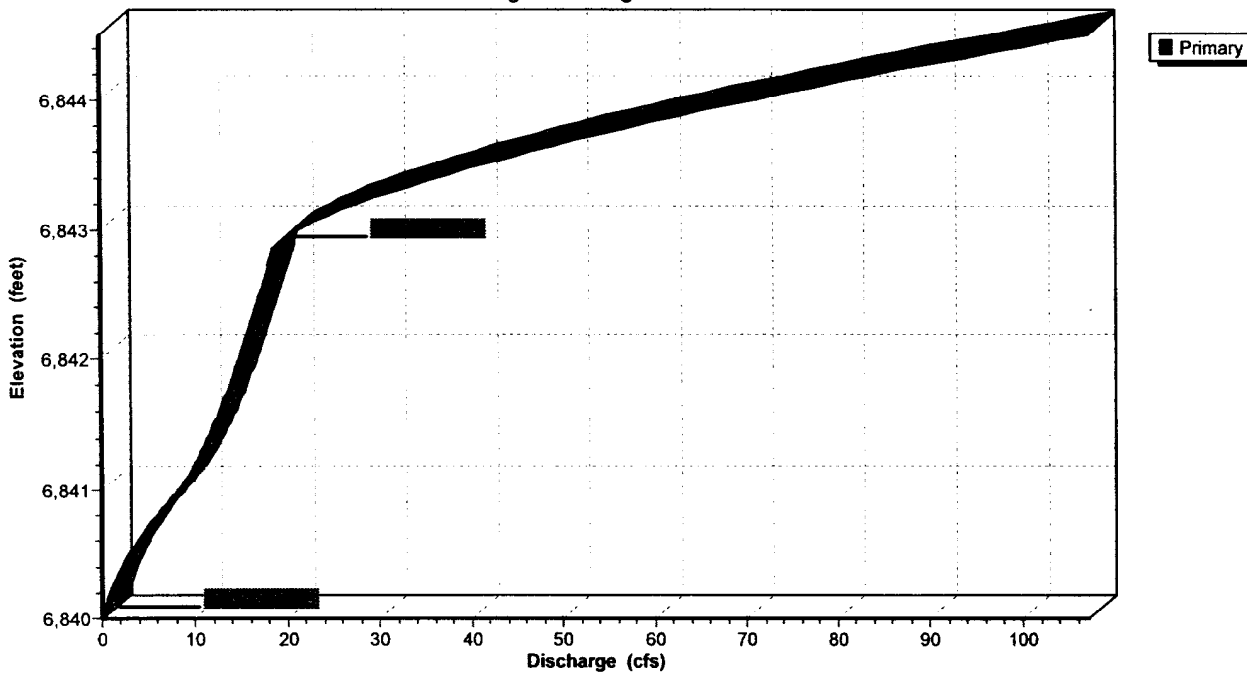
Pond Pond: West PLD

Hydrograph

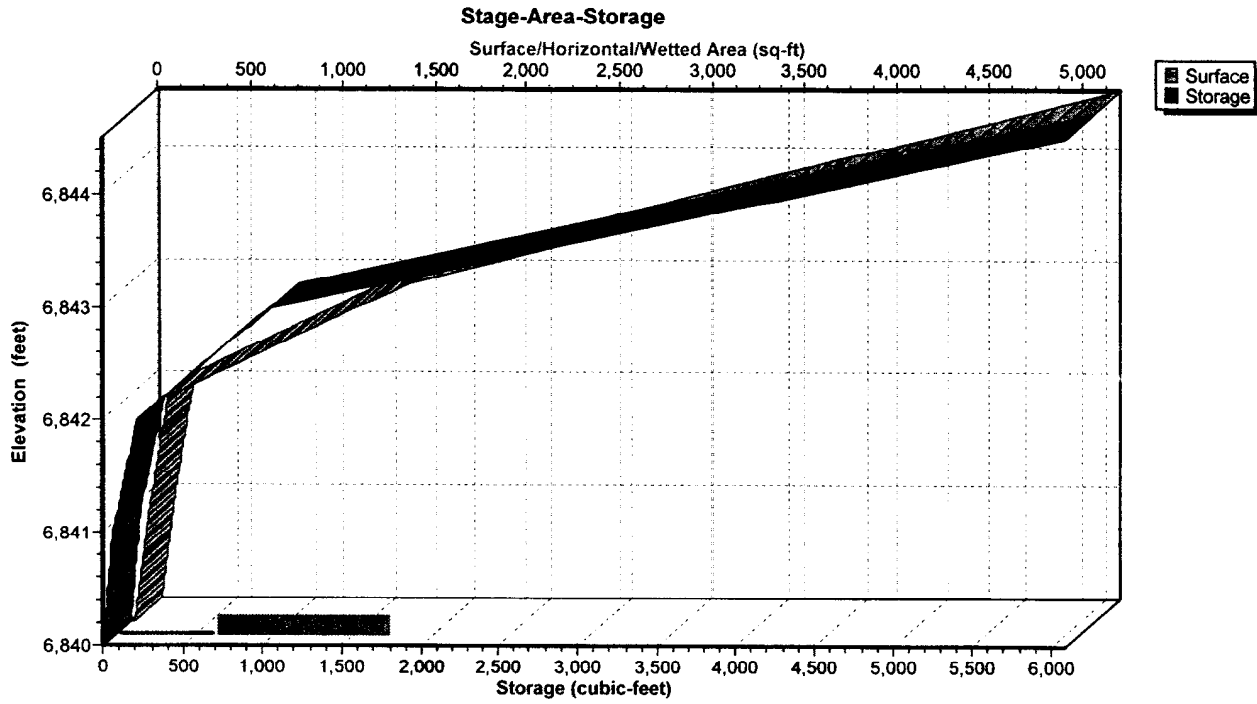


Pond Pond: West PLD

Stage-Discharge



Pond Pond: West PLD



Appendix H: Operations and Maintenance Manual

**OPERATION AND MAINTENANCE MANUAL
MERIDIAN CROSSING
PARK PLACE ENTERPRISES
EL PASO COUNTY, COLORADO**

May 2008

PREPARED FOR:

Park Place Enterprises
15 Miranda Road
Colorado Springs, CO 80906

PREPARED BY:

Springs Engineering
31 N. Tejon Street
Suite 315
Colorado Springs, CO 80903

PROJECT NO. 07-057-0032

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DESCRIPTION OF CONSTRUCTION	2
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INSPECTION AND MAINTENANCE	2
POROUS LANDSCAPE DETENTION FACILITY	2
OPERATION & MAINTENANCE LOG	4

Introduction

This Operation and Maintenance Plan is being submitted on behalf of Park Place Enterprises for a development known as Meridian Crossing in Falcon, Colorado. The purpose of this Operation and Maintenance Manual (O&M) is to identify facilities which are to be maintained by the Meridian Crossing Properties Owners Association (POA) and the frequency with which these items are to be maintained.

General Location and Description

Meridian Crossing is currently zoned CR and the proposed development includes 6 commercial lots, proposed water quality facilities, streets, and utilities.

Meridian Crossing is approximately 9.5 acres and is located north of the intersection of Meridian Road and Old Meridian Road in Falcon, Colorado, Section 12, Township 13 South, Range 65 West of the 6th Principal Meridian.

Description of Construction

Construction will consist of site grading, utility installation, and road paving. Approximately 9.5 acres of the site will be graded for construction of the proposed commercial units. Erosion control will be provided prior to construction.

Facilities

Water quality facilities will be owned and maintained by the POA. Water and sanitary sewer will be maintained by the Falcon Highlands Metropolitan District. All other utilities are to be maintained by their respective owners.

Inspection and Maintenance

A thorough inspection of the permanent structures shall be performed every 30 days as well as after any significant rain or snowmelt event. Inspectors are to look for any significant deterioration of the facilities including:

- Erosion of channels and side slopes.
- Accumulated trash or debris.

Repairs and removal of debris shall occur as soon as practical.

Porous Landscape Detention Facility

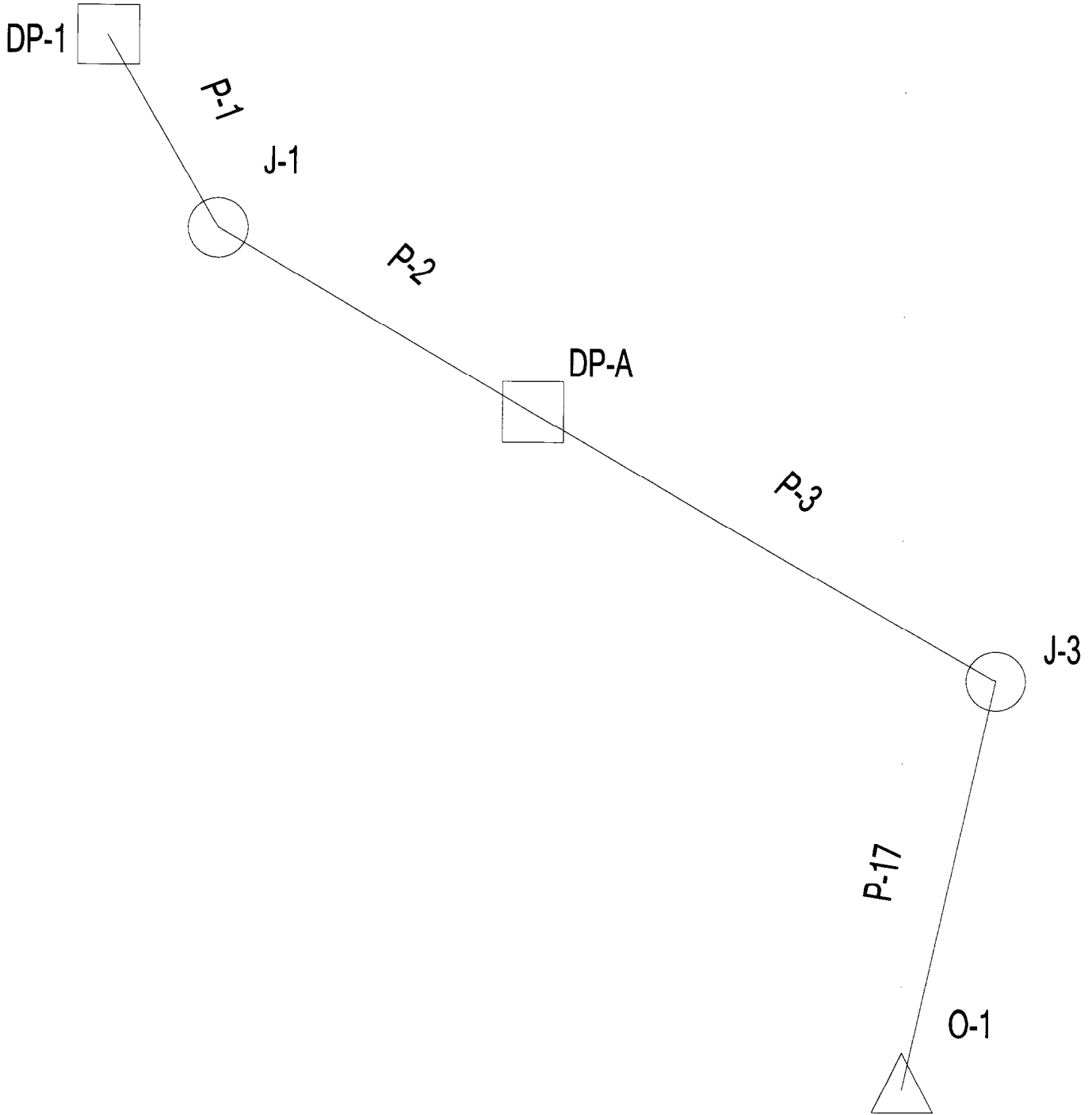
Lawn mowing and vegetative care shall be performed routinely, as aesthetic requirements demand. This shall limit unwanted vegetation. Irrigated turf grass shall be between 2 and 4 inches in height and non irrigated native turf grasses shall be 4 to 6 inches in height. Debris and litter removal shall be performed routinely, as aesthetic requirements demand. Removal of debris and litter from any detention area minimizes clogging of the sand media. Landscaping removal and replacement shall be done every 5 to 10 years depending on infiltration rates needed to drain the area in 12 hours or less. Over time the sandy loam

sandy loam turf will clog. The layer will need to be replaced, along with all turf and other vegetation growing on the surface, to rehabilitate infiltration rates. Bin-annual inspections of the hydraulic performance of the area will need to be performed. This will determine if the sand media is allowing acceptable infiltration.

An Operation and Maintenance Log follows.

Appendix I: Ultimate Design StormCAD Calculations

Scenario: 100-year



Analysis Results

Scenario: 100-year

Note:

The input data may have been modified since the last calculation was performed.
The calculated results may be outdated.

Title: Old Meridian Road
 Project Engineer: Charlene Sammons
 Project Date: 11/14/07
 Comments: Storm in Old Meridian Road for Meridian Crossing Storm Sewer

Scenario Summary

Scenario	100-year
Physical Properties Alternat	Base-Physical Properties
Catchments Alternative	Catchments-100-year
System Flows Alternative	Base-System Flows
Structure Headlosses Alterr	Base-Structure Headlosses
Boundary Conditions Altern	Base-Boundary Conditions
Design Constraints Alternat	Base-Design Constraints
Capital Cost Alternative	Base-Cost
User Data Alternative	Base-User Data

Network Inventory

Number of Pipes	4	Number of Inlets	2
- Circular Pipes:	4	- Grate Inlets:	0
- Box Pipes:	0	- Curb Inlets:	2
- Arch Pipes:	0	- Combination Inlets:	0
- Vertical Elliptical Pipes:	0	- Slot Inlets:	0
- Horizontal Elliptical Pipes:	0	- Grate Inlets in Ditch:	0
Number of Junctions	2	- Generic Inlets:	0
Number of Outlets	1		

Circular Pipes Inventory

18 inch	274.57 ft	24 inch	55.00 ft
Total Length	329.57 ft		

Curb Inlet Inventory

Type R 10'	2
------------	---

Inlet elements for network with outlet: O-1

Label	Inlet	Total Flow (cfs)	Total Intercepted Flow (cfs)	Total Bypassed Flow (cfs)	Bypass Target	Capture Efficiency (%)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	Headloss Method
DP-1	Curb Type R	7.61	7.61	10.28	DP-A	42.5	3,848.83	3,848.83	0.00	Absolut
DP-A	Curb Type R	9.39	2.10	14.37	<Automati	12.8	3,846.39	3,846.08	0.31	Standar

Junction elements for network with outlet: O-1

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	Headloss Method	System Additional Flow (cfs)	System Known Flow (cfs)	System Rational Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
J-1	3,847.76	3,847.46	0.30	Standar	0.00	0.00	7.55	8.50	6.59	0.88
J-3	3,844.91	3,844.91	0.00	Absolut	0.00	0.00	9.30	8.13	7.63	1.13

Analysis Results

Scenario: 100-year

Outlet: O-1

Label	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)	Gravity Element Headloss (ft)	System Additional Flow (cfs)	System Known Flow (cfs)	System Rational Flow (cfs)	System Intensity (in/hr)	System Flow Time (min)	System CA (acres)
O-1	3,843.10	3,843.10	0.00	0.00	0.00	9.22	8.06	7.83	1.13

Pipe elements for network with outlet: O-1

Label	Section Shape	Section Size	Length (ft)	Number of Sections	Constructed Slope (ft/ft)	Energy Slope (ft/ft)	Total System Flow (cfs)	Average Velocity (ft/s)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Hydraulic Grade Line In (ft)	Hydraulic Grade Line Out (ft)
P-1	Circular	18 inch	71.21	1	0.012	0.011	7.61	6.31	3,847.76	6,846.90	3,848.83	3,847.79
P-2	Circular	18 inch	17.16	1	0.009	0.009	7.55	5.53	3,846.40	6,845.29	3,847.46	3,846.39
P-3	Circular	18 inch	86.20	1	0.009	0.009	9.39	6.33	3,844.90	6,844.13	3,846.08	3,845.29
P-17	Circular	24 inch	55.00	1	0.007	0.010	9.30	4.62	3,843.50	6,843.10	3,844.91	3,844.19

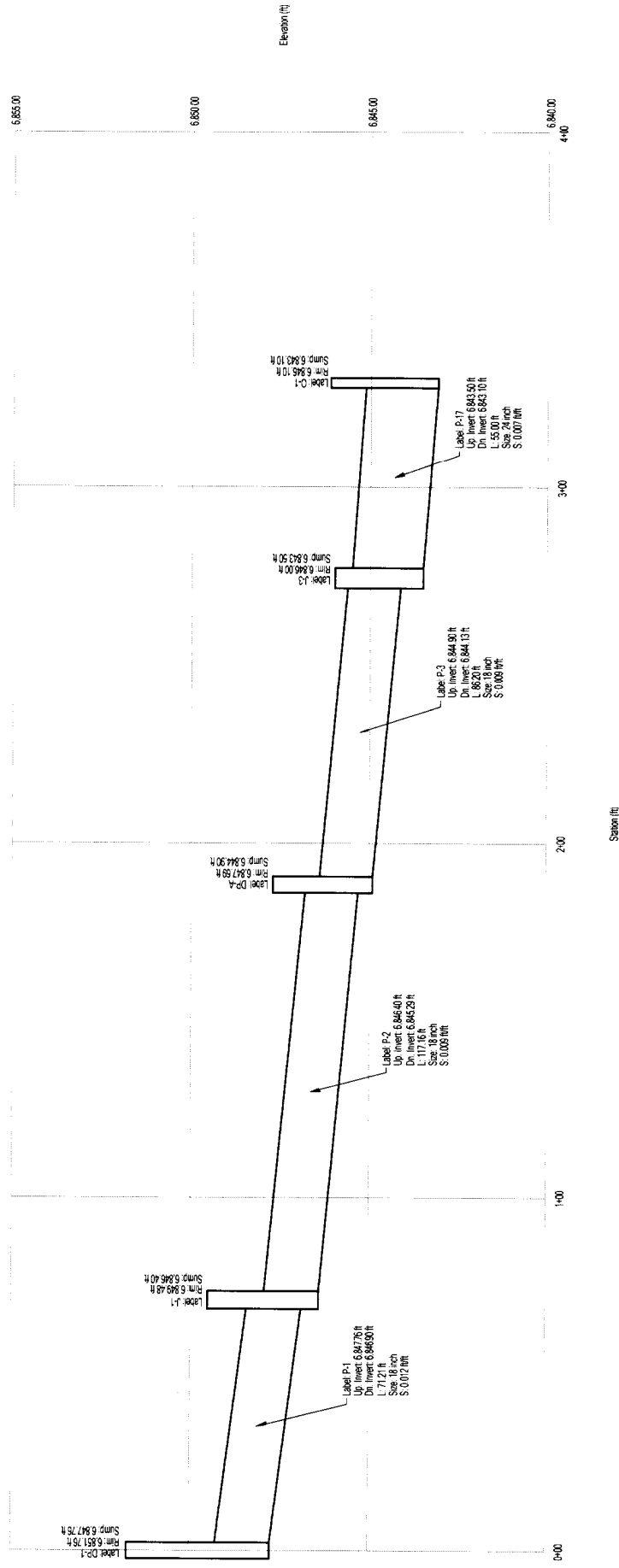
Scenario: 100-year

Combined Pipe\Node Report

Label	Up. Node	Dn. Node	L (ft)	Up. Inlet Area (acres)	Up. Inlet Rat. Coef.	Up. Inlet Area (acres)	Up. Calc. Sys. CA (acres)	Up. Inlet Rat. Q (cfs)	Size	Q Full (cfs)	Avg. v (ft/s)	Up. Gr Elev. (ft)	HGL In (ft)	Up. Invert (ft)	Dn. Gr. Elev. (ft)	HGL Out (ft)	Dn. Invert (ft)	S (ft/ft)
P-1	DP-1	J-1	71.21	2.07	1.00	2.07	0.88	17.89	18 inch	11.54	6.31	6,851.76	6,848.83	6,847.76	6,849.48	6,847.79	6,846.90	0.012
P-2	J-1	DP-A	117.16	N/A	N/A	N/A	0.88	N/A	18 inch	10.22	5.53	6,849.48	6,847.46	6,846.40	6,847.69	6,846.39	6,845.29	0.009
P-3	DP-A	J-3	86.20	0.80	1.00	0.80	1.13	6.63	18 inch	9.93	6.33	6,847.69	6,846.08	6,844.90	6,846.00	6,845.29	6,844.13	0.009
P-17	J-3	O-1	55.00	N/A	N/A	N/A	1.13	N/A	24 inch	10.45	4.62	6,846.00	6,844.91	6,843.50	6,846.10	6,844.19	6,843.10	0.007

Profile Scenario: 100-year

Profile: Ultimate
Scenario: 100-year



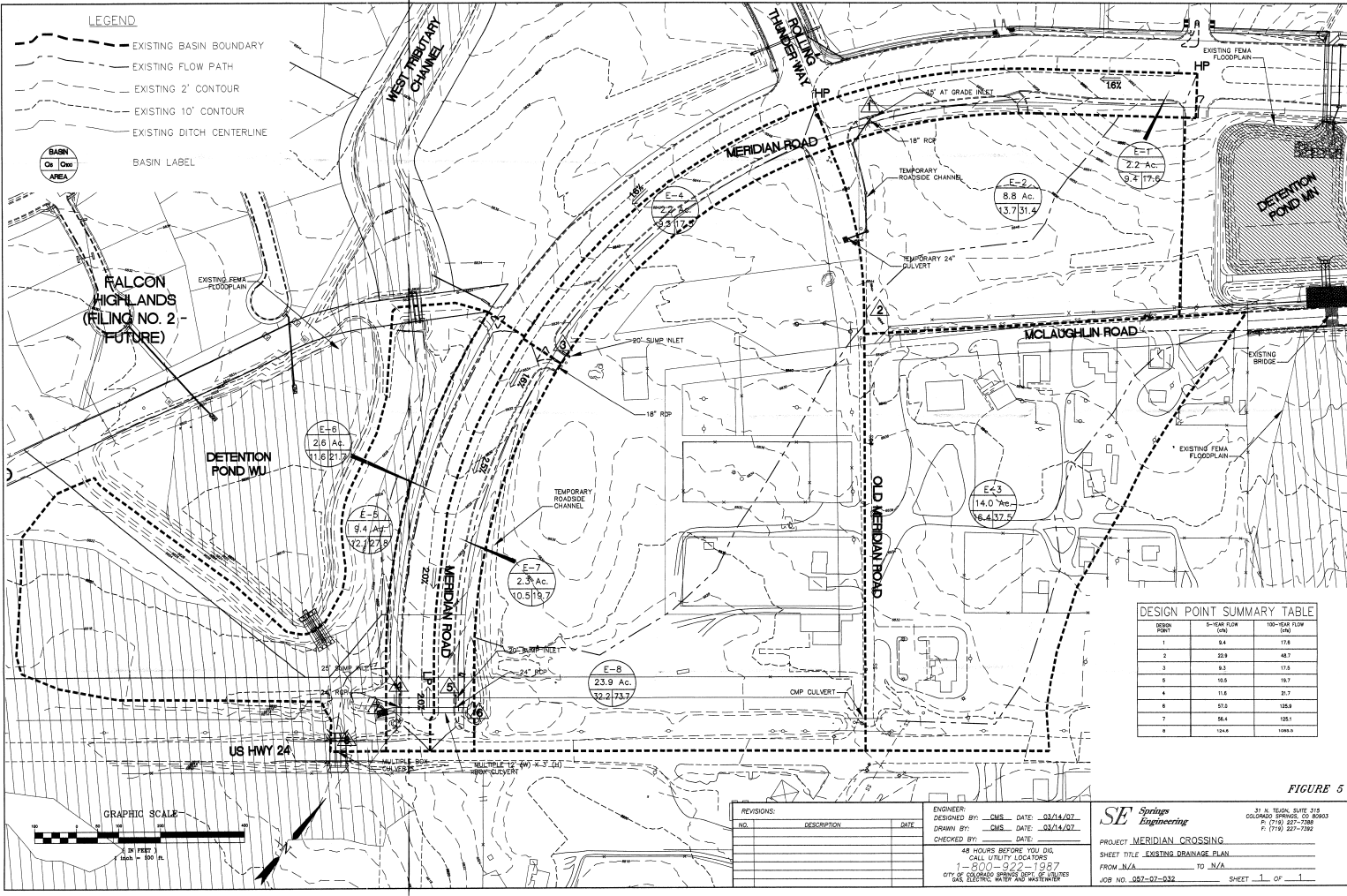
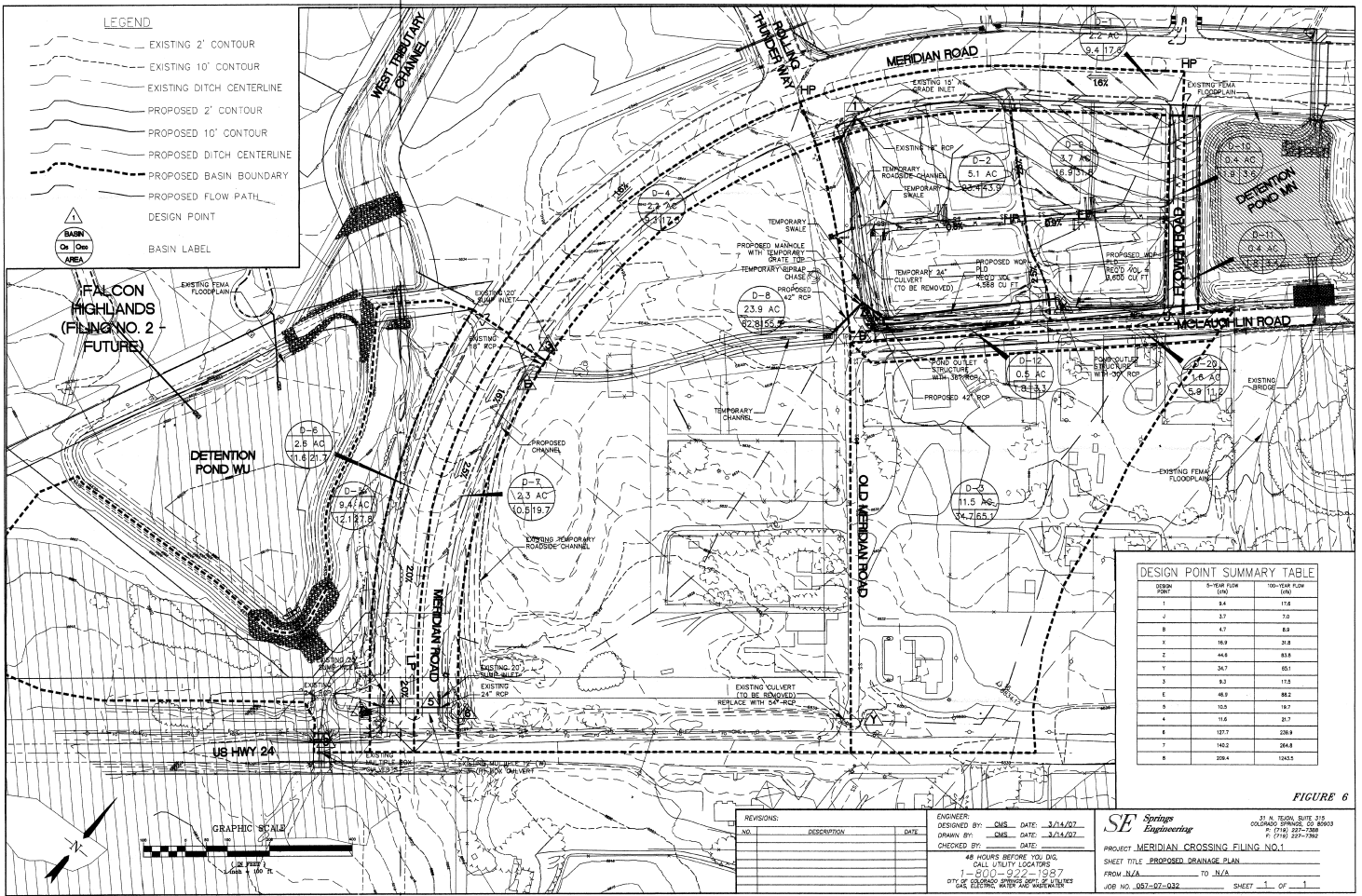


FIGURE 5



LEGEND

- - - - - EXISTING 2' CONTOUR
- - - - - EXISTING 10' CONTOUR
- - - - - EXISTING DITCH CENTERLINE
- - - - - PROPOSED 2' CONTOUR
- - - - - PROPOSED 10' CONTOUR
- - - - - PROPOSED DITCH CENTERLINE
- - - - - PROPOSED BASIN BOUNDARY
- - - - - PROPOSED FLOW PATH
- ▲ DESIGN POINT
- BASIN LABEL

DESIGN POINT SUMMARY TABLE

DESIGN POINT	5-YEAR FLOOD (CFS)	100-YEAR FLOOD (CFS)
1	84	172
2	37	132
3	47	83
4	163	214
5	444	838
6	347	651
7	83	172
8	483	882
9	108	187
10	116	217
11	1277	2268
12	1462	2648
13	2084	12455

FIGURE 6

REVISIONS:

NO.	DESCRIPTION	DATE

ENGINEER: DESIGNED BY: CMG DATE: 3/14/07
 DRAWN BY: CMG DATE: 3/14/07
 CHECKED BY: _____ DATE: _____

48 HOURS BEFORE YOU DIG
 CALL UTILITY LOCATORS
 800-927-1997
 CITY OF SPRINGFIELD, ILLINOIS
 USE BEFORE ANY OTHER VERSION

SE Springs Engineering
 PROJECT: MERIDIAN CROSSING FILING NO. 1
 SHEET TITLE: PROPOSED DRAINAGE PLAN
 FROM: N/A TO: N/A
 JOB NO. 083-07-032 SHEET 1 OF 1



APPENDIX F: PLD SITE PHOTOS

