

PRELIMINARY/ FINAL DRAINAGE REPORT FOR RIVERBEND CROSSING FILINGS NO. 1 AND 2

SEPTEMBER 2018

Prepared for:

Avatar Fountain, LP.
6800 Jericho Tpke., Suite 120W #204
Syosset, NY 11791

Prepared By:



PCD FILE NOS:
SP 187
SF 1843
SF 1844

CONNECT RESOURCES

PRELIMINARY/FINAL DRAINAGE REPORT FOR REIVERBEND CROSSING FILING
NO. 1 AND 2

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Certification Statement:

This report and plan for the preliminary and final drainage design for the RIVERBEND CROSSING was prepared by me (or under my direct supervision) in accordance with the provisions of City of Colorado Springs/El Paso County Drainage Criteria Manual Volumes 1 and 2 Drainage Design and Technical Criteria for the owners thereof. I understand that El Paso County does not and will not assume liability for drainage facilities designed by others.

David L. Mijares, Colorado PE #40510
For and on behalf of Catamount Engineering

Date

Developer's Statement:

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

AVATAR FOUNTAN, LP. hereby certifies that the drainage facilities for RIVERBEND CROSSING shall be constructed according to the design presented in this report. I understand that El Paso County does not and will not assume liability for the drainage facilities designed and or certified by my engineer and that the El Paso County reviews drainage plans pursuant to Colorado Revised Statues, Title 30, Article 28; but cannot, on behalf of RIVERBEND CROSSING guarantee that final drainage design review will absolve AVATAR FOUNTAIN, LP. and/or their successors and/or assigns of future liability for improper design. I further understand that approval of the final plat does not imply approval of my engineer's drainage design.

AVATAR FOUNTAIN, LP.
Business Name

By: Alan Toth

Title: Managing Partner

Address: 6800 Jericho Turnpike, Suite 120W #204
Syosset, NY 11791

El Paso County:

Filed in accordance with the requirements of the El Paso County land Development Code and the Drainage Criteria manual Volumes 1 and 2, and the El Paso County Engineering Criteria Manual, latest revision.

Jennifer Irvine, PE
County Engineer/ECM Administrator

Date

Conditions:

**PRELIMINARY/FINAL DRAINAGE REPORT FOR REIVERBEND CROSSING
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PURPOSE

The purpose of this drainage report is to identify existing drainage patterns and establish outfall scenarios from the proposed development. The site is contained within the West Little Johnson Drainage Basin and outfalls directly to Fountain Creek. The parcel was previously studied in the Little Johnson/Security Creek Drainage Basin Planning Study prepared by Simons, Li and Associates, dated December 1987, and the Preliminary Drainage Report for Riverbend Crossing., prepared by Nolte and Associates, dated February 14, 2007. The Little Johnson Drainage Basin Planning Study identifies the parcel as direct flow into Fountain Creek and does not propose improvements to the adjacent reach. The overall Riverbend development consists of two overall projects, The Riverbend Crossing residential subdivision filings 1 and 2 to be developed in El Paso County; and the Riverbend Crossing Commons Development to be developed within City of Fountain. This report develops broad analysis of both El Paso County and Fountain development parcels and provides Final Drainage Report detail for the residential parcels.

GENERAL LOCATION AND DESCRIPTION

The Riverbend Crossing Developments are located within the NE ¼ of Section 14, Township 15 South and Range 66 West of the 6th principal meridian. The proposed commercial parcel contains approximately 10.69 acres to be developed within the City of Fountain incorporation limits. The existing commercial development is proposed to have the majority of buildings and infrastructure demolished and reconstruction of the site will incorporate access to the proposed commercial development.

The proposed residential developments contain approximately 52.0 acres of undeveloped land with approximately 10 acres located within the existing Fountain Creek 100-year floodplain. Improvements are proposed in the portions of the property identified as outside of the existing floodplain. Residential development is proposed to be completed in 2 filings. Filing No. 1 will contain 136 residential lots situated on approximately 36.5 acres within the easterly and southerly portions of the residential parcel. Filing No. 2 will contain 86 residential lots on approximately 15.5 acres. The 10 acres within the floodplain not proposed for development are contained within the boundary of Filing No. 1.

The overall development is bounded to the north and west by undeveloped land zoned A-5, to the east by U.S. Highway 85/87 and Southmoor Drive, and to the south by Fountain Creek. The easterly portions of the development contained within the City of Fountain incorporation limits are predominantly zoned commercial and the southerly and westerly portions of the development are zoned PUD. An RS-5000 zone is being sought with entitlement applications within the El Paso County portions.

Existing soils on the site consist of Limon clay (Hydrologic Group 'C'), Schamber-Razor complex (Hydrologic Group 'A'), and Nunn clay loam (Hydrologic Group 'C'). Soils have been identified as determined by the Natural Resources Conservation Service Web Soil Survey. Hydrologic Group 'C' soils have been used in hydrologic calculations.

The 10.69 acres contained within the commercial site contains existing structures, paved parking, and paved drive aisles with little existing vegetation. The 52.0 acre residential portion remains substantially ungraded and vegetated with native grasses and volunteer trees and shrubs within roadside ditches and established drainage swales.

The property contains an abandoned irrigation pond within the northern portion of the site that was historically supplied by two wells located within the commercial development. There is no active irrigation within the parcel currently. The parcel contains an abandoned sewer outfall crossing the site that previously served the commercial development. The abandoned sewer conveyed sewage to a lagoon system located within the Fountain Creek Floodplain. The lagoon was filled when central sewer became available to the commercial development from Security Sanitation District. No development is proposed in the location of the filled lagoon.

The existing commercial site sits 10-15 feet higher than the undeveloped residential portion of the parcels and runoff sheetflows predominantly at 1%-1.5% to the south and into Southmoor Drive. Flows are contained within the Southmoor Drive roadside ditch and conveyed southwest to Fountain Creek. The undeveloped portion of the Riverbend Crossing Developments falls flows predominantly to the south at an average slope of 1.5%

The majority of the site is located within Shaded Zone X (500-year) floodplain and the southern portion of the site is contained within a F.E.M.A. designated Zone AE (100-year) floodplain per FIRM panels 08041C0763G and 08041C0951 F, effective December 07, 2018. The F.E.M.A. Flood Insurance Rate Map has been provided. The portion of the site within the Zone AE floodplain will not be utilized for residential development.

A portion of the FIRM Panels were further modified by LOMR 17-08-0467P effective 1/15/2019. The LOMR modified floodplain affected by Security Creek and shows 100-YR flood being contained east of the centerline of Highway 85/87. LOMR revisions did not remove shaded zone X contained within the subject property.

EXISTING DRAINAGE

The parcels are located within the West Little Johnson Drainage Basin and are directly tributary to Fountain Creek within the reach. The Little Johnson/Security Creek Drainage Basin Planning Study identifies three separate sub-basins (75,76, and 77) within the parcel. The majority of the parcels are identified as within Shaded Zone X 500-year floodplain and the southerly portion of the property not proposed for development lies with Zone AE 100-yr floodplain and floodway. The effective firm panel is included in the appendix of the report. The West Little Johnson drainage basin contains approximately five square miles located in the semi-arid region of the high plains. Precipitation within the basin ranges from 14 to 16 inches per year with thunderstorms typical in the summer months.

The existing drainage patterns for the parcel were summarized in the "Preliminary Drainage Study Riverbend Crossing", prepared by Nolte and Associates, inc. dated 2/14/2007. No development

within the parcel has been pursued since the Nolte analysis was completed and the existing drainage analysis has been accepted in this report. The northerly adjacent parcel has been developed for use as St. Dominic Catholic Church. Flows from the Church are collected on-site and conveyed to an existing full spectrum extended detention basin within the southwest corner of the church parcel. Outfall from the extended detention basin is piped to outfall to an existing swale within a drainage easement on agricultural property west of the Riverbend properties and does not enter the subject property. No other changes to surrounding properties are evident.

The report indicates the 3 sub-basins identified in the Drainage Basin Planning Study as sub-basins 75,76, and 77. The basins are direct flow basins directly tributary to Fountain Creek and traverse the site from north to south where they enter Fountain Creek.

Basin 77 represents the existing commercial center development northwest of proposed Riverbend Crossing Filings No. 1 and 2 and the southeasterly portion of the residential filings. Redevelopment of the commercial development within the City of Fountain is being concurrently pursued by the developer of both properties. Existing flows entering the residential portion at the southern limits of the commercial development were modeled as $Q_5=25.99$ cfs, $Q_{100}=45.15$ cfs in the Preliminary Drainage Report and are conveyed in a drainage swale to outfall within Fountain Creek. Total outfall to Fountain Creek from Basin 77 was $Q_5=15.28$ cfs, $Q_{100}=31.70$ cfs.

Basin 76 represents the central portion of the undeveloped parcel and the northwesterly portion of the existing commercial development and is directly tributary to Fountain Creek. The property north of Basin 76 is contained within the St. Dominic's Church Subdivision. Storm runoff from the St. Dominic's Church Subdivision is collected on-site and conveyed through a private detention pond prior to historic release west of the parcel. The Preliminary Drainage Report shows $Q_5=6.89$ cfs, $Q_{100}=12.07$ cfs entering the residential parcel from the northwest corner of the commercial development and exhibits $Q_5=11.87$ cfs, $Q_{100}=28.05$ cfs leaving the site and entering Fountain Creek.

Basin 75 contains the westerly portion of the proposed residential development. The preliminary drainage report indicates that $Q_5=20.28$ cfs, $Q_{100}=45.99$ cfs enter the west side of the parcel from the adjacent agricultural property. Topography does not indicate a channelized flow but rather overland flow from the west. The anticipated long term use for the adjacent parcel is to remain agricultural. The foundation that owns the parcel is extending an irrigation ditch along the west boundary of the subject property to divert flows from the adjacent parcel south to Fountain Creek. An additional 15' setback is proposed in the residential development plan to allow for grading of a fill slope to convey flows south the Fountain Creek.

DEVELOPED DRAINAGE BASINS

The intent of the proposed development is to follow closely to historic drainage patterns while satisfying current El Paso County development and water quality criteria. The area of the site proposed for impervious development will be contained within the parking/private roadway section and private on-site storm sewer system conveying flows to a full spectrum detention basin and water quality facility within the southeast portion of the site prior to outfall to Fountain Creek.

Development of the site includes 225 residential lots, roadway and utility infrastructure to be constructed in 2 filings. Due to limited grade within the site necessitating flat roadway sections minimal drainage will be conveyed within the street roadway sections and drainage will primarily be conveyed is public storm drain systems conveying flows to outfall within a private extended detention basin. The private extended detention basin will be developed to accept developed runoff from the proposed redeveloped commercial center along the parcel's northeasterly boundary.

Flows generated within the proposed commercial center redevelopment will be conveyed within the commercial curb lines and private storm drain to be constructed along the southerly property boundary and outfall directly to the proposed shared extended detention basin.

Offsite Basins

Offsite flow (Historic sub-basin 75) generates runoff of $Q_5=20.28$ cfs, $Q_{100}=45.99$ cfs from the adjacent westerly agricultural parcel currently enter the property at an existing low point along the westerly boundary. Flows will be conveyed in a 10' wide constructed grass swale at 0.5% with 3:1 side slopes from the existing low point south approximately 300' to outfall within the adjacent reach of Fountain Creek. See calculations in the Appendix.

Historic Basins 76 and 77 have been remodeled in this report as either 'B' designated basins within the interior or 'C' basins representing the commercial development. 'B' designated basins will be conveyed within interior street, inlet, and storm sewer conveyance systems to the proposed full spectrum pond proposed with the development. 'C' designated basins will be conveyed in a separate storm sewer system developed in conjunction with commercial redevelopment to outfall to the shared on-site extended detention system. The detailed drainage plan for the commercial development is currently under review with the city of Fountain and a composite basin representing the commercial development is utilized in calculation for sizing the shared pond. Runoff from the commercial development will not directly enter the Riverbend Crossing residential development with the exception of storm sewer outfall directly to the pond.

'A Basins'

Basin A1 (1.30 Acres, $Q_2=0.2$ cfs, $Q_5=0.6$ cfs, $Q_{10}=1.1$ cfs, $Q_{25}=1.8$ cfs, $Q_{50}=2.3$ cfs, and $Q_{100}=2.9$ cfs) consists of the proposed diversion swale along the southern portion of the western boundary. Development will consist of a grass lined swale intended to divert offsite flows south to Fountain Creek. The swale will be located within a tract to be owned and maintained by the Riverbend Crossing Metropolitan District.

Update narrative to discuss why no WQCV is provided for this basin. Per the recent ECM update 100% of the applicable development site must be captured for WQ with specific exclusions. Identify the specific criteria in ECM I.7.1.B which excludes Basin A from providing permanent WQCV. Unresolved.

offsite high flows from the adjacent parcel to the proposed diversion channel is not intended to be captured for WQ with specific exclusions. The proposed diversion channel is not intended to be captured for WQ with specific exclusions. Identify the specific criteria in ECM I.7.1.B which excludes Basin A from providing permanent WQCV. Unresolved.

Revise. There is no bullet item 4. Per the previous comment above example narrative staff is expecting Basin A1 does not require permanent water quality control measure since it is an open space area with land disturbance to undeveloped land that will remain undeveloped per ECM Appendix I Section I.7.1.B.7

residential development. Runoff from 'B' sections, be collected in Type 'R' inlets and extended detention basin.

Basin B17 (0.87 acres) contains the rear portions of lots adjacent to Fountain Creek. Runoff generated within Basin B17 will be collected in private area drains placed along rear lot lines. Flows will be conveyed in a private 8" HDPE storm sewer to confluence with the 48" HDPE at storm design point 12.

Basin B18 (4.86 acres) consists of rear portions of lots adjacent to the proposed extended detention basin and the extended detention basin. Runoff generated within Basin B18 will sheet flow overland directly to the proposed extended detention basin.

Provide inlet calculations for the inlets located at basins B1 and B2.

BASIN	AREA	Q ₂	Q ₅	Q ₁₀	Q ₂₅	Q ₅₀	Q ₁₀₀	Type R Inlet
B1	1.70	2.1	2.9	3.8	4.9	5.8	6.9	10'
B2	1.33	2.0	2.7	3.4	4.3	5.1	6.0	10'
B3	2.29	2.9	4.0	5.2	6.6	7.9	9.3	10'
B4	1.26	1.8	2.5	3.2	4.0	4.8	5.6	5'
B5	3.57	4.8	6.6	8.4	10.5	12.5	14.6	10'
B6	1.67	2.1	2.9	3.7	4.6	5.5	6.4	10'
B7	3.79	4.0	5.7	7.7	9.9	11.9	14.2	10'
B8	0.33	0.5	0.7	0.9	1.1	1.3	1.6	5'
B9	3.19	4.3	5.9	7.6	9.5	11.3	13.2	10'
B10	2.15	3.0	4.0	5.2	6.5	7.6	9.0	10'
B11	4.41	6.0	8.2	10.6	13.2	15.7	18.4	15'
B12	3.74	5.1	7.0	9.0	11.3	13.3	15.6	10'
B13	1.96	2.6	3.5	4.6	5.7	6.7	7.9	DP-A1
B14	1.35	1.8	2.5	3.2	4.0	4.7	5.5	5'
B15	1.15	1.2	1.6	2.1	2.6	3.1	3.7	5'
B16	2.19	2.4	3.3	4.2	5.3	6.3	7.4	DP-A1
B17	0.87	1.2	1.7	2.2	2.7	3.2	3.7	AREA
B18	4.86	1.9	3.6	5.8	8.5	10.7	13.3	POND

B12 inlet size is 15' per the inlet calculation

15' correct

15' per inlet cal

15' collect

The development contains roadways with minimum grades of 1.0%. Roadway conveyance at minimum grade of 1.0% is $Q_5=8.5$ cfs and $Q_{100}=37$ cfs exceeding individual basin runoff. Inlets were developed in sump locations throughout the development and flow-by is not anticipated. Inlet calculations for Basins B1 through B16 are provided in the appendix. Lots will be developed with side lot line swales directed to the streets. Lot templates for 'A' lot grading, 'B' lot grading, and a limited number of Walkout units will be provided in the final grading plan.

Basin B13 and Basin B16 are combined in the southerly knuckle at design Point A1. Combined flows at Design Point A1 of $Q_5=6.3$ cfs and $Q_{100}=13.9$ cfs are collected in a 20' sump inlet within the knuckle. Inlet calculation is provided in the appendix.

Missing from drainage map. Is this supposed to be DP-P4?

Revised to DP-P4

'C Basins'

Basin C/ DP-P4 (11.25 Acres, $Q_5=38.5$ cfs, $Q_{100}=70.2$ cfs) represents the combined flow generated within the commercial development. Runoff generated within the commercial development sheet flows within the proposed curb line and is collected within private inlets on-site and will be conveyed in a private storm sewer to the residential development represented by Pipe Design Point C1. Runoff will be conveyed in a proposed private 30" storm sewer to confluence with the proposed public storm sewer system within the residential development. From the confluence commercial runoff will be conveyed in public (El Paso sewer to outfall within the sub-regional extended detention basin.

Update. Construction plans and map notes

36" Now 42"

Overall contribution of the redeveloped commercial development is comparable with the historical analysis contributions from basin 77 ($Q_5=25.99$ cfs, $Q_{100}=45.15$ cfs) and commercial portion of Basin 76 ($Q_5=6.89$ cfs, $Q_{100}=12.07$ cfs) for a total commercial runoff of ($Q_5=32.88$ cfs, $Q_{100}=57.22$ cfs). Developed runoff from the commercial development at Design Point P4 is $Q_5=38.5$ cfs, $Q_{100}=70.2$ cfs.

The "Master Development Drainage Report for Riverbend Crossing and Final Drainage Report for Riverbend Crossing Commons" has completed review with City of Fountain. Approval is contingent upon providing final El Paso County/City of Fountain pond maintenance agreement and offsite extended detention basin Operation and Maintenance plan submitted as submitted to El Paso County.

Storm Sewer

Flows collected within 'B' designated basin inlets will be conveyed in a public storm sewer system located predominantly within the street ROW which outfalls to the private extended detention basin. Hydraulic grade line calculations developed with UDCFD UDSEWER are provided in the appendix of this report.

Pipe Design Point P4 (commercial outfall) ($Q_5=38.5$ cfs and $Q_{100}=70.2$) represents combined flows from the commercial development and will be conveyed in a public 36" RCP at a minimum grade of 0.5%.

Pipe Design Point 1 ($Q_5=40.5$ cfs and $Q_{100}=76.3$) represents combined flows from basins B1 and B2 and Design Point P4 (commercial) and will be conveyed in a public 36" RCP.

Pipe Design Point 2 ($Q_5=6.4$ cfs and $Q_{100}=14.6$) represents combined flows from basins B3 and B4 and will be conveyed in a public 24" RCP at a minimum grade of 1.8%.

Pipe Design Point 3 ($Q_5=14.9$ cfs and $Q_{100}=33.6$) represents combined flows from basins B5 and B6 and Pipe Design Point 2. Combined flows will be conveyed in a public 24" RCP at a minimum grade of 0.65%.

Pipe Design Point 5 ($Q_5=6.3$ cfs and $Q_{100}=15.5$) represents combined flows from basins B7 and B8 and will be conveyed in a public 24" RCP at a minimum grade of 0.5%.

Pipe Design Point 6 ($Q_5=15.8$ cfs and $Q_{100}=36.5$) represents combined flows from basins B9 and B10 and Pipe Design Point 5. Combined flows will be conveyed in a public 30" RCP at a minimum grade of 0.66%.

Pipe Design Point 7 ($Q_5=36.5$ cfs and $Q_{100}=82.5$) represents combined flows from Pipe Design Points 6 and 11. Combined flows will be conveyed in a public 48" RCP at a minimum grade of 0.7%.

Pipe Design Point 8 ($Q_5=15.2$ cfs and $Q_{100}=33.9$) represents combined flows from basins B11 and B12 and will be conveyed in a public 30" RCP at a minimum grade of 0.70%.

Pipe Design Point 9 ($Q_5=21.1$ cfs and $Q_{100}=46.9$) represents combined flows from Basin B9 and Pipe Design Point 8. Combined flows will be conveyed in a public 36" RCP at a minimum grade of 0.50%.

Pipe Design Point 10 ($Q_5=23.4$ cfs and $Q_{100}=52.1$) represents combined flows from Pipe Design Point 9 and overland Design Point A1. Combined flows will be conveyed in a private 36" RCP pipe at a minimum grade of 0.065%. Emergency overflow will utilize a depressed driveway access for the adjacent lift station flag lot access and convey overflow directly to Fountain Creek.

Pipe Design Point 11 ($Q_5=23.8$ cfs and $Q_{100}=53.0$) represents combined flows from Pipe Design Point 10 and Basin B15. Combined flows will be conveyed in a private 36" RCP pipe at a minimum grade of 0.065%.

Pipe Design Point 4 ($Q_5=80.0$ cfs and $Q_{100}=168.9$) represents combined flows from Pipe Design Points 1,3, and 7. Combined flows will be conveyed in a private 60" RCP at a minimum grade of 0.75%.

Pipe Design Point 12 ($Q_5=81.2$ cfs and $Q_{100}=171.7$) represents combined flows from Pipe Design Points 4,3, and Basin B17. Combined flows will be conveyed in a private 60" RCP at a minimum grade of 0.75%.

CHANNEL BANK STABILIZATION

Fountain Creek in this location is considered a large conveyance natural channel. The existing channel bed is composed of sand and gravel with scattered boulders and cobbles. The channel bottom width varies from 300' to 1,250'. This channel reach relatively straight except for the south end where it makes an abrupt bend just downstream of the project limits. The channel conveys flow from north to south with bed slopes in the range of 0.9% to 1.4%. Moderate to dense vegetation (brush and grasses) are present along the channel edges. The project study reach is approximately 1,615' long. Refer to the Drainage Map in the appendix which depicts the proposed conditions.

The study in effect for this reach is the FEMA Flood Insurance Study (FIS) 08041CV dated December 7, 2018. A flow value for the 100yr design storm was determined using the FIS Summary of Discharges table values of mean velocity and section area at section "CH" which was the location adjacent to the project which yielded the highest flow value of ~26,300cfs. Additional hydraulic analysis was performed for this project using USACE HECRAS software, version 5.07. A supercritical flow regime was used to compute the theoretical water surface elevation profile and determine flow velocities, Froude numbers, and flow depths. A roughness coefficient value of 0.035 was used for design purposes per criteria recommendation. Although the channel has a higher roughness coefficient based on actual bed material and vegetation conditions, using the lower value is a more conservative design approach.

Resultant velocities ranged from ~9ft/s to ~15ft/s. Flow depths ranged from 4ft to 7ft. Froude numbers were determined to be around 1.0. Using this data and the UDFCD Volume 1, Chapter 8 design methodology for channel design, bank lining was deemed necessary. Erosion protection in the form of 12" soil riprap (24' thick) will be provided along the proposed 2.5:1 side slopes. Assuming the channel is entirely composed of non-cohesive soils, the riprap toe protection will be constructed with a five foot bury depth. The height of the soil riprap on the slope will be based on the FIS flow depths and not the model results, which is a conservative approach since the model revealed lower depths. A freeboard value of 1.5' will be used per criteria recommendations. Refer to appendix for additional hydraulic analysis and design information.

EXTENDED DETENTION BASIN

The parcel proposes to develop 54.90 acres within the West Little Johnson Drainage Basin directly tributary to Fountain Creek requiring development of water quality treatment and full-spectrum detention per the criteria of the El Paso County Drainage Criteria Manual Volume 2. The proposed extended detention basin will be developed to provide water quality and full spectrum detention for both the Riverbend Crossing residential development Filings No. 1 and 2 and the Riverbend Crossing Commons Commercial development within the City of Fountain. The proposed Extended Detention Basin located in the southerly portion of the development has 54.90 tributary acres of development with an average imperviousness of 65.40%. Full spectrum pond development requires 1.170 acre-ft of water quality capture volume ponding to an elevation of 5685.95, an EURV volume of 3.71-acre ft, and a total volume of 6.17 acre-ft ponding to an elevation of 5689.98 providing full spectrum detention including the 100-YR event.

Runoff generated within the site will be conveyed to the pond through storm sewer systems or as direct sheetflow. The storm sewer systems will outfall directly to 6" concrete forebays with baffle providing adequate protection at discharge point. The concrete forebays require a total volume of 1,525 cubic feet of volume (2% of the design WQCV). The forebay will be constructed of a concrete slab with sides conforming to the pond slopes and 1' wall with a 9" rectangular notch which outfalls to the proposed trickle channel at the downstream end.

The pond will be constructed with 4:1 minimum side slopes to be vegetated per the final landscape plan. A 4' wide by 6" deep concrete trickle channel with a 0.5% longitudinal slope will convey low flows across the pond bottom to the micropool/outlet structure. The trickle channel will outfall to a 17' long by 7' wide by 2.5' deep concrete micropool. The micropool will provide a surface area of 120 square feet and an initial surcharge volume of 80 cubic feet utilizing an 8" initial surcharge depth.

A portion of the pond is situated below the Base Flood Elevation of the 100-YR recurrence event within the adjacent portion of Fountain Creek, 5689.00. Excess volume exceeding the 100-YR event volume above the base flood elevation was incorporated into the pond to overcome backwater effects should the subdivision experience a 100-YR event concurrent with passage of maximum flood event within the adjacent reach of Fountain Creek.

The outlet structure will consist of a concrete box with orifice plate and screen providing water quality outlet and weir with trash rack for larger storm outfall. The pond will outfall through a private 30" RCP pipe system directly to Fountain Creek.

The emergency spillway will consist of a 60' weir along the southerly end of the pond at an elevation of 5691.00. The overflow area will consist of 12" depth of type VL soil riprap.

Outfall from the extended detention basin of $Q_2=1.0$ cfs, $Q_5=2.6$ cfs, $Q_{10}=7.8$ cfs, $Q_{25}=18.2$ cfs, $Q_{50}=27.2$ cfs, and $Q_{100}=36.4$ will be conveyed in a private 30" RCP. Combined flows at Design P-out is less than historic runoff from basins 75,76, and 77. Outfall from the onsite extended detention basin will be conveyed directly to Fountain Creek through the private 30" HDPE and full spectrum release will have no impacts on the Fountain Creek Drainage.

4-STEP PROCESS

STEP 1: EMPLOY RUNOFF REDUCTION PRACTICES

The development addresses Low Impact Development strategies primarily through the utilization of landscape swales within sides and rear of proposed residential lots and directing runoff from buildings and walkways through swales with minimal longitudinal grade prior to outfall to street collection and storm conveyance systems.

STEP 2: STABILIZE DRAINAGEWAYS

The ultimate recipient of runoff from the site is Fountain Creek. Flows from the site are tributary to the full spectrum extended detention basin constructed on site with development of the Riverbend Crossing community and commercial center attenuating flows to predevelopment levels. Existing slopes adjacent to Fountain Creek will be anchored with rip rap toe protection and excessive slopes will be lain back to a slope of 2.5 Horizontal to 1' Vertical. 100=YR slope protection will be installed within reconstructed slopes.

STEP 3: PROVIDE WATER QUALITY CAPTURE VALUME

On-site flow is directed to the on-site private proposed full spectrum extended detention basin constructed with development of the project which outfalls directly to historic outfall within Fountain Creek. The extended detention basin provides Water Quality Capture Volume required for this site and concurrent commercial development and attenuates release of flows to approximate historic runoff.

STEP 4: CONSIDER NEED FOR INDUSTRIAL AND COMMERCIAL BMP'S

A Grading, Erosion Control, and Stormwater Quality Plan and narrative will be approved by El Paso County prior to any soil disturbance. The erosion control plan will include specific source control BMP's as well as defined overall site management practices for the construction period. The grading narrative will address materials storage and spill containment during construction operations. No industrial processes are proposed in development of the Riverbend Community.

COST ESTIMATE

Public Improvements Non-reimbursable

5' Type R Inlet	2 EA	@\$	3,800/EA	\$	7,600
10' Type R Inlet	9 EA	@\$	5,500/EA	\$	49,500
15' Type R Inlet	1 EA	@\$	8,000/EA	\$	8,000
20' Type R Inlet	1 EA	@\$	10,000/EA	\$	10,000
Type I Manhole	11 EA	@\$	4,000/EA	\$	40,000
18" RCP	213 LF	@\$	45/LF	\$	9,585
24" RCP	2,102 LF	@\$	55/LF	\$	115,610
30" RCP	1,411 LF	@\$	68/LF	\$	95,948
42" RCP	152 LF	@\$	90/LF	\$	13,680
48" RCP	151 LF	@\$	110/LF	\$	16,610

SUBTOTAL \$ **366,533**
10% CONTINGENCY \$ 36,653
TOTAL \$ **403,168**

Private Improvements Non-reimbursable

48" HDPE	552 LF	@\$	85/LF	\$	46,920
WATER QUALITY POND	1 EA	@\$	65,000/EA	\$	65,000
BANK STABILIZATION <i>soil riprap</i>	3,345 TONS	@\$	80/TON	\$	267,600

SUBTOTAL \$ **379,520**
10% CONTINGENCY \$ 37,952
TOTAL \$ **417,452**

DRAINAGE FEE CALCULATION

Riverbend Crossing Filing No. 1 contains 36.5 acres to be platted within the West Little Johnson Drainage Basin. Riverbend Crossing Filing No. 2 contains 15.5 acres to be platted within the West Little Johnson Drainage Basin. The 2018 fee for the West Little Johnson Drainage Basin (A miscellaneous Drainage Basin) is \$1,133/ per impervious acre.

Filing No.1-36.547 total acres.

Use	Acres	Imperviousness
1/8 acre or less	23.45	65%
Open Space	13.09	7%
Composite Imperviousness:	44.2%	

$$36.547 \text{ acres} \times 44.2\% \times \$1,133.00 = \$18,311$$

Filing No.2-15.452 total acres.

Use	Acres	Imperviousness
1/8 acre or less	14.48	65%
Open Space	0.97	7%
Composite Imperviousness:	61.4%	

$$15.452 \text{ acres} \times 61.4\% \times \$1,133.00 = \$10,742$$

Add a statement that there are no bridge fees in the West Little Johnson Drainage Basin.

ADDGD

Include a Header/Title. Typical for all worksheets.
 Unresolved. **ADDed**

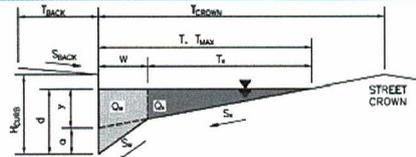
DESIGN POINT	AREA TOTAL (Acres)	WEIGHTED						TOTAL (min)	INTENSITY						TOTAL FLOWS						
		C ₂	C ₅	C ₁₀	C ₂₅	C ₅₀	C ₁₀₀		I ₂ (in/hr)	I ₅ (in/hr)	I ₁₀ (in/hr)	I ₂₅ (in/hr)	I ₅₀ (in/hr)	I ₁₀₀ (in/hr)	Q ₂ (cfs)	Q ₅ (cfs)	Q ₁₀ (cfs)	Q ₂₅ (cfs)	Q ₅₀ (cfs)	Q ₁₀₀ (cfs)	
COMMERCIAL	11.26	0.8	0.82	0.84	0.87	0.88	0.89	9.7	3.3	4.2	4.9	5.6	6.3	7	30.0	38.5	46.1	54.5	62.1	70.2	
1	14.29	0.72	0.75	0.77	0.81	0.82	0.84	12.5	3.0	3.8	4.4	5.1	5.7	6.4	31.2	40.5	49.0	58.5	67.0	76.3	
P4	11.26	0.80	0.82	0.84	0.87	0.88	0.89	9.7													
BASIN B1	1.70	0.41	0.45	0.51	0.57	0.60	0.63	12.5													
BASIN B2	1.33	0.45	0.49	0.54	0.59	0.62	0.65	10.2													
2	3.55	0.42	0.47	0.52	0.58	0.61	0.64	12.3	3.0	3.8	4.5	5.1	5.7	6.4	4.6	6.4	8.3	10.4	12.4	14.6	
BASIN B3	2.29	0.41	0.46	0.51	0.57	0.60	0.64	12.3													
BASIN B4	1.26	0.45	0.49	0.54	0.59	0.62	0.65	10.5	2.8	3.5	4.1	4.7	5.3	5.9	10.9	14.9	19.2	24.1	28.6	33.6	
3	8.79	0.44	0.48	0.53	0.58	0.62	0.65	15.0													
BASIN B5	3.57	0.45	0.49	0.54	0.59	0.62	0.65	12.9													
BASIN B6	1.67	0.45	0.49	0.54	0.59	0.62	0.65	15.0													
DP-2	3.55	0.42	0.47	0.52	0.58	0.61	0.64	12.3													
4	49.18	0.52	0.56	0.60	0.65	0.68	0.70	22.4	2.3	2.9	3.4	3.9	4.4	4.9	59.7	80.0	100.8	124.2	145.3	168.9	
DP-1	14.29	0.72	0.75	0.77	0.81	0.82	0.84	12.5													
DP-7	26.10	0.44	0.48	0.53	0.58	0.61	0.65	22.4													
DP-3	8.79	0.44	0.48	0.53	0.58	0.62	0.65	15.0													
5	4.12	0.37	0.43	0.49	0.55	0.59	0.62	14.2	2.9	3.6	4.2	4.8	5.4	6.1	4.4	6.3	8.4	10.9	13.0	15.5	
BASIN B7	3.79	0.37	0.42	0.48	0.55	0.58	0.62	14.2													
BASIN B8	0.33	0.45	0.49	0.54	0.59	0.62	0.65	8.5													
6	9.46	0.42	0.46	0.52	0.57	0.61	0.64	14.2	2.9	3.6	4.2	4.8	5.4	6.1	11.3	15.8	20.6	26.0	31.0	36.5	
BASIN B9	3.19	0.45	0.49	0.54	0.59	0.62	0.65	12.5													
BASIN B10	2.15	0.45	0.49	0.54	0.59	0.62	0.65	12.3													
DP-5	4.12	0.37	0.43	0.49	0.55	0.59	0.62	14.2													
7	26.10	0.44	0.48	0.53	0.58	0.61	0.65	22.4	2.3	2.9	3.4	3.9	4.4	4.9	26.7	36.5	47.2	59.3	70.2	82.5	
DP-11	16.64	0.45	0.49	0.54	0.59	0.62	0.65	22.4													
DP-6	9.46	0.42	0.46	0.52	0.57	0.61	0.64	14.2													
8	8.15	0.45	0.49	0.54	0.59	0.62	0.65	12.3	3.0	3.8	4.5	5.1	5.7	6.4	11.2	15.2	19.6	24.5	28.9	33.9	
BASIN B11	4.41	0.45	0.49	0.54	0.59	0.62	0.65	12.3													
BASIN B12	3.74	0.45	0.49	0.54	0.59	0.62	0.65	12.2													
9	11.34	0.45	0.49	0.54	0.59	0.62	0.65	12.5	3.0	3.8	4.4	5.1	5.7	6.4	15.5	21.1	27.1	33.8	40.0	46.9	
B9	3.19	0.45	0.49	0.54	0.59	0.62	0.65	12.5													
DP-8	8.15	0.45	0.49	0.54	0.59	0.62	0.65	12.3	2.5	3.1	3.6	4.1	4.6	5.2	4.6	6.3	8.1	10.1	11.9	13.9	
A1	4.15	0.45	0.49	0.54	0.59	0.62	0.65	20.1													
BASIN B13	1.96	0.45	0.49	0.54	0.59	0.62	0.65	13.4													
BASIN B16	2.19	0.45	0.49	0.54	0.59	0.62	0.65	20.1													
10	15.49	0.45	0.49	0.54	0.59	0.62	0.65	20.1	2.5	3.1	3.6	4.1	4.6	5.2	17.2	23.4	30.1	37.5	44.4	52.1	
DP A1	4.15	0.45	0.49	0.54	0.59	0.62	0.65	20.1													
DP-9	11.34	0.45	0.49	0.54	0.59	0.62	0.65	12.5													
11	16.64	0.45	0.49	0.54	0.59	0.62	0.65	22.4	2.3	2.9	3.4	3.9	4.4	4.9	17.5	23.8	30.6	38.2	45.2	53.0	
BASIN B15	1.15	0.45	0.49	0.54	0.59	0.62	0.65	22.4													
DP-10	15.49	0.45	0.49	0.54	0.59	0.62	0.65	20.1													
12	50.05	0.52	0.56	0.60	0.65	0.67	0.70	22.4	2.3	2.9	3.4	3.9	4.4	4.9	60.6	81.2	102.4	126.2	147.7	171.7	
BASIN B17	0.87	0.45	0.49	0.54	0.59	0.62	0.65	11.3													
DP-4	49.18	0.52	0.56	0.60	0.65	0.68	0.70	22.4													
P-IN	54.91	0.49	0.53	0.58	0.63	0.66	0.69	22.4	2.3	2.9	3.4	3.9	4.4	4.9	62.5	84.8	108.0	134.5	158.2	184.7	
BASIN B18	4.86	0.16	0.25	0.34	0.44	0.49	0.55	21.4													
DP-12	50.05	0.52	0.56	0.60	0.65	0.67	0.70	22.4													
P-OUT	54.90														1.0	2.6	7.8	18.2	27.2	36.4	
POND OUTLET									POND ROUTED SEE UD DENTENTION												

Calculated by: DLM
 Date: 8/12/2018

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Riverbend Crossing
 Inlet ID: B14



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width
 Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

T_{BACK} =	7.5	ft
S_{BACK} =	0.012	ft/ft
n_{BACK} =	0.020	
H_{CURB} =	6.00	inches
T_{CROWN} =	16.2	ft
W =	2.00	ft
S_X =	0.020	ft/ft
S_W =	0.083	ft/ft
S_L =	0.000	ft/ft
n_{STREET} =	0.016	
T_{MAX} =	Minor Storm: 16.2 Major Storm: 16.2	ft
d_{MAX} =	Minor Storm: 5.1 Major Storm: 7.8	inches
Q_{allow} =	Minor Storm: SUMP Major Storm: SUMP	cfs

Revise longitudinal slope. Inlet is at grade.

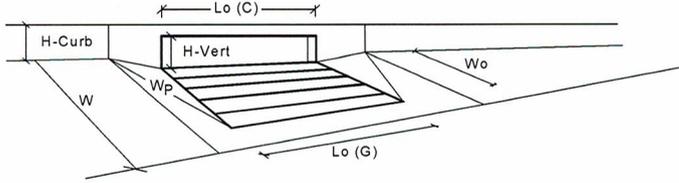
Revised to At Grade

Revise inlet calc. Construction plans show this as an at-grade inlet.

REVISED TO 0.67 SLOPE

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	1	1	
Water Depth at Flowline (outside of local depression)	5.1	7.8	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.26	0.48	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.65	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	1.00	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	3.7	9.0	cfs
Q PEAK REQUIRED	2.5	5.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

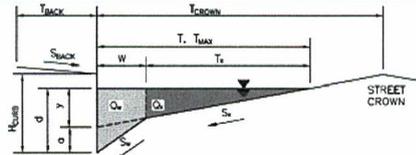
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Riverbend Crossing

Inlet ID:

B15



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

T_{BACK} = 7.5 ft
 S_{BACK} = 0.012 ft/ft
 n_{BACK} = 0.020

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width
 Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

H_{CURB} = 6.00 inches
 T_{CROWN} = 16.2 ft
 W = 2.00 ft
 S_X = 0.020 ft/ft
 S_W = 0.083 ft/ft
 S₀ = 0.000 ft/ft
 n_{STREET} = 0.016

Handwritten note: 0.5790

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

	Minor Storm	Major Storm	
T _{MAX}	16.2	16.2	ft
d _{MAX}	5.1	7.8	inches

MINOR STORM Allowable Capacity is based on Depth Criterion
 MAJOR STORM Allowable Capacity is based on Depth Criterion

Q_{allow} =

Minor Storm	Major Storm
SUMP	SUMP

 cfs

Revise. P&P shows 1% slope towards the inlet.

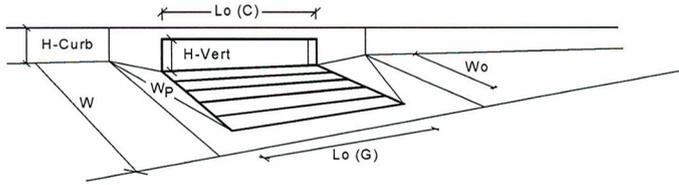
Revise inlet calc. Construction plans show this as an at-grade inlet.

Revise inlet calc. Construction plans show this as an at-grade inlet.

Revised Slope

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR	
Type of Inlet	CDOT Type R Curb Opening	CDOT Type R Curb Opening			
Local Depression (additional to continuous gutter depression 'a' from above)		$a_{local} = 3.00$	3.00	inches	
Number of Unit Inlets (Grate or Curb Opening)		$N_o = 1$	1		
Water Depth at Flowline (outside of local depression)		Ponding Depth = 5.1 (MINOR) / 5.4 (MAJOR) inches			
Grate Information		MINOR		MAJOR	
Length of a Unit Grate		Override Depths			
Width of a Unit Grate		$L_o (G) = N/A$	N/A	feet	
Area Opening Ratio for a Grate (typical values 0.15-0.90)		$W_o = N/A$	N/A	feet	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)		$A_{ratio} = N/A$	N/A		
Grate Weir Coefficient (typical value 2.15 - 3.60)		$C_l (G) = N/A$	N/A		
Grate Orifice Coefficient (typical value 0.60 - 0.80)		$C_w (G) = N/A$	N/A		
		$C_o (G) = N/A$	N/A		
Curb Opening Information		MINOR		MAJOR	
Length of a Unit Curb Opening		$L_o (C) = 5.00$	5.00	feet	
Height of Vertical Curb Opening in Inches		$H_{vert} = 6.00$	6.00	inches	
Height of Curb Orifice Throat in Inches		$H_{throat} = 6.00$	6.00	inches	
Angle of Throat (see USDCM Figure ST-5)		$\theta = 63.40$	63.40	degrees	
Side Width for Depression Pan (typically the gutter width of 2 feet)		$W_p = 2.00$	2.00	feet	
Clogging Factor for a Single Curb Opening (typical value 0.10)		$C_l (C) = 0.10$	0.10		
Curb Opening Weir Coefficient (typical value 2.3-3.7)		$C_w (C) = 3.60$	3.60		
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)		$C_o (C) = 0.67$	0.67		
Low Head Performance Reduction (Calculated)		MINOR		MAJOR	
Depth for Grate Midwidth		$d_{grate} = N/A$	N/A	ft	
Depth for Curb Opening Weir Equation		$d_{curb} = 0.26$	0.28	ft	
Combination Inlet Performance Reduction Factor for Long Inlets		$RF_{combination} = 0.65$	0.69		
Curb Opening Performance Reduction Factor for Long Inlets		$RF_{curb} = 1.00$	1.00		
Grated Inlet Performance Reduction Factor for Long Inlets		$RF_{grate} = N/A$	N/A		
Total Inlet Interception Capacity (assumes clogged condition)		MINOR		MAJOR	
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)		$Q_a = 3.7$	4.2	cfs	
		$Q_{PEAK REQUIRED} = 1.6$	3.7	cfs	

Sewer Input Summary:

HGL Revised

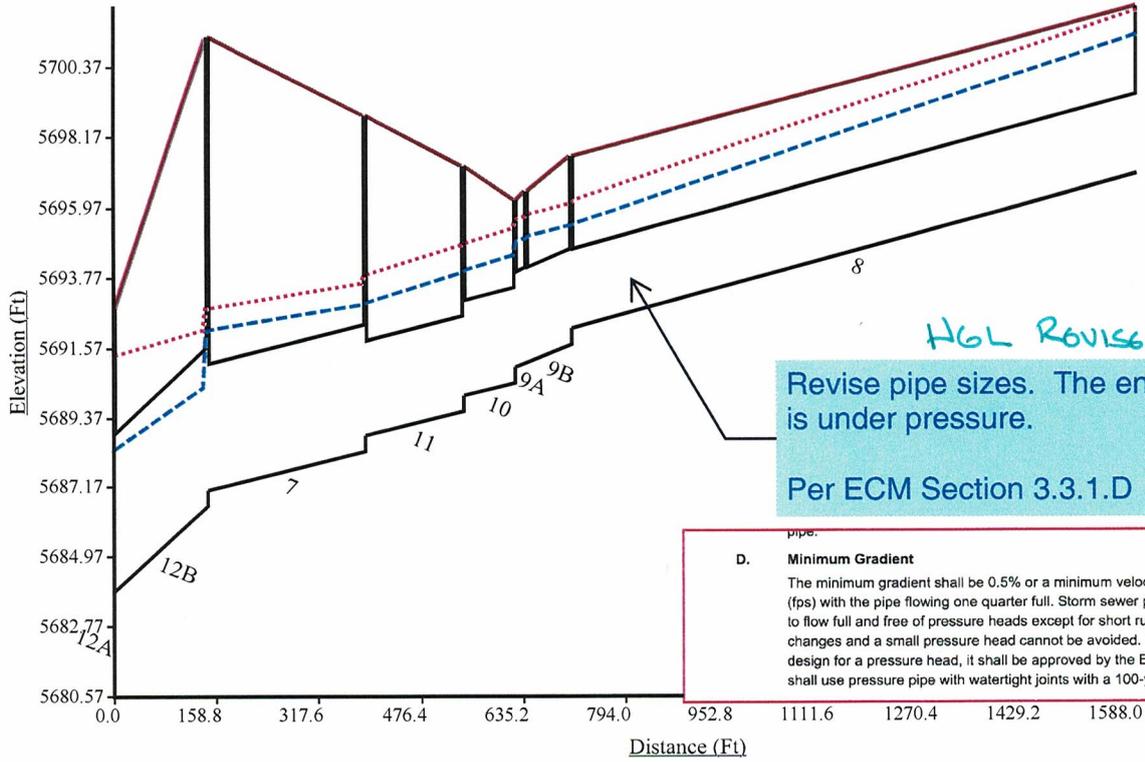
Element Name	Sewer Length (ft)	Elevation			Loss Coefficients			Given Dimensions		
		Downstream Invert (ft)	Slope (%)	Upstream Invert (ft)	Mannings n	Bend Loss	Lateral Loss	Cross Section	Rise (ft or in)	Span (ft or in)
12A	115.29	5682.55	0.72	5683.38	0.013	0.00	0.00	CIRCULAR	60.00 in	60.00 in
12B	145.28	5683.88	1.87	5686.59	0.013	1.00	0.00	CIRCULAR	60.00 in	60.00 in
3	96.23	5690.94	3.99	5694.78	0.013	0.05	0.00	CIRCULAR	24.00 in	24.00 in
2A	496.00	5695.03	1.79	5703.93	0.013	0.05	0.00	CIRCULAR	24.00 in	24.00 in
2B	151.04	5704.18	1.71	5706.76	0.013	0.05	0.00	CIRCULAR	0.00 in	0.00 in
2C	80.20	5707.01	2.76	5709.22	0.013	0.37	0.00	CIRCULAR	24.00 in	24.00 in
1A	493.59	5687.09	3.44	5704.09	0.013	1.00	0.00	CIRCULAR	36.00 in	36.00 in
1B	95.58	5704.34	1.00	5705.30	0.013	1.00	0.00	CIRCULAR	36.00 in	36.00 in
P4A	160.58	5705.55	0.50	5706.36	0.013	0.05	0.00	CIRCULAR	36.00 in	36.00 in
7	244.50	5687.09	0.50	5688.31	0.013	1.00	0.00	CIRCULAR	48.00 in	48.00 in
6	100.08	5689.31	2.77	5692.08	0.013	1.00	0.00	CIRCULAR	30.00 in	30.00 in
5A	458.00	5692.33	1.63	5699.80	0.013	0.05	0.00	CIRCULAR	24.00 in	24.00 in
5B	403.87	5700.05	0.84	5703.45	0.013	0.05	0.00	CIRCULAR	24.00 in	24.00 in
11	153.10	5688.81	0.50	5689.58	0.013	0.05	0.00	CIRCULAR	36.00 in	36.00 in
10	80.48	5690.08	0.50	5690.48	0.013	0.05	0.00	CIRCULAR	36.00 in	36.00 in
9A	17.37	5690.98	0.81	5691.12	0.013	0.43	0.00	CIRCULAR	36.00 in	36.00 in
9B	70.18	5691.12	0.83	5691.70	0.013	0.10	0.00	CIRCULAR	36.00 in	36.00 in
8	877.26	5692.20	0.55	5697.05	0.013	0.05	0.00	CIRCULAR	30.00 in	30.00 in

Sewer Flow Summary:

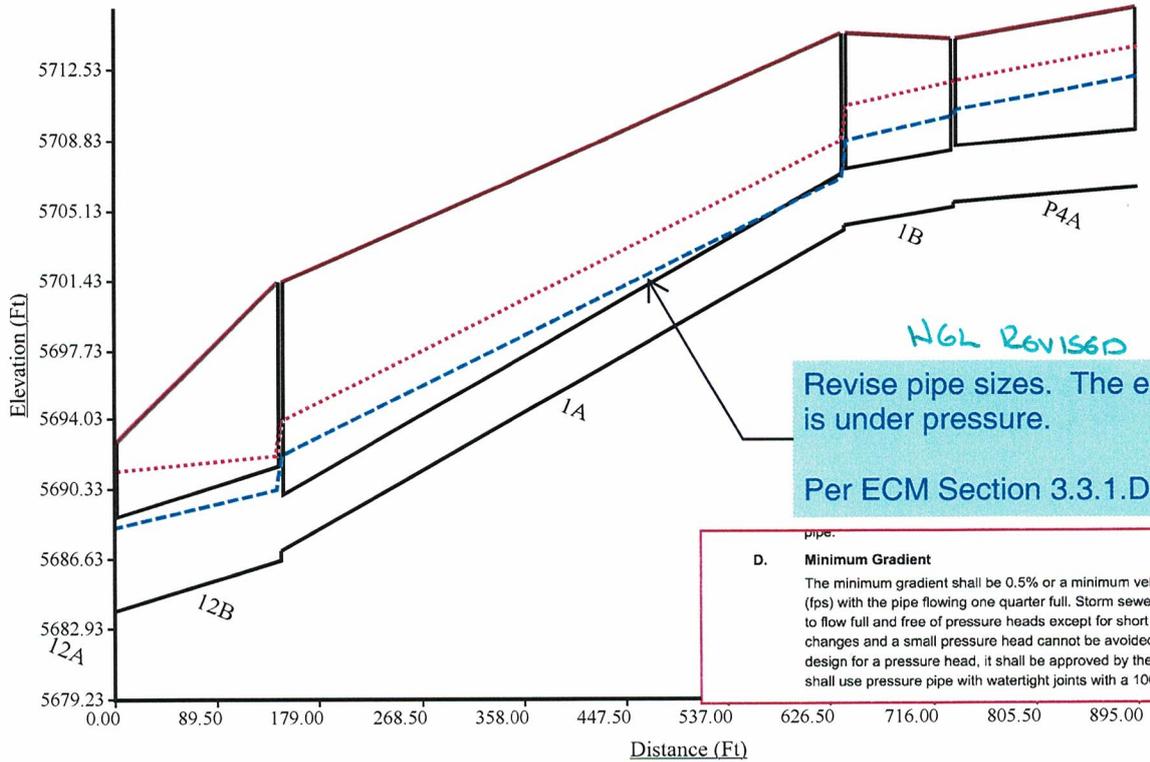
Adjust. Max velocity is 18fps

Element Name	Full Flow Capacity		Critical Flow		Normal Flow				Flow (cfs)	Surcharged Length (ft)	Comment
	Flow (cfs)	Velocity (fps)	Depth (in)	Velocity (fps)	Depth (in)	Velocity (fps)	Froude Number	Flow Condition			
12A	221.59	11.29	45.06	10.85	39.67	12.46	1.29	Supercritical	171.70	0.00	
12B	356.66	18.16	44.70	10.77	29.06	17.92	2.30	Supercritical	168.90	0.00	
3	45.31	14.42	22.89	10.88	15.39	15.79	2.64	Supercritical	33.60	0.00	
2A	30.39	9.67	16.52	6.33	11.72	9.58	1.93	Supercritical Jump	14.60	85.22	
2B	13.76	7.79	16.44	8.04	14.56	8.88	1.37	Supercritical	13.60	0.00	
2C	37.66	11.99	15.93	6.14	9.97	11.02	2.45	Supercritical	13.60	0.00	
1A	124.11	17.56	32.80	11.29	20.41	18.46	2.76	Supercritical Jump	76.30	94.96	Velocity is Too High
1B	67.02	9.48	36.00	10.79	36.00	10.79	0.00	Pressurized	76.30	95.58	
P4A	47.50	6.72	36.00	9.93	36.00	9.93	0.00	Pressurized	70.20	160.58	

WEST



EAST



Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Sheet 1 of 4

Designer: David Mijares
Company: Catamount
Date: September 12, 2019
Project: Riverbend Crossing
Location: Extended Detention Basin 1

1. Basin Storage Volume

- A) Effective Imperviousness of Tributary Area, I_a
- B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)
- C) Contributing Watershed Area
- D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm
- E) Design Concept
(Select EURV when also designing for flood control)
- F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i)) / 12 * Area$)
- G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_b * (V_{DESIGN} / 0.43))$)
- H) User Input of Water Quality Capture Volume (WQCV) Design Volume
(Only if a different WQCV Design Volume is desired)
- I) Predominant Watershed NRCS Soil Group
- J) Excess Urban Runoff Volume (EURV) Design Volume
 For HSG A: $EURV_A = 1.68 * i^{0.8}$
 For HSG B: $EURV_B = 1.38 * i^{0.8}$
 For HSG C/D: $EURV_{C/D} = 1.20 * i^{0.8}$

$I_a =$ 65.4 %
 $i =$ 0.654
 Area = 54.900 ac
 $d_b =$ _____ in

Choose One
 Water Quality Capture Volume (WQCV)
 Excess Urban Runoff Volume (EURV)

$V_{DESIGN} =$ 1.170 ac-ft
 $V_{DESIGN\ OTHER} =$ _____ ac-ft
 $V_{DESIGN\ USER} =$ _____ ac-ft

Choose One
 A
 B
 C / D

$EURV =$ _____ ac-ft

Revise to EURV. Design is for Full Spectrum Detention.
Revised
WQCV selected. Soil group not required.
update
Revised

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.)

L : W = 4.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred)

Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

Concrete Forebay with Pipe outfalls and Energy Baffle per Urban Drainage Criteria

Design Procedure Form: Extended Detention Basin (EDB)

Sheet 2 of 4

Designer: David Mijares
 Company: Catamount
 Date: September 12, 2019
 Project: Riverbend Crossing
 Location: Extended Detention Basin 1

<p>5. Forebay</p> <p>A) Minimum Forebay Volume ($V_{MIN} = 3\%$ of the WQCV)</p> <p>B) Actual Forebay Volume</p> <p>C) Forebay Depth ($D_F = 30$ inch maximum)</p> <p>D) Forebay Discharge</p> <p>i) Undetained 100-year Peak Discharge</p> <p>ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$)</p> <p>E) Forebay Discharge Design</p> <p>F) Discharge Pipe Size (minimum 3-inches)</p> <p>G) Rectangular Notch Width</p>	<p>$V_{MIN} = 0.035$ ac-ft</p> <p>$V_F = 0.035$ ac-ft</p> <p>$D_F = 30.0$ in</p> <p>$Q_{100} = 180.80$ cfs</p> <p>$Q_F = 3.62$ cfs</p> <p>Choose One</p> <p><input type="radio"/> Berm With Pipe</p> <p><input checked="" type="radio"/> Wall with Rect. Notch</p> <p><input type="radio"/> Wall with V-Notch Weir</p> <p>Calculated $D_p =$ _____ in</p> <p>Calculated $W_N = 9.3$ in</p>
<p>6. Trickle Channel</p> <p>A) Type of Trickle Channel</p> <p>F) Slope of Trickle Channel</p>	<p>Choose One</p> <p><input checked="" type="radio"/> Concrete</p> <p><input type="radio"/> Soft Bottom</p> <p>S = _____ ft / ft</p>
<p>7. Micropool and Outlet Structure</p> <p>A) Depth of Micropool (2.5-feet minimum)</p> <p>B) Surface Area of Micropool (10 ft² minimum)</p> <p>C) Outlet Type</p> <p>D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention)</p> <p>E) Total Outlet Area</p>	<p>$D_M =$ _____ ft</p> <p>$A_M =$ _____ sq ft</p> <p>Choose One</p> <p><input type="radio"/> Orifice Plate</p> <p><input type="radio"/> Other (Describe):</p> <p>_____</p> <p>_____</p> <p>$D_{orifice} =$ _____ inches</p> <p>$A_{ot} =$ _____ square inches</p>

fill in this section

Filled in

Design Procedure Form: Extended Detention Basin (EDB)

Sheet 3 of 4

Designer: David Mijares
Company: Catamount
Date: September 12, 2019
Project: Riverbend Crossing
Location: Extended Detention Basin 1

<p>8. Initial Surcharge Volume</p> <p>A) Depth of Initial Surcharge Volume (Minimum recommended depth is 4 inches)</p> <p>B) Minimum Initial Surcharge Volume (Minimum volume of 0.3% of the WQCV)</p> <p>C) Initial Surcharge Provided Above Micropool</p>	<p>$D_{IS} =$ _____ in</p> <p>$V_{IS} =$ _____ cu ft</p> <p>$V_s =$ _____ cu ft</p>
<p>9. Trash Rack</p> <p>A) Water Quality Screen Open Area: $A_t = A_{st} * 38.5 * (e^{-0.095D})$</p> <p>B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open area to the total screen area for the material specified.)</p> <p style="text-align: center;">Other (Y/N): <u> N </u></p> <p>C) Ratio of Total Open Area to Total Area (only for type "Other")</p> <p>D) Total Water Quality Screen Area (based on screen type)</p> <p>E) Depth of Design Volume (EURV or WQCV) (Based on design concept chosen under 1E)</p> <p>F) Height of Water Quality Screen (H_{TR})</p> <p>G) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended)</p>	<p>$A_t =$ _____ square inches</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>User Ratio = _____</p> <p>$A_{total} =$ _____ sq. in.</p> <p>$H =$ _____ feet</p> <p>$H_{TR} =$ _____ inches</p> <p>$W_{opening} =$ _____ inches</p>

complete item 8 and 9

Design Procedure Form: Extended Detention Basin (EDB)

Sheet 4 of 4

Designer: David Mijares
Company: Catamount
Date: September 12, 2019
Project: Riverbend Crossing
Location: Extended Detention Basin 1

<p>10. Overflow Embankment</p> <p>A) Describe embankment protection for 100-year and greater overtopping:</p> <p>B) Slope of Overflow Embankment (Horizontal distance per unit vertical, 4:1 or flatter preferred)</p>	<hr/> <hr/> <hr/> <hr/>
<p>11. Vegetation</p>	<p>Choose One</p> <p><input type="radio"/> Irrigated</p> <p><input type="radio"/> Not Irrigated</p>
<p>12. Access</p> <p>A) Describe Sediment Removal Procedures</p>	<hr/> <hr/> <hr/> <hr/> <hr/> <hr/>
<p>Notes: _____</p> <hr/> <hr/> <hr/>	

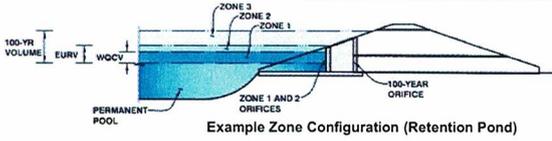
complete items 10, 11, 12



Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: RIVERBEND CROSSING
Basin ID: EXTENDED DETENTION BASIN



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.95	1.170	Orifice Plate
Zone 2 (EURV)	5.08	2.542	Orifice Plate
Zone 3 (100-year)	6.74	2.458	Weir&Pipe (Restrict)
Total		6.169	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = inches

Calculated Parameters for Plate

WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.69	3.39					
Orifice Area (sq. inches)	5.56	5.56	12.00					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft ²
Vertical Orifice Centroid =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	<input type="text" value="5.08"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	<input type="text" value="8.00"/>	<input type="text" value="N/A"/>	feet
Overflow Weir Slope =	<input type="text" value="4.00"/>	<input type="text" value="N/A"/>	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	<input type="text" value="6.00"/>	<input type="text" value="N/A"/>	feet
Overflow Grate Open Area % =	<input type="text" value="70%"/>	<input type="text" value="N/A"/>	%, grate open area/total area
Debris Clogging % =	<input type="text" value="50%"/>	<input type="text" value="N/A"/>	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H ₁ =	<input type="text" value="6.58"/>	<input type="text" value="N/A"/>	feet
Over Flow Weir Slope Length =	<input type="text" value="6.18"/>	<input type="text" value="N/A"/>	feet
Grate Open Area / 100-yr Orifice Area =	<input type="text" value="7.06"/>	<input type="text" value="N/A"/>	should be ≥ 4
Overflow Grate Open Area w/o Debris =	<input type="text" value="34.63"/>	<input type="text" value="N/A"/>	ft ²
Overflow Grate Open Area w/ Debris =	<input type="text" value="17.32"/>	<input type="text" value="N/A"/>	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	<input type="text" value="2.00"/>	<input type="text" value="N/A"/>	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	<input type="text" value="30.00"/>	<input type="text" value="N/A"/>	inches
Restrictor Plate Height Above Pipe Invert =	<input type="text" value="30.00"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	<input type="text" value="4.91"/>	<input type="text" value="N/A"/>	ft ²
Outlet Orifice Centroid =	<input type="text" value="1.25"/>	<input type="text" value="N/A"/>	feet
Half-Central Angle of Restrictor Plate on Pipe =	<input type="text" value="3.14"/>	<input type="text" value="N/A"/>	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = feet
Spillway End Slopes = H:V
Freeboard above Max Water Surface = feet

Calculated Parameters for Spillway

Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres

Routed Hydrograph Results

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.14
Calculated Runoff Volume (acre-ft) =	1.170	3.712	3.254	4.504	5.530	6.972	8.198	9.691	12.843
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	1.169	3.707	3.250	4.499	5.516	6.964	8.180	9.671	12.826
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.11	0.30	0.70	0.97	1.32	2.01
Predevelopment Peak Q (cfs) =	0.0	0.0	0.7	6.1	16.7	38.3	53.1	72.3	110.4
Peak Inflow Q (cfs) =	21.8	68.2	60.0	82.5	100.7	126.4	147.8	173.9	228.4
Peak Outflow Q (cfs) =	0.5	1.3	1.2	4.2	12.3	28.2	42.8	61.8	70.5
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.7	0.7	0.7	0.8	0.9	0.6
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Grate 1	Spillway				
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.1	0.3	0.8	1.2	1.7	2.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	69	65	72	71	69	67	66	62
Time to Drain 99% of Inflow Volume (hours) =	40	74	69	77	77	77	76	75	74
Maximum Ponding Depth (ft) =	2.87	4.95	4.62	5.41	5.82	6.30	6.63	6.98	8.02
Area at Maximum Ponding Depth (acres) =	0.95	1.37	1.31	1.42	1.47	1.52	1.56	1.59	1.72
Maximum Volume Stored (acre-ft) =	1.095	3.536	3.093	4.166	4.759	5.491	5.998	6.549	8.256

Outlet design does not match the construction drawings. Revise.

REVISE

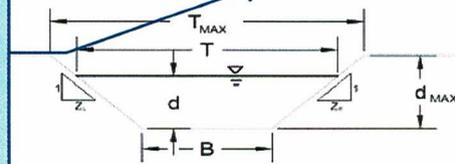
Update Project Name and ID. Staff's assuming this is for the channel in basin A.

Unresolved.

FXGD
UAMW

AREA INLET IN A TRAPEZOIDAL GRASS-LINED CHANNEL

Enter Your Project Name Here
Enter Your Inlet ID Here



Grass Type	Limiting Manning's n
A	0.06
B	0.04
C	0.033
D	0.03
E	0.024

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

NRCS Vegetal Retardance (A, B, C, D, or E)

Manning's n (leave cell blank to manually enter an n value)

Channel Invert Slope

Bottom Width

Left Side Slope

Right Side Slope

Check one of the following soil types:

Soil Type	Max. Velocity (V _{MAX})	Max Froude No. (F _{MAX})
Sandy	5.0 fps	0.50
Non-Sandy	7.0 fps	0.80

A, B, C, D or E

n = 0.035

S₀ = 0.0040 ft/ft

B = 10.00 ft

Z1 = 4.00 ft/ft

Z2 = 4.00 ft/ft

Choose One:

Sandy

Non-Sandy

Max. Allowable Top Width of Channel for Minor & Major Storm

Max. Allowable Water Depth in Channel for Minor & Major Storm

	Minor Storm	Major Storm	feet
T _{MAX}	22.00	22.00	
d _{MAX}	0.80	1.25	

Maximum Channel Capacity Based On Allowable Top Width

Max. Allowable Top Width

Water Depth

Flow Area

Wetted Perimeter

Hydraulic Radius

Manning's n based on NRCS Vegetal Retardance

Flow Velocity

Velocity-Depth Product

Hydraulic Depth

Froude Number

Max. Flow Based On Allowable Top Width

	Minor Storm	Major Storm	feet
T _{MAX}	22.00	22.00	
d	1.50	1.50	
A	24.00	24.00	sq ft
P	22.37	22.37	ft
R	1.07	1.07	ft
n	0.035	0.035	
V	2.82	2.82	fps
VR	3.03	3.03	ft ² /s
D	1.09	1.09	ft
Fr	0.48	0.48	
Q _T	67.72	67.72	cfs

Maximum Channel Capacity Based On Allowable Water Depth

Max. Allowable Water Depth

Top Width

Flow Area

Wetted Perimeter

Hydraulic Radius

Manning's n based on NRCS Vegetal Retardance

Flow Velocity

Velocity-Depth Product

Hydraulic Depth

Froude Number

Max. Flow Based On Allowable Water Depth

	Minor Storm	Major Storm	feet
d _{MAX}	0.80	1.25	
T	16.40	20.00	feet
A	10.56	18.75	square feet
P	16.60	20.31	feet
R	0.64	0.92	feet
n	0.035	0.035	
V	1.99	2.55	fps
VR	1.27	2.36	ft ² /s
D	0.64	0.94	feet
Fr	0.44	0.46	
Q _d	21.03	47.87	cfs

Allowable Channel Capacity Based On Channel Geometry

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

	Minor Storm	Major Storm	cfs
Q _{allow}	21.03	47.87	
d _{allow}	0.80	1.25	ft

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow

Water Depth

Top Width

Flow Area

Wetted Perimeter

Hydraulic Radius

Manning's n based on NRCS Vegetal Retardance

Flow Velocity

Velocity-Depth Product

Hydraulic Depth

Froude Number

	Minor Storm	Major Storm	cfs
Q ₀	20.28	46.00	
d	0.78	1.22	feet
T	16.27	19.79	feet
A	10.30	18.23	square feet
P	16.47	20.09	feet
R	0.63	0.91	feet
n	0.035	0.035	
V	1.97	2.52	fps
VR	1.23	2.29	ft ² /s
D	0.63	0.92	feet
Fr	0.44	0.46	

Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Provide an exhibit showing the cross section location or include in the drainage map.

BANK LINING DESIGN CALCULATIONS

11/23/2019

ADDED TO D2. MAP

HECRAS INPUT

1. EXISTING CONDITIONS CHANNEL WITH 2.5:1 SIDE SLOPES ALONG PROJECT
2. SUPERCRITICAL FLOW REGIME ASSUMED BASED ON N=0.035 (PER TABLE 8-5)
3. 100YR STORM FLOW FROM FIS MAX AT SECTION "CH" ROUNDED TO 26,300CFS

NORMAL DEPTH ADDED

HECRAS OUTPUT

Identify the downstream boundary condition used.

HEC-RAS Plan: 100YrSuper River: Fountain Creek Reach: Main Stem Profile: PF 1												
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main Stem	1615	PF 1	26300.00	5686.80	5690.50	5690.50	5691.72	0.013669	8.86	2969.15	1248.13	1.01
Main Stem	1225	PF 1	26300.00	5682.40	5686.73	5686.73	5688.55	0.011757	10.85	2436.01	681.43	1.01
Main Stem	800	PF 1	26300.00	5678.60	5682.29	5682.29	5683.72	0.012542	9.60	2744.29	959.93	1.00
Main Stem	375	PF 1	26300.00	5673.90	5678.55	5678.55	5680.70	0.010990	11.82	2243.21	527.70	1.00
Main Stem	0	PF 1	26300.00	5671.00	5677.49	5677.49	5680.71	0.009575	14.46	1843.01	291.72	1.00

RIPRAP DESIGN

UDFCD Volume 1, Chapter 8

$$d_{50} \geq \left[\frac{V S^{0.17}}{4.5(G_s - 1)^{0.66}} \right]^2$$

Equation 8-11

Where:

V = mean channel velocity (ft/sec)

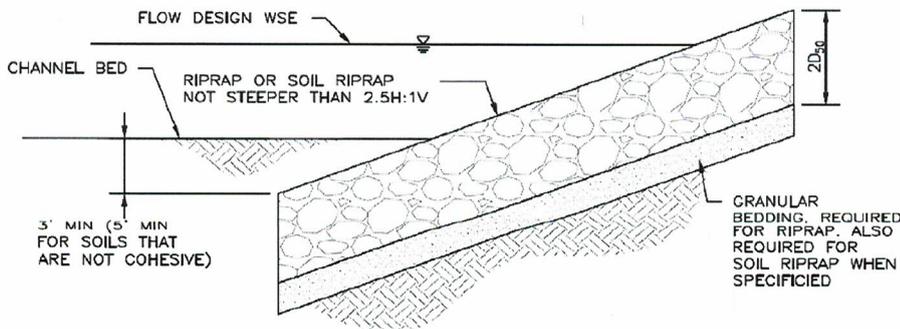
S = longitudinal channel slope (ft/ft)

d₅₀ = mean rock size (ft)

G_s = specific gravity of stone (minimum = 2.50, typically 2.5 to 2.7). Note: In this equation (G_s - 1) considers the buoyancy of the water, in that the specific gravity of water is subtracted from the specific gravity of the rock.

Begin Sta	End Sta	V	S	G _s	d50(ft)	d50(in)	
1615	1225	9.86	0.0113	2.5	0.61	7.3	Use Type L=9in
1225	800	10.22	0.0089	2.5	0.61	7.3	Use Type L=9in
800	375	10.71	0.0111	2.5	0.72	8.6	Use Type L=9in
375	0	13.14	0.0077	2.5	0.95	11.5	Use Type M=12in

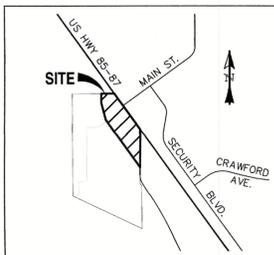
USE TYPE M SOIL RIPRAP ENTIRE REACH



THICKNESS - 2*D50 = 24"

SIDE SLOPE - 2.5:1

BURY DEPTH - 5' (ASSUME NOT COHESIVE)

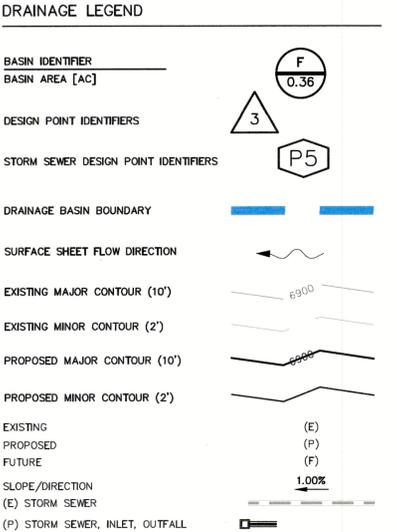


VICINITY MAP
SCALE: N.T.S.

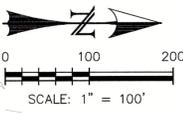
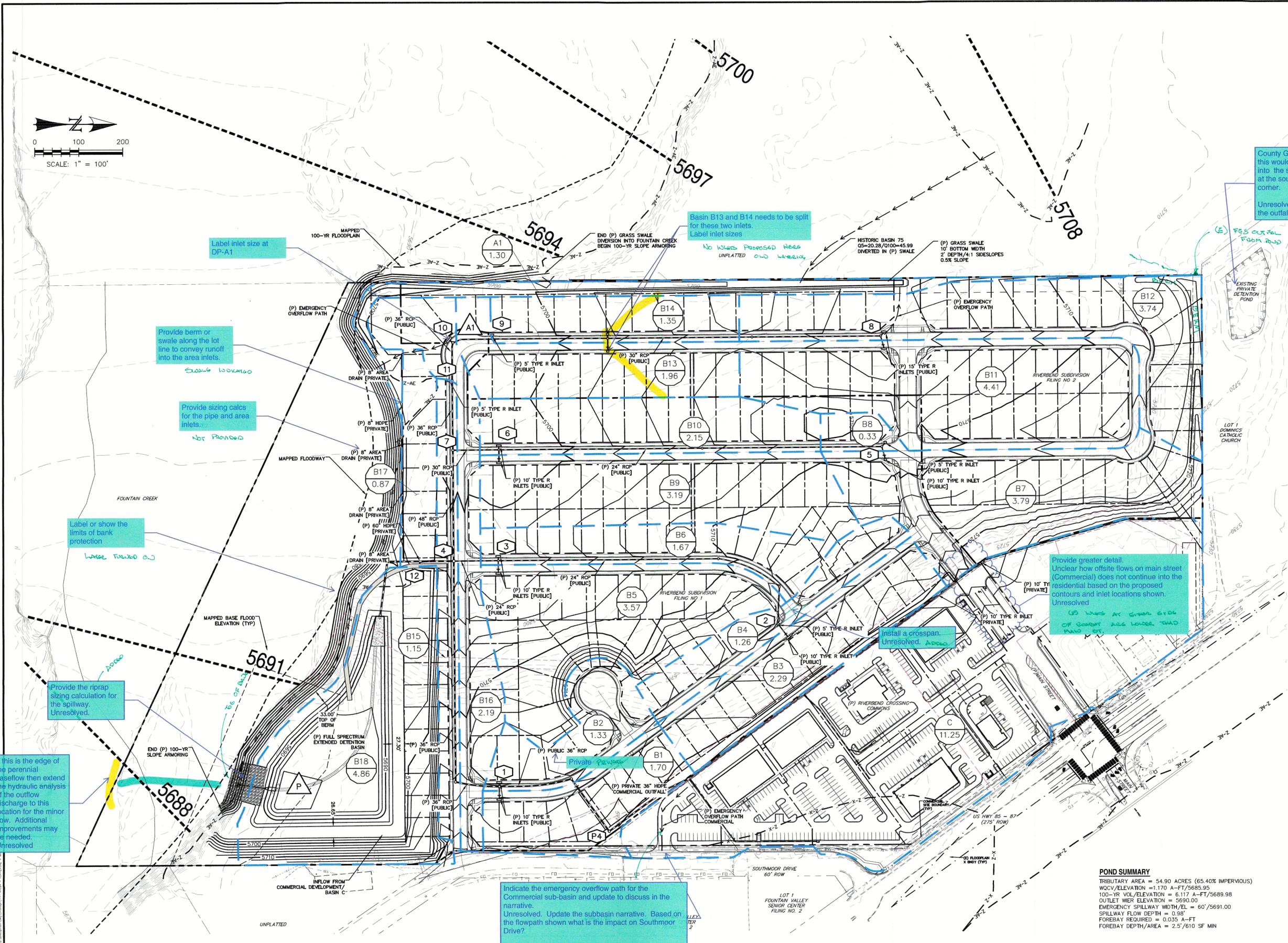
County GIS shows this would drain back into the subdivision at the southwest corner.
Unresolved. Show the outfall location.

PROPOSED DRAINAGE BASINS							
BASIN	AREA (ACRES)	Q2 (CFS)	Q5 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
B1	1.30	0.2	0.6	1.1	1.8	2.3	2.9
B2	1.70	2.1	2.9	3.8	4.9	5.8	6.9
B3	1.33	2.0	2.7	3.4	4.3	5.1	6.0
B4	2.29	2.9	4.0	5.2	6.6	7.9	9.3
B5	1.26	1.8	2.5	3.2	4.0	4.8	5.6
B6	3.57	4.8	6.6	8.4	10.5	12.6	14.6
B7	1.67	2.1	2.9	3.7	4.6	5.5	6.4
B8	3.79	4.0	5.7	7.7	9.9	11.9	14.2
B9	0.33	0.5	0.7	0.9	1.1	1.3	1.6
B10	3.19	4.3	5.9	7.6	9.5	11.3	13.2
B11	2.15	3.0	4.0	5.2	6.5	7.6	9.0
B12	4.41	6.0	8.2	10.6	13.2	15.7	18.4
B13	3.74	5.1	7.0	9.0	11.3	13.3	15.6
B14	1.96	2.6	3.5	4.6	5.7	6.7	7.9
B15	1.35	1.8	2.5	3.2	4.0	4.7	5.5
B16	1.15	1.2	1.6	2.1	2.6	3.1	3.7
B17	2.19	2.4	3.3	4.2	5.3	6.3	7.4
B18	0.87	1.2	1.7	2.2	2.7	3.2	3.7
B19	4.86	1.9	3.6	5.8	8.5	10.7	13.3
C	11.25	30.0	38.5	46.0	54.5	62.0	70.2

PROPOSED DESIGN POINTS						
DESIGN POINT	Q2 (CFS)	Q5 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
P1	30.0	38.5	46.1	54.5	62.1	70.2
1	31.2	40.5	49.0	58.5	67.0	76.3
2	4.6	6.4	8.3	10.4	12.4	14.6
3	10.9	14.9	19.2	24.1	28.6	33.6
4	59.7	80.0	100.8	124.2	145.3	168.9
5	4.4	6.3	8.4	10.9	13.0	15.5
6	11.3	15.8	20.6	26.0	31.0	36.5
7	26.7	36.5	47.2	59.3	70.2	82.5
8	11.2	15.2	19.6	24.5	28.9	33.9
9	15.5	21.1	27.1	33.8	40.0	46.9
10	17.2	23.4	30.1	37.5	44.4	52.1
11	17.5	23.8	30.6	38.2	45.2	53.0
12	60.6	81.2	102.4	126.2	147.7	171.7
A1	4.6	6.3	8.1	10.1	11.9	13.9
P-IN	62.5	84.8	108.0	134.5	158.2	184.7
P-OUT	1.0	2.6	7.8	18.2	27.2	36.4



POND SUMMARY
 TRIBUTARY AREA = 54.90 ACRES (65.40% IMPERVIOUS)
 WQV/ELEVATION = 1,170 A-FT/5685.95
 100-YR VOL/ELEVATION = 6,117 A-FT/5689.98
 OUTLET MER ELEVATION = 5690.00
 EMERGENCY SPILLWAY WIDTH/EL = 60'/5691.00
 SPILLWAY FLOW DEPTH = 0.98'
 FOREBAY REQUIRED = 0.035 A-FT
 FOREBAY DEPTH/AREA = 2.5'/610 SF MIN



Label inlet size at DP-A1

Provide berm or swale along the lot line to convey runoff into the area inlets.

Provide sizing calcs for the pipe and area inlets.

Label or show the limits of bank protection

Provide the riprap sizing calculation for the spillway.

If this is the edge of the perennial baseflow then extend the hydraulic analysis of the outflow discharge to this location for the minor flow. Additional improvements may be needed.

Indicate the emergency overflow path for the Commercial sub-basin and update to discuss in the narrative.

Provide greater detail. Unclear how offsite flows on main street (Commercial) does not continue into the residential based on the proposed contours and inlet locations shown.

Install a crossspan.

Basin B13 and B14 needs to be split for these two inlets.

REV.	DESCRIPTION	DATE

PREPARED FOR:
AVATAR EQUITIES
 6800 JERICHO TURNPIKE
 SUITE 120W, #204
 SYOSSET, NY 11791



RIVERBEND CROSSING
PROPOSED DRAINAGE PLAN

DESIGNED BY: DLM	DRAWN BY: DBM
SCALE: 1" = 100'	DATE: 09/12/19
JOB NUMBER: 17-115	SHEET: 1 OF X