Final Drainage Report
for
Rolling Hills Ranch Standalone Grading
at
Meridian Ranch

EL PASO COUNTY, COLORADO

March 2020

Prepared For:

GTL DEVELOPMENT, INC.
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Prepared By:
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EGP203

PCD Project No. PUDSP-19-009

CERTIFICATIONS

Design Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

______________________________________ ________________

Thomas A. Kerby, P.E. #31429 Date

Owner/Developer's Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

______________________________________ _______________

Raul Guzman, Vice President Date
GTL Development, Inc.
P.O. Box 80036
San Diego, CA 92138

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 & 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

______________________________________ ____________

Jennifer Irvine, P.E. Date
County Engineer / ECM Administrator

Rolling Hills Ranch at Meridian Ranch PUD
Final Drainage Report

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EXECUTIVE SUMMARY

The purpose of the following Final Drainage Report (FDR) is to present the changes to the drainage patterns as a result the Rolling Hills Ranch Standalone Grading (Rolling Hills Ranch Grading) development. Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM) (1994 version) and portions of the City of Colorado Springs Drainage Criteria Manual, Volume 1 (DCM-1) ((2014 version).

This report based on the current version of the Meridian Ranch Sketch Plan amendment as adopted by the El Paso County Board of Commissioners on March 13, 2018. Hydrologic calculations follow method outlined in Chapter 6 of the 2014 version of the City of Colorado Springs Drainage Criteria Manual (COSDCM) as adopted by the El Paso County Board of County Commissioners by Resolution 15-042. Chapter 6 addresses the hydrologic calculation methods and includes an updated hydrograph to be used with storm drainage runoff. The Board adopted by the same resolution, Section 3.2.1 of Chapter 13 of the COSDCM referencing Full Spectrum Detention; the concept "provides better control of the full range of runoff rates that pass-through detention facilities than the convention multi-stage concept. This section of the COSDCM identifies the necessity to provide full spectrum detention but does not prescribe a methodology to reach such the detention requirements. This report includes hydrologic models from HEC-HMS for the historic, interim and future conditions for the 2-yr, 5-yr, 10-yr, 25-yr, 50-yr, and 100-yr design storm frequencies. The interim and the future conditions include detention facilities sized and modeled such that *"frequent and infrequent inflows are released at rates approximating undeveloped conditions"*

Rolling Hills Ranch Grading encompasses 252± acres and is located in Sections 20 and 29, Township 12 South, Range 64 West of the 6^{th} Principal Meridian. It is approximately 12 miles northeast of the city of Colorado Springs, 2.5 miles north of the unincorporated town of Falcon, and immediately north of the Woodmen Hills development.

Rolling Hills Ranch is located within Gieck Ranch Drainage Basin. The Gieck Ranch Basin has been studied but has not received final approval from El Paso County. The developer has agreed to meet the requirements of the studied Gieck Ranch Basin but as yet to be approved Drainage Basin Study.

Based on the design parameters the development of the project will not adversely affect downstream properties.

INTRODUCTION

Purpose

The purpose of the following Final Drainage Report (FDR) is to present proposed changes to the drainage patterns as a result of the development of Rolling Hills Ranch Grading. The report outlines the proposed drainage mitigation based on calculated developed flows in excess of allowable exiting runoff discharge.

Scope

The scope of this report includes:

- Location and description of the proposed development stating the proposed land use, density, acreage and adjacent features to the site.
- Calculations for design peak flows from all off-site tributary drainage areas.
- Calculations for design peak flows within the proposed project area for all drainage areas.
- Discussion of major drainage facilities required as a result of the development.
- Discussion and analysis of existing and proposed facilities.

Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM) (1994 version) and those portions of the City of Colorado Springs Drainage Criteria Manual, Volume 1 (DCM-1) ((2014 version) adopted by Resolution 15-042 of the El Paso County Board of County Commissioners.

Background

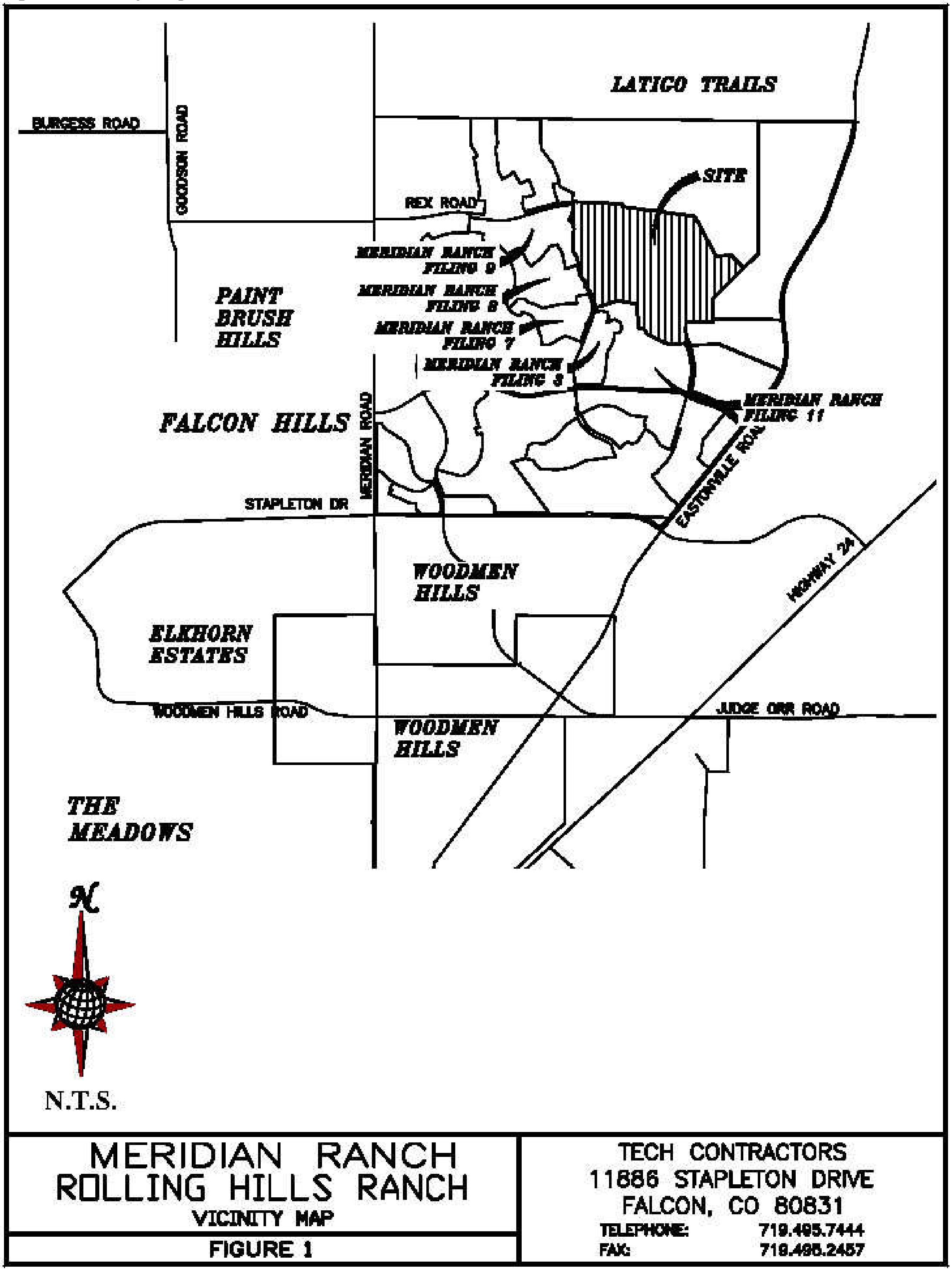
On November 16, 2000 the El Paso County Board of County Commissioners approved the rezoning of the Meridian Ranch project (PUD-00-010) from A-35 to PUD with several conditions. Condition number seven stated in part that "drainage plans shall release and/or retain at approximately eight percent (80%) of historic rates." At the time of the initial approvals there were no drainage improvements downstream of the Meridian Ranch project and the existing natural channels were shallow and undefined.

The Sketch Plan Amendment (SKP-17-001) was processed and approved in 2018 by the El Paso County Board of County Commissioners by resolution 18-104 for Meridian Ranch. The resolution eliminated the required restriction of 80% of historic peak flow rates mentioned above. The detention pond proposed with this project will release at historic or less peak flow rates as per the current El Paso County stormwater requirements.

No development has occurred downstream of this project except for portions of the Falcon Regional Park providing ballparks and associated parking. The Meridian Ranch MDDP and this report indicate the Eastonville Road culvert crossing located downstream of this project does not provide enough capacity for the historic flow rates. It is anticipated that this culvert will be upgraded at the time of the Eastonville Road construction.

Current calculations show the future design discharge of the proposed Pond G to the Falcon Regional Park to be below historic flow rates at full buildout for the full spectrum of design storms.

Figure 1: Vicinity Map



EXISTING CONDITIONS

General Location

Rolling Hills Ranch Grading project encompasses 252± acres and is located in Sections 20 and 29, Township 12 South, Range 64 West of the 6th Principal Meridian. It is approximately 12 miles northeast of the city of Colorado Springs, 2.5 miles north of the unincorporated town of Falcon, and immediately north of the Woodmen Hills development.

Land Use

Historically, ranching dominated the area surrounding Meridian Ranch; however, currently urbanization has occurred in the general vicinity. Most notably, urbanization is occurring to the north with Latigo Trails, to the south in the Woodmen Hills Subdivision, to the east in Four Way Ranch, to the west in the Falcon Hills subdivision, and to the northwest in the Paint Brush Hills subdivision.

Climate

Mild summers and winter, light precipitation; high evaporation and moderately high wind velocities characterize the climate of the study area. The average annual monthly temperature is 48.4 F with an average monthly low of 30.3 F in the winter and an average monthly high of 68.1 F in the summer. Two years in ten will have maximum temperature higher than 98 F and a minimum temperature lower than –16 F. Precipitation averages 15.73" annually, with 80% of this occurring during the months of April through September. The average annual Class A pan evaporation is 45 inches. (Soil Survey of El Paso County Area, Colorado).

Topography and Floodplains

The topography of the site is typical of a high desert, short prairie grass with relatively flat slopes generally ranging from 2% to 4%. The project site drains generally from the northwest to southeast and is tributary to the Black Squirrel Creek.

The Flood Insurance Rate Maps (FIRM No. 08041C0552G, dated 12/07/2018) indicates there is a portion of the project located within a designated floodplain. Please see Figure 2: Rolling Hills Ranch Grading Federal Emergency Management Agency (FEMA) Floodplain Map.

The designated floodplain is located within a drainage open space identified as Tract C. The floodplain is identified as a Zone AE with an elevation of 7060 based on the NAVD88 Datum. The topography is based on the NGVD29 Datum, therefore an adjustment of 3.9-ft. to the base flood elevation shown in the map is required. The net result is a base flood elevation of 7056 for this location.

Geology

The National Resources Conservation Service (NRCS) soil survey records indicate that the service area is predominately covered by soils classified in the Columbine (65 ac.) and Stapleton series (188 ac.). These series are categorized in the Hydrological Soil Groups A & B.

The Columbine (19) gravelly sandy loam is a deep, well-drained to excessively drained soil formed in coarse textured material on alluvial terraces, fans and flood plains. Permeability of

Figure 2: FEMA Floodplain Map

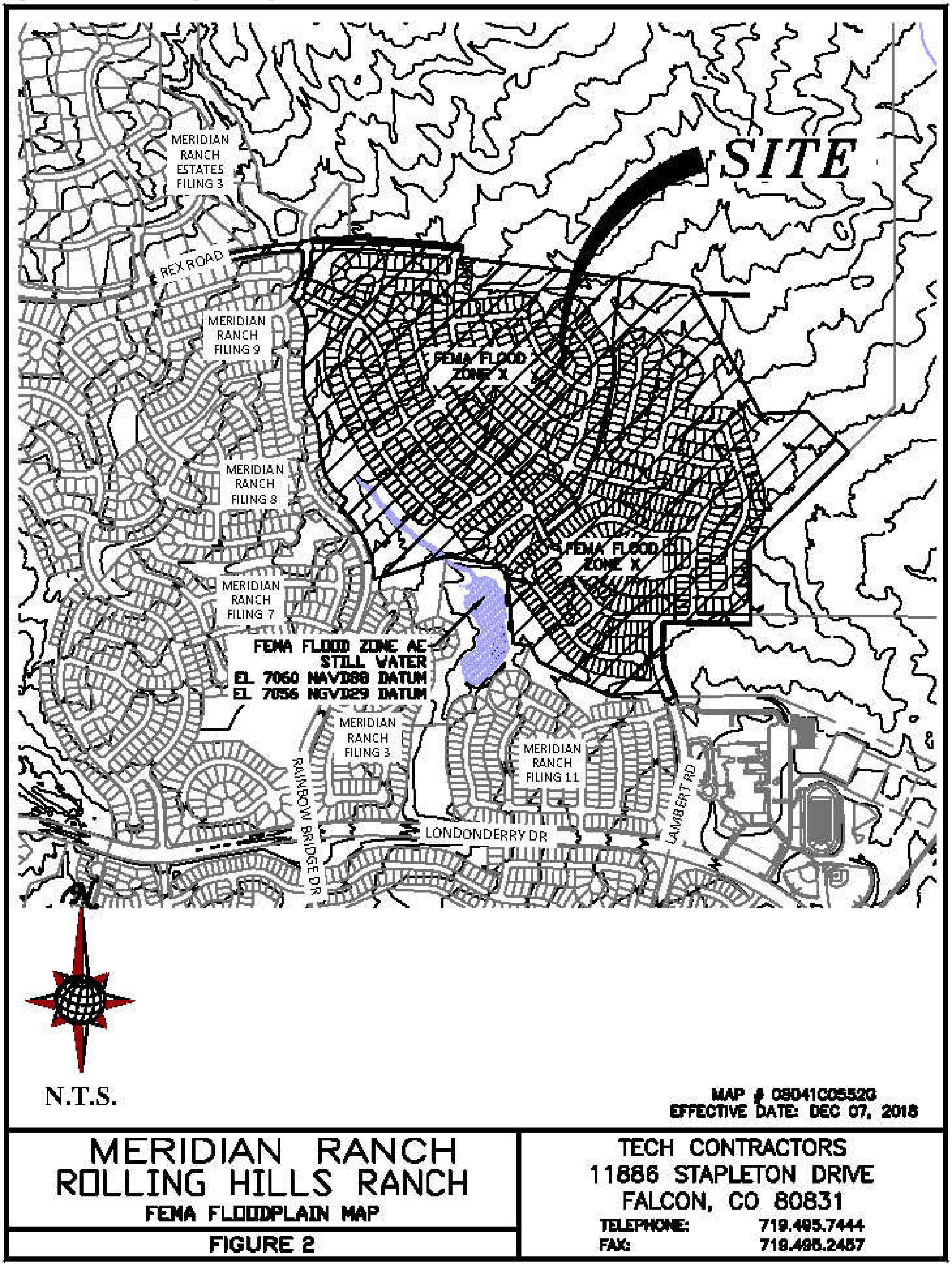
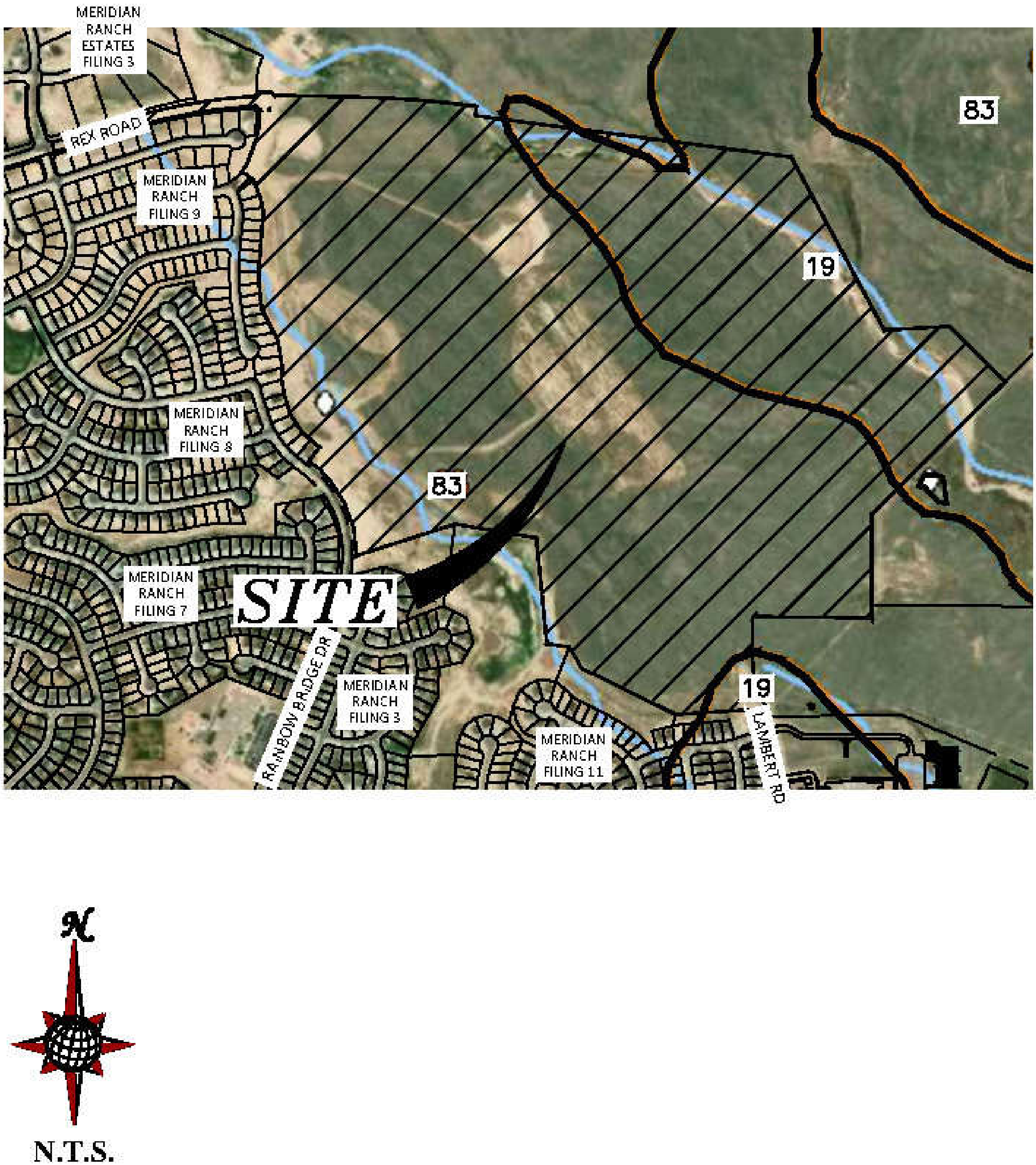


Figure 3: Soils Map



MERIDIAN RANCH ROLLING HILLS RANCH SOILS MAP	TECH CONTRACTORS 11886 STAPLETON DRIVE FALCON, CO 80831 TELEPHONE: 719.495.7444 FAX: 719.495.2457
FIGURE 3	

this soil is very rapid. Available water capacity is low to moderate, surface runoff is slow, and the hazard of erosion is slight to moderate. The Columbine series is categorized as a Hydrological Soil Group A.

This soil is used mainly for grazing livestock, for wildlife habitat and for home sites. The main limitation of this soil for urban development is a hazard of flooding in some areas.

The Stapleton (83) sandy loam is a deep, non-calcareous, well-drained soil formed in alluvium derived from arkosic bedrock on uplands. Permeability of this soil is rapid. Available water capacity is moderate, surface runoff is slow, and the hazard of erosion and soil blowing is moderate. The Stapleton series is categorized as a Hydrological Soil Group B.

This soil is suited to habitat for open land and rangeland wildlife. The main limitation of this soil for urban development is frost-action potential.

Typically, these soils are well-drained, gravelly sandy loams that form on alluvial terraces and fans and exhibit high permeability and low available water capacity with depth to bedrock greater than 6 feet.

Note: (#) indicates Soil Conservation Survey soil classification number. See Figure 3 Rolling Hills Ranch Grading – Soils Map.

Natural Hazards Analysis

Natural hazards analysis indicates that no unusual surface or subsurface hazards are located near the vicinity. However, because the soils are cohesionless, sloughing of steep banks during drilling and/or excavation could occur. By citing improvements in a manner that provides an opportunity to lay the banks of excavations back at a 1:1 slope during construction, the problems associated with sloughing soils can be minimized.

<u>DRAINAGE BASINS AND SUB-BASINS</u>

The site is near the top of the Gieck Ranch Drainage Basin and accepts flow from areas north of the project site within portions of Meridian Ranch.

Three different scenarios were analyzed for the drainage conditions for the project.

The first scenario analyzes the historic conditions for Meridian Ranch. This condition has all of Meridian Ranch in the pre-development state; where the entirety of Meridian Ranch is modeled in its undeveloped, undisturbed condition, alternatively called the historic condition.

The second scenario is the interim conditions scenario and it consists of the current existing conditions for all tributary areas whether developed or undeveloped/historic with the addition of Rolling Hills Ranch Grading in the proposed developed condition. The current existing conditions assume all approved projects tributary to Rolling Hills Ranch and the Rolling Hills Ranch Grading are at full buildout. This condition was analyzed to ensure the full spectrum of historic flow rates exiting the Meridian Ranch development are maintained after the development of Rolling Hills Ranch Grading is completed.

The interim scenario was analyzed to ensure that the historic flow rates at the outlets of the proposed Pond G (Design Point G12) located upstream of and adjacent to the Falcon Regional Park and Pond E (Design Points H08 & H09) located along Eastonville Road were maintained. The development of Rolling Hills Ranch will complete the development of the areas tributary to Ponds D & E.

The final scenario analyzes the future build out conditions for the entirety of Meridian Ranch to ensure the storm drain facilities located at the discharge points of the project are able to properly convey the full spectrum of historic peak flow rates as the storm drainage exits the Meridian Ranch project along Eastonville Road and/or the Falcon Regional Park.

DRAINAGE DESIGN CRITERIA

SCS Hydrograph Procedure

The US Army Corp of Engineers HEC-HMS computer program was used to model the Soil Conservation Service (SCS) Hydrograph procedure to determine final design parameters for the major drainage facilities within the project. Onsite basin areas were calculated using aerial topography of the site and approved final design data. Times of concentration were estimated using the SCS procedures described in the DCM. Based upon the hydrologic soil type, the natural conditions found in the basins and the runoff curve numbers (CN) chart from Table 6-10 of the City of Colorado Springs DCM for Antecedent Runoff Condition II (ARC II), the following CN values were used for the given conditions.

Table 1: SCS Runoff Curve Numbers

Condition	CN*
Residential Lots (5 acre)	63
Residential Lots (2.5 acre)	66
Residential Lots (1 acre)	68
Residential Lots (1/2 acre)	70
Residential Lots (1/3 acre)	72
Residential Lots (1/4 acre)	75
Residential Lots (1/5 acre)	78
Residential Lots (1/6 acre)	80

School	80
Parks/Open Space	62
Commercial	85
Roadways	98
Graded	67
Golf Course	62
Latigo Undeveloped	65
Undeveloped	61

*Curve Numbers were interpolated and based on amount of impervious area per lot. The 24 hour storm precipitation values were selected from the NOAA Atlas 14, Volume 8, Version 2 for the Meridian Ranch location (Latitude 38.9783°, Longitude -104.5842°, Elevation 7054 ft). These numbers along with SCS information were used as input to the U.S. Army Corp of Engineers HEC-HMS computer model to determine design runoffs. See the table for all the design storm events in Appendix A. These numbers along with SCS information were used as input to the U.S. Army Corp of Engineers HEC-HMS computer model to determine design runoffs.

Full Spectrum Design

The City of Colorado Springs adopted a new Drainage Criteria Manual (DCM) in 2014 which incorporated the use of *Full Spectrum Design* for storm drainage analysis for projects located within the city limits. El Paso County adopted portions of the City's 2014 DCM by resolution in January 2015; the County resolution adopted Chapter 6 (Hydrology) and Section 3.2.1 of Chapter 13 (Full Spectrum Detention) for projects outside of the City of

Colorado Springs establishing a 1 year review period to analyze the impacts of the Full Spectrum Design on the storm drainage analysis of projects. This report has incorporated the use of full spectrum in the analysis using the SCS Method to determine the size requirements for the detention pond during the interim and future conditions.

The idea behind full spectrum detention is to release the developed runoff flow rates that will approximate those of the pre-developed condition. The design of Pond G and the outlet control structure meets or exceeds the intent and spirit of the concept.

Table 2: Detention Pond Summary:

POND F				
	PEAK INFLOW	PEAK OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	FT
INTERIM CONDITIONS				
2-YEAR STORM	4.6	2.2	0.9	7130.1
5-YEAR STORM	21	7.9	1.8	7131.2
10-YEAR STORM	53	17	3.4	7132.7
25-YEAR STORM	120	61	5.3	7134.1
50-YEAR STORM	193	122	6.7	7134.9
100-YEAR STORM	285	178	8.8	7136.0
FUTURE CONDITIONS				
2-YEAR STORM	5.0	2.4	1.0	7130.2
5-YEAR STORM	22	8.5	1.9	7131.3
10-YEAR STORM	54	17	3.5	7132.8
25-YEAR STORM	123	64	5.4	7134.2
50-YEAR STORM	200	125	6.8	7135.0
100-YEAR STORM	293	179	8.9	7136.0

DRAINAGE CALCULATIONS

SCS General Overview

The project is located within the Gieck Ranch Drainage Basin; storm water runoff will be conveyed across the site overland and within existing and proposed storm drain networks to existing and proposed detention ponds. Those portions of the site tributary the existing Detention Pond D will be directed to an existing sedimentation pond to be located upstream of the pond then conveyed to the pond. Portions of the site are tributary the existing Detention Pond E; runoff will be directed to an existing sedimentation pond to be located upstream at the existing northern terminus of Lambert Road, collected then conveyed via an existing storm drain system to the pond. Portions of the site tributary to the proposed Detention Pond G will be directed to temporary sedimentation pond before being released into the existing natural channel and conveyed to the proposed pond. Additionally, the proposed detention Pond G will be utilized as a combination sedimentation/detention pond until such time as the tributary areas establish sufficient ground cover or development in the area is complete.

The detention facilities have been adequately sized such that the developed flows detained and released will approximate the historic flow rates for the various design storm events as outlined in the El Paso County DCM and those sections of the City of Colorado Springs DCM-1 adopted by the El Paso County Board of County Commissioners. Existing facilities located downstream of the proposed development have been designed and/or constructed to accept the given release flow rates from Meridian Ranch. Those existing facilities have been reviewed sufficiently to verify the capacity to convey the storm flow rates from Meridian Ranch. See approved Meridian Ranch MDDP, dated January 2018.

Rear lots adjacent to the Falcon Regional Park will drain into open space and then will be directed via a shallow swale to the proposed detention pond prior to exiting the site.

The analysis shows the portion of the site tributary to existing Pond E releasing the developed peak flows below the historic flow rates for the full spectrum of design storms using the newly adopted unit hydrograph from the City DCM-1.

Figure 5: Meridian Ranch SCS Calculations – Historic Conditions Map, Figure 6: Meridian Ranch SCS Calculations – Interim Conditions Map and Figure 7: Meridian Ranch SCS Calculations – Future Conditions Map depict the historic, interim and future general drainage patterns for Rolling Hills Ranch Grading.

The purpose of this report is to show that the development of Rolling Hills Ranch Grading will not adversely impact the existing drainage facilities adjacent to and downstream of the developed area and the existing Ponds D & E are properly sized for the anticipated future development of Rolling Hills Ranch.

SCS Calculations

Historic Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics under Existing Conditions using the SCS calculation method. Please refer to Figure 5 - Meridian Ranch SCS Calculations - Historic Basin Map.

Table 3: Historic Drainage Basins – SCS

HISTORIC MDDP (Full Spectrum)							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	81	53	31	12	3.9	0.5
OS06-G02	0.1313	79	52	31	12	3.8	0.5
OS05	0.0578	40	26	16	5.9	1.8	0.2
OS05-G01	0.0578	38	26	16	5.7	1.8	0.2
HG01	0.0547	33	21	13	4.8	1.6	0.2
G01	0.1125	71	47	28	10	3.3	0.5
G01-G02	0.1125	70	47	27	10	3.3	0.5
HG02	0.0906	46	30	18	6.9	2.4	0.4
G02	0.3344	194	129	76	28	9.4	1.4
G02-G03	0.3344	192	127	75	28	9.3	1.4
HG03	0.1828	79	51	31	12	4.4	0.8
OS07	0.0328	25	17	11	4.6	1.7	0.3
OS07-G03	0.0328	24	17	9.9	4.4	1.7	0.3

HISTORIC MDDP (Full Spectrum)							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
G03	0.55	295	195	115	44	15	2.4
G03-G04	0.55	286	192	113	43	15	2.4
OS09	0.1547	92	64	41	19	8.5	2.0
OS09-G04	0.1547	91	63	41	19	8.5	2.0
HG04	0.0891	40	27	16	6.1	2.2	0.4
HG05	0.1125	50	33	19	7.6	2.7	0.5
OS08	0.0406	36	25	17	7.9	3.5	0.8
OS08-G04	0.0406	34	24	15	7.6	3.5	0.8
G04	0.9469	502	336	200	78	28	4.9
G04-G05	0.9469	496	322	193	78	28	4.9
HG06A	0.1375	50	33	20	7.8	2.9	0.5
G05	1.0844	544	355	212	86	31	5.4
G05-G06	1.0844	530	353	211	86	31	5.4
HG06B	0.1031	34	22	13	5.4	2.1	0.4
G06	1.1875	561	375	225	91	33	5.8
HG07	0.0984	47	31	18	7.1	2.4	0.4
HG07-G11	0.0984	47	31	18	7.0	2.4	0.4
HG08	0.1328	73	48	28	11	3.6	0.5
G11	0.2312	115	75	44	17	5.7	0.9
G11-G12	0.2312	114	75	44	17	5.6	0.9
HG09	0.1781	73	48	29	11	4.1	0.7
G12	0.4093	187	122	72	28	9.7	1.6
G12-H08	0.4093	183	121	71	28	9.7	1.6
HG10	0.1375	39	26	16	6.5	2.6	0.5
H08	0.5468	216	142	85	34	12	2.1
HG14	0.2297	81	53	32	13	4.8	0.9
HG13	0.0844	55	37	23	9.8	3.9	0.7
G07	0.0844	55	37	23	9.8	3.9	0.7
G07-G08	0.0844	54	37	23	9.7	3.8	0.7
G08	0.3141	119	78	48	20	7.6	1.5
HG15	0.2563	70	46	28	12	4.7	0.9
H13	0.2563	70	46	28	12	4.7	0.9
HG11	0.2047	77	51	30	12	4.5	0.8
H09	0.2047	77	51	30	12	4.5	0.8
HG12	0.1297	57	38	22	8.7	3.1	0.5
H10	0.1297	57	38	22	8.7	3.1	0.5

See approved Meridian Ranch MDDP (EPC File SKP171) dated January 2018 for complete hydrologic calculations and maps.

Interim Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics for the interim conditions using the SCS calculation method. Please refer to Figure 5 - Meridian Ranch SCS Calculations – Interim Basins Map

Table 4: Interim Drainage Basins-SCS

INTERIM MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	80	52	30	12	3.8	0.5
G1a	0.1313	80	52	30	12	3.8	0.5
G1a-G2	0.1313	79	52	30	11	3.6	0.5
OS05	0.0578	39	26	15	5.6	1.8	0.2
OS05-G1	0.0578	39	25	15	5.5	1.7	0.2
FG01	0.0531	31	22	14	6.9	3.3	0.9
FG01-G1	0.0531	31	22	14	6.9	3.3	0.9
G1	0.1109	61	41	25	11	4.8	1.1
G1-G2	0.1109	60	41	25	11	4.8	1.1
FG02	0.0391	32	22	14	6.2	2.6	0.5

INTERIM MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
G2	0.2813	166	112	67	27	10	1.9
G2-G3	0.2813	163	108	66	27	10	1.9
FG03	0.0203	24	17	12	5.9	0.8	0.8
FG04	0.0172	22	16	11	5.8	3.1	0.9
G3	0.3188	184	123	74	31	11	2.4
G3-POND F	0.3188	183	121	74	31	11	2.4
FG06	0.0658	46	32	20	9.1	3.9	0.8
OS07a-POND F	0.0170	13	9.0	5.7	2.4	0.9	0.1
POND F IN	0.4596	285	193	120	53	21	4.6
POND F	0.4596	178	122	61	17	7.9	2.2
POND F-G7	0.4596	177	121	61	17	7.9	2.2
FG22	0.0658	51	36	24	12	5.6	1.4
FG23a	0.0177	18	13	9.0	4.9	2.7	1.0
OS07b	0.0156	15	10.0	6.2	2.6	1.0	0.1
OS07b-G7	0.0156	13	9.2	5.4	2.3	0.9	0.1
G7	0.5587	215	145	73	22	10	2.8
G7-G10	0.5587	215	145	72	21	10	2.8
FG24	0.2503	110	73	43	17	6.2	1.1
OS09	0.1527	90	62	39	18	8.2	1.9
OS09-G10	0.1527	88	61	39	18	8.2	1.9
OS08	0.0397	35	24	16	7.5	3.4	0.7
OS08-G8	0.0397	34	24	15	7.2	3.3	0.7
G8	0.4427	227	154	94	41	17	3.4
G8-G10	0.4427	225	151	94	41	17	3.4
FG23b	0.0359	21	14	8.4	3.3	1.2	0.2
G10	1.0373	444	287	146	60	25	5.6
G10-G11	1.0373	442	285	146	60	25	5.6
FG23c	0.0070	5.8	4.2	2.8	1.4	0.7	0.2
G11	1.0443	446	287	147	60	25	5.7
FG25	0.1086	85	64	46	27	17	7.5
FG28	0.0673	38	26	16	6.7	2.7	0.5
POND G IN	1.2202	558	363	202	91	42	11
POND G	1.2202	394	238	118	34	13	4.1
G12	1.2202	394	238	118	34	13	4.1
G12-G06	1.2202	394	237	118	34	13	4.1
FG29	0.0997	60	39	23	8.7	2.8	0.4
FG32	0.0402	29	19	11	4.2	1.3	0.2
FG32-G06	0.0402	28	19	11	4.1	1.3	0.2
G06	1.3601	414	249	125	37	14	4.5
FG10A	0.0806	81	61	43	25	15	6.5
FG08A	0.0750	116	90	66	41	27	13
FG08A-G05	0.0750	110	86	64	41	27	13
FG08B	0.0630	86	67	49	31	20	10
FG08B-G05	0.0630	84	65	48	29	19	10
FG11	0.0625	75	59	44	28	19	9.8
FG09	0.0484	48	36	25	14	8.3	3.2
FG09-G05	0.0484	48	36	25	14	8.0	3.2
FG10B	0.0416	42	31	22	12	7.0	2.7
G05	0.3711	433	330	239	145	93	45
FG13	0.0534	34	24	15	7.5	3.6	0.9
FG12	0.0328	50	40	30	20	14	7.8
POND D IN	0.4573	509	387	280	168	107	52
POND D	0.4573	136	92	52	18	11	4.0
POND D-G17	0.4573	135	92	52	18	11	4.0
FG15	0.0103	15	12	9.0	5.8	3.9	2.1
FG15-G17A	0.0103	15	12	9.0	5.8	3.9	2.1
G17A	0.4676	138	94	53	19	12	4.1
FG14	0.1000	98	74	53	32	20	9.2
G17	0.5676	198	134	76	42	25	12
G17-G18	0.5676	197	133	75	42	25	12
FG16	0.0791	133	104	78	50	34	18
G18	0.6467	242	179	129	78	51	25
G18-POND E	0.6467	242	177	127	77	50	25
FG31	0.0922	116	92	69	45	31	17
FG30	0.0389	30	20	12	4.3	1.3	0.2
FG30-PONDHS	0.0389	28	19	11	4.2	1.2	0.2

INTERIM MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
POND HS	0.1311	112	63	40	28	19	10.0
FG17a	0.0694	101	78	57	35	23	12
FG17a-POND E	0.0694	99	76	56	35	23	11.6
FG18	0.0644	56	42	30	18	11	4.7
FG18-POND E	0.0644	56	42	30	17	11	4.6
FG19	0.0527	84	66	50	33	23	13.1
FG17c	0.0313	31	22	14	6.5	2.9	0.5
FG17b	0.0214	39	31	24	16	11	6.1
POND E IN	1.0170	553	424	308	190	122	62.8
POND E	1.0170	237	148	77	27	14	5.8
H08	1.0170	202	133	69	21	10	3.5
H09	0.0000	35	15	8.0	5.6	3.8	2
FG34	0.0836	48	32	19	7.0	2.3	0.3
G14	0.0836	48	32	19	7.0	2.3	0.3
G14-G15	0.0836	48	32	18	7.0	2.3	0.3
FG35	0.0586	19	13	7.4	3.0	1.2	0.2
G15	0.1422	60	39	23	8.9	3.1	0.6
G15-G08	0.1422	59	39	23	8.8	3.1	0.6
FG37	0.1203	44	29	17	6.8	2.5	0.5
FG36	0.0281	16	10	6.0	2.4	0.9	0.1
FG36-G08	0.0281	15	10	6.0	2.4	0.9	0.1
G08	0.2906	114	75	44	18	6.4	1.1

See approved Meridian Ranch MDDP (EPC File SKP171) dated January 2018 for complete hydrologic calculations and maps.

Future Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics for the future conditions using the SCS calculation method. Please refer to Figure 6 - Meridian Ranch SCS Calculations – Future Basins Map

Table 5: Future Drainage Basins-SCS

FUTURE MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	80	52	30	12	3.8	0.5
G1a	0.1313	80	52	30	12	3.8	0.5
G1a-G2	0.1313	79	52	30	11	3.6	0.5
OS05	0.0578	39	26	15	5.6	1.8	0.2
OS05-G1	0.0578	39	25	15	5.5	1.7	0.2
FG01	0.0538	31	22	14	7.0	3.4	0.9
FG01-G1	0.0538	31	22	14	7.0	3.4	0.9
G1	0.1116	61	41	25	11	4.9	1.1
G1-G2	0.1116	61	41	25	11	4.8	1.1
FG02	0.0391	32	22	14	6.4	2.7	0.5
G2	0.2820	167	112	67	27	10	1.9
G2-G3	0.2820	163	109	66	27	10	1.9
FG03	0.0203	24	17	12	5.9	0.8	0.8
FG04	0.0172	22	16	11	5.8	3.1	0.9
G3	0.3195	185	123	74	31	11	2.4
G3-POND F	0.3195	183	121	74	31	11	2.4
FG06	0.0675	56	40	26	12	5.8	1.3
FG05	0.0580	45	33	23	12	6.7	2.4
OS07a	0.0170	14	9.2	5.7	2.5	0.9	0.1
OS07a-POND F	0.0170	13	9.0	5.7	2.4	0.9	0.1
POND F IN	0.4620	293	200	123	54	22	5.0
POND F	0.4620	179	125	64	17	8.5	2.4
POND F-G7	0.4620	179	124	63	17	8.5	2.4

FUTURE MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
FG21b	0.0170	26	20	16	10.2	7.0	4.0
FG21a	0.0072	6.1	4.1	2.4	0.9	0.3	0.0
FG21a-G7	0.0072	5.8	3.4	2.2	0.8	0.3	0.0
G7	0.4862	188	130	67	18	9.2	4.0
G7-G8	0.4862	188	130	67	18	9.2	3.9
FG22	0.1380	102	73	47	24	12	3.3
OS08	0.0406	35	25	16	7.7	3.4	0.7
OS08-G8	0.0406	34	24	15	7.5	3.4	0.7
FG23a	0.0216	21	15	10	5.2	2.7	0.8
OS07b	0.0156	15	10	6.2	2.6	1.0	0.1
OS07b-G7	0.0156	14	9.7	6.0	2.4	0.9	0.1
G8	0.7020	297	192	97	48	25	7.9
G8-G10	0.7020	294	191	96	47	24	7.9
OS09	0.1527	90	62	39	18	8.2	1.9
OS09-G10	0.1527	88	62	39	18	8.2	1.9
FG24	0.1373	105	76	50	26	13	4.0
G9	0.2900	180	125	81	38	17	4.4
G9-G10	0.2900	178	125	79	37	17	4.4
FG23b	0.0286	23	16	10	4.6	2.0	0.4
G10	1.0206	484	313	176	80	39	12
G10-G11	1.0206	480	311	174	80	39	12
FG23c	0.0122	12	8.7	5.7	3.0	1.5	0.4
G11	1.0328	485	314	177	81	40	12
FG25	0.1086	85	64	46	27	17	7.5
FG26	0.0863	78	58	40	22	12	4.6
FG26-POND G	0.0863	77	57	39	22	12	4.5
FG27	0.0500	52	40	29	17	11	5.0
FG28	0.0245	18	13	8.5	4.1	2.0	0.5
POND G IN	1.3022	691	459	288	146	79	28
POND G	1.3022	478	331	173	56	22	5.3
G12	1.3022	478	331	173	56	22	5.3
G12-G06	1.3022	477	329	173	56	22	5.3
FG29	0.0997	60	39	23	8.7	2.8	0.4
FG32	0.0402	72	57	44	29	20	11
FG32-G06	0.0402	69	54	41	27	18	11
G06	1.4421	505	348	184	61	24	11
FG08A	0.0750	116	90	66	41	27	13.4
FG08A-G05	0.0750	110	86	64	41	27	13
FG08B	0.0630	86	67	49	31	20	10
FG08B-G05	0.0630	84	65	48	29	19	10
FG09	0.0484	48	36	25	14	8	3
FG09-G05	0.0484	48	36	25	14	8	3.2
FG10B	0.0416	42	31	22	12	7.0	2.7
G05	0.2280	282	215	156	94	58.8	28.7
FG10A	0.0806	81	61	43	25	15.0	6.5
FG11	0.0625	75	59	44	28	19	10
FG13	0.0534	34	24	15	7.5	3.6	0.9
FG12	0.0328	50	40	30	20	14	7.8
POND D IN	0.4573	509	387	280	168	107	52
POND D	0.4573	134	91	50	19	12	4.3
POND D-G17	0.4573	134	91	50	19	12	4.3
FG15	0.0103	15	12	9.0	5.8	3.9	2.1
FG15-G17A	0.0103	15	12	9.0	5.8	3.9	2.1
G17A	0.4676	137	93	51	19	12	4.4
FG14	0.1000	98	74	53	32	20	9.2
G17	0.5676	196	132	75	43	25	12
G17-G18	0.5676	196	131	75	43	25	12
FG16	0.0791	133	104	78	50	34	18
G18	0.6467	240	178	128	79	51	26
G18-POND E	0.6467	240	176	126	78	50	25
FG31	0.0922	116	92	69	45	31	17
FG30	0.0389	73	57	44	29	20	11
FG30-PONDHS	0.0389	70	56	42	27	18	11

FUTURE MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
POND HS	0.1311	153	106	53	36	26	15
FG17a	0.0694	101	78	57	35	23	12
FG17a-POND E	0.0694	99	76	56	35	23	12
FG18	0.0644	56	42	30	18	11	4.7
FG18-POND E	0.0644	56	42	30	17	11	4.6
FG19	0.0527	84	66	50	33	23	13
FG17c	0.0313	31	22	14	6.5	2.9	0.5
FG17b	0.0214	39	31	24	16	11	6.1
POND E IN	1.0170	610	432	318	197	126	64
POND E	1.0170	242	153	80	30	16	6.6
H08	1.0170	205	137	72	24	12	4.1
H09	0.0000	37	16	8.3	5.9	4.1	2.4
FG34	0.0600	34	23	13	5.5	2.0	0.3
G14	0.0600	34	23	13	5.5	2.0	0.3
G14-G15	0.0600	34	22	13	5.4	2.0	0.3
FG35	0.0344	20	13	8.3	3.5	1.5	0.3
G15	0.0944	53	36	21	8.7	3.3	0.6
G15-G08	0.0944	52	35	21	8.7	3.3	0.6
FG37	0.0797	41	27	16	6.0	2.0	0.3
FG36	0.0281	14	9.4	5.5	2.1	0.7	0.1
FG36-G08	0.0281	14	9.3	5.4	2.1	0.7	0.1
G08	0.2022	106	69	41	16	5.8	1.0

See approved Meridian Ranch MDDP (EPC File SKP171) dated January 2018 for complete hydrologic calculations and maps.

Rational Calculations

The Rational Hydrologic Calculation Method was used to estimate the total runoff from the 5-year and the 100-year design storm and thus establish the storm drainage system design. Using the rational calculation methodology outlined in the Hydrology Section (Ch 6) of the COSDCM coupled with the El Paso County EPCDCM an effective storm drainage design for Rolling Hills Ranch Grading has been designed. The storm drainage facilities have been designed such that the minor storm will be captured by the inlets and conveyed by the storm drain pipes such that the street flow does not overtop the curbs. The storm drainage facility has been designed such that the major storm will be captured by the inlets and conveyed by the storm drain pipes such that the street flow does not exceed the right-of-way widths for residential streets and the hydraulic grade line will be less than one foot below the surface.

The site is located within the Gieck Ranch Drainage Basin; the project will discharge the collected surface flow from the project into an existing natural drainage course or into existing downstream facilities properly sized to safely convey the storm water flows away from the project without damaging adjacent property.

Rational hydrologic calculations were performed for the entire PUD area and hydraulic calculations will be provided in the final drainage report at final plat. The storm drain runoff will be collected by a series of inlets and storm drain pipe then conveyed through the project and discharged either into an existing storm drain system located within Lambert Road discharged into the existing Pond E, directly into Pond D or into the proposed Pond G.

Not relevant information for the Early Grading drainage report. Modify to analyze conveyance via swales and sizing of temporary sediment basins during the earthmoving operation

Rational Narrative

The following is a detailed narrative of the storm drainage system located in Rolling Hills Ranch Grading. The description is organized by system beginning on the west in the Bennett Ranch portion of Rolling Hills Ranch and ending on the east side of the project in the Gieck Ranch Basin.

Offsite Storm Drain System

- Basin OS1 (4.4 acres, Q_5= 1.3 CFS, Q_{100} = 8.6 CFS) is located north the existing Meridian Service Metropolitan District Water Filtration Building on land that will remain in undeveloped. Underlying the land are major transmission watermains and the master plan for this area is to remain undeveloped in its natural condition. The area will receive no developed runoff and the surface runoff will sheet flow will collect in an existing swale and traverse the area in a southerly direction toward a proposed Type C inlet CB4. All of the runoff is captured by the inlet and conveyed toward the future rip-rap lined channel to be constructed with Rolling Hills Ranch Filing 1 located adjacent and south of the filtration building. The captured flow is conveyed downstream via a 24” RCP.

Storm Drain System A

Storm Drainage System A meets the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge into Waters of the State. The discharge point is located upstream of a Regional Detention Facility with WQCV incorporated into the design and construction. At least 20 percent of the upstream imperviousness within the catchment must be disconnected from the storm drainage system and drain though a pervious area that makes up at least 10 percent of the disconnected impervious area. The rooftops within this catchment make up more than 20 percent of the total impervious area of the catchment and is discharged via roof downspouts and drains across the front yard pervious areas equaling more than 10 percent of the rooftop area. Please see Appendix F for information and exhibits.

- Basin A01 (7.5 acres, Q_5= 8.6 CFS, Q_{100} = 22 CFS) contains lots in Rolling Hills Ranch 3 adjacent to Rex Rd and along Monument Vista Ln at the northern end of the project. The surface runoff will sheet flow off of the residential lots and directed to a 15' Type R forced sump inlet located at I01. All of the 5-year storm flow is captured by this inlet (Q_5= 8.6 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 8.6 CFS) continuing downstream to Inlet 02. The captured flow is conveyed downstream via a 24” RCP to Storm Manhole 01.

- Basin A02 (2.2 acres, Q_5= 2.0 CFS, Q_{100} = 6.5 CFS) contains lots within Rolling Hills 3 along Monument Vista Ln and Rolling Ranch Dr. The surface runoff will sheet flow off of the residential lots and be directed to the street, where it is combined with the flow from Basin A01 for a total flow of Q_5= 2.0 CFS, Q_{100} = 14 CFS then conveyed downstream to a 10' Type R sump inlet located at I02. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 01.

- Basin A03 (0.8 acres, Q_5= 1.0 CFS, Q_{100} = 3.1 CFS) contains lots in Rolling Hills 3 along the east side of Rolling Ranch Dr. The surface runoff will sheet flow off of the residential lots and be directed to the street then to a 5' Type R sump inlet located at I03. All of the flow is captured by this inlet and the combined flow (Q_5= 11 CFS, Q_{100} = 30 CFS) is conveyed downstream via a 24" RCP to a permanent sedimentation/WQCV BMP prior to releasing to the natural arroyo.

  Should the sump inlets (I02 & I03) become blocked and cannot take capture all the runoff at the sump inlets, the surface flow will travel overland within lots 595 & 596.

<u>Storm Drain System B</u>

Storm Drainage System B meets the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge into Waters of the State. The discharge point is located upstream of a Regional Detention Facility with WQCV incorporated into the design and construction. At least 20 percent of the upstream imperviousness within the catchment must be disconnected from the storm drainage system and drain though a pervious area that makes up at least 10 percent of the disconnected impervious area. The rooftops within this catchment make up more than 20 percent of the total impervious area of the catchment and is discharged via roof downspouts and drains across the front yard pervious areas equaling more than 10 percent of the rooftop area. Please see Appendix F for information and exhibits.

- Basin B01 (2.3 acres, Q_5= 2.2 CFS, Q_{100} = 6.4 CFS) contains lots in Rolling Hills Ranch 1 along east side of Rolling Peaks Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I04. All of the flow is captured by this inlet and conveyed downstream via a 18" RCP to Inlet 05.

- Basin B02 (5.6 acres, Q_5= 5.2 CFS, Q_{100} = 15 CFS) contains lots in Rolling Hills Ranch 1 along west side of Rolling Peaks Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 15' Type R forced sump inlet located at I05. All of the 5-year storm flow is captured by this inlet (Q_5= 5.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 1.8 CFS) continuing downstream to Inlet 14. The captured flow is conveyed downstream via a 24" RCP to Storm Manhole 02 then to Storm Manhole 03.

- The total pipe flow conveyed to Storm Manhole 03 is Q_5= 7.3 CFS, Q_{100} = 20 CFS.

- Basin B03 (4.3 acres, Q_5= 4.2 CFS, Q_{100} = 12 CFS) contains lots along Rolling Mesa Dr, Evening Creek Dr and Monument Vista Ln in Rolling Hills Ranch 1 and 3. The surface runoff will sheet flow off of the residential lots and be conveyed Design Point 1 (DP01) at the intersection of Rolling Mesa Dr and Evening Creek Dr. The crosses the intersection via a crosspan then continues along Rolling Mesa Dr through Basin B04 to inlet I06.

- Basin B04 (3.0 acres, Q_5= 2.9 CFS, Q_{100} = 8.5 CFS) contains lots along the east side of Rolling Mesa Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 20’ Type R forced sump inlet located at I06 where it combines with the surface runoff from DP01. All of the 5-year storm flow is captured by this inlet (Q_5= 6.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 17 CFS) with the remaining flow (Q_{100} = 1.2 CFS) continuing downstream to Inlet 10. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 03 where it is combined with flow from MH02 then conveyed to Storm Manhole 04.

- The total pipe flow conveyed from MH03 to Storm Manhole 04 via a 30” RCP is Q_5= 13 CFS, Q_{100} = 36 CFS.

- Basin B05 (3.2 acres, Q_5= 3.1 CFS, Q_{100} = 9.1 CFS) contains lots in Rolling Hills Ranch 1 along the west side of Rolling Mesa Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I07. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 04.

- The total pipe flow conveyed from MH04 to Storm Manhole 05 via a 36” RCP is Q_5= 16 CFS, Q_{100} = 44 CFS.

- Basin B06 (3.1 acres, Q_5= 3.3 CFS, Q_{100} = 9.9 CFS) contains lots in Rolling Hills Ranch 1 along the east side of Evening Creek Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I08. Most of the flow is captured by this inlet (Q_5= 3.7 CFS, Q_{100} = 9.2 CFS) with the remaining (Q_5= 0.5 CFS, Q_{100} = 3.4 CFS) continuing downstream to Inlet 12. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 05.

- The total pipe flow conveyed from MH05 to Storm Manhole 06 via a 36” RCP is Q_5= 18 CFS, Q_{100} = 51 CFS.

- Basin B07 (4.8 acres, Q_5= 4.3 CFS, Q_{100} = 13 CFS) contains lots in Rolling Hills Ranch 1 along the west side of Evening Creek Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 20' Type R flow-by inlet located at I09. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 06.

- The total pipe flow conveyed from MH06 to Storm Manhole 07, then to MH10 via a 36” RCP is Q_5= 22 CFS, Q_{100} = 59 CFS.

- Basin B08 (2.5 acres, Q_5= 2.5 CFS, Q_{100} = 7.3 CFS) contains lots in Rolling Hills Ranch 1 along east side of Rolling Mesa Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I10. 100-year flow-by from inlet I06 contributes minor flows to inlet I10 for a total 100-year flow of 7.6 CFS. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manholes 08 & 09.

- Basin B09 (2.7 acres, Q_5= 2.6 CFS, Q_{100} = 7.7 CFS) contains lots in Rolling Hills Ranch 1 along south side of Parkland Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R sump inlet located at I11. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 09.

- Basin B10 (3.3 acres, Q_5= 3.1 CFS, Q_{100} = 9.2 CFS) contains lots in Rolling Hills Ranch 1 along west side of Rolling Mesa Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 20' Type R sump inlet located at I12 where it is combined with the surface flow from Basin B11. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 09.

- Basin B11 (3.1 acres, Q_5= 2.9 CFS, Q_{100} = 8.6 CFS) contains lots in Rolling Hills Ranch 1 along east side of Evening Creek Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 20' Type R sump inlet located at I12 where it is combined with the surface flow from Basin B10 and flow-by from B07. All of the flow (Q_5= 5.4 CFS, Q_{100} = 18 CFS) is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 09.

- The total pipe flow conveyed to Storm Manhole 09 is Q_5= 10 CFS, Q_{100} = 31 CFS and is conveyed to Manhole 10 via a 24” RCP. At manhole 10, the flow will combine with the flow from Storm Manhole 07 for a total flow of 30 CFS for the 5-year event and 85 CFS for the 100-year event. The pipe will discharge via a 42” RCP to the existing Pond D constructed in 2012.

<u>Storm Drain System C</u>

Storm Drainage System C meets the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge. This catchment discharges the collected stormwater directly into a Regional Detention Facility with WQCV incorporated into the design and construction. Please see Appendix F for information and exhibits.

- Basin C01 (3.2 acres, Q_5= 3.1 CFS, Q_{100} = 9.0 CFS) contains lots in Rolling Hills Ranch 1 along east side of Rolling Peaks Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I13. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to Storm Manhole 11.

- Basin C02 (3.5 acres, Q_5= 3.4 CFS, Q_{100} = 10 CFS) contains lots in Rolling Hills Ranch 1 along west side of Rolling Peaks Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 15' Type R forced sump inlet located at I14. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 11.

- The total pipe flow conveyed from MH11 to Storm Manhole 12 via a 24" RCP is Q_5= 6.0 CFS, Q_{100} = 18 CFS.

- Basin C03 (1.3 acres, Q_5= 1.4 CFS, Q_{100} = 4.0 CFS) contains lots along Rolling Peaks Dr, Parkland Dr and Crooked Hill Dr in Rolling Hills Ranch 1 and 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 5'Type R forced sump inlet located at I15. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 12.

- Basin C04 (3.1 acres, Q_5= 3.2 CFS, Q_{100} = 9.4 CFS) contains lots along Rolling Peaks Dr, Parkland Dr and Crooked Hill Dr in Rolling Hills Ranch 1. The surface runoff will sheet flow off of the residential lots and be conveyed to a 5'Type R forced sump inlet located at I16. All of the 5-year storm flow is captured by this inlet (Q_5= 3.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 6.3 CFS) with the remaining flow (Q_{100} = 3.1 CFS) continuing downstream to Inlet 18. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 12.

- The total pipe flow conveyed from MH12 to Storm Manhole 13 via a 30" RCP is Q_5= 9.5 CFS, Q_{100} = 26 CFS.

- Basin C05 (0.6 acres, Q_5= 0.6 CFS, Q_{100} = 1.8 CFS) contains lots along Rolling Peaks Dr and Crooked Hill Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 5'Type R sump inlet located at I17. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 13.

- Basin C06 (1.0 acres, Q_5= 1.0 CFS, Q_{100} = 3.1 CFS) contains lots along Rolling Peaks Dr Crooked Hill Dr in Rolling Hills Ranch 1. The surface runoff will sheet flow off of the residential lots, combine with flow-by (Q_{100} = 3.1 CFS) from inlet I16 and be conveyed to a 5'Type R sump inlet located at I18. All of the flow (Q_5= 1.0 CFS, Q_{100} = 6.0 CFS) is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 13.

- The total pipe flow conveyed from MH13 to Storm Manhole 14 via a 36" RCP is Q_5= 11 CFS, Q_{100} = 32 CFS.

- Basin C07 (0.9 acres, Q_5= 0.9 CFS, Q_{100} = 2.5 CFS) contains runoff from an open space tract in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the adjacent residential lots and be conveyed to a Type C grated inlet located at CB1. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 14.

- The total pipe flow conveyed to Storm Manhole 14 is Q_5= 11 CFS, Q_{100} = 34 CFS and is conveyed to Pond D via a 31” RCP.

Storm Drain System D

Storm Drainage System D meets the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge. This catchment discharges the collected stormwater directly into a Regional Detention Facility with WQCV incorporated into the design and construction. Please see Appendix F for information and exhibits.

- Basin D01 (6.9 acres, Q_5= 6.8 CFS, Q_{100} = 19 CFS) contains lots in Rolling Hills Ranch 3 along east side of Bluffpoint Dr and Crooked Bluff Dr. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 15' Type R forced sump inlet located at I19. All of the 5-year storm flow is captured by this inlet (Q_5= 6.8 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 5.7 CFS) continuing downstream to Design Point 2. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 15 and via a 24” RCP to Storm Manhole 16.

- Basin D02 (3.8 acres, Q_5= 3.8 CFS, Q_{100} = 11 CFS) contains lots in Rolling Hills Ranch 3 along west side of Crooked Bluff Dr. The surface runoff will sheet flow off of the residential lots directed to the street to Design Point 2 then combined with flow-by from I19 for a 5-year flow of 3.8 CFS and a 100-year flow of 16 CFS. The surface flow will continue inlet I20.

- Basin D03 (3.8 acres, Q_5= 4.1 CFS, Q_{100} = 12 CFS) contains lots along the west side of Coastal Hills Ln in Rolling Hills Ranch. The surface runoff will sheet flow off of the residential lots and be conveyed to a 20’ Type R forced sump inlet located at I20 where it is combined with the surface flow from DP2 for a 5-year flow of 7.3 CFS and a 100-year flow of 21 CFS. All of the 5-year storm flow is captured by this inlet (Q_5= 7.3 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 17 CFS) with the remaining flow (Q_{100} = 3.4 CFS) continuing downstream to Design Point 3. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 16.

- The total pipe flow conveyed from MH16 to Storm Manhole 17 via a 30” RCP is Q_5= 14 CFS, Q_{100} = 30 CFS.

- Basin D04 (5.3 acres, Q_5= 5.0 CFS, Q_{100} = 14 CFS) contains lots along the west side of Coastal Hills Ln and Bluffpoint Dr in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the residential lots directed to the street to Design Point 3 then combined with flow-by from I20 for a 5-year flow of 5.0 CFS and a 100-year flow of 17 CFS. The surface flow will continue inlet I21.

- Basin D05 (2.0 acres, Q_5= 2.2 CFS, Q_{100} = 6.3 CFS) contains lots along Crooked Hill Dr and Rolling Ranch Dr in Rolling Hills Ranch 1. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15’ Type R forced sump inlet located at I21 where it is combined with the surface flow from DP3 and flow-by from I22 for a 5-year flow of 6.9 CFS and a 100-year flow of 24 CFS. All of the 5-year storm flow is captured by this inlet (Q_5= 6.9 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 10 CFS) continuing downstream to inlet I26. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 17.

- The total pipe flow conveyed from MH17 to Storm Manhole 19 via a 30” RCP is Q_5= 20 CFS, Q_{100} = 41 CFS.

- Basin D06 (3.2 acres, Q_5= 3.2 CFS, Q_{100} = 9.0 CFS) contains lots along the east side of Rolling Ranch Dr in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15’Type R flow-by inlet located at I22. Most of the flow is captured by this inlet (Q_5= 2.6 CFS, Q_{100} = 6.2 CFS) with the remaining (Q_5= 0.6 CFS, Q_{100} = 2.8 CFS) continuing downstream to Inlet 21. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 18.

- Basin D07 (6.6 acres, Q_5= 6.9 CFS, Q_{100} = 20 CFS) contains lots along the west side of Rolling Ranch Dr in Rolling Hills Ranch 31. The surface runoff will sheet flow off of the residential lots and be conveyed to a 10’ Type R forced sump inlet located at I23. All of the 5-year storm flow is captured by this inlet (Q_5= 6.9 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 9.9 CFS) with the remaining flow (Q_{100} = 9.6 CFS) continuing downstream to inlet I24. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 18.

- The total pipe flow conveyed from MH18 to Storm Manhole 19 via a 24” RCP is Q_5= 9.1 CFS, Q_{100} = 16 CFS.

- Basin D08 (1.6 acres, Q_5= 1.8 CFS, Q_{100} = 5.1 CFS) contains lots along the west side of Rolling Ranch Dr in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the residential lots and be conveyed to a 10’ Type R forced sump inlet located at I24. All of the 5-year storm flow is captured by this inlet (Q_5= 1.8 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 9.9 CFS) with the remaining flow (Q_{100} = 3.5 CFS) continuing downstream to inlet I25. The captured flow is conveyed downstream via an 18” RCP to Storm Manhole 18.

- The total pipe flow conveyed from MH19 to Storm Manhole 20 via a 42” RCP is Q_5= 29 CFS, Q_{100} = 64 CFS.

- Basin D09 (0.9 acres, Q_5= 1.2 CFS, Q_{100} = 3.4 CFS) contains runoff from an open space tract in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the adjacent residential lots and be conveyed to a Type C grated inlet located at CB2. All of the flow is captured by this inlet and conveyed downstream via an 18” RCP to inlet I25.

- Basin D10 (0.8 acres, Q_5= 0.9 CFS, Q_{100} = 2.5 CFS) contains lots along Crooked Hill Dr and Rolling Ranch Dr in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the residential lots and be conveyed to a 10' Type R sump inlet located at I25. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 20.

- Basin D11 (4.2 acres, Q_5= 2.4 CFS, Q_{100} = 6.9 CFS) contains runoff from an open space in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the adjacent residential lots and be conveyed to an 18" flared end section located at ES1. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to inlet I26.

- Basin D12 (2.7 acres, Q_5= 2.4 CFS, Q_{100} = 6.9 CFS) contains lots along Crooked Hill Dr and Rolling Ranch Dr in Rolling Hills Ranch 3. The surface runoff will sheet flow off of the residential lots, combine with flow-by (Q_{100} = 9.5 CFS) from inlet I21 and be conveyed to a 20' Type R sump inlet located at I26. All of the flow is captured by this inlet and conveyed downstream via a 24" RCP to Storm Manhole 20.

- The total pipe flow conveyed from MH20 to Storm Manhole 21 via a 42" RCP is Q_5= 29 CFS, Q_{100} = 71 CFS.

- Basin D13 (1.8 acres, Q_5= 2.2 CFS, Q_{100} = 5.8 CFS) contains lots along the west side of Rolling Ranch Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15'Type R flow-by inlet located at I27. Most of the flow is captured by this inlet (Q_5= 1.7 CFS, Q_{100} = 3.9 CFS) with the remaining (Q_5= 0.4 CFS, Q_{100} = 1.8 CFS) continuing downstream to inlet I33. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 21.

- The total pipe flow conveyed from MH21 to Storm Manhole 23 via a 42" RCP is Q_5= 29 CFS, Q_{100} = 72 CFS.

- Basin D14 (6.5 acres, Q_5= 6.3 CFS, Q_{100} = 18 CFS) contains lots in Rolling Hills Ranch 2 along Overlook Bluff Ln, Foggy Meadows Dr and Foggy Bend Ln. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I28. All of the 5-year storm flow is captured by this inlet (Q_5= 6.3 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 9.9 CFS) with the remaining flow (Q_{100} = 7.9 CFS) continuing downstream to inlet I31. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 22.

- Basin D15 (6.4 acres, Q_5= 6.2 CFS, Q_{100} = 18 CFS) contains lots in Rolling Hills Ranch 2 along Overlook Bluff Ln, Foggy Meadows Dr and Foggy Bend Ln. The surface runoff will sheet flow off of the residential lots and directed to the street then to a 10' Type R forced sump inlet located at I29. All of the 5-year storm flow is captured by this inlet (Q_5= 6.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 9.9 CFS) with the remaining flow (Q_{100} = 7.7 CFS) continuing downstream to inlet I30. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 22.

- The total pipe flow conveyed from MH22 to Storm Manhole 23 via a 24" RCP is Q_5= 12 CFS, Q_{100} = 20 CFS and combine with the pipe flow from MH21. The total pipe flow conveyed from MH23 to Storm Manhole 24 via a 28" RCP is Q_5= 35 CFS, Q_{100} = 84 CFS.

- Basin D16 (4.0 acres, Q_5= 4.2 CFS, Q_{100} = 12 CFS) contains lots along Morning Hills Dr, Morning Ridge Ln and Foggy Meadows Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 10'Type R forced sump inlet located at I30.. All of the 5-year storm flow is captured by this inlet (Q_5= 4.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 9.9 CFS) with the remaining flow (Q_{100} = 7.3 CFS) continuing downstream to inlet I31. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 24

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- The total pipe flow conveyed from MH24 to Storm Manhole 25 then to Storm Manhole 26 via a 48" RCP is Q_5= 37 CFS, Q_{100} = 90 CFS.

- Basin D17 (5.1 acres, Q_5= 5.3 CFS, Q_{100} = 15 CFS) contains lots in Rolling Hills Ranch 2 along Morning Hills Dr and Foggy Meadows Dr. The surface runoff will sheet flow off of the residential lots combined with additional surface flow from I28 and I30, then directed along the street then to a 15' Type R sump inlet located at I31. All of the 5-year storm flow is captured by this inlet (Q_5= 5.3 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 24 CFS) with the remaining flow (Q_{100} = 2.5 CFS) continuing downstream to inlet I32. The captured flow is conveyed downstream via a 30" RCP to Storm Manhole 26.

- The total pipe flow conveyed from MH26 to inlet 32 via a 48" RCP is Q_5= 41 CFS, Q_{100} = 106 CFS.

- Basin D18 (3.1 acres, Q_5=3.0 CFS, Q_{100} = 8.4 CFS) contains lots in Rolling Hills Ranch 2 along Morning Hills Dr, Morning Ridge Ln and Overlook Bluff Ln. The surface runoff will sheet flow off of the residential lots directed to the street to a 15' Type R sump inlet located at I32 and combined with flow-by from I31. All of the flow (Q_5= 3.0 CFS, Q_{100} = 10 CFS) is captured by this inlet and combined with flow from Storm Manhole 26.
  Should the sump inlets (I31 & I32) become blocked and cannot take capture all the runoff at the sump inlets, the surface flow will travel overland within an open space between lots 441 & 442.

- The pipe flow conveyed from Storm Manhole 26 is Q_5= 41 CFS, Q_{100} = 106 CFS is combined with the surface flow captured by I32 (Q_5= 3.0 CFS, Q_{100} = 10 CFS) for a total 5-year flow of 43 CFS and a total 100-year flow of 113 CFS, then conveyed to the proposed Pond G.

Storm Drain System E

Storm Drainage System E meets the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge. This catchment discharges the collected stormwater directly into a Regional Detention Facility with WQCV incorporated into the design and construction. Please see Appendix F for information and exhibits.

- Basin E01 (5.4 acres, Q_5= 6.2 CFS, Q_{100} = 17 CFS) contains lots along Valley Peak Dr, Rolling Ranch Dr and Woods Grove Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 20' Type R forced sump inlet located at I33. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 27 then to Storm Manhole 28.

- Basin E02 (6.5 acres, Q_5= 7.3 CFS, Q_{100} = 19 CFS) contains lots along Valley Peak Dr, Woods Grove Dr and Savannah Falls Ct in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 20' Type R forced sump inlet located at I30. All of the 5-year storm flow is captured by this inlet (Q_5= 7.3 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 17 CFS) with the remaining flow (Q_{100} = 2.3 CFS) continuing downstream to inlet I37. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 28.

- The total pipe flow conveyed from MH28 to Storm Manhole 29 via a 30" RCP is Q_5= 14 CFS, Q_{100} = 35 CFS.

- Basin E03 (5.8 acres, Q_5= 6.5 CFS, Q_{100} = 17 CFS) contains lots along Rolling Ranch Dr, Woods Grove Dr, New Ranch Ln and Morning Hills Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15' Type R forced sump inlet located at I35. All of the 5-year storm flow is captured by this inlet (Q_5=6.5 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 3.7 CFS) continuing downstream to inlet I36. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 29.

- The total pipe flow conveyed from MH29 to Storm Manhole 30 via a 36" RCP is Q_5= 20 CFS, Q_{100} = 47 CFS.

- Basin E04 (3.1 acres, Q_5= 3.9 CFS, Q_{100} = 9.7 CFS) contains lots along New Ranch Ln and Morning Hills Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15' Type R forced sump inlet located at I36. All of the flow (Q_5= 3.9 CFS, Q_{100} = 13 CFS) is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 30.

- Basin E05 (2.6 acres, Q_5= 2.7 CFS, Q_{100} = 7.2 CFS) contains lots along Woods Grove Dr and Savannah Falls Ct in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15' Type R flow-by inlet located at I37. Most of the flow is captured by this inlet (Q_5= 2.3 CFS, Q_{100} = 6.0 CFS) with the remaining (Q_5= 0.4 CFS, Q_{100} = 2.7 CFS) continuing downstream to Inlet I41. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 30.

- The total pipe flow conveyed from MH30 to Storm Manhole 31 and Storm Manhole 36 via a 36" RCP is Q_5= 24 CFS, Q_{100} = 61 CFS.

- Basin E06 (1.3 acres, Q_5= 1.4 CFS, Q_{100} = 4.2 CFS) contains lots along Valley Peak Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 5' Type R forced sump inlet located at I38. All of the flow is captured by this inlet and conveyed downstream via an 18" RCP to Storm Manhole 32 then to Storm Manhole 33.

- Basin E07 (2.1 acres, Q_5= 2.5 CFS, Q_{100} = 6.7 CFS) contains lots along Rolling Peaks Dr and Valley Peak Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15' Type R flow-by inlet located at I39. Most of the flow is captured by this inlet (Q_5= 2.0 CFS, Q_{100} = 4.5 CFS) with the remaining (Q_5= 0.5 CFS, Q_{100} = 2.2 CFS) continuing downstream to Inlet I41. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 33.

- The total pipe flow conveyed from MH33 to Storm Manhole 34 via an 18" RCP is Q_5= 3.5 CFS, Q_{100} = 8.5 CFS.

- Basin E08 (4.2 acres, Q_5= 4.8 CFS, Q_{100} = 13 CFS) contains lots surrounded by Rolling Peaks Dr, Valley Peak Dr, Summer Ridge Dr and Bridge Way in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 10' Type R forced sump inlet located at I40. All of the 5-year storm flow is captured by this inlet (Q_5=4.8 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 10 CFS) with the remaining flow (Q_{100} = 2.8 CFS) continuing downstream to an existing inlet located at the intersection of Park Gate Dr. with Lambert Rd. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 34.

- The total pipe flow conveyed from MH34 to Storm Manhole 35 then to Storm Manhole 36 via a 24" RCP is Q_5= 8.0 CFS, Q_{100} = 18 CFS.

- Basin E09 (5.4 acres, Q_5= 6.2 CFS, Q_{100} = 17 CFS) contains lots along Rolling Peaks Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 15' Type R sump inlet located at I41. All of the 5-year storm flow is captured by this inlet (Q_5=6.2 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 14 CFS) with the remaining flow (Q_{100} = 3.9 CFS) continuing downstream to Inlet I43. The captured flow is conveyed downstream via a 24" RCP to Storm Manhole 36.

- The total combined pipe flow from MH30, MH34 and I41 is conveyed to Storm Manhole 37 via a 42" RCP is Q_5= 35 CFS, Q_{100} = 86 CFS.

- Basin E10 (7.0 acres, Q_5= 7.0 CFS, Q_{100} = 10 CFS) contains lots along Summer Ridge Dr in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a 20' Type R sump inlet located at I42. All of the flow is captured by this inlet and conveyed downstream via a 24" RCP to Storm Manhole 37.

- The total combined pipe flow from MH37 is conveyed to Storm Manhole 38 via a 48" RCP is Q_5= 41 CFS, Q_{100} = 102 CFS.

- Basin E11 (13 acres, Q_5= 6.3 CFS, Q_{100} = 18 CFS) contains runoff from an open space tract in Rolling Hills Ranch 2. The surface runoff will sheet flow off of the residential lots and be conveyed to a Type C grated inlet located at CB3. All of the flow is captured by this inlet and conveyed downstream via a 24" RCP to Storm Manhole 37.

- Basin E12 (1.6 acres, Q_5= 3.6 CFS, Q_{100} = 7.5 CFS) contains runoff from Rolling Peaks Dr and Lambert Rd in Rolling Hills Ranch 2. The surface runoff will collected in the curb and gutter then conveyed to a 20' Type R flow-by inlet located at I43. Most of the flow is captured by this inlet (Q_5= 3.2 CFS, Q_{100} = 7.1 CFS) with the remaining (Q_5= 0.4 CFS, Q_{100} = 2.1 CFS) continuing downstream to Inlet I41. The captured flow is conveyed downstream via an 18" RCP to Storm Manhole 38.

- The total combined pipe flow from MH38, I43 and CB3 is conveyed to an existing Storm Manhole EJ02 via a 54" RCP is Q_5= 52 CFS, Q_{100} = 131 CFS.

- Basin E13 (6.0 acres, Q_5= 8.2 CFS, Q_{100} = 19 CFS) contains runoff from Park Gate Rd, Lambert Rd. found in Meridian Ranch Filing 11A and Rolling Peaks Dr in Rolling Hills Ranch 2 The surface runoff will sheet flow off of the residential lots and be conveyed to an existing 15' Type R forced sump inlet constructed with the improvements associated with Meridian Ranch Filing 11A located at EI1. All of the 5-year storm flow is captured by this inlet (Q_5=6.0 CFS) and most of the 100-yr storm flow is captured (Q_{100} = 13 CFS) with the remaining flow (Q_{100} = 6.5 CFS) continuing downstream to an existing inlet located along the west side of Lambert Rd. The captured flow is conveyed downstream via an 18" RCP to existing manhole EJ01.

- The existing storm drain system at existing manhole EJ01 conveys storm flow from other parts of Meridian Ranch Filing 11A and the discharge from Pond D. The flow rates upstream of EJ01 as from the SCS model are 12 CFS for the 5-year storm and 136 CFS for the 100-year storm. The coefficient-area (CA) figure from the approved Final Drainage Report for Meridian Ranch Filing 11A and the time of concentration was adjusted to match the flow rate from the SCS Model to replicate the flow rate in the storm drain. The total flow from Meridian Ranch Filing 11A from MH EJ01 to EJ02 is 22 CFS for the 5-year storm and 140 CFS for the 100-year storm.

- The total combined storm flow at MH EJ02 from Rolling Hills, Meridian Ranch Filing 11A and the discharge from Pond D is 39 CFS for the 5-year storm and 182 CFS for the 100-year storm. The existing storm drain located within Lambert Rd was installed with the construction of the Falcon High School in 2007. The anticipated 10-year flow rate at 128 CFS and the 100-year flow rate for the storm drain was 245 CFS per the approved 2007 Londonderry-Lambert Final Drainage Report. The approved Final Drainage Report for Meridian Ranch Filing 11A shows the 5-year flow rate at 63 CFS and 212 CFS for the 100-year storm. These calculations result buildout flow rates (Q_5= 39 CFS, Q_{100} = 182 CFS) below the previously approved drainage reports, therefore this development will not have any adverse impacts on the existing storm drain located in Lambert Road.

Various Rear yard discharges to Waters of the State

There are various areas along natural and manmade drainage courses that meet the requirements of as found in the El Paso County Engineering Criteria Manual I.7.1.C.5. (ECM) for storm water quality and discharge into Waters of the State. These rear yards discharge into drainage courses located upstream of a Regional Detention Facility with WQCV incorporated into the design and construction. At least 20 percent of the upstream imperviousness within the catchment must be disconnected from the storm drainage system and drain though a pervious area that makes up at least 10 percent of the disconnected impervious area. The rooftops within this catchment make up more than 20 percent of the total impervious area of the catchment and is discharged via roof downspouts and drains across the front yard pervious areas equaling more than 10 percent of the rooftop area. Please see Appendix F for information and exhibits.

DETENTION PONDS

Existing Pond D Detention Storage Criteria

The existing Detention Pond D is located east of Rainbow Bridge Dr., northeast of Meridian Ranch Filing 3, and was constructed as a part of the Meridian Ranch Filing 3 Improvements; the pond is owned and maintained by the Meridian Service Metropolitan District (MSMD). It has been in operation since 2012 with no reported issues. A maintenance agreement between the Meridian Service Metropolitan District and El Paso County has been recorded as a part of the Meridian Ranch Filing 3 Final Plat process.

The SCS calculation method was used to determine inflow and outflow from the detention pond to ensure the developed runoff does not overcharge the pond and the discharges do not adversely impact drainage patterns downstream. Pond D and existing Pond E work in series such that the peak flow rates from the Meridian Ranch development do not adversely affect the drainage patterns downstream of Eastonville Road. Storm drainage runoff will enter the pond from upstream development via existing pipe networks and overland from existing rear lots adjacent to the pond. The ultimate future build-out design of the tributary areas was analyzed to ensure the sizing of the pond would be adequate after development of Meridian Ranch is complete. This SCS calculation can be found in the appendix.

An analysis of the SCS calculations show the development of Rolling Hills Ranch and the discharge flow rates from Pond D do not adversely impact the downstream drainage patterns. No additional improvements or modifications are necessary to this pond as a result of the full buildout of Rolling Hills Ranch Grading. Table 6 provides summary data for the various design storms for the completed development for all areas tributary to Pond D including Rolling Hills Ranch Grading. Rolling Hills Ranch completes the development of all areas tributary to Pond E.

Water quality (WQCV) was added to the required storage volume when the pond was designed and constructed in 2012. The pond was constructed to meet the final build out condition. The WQCV of 1.0 ac-ft. was added to the detention of the minor storm and half (0.5 ac-ft.) was added to the detention volume of the major storm. This was accomplished with respect to the HEC-HMS computer run by providing a starting detention volume of 1.0 ft. for the 5-year storm and 0.5 ft. for the 100-year storm. The resulting storage elevations remain well below the emergency spillway elevation. See Appendix B for more information.

The WQCV was calculated by using the equations found in Volume 2, of the Drainage Criteria Manual (DCM). The release rate from the WQCV is generally very small, which helps minimize downstream impacts. Detaining the WQCV also serves to cleanse the "first flush" of runoff from the higher initial concentration of sediment and pollutants by allowing for settlement to occur. This greatly improves the quality of runoff, leaving the facility and reduces the potential for erosion. The positive impact on water quality is expected to be significant, particularly during the construction phase of the development.

Table 6: Existing Pond D Summary Data

EXISTING POND D				
	PEAK INFLOW	PEAK OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	FT
INTERIM CONDITIONS				
2-YEAR STORM	52	4.0	4.8	7053.2
5-YEAR STORM	107	11	7.4	7053.9
10-YEAR STORM	168	18	11.1	7054.7
25-YEAR STORM	280	52	15.9	7055.6
50-YEAR STORM	387	92	20.1	7056.3
100-YEAR STORM	509	136	25.5	7057.1
FUTURE CONDITIONS				
2-YEAR STORM	52	4.3	4.8	7053.2
5-YEAR STORM	107	12	7.3	7053.9
10-YEAR STORM	168	19	11.1	7054.7
25-YEAR STORM	280	50	15.9	7055.6
50-YEAR STORM	387	91	20.1	7056.3
100-YEAR STORM	509	134	25.5	7057.1

Existing Pond E Detention Storage Criteria

Existing Detention Pond E is located south of Londonderry and west of Eastonville, and was constructed as a part of the Meridian Ranch Filing 11 Grading, the is owned and maintained by the Meridian Service Metropolitan District (MSMD). It has been in operation since 2013 with no reported issues. A maintenance agreement between the Meridian Service Metropolitan District and El Paso County has been recorded as a part of the Meridian Ranch Filing 11A Final Plat process.

The SCS calculation method was used to determine inflow and outflow from the detention pond to ensure the developed runoff does not overcharge the pond and the discharges do not adversely impact drainage patterns downstream of Eastonville Road. Storm drainage runoff will enter the pond from upstream development via existing pipe networks and overland from existing rear lots adjacent to the pond. The ultimate future build-out design of the tributary areas was analyzed to insure the sizing of the pond would be adequate after development of Meridian Ranch is complete. This SCS calculation can be found in the appendix.

An analysis of the SCS calculations show the development of Rolling Hills Ranch and the discharge flow rates from Pond E approximate those of the historic flow rates at Eastonville Road. No additional improvements or modifications are necessary to this pond as a result of the full buildout of Rolling Hills Ranch Grading. Table 7 provides summary data for the various design storms for the completed development for all areas tributary to Pond E including Rolling Hills Ranch Grading. Rolling Hills Ranch completes the development of all areas tributary to Pond E.

Table 7: Existing Pond E Summary Data

EXISTING POND E				
	PEAK INFLOW	PEAK OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	FT
INTERIM CONDITIONS				
2-YEAR STORM	63	5.8	10.3	6970.5
5-YEAR STORM	122	14	17.5	6971.3
10-YEAR STORM	190	27	22.4	6971.8
25-YEAR STORM	308	77	29.4	6972.5
50-YEAR STORM	424	148	35.8	6973.0
100-YEAR STORM	553	237	41.7	6973.5
FUTURE CONDITIONS				
2-YEAR STORM	64	6.6	10.9	6970.6
5-YEAR STORM	126	16	18.0	6971.4
10-YEAR STORM	197	30	23.0	6971.9
25-YEAR STORM	318	80	30.0	6972.5
50-YEAR STORM	432	153	36.4	6973.1
100-YEAR STORM	610	242	42.4	6973.6

Water quality (WQCV) was added to the required storage volume when the pond was designed and constructed in 2013. The pond was constructed to meet the final build out condition. The WQCV of 1.5 ac-ft. was added to the detention of the minor storm and half (0.75 ac-ft.) was added to the detention volume of the major storm. This was accomplished with respect to the HEC-HMS computer run by providing a starting detention volume of 1.5 ft. for the 5-year storm and 0.75 ft. for the 100-year storm. The resulting storage elevations remain well below the emergency spillway elevation. See Appendix B for more information.

The WQCV was calculated by using the equations found in Volume 2, of the Drainage Criteria Manual (DCM). The release rate from the WQCV is generally very small, which helps minimize downstream impacts. Detaining the WQCV also serves to cleanse the "first flush" of runoff from the higher initial concentration of sediment and pollutants by allowing for settlement to occur. This greatly improves the quality of runoff, leaving the facility and reduces the potential for erosion. The positive impact on water quality is expected to be significant, particularly during the construction phase of the development.

Pond G Detention Storage Criteria

Detention Pond G is to be constructed with Rolling Hills Ranch Grading grading in anticipation of the future development of the Rolling Hills Ranch Grading in accordance with the approved Sketch Plan. The pond will be located within the Gieck Ranch Drainage Basin in the eastern portion of Rolling Hills Ranch adjacent to the Falcon Regional Park. The pond will be owned and maintained by the Meridian Service Metropolitan District (MSMD) and a maintenance agreement between the Meridian Service Metropolitan District and El Paso County will be recorded with the Rolling Hills Ranch Filing 1 final plat.

Table 8: Pond G Summary Data

POND G				
	PEAK INFLOW	PEAK OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	FT
INTERIM CONDITIONS				
2-YEAR STORM	11	4.1	3.0	7026.2
5-YEAR STORM	42	13	7.1	7027.2
10-YEAR STORM	91	34	13.8	7027.6
25-YEAR STORM	202	118	18.0	7028.4
50-YEAR STORM	363	238	18.0	7029.1
100-YEAR STORM	558	394	22.1	7029.8
FUTURE CONDITIONS				
2-YEAR STORM	28	5.3	5.3	7026.8
5-YEAR STORM	79	22	8.3	7027.4
10-YEAR STORM	146	56	11.1	7027.9
25-YEAR STORM	288	173	15.8	7028.7
50-YEAR STORM	459	331	20.4	7029.5
100-YEAR STORM	691	478	25.8	7030.3

Pond G and existing Pond F located upstream of Rolling Hills Ranch work in series such that the peak flow rates from the Meridian Ranch development do not adversely affect the drainage patterns downstream of the Meridian Ranch project. The pond is designed to accommodate the developed final inflow from all of the remaining areas to be developed within Meridian Ranch minus the areas tributary to Ponds D and E. Permanent concrete control structure has been designed to handle full build out of the tributary area and reduce the developed flows to approximate the historic peak flow rates for the full spectrum of design storms.

WQCV calculations were completed for Pond G based on proposed future development of the proposed tributary area to the pond; this analysis shows that Pond G will require 0.9 acre-ft of storage for water quality for all the areas tributary to the pond. The control structure at DP H12 is proposed to consist of a 12" diameter water quality control riser with a trash grate having a top elevation of 7025.20 to achieve the required 0.9 ac-ft of storage.

The WQCV was calculated by using the equations found in Volume 2, of the Drainage Criteria Manual (DCM). The release rate from the WQCV is generally very small, which helps minimize downstream impacts. Detaining the WQCV also serves to cleanse the "first flush" of runoff from the higher initial concentration of sediment and pollutants by allowing for settlement to occur. This greatly improves the quality of runoff, leaving the facility and reduces the potential for erosion. The positive impact on water quality is expected to be significant, particularly during the construction phase of the development.

The proposed concrete control structure the outlet of Pond G will attenuate the peak developed flow rates to approximately historic peak rates for the full spectrum of design storms as per the requirements set forth in Resolution 15-042 adopted by the Board of County Commissioners, County of El Paso. The control structure consists of a water quality control standpipe, a rectangular slotted orifice located on the front and a grated top to reduce the developed peak flow rates. Table 8 provides summary data for the various design storms for the completed development for all areas tributary to Pond G including Rolling Hills Ranch Grading.

Downstream Analysis

The outlets (DP H08 & H09) for Pond E located along Eastonville Road upstream of 4-Way Ranch Filing 1 were analyzed in detail with the 2018 MDDP associated with the most recent Meridian Ranch Sketch Plan Amendment. The information can be found in Appendix D of the January 2018 Meridian Ranch MDDP. Below you will find a summary table providing release rates of flow for each Pond E outlet. See the Downstream Channel Analysis Appendix in the WindingWalk Filing 1 Final Drainage Report for a letter to the El Paso County Engineer regarding channel stability and analysis.

The outlet (DP G12) for Pond G is located west of the Falcon Regional Park, upstream of Eastonville Rd (DP G06). Pond G will discharge 479 CFS during the 100-yr storm event into an existing natural drainage course that traverses the regional park. The 100-year historical peak flow rate at the western boundary of the regional park is 544 CFS. The calculated 100-year developed flow rate will be 88% of the historic flow rate. The developed peak flow rate for the full spectrum of design storms are calculated to be below that of the corresponding historic peak flow rates. See Table 9 for a complete comparative list of the peak flow rates for the key design points impacted by the development of Rolling Hills Ranch.

Table 9: Key Design Point Comparison - SCS

MERIDIAN RANCH DISCHARGE KEY DESIGN POINTS (FUTURE)						
		PEAK DISCHARGE Q_{100} (CFS)	PEAK DISCHARGE Q_{50} (CFS)	PEAK DISCHARGE Q_{25} (CFS)	PEAK DISCHARGE Q_{10} (CFS)	PEAK DISCHARGE Q_5 (CFS)
G12 - POND G OUTLET REGIONAL PARK (G05 - HISTORIC)	Historic	544	355	212	86	31
	Future	478	331	173	56	22
	% of Historic	88%	93%	82%	66%	72%
G06 - EASTONVILLE ROAD[1]	Historic	561	375	225	91	33
	Future	505	348	184	60.9	24
	% of Historic	90%	93%	82%	67%	75%
H08 - EASTONVILLE ROAD (POND E NORTH OUTLET)	Historic	216	142	85	34	12
	Future	205	137	72	24	12
	% of Historic	95%	96%	85%	72%	97%
H09 - EASTONVILLE ROAD (POND E SOUTH OUTLET)	Historic	77	51	30	12	4.5
	Future	37	16	8.3	5.9	4.1
	% of Historic	48%	31%	27%	49%	92%
G14 - REGIONAL PARK (G07 - HISTORIC)	Historic	55	37	23	9.8	3.9
	Future	34	23	13	5.5	2.0
	% of Historic	62%	61%	58%	56%	51%
G08 - EASTONVILLE ROAD[1]	Historic	119	78	48	20	7.6
	Future	106	69	41	16	5.8
	% of Historic	90%	89%	87%	82%	77%

[1] Flow rate at Eastonville Rd. listed for reference only

POND F – POND G CHANNEL

Methodology and Background

The drainage way within the proposed development is best characterized as wide sandy bottom natural arroyo, with some amounts of vegetation along the side embankments. The drainage way conveys the storm runoff released from existing Pond F and surrounding areas easterly to the proposed Pond G. The drainage course conveys water only during runoff events. The arroyo will require relocation and shaping immediately downstream of Pond F as it runs along the north side of future Rex Road. The channel will remain in its natural condition between Rex Road and Pond G. A HecRas hydraulic analysis for this drainage course will be included within the Rolling Hills Ranch Filing 1 Final Drainage Report for the purpose of demonstrating the stability of the sandy bottom channel after development occurs in the surrounding area.

Due to the nature and conditions of the existing channel, efforts will be made to preserve it as close to the natural conditions outside the limits of the development. This natural drainage way can be defined as a ‘straight’ channel, it does not follow sinuous course. It is not braided or excessively meandering. The drainage path does have some minor meanderings but does not have multiple channels divided by bars and islands or large alternating S-shaped bends with deep scour pools.

Development will always alter the natural drainage system, such as increasing the peak flow rates, decreasing the sediment load, encroaching into the floodplain, etc. This drainage way has experienced a decreased sediment load with the construction of Pond F at the upstream end. The flow rates at the upper end are slightly less than historic and higher at the lower end prior to entering the proposed Pond G detention pond. The flow rates are then lowered as they are released from Pond G.

EROSION CONTROL DESIGN

General Concept

Historically, erosion on this property has been held to a minimum by a variety of natural features and agricultural practices including:

- Substantial prairie grass growth
- Construction of drainage arresting berms
- Construction of multiple stock ponds along drainage courses

Existing temporary sediment ponds will also help to minimize erosion by reducing both the volume and velocity of the peak runoff.

During construction, best management practices (BMP) for erosion control will be employed based on El Paso county Criteria. BMP's will be utilized as deemed necessary by the contractor and/or engineer and are not limited to the measures shown on the construction drawing set. The contractor shall minimize the amount of area disturbed during all construction activities.

In general the following shall be applied in developing the sequence of major activities:

- Install down-slope and side-slope perimeter BMP's before the land disturbing activity occurs.
- Do not disturb an area until it is necessary for the construction activity to proceed
- Cover or stabilize as soon as possible.
- Time the construction activities to reduce the impacts from seasonal climatic changes or weather events.
- The construction of filtration BMP's should wait until the end of the construction project when upstream drainage areas have been stabilized.
- Do not remove the temporary perimeter controls until after all upstream areas are stabilized.

Four Step Process

The following four step process is recommended for selecting structural BMP's in developing urban areas:

Step 1: Employ Runoff Reduction Practices

This development incorporates wider rights-of-way than other developments, thus decreasing the amount area devoted to pavement. The rights-of-way within Meridian Ranch are 20% wider, 60 ft. instead of 50 ft., creating more landscaped area within the development.

The project has over ten acres of open space, accounting for over 20% of the entire project, creating a lower density development.

Homeowners and builders are encouraged to direct roof drains to the sideyards where the runoff will travel overland to the streets and creating an opportunity to allow the runoff to infiltrate into the ground.

The development has been designed to direct surface sheet flow from rear yard space toward the natural open space between the home sites and the drainage courses (see below) thus increasing the infiltration and serving to reduce the total runoff from the project site.

Step 2: Stabilize Drainageways

The drainage swale located on the west side of the project was designed to have a wide flat bottom and slope reducing the velocity of the concentrated flow traveling along the drainageway. The construction of the swale also included erosion control mat along the entire length of the swale. At steeper sections of the swale straw logs or rip-rap has been installed to reduce velocities and erosion. This swale discharges directly into an existing extended detention pond with WQCV built into the design.

A natural arroyo drainage course exists adjacent to the project on the northeast side. This natural sandy bottom arroyo will readily infiltrate runoff during lower intensity, more frequent rain events; decreasing the total stormwater volume leaving the sight.

Step 3: Provide Water Quality Capture Volume (WQCV)

An existing extended detention pond with water quality capture volume is located to the east of the project that was designed to accommodate the runoff from this development.

The project includes a proposed extended detention pond along the eastern boundary of the project. The WQCV within the proposed detention pond is of sufficient size to accommodate the runoff from this project and all future projects tributary to the proposed detention pond.

Step 4: Consider Need for Industrial and Commercial BMP's

This project is neither industrial nor commercial and therefore this section does not apply.

The measures from Steps 1, 2, & 3 incorporated into the design of the project work together to promote greater infiltration rates and reduce the total volume of storm runoff from the project. A key component of the design is the overland sheet flow directed toward the drainage swales, this allows the runoff to move across the land at a lower rate and increase the likelihood of infiltration. By directing the runoff toward the sandy bottom arroyo, the water has increased chances to infiltrate. By providing a regional water quality facility the design provides greater flexibility to direct the runoff to natural swales to convey to the facility as opposed to conveyance through storm drain pipe.

Temporary Sedimentation Pond

Temporary sedimentation ponds installed during the overlot grading process will act as the primary water quality control for the areas upstream during construction. Runoff will travel overland toward the existing sedimentation ponds, collected and diverted into the proposed storm drain system and discharged into existing downstream systems. The pond will provide initial sediment control over exposed upstream areas.

Detention Pond

The existing and proposed detention ponds will act as the primary water quality control for the areas within the project boundaries. Runoff will travel overland toward the natural drainage swales or be collected by the proposed storm drainage system and diverted into the detention pond where practical. The pond will serve a dual purpose: first, by facilitating the settling of sediment in runoff during and after construction (by means of the WQCV) and, second, by maintaining runoff at or below existing levels.

Silt Fence

Silt fence will be place along downstream limits of disturbed areas. This will prevent suspended sediment from leaving the site during infrastructure construction. Silt fencing is to remain in place until vegetation is reestablished.

Erosion Bales

Erosion bales will be placed ten (10) feet from the inlet of all culverts during construction to prevent culverts from filling with sediment. Erosion bales will remain in place until vegetation is reestablished. Erosion bale checks will be used on slopes greater than 1 percent to reduce flow velocities until vegetation is reestablished.

Miscellaneous

Best erosion control practices will be utilized as deemed necessary by the Contractor or Engineer and are not limited to the measures described above.

Add a fee section stating at this time the Geick Drainage Basin is not included in the El Paso County Drainage Basin Fee Program.

REFERENCES

1. "City of Colorado Springs/El Paso County Drainage Criteria Manual" September 1987, Revised November 1991, Revised October 1994.

2. Chapter 6, Hydrology and Chapter 11, Storage, Section 3.2.1 of the "City of Colorado Springs Drainage Criteria Manual" May 2014.

3. "Volume 2, El Paso County/City of Colorado Springs Drainage Criteria Manual-Stormwater Quality Policies, Procedures and Best Management Practices" November 1, 2002.

4. Flood Insurance Rate Study for El Paso County, Colorado and Incorporated Areas. Federal Emergency Management Agency, Revised March 17, 1997.

5. Soils Survey of El Paso County area, Natural Resources Conservation Services of Colorado.

6. Master Development Drainage Plan Meridian Ranch. August 2000. Prepared by URS Corp.

7. Revision to Master Development Drainage Plan Meridian Ranch. May 2015. Prepared by Tech Contractors.

8. Master Development Drainage Plan Latigo Trails. October 2001. Prepared by URS Corp.

9. Final Drainage Report for Meridian Ranch Filing 1. November 2001. Prepared by URS Corp.

10. Preliminary Drainage Plan for Meridian Ranch Phase II. September 2003. Prepared by URS.

11. Final Drainage Plan for The Trails Filing No.7. March 2005. Prepared by URS.

12. Final Drainage Report for Meridian Ranch Filing 3. August 2011. Prepared by Tech Contractors.

13. Preliminary and Final Drainage Report for Meridian Ranch Filing 7. June 2012. Prepared by Tech Contractors.

14. Final Drainage Report for Meridian Ranch Estates Filing 2. July 2013. Prepared by Tech Contractors.

15. Final Drainage Report for Meridian Ranch Filing 11A. March 2014. Prepared by Tech Contractors.

16. Preliminary and Final Drainage Report for Meridian Ranch Filing 8. December 2014. Prepared by Tech Contractors.

17. Preliminary and Final Drainage Report for Meridian Ranch Filing 4B. April 2014. Prepared by Tech Contractors.

18. Final Drainage Report for Stonebridge Filing 1 at Meridian Ranch. June 2014. Prepared by Tech Contractors.

19. Final Drainage Report for Meridian Ranch Filing 9. May 2015. Prepared by Tech Contractors.

20. Revision to Master Development Drainage Plan Meridian Ranch. July 2015. Prepared by Tech Contractors.

21. Final Drainage Report for Meridian Ranch Estates Filing 3. October 2015. Prepared by Tech Contractors.

22. Final Drainage Report for the Vistas Filing 1 at Meridian Ranch. July 2016. Prepared by Tech Contractors.

23. Final Drainage Report for Stonebridge Filing 2 at Meridian Ranch. September 2016. Prepared by Tech Contractors.

24. Final Drainage Report for Stonebridge Filing 3 at Meridian Ranch. April 2017. Prepared by Tech Contractors.

25. Interim Drainage Report for WindingWalk Grading. February 2018. Prepared by Tech Contractors.

26. Revision to Master Development Drainage Plan Meridian Ranch. January 2018. Prepared by Tech Contractors.

27. Preliminary Drainage Report for WindingWalk Filings 1 & 2 PUD and Final Drainage Report for WindingWalk Filing 1 at Meridian Ranch. April 2018. Prepared by Tech Contractors.

28. Final Drainage Report for WindingWalk Filing 2 at Meridian Ranch. August 2018. Prepared by Tech Contractors.

29. Final Drainage Report for Stonebridge Filing 4 at Meridian Ranch. September 2018. Prepared by Tech Contractors.

30. "Urban Storm Drainage Criteria Manual" September 1969, Revised January 2016.

31. Design Guidelines & Criteria – Channels & Hydraulic Structures on Sandy Soil, June 1981 by Simons, Li & Associates.

Appendices

Appendix A – Rational Calculations

INSERT RATIONAL APPENDIX STANDARD INSERT

RUNOFF COEFFICIENTS

Update inserts.

INTENSITY CURVES

Replace

COMPOSITE 'C' FACTORS

PROJECT: **Rolling Hills Ranch PUD** 2/3/2020

BASIN DESIGNATION	AREA (AC.) UNDEV	2 DU/AC	3 DU/AC	4 DU/AC	5 DU/AC	STREETS	OPEN SPACE PARKS/GC	TOTAL	COMPOSITE FACTOR 5-year	100-year	Percent Impervious
OS1	4.4							4.4	**0.09**	**0.36**	**0.0%**
A01		1.2	2.2			1.1	2.9	7.5	**0.34**	**0.52**	**28.8%**
A02		0.8	1.4					2.2	**0.24**	**0.47**	**28.2%**
A03		0.3	0.5			0.1		0.8	**0.29**	**0.50**	**33.7%**
B01			1.1	1.2				2.3	**0.28**	**0.49**	**35.1%**
B02			2.8	2.9				5.6	**0.28**	**0.49**	**35.1%**
B03			1.8	1.9			0.7	4.3	**0.27**	**0.47**	**29.6%**
B04			1.5	1.6				3.0	**0.28**	**0.49**	**35.1%**
B05			1.6	1.7				3.2	**0.28**	**0.49**	**35.1%**
B06			1.5	1.6				3.1	**0.28**	**0.49**	**35.1%**
B07			2.3	2.4				4.8	**0.28**	**0.49**	**35.1%**
B08			1.2	1.3				2.5	**0.28**	**0.49**	**35.1%**
B09			1.3	1.4				2.7	**0.28**	**0.49**	**35.1%**
B10			1.6	1.7				3.3	**0.28**	**0.49**	**35.1%**
B11			1.5	1.6				3.1	**0.28**	**0.49**	**35.1%**
C01			1.5	1.6				3.2	**0.28**	**0.49**	**35.1%**
C02			1.7	1.8				3.5	**0.28**	**0.49**	**35.1%**
C03			0.7	0.7				1.3	**0.28**	**0.49**	**35.1%**
C04			1.5	1.6				3.1	**0.28**	**0.49**	**35.1%**
C05			0.3	0.3				0.6	**0.28**	**0.49**	**35.2%**
C06			0.5	0.5				1.0	**0.28**	**0.49**	**35.1%**
C07			0.2	0.2			0.6	0.9	**0.25**	**0.44**	**14.4%**
D01			1.0	5.9				6.9	**0.29**	**0.50**	**38.6%**
D02			0.6	3.3				3.8	**0.29**	**0.50**	**38.6%**
D03			0.6	3.3				3.8	**0.29**	**0.50**	**38.6%**
D04			0.8	4.5				5.3	**0.29**	**0.50**	**38.6%**
D05			0.3	1.7				2.0	**0.29**	**0.50**	**38.6%**
D06			0.5	2.7				3.2	**0.29**	**0.50**	**38.6%**
D07			0.9	5.7				6.6	**0.29**	**0.50**	**38.6%**
D08			0.2	1.4				1.6	**0.29**	**0.50**	**38.6%**
D09			0.1	0.5			1.1	1.6	**0.26**	**0.44**	**13.9%**
D10			0.1	0.7				0.8	**0.29**	**0.50**	**38.5%**
D11			0.4	2.2			1.6	4.2	**0.27**	**0.46**	**24.6%**
D12			0.4	2.3				2.7	**0.29**	**0.50**	**38.6%**
D13				0.9	0.9			1.8	**0.32**	**0.51**	**41.4%**
D14			0.9	5.5				6.5	**0.29**	**0.50**	**38.6%**
D15			0.9	5.4				6.4	**0.29**	**0.50**	**38.6%**
D16			0.6	3.4				4.0	**0.29**	**0.50**	**38.6%**
D17			0.7	4.4				5.1	**0.29**	**0.50**	**38.6%**
D18			0.5	2.7				3.1	**0.29**	**0.50**	**38.6%**
E01				2.8	2.6			5.4	**0.32**	**0.51**	**41.4%**
E02				3.4	3.1			6.5	**0.32**	**0.51**	**41.4%**
E03				3.1	2.8			5.8	**0.32**	**0.51**	**41.4%**
E04				1.4	1.3	0.3	0.2	3.1	**0.37**	**0.54**	**43.8%**
E05				1.3	1.2			2.6	**0.32**	**0.51**	**41.4%**
E06				0.7	0.6			1.3	**0.32**	**0.51**	**41.4%**
E07				1.1	1.0			2.1	**0.32**	**0.51**	**41.4%**
E08				2.2	2.0			4.2	**0.32**	**0.51**	**41.4%**
E09				2.9	2.6			5.4	**0.32**	**0.51**	**41.4%**
E10				3.7	3.3			7.0	**0.32**	**0.51**	**41.4%**
E11				1.0	2.1		9.9	13.0	**0.26**	**0.44**	**11.6%**
E12						1.0	0.6	1.6	**0.64**	**0.74**	**61.4%**
E13				1.3	2.5	1.0	1.2	6.0	**0.41**	**0.57**	**44.0%**
								72.2		**Composite:**	**34.7%**
TOTAL	4	3	39	107	26	3	20	202.1	**0.30**	**0.49**	**34.7%**

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Replace

DATE: 2/3/2020

TIME OF CONCENTRATION

PROJECT: **Rolling Hills Ranch PUD**

TIME OF CONCENTRATION																	
SUBBASIN DATA			INIT./OVERLAND TIME (T_i)				TRAVEL TIME (T_t)							TOTAL	Tc Check (Urbanized Basins)		FINAL T_c
BASIN DESIGNATION	C_5	AREA (AC)	LENGTH (FT)	ΔH	SLOPE %	Ti (Min.)*	LENGTH (FT)	ΔH	SLOPE %	CONVEYANCE TYPE	CONVEYANCE COEF.	VEL. (FPS)	Tt(Min.)**	Ti+Tt(Min.)	L (FT)	Tc = (L/180) + 10	(min)
OS1	0.09	4.4	185	6.0	3.2%	17.0	1275	34	2.7%	L	7	1.1	18.6	35.6	1460.00	18.1	**18.1**
A01	0.34	7.5	185	9.0	4.9%	11.2	985	25	2.5%	P	20	3.2	5.2	16.4	1170.00	16.5	**16.4**
A02	0.24	2.2	214	4.5	2.1%	18.0	230	4	1.7%	P	20	2.6	1.5	19.5	444.00	12.5	**12.5**
A03	0.29	0.8	25	0.5	2.0%	5.9	230	4	1.7%	P	20	2.6	1.5	7.4	255.00	11.4	**7.4**
B01	0.28	2.3	242	6.0	2.5%	17.4	838	16	1.9%	P	20	2.8	5.1	22.4	1080.00	16.0	**16.0**
B02	0.28	5.6	300	9.0	3.0%	18.2	902	17	1.9%	P	20	2.7	5.5	23.6	1202.00	16.7	**16.7**
B03	0.27	4.3	280	10.0	3.6%	16.7	494	11	2.1%	P	20	2.9	2.8	19.5	774.00	14.3	**14.3**
B04	0.28	3.0	43	0.9	2.0%	7.9	1352	26	1.9%	P	20	2.8	8.1	16.0	1395.00	17.8	**16.0**
B05	0.28	3.2	130	2.6	2.0%	13.7	845	20	2.4%	P	20	3.1	4.6	18.3	975.00	15.4	**15.4**
B06	0.28	3.1	30	0.6	2.0%	6.6	914	19	2.1%	P	20	2.9	5.3	11.9	944.00	15.2	**11.9**
B07	0.28	4.8	67	1.3	2.0%	9.8	1380	25	1.8%	P	20	2.7	8.5	18.4	1447.00	18.0	**18.0**
B08	0.28	2.5	155	3.2	2.1%	14.8	731	16	2.2%	P	20	3.0	4.1	18.9	886.00	14.9	**14.9**
B09	0.28	2.7	155	3.2	2.1%	14.8	916	18	1.9%	P	20	2.8	5.5	20.3	1071.00	16.0	**16.0**
B10	0.28	3.3	160	3.2	2.0%	15.2	962	18	1.8%	P	20	2.7	5.9	21.1	1122.00	16.2	**16.2**
B11	0.28	3.1	155	3.2	2.1%	14.8	843	18	2.1%	P	20	2.9	4.9	19.7	998.00	15.5	**15.5**
C01	0.28	3.2	155	3.2	2.1%	14.8	745	20	2.7%	P	20	3.3	3.8	18.6	900.00	15.0	**15.0**
C02	0.28	3.5	160	4.2	2.6%	13.9	745	20	2.7%	P	20	3.3	3.8	17.6	905.00	15.0	**15.0**
C03	0.28	1.3	135	2.7	2.0%	13.9	404	4	1.0%	P	20	2.0	3.4	17.3	539.00	13.0	**13.0**
C04	0.28	3.1	217	4.5	2.1%	17.5	346	3	0.9%	P	20	1.9	3.1	20.6	563.00	13.1	**13.1**
C05	0.28	0.6	80	1.6	2.0%	10.7	334	3	0.9%	P	20	1.9	2.9	13.7	414.00	12.3	**12.3**
C06	0.28	1.0	50	1.0	2.0%	8.5	602	5	0.8%	P	20	1.8	5.5	14.0	652.00	13.6	**13.6**
C07	0.25	0.9	160	3.0	1.9%	15.9	167	2	1.0%	G	15	1.5	1.8	17.8	327.00	11.8	**11.8**
D01	0.29	6.9	125	2.5	2.0%	13.1	1060	23	2.2%	P	20	2.9	6.0	19.1	1185.00	16.6	**16.6**
D02	0.29	3.8	260	10.0	3.8%	15.2	880	16	1.8%	P	20	2.7	5.4	20.7	1140.00	16.3	**16.3**
D03	0.29	3.8	40	0.8	2.0%	7.4	1140	28	2.4%	P	20	3.1	6.1	13.5	1180.00	16.6	**13.5**

TIME OF CONCENTRATION

SUBBASIN DATA			INIT./OVERLAND TIME (T_i)				TRAVEL TIME (T_t)							TOTAL	Tc Check (Urbanized Basins)		FINAL T_c
BASIN DESIGNATION	C_5	AREA (AC)	LENGTH (FT)	ΔH	SLOPE %	Ti (Min.)*	LENGTH (FT)	ΔH	SLOPE %	CONVEYANCE TYPE	CONVEYANCE COEF.	VEL. (FPS)	Tt(Min.)**	Ti+Tt(Min.)	L (FT)	Tc = (L/180) + 10	(min)
D04	0.29	5.3	90	1.8	2.0%	11.1	1350	32	2.4%	P	20	3.1	7.3	18.4	1440.00	18.0	**18.0**
D05	0.29	2.0	155	3.1	2.0%	14.6	350	5	1.4%	P	20	2.4	2.4	17.1	505.00	12.8	**12.8**
D06	0.29	3.2	140	2.8	2.0%	13.9	1005	26	2.6%	P	20	3.2	5.2	19.1	1145.00	16.4	**16.4**
D07	0.29	6.6	160	3.2	2.0%	14.9	675	20	3.0%	P	20	3.4	3.3	18.1	835.00	14.6	**14.6**
D08	0.29	1.6	150	3.0	2.0%	14.4	405	4	1.0%	P	20	2.0	3.4	17.8	555.00	13.1	**13.1**
D09	0.26	1.6	175	3.5	2.0%	16.2	285	3	1.0%	L	7	0.7	6.8	23.0	NON-URBAN AREA		**23.0**
D10	0.29	0.8	80	1.6	2.0%	10.5	435	4	0.9%	P	20	1.9	3.8	14.3	515.00	12.9	**12.9**
D11	0.27	4.2	195	6.0	3.1%	14.6	975	10	1.0%	L	7	0.7	22.9	37.5	NON-URBAN AREA		**37.5**
D12	0.29	2.7	150	3.0	2.0%	14.4	1565	16	1.0%	P	20	2.0	12.9	27.3	1715.00	19.5	**19.5**
D13	0.32	1.8	145	2.9	2.0%	13.6	405	8	2.0%	P	20	2.8	2.4	16.0	550.00	13.1	**13.1**
D14	0.29	6.5	150	3.0	2.0%	14.4	1120	14	1.3%	P	20	2.2	8.3	22.7	1270.00	17.1	**17.1**
D15	0.29	6.4	145	2.9	2.0%	14.1	1110	15	1.4%	P	20	2.3	8.0	22.1	1255.00	17.0	**17.0**
D16	0.29	4.0	255	5.1	2.0%	18.8	500	13	2.6%	P	20	3.2	2.6	21.3	755.00	14.2	**14.2**
D17	0.29	5.1	245	4.9	2.0%	18.4	660	14	2.1%	P	20	2.9	3.8	22.2	905.00	15.0	**15.0**
D18	0.29	3.1	100	2.0	2.0%	11.7	1390	24	1.7%	P	20	2.6	8.8	20.6	1490.00	18.3	**18.3**
E01	0.32	5.4	165	3.3	2.0%	14.5	672	17	2.5%	P	20	3.2	3.5	18.0	837.00	14.7	**14.7**
E02	0.32	6.5	268	13.0	4.9%	13.8	700	20	2.9%	P	20	3.4	3.5	17.2	968.00	15.4	**15.4**
E03	0.32	5.8	247	6.0	2.4%	16.6	795	6	0.8%	P	20	1.7	7.6	24.3	1042.00	15.8	**15.8**
E04	0.37	3.1	50	1.0	2.0%	7.6	1115	8	0.7%	P	20	1.7	11.0	18.5	1165.00	16.5	**16.5**
E05	0.32	2.6	242	12.0	5.0%	13.0	1140	26	2.3%	P	20	3.0	6.3	19.3	1382.00	17.7	**17.7**
E06	0.32	1.3	140	2.8	2.0%	13.4	307	6	2.0%	P	20	2.8	1.8	15.2	447.00	12.5	**12.5**
E07	0.32	2.1	280	11.0	3.9%	15.1	200	8	4.0%	P	20	4.0	0.8	15.9	480.00	12.7	**12.7**
E08	0.32	4.2	140	2.8	2.0%	13.4	740	16	2.2%	P	20	2.9	4.2	17.6	880.00	14.9	**14.9**
E09	0.32	5.4	255	8.0	3.1%	15.5	625	18	2.9%	P	20	3.4	3.1	18.6	880.00	14.9	**14.9**
E10	0.32	7.0	172	6.0	3.5%	12.3	1583	35	2.2%	P	20	3.0	8.9	21.2	1755.00	19.8	**19.8**
E11	0.26	13.0	182	3.0	1.6%	17.5	1696	35	2.1%	L	7	1.0	28.1	45.6	NON-URBAN AREA		**45.6**
E12	0.64	1.6	25	0.5	2.0%	3.3	1350	12	0.9%	P	20	1.9	11.9	15.3	1375.00	17.6	**15.3**
E13	0.41	6.0	161	6.0	3.7%	10.3	1188	22	1.9%	P	20	2.7	7.3	17.6	1349.00	17.5	**17.5**

TIME OF CONCENTRATION												
SUBBASIN DATA			INIT./OVERLAND TIME (T_i)				TRAVEL TIME (T_t)					
BASIN DESIGNATION	C_5	AREA (AC)	LENGTH (FT)	ΔH	SLOPE %	Ti (Min.)*	LENGTH (FT)	ΔH	SLOPE %	CONVEYANCE TYPE	CONVEYANCE COEF.	VE (F
			11A Designation									
Ex1	0.61	1.1	4		FROM APPROVED MERIDIAN RANCH FILING 11A FINAL DI							
Ex2	0.55	1.3	5									
Ex3	0.77	1.7	20									
Ex4	0.53	1.8	15									
Ex5	0.57	2.3	14									
Ex6	0.47	1.4	16									
Ex7	0.44	1.4	17									
Ex8	0.38	2.2	18									

Notes:	$* \; T_i = \dfrac{*\; T_i = 0.395\,(1.1-C5)L^{0.5}}{S^{0.33}}$	
	$V = C_V S_W^{0.5}$	$** \; Tt = L \times V$

TYPE O
HEAVY MEADOW
TILLAGE/FIELD
RIPRAP (not buried)
SHORT PASTURE AND
NEARLY BARE GROUN
GRASSED WATERWAY
PAVED AREAS

fix.

Remove and replace with the simplified tributary areas to the sediment ponds.

STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)
SURFACE ROUTING

PROJECT: **Rolling Hills Ranch PUD**

Date: 2/3/2020

DESIGN POINT	DIRECT RUNOFF											TOTAL RUNOFF							OVERLAND TRAVEL TIME						
	BASIN	AREA (AC)	Tc (Min.)	I (in./ hr.)		COEFF. ©		CA		Q		Sum Tc (min.)	I (in./ hr.)		CA		Q		DESTINATION DP	CONVEYANCE TYPE	COEFFICIENT C_V	SLOPE %	VEL. (FPS)	LENGTH (FT)	TRAVEL TIME Tt
				(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)		(5 YR)	(100 YR)	(5 YR)	(100 YR)	**(5 YR)**	**(100 YR)**							
	DEVELOPED																								
CB4	OS1	4.4	18.1	3.24	5.44	0.09	0.36	0.40	1.58	1.3	9						**1.3**	**8.6**							
I01	A01	7.5	16.4	3.39	5.68	0.34	0.52	2.55	3.90	8.6	22						**8.6**	**22**	I02	P	20.0	1.00%	2.0	61	0.5
I02	A02	2.2	12.5	3.80	6.38	0.24	0.47	0.53	1.03	2.0	6.5	16.9	3.34	5.61	0.53	2.54	**2.0**	**14**							
I03	A03	0.8	7.4	4.59	7.71	0.29	0.50	0.23	0.40	1.0	3.1						**1.0**	**3.1**							
I04	B01	2.3	16.0	3.42	5.75	0.28	0.49	0.63	1.11	2.2	6.4						**2.2**	**6.4**							
I05	B02	5.6	16.7	3.36	5.64	0.28	0.49	1.55	2.73	5.2	15						**5.2**	**15**	I14	P	20.0	2.54%	3.2	865	4.5
DP1	B03	4.3	14.3	3.59	6.03	0.27	0.47	1.17	2.05	4.2	12						**4.2**	**12**	I06	P	20.0	2.23%	3.0	963	5.4
I06	B04	3.0	16.0	3.42	5.75	0.28	0.49	0.84	1.47	2.9	8.5	19.7	3.11	5.23	2.01	3.52	**6.2**	**18**	I10	P	20.0	2.00%	2.8	852	5.0
I07	B05	3.2	15.4	3.48	5.84	0.28	0.49	0.89	1.56	3.1	9.1						**3.1**	**9.1**							
I08	B06	3.1	11.9	3.87	6.50	0.28	0.49	0.86	1.52	3.3	9.9						**3.3**	**9.9**							
I09	B07	4.8	18.0	3.24	5.45	0.28	0.49	1.31	2.31	4.3	13						**4.3**	**13**	I12	P	20.0	1.86%	2.7	970	5.9
I10	B08	2.5	14.9	3.53	5.92	0.28	0.49	0.70	1.23	2.5	7.3	19.9	3.09	5.19	0.70	1.46	**2.5**	**7.6**							
I11	B09	2.7	16.0	3.43	5.76	0.28	0.49	0.76	1.33	2.6	7.7						**2.6**	**7.7**							
I12	B10	3.3	16.2	3.40	5.71	0.28	0.49	0.92	1.62	3.1	9.2						**3.1**	**9.2**							
I12	B11	3.1	15.5	3.47	5.82	0.28	0.49	0.84	1.48	2.9	8.6	24.0	2.82	4.73	1.92	3.73	**5.4**	**18**							
I13	C01	3.2	15.0	3.52	5.91	0.28	0.49	0.87	1.53	3.1	9.0						**3.1**	**9.0**							
I14	C02	3.5	15.0	3.52	5.91	0.28	0.49	0.98	1.72	3.4	10	21.2	3.00	5.04	0.98	2.04	**3.4**	**10**							
I15	C03	1.3	13.0	3.74	6.27	0.28	0.49	0.37	0.65	1.4	4.0						**1.4**	**4.0**							
I16	C04	3.1	13.1	3.72	6.25	0.28	0.49	0.85	1.50	3.2	9.4						**3.2**	**9.4**	I18	P	20.0	1.00%	2.0	165	1.4
I17	C05	0.6	12.3	3.82	6.41	0.28	0.49	0.16	0.28	0.6	1.8						**0.6**	**1.8**							
I18	C06	1.0	13.6	3.67	6.15	0.28	0.49	0.28	0.50	1.0	3.1	14.5	3.57	6.00	0.28	1.00	**1.0**	**6.0**							
CB1	C07	0.9	11.8	3.88	6.51	0.25	0.44	0.22	0.39	0.9	2.5						**0.9**	**2.5**							
I19	D01	6.9	16.6	3.37	5.66	0.29	0.50	2.01	3.41	6.8	19						**6.8**	**19**	DP2	P	20.0	9.50%	6.2	110	0.3
DP2	D02	3.8	16.3	3.39	5.70	0.29	0.50	1.12	1.90	3.8	11	16.9	3.34	5.61	1.12	2.90	**3.8**	**16**	I20	P	20.0	0.95%	1.9	210	1.8
I20	D03	3.8	13.5	3.67	6.17	0.29	0.50	1.12	1.90	4.1	12	18.1	3.24	5.43	2.25	3.80	**7.3**	**21**	DP3	P	20.0	0.50%	1.4	40	0.5
DP3	D04	5.3	18.0	3.25	5.45	0.29	0.50	1.55	2.63	5.0	14	18.6	3.20	5.37	1.55	3.26	**5.0**	**17**	I24	P	20.0	0.70%	1.7	285	2.8
I21	D05	2.0	12.8	3.76	6.31	0.29	0.50	0.59	0.99	2.2	6.3	21.4	2.99	5.01	2.31	4.75	**6.9**	**24**	I26	P	20.0	0.95%	1.9	215	1.8
I22	D06	3.2	16.4	3.39	5.69	0.29	0.50	0.94	1.59	3.2	9.0						**3.2**	**9.0**	I24	P	20.0	1.40%	2.4	350	2.5
I23	D07	6.6	14.6	3.56	5.97	0.29	0.50	1.93	3.27	6.9	20						**6.9**	**20**	I23	P	20.0	0.95%	1.9	315	2.7
I24	D08	1.6	13.1	3.73	6.26	0.29	0.50	0.48	0.81	1.8	5.1	17.3	3.30	5.55	0.48	2.42	**1.8**	**13**	I25	P	20.0	0.95%	1.9	220	1.9
CB2	D09	1.6	23.0	2.88	4.83	0.26	0.44	0.42	0.71	1.2	3.4						**1.2**	**3.4**							
I25	D10	0.8	12.9	3.75	6.30	0.29	0.50	0.24	0.40	0.9	2.5	19.2	3.15	5.29	0.24	1.03	**0.9**	**5.4**							
ES1	D11	4.2	37.5	2.15	3.60	0.27	0.46	1.13	1.93	2.4	6.9						**2.4**	**6.9**							
I26	D12	2.7	19.5	3.13	5.25	0.29	0.50	0.78	1.32	2.4	6.9	23.3	2.86	4.80	0.78	3.36	**2.4**	**16**							
I27	D13	1.8	13.1	3.73	6.26	0.32	0.51	0.58	0.92	2.2	5.8						**2.2**	**5.8**	I33	P	20.0	2.40%	3.1	706	3.8
I28	D14	6.5	17.1	3.33	5.59	0.29	0.50	1.89	3.20	6.3	18						**6.3**	**18**	I31	P	20.0	2.00%	2.8	803	4.7
I29	D15	6.4	17.0	3.34	5.60	0.29	0.50	1.86	3.15	6.2	18						**6.2**	**18**	I30	P	20.0	2.25%	3.0	622	3.5
I30	D16	4.0	14.2	3.60	6.05	0.29	0.50	1.18	1.99	4.2	12	20.4	3.06	5.13	1.18	3.37	**4.2**	**17**	I31	P	20.0	0.90%	1.9	162	1.4
I31	D17	5.1	15.0	3.52	5.91	0.29	0.50	1.50	2.54	5.3	15	21.9	2.96	4.96	1.50	5.39	**5.3**	**27**	I32	P	20.0	0.50%	1.4	30	0.4

DESIGN POINT	DIRECT RUNOFF											TOTAL RUNOFF							OVERLAND TRAVEL TIME						
				I (in./ hr.)		COEFF. ©		CA		Q			I (in./ hr.)		CA		Q								
	BASIN	AREA (AC)	Tc (Min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	Sum Tc (min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	**(5 YR)**	**(100 YR)**	DESTINATION DP	CONVEYANCE TYPE	COEFFICIENT C_V	SLOPE %	VEL. (FPS)	LENGTH (FT)	TRAVEL TIME Tt
	DEVELOPED																								
I32	D18	3.1	18.3	3.22	5.41	0.29	0.50	0.92	1.55	3.0	8.4	22.2	2.93	4.92	0.92	2.06	**3.0**	**10**							
I33	E01	5.4	14.7	3.56	5.97	0.32	0.51	1.74	2.77	6.2	17	16.9	3.35	5.62	1.85	3.06	**6.2**	**17**	I34	P	20.0	1.90%	2.8	315	1.9
I34	E02	6.5	15.4	3.48	5.85	0.32	0.51	2.10	3.33	7.3	19	18.8	3.19	5.35	2.10	3.33	**7.3**	**19**	I37	P	20.0	1.40%	2.4	360	2.5
I35	E03	5.8	15.8	3.44	5.78	0.32	0.51	1.88	2.99	6.5	17						**6.5**	**17**	I36	P	20.0	0.85%	1.8	175	1.6
I36	E04	3.1	16.5	3.38	5.67	0.37	0.54	1.15	1.71	3.9	9.7	17.4	3.30	5.54	1.15	2.35	**3.9**	**13**							
I37	E05	2.6	17.7	3.27	5.50	0.32	0.51	0.83	1.31	2.7	7.2	21.3	3.00	5.03	0.83	1.73	**2.7**	**8.7**	I41	P	20.0	0.90%	1.9	280	2.5
I38	E06	1.3	12.5	3.80	6.37	0.32	0.51	0.41	0.65	1.6	4.2						**1.6**	**4.2**							
I39	E07	2.1	12.7	3.77	6.34	0.32	0.51	0.66	1.05	2.5	6.7						**2.5**	**6.7**	I41	P	20.0	2.80%	3.3	675	3.4
I40	E08	4.2	14.9	3.53	5.93	0.32	0.51	1.35	2.14	4.8	13						**4.8**	**13**	EI1	P	20.0	2.30%	3.0	1290	7.1
I41	E09	5.4	14.9	3.53	5.93	0.32	0.51	1.76	2.80	6.2	17	23.8	2.83	4.75	2.05	3.69	**6.2**	**18**	I43	P	20.0	1.10%	2.1	545	4.3
I42	E10	7.0	19.8	3.11	5.22	0.32	0.51	2.26	3.59	7.0	19						**7.0**	**19**							
CB3	E11	13.0	45.6	1.85	3.11	0.26	0.44	3.42	5.69	6.3	18						**6.3**	**18**							
I43	E12	1.6	15.3	3.49	5.86	0.64	0.74	1.02	1.19	3.6	7.0	25.6	2.72	4.56	1.02	2.02	**3.6**	**9.2**	EI3	P	20.0	1.25%	2.2	1190	8.9
EI1	E13	6.0	17.5	3.29	5.52	0.41	0.57	2.48	3.45	8.2	19	22.0	2.95	4.95	2.48	3.92	**8.2**	**19**	EI2	P	20.0	1.25%	2.2	560	4.2
EI2	Ex1	1.1	11.2	3.96	6.65	0.61	0.72	0.65	0.76	2.6	5.1	21.7	2.97	4.98	0.65	2.07	**2.6**	**10**	EI9	P	20.0	1.25%	2.2	565	4.2
EI9	Ex2	1.3	10.6	4.04	6.79	0.55	0.67	0.69	0.84	2.8	5.7	25.9	2.70	4.54	1.16	2.39	**3.1**	**11**							
EI3	Ex3	1.7	15.5	3.47	5.83	0.77	0.85	1.31	1.45	4.6	8.5	24.1	2.81	4.71	1.45	1.92	**4.6**	**9.1**	EX						
EI4	Ex4	1.8	11.8	3.88	6.52	0.53	0.65	0.96	1.18	3.7	7.7						**3.7**	**7.7**	[2] Q_5=1.2 CFS, Q_{100}=3.1 TO STONEBRIDGE FILING 3						
EI5	Ex5	2.3	14.8	3.54	5.94	0.57	0.68	1.28	1.54	4.5	9.1						**4.5**	**9.1**	EI6	P	20.0	4.00%	4.0	805	3.4
EI6	Ex6	1.4	10.4	4.07	6.83	0.47	0.60	0.65	0.83	2.6	5.7	18.2	3.23	5.43	0.94	1.32	**3.0**	**7.2**	EI7	P	20.0	4.00%	4.0	700	2.9
EI7	Ex7	1.4	10.0	4.13	6.93	0.44	0.58	0.63	0.83	2.6	5.8	21.1	3.01	5.05	0.83	1.25	**2.6**	**6.3**	EI8	P	20.0	2.00%	2.8	410	2.4
EI8	Ex8	2.2	12.0	3.86	6.47	0.38	0.54	0.85	1.21	3.3	7.8	23.5	2.85	4.78	1.01	1.59	**3.3**	**7.8**	EI9	P	20.0	2.00%	2.8	50	0.3

TYPE OF SURFACE		C_V
HEAVY MEADOW	H	3
TILLAGE/FIELD	T	5
RIPRAP (not buried)	R	7
SHORT PASTURE AND LAWNS	L	7
NEARLY BARE GROUND	B	10
GRASSED WATERWAY	G	15
PAVED AREAS	P	20

remove data not relevant to the project submitted.

STORM DRAINAGE SYSTEM DESIGN
INLET CALCULATIONS

PROJECT: **Rolling Hills Ranch PUD** Date: 2/3/2020

								Q_{Total}		$Q_{Capture}$				$Q_{Flow-by}$				DEPTH (max)		SPREAD	
DP	BASIN	Inlet size L(i)	Proposed or Existing	INLET TYPE	CROSS SLOPE	STREET SLOPE	T_c	Q_5 (cfs)	Q_{100} (cfs)	Q_5 (cfs)	Q_{100} (cfs)	$CA_{eqv.}$ (5-yr)	$CA_{eqv.}$ (100-yr)	Q_5 (cfs)	Q_{100} (cfs)	$CA_{eqv.}$ (5-yr)	$CA_{eqv.}$ (100-yr)	Q_5 (ft)	Q_{100} (ft)	Q_5 (ft)	Q_{100} (ft)
CB4	OS1	Type C	PROP	SUMP	2.0%		18.1	1.3	9	1.3	8.6	0.40	1.58	-	-	-	-	0.18	0.51		
I01	A01	15	PROP	SUMP[1]	2.0%		16.4	8.6	22	8.6	14	2.55	2.39	-	8.6	-	1.51	0.47	0.47		
I02	A02	10	PROP	SUMP	2.0%		16.9	2.0	14	2.0	14	0.60	2.54	-	-	-	-	0.50	0.70		
I03	A03	5	PROP	SUMP	2.0%		7.4	1.0	3.1	1.0	3.1	0.23	0.40	-	-	-	-	0.50	0.70		
I04	B01	10	PROP	SUMP[1]	2.0%		16.0	2.2	6.4	2.2	6.4	0.63	1.11	-	-	-	-	0.47	0.47		
I05	B02	15	PROP	SUMP[1]	2.0%		16.7	5.2	15	5.2	14	1.55	2.41	-	1.8	-	0.33	0.47	0.47		
I06	B04	20	PROP	SUMP[1]	2.0%		19.7	6.2	18	6.2	17	2.01	3.30	-	1.2	-	0.23	0.47	0.47		
I07	B05	10	PROP	SUMP[1]	2.0%		15.4	3.1	9.1	3.1	9.1	0.89	1.56	-	-	-	-	0.47	0.47		
I08	B06	10	PROP	SUMP[1]	2.0%		11.9	3.3	9.9	3.3	9.9	0.86	1.52	-	-	-	-	0.47	0.47		
I09	B07	20	PROP	FLOW-BY	2.0%	1.0%	18.0	4.3	13	3.7	9.2	1.15	1.68	0.5	3.4	0.16	0.63	0.33	0.46	12.4	18.6
I10	B08	10	PROP	SUMP[1]	2.0%		19.9	2.5	7.6	2.5	7.6	0.80	1.46	-	-	-	-	0.47	0.47		
I11	B09	10	PROP	SUMP	2.0%		16.0	2.6	7.7	2.6	7.7	0.76	1.33	-	-	-	-	0.50	0.70		
I12	B10 B11	20	PROP	SUMP	2.0%		24.0	5.4	18	5.4	18	1.92	3.73	-	-	-	-	0.50	0.70		
I13	C01	10	PROP	SUMP[1]	2.0%		15.0	3.1	9.0	3.1	9.0	0.87	1.53	-	-	-	-	0.47	0.47		
I14	C02	15	PROP	SUMP[1]	2.0%		21.2	3.4	10	3.4	10	1.14	2.04	-	-	-	-	0.47	0.47		
I15	C03	5	PROP	SUMP[1]	2.0%		13.0	1.4	4.0	1.4	4.0	0.37	0.65	-	-	-	-	0.47	0.47		
I16	C04	5	PROP	SUMP[1]	2.0%		13.1	3.2	9.4	3.2	6.3	0.85	1.01	-	3.1	-	0.50	0.47	0.47		
I17	C05	5	PROP	SUMP	2.0%		12.3	0.6	1.8	0.6	1.8	0.16	0.28	-	-	-	-	0.50	0.70		
I18	C06	5	PROP	SUMP	2.0%		14.5	1.0	6.0	1.0	6.0	0.29	1.00	-	-	-	-	0.50	0.70		
CB1	C07	Type C	PROP	SUMP	2.0%		11.8	0.9	2.5	0.9	2.5	0.22	0.39	-	-	-	-	0.13	0.27		
I19	D01	15	PROP	SUMP[1]	2.0%		16.6	6.8	19	6.8	14	2.01	2.40	-	5.7	-	1.00	0.47	0.47		
I20	D03	20	PROP	SUMP[1]	2.0%		18.1	7.3	21	7.3	17	2.25	3.17	-	3.4	-	0.63	0.47	0.47		
I21	D05	15	PROP	SUMP[1]	2.0%		21.4	6.9	24	6.9	14	2.31	2.71	-	10	-	2.04	0.47	0.47		
I22	D06	15	PROP	FLOW-BY	2.0%	1.0%	16.4	3.2	9.0	2.6	6.2	0.77	1.09	0.6	2.8	0.17	0.50	0.31	0.41	11.1	16.5
I23	D07	10	PROP	SUMP[1]	2.0%		14.6	6.9	20	6.9	9.9	1.93	1.66	-	9.6	-	1.60	0.47	0.47		
I24	D08	10	PROP	SUMP[1]	2.0%		17.3	1.8	13	1.8	9.9	0.54	1.79	-	3.5	-	0.63	0.47	0.47		

[1] Forced sump at intersection

								Q_{Total}		$Q_{Capture}$				$Q_{Flow\text{-}by}$				DEPTH (max)		SPREAD	
DP	BASIN	Inlet size L(i)	Proposed or Existing	INLET TYPE	CROSS SLOPE	STREET SLOPE	T_c	Q_5 (cfs)	Q_{100} (cfs)	Q_5 (cfs)	Q_{100} (cfs)	$CA_{eqv.}$ (5-yr)	$CA_{eqv.}$ (100-yr)	Q_5 (cfs)	Q_{100} (cfs)	$CA_{eqv.}$ (5-yr)	$CA_{eqv.}$ (100-yr)	Q_5 (ft)	Q_{100} (ft)	Q_5 (ft)	Q_{100} (ft)
CB2	D09	Type C	PROP	SUMP	2.0%		23.0	1.2	3.4	1.2	3.4	0.42	0.71	-	-	-	-	0.17	0.34		
I25	D10	10	PROP	SUMP	2.0%		19.2	0.9	5.4	0.9	5.4	0.28	1.03	-	-	-	-	0.50	0.70		
ES1	D11	FES	PROP	SUMP	2.0%		37.5	2.4	6.9	2.4	6.9	1.13	1.93	-	-	-	-	0.27	0.47		
I26	D12	20	PROP	SUMP	2.0%		23.3	2.4	16	2.4	16	0.85	3.36	-	-	-	-	0.50	0.70		
I27	D13	15	PROP	FLOW-BY	2.0%	2.0%	13.1	2.2	5.8	1.7	3.9	0.47	0.63	0.4	1.8	0.11	0.29	0.25	0.33	8.5	12.2
I28	D14	10	PROP	SUMP[1]	2.0%		17.1	6.3	18	6.3	9.9	1.89	1.78	-	7.9	-	1.42	0.47	0.47		
I29	D15	10	PROP	SUMP[1]	2.0%		17.0	6.2	18	6.2	9.9	1.86	1.77	-	7.7	-	1.37	0.47	0.47		
I30	D16	10	PROP	SUMP[1]	2.0%		20.4	4.2	17	4.2	9.9	1.39	1.94	-	7.3	-	1.43	0.47	0.47		
I31	D17	15	PROP	SUMP	2.0%		21.9	5.3	27	5.3	24	1.79	4.89	-	2.5	-	0.51	0.50	0.70		
I32	D18	15	PROP	SUMP	2.0%		22.2	3.0	10	3.0	10	1.01	2.06	-	-	-	-	0.50	0.70		
							0.0														
I33	E01	20	PROP	SUMP[1]	2.0%		16.9	6.2	17	6.2	17	1.85	3.06	-	-	-	-	0.47	0.47		
I34	E02	20	PROP	SUMP[1]	2.0%		18.8	7.3	19	7.3	17	2.29	3.22	-	2.3	-	0.42	0.47	0.47		
I35	E03	15	PROP	SUMP[1]	2.0%		15.8	6.5	17	6.5	14	1.88	2.35	-	3.7	-	0.64	0.47	0.47		
I36	E04	15	PROP	SUMP[1]	2.0%		17.4	3.9	13	3.9	13	1.18	2.35	-	-	-	-	0.47	0.47		
I37	E05	15	PROP	FLOW-BY	2.0%	1.0%	21.3	2.7	8.7	2.3	6.0	0.76	1.20	0.4	2.7	0.14	0.54	0.29	0.41	10.5	16.2
I38	E06	5	PROP	SUMP[1]	2.0%		12.5	1.6	4.2	1.6	4.2	0.41	0.65	-	-	-	-	0.47	0.47		
I39	E07	15	PROP	FLOW-BY	2.0%	2.0%	12.7	2.5	6.7	2.0	4.5	0.52	0.70	0.5	2.2	0.14	0.35	0.26	0.34	8.9	12.9
I40	E08	10	PROP	SUMP[1]	2.0%		14.9	4.8	13	4.8	9.9	1.35	1.67	-	2.8	-	0.47	0.47	0.47		
I41	E09	15	PROP	SUMP[1]	2.0%		23.8	6.2	18	6.2	14	2.20	2.86	-	3.9	-	0.83	0.47	0.47		
I42	E10	20	PROP	SUMP	2.0%		19.8	7.0	19	7.0	19	2.26	3.59	-	-	-	-	0.50	0.70		
CB3	E11	Type C	PROP	SUMP	2.0%		45.6	6.3	18	6.3	18	3.42	5.69	-	-	-	-	0.45	0.70		
I43	E12	20	PROP	FLOW-BY	2.0%	1.0%	25.6	3.6	9.2	3.2	7.1	1.19	1.55	0.4	2.1	0.13	0.47	0.32	0.42	11.6	16.6
EI1	E13	15	PROP	SUMP[1]	2.0%		22.0	8.2	19	8.2	13	2.77	2.61	-	6.5	-	1.31	0.45	0.45		
EI2	Ex1	10	PROP	FLOW-BY	2.0%	1.3%	21.7	2.6	10	1.8	5.1	0.61	1.01	0.8	5.3	0.26	1.06	0.28	0.41	9.8	16.5
EI9	Ex2	10	PROP	SUMP	2.0%		25.9	3.1	11	3.1	11	1.16	2.39	-	-	-	-	0.50	0.70		
EI3	Ex3	15	PROP	FLOW-BY	2.0%	1.0%	24.1	4.6	9.1	3.5	6.2	1.25	1.32	1.0	2.8	0.37	0.60	0.34	0.41	12.7	16.5
EI4	Ex4	10	PROP	FLOW-BY	2.0%	1.0%	11.8	3.7	7.7	2.5	4.6	0.65	0.71	1.2	3.1	0.31	0.47	0.32	0.39	11.8	15.5
EI5	Ex5	15	PROP	FLOW-BY	2.0%	1.0%	14.8	4.5	9.1	3.5	6.3	0.99	1.05	1.0	2.9	0.29	0.48	0.34	0.42	12.7	16.5
EI6	Ex6	20	PROP	FLOW-BY	2.0%	4.0%	18.2	3.0	7.2	2.4	4.9	0.74	0.90	0.6	2.3	0.20	0.42	0.25	0.32	8.4	11.6
EI7	Ex7	20	PROP	FLOW-BY	2.0%	4.0%	21.1	2.6	6.3	2.1	4.4	0.70	0.87	0.5	1.9	0.17	0.38	0.24	0.31	8.0	11.1
EI8	Ex8	15	PROP	FLOW-BY	2.0%	1.0%	23.5	3.3	7.8	2.7	5.5	0.93	1.15	0.6	2.3	0.21	0.48	0.31	0.40	11.2	15.6

[1] Forced sump at intersection

STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)
PIPE ROUTING

PROJECT: **Rolling Hills Ranch PUD** Date: 2/3/2020

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW							SYSTEM FLOW							TRAVEL TIME						
			I (in./ hr.)		CA		Q			I (in./ hr.)		CA		Q								
		Tc (Min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	Sum Tc (min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate)*	TRAVEL TIME Tt
CB4	OS1	18.1	3.2	5.4	0.40	1.58	1.3	8.6						1.3	8.6	24.0	0.013	OSJ1	1.02%	229	7.3	0.5
OSJ1									18.6	3.20	5.36	0.40	1.58	1.3	8.5	24.0	0.013	OSJ2	1.99%	396	10.2	0.6
OSJ2									19.3	3.14	5.28	0.40	1.58	1.2	8.4	24.0	0.013	OS5	4.67%	177	15.6	0.2
OS5									19.5	3.13	5.25	0.40	1.58	1.2	8.3							
I01	A01	16.4	3.39	5.68	2.55	2.39	8.6	14						8.6	14	18	0.013	J01	0.99%	65	6	0.2
I02	A02	16.9	3.34	5.61	0.60	2.54	2.0	14						2.0	14	18	0.013	J01	8.57%	4.7	17	0.0
J01									16.9	3.34	5.61	3.15	4.93	11	28	24	0.013	I03	1.01%	25	7	0.1
I03	A03	7.4	4.59	7.71	0.23	0.40	1.0	3.1	17.0	3.34	5.60	3.37	5.32	11	30	24	0.013	OS1	2.67%	172	12	0.2
									17.2													
I04	B01	16.0	3.42	5.75	0.63	1.11	2.2	6.4						2.2	6.4	18	0.013	I05	0.53%	75	4	0.3
I05	B02	16.7	3.36	5.64	1.55	2.41	5.2	14	16.7	3.36	5.64	2.18	3.52	7.3	20	24	0.013	J02	0.97%	5.2	7	0.0
J02									16.7	3.36	5.64	2.18	3.52	7.3	20	24	0.013	J03	0.51%	215	5	0.7
I06	B04	19.7	3.11	5.23	2.01	3.30	6.2	17						6.2	17	18	0.013	J03	19.34%	5.2	26	0.0
J03									19.7	3.11	5.23	4.19	6.82	13	36	30	0.013	J04	0.53%	75	6	0.2
I07	B05	15.4	3.48	5.84	0.89	1.56	3.1	9.1						3.1	9.1	18	0.013	J04	19.34%	5.2	26	0.0
J04									19.9	3.10	5.20	5.08	8.38	16	44	36	0.013	J05	0.51%	225	7	0.6
I08	B06	11.9	3.87	6.50	0.86	1.52	3.3	9.9						3.3	9.9	18	0.013	J05	19.34%	5.2	26	0.0
J05									20.4	3.06	5.13	5.94	9.90	18	51	36	0.013	J06	0.54%	64	7	0.2
I09	B07	18.0	3.24	5.45	1.15	1.68	3.7	9.2						3.7	9.2	18	0.013	J06	9.67%	5.2	19	0.0
J06									20.6	3.05	5.11	7.08	11.58	22	59	36	0.013	J07	1.29%	448	11	0.7
J07									21.3	3.00	5.03	7.08	11.58	21	58	36	0.013	J10	2.46%	407	15	0.5
I10	B08	19.9	3.09	5.19	0.80	1.46	2.5	7.6						2.5	7.6	18	0.013	J08	0.56%	54	4	0.2
J08									20.1	3.08	5.17	0.80	1.46	2.5	7.5	18	0.013	J09	0.75%	193	5	0.6
I11	B09	16.0	3.43	5.76	0.76	1.33	2.6	7.7						2.6	7.7	18	0.013	J09	0.99%	25	6	0.1
I12	B10 B11	24.0	2.82	4.73	1.92	3.73	5.4	18						5.4	18	18	0.013	J09	4.84%	5.2	13	0.0
J09									24.0	2.82	4.73	3.48	6.51	9.8	31	24	0.013	J10	0.60%	83	6	0.2
J10									24.2	2.80	4.70	10.56	18.09	30	85	42	0.013	OS2	2.06%	267	15	0.3

* Velocity estimated for calculation of travel time. Refer to Hydraulics for calculated velocity.

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW							SYSTEM FLOW							TRAVEL TIME						
			I (in./ hr.)		CA		Q			I (in./ hr.)		CA		Q								
		Tc (Min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	Sum Tc (min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	**(5 YR)**	**(100 YR)**	PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate)*	TRAVEL TIME Tt
I13	C01	15.0	3.52	5.91	0.87	1.53	3.1	9.0						**3.1**	**9.0**	24	0.013	J11	1.00%	45	7	0.1
I14	C02	21.2	3.00	5.04	1.14	2.04	3.4	10						**3.4**	**10**	24	0.013	J11	2.58%	25	12	0.0
J11									21.2	3.00	5.03	2.01	3.57	**6.0**	**18**	24	0.013	J12	1.02%	295	7	0.7
I15	C03	13.0	3.74	6.27	0.37	0.65	1.4	4.0						**1.4**	**4.0**	18	0.013	J12	1.00%	45	6	0.1
I16	C04	13.1	3.72	6.25	0.85	1.01	3.2	6.3						**3.2**	**6.3**	18	0.013	J12	1.59%	25	8	0.1
J12									21.9	2.95	4.96	3.23	5.22	**9.5**	**26**	30	0.013	J13	0.64%	165	7	0.4
I17	C05	12.3	3.82	6.41	0.16	0.28	0.6	1.8						**0.6**	**1.8**	18	0.013	J13	0.99%	25	6	0.1
I18	C06	14.5	3.57	6.00	0.29	1.00	1.0	6.0						**1.0**	**6.0**	18	0.013	J13	4.84%	5.2	13	0.0
J13									22.3	2.92	4.91	3.68	6.50	**11**	**32**	36	0.013	J14	0.98%	77	9	0.1
CB1	C07	11.8	3.88	6.51	0.22	0.39	0.9	2.5						**0.9**	**2.5**	18	0.013	J14	2.81%	68	10	0.1
J14									22.5	2.92	4.89	3.91	6.89	**11**	**34**	36	0.013	OS3	1.03%	472	10	0.8
									23.3													
I19	D01	16.6	3.37	5.66	2.01	2.40	6.8	14						**6.8**	**14**	18	0.013	J15	1.03%	54	6	0.1
J15									16.7	3.36	5.64	2.01	2.40	**6.8**	**14**	24	0.013	J16	7.30%	252	20	0.2
I20	D03	18.1	3.24	5.43	2.25	3.17	7.3	17						**7.3**	**17**	18	0.013	J16	0.99%	25	6	0.1
J16									18.1	3.24	5.43	4.26	5.57	**14**	**30**	30	0.013	J17	0.57%	331	6	0.9
I21	D05	21.4	2.99	5.01	2.31	2.71	6.9	14						**6.9**	**14**	24	0.013	J17	0.97%	5.2	7	0.0
J17									21.5	2.98	5.01	6.57	8.28	**20**	**41**	30	0.013	J19	1.02%	25	8	0.0
I22	D06	16.4	3.39	5.69	0.77	1.09	2.6	6.2						**2.6**	**6.2**	18	0.013	J18	3.01%	43	10	0.1
I23	D07	14.6	3.56	5.97	1.93	1.66	6.9	9.9						**6.9**	**9.9**	18	0.013	J18	1.00%	45	6	0.1
J18									16.4	3.38	5.68	2.70	2.75	**9.1**	**16**	24	0.013	J19	0.98%	296	7	0.7
I24	D08	17.3	3.30	5.55	0.54	1.79	1.8	9.9						**1.8**	**9.9**	24	0.013	J19	0.90%	45	7	0.1
J19									21.5	2.98	5.00	9.80	12.83	**29**	**64**	42	0.013	J20	2.78%	204	17	0.2
CB2	D09	23.0	2.88	4.83	0.42	0.71	1.2	3.4						**1.2**	**3.4**	18	0.013	I25	4.21%	32	12	0.0
I25	D10	19.2	3.15	5.29	0.28	1.03	0.9	5.4	23.1	2.88	4.83	0.70	1.74	**2.0**	**8.4**	18	0.013	J20	8.11%	25	17	0.0
ES1	D11	37.5	2.15	3.60	1.13	1.93	2.4	6.9						**2.4**	**6.9**	18	0.013	I26	0.56%	54	4	0.2
I26	D12	23.3	2.86	4.80	0.85	3.36	2.4	16	37.7	2.14	3.59	1.99	5.28	**4.3**	**19**	24	0.013	J20	1.07%	4.7	7	0.0
J20									37.7	2.14	3.59	12.49	19.85	**29**	**71**	42	0.013	J21	0.75%	510	9	0.9
I27	D13	13.1	3.73	6.26	0.47	0.63	1.7	3.9						**1.7**	**3.9**	18	0.013	J21	1.01%	30	6	0.1
J21									38.6	2.10	3.53	12.96	20.48	**29**	**72**	42	0.013	J23	2.59%	301	17	0.3
I28	D14	17.1	3.33	5.59	1.89	1.78	6.3	9.9						**6.3**	**9.9**	18	0.013	J22	1.40%	32	7	0.1
I29	D15	17.0	3.34	5.60	1.86	1.77	6.2	9.9						**6.2**	**9.9**	18	0.013	J22	0.99%	25	6	0.1
J22									17.1	3.32	5.58	3.75	3.55	**12**	**20**	24	0.013	J23	5.17%	24	16	0.0

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW							SYSTEM FLOW							TRAVEL TIME						
			I (in./ hr.)		CA		Q			I (in./ hr.)		CA		Q								
		Tc (Min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	Sum Tc (min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	**(5 YR)**	**(100 YR)**	PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate)*	TRAVEL TIME Tt
J23									38.9	2.09	3.51	16.71	24.03	**35**	**84**	48	0.013	J24	2.19%	595	17	0.6
I30	D16	20.4	3.06	5.13	1.39	1.94	4.2	9.9						**4.2**	**9.9**	18	0.013	J24	8.57%	4.7	17	0.0
J24									39.5	2.07	3.47	18.10	25.97	**37**	**90**	48	0.013	J25	1.00%	40	11	0.1
J25									39.6	2.07	3.47	18.10	25.97	**37**	**90**	48	0.013	J26	1.87%	129	16	0.1
I31	D17	21.9	2.96	4.96	1.79	4.89	5.3	24						**5.3**	**24**	30	0.013	J26	6.10%	4.9	21	0.0
J26									39.7	2.06	3.46	19.88	30.86	**41**	**107**	48	0.013	I32	1.03%	24	12	0.0
I32	D18	22.2	2.93	4.92	1.01	2.06	3.0	10	39.7	2.06	3.45	20.89	32.91	**43**	**114**	54	0.013	OS4	0.60%	195	10	0.3
									40.1													
I33	E01	16.9	3.35	5.62	1.85	3.06	6.2	17						**6.2**	**17**	18	0.013	J27	0.76%	53	5	0.2
J27									17.0	3.33	5.59	1.85	3.06	**6.2**	**17**	18	0.013	J28	2.35%	245	9	0.4
I34	E02	18.8	3.19	5.35	2.29	3.22	7.3	17.2						**7.3**	**17**	18	0.013	J28	0.99%	25	6	0.1
J28									17.5	3.29	5.53	4.15	6.28	**14**	**35**	30	0.013	J29	1.91%	175	12	0.3
I35	E03	15.8	3.44	5.78	1.88	2.35	6.5	13.6						**6.5**	**14**	18	0.013	J29	1.00%	45	6	0.1
J29									17.7	3.27	5.49	6.03	8.63	**20**	**47**	36	0.013	J30	0.70%	179	8	0.4
I36	E04	17.4	3.30	5.54	1.18	2.35	3.9	13.0						**3.9**	**13**	18	0.013	J30	1.01%	25	6	0.1
I37	E05	21.3	3.00	5.03	0.76	1.20	2.3	6.0						**2.3**	**6.0**	18	0.013	J30	5.35%	4.7	14	0.0
J30									21.3	3.00	5.03	7.97	12.18	**24**	**61**	36	0.013	J31	1.03%	44	10	0.1
J31									21.4	2.99	5.02	7.97	12.18	**24**	**61**	36	0.013	J36	0.79%	272	8	0.5
I38	E06	12.5	3.80	6.37	0.41	0.65	1.6	4.2						**1.6**	**4.2**	18	0.013	J32	1.16%	90	6	0.2
J32									12.7	3.77	6.33	0.41	0.65	**1.5**	**4.1**	18	0.013	J33	4.61%	348	13	0.5
I39	E07	12.7	3.77	6.34	0.52	0.70	2.0	4.5						**2.0**	**4.5**	18	0.013	J33	3.70%	26	11	0.0
J33									13.2	3.72	6.24	0.93	1.36	**3.5**	**8.5**	18	0.013	J34	1.95%	151	8	0.3
I40	E08	14.9	3.53	5.93	1.35	1.67	4.8	9.9						**4.8**	**9.9**	18	0.013	J34	1.04%	24	6	0.1
J34									15.0	3.53	5.92	2.28	3.03	**8.0**	**18**	24	0.013	J35	3.02%	478	13	0.6
J35									15.6	3.46	5.81	2.28	3.03	**8.0**	**18**	24	0.013	J36	1.29%	62	8	0.1
I41	E09	23.8	2.83	4.75	2.20	2.86	6.2	14						**6.2**	**14**	24	0.013	J36	1.03%	24	7	0.1
J36									23.8	2.83	4.75	12.45	18.07	**35**	**86**	42	0.013	J37	1.03%	316	11	0.5
I42	E10	19.8	3.11	5.22	2.26	3.59	7.0	19						**7.0**	**19**	24	0.013	J37	1.04%	106	7	0.2
J37									24.3	2.80	4.69	14.71	21.66	**41**	**102**	48	0.013	J38	1.22%	201	13	0.3
CB3	E11	45.6	1.85	3.11	3.42	5.69	6.3	18						**6.3**	**18**	18	0.013	J38	1.52%	112	7	0.3
I43	E12	25.6	2.72	4.56	1.19	1.55	3.2	7.1						**3.2**	**7.1**	18	0.013	J38	1.14%	13	6	0.0

* Velocity estimated for calculation of travel time. Refer to Hydraulics for calculated velocity.

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW							SYSTEM FLOW							TRAVEL TIME						
			I (in./ hr.)		CA		Q			I (in./ hr.)		CA		Q								
		Tc (Min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	Sum Tc (min.)	(5 YR)	(100 YR)	(5 YR)	(100 YR)	**(5 YR)**	**(100 YR)**	PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate)*	TRAVEL TIME Tt
J38									25.7	2.72	4.56	19.32	28.90	**52**	**132**	54	0.013	EJ01	2.39%	227	19	0.2
EI1	E13	22.0	2.95	4.95	2.77	2.61	8.2	13						**8.2**	**13**	18	0.013	EJ02	2.20%	4.5	9	0.0
CA's FROM MERIDIAN RANCH FILING 11A FDR, TIME OF CONCENTRATION ADJUSTED TO MATCH FLOW RATE FROM SCS METHOD									87.5	0.88	1.47	22.47	92.72	**20**	**136**							
EJ02									87.5	0.88	1.47	25.24	95.33	**22**	**140**	54	0.013	EJ01	0.49%	67	9	0.1

Storm Drain Hydraulics:

STORM DRAINAGE SYSTEM DESIGN
HYDRAULICS

PROJECT: **Rolling Hills Ranch PUD** Date: 2/3/2020

Label	Upstrm Node	Dnstrm Node	Intlet CA (acres)	Inlet Tc (min)	Inlet Flow (ft³/s)	System CA (acres)	System Flow Time (min)	System Intensity (in/hr)	Section Size (in)	Length (ft)	Slope (%)	Capacity (Full Flow) (ft³/s)	System Flow (ft³/s)	Velocity (Ave) (ft/s)	Elevation Ground (Upstrm) (ft)	Hydraulic Grade Line (Upstrm) (ft)	Invert (Upstrm) (ft)	Elevation Ground (Dnstrm) (ft)	Hydraulic Grade Line (Dnstrm) (ft)	Invert (Dnstrm) (ft)
P99	CB4	OSJ1	1.58	18.1	8.7	1.58	18.1	5.44	24	228.62	1.01%	23	8.7	6.7	7142.00	7137.8	7136.75	7139.93	7135.5	7134.45
P100	OSJ1	OSJ2				1.58	18.7	5.36	24	396.00	1.99%	32	8.5	8.6	7139.93	7135.5	7134.45	7132.00	7127.6	7126.55
P101	OSJ2	OS5				1.58	19.4	5.26	24	183.00	4.67%	49	8.4	11.6	7132.00	7127.6	7126.55	7123.00	7118.6	7118.00

Appendix B - HEC-HMS Data

Input Data

Rolling Hills Ranch PUD

BASIN	AREA (acre)	AREA (mi²)	CURVE NO.	LAG TIME (min)
HISTORIC				
OS05	37	0.0578	61.0	15.2
OS06	84	0.1313	61.0	18.7
OS07	21	0.0328	63.1	15.4
OS08	26	0.0406	65.7	15.9
OS09	98	0.1527	65.0	29.5
HG01	35	0.0547	61.0	19.6
HG02	58	0.0906	61.0	25.4
HG03	117	0.1828	61.1	33.8
HG04	57	0.0891	61.0	30.7
HG05	72	0.1125	61.0	31.8
HG06A	88	0.1375	61.0	43.2
HG06B	66	0.1031	61.0	49.5
HG07	63	0.0984	61.0	28.3
HG08	85	0.1328	61.0	22.9
HG09	114	0.1781	61.0	35.6
HG10	88	0.1375	61.0	61.4
HG11	131	0.2047	61.0	40.4
HG12	83	0.1297	61.0	32.0
HG13	54	0.0844	63.1	21.2
HG14	147	0.2297	61.0	45.1
HG15	164	0.2563	61.0	65.1
HG18	21	0.0328	61.0	14.1
HG19	3	0.0047	61.0	6.1
HG20	1	0.0016	61.0	6.9
HG21	14	0.0219	61.0	13.8

BASIN	AREA (acre)	AREA (mi²)	CURVE NO.	LAG TIME (min)
INTERIM				
OS05	37	0.0578	61.0	15.2
OS06	84	0.1313	61.0	18.7
OS07a	11	0.0170	63.1	13.9
OS07b	10	0.0156	63.1	10.9
OS08	25	0.0397	65.7	15.9
OS09	98	0.1527	65.0	29.5
FG01	34	0.0531	66.4	33.8
FG02	25	0.0391	64.4	16.1
FG03	13	0.0203	68.0	11.6
FG04	11	0.0172	68.0	7.6
FG05	37	0.0580	70.1	28.7
FG06	39	0.0608	65.4	18.2
FG08A	48	0.0750	76.8	13.3
FG08B	40	0.0630	76.7	16.6
FG09	31	0.0484	71.7	20.8
FG10a	52	0.0806	73.2	23.3
FG10b	27	0.0416	71.4	20.0
FG11	40	0.0625	78.2	23.2
FG12	21	0.0328	80.0	16.1
FG13	34	0.0534	66.3	29.6
FG14	64	0.1000	74.6	26.4
FG15	7	0.0103	78.6	15.6
FG16	51	0.0791	78.8	13.0
FG17a	44	0.0694	76.5	14.4
FG17b	14	0.0214	79.9	11.4
FG17c	20	0.0313	65.2	11.8
FG18	41	0.0644	73.5	29.9
FG19	34	0.0527	80.3	15.3
FG19a	5	0.0077	75.2	16.4
FG20	7	0.0109	92.9	10.1
FG22	42	0.0658	64.5	20.9
FG23a	11	0.0177	70.3	18.7
FG23b	23	0.0359	61.8	21.5
FG23c	5	0.0070	67.8	20.6
FG24	160	0.2503	61.3	32.3
FG25	70	0.1086	74.1	36.6
FG28	43	0.0673	63.0	25.6
FG29	64	0.0997	61.0	19.1
FG30	25	0.0389	61.0	12.0
FG31	59	0.0922	80.0	24.0
FG32	26	0.0402	61.0	13.6
FG33	19	0.0302	69.2	19.3
FG34	54	0.0836	61.0	20.1
FG35	38	0.0586	61.1	50.0
FG36	18	0.0281	61.6	24.3
FG37	77	0.1203	61.0	41.6

BASIN	AREA (acre)	AREA (mi²)	CURVE NO.	LAG TIME (min)
FUTURE				
OS05	37	0.0578	61.0	15.2
OS06	84	0.1313	61.0	18.7
OS07a	11	0.0170	63.1	13.9
OS07b	10	0.0156	63.1	10.9
OS08	26	0.0406	65.7	15.9
OS09	98	0.1527	65.0	29.5
FG01	34	0.0538	66.4	33.8
FG02	25	0.0391	64.6	16.1
FG03	13	0.0203	68.0	11.6
FG04	11	0.0172	68.0	7.6
FG05	37	0.0580	70.1	28.7
FG06	39	0.0608	65.4	18.4
FG08A	48	0.0750	76.8	13.3
FG08B	40	0.0630	76.7	16.6
FG09	31	0.0484	71.7	20.8
FG10a	52	0.0806	73.2	23.3
FG10b	27	0.0416	71.4	20.0
FG11	40	0.0625	78.2	23.2
FG12	21	0.0328	80.0	16.1
FG13	34	0.0534	66.3	29.6
FG14	64	0.1000	74.6	26.4
FG15	7	0.0103	78.6	15.6
FG16	51	0.0791	78.8	13.0
FG17a	44	0.0694	76.5	14.4
FG17b	14	0.0214	79.9	11.4
FG17c	20	0.0313	65.2	11.8
FG18	41	0.0644	73.5	29.9
FG19	34	0.0527	80.3	15.3
FG19a	5	0.0077	75.2	16.4
FG20	7	0.0109	92.9	10.1
FG21a	5	0.0072	63.9	10.1
FG21b	11	0.0170	78.5	15.3
FG22	88	0.1380	67.3	24.8
FG23a	14	0.0216	68.6	18.0
FG23b	18	0.0286	64.7	16.5
FG23c	8	0.0122	67.3	14.0
FG24	88	0.1373	68.1	24.9
FG25	70	0.1086	74.1	36.6
FG26	55	0.0863	70.7	23.1
FG27	32	0.0500	74.7	23.9
FG28	16	0.0245	66.6	23.0
FG29	64	0.0997	61.0	19.1
FG30	25	0.0389	80.0	10.9
FG31	59	0.0922	80.0	24.0
FG32	26	0.0402	80.0	12.1
FG33	19	0.0302	73.5	19.3
FG34	38	0.0600	62.0	23.5
FG35	22	0.0344	63.4	26.4
FG36	18	0.0281	61.0	25.0
FG37	51	0.0797	61.0	24.7

INSERT PRECIPITATION CHART

HISTORIC MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
OS06	0.1313	81	01Jul2015, 12:12	9.4
OS06-G02	0.1313	79	01Jul2015, 12:24	9.3
OS05	0.0578	40	01Jul2015, 12:12	4.2
OS05-G01	0.0578	38	01Jul2015, 12:12	4.1
HG01	0.0547	33	01Jul2015, 12:12	3.9
G01	0.1125	71	01Jul2015, 12:12	8.0
G01-G02	0.1125	70	01Jul2015, 12:24	7.9
HG02	0.0906	46	01Jul2015, 12:24	6.5
G02	0.3344	194	01Jul2015, 12:24	23.7
G02-G03	0.3344	192	01Jul2015, 12:30	23.4
HG03	0.1828	79	01Jul2015, 12:30	13.1
OS07	0.0328	25	01Jul2015, 12:12	2.6
OS07-G03	0.0328	24	01Jul2015, 12:30	2.5
G03	0.5500	295	01Jul2015, 12:30	38.9
G03-G04	0.5500	286	01Jul2015, 12:30	38.6
OS09	0.1547	92	01Jul2015, 12:24	13.3
OS09-G04	0.1547	91	01Jul2015, 12:30	13.2
HG04	0.0891	40	01Jul2015, 12:30	6.3
HG05	0.1125	50	01Jul2015, 12:30	8.0
OS08	0.0406	36	01Jul2015, 12:12	3.6
OS08-G04	0.0406	34	01Jul2015, 12:30	3.5
G04	0.9469	502	01Jul2015, 12:30	69.6
G04-G05	0.9469	496	01Jul2015, 12:36	69.3
HG06A	0.1375	50	01Jul2015, 12:42	9.7
G05	1.0844	544	01Jul2015, 12:36	79.1
G05-G06	1.0844	530	01Jul2015, 12:36	78.6
HG06B	0.1031	34	01Jul2015, 12:48	7.3
G06	1.1875	561	01Jul2015, 12:36	85.9
HG07	0.0984	47	01Jul2015, 12:24	7.0
HG07-G11	0.0984	47	01Jul2015, 12:30	7.0
HG08	0.1328	73	01Jul2015, 12:18	9.5
G11	0.2312	115	01Jul2015, 12:24	16.5
G11-G12	0.2312	114	01Jul2015, 12:30	16.3
HG09	0.1781	73	01Jul2015, 12:30	12.7
G12	0.4093	187	01Jul2015, 12:30	29.0
G12-H08	0.4093	183	01Jul2015, 12:36	28.3
HG10	0.1375	39	01Jul2015, 13:06	9.6
H08	0.5468	216	01Jul2015, 12:42	38.0
HG14	0.2297	81	01Jul2015, 12:42	16.2
HG13	0.0844	55	01Jul2015, 12:18	6.7
G07	0.0844	55	01Jul2015, 12:18	6.7
G07-G08	0.0844	54	01Jul2015, 12:18	6.6
G08	0.3141	119	01Jul2015, 12:30	22.9
HG15	0.2563	70	01Jul2015, 13:06	17.9
H13	0.2563	70	01Jul2015, 13:06	17.9
HG11	0.2047	77	01Jul2015, 12:36	14.5
H09	0.2047	77	01Jul2015, 12:36	14.5
HG12	0.1297	57	01Jul2015, 12:30	9.2
H10	0.1297	57	01Jul2015, 12:30	9.2

Highlighted green rows reference key design points (Typical all charts this section)

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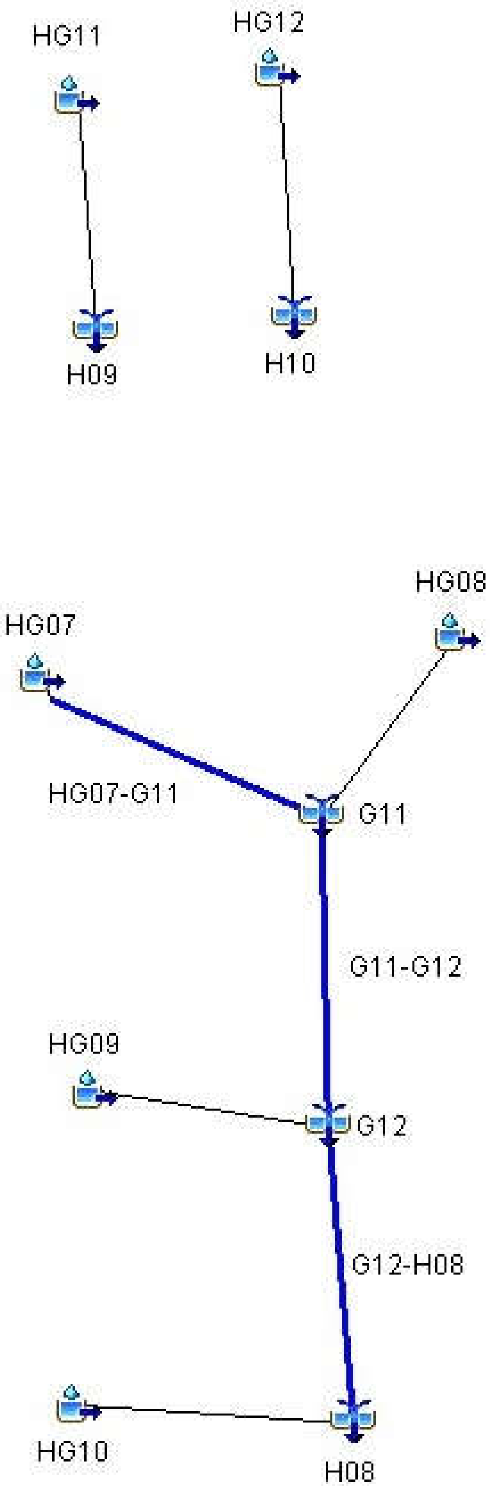

HISTORIC MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
OS06	0.1313	53	01Jul2015, 12:12	6.6
OS06-G02	0.1313	52	01Jul2015, 12:24	6.5
OS05	0.0578	26	01Jul2015, 12:12	2.9
OS05-G01	0.0578	26	01Jul2015, 12:18	2.9
HG01	0.0547	21	01Jul2015, 12:18	2.8
G01	0.1125	47	01Jul2015, 12:18	5.6
G01-G02	0.1125	47	01Jul2015, 12:24	5.5
HG02	0.0906	30	01Jul2015, 12:24	4.5
G02	0.3344	129	01Jul2015, 12:24	16.6
G02-G03	0.3344	127	01Jul2015, 12:30	16.3
HG03	0.1828	51	01Jul2015, 12:30	9.2
OS07	0.0328	17	01Jul2015, 12:12	1.9
OS07-G03	0.0328	17	01Jul2015, 12:30	1.8
G03	0.5500	195	01Jul2015, 12:30	27.3
G03-G04	0.5500	192	01Jul2015, 12:36	27.0
OS09	0.1547	64	01Jul2015, 12:24	9.7
OS09-G04	0.1547	63	01Jul2015, 12:36	9.5
HG04	0.0891	27	01Jul2015, 12:30	4.5
HG05	0.1125	33	01Jul2015, 12:30	5.6
OS08	0.0406	25	01Jul2015, 12:12	2.6
OS08-G04	0.0406	24	01Jul2015, 12:36	2.5
G04	0.9469	336	01Jul2015, 12:36	49.1
G04-G05	0.9469	322	01Jul2015, 12:42	48.9
HG06A	0.1375	33	01Jul2015, 12:42	6.8
G05	1.0844	355	01Jul2015, 12:42	55.7
G05-G06	1.0844	353	01Jul2015, 12:42	55.3
HG06B	0.1031	22	01Jul2015, 12:54	5.1
G06	1.1875	375	01Jul2015, 12:42	60.4
HG07	0.0984	31	01Jul2015, 12:24	4.9
HG07-G11	0.0984	31	01Jul2015, 12:30	4.9
HG08	0.1328	48	01Jul2015, 12:18	6.7
G11	0.2312	75	01Jul2015, 12:24	11.6
G11-G12	0.2312	75	01Jul2015, 12:30	11.4
HG09	0.1781	48	01Jul2015, 12:36	8.9
G12	0.4093	122	01Jul2015, 12:30	20.3
G12-H08	0.4093	121	01Jul2015, 12:42	19.8
HG10	0.1375	26	01Jul2015, 13:06	6.7
H08	0.5468	142	01Jul2015, 12:42	26.6
HG14	0.2297	53	01Jul2015, 12:48	11.4
HG13	0.0844	37	01Jul2015, 12:18	4.8
G07	0.0844	37	01Jul2015, 12:18	4.8
G07-G08	0.0844	37	01Jul2015, 12:24	4.7
G08	0.3141	78	01Jul2015, 12:30	16.1
HG15	0.2563	46	01Jul2015, 13:12	12.5
H13	0.2563	46	01Jul2015, 13:12	12.5
HG11	0.2047	51	01Jul2015, 12:42	10.2
H09	0.2047	51	01Jul2015, 12:42	10.2
HG12	0.1297	38	01Jul2015, 12:30	6.5
H10	0.1297	38	01Jul2015, 12:30	6.5

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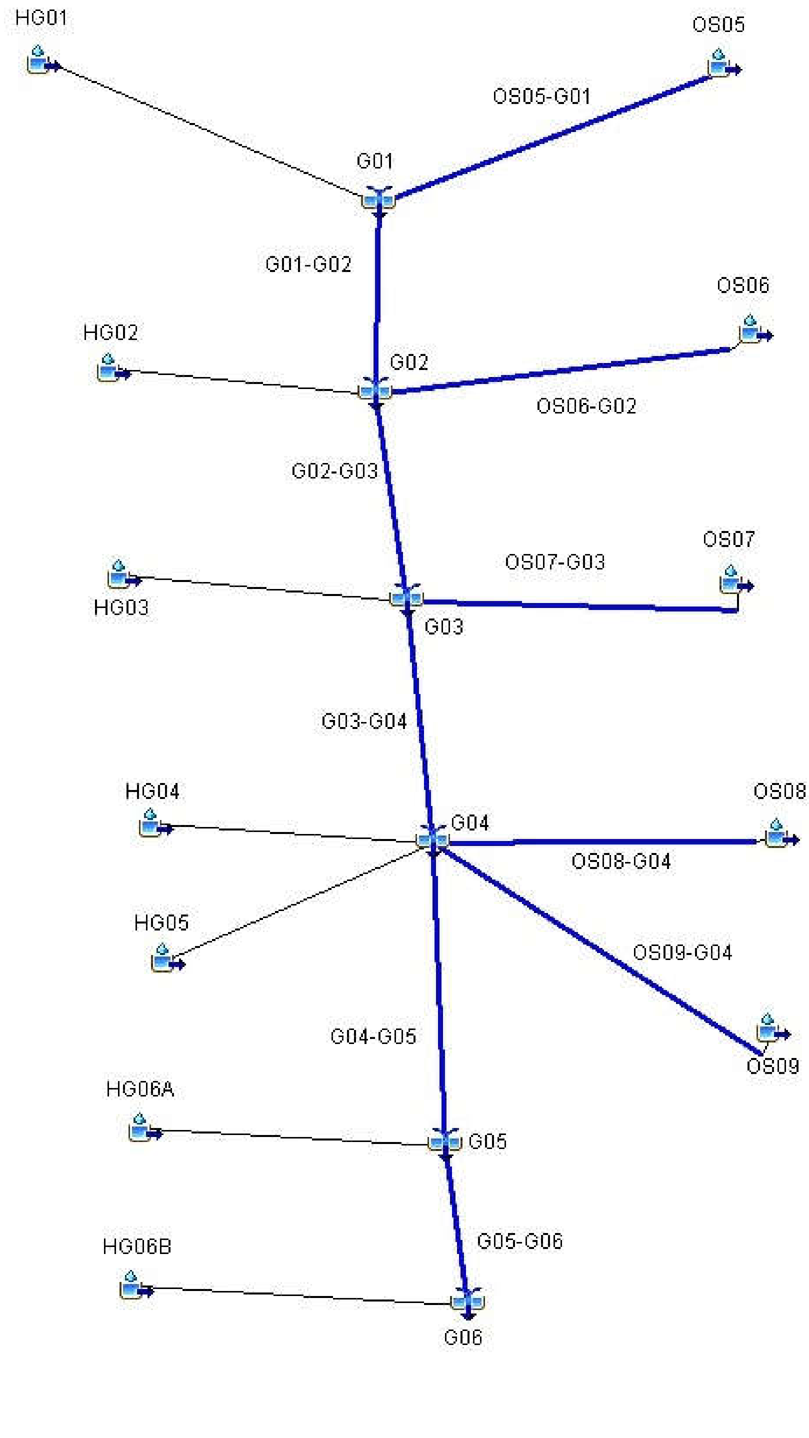
GIECK. HISTORIC



HISTORIC MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
OS06	0.1313	31	01Jul2015, 12:18	4.4
OS06-G02	0.1313	31	01Jul2015, 12:24	4.3
OS05	0.0578	16	01Jul2015, 12:12	1.9
OS05-G01	0.0578	16	01Jul2015, 12:18	1.9
HG01	0.0547	13	01Jul2015, 12:18	1.8
G01	0.1125	28	01Jul2015, 12:18	3.7
G01-G02	0.1125	27	01Jul2015, 12:24	3.7
HG02	0.0906	18	01Jul2015, 12:24	3.0
G02	0.3344	76	01Jul2015, 12:24	11.0
G02-G03	0.3344	75	01Jul2015, 12:36	10.7
HG03	0.1828	31	01Jul2015, 12:36	6.1
OS07	0.0328	11	01Jul2015, 12:12	1.3
OS07-G03	0.0328	9.9	01Jul2015, 12:36	1.2
G03	0.5500	115	01Jul2015, 12:36	18.0
G03-G04	0.5500	113	01Jul2015, 12:42	17.8
OS09	0.1547	41	01Jul2015, 12:30	6.7
OS09-G04	0.1547	41	01Jul2015, 12:36	6.5
HG04	0.0891	16	01Jul2015, 12:30	2.9
HG05	0.1125	19	01Jul2015, 12:30	3.7
OS08	0.0406	17	01Jul2015, 12:12	1.8
OS08-G04	0.0406	15	01Jul2015, 12:42	1.8
G04	0.9469	200	01Jul2015, 12:42	32.8
G04-G05	0.9469	193	01Jul2015, 12:42	32.6
HG06A	0.1375	20	01Jul2015, 12:48	4.5
G05	1.0844	212	01Jul2015, 12:42	37.1
G05-G06	1.0844	211	01Jul2015, 12:48	36.8
HG06B	0.1031	13	01Jul2015, 12:54	3.4
G06	1.1875	225	01Jul2015, 12:48	40.2
HG07	0.0984	18	01Jul2015, 12:30	3.3
HG07-G11	0.0984	18	01Jul2015, 12:30	3.2
HG08	0.1328	28	01Jul2015, 12:18	4.4
G11	0.2312	44	01Jul2015, 12:24	7.6
G11-G12	0.2312	44	01Jul2015, 12:30	7.5
HG09	0.1781	29	01Jul2015, 12:36	5.9
G12	0.4093	72	01Jul2015, 12:36	13.4
G12-H08	0.4093	71	01Jul2015, 12:48	13.0
HG10	0.1375	16	01Jul2015, 13:06	4.5
H08	0.5468	85	01Jul2015, 12:48	17.5
HG14	0.2297	32	01Jul2015, 12:48	7.5
HG13	0.0844	23	01Jul2015, 12:18	3.2
G07	0.0844	23	01Jul2015, 12:18	3.2
G07-G08	0.0844	23	01Jul2015, 12:24	3.2
G08	0.3141	48	01Jul2015, 12:36	10.7
HG15	0.2563	28	01Jul2015, 13:12	8.3
H13	0.2563	28	01Jul2015, 13:12	8.3
HG11	0.2047	30	01Jul2015, 12:42	6.7
H09	0.2047	30	01Jul2015, 12:42	6.7
HG12	0.1297	22	01Jul2015, 12:30	4.3
H10	0.1297	22	01Jul2015, 12:30	4.3

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HISTORIC MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
OS06	0.1313	12	01Jul2015, 12:18	2.2
OS06-G02	0.1313	12	01Jul2015, 12:30	2.2
OS05	0.0578	5.9	01Jul2015, 12:12	1.0
OS05-G01	0.0578	5.7	01Jul2015, 12:24	1.0
HG01	0.0547	4.8	01Jul2015, 12:18	0.9
G01	0.1125	10	01Jul2015, 12:18	1.9
G01-G02	0.1125	10	01Jul2015, 12:36	1.8
HG02	0.0906	6.9	01Jul2015, 12:30	1.5
G02	0.3344	28	01Jul2015, 12:30	5.5
G02-G03	0.3344	28	01Jul2015, 12:48	5.4
HG03	0.1828	12	01Jul2015, 12:36	3.1
OS07	0.0328	4.6	01Jul2015, 12:12	0.7
OS07-G03	0.0328	4.4	01Jul2015, 12:42	0.7
G03	0.5500	44	01Jul2015, 12:48	9.1
G03-G04	0.5500	43	01Jul2015, 12:54	9.0
OS09	0.1547	19	01Jul2015, 12:30	3.7
OS09-G04	0.1547	19	01Jul2015, 12:42	3.6
HG04	0.0891	6.1	01Jul2015, 12:36	1.5
HG05	0.1125	7.6	01Jul2015, 12:36	1.9
OS08	0.0406	7.9	01Jul2015, 12:12	1.0
OS08-G04	0.0406	7.6	01Jul2015, 12:48	1.0
G04	0.9469	78	01Jul2015, 12:48	17.0
G04-G05	0.9469	78	01Jul2015, 12:54	16.8
HG06A	0.1375	7.8	01Jul2015, 12:54	2.3
G05	1.0844	86	01Jul2015, 12:54	19.1
G05-G06	1.0844	86	01Jul2015, 13:00	18.9
HG06B	0.1031	5.4	01Jul2015, 13:00	1.7
G06	1.1875	91	01Jul2015, 13:00	20.6
HG07	0.0984	7.1	01Jul2015, 12:30	1.6
HG07-G11	0.0984	7.0	01Jul2015, 12:36	1.6
HG08	0.1328	11	01Jul2015, 12:24	2.2
G11	0.2312	17	01Jul2015, 12:30	3.9
G11-G12	0.2312	17	01Jul2015, 12:42	3.8
HG09	0.1781	11	01Jul2015, 12:42	3.0
G12	0.4093	28	01Jul2015, 12:42	6.8
G12-H08	0.4093	28	01Jul2015, 13:00	6.5
HG10	0.1375	6.5	01Jul2015, 13:18	2.2
H08	0.5468	34	01Jul2015, 13:00	8.8
HG14	0.2297	13	01Jul2015, 12:54	3.8
HG13	0.0844	9.8	01Jul2015, 12:18	1.7
G07	0.0844	9.8	01Jul2015, 12:18	1.7
G07-G08	0.0844	9.7	01Jul2015, 12:30	1.7
G08	0.3141	20	01Jul2015, 12:36	5.5
HG15	0.2563	12	01Jul2015, 13:24	4.2
H13	0.2563	12	01Jul2015, 13:24	4.2
HG11	0.2047	12	01Jul2015, 12:48	3.4
H09	0.2047	12	01Jul2015, 12:48	3.4
HG12	0.1297	8.7	01Jul2015, 12:36	2.2
H10	0.1297	8.7	01Jul2015, 12:36	2.2

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HISTORIC MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
OS06	0.1313	3.9	01Jul2015, 12:24	1.1
OS06-G02	0.1313	3.8	01Jul2015, 12:42	1.1
OS05	0.0578	1.8	01Jul2015, 12:18	0.5
OS05-G01	0.0578	1.8	01Jul2015, 12:30	0.5
HG01	0.0547	1.6	01Jul2015, 12:24	0.5
G01	0.1125	3.3	01Jul2015, 12:30	1.0
G01-G02	0.1125	3.3	01Jul2015, 12:42	0.9
HG02	0.0906	2.4	01Jul2015, 12:36	0.8
G02	0.3344	9.4	01Jul2015, 12:42	2.8
G02-G03	0.3344	9.3	01Jul2015, 13:00	2.7
HG03	0.1828	4.4	01Jul2015, 12:48	1.6
OS07	0.0328	1.7	01Jul2015, 12:18	0.4
OS07-G03	0.0328	1.7	01Jul2015, 13:00	0.4
G03	0.5500	15	01Jul2015, 13:00	4.7
G03-G04	0.5500	15	01Jul2015, 13:12	4.5
OS09	0.1547	8.5	01Jul2015, 12:36	2.1
OS09-G04	0.1547	8.5	01Jul2015, 12:48	2.0
HG04	0.0891	2.2	01Jul2015, 12:42	0.8
HG05	0.1125	2.7	01Jul2015, 12:42	1.0
OS08	0.0406	3.5	01Jul2015, 12:12	0.6
OS08-G04	0.0406	3.5	01Jul2015, 13:00	0.6
G04	0.9469	28	01Jul2015, 13:12	8.9
G04-G05	0.9469	28	01Jul2015, 13:18	8.8
HG06A	0.1375	2.9	01Jul2015, 13:00	1.2
G05	1.0844	31	01Jul2015, 13:18	9.9
G05-G06	1.0844	31	01Jul2015, 13:24	9.8
HG06B	0.1031	2.1	01Jul2015, 13:12	0.9
G06	1.1875	33	01Jul2015, 13:24	10.6
HG07	0.0984	2.4	01Jul2015, 12:42	0.8
HG07-G11	0.0984	2.4	01Jul2015, 12:48	0.8
HG08	0.1328	3.6	01Jul2015, 12:30	1.1
G11	0.2312	5.7	01Jul2015, 12:42	2.0
G11-G12	0.2312	5.6	01Jul2015, 12:54	1.9
HG09	0.1781	4.1	01Jul2015, 12:48	1.5
G12	0.4093	9.7	01Jul2015, 12:54	3.4
G12-H08	0.4093	9.7	01Jul2015, 13:18	3.3
HG10	0.1375	2.6	01Jul2015, 13:30	1.1
H08	0.5468	12	01Jul2015, 13:18	4.4
HG14	0.2297	4.8	01Jul2015, 13:06	1.9
HG13	0.0844	3.9	01Jul2015, 12:24	0.9
G07	0.0844	3.9	01Jul2015, 12:24	0.9
G07-G08	0.0844	3.8	01Jul2015, 12:36	0.9
G08	0.3141	7.6	01Jul2015, 12:54	2.8
HG15	0.2563	4.7	01Jul2015, 13:36	2.1
H13	0.2563	4.7	01Jul2015, 13:36	2.1
HG11	0.2047	4.5	01Jul2015, 13:00	1.7
H09	0.2047	4.5	01Jul2015, 13:00	1.7
HG12	0.1297	3.1	01Jul2015, 12:42	1.1
H10	0.1297	3.1	01Jul2015, 12:42	1.1

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HISTORIC MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
OS06	0.1313	0.5	01Jul2015, 13:30	0.4
OS06-G02	0.1313	0.5	01Jul2015, 14:00	0.3
OS05	0.0578	0.2	01Jul2015, 13:18	0.2
OS05-G01	0.0578	0.2	01Jul2015, 13:36	0.2
HG01	0.0547	0.2	01Jul2015, 13:30	0.1
G01	0.1125	0.5	01Jul2015, 13:36	0.3
G01-G02	0.1125	0.5	01Jul2015, 14:00	0.3
HG02	0.0906	0.4	01Jul2015, 13:42	0.2
G02	0.3344	1.4	01Jul2015, 13:54	0.9
G02-G03	0.3344	1.4	01Jul2015, 14:30	0.8
HG03	0.1828	0.8	01Jul2015, 13:48	0.5
OS07	0.0328	0.3	01Jul2015, 12:54	0.1
OS07-G03	0.0328	0.3	01Jul2015, 14:12	0.1
G03	0.5500	2.4	01Jul2015, 14:18	1.4
G03-G04	0.5500	2.4	01Jul2015, 14:36	1.3
OS09	0.1547	2.0	01Jul2015, 12:54	0.8
OS09-G04	0.1547	2.0	01Jul2015, 13:18	0.8
HG04	0.0891	0.4	01Jul2015, 13:48	0.2
HG05	0.1125	0.5	01Jul2015, 13:48	0.3
OS08	0.0406	0.8	01Jul2015, 12:24	0.2
OS08-G04	0.0406	0.8	01Jul2015, 13:36	0.2
G04	0.9469	4.9	01Jul2015, 14:30	2.9
G04-G05	0.9469	4.9	01Jul2015, 14:42	2.8
HG06A	0.1375	0.5	01Jul2015, 14:12	0.4
G05	1.0844	5.4	01Jul2015, 14:42	3.2
G05-G06	1.0844	5.4	01Jul2015, 14:54	3.1
HG06B	0.1031	0.4	01Jul2015, 14:24	0.3
G06	1.1875	5.8	01Jul2015, 14:54	3.4
HG07	0.0984	0.4	01Jul2015, 13:42	0.3
HG07-G11	0.0984	0.4	01Jul2015, 14:00	0.3
HG08	0.1328	0.5	01Jul2015, 13:36	0.4
G11	0.2312	0.9	01Jul2015, 13:48	0.6
G11-G12	0.2312	0.9	01Jul2015, 14:12	0.6
HG09	0.1781	0.7	01Jul2015, 13:54	0.5
G12	0.4093	1.6	01Jul2015, 14:06	1.0
G12-H08	0.4093	1.6	01Jul2015, 14:54	0.9
HG10	0.1375	0.5	01Jul2015, 14:42	0.3
H08	0.5468	2.1	01Jul2015, 14:48	1.3
HG14	0.2297	0.9	01Jul2015, 14:18	0.6
HG13	0.0844	0.7	01Jul2015, 13:00	0.3
G07	0.0844	0.7	01Jul2015, 13:00	0.3
G07-G08	0.0844	0.7	01Jul2015, 13:18	0.3
G08	0.3141	1.5	01Jul2015, 13:54	0.9
HG15	0.2563	0.9	01Jul2015, 14:48	0.6
H13	0.2563	0.9	01Jul2015, 14:48	0.6
HG11	0.2047	0.8	01Jul2015, 14:06	0.5
H09	0.2047	0.8	01Jul2015, 14:06	0.5
HG12	0.1297	0.5	01Jul2015, 13:48	0.3
H10	0.1297	0.5	01Jul2015, 13:48	0.3

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INTERIM MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
OS06	0.1313	80	01Jul2015, 12:12	9.3
G1a	0.1313	80	01Jul2015, 12:12	9.3
G1a-G2	0.1313	79	01Jul2015, 12:18	9.2
OS05	0.0578	39	01Jul2015, 12:12	4.1
OS05-G1	0.0578	39	01Jul2015, 12:12	4.1
FG01	0.0531	31	01Jul2015, 12:30	4.8
FG01-G1	0.0531	31	01Jul2015, 12:30	4.8
G1	0.1109	61	01Jul2015, 12:18	8.9
G1-G2	0.1109	60	01Jul2015, 12:18	8.9
FG02	0.0391	32	01Jul2015, 12:12	3.3
G2	0.2813	166	01Jul2015, 12:18	21.4
G2-G3	0.2813	163	01Jul2015, 12:18	21.2
FG03	0.0203	24	01Jul2015, 12:06	2.0
FG04	0.0172	22	01Jul2015, 12:00	1.7
G3	0.3188	184	01Jul2015, 12:18	24.9
G3-POND F	0.3188	183	01Jul2015, 12:18	24.9
FG06	0.0658	46	01Jul2015, 12:18	5.5
OS07a-POND F	0.0170	13	01Jul2015, 12:18	1.3
POND F	0.4596	177	01Jul2015, 12:42	35.8
POND F-G7	0.4596	177	01Jul2015, 12:42	35.6
FG22	0.0658	51	01Jul2015, 12:18	6.0
FG23a	0.0177	18	01Jul2015, 12:12	1.9
OS07b	0.0156	15	01Jul2015, 12:06	1.2
OS07b-G7	0.0156	13	01Jul2015, 12:24	1.2
G7	0.5587	215	01Jul2015, 12:36	44.7
G7-G10	0.5587	215	01Jul2015, 12:42	44.3
FG24	0.2503	110	01Jul2015, 12:30	17.9
OS09	0.1527	90	01Jul2015, 12:24	13.0
OS09-G10	0.1527	88	01Jul2015, 12:30	12.9
OS08	0.0397	35	01Jul2015, 12:12	3.5
OS08-G8	0.0397	34	01Jul2015, 12:24	3.4
G8	0.4427	227	01Jul2015, 12:30	34.1
G8-G10	0.4427	225	01Jul2015, 12:30	34.0
FG23b	0.0359	21	01Jul2015, 12:18	2.6
G10	1.0373	444	01Jul2015, 12:36	81.0
G10-G11	1.0373	442	01Jul2015, 12:36	80.9
FG23c	0.0070	6	01Jul2015, 12:12	0.7
G11	1.0443	445	01Jul2015, 12:36	81.6
FG25	0.1086	85	01Jul2015, 12:30	13.3
FG28	0.0673	38	01Jul2015, 12:18	5.2
POND G IN	1.2202	558	01Jul2015, 12:36	100.2
POND G	1.2202	399	01Jul2015, 13:00	91.4
G12	1.2202	399	01Jul2015, 13:00	91.4
G12-G06	1.2202	398	01Jul2015, 13:06	90.8
FG29	0.0997	60	01Jul2015, 12:12	7.1
FG32	0.0402	29	01Jul2015, 12:06	2.9
FG32-G06	0.0402	28	01Jul2015, 12:12	2.8
G06	1.3601	418	01Jul2015, 13:06	100.7
FG10A	0.0806	81	01Jul2015, 12:18	9.6
FG08A	0.0750	116	01Jul2015, 12:06	10.2

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
FG08A-G05	0.0750	110	01Jul2015, 12:12	10.2
FG08B	0.0630	86	01Jul2015, 12:12	8.5
FG08B-G05	0.0630	84	01Jul2015, 12:12	8.5
FG11	0.0625	75	01Jul2015, 12:18	8.9
FG09	0.0484	48	01Jul2015, 12:12	5.5
FG09-G05	0.0484	48	01Jul2015, 12:18	5.5
FG10B	0.0416	42	01Jul2015, 12:12	4.7
G05	0.3711	433	01Jul2015, 12:12	47.3
FG13	0.0534	34	01Jul2015, 12:24	4.8
FG12	0.0328	50	01Jul2015, 12:12	5.0
POND D IN	0.4573	509	01Jul2015, 12:12	57.0
POND D	0.4573	134	01Jul2015, 13:00	46.8
POND D-G17	0.4573	134	01Jul2015, 13:00	46.8
FG15	0.0103	15	01Jul2015, 12:06	1.5
FG15-G17A	0.0103	15	01Jul2015, 12:12	1.5
G17A	0.4676	137	01Jul2015, 13:00	48.2
FG14	0.1000	98	01Jul2015, 12:18	12.5
G17	0.5676	196	01Jul2015, 12:30	60.8
G17-G18	0.5676	196	01Jul2015, 12:36	60.7
FG16	0.0791	133	01Jul2015, 12:06	11.5
G18	0.6467	240	01Jul2015, 12:24	72.2
G18-POND E	0.6467	240	01Jul2015, 12:24	72.2
FG31	0.0922	116	01Jul2015, 12:18	13.9
FG30	0.0389	30	01Jul2015, 12:06	2.8
FG30-PONDHS	0.0389	28	01Jul2015, 12:18	2.7
POND HS	0.1311	112	01Jul2015, 12:30	16.6
FG17a	0.0694	101	01Jul2015, 12:06	9.4
FG17a-POND E	0.0694	99	01Jul2015, 12:06	9.4
FG18	0.0644	56	01Jul2015, 12:24	7.8
FG18-POND E	0.0644	56	01Jul2015, 12:24	7.8
FG19	0.0527	84	01Jul2015, 12:06	8.1
FG17c	0.0313	31	01Jul2015, 12:06	2.7
FG17b	0.0214	39	01Jul2015, 12:06	3.2
POND E IN	1.0170	552	01Jul2015, 12:12	119.8
POND E	1.0170	233	01Jul2015, 13:36	95.8
H08	1.0170	200	01Jul2015, 13:36	83.8
H09	0.0000	34	01Jul2015, 13:36	12.0
FG34	0.0836	48	01Jul2015, 12:18	5.9
G14	0.0836	48	01Jul2015, 12:18	5.9
G14-G15	0.0836	48	01Jul2015, 12:18	5.9
FG35	0.0586	19	01Jul2015, 12:48	4.1
G15	0.1422	60	01Jul2015, 12:24	10.0
G15-G08	0.1422	59	01Jul2015, 12:24	9.9
FG37	0.1203	44	01Jul2015, 12:42	8.4
FG36	0.0281	16	01Jul2015, 12:18	2.0
FG36-G08	0.0281	15	01Jul2015, 12:24	2.0
G08	0.2906	114	01Jul2015, 12:30	20.3

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
OS06	0.1313	52	01Jul2015, 12:12	6.5
G1a	0.1313	52	01Jul2015, 12:12	6.5
G1a-G2	0.1313	52	01Jul2015, 12:18	6.5
OS05	0.0578	26	01Jul2015, 12:12	2.9
OS05-G1	0.0578	25	01Jul2015, 12:12	2.9
FG01	0.0531	22	01Jul2015, 12:30	3.5
FG01-G1	0.0531	22	01Jul2015, 12:30	3.5
G1	0.1109	41	01Jul2015, 12:18	6.4
G1-G2	0.1109	41	01Jul2015, 12:18	6.4
FG02	0.0391	22	01Jul2015, 12:12	2.4
G2	0.2813	112	01Jul2015, 12:18	15.2
G2-G3	0.2813	108	01Jul2015, 12:24	15.1
FG03	0.0203	17	01Jul2015, 12:06	1.5
FG04	0.0172	16	01Jul2015, 12:00	1.3
G3	0.3188	123	01Jul2015, 12:18	17.8
G3-POND F	0.3188	121	01Jul2015, 12:18	17.8
FG06	0.0658	32	01Jul2015, 12:18	4.0
OS07a-POND F	0.0170	9	01Jul2015, 12:18	0.9
POND F	0.4596	121	01Jul2015, 12:42	25.7
POND F-G7	0.4596	120	01Jul2015, 12:48	25.5
FG22	0.0658	36	01Jul2015, 12:18	4.4
FG23a	0.0177	13	01Jul2015, 12:12	1.4
OS07b	0.0156	10.0	01Jul2015, 12:06	0.9
OS07b-G7	0.0156	9	01Jul2015, 12:24	0.9
G7	0.5587	145	01Jul2015, 12:42	32.2
G7-G10	0.5587	144	01Jul2015, 12:48	31.9
FG24	0.2503	73	01Jul2015, 12:30	12.6
OS09	0.1527	62	01Jul2015, 12:24	9.4
OS09-G10	0.1527	61	01Jul2015, 12:36	9.3
OS08	0.0397	24	01Jul2015, 12:12	2.6
OS08-G8	0.0397	24	01Jul2015, 12:24	2.5
G8	0.4427	154	01Jul2015, 12:30	24.4
G8-G10	0.4427	151	01Jul2015, 12:30	24.3
FG23b	0.0359	14	01Jul2015, 12:18	1.9
G10	1.0373	287	01Jul2015, 12:42	58.1
G10-G11	1.0373	285	01Jul2015, 12:42	58.0
FG23c	0.0070	4.2	01Jul2015, 12:18	0.5
G11	1.0443	287	01Jul2015, 12:42	58.5
FG25	0.1086	64	01Jul2015, 12:30	10.2
FG28	0.0673	26	01Jul2015, 12:24	3.7
POND G IN	1.2202	363	01Jul2015, 12:42	72.5
POND G	1.2202	237	01Jul2015, 13:12	64.3
G12	1.2202	237	01Jul2015, 13:12	64.3
G12-G06	1.2202	236	01Jul2015, 13:18	63.8
FG29	0.0997	39	01Jul2015, 12:18	5.0
FG32	0.0402	19	01Jul2015, 12:12	2.0
FG32-G06	0.0402	19	01Jul2015, 12:12	2.0
G06	1.3601	248	01Jul2015, 13:18	70.8
FG10A	0.0806	61	01Jul2015, 12:18	7.3
FG08A	0.0750	90	01Jul2015, 12:06	7.9

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
FG08A-G05	0.0750	86	01Jul2015, 12:12	7.9
FG08B	0.0630	67	01Jul2015, 12:12	6.6
FG08B-G05	0.0630	65	01Jul2015, 12:12	6.6
FG11	0.0625	59	01Jul2015, 12:18	7.0
FG09	0.0484	36	01Jul2015, 12:12	4.1
FG09-G05	0.0484	36	01Jul2015, 12:18	4.1
FG10B	0.0416	31	01Jul2015, 12:12	3.5
G05	0.3711	330	01Jul2015, 12:12	36.4
FG13	0.0534	24	01Jul2015, 12:24	3.5
FG12	0.0328	40	01Jul2015, 12:12	3.9
POND D IN	0.4573	387	01Jul2015, 12:12	43.9
POND D	0.4573	91	01Jul2015, 13:06	34.7
POND D-G17	0.4573	91	01Jul2015, 13:06	34.6
FG15	0.0103	12	01Jul2015, 12:12	1.2
FG15-G17A	0.0103	12	01Jul2015, 12:12	1.2
G17A	0.4676	93	01Jul2015, 13:06	35.8
FG14	0.1000	74	01Jul2015, 12:18	9.6
G17	0.5676	132	01Jul2015, 12:36	45.4
G17-G18	0.5676	131	01Jul2015, 12:36	45.4
FG16	0.0791	104	01Jul2015, 12:06	9.0
G18	0.6467	178	01Jul2015, 12:12	54.4
G18-POND E	0.6467	176	01Jul2015, 12:12	54.4
FG31	0.0922	92	01Jul2015, 12:18	11.0
FG30	0.0389	20	01Jul2015, 12:06	1.9
FG30-PONDHS	0.0389	19	01Jul2015, 12:18	1.9
POND HS	0.1311	63	01Jul2015, 12:36	12.9
FG17a	0.0694	78	01Jul2015, 12:06	7.3
FG17a-POND E	0.0694	76	01Jul2015, 12:06	7.3
FG18	0.0644	42	01Jul2015, 12:24	5.9
FG18-POND E	0.0644	42	01Jul2015, 12:24	5.9
FG19	0.0527	66	01Jul2015, 12:06	6.4
FG17c	0.0313	22	01Jul2015, 12:06	2.0
FG17b	0.0214	31	01Jul2015, 12:06	2.6
POND E IN	1.0170	423	01Jul2015, 12:12	91.3
POND E	1.0170	144	01Jul2015, 14:00	68.1
H08	1.0170	130	01Jul2015, 14:00	59.8
H09	0.0000	14	01Jul2015, 14:00	8.3
FG34	0.0836	32	01Jul2015, 12:18	4.2
G14	0.0836	32	01Jul2015, 12:18	4.2
G14-G15	0.0836	32	01Jul2015, 12:24	4.1
FG35	0.0586	13	01Jul2015, 12:54	2.9
G15	0.1422	39	01Jul2015, 12:24	7.0
G15-G08	0.1422	39	01Jul2015, 12:30	6.9
FG37	0.1203	29	01Jul2015, 12:42	5.9
FG36	0.0281	10	01Jul2015, 12:18	1.4
FG36-G08	0.0281	10	01Jul2015, 12:24	1.4
G08	0.2906	75	01Jul2015, 12:30	14.3

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
OS06	0.1313	30	01Jul2015, 12:18	4.3
G1a	0.1313	30	01Jul2015, 12:18	4.3
G1a-G2	0.1313	30	01Jul2015, 12:18	4.2
OS05	0.0578	15	01Jul2015, 12:12	1.9
OS05-G1	0.0578	15	01Jul2015, 12:12	1.9
FG01	0.0531	14	01Jul2015, 12:30	2.4
FG01-G1	0.0531	14	01Jul2015, 12:30	2.4
G1	0.1109	25	01Jul2015, 12:18	4.3
G1-G2	0.1109	25	01Jul2015, 12:24	4.3
FG02	0.0391	14	01Jul2015, 12:12	1.6
G2	0.2813	67	01Jul2015, 12:18	10.2
G2-G3	0.2813	66	01Jul2015, 12:24	10.1
FG03	0.0203	12	01Jul2015, 12:06	1.0
FG04	0.0172	11	01Jul2015, 12:00	0.9
G3	0.3188	74	01Jul2015, 12:24	12.0
G3-POND F	0.3188	74	01Jul2015, 12:24	12.0
FG06	0.0658	20	01Jul2015, 12:18	2.7
OS07a-POND F	0.0170	6	01Jul2015, 12:24	0.6
POND F	0.4596	60	01Jul2015, 12:54	17.2
POND F-G7	0.4596	60	01Jul2015, 13:00	17.1
FG22	0.0658	24	01Jul2015, 12:18	3.1
FG23a	0.0177	9	01Jul2015, 12:12	1.0
OS07b	0.0156	6.2	01Jul2015, 12:06	0.6
OS07b-G7	0.0156	5	01Jul2015, 12:30	0.6
G7	0.5587	72	01Jul2015, 13:00	21.8
G7-G10	0.5587	72	01Jul2015, 13:06	21.5
FG24	0.2503	43	01Jul2015, 12:30	8.3
OS09	0.1527	39	01Jul2015, 12:30	6.5
OS09-G10	0.1527	39	01Jul2015, 12:36	6.3
OS08	0.0397	16	01Jul2015, 12:12	1.8
OS08-G8	0.0397	15	01Jul2015, 12:24	1.7
G8	0.4427	94	01Jul2015, 12:30	16.3
G8-G10	0.4427	94	01Jul2015, 12:36	16.2
FG23b	0.0359	8	01Jul2015, 12:18	1.2
G10	1.0373	146	01Jul2015, 12:48	39.0
G10-G11	1.0373	145	01Jul2015, 12:48	39.0
FG23c	0.0070	2.8	01Jul2015, 12:18	0.4
G11	1.0443	146	01Jul2015, 12:48	39.3
FG25	0.1086	46	01Jul2015, 12:30	7.5
FG28	0.0673	15.9	01Jul2015, 12:24	2.5
POND G IN	1.2202	202	01Jul2015, 12:36	49.3
POND G	1.2202	118	01Jul2015, 13:36	41.8
G12	1.2202	118	01Jul2015, 13:36	41.8
G12-G06	1.2202	117	01Jul2015, 13:42	41.5
FG29	0.0997	23	01Jul2015, 12:18	3.3
FG32	0.0402	11	01Jul2015, 12:12	1.3
FG32-G06	0.0402	11	01Jul2015, 12:12	1.3
G06	1.3601	124	01Jul2015, 13:36	46.0
FG10A	0.0806	43	01Jul2015, 12:18	5.4
FG08A	0.0750	66	01Jul2015, 12:06	5.9

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
FG08A-G05	0.0750	64	01Jul2015, 12:12	5.9
FG08B	0.0630	49	01Jul2015, 12:12	5.0
FG08B-G05	0.0630	48	01Jul2015, 12:12	4.9
FG11	0.0625	44	01Jul2015, 12:18	5.2
FG09	0.0484	25	01Jul2015, 12:18	3.0
FG09-G05	0.0484	25	01Jul2015, 12:18	3.0
FG10B	0.0416	22	01Jul2015, 12:12	2.5
G05	0.3711	239	01Jul2015, 12:12	26.9
FG13	0.0534	15	01Jul2015, 12:24	2.4
FG12	0.0328	30	01Jul2015, 12:12	3.0
POND D IN	0.4573	280	01Jul2015, 12:12	32.4
POND D	0.4573	50	01Jul2015, 13:24	24.3
POND D-G17	0.4573	50	01Jul2015, 13:24	24.3
FG15	0.0103	9.0	01Jul2015, 12:12	0.9
FG15-G17A	0.0103	9.0	01Jul2015, 12:12	0.9
G17A	0.4676	51	01Jul2015, 13:24	25.2
FG14	0.1000	53	01Jul2015, 12:18	7.1
G17	0.5676	75	01Jul2015, 12:24	32.3
G17-G18	0.5676	75	01Jul2015, 12:24	32.2
FG16	0.0791	78	01Jul2015, 12:06	6.8
G18	0.6467	128	01Jul2015, 12:12	39.1
G18-POND E	0.6467	126	01Jul2015, 12:12	39.0
FG31	0.0922	69	01Jul2015, 12:18	8.4
FG30	0.0389	12	01Jul2015, 12:06	1.3
FG30-PONDHS	0.0389	11	01Jul2015, 12:18	1.3
POND HS	0.1311	40	01Jul2015, 12:42	9.6
FG17a	0.0694	57	01Jul2015, 12:06	5.4
FG17a-POND E	0.0694	56	01Jul2015, 12:12	5.4
FG18	0.0644	30	01Jul2015, 12:24	4.3
FG18-POND E	0.0644	30	01Jul2015, 12:24	4.3
FG19	0.0527	50	01Jul2015, 12:06	4.9
FG17c	0.0313	14	01Jul2015, 12:06	1.4
FG17b	0.0214	24	01Jul2015, 12:06	1.9
POND E IN	1.0170	308	01Jul2015, 12:12	66.5
POND E	1.0170	74	01Jul2015, 14:42	44.4
H08	1.0170	66	01Jul2015, 14:42	37.8
H09	0.0000	7.9	01Jul2015, 14:42	6.6
FG34	0.0836	19	01Jul2015, 12:18	2.7
G14	0.0836	19	01Jul2015, 12:18	2.7
G14-G15	0.0836	18	01Jul2015, 12:24	2.7
FG35	0.0586	7	01Jul2015, 12:54	1.9
G15	0.1422	23	01Jul2015, 12:30	4.6
G15-G08	0.1422	23	01Jul2015, 12:30	4.5
FG37	0.1203	17	01Jul2015, 12:42	3.9
FG36	0.0281	6	01Jul2015, 12:24	1.0
FG36-G08	0.0281	6	01Jul2015, 12:30	0.9
G08	0.2906	44	01Jul2015, 12:36	9.3

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
OS06	0.1313	12	01Jul2015, 12:18	2.2
G1a	0.1313	12	01Jul2015, 12:18	2.2
G1a-G2	0.1313	11	01Jul2015, 12:24	2.1
OS05	0.0578	6	01Jul2015, 12:12	1.0
OS05-G1	0.0578	6	01Jul2015, 12:18	1.0
FG01	0.0531	7	01Jul2015, 12:36	1.4
FG01-G1	0.0531	7	01Jul2015, 12:36	1.4
G1	0.1109	11	01Jul2015, 12:24	2.3
G1-G2	0.1109	11	01Jul2015, 12:30	2.3
FG02	0.0391	6	01Jul2015, 12:12	0.9
G2	0.2813	27	01Jul2015, 12:24	5.3
G2-G3	0.2813	27	01Jul2015, 12:30	5.3
FG03	0.0203	6	01Jul2015, 12:06	0.6
FG04	0.0172	6	01Jul2015, 12:06	0.5
G3	0.3188	31	01Jul2015, 12:30	6.4
G3-POND F	0.3188	31	01Jul2015, 12:30	6.4
FG06	0.0658	9	01Jul2015, 12:18	1.5
OS07a-POND F	0.0170	2	01Jul2015, 12:30	0.3
POND F	0.4596	17	01Jul2015, 13:48	9.3
POND F-G7	0.4596	17	01Jul2015, 13:54	9.2
FG22	0.0658	12	01Jul2015, 12:18	1.7
FG23a	0.0177	5	01Jul2015, 12:12	0.6
OS07b	0.0156	3	01Jul2015, 12:06	0.3
OS07b-G7	0.0156	2.3	01Jul2015, 12:36	0.3
G7	0.5587	22	01Jul2015, 12:54	11.9
G7-G10	0.5587	22	01Jul2015, 13:06	11.7
FG24	0.2503	17.0	01Jul2015, 12:36	4.2
OS09	0.1527	18	01Jul2015, 12:30	3.5
OS09-G10	0.1527	18	01Jul2015, 12:42	3.5
OS08	0.0397	8	01Jul2015, 12:12	1.0
OS08-G8	0.0397	7	01Jul2015, 12:30	1.0
G8	0.4427	41	01Jul2015, 12:36	8.6
G8-G10	0.4427	40.8	01Jul2015, 12:42	8.6
FG23b	0.0359	3.3	01Jul2015, 12:24	0.6
G10	1.0373	60	01Jul2015, 12:48	20.9
G10-G11	1.0373	60	01Jul2015, 12:48	20.9
FG23c	0.0070	1.4	01Jul2015, 12:18	0.2
G11	1.0443	61	01Jul2015, 12:48	21.1
FG25	0.1086	27	01Jul2015, 12:36	4.7
FG28	0.0673	6.7	01Jul2015, 12:24	1.3
POND G IN	1.2202	91	01Jul2015, 12:42	27.1
POND G	1.2202	35	01Jul2015, 14:48	20.9
G12	1.2202	35	01Jul2015, 14:48	20.9
G12-G06	1.2202	35	01Jul2015, 15:00	20.7
FG29	0.0997	8.7	01Jul2015, 12:18	1.6
FG32	0.0402	4	01Jul2015, 12:12	0.7
FG32-G06	0.0402	4	01Jul2015, 12:18	0.7
G06	1.3601	37	01Jul2015, 14:54	23.0
FG10A	0.0806	25	01Jul2015, 12:18	3.3
FG08A	0.0750	41	01Jul2015, 12:06	3.8

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
FG08A-G05	0.0750	41	01Jul2015, 12:12	3.8
FG08B	0.0630	31	01Jul2015, 12:12	3.2
FG08B-G05	0.0630	29	01Jul2015, 12:18	3.2
FG11	0.0625	28	01Jul2015, 12:18	3.4
FG09	0.0484	14	01Jul2015, 12:18	1.8
FG09-G05	0.0484	14	01Jul2015, 12:18	1.8
FG10B	0.0416	12	01Jul2015, 12:18	1.5
G05	0.3711	145	01Jul2015, 12:18	17.1
FG13	0.0534	7.5	01Jul2015, 12:30	1.4
FG12	0.0328	20	01Jul2015, 12:12	2.0
POND D IN	0.4573	168	01Jul2015, 12:18	20.4
POND D	0.4573	19	01Jul2015, 14:24	14.8
POND D-G17	0.4573	19	01Jul2015, 14:24	14.8
FG15	0.0103	5.8	01Jul2015, 12:12	0.6
FG15-G17A	0.0103	5.8	01Jul2015, 12:12	0.6
G17A	0.4676	19	01Jul2015, 14:12	15.3
FG14	0.1000	32	01Jul2015, 12:24	4.5
G17	0.5676	43	01Jul2015, 12:24	19.8
G17-G18	0.5676	43	01Jul2015, 12:30	19.8
FG16	0.0791	50	01Jul2015, 12:06	4.5
G18	0.6467	79	01Jul2015, 12:12	24.3
G18-POND E	0.6467	78	01Jul2015, 12:12	24.3
FG31	0.0922	45	01Jul2015, 12:18	5.6
FG30	0.0389	4	01Jul2015, 12:12	0.7
FG30-PONDHS	0.0389	4	01Jul2015, 12:24	0.6
POND HS	0.1311	28	01Jul2015, 12:42	6.2
FG17a	0.0694	35	01Jul2015, 12:06	3.5
FG17a-POND E	0.0694	35	01Jul2015, 12:12	3.5
FG18	0.0644	18	01Jul2015, 12:24	2.7
FG18-POND E	0.0644	17	01Jul2015, 12:30	2.7
FG19	0.0527	33	01Jul2015, 12:12	3.3
FG17c	0.0313	6.5	01Jul2015, 12:06	0.8
FG17b	0.0214	16	01Jul2015, 12:06	1.3
POND E IN	1.0170	190	01Jul2015, 12:12	42.0
POND E	1.0170	28	01Jul2015, 18:00	22.9
H08	1.0170	22	01Jul2015, 18:00	17.8
H09	0.0000	5.7	01Jul2015, 18:00	5.1
FG34	0.0836	7	01Jul2015, 12:18	1.4
G14	0.0836	7	01Jul2015, 12:18	1.4
G14-G15	0.0836	7	01Jul2015, 12:30	1.4
FG35	0.0586	3	01Jul2015, 13:00	1.0
G15	0.1422	9	01Jul2015, 12:36	2.3
G15-G08	0.1422	9	01Jul2015, 12:36	2.3
FG37	0.1203	7	01Jul2015, 12:48	2.0
FG36	0.0281	2	01Jul2015, 12:24	0.5
FG36-G08	0.0281	2	01Jul2015, 12:36	0.5
G08	0.2906	18	01Jul2015, 12:42	4.7

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
OS06	0.1313	3.8	01Jul2015, 12:24	1.1
G1a	0.1313	3.8	01Jul2015, 12:24	1.1
G1a-G2	0.1313	3.6	01Jul2015, 12:36	1.1
OS05	0.0578	1.8	01Jul2015, 12:18	0.5
OS05-G1	0.0578	1.7	01Jul2015, 12:24	0.5
FG01	0.0531	3.3	01Jul2015, 12:36	0.8
FG01-G1	0.0531	3.3	01Jul2015, 12:36	0.8
G1	0.1109	4.8	01Jul2015, 12:36	1.3
G1-G2	0.1109	4.8	01Jul2015, 12:36	1.3
FG02	0.0391	2.6	01Jul2015, 12:18	0.5
G2	0.2813	10	01Jul2015, 12:30	2.9
G2-G3	0.2813	10	01Jul2015, 12:42	2.8
FG03	0.0203	0.8	01Jul2015, 12:12	0.2
FG04	0.0172	3.1	01Jul2015, 12:06	0.3
G3	0.3188	11	01Jul2015, 12:36	3.3
G3-POND F	0.3188	11	01Jul2015, 12:42	3.3
FG06	0.0658	3.9	01Jul2015, 12:24	0.8
OS07a-POND F	0.0170	0.9	01Jul2015, 12:36	0.2
POND F	0.4596	8.0	01Jul2015, 14:12	5.0
POND F-G7	0.4596	8.0	01Jul2015, 14:18	4.9
FG22	0.0658	5.6	01Jul2015, 12:18	1.0
FG23a	0.0177	2.7	01Jul2015, 12:18	0.4
OS07b	0.0156	1.0	01Jul2015, 12:12	0.2
OS07b-G7	0.0156	0.9	01Jul2015, 12:42	0.2
G7	0.5587	10.0	01Jul2015, 14:00	6.5
G7-G10	0.5587	10.0	01Jul2015, 14:12	6.4
FG24	0.2503	6.2	01Jul2015, 12:42	2.2
OS09	0.1527	8.2	01Jul2015, 12:36	2.0
OS09-G10	0.1527	8.2	01Jul2015, 12:48	2.0
OS08	0.0397	3.4	01Jul2015, 12:12	0.6
OS08-G8	0.0397	3.3	01Jul2015, 12:36	0.6
G8	0.4427	17.1	01Jul2015, 12:42	4.7
G8-G10	0.4427	17.1	01Jul2015, 12:48	4.7
FG23b	0.0359	1.2	01Jul2015, 12:30	0.3
G10	1.0373	25	01Jul2015, 12:48	11.4
G10-G11	1.0373	25	01Jul2015, 12:54	11.4
FG23c	0.0070	0.7	01Jul2015, 12:18	0.1
G11	1.0443	25	01Jul2015, 12:54	11.5
FG25	0.1086	17	01Jul2015, 12:36	3.1
FG28	0.0673	2.7	01Jul2015, 12:30	0.7
POND G IN	1.2202	42	01Jul2015, 12:48	15.3
POND G	1.2202	13	01Jul2015, 17:36	9.7
G12	1.2202	13	01Jul2015, 17:36	9.7
G12-G06	1.2202	13	01Jul2015, 17:42	9.5
FG29	0.0997	2.8	01Jul2015, 12:24	0.9
FG32	0.0402	1	01Jul2015, 12:18	0.3
FG32-G06	0.0402	1	01Jul2015, 12:24	0.3
G06	1.3601	14	01Jul2015, 17:42	10.7
FG10A	0.0806	15	01Jul2015, 12:18	2.2
FG08A	0.0750	27	01Jul2015, 12:06	2.6

Highlighted green rows reference key design points (Typical all charts this section)

INTERIM MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
FG08A-G05	0.0750	27	01Jul2015, 12:12	2.6
FG08B	0.0630	20	01Jul2015, 12:12	2.2
FG08B-G05	0.0630	19	01Jul2015, 12:18	2.2
FG11	0.0625	19	01Jul2015, 12:18	2.4
FG09	0.0484	8	01Jul2015, 12:18	1.2
FG09-G05	0.0484	8	01Jul2015, 12:24	1.2
FG10B	0.0416	7	01Jul2015, 12:18	1.0
G05	0.3711	93	01Jul2015, 12:18	11.4
FG13	0.0534	4	01Jul2015, 12:30	0.8
FG12	0.0328	14	01Jul2015, 12:12	1.4
POND D IN	0.4573	107	01Jul2015, 12:18	13.6
POND D	0.4573	12	01Jul2015, 14:36	9.2
POND D-G17	0.4573	12	01Jul2015, 14:42	9.2
FG15	0.0103	4	01Jul2015, 12:12	0.4
FG15-G17A	0.0103	4	01Jul2015, 12:12	0.4
G17A	0.4676	12	01Jul2015, 14:36	9.6
FG14	0.1000	20	01Jul2015, 12:24	3.0
G17	0.5676	25	01Jul2015, 12:24	12.5
G17-G18	0.5676	25	01Jul2015, 12:24	12.5
FG16	0.0791	34	01Jul2015, 12:06	3.1
G18	0.6467	51	01Jul2015, 12:12	15.7
G18-POND E	0.6467	50	01Jul2015, 12:12	15.6
FG31	0.0922	31	01Jul2015, 12:18	3.9
FG30	0.0389	1	01Jul2015, 12:12	0.3
FG30-PONDHS	0.0389	1	01Jul2015, 12:36	0.3
POND HS	0.1311	19	01Jul2015, 12:42	4.3
FG17a	0.0694	23	01Jul2015, 12:12	2.4
FG17a-POND E	0.0694	23	01Jul2015, 12:12	2.4
FG18	0.0644	11	01Jul2015, 12:30	1.8
FG18-POND E	0.0644	11	01Jul2015, 12:30	1.8
FG19	0.0527	23	01Jul2015, 12:12	2.3
FG17c	0.0313	3	01Jul2015, 12:12	0.4
FG17b	0.0214	11	01Jul2015, 12:06	0.9
POND E IN	1.0170	123	01Jul2015, 12:12	27.7
POND E	1.0170	15	01Jul2015, 20:18	12.1
H08	1.0170	11	01Jul2015, 20:18	8.6
H09	0.0000	4	01Jul2015, 20:18	3.5
FG34	0.0836	2	01Jul2015, 12:24	0.7
G14	0.0836	2	01Jul2015, 12:24	0.7
G14-G15	0.0836	2	01Jul2015, 12:42	0.7
FG35	0.0586	1	01Jul2015, 13:12	0.5
G15	0.1422	3	01Jul2015, 12:48	1.2
G15-G08	0.1422	3	01Jul2015, 12:54	1.2
FG37	0.1203	3	01Jul2015, 13:00	1.0
FG36	0.0281	1	01Jul2015, 12:30	0.3
FG36-G08	0.0281	1	01Jul2015, 12:42	0.3
G08	0.2906	6	01Jul2015, 12:54	2.4

Highlighted green rows reference key design points (Typical all charts this section)

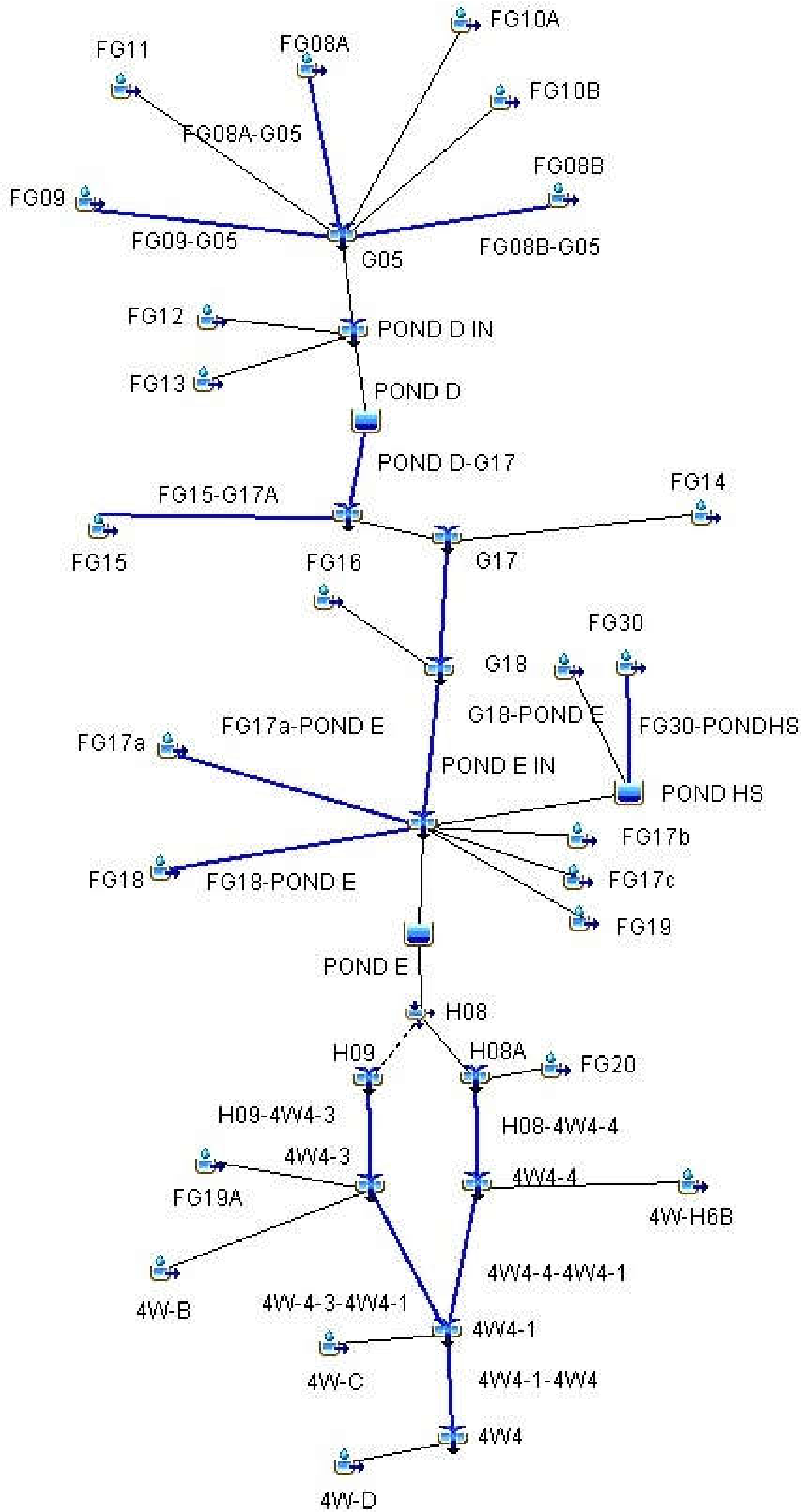
INTERIM MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
OS06	0.1313	0.5	01Jul2015, 13:30	0.3
G1a	0.1313	0.5	01Jul2015, 13:30	0.3
G1a-G2	0.1313	0.5	01Jul2015, 13:48	0.3
OS05	0.0578	0.2	01Jul2015, 13:24	0.2
OS05-G1	0.0578	0.2	01Jul2015, 13:30	0.2
FG01	0.0531	0.9	01Jul2015, 12:48	0.3
FG01-G1	0.0531	0.9	01Jul2015, 12:48	0.3
G1	0.1109	1.1	01Jul2015, 12:54	0.5
G1-G2	0.1109	1.1	01Jul2015, 13:00	0.5
FG02	0.0391	0.5	01Jul2015, 12:36	0.2
G2	0.2813	1.9	01Jul2015, 13:18	1.0
G2-G3	0.2813	1.9	01Jul2015, 13:30	1.0
FG03	0.0203	0.8	01Jul2015, 12:12	0.2
FG04	0.0172	0.9	01Jul2015, 12:06	0.1
G3	0.3188	2.4	01Jul2015, 13:24	1.3
G3-POND F	0.3188	2.4	01Jul2015, 13:30	1.3
FG06	0.0658	0.8	01Jul2015, 12:42	0.3
OS07a-POND F	0.0170	0.1	01Jul2015, 13:30	0.1
POND F	0.4596	2.3	01Jul2015, 16:12	1.9
POND F-G7	0.4596	2.3	01Jul2015, 16:24	1.9
FG22	0.0658	1.4	01Jul2015, 12:30	0.4
FG23a	0.0177	1.0	01Jul2015, 12:18	0.2
OS07b	0.0156	0.1	01Jul2015, 12:48	0.1
OS07b-G7	0.0156	0.1	01Jul2015, 13:42	0.1
G7	0.5587	2.9	01Jul2015, 16:30	2.6
G7-G10	0.5587	2.9	01Jul2015, 16:48	2.5
FG24	0.2503	1.1	01Jul2015, 13:48	0.7
OS09	0.1527	1.9	01Jul2015, 12:54	0.8
OS09-G10	0.1527	1.9	01Jul2015, 13:18	0.8
OS08	0.0397	0.7	01Jul2015, 12:24	0.2
OS08-G8	0.0397	0.7	01Jul2015, 12:54	0.2
G8	0.4427	3.4	01Jul2015, 13:24	1.7
G8-G10	0.4427	3.4	01Jul2015, 13:30	1.7
FG23b	0.0359	0.2	01Jul2015, 13:18	0.1
G10	1.0373	5.7	01Jul2015, 13:54	4.3
G10-G11	1.0373	5.7	01Jul2015, 13:54	4.3
FG23c	0.0070	0.2	01Jul2015, 12:24	0.1
G11	1.0443	5.8	01Jul2015, 13:42	4.3
FG25	0.1086	7.5	01Jul2015, 12:36	1.7
FG28	0.0673	0.5	01Jul2015, 13:12	0.3
POND G IN	1.2202	10.8	01Jul2015, 13:12	6.3
POND G	1.2202	4.2	01Jul2015, 22:30	4.0
G12	1.2202	4.2	01Jul2015, 22:30	4.0
G12-G06	1.2202	4.2	01Jul2015, 22:48	3.9
FG29	0.0997	0.4	01Jul2015, 13:36	0.3
FG32	0.0402	0	01Jul2015, 13:18	0.1
FG32-G06	0.0402	0	01Jul2015, 13:30	0.1
G06	1.3601	5	01Jul2015, 23:42	4.3
FG10A	0.0806	6.5	01Jul2015, 12:24	1.1
FG08A	0.0750	13	01Jul2015, 12:12	1.5

Highlighted green rows reference key design points (Typical all charts this section)

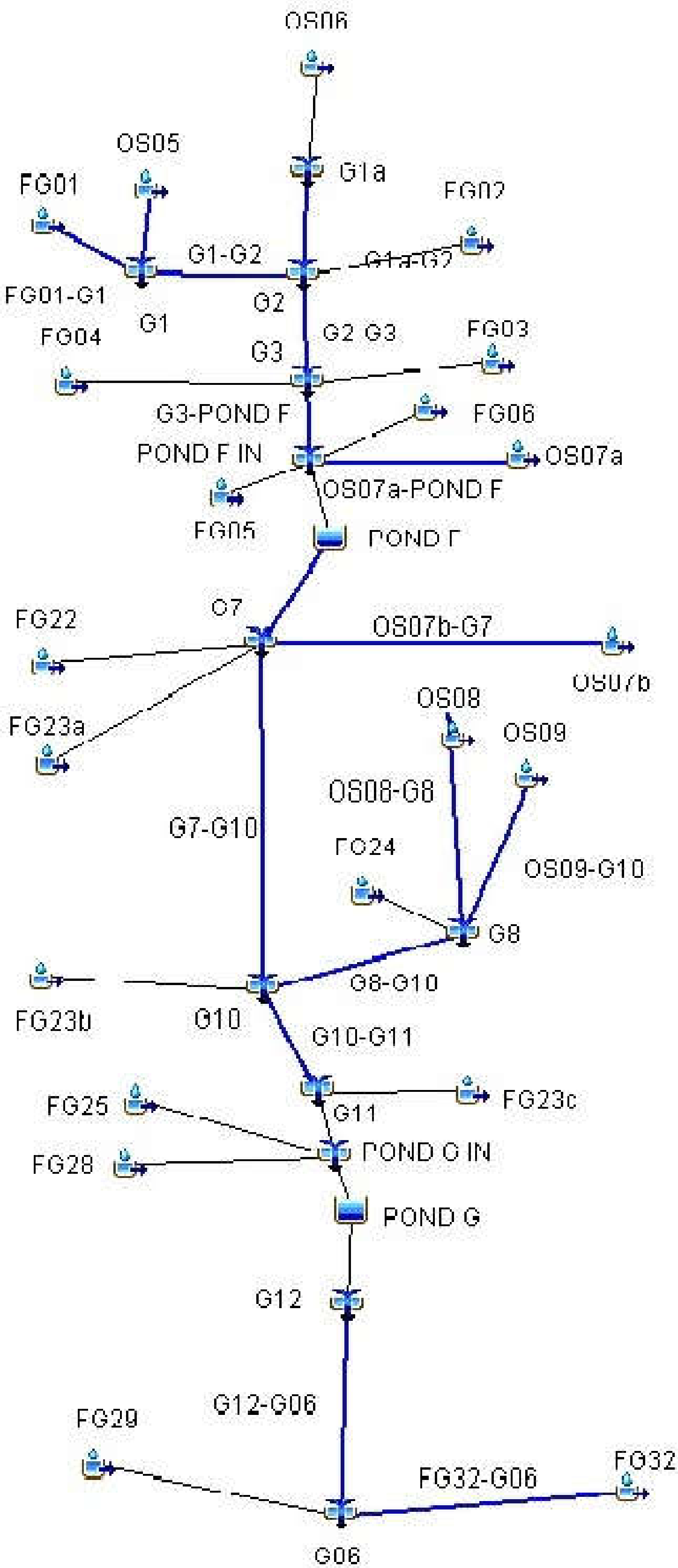
INTERIM MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
FG08A-G05	0.0750	13	01Jul2015, 12:18	1.5
FG08B	0.0630	10	01Jul2015, 12:12	1.2
FG08B-G05	0.0630	10	01Jul2015, 12:18	1.2
FG11	0.0625	10	01Jul2015, 12:18	1.4
FG09	0.0484	3	01Jul2015, 12:18	0.6
FG09-G05	0.0484	3	01Jul2015, 12:24	0.6
FG10B	0.0416	3	01Jul2015, 12:18	0.5
G05	0.3711	45	01Jul2015, 12:18	6.3
FG13	0.0534	1	01Jul2015, 12:42	0.3
FG12	0.0328	8	01Jul2015, 12:12	0.8
POND D IN	0.4573	52	01Jul2015, 12:18	7.5
POND D	0.4573	4	01Jul2015, 18:00	4.0
POND D-G17	0.4573	4	01Jul2015, 18:06	4.0
FG15	0.0103	2	01Jul2015, 12:12	0.2
FG15-G17A	0.0103	2	01Jul2015, 12:12	0.2
G17A	0.4676	4	01Jul2015, 17:54	4.2
FG14	0.1000	9	01Jul2015, 12:24	1.6
G17	0.5676	12	01Jul2015, 12:24	5.8
G17-G18	0.5676	12	01Jul2015, 12:30	5.8
FG16	0.0791	18	01Jul2015, 12:06	1.9
G18	0.6467	26	01Jul2015, 12:12	7.7
G18-POND E	0.6467	25	01Jul2015, 12:12	7.6
FG31	0.0922	17	01Jul2015, 12:18	2.4
FG30	0.0389	0	01Jul2015, 13:18	0.1
FG30-PONDHS	0.0389	0	01Jul2015, 13:48	0.1
POND HS	0.1311	10	01Jul2015, 12:42	2.5
FG17a	0.0694	12	01Jul2015, 12:12	1.3
FG17a-POND E	0.0694	12	01Jul2015, 12:12	1.3
FG18	0.0644	5	01Jul2015, 12:30	0.9
FG18-POND E	0.0644	5	01Jul2015, 12:30	0.9
FG19	0.0527	13	01Jul2015, 12:12	1.4
FG17c	0.0313	1	01Jul2015, 12:18	0.2
FG17b	0.0214	6	01Jul2015, 12:06	0.6
POND E IN	1.0170	63	01Jul2015, 12:12	14.5
POND E	1.0170	6	02Jul2015, 00:00	5.7
H08	1.0170	4	02Jul2015, 00:00	3.4
H09	0.0000	2	02Jul2015, 00:00	2.3
FG34	0.0836	0	01Jul2015, 13:36	0.2
G14	0.0836	0	01Jul2015, 13:36	0.2
G14-G15	0.0836	0	01Jul2015, 14:00	0.2
FG35	0.0586	0	01Jul2015, 14:24	0.2
G15	0.1422	1	01Jul2015, 14:06	0.4
G15-G08	0.1422	1	01Jul2015, 14:18	0.4
FG37	0.1203	0	01Jul2015, 14:12	0.3
FG36	0.0281	0	01Jul2015, 13:30	0.1
FG36-G08	0.0281	0	01Jul2015, 13:48	0.1
G08	0.2906	1	01Jul2015, 14:12	0.7

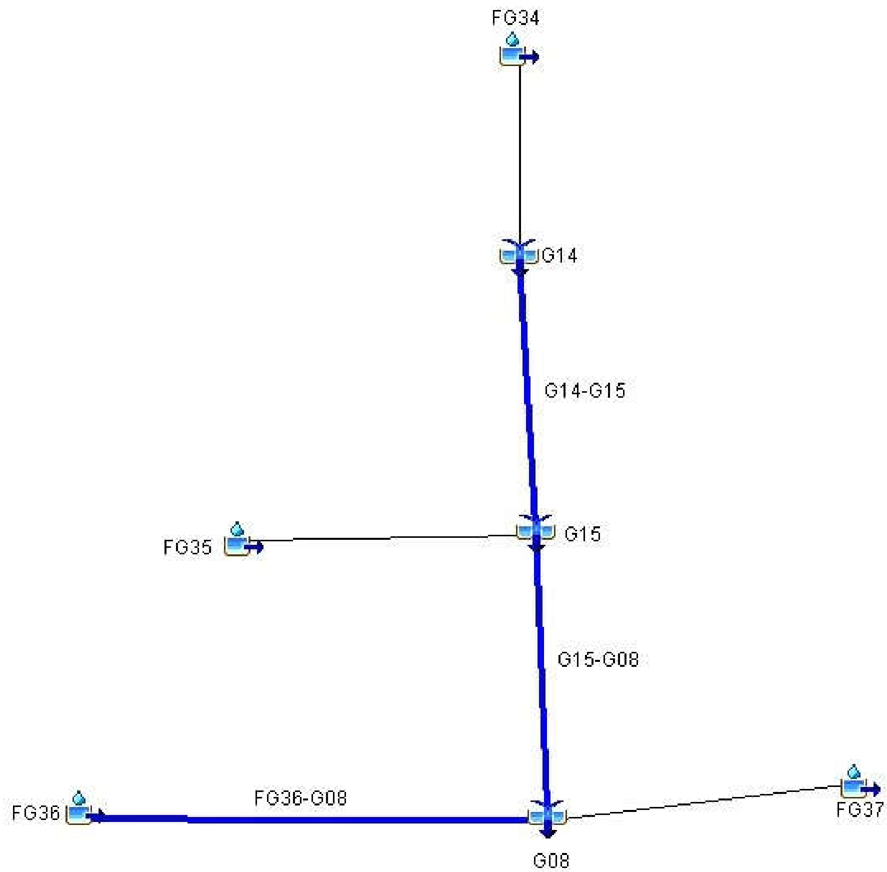
Highlighted green rows reference key design points (Typical all charts this section)

HAEGLER INTERIM CONDITIONS



GIECK INTERIM CONDITIONS




FG34
G14
G14-G15
FG35
G15
G15-G08
FG36
FG36-G08
FG37
G08

FUTURE MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
OS06	0.1313	80	01Jul2015, 12:12	1.3
G1a	0.1313	80	01Jul2015, 12:12	1.3
G1a-G2	0.1313	79	01Jul2015, 12:18	1.3
OS05	0.0578	39	01Jul2015, 12:12	1.3
OS05-G1	0.0578	39	01Jul2015, 12:12	1.3
FG01	0.0538	31	01Jul2015, 12:30	1.7
FG01-G1	0.0538	31	01Jul2015, 12:30	1.7
G1	0.1116	61	01Jul2015, 12:18	1.5
G1-G2	0.1116	61	01Jul2015, 12:18	1.5
FG02	0.0391	32	01Jul2015, 12:12	1.6
G2	0.2820	167	01Jul2015, 12:18	1.4
G2-G3	0.2820	163	01Jul2015, 12:18	1.4
FG03	0.0203	24	01Jul2015, 12:06	1.8
FG04	0.0172	22	01Jul2015, 12:00	1.8
G3	0.3195	185	01Jul2015, 12:18	1.5
G3-POND F	0.3195	183	01Jul2015, 12:18	1.5
FG06	0.0675	56	01Jul2015, 12:12	1.7
FG05	0.0580	45	01Jul2015, 12:24	2.0
OS07a	0.0170	14	01Jul2015, 12:06	1.5
OS07a-POND F	0.0170	13	01Jul2015, 12:18	1.4
POND F IN	0.4620	293	01Jul2015, 12:18	1.6
POND F	0.4620	179	01Jul2015, 12:42	1.5
POND F-G7	0.4620	179	01Jul2015, 12:42	1.5
FG21b	0.0170	26	01Jul2015, 12:12	2.8
FG21a	0.0072	6	01Jul2015, 12:06	1.3
FG21a-G7	0.0072	6	01Jul2015, 12:18	1.3
G7	0.4862	188	01Jul2015, 12:42	1.5
G7-G8	0.4862	188	01Jul2015, 12:42	1.5
FG22	0.1380	102	01Jul2015, 12:18	1.8
OS08	0.0406	35	01Jul2015, 12:12	1.7
OS08-G8	0.0406	34	01Jul2015, 12:12	1.7
FG23a	0.0216	21	01Jul2015, 12:12	1.9
OS07b	0.0156	15	01Jul2015, 12:06	1.5
OS07b-G7	0.0156	14	01Jul2015, 12:12	1.5
G8	0.7020	297	01Jul2015, 12:30	1.6
G8-G10	0.7020	294	01Jul2015, 12:30	1.6
OS09	0.1527	90	01Jul2015, 12:24	1.6
OS09-G10	0.1527	88	01Jul2015, 12:36	1.6
FG24	0.1373	105	01Jul2015, 12:18	1.8
G9	0.2900	180	01Jul2015, 12:24	1.7
G9-G10	0.2900	178	01Jul2015, 12:30	1.7
FG23b	0.0286	23	01Jul2015, 12:12	1.6
G10	1.0206	484	01Jul2015, 12:30	1.6
G10-G11	1.0206	480	01Jul2015, 12:30	1.6
FG23c	0.0122	12	01Jul2015, 12:06	1.8
G11	1.0328	485	01Jul2015, 12:30	1.6
FG25	0.1086	85	01Jul2015, 12:30	2.3
FG26	0.0863	78	01Jul2015, 12:18	2.0
FG26-POND G	0.0863	77	01Jul2015, 12:18	2.0
FG27	0.0500	52	01Jul2015, 12:18	2.4
FG28	0.0245	18	01Jul2015, 12:18	1.7

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
POND G IN	1.3022	691	01Jul2015, 12:30	1.7
POND G	1.3022	479	01Jul2015, 12:54	1.6
G12	1.3022	479	01Jul2015, 12:54	1.6
G12-G06	1.3022	479	01Jul2015, 13:00	1.6
FG29	0.0997	60	01Jul2015, 12:12	1.3
FG32	0.0402	72	01Jul2015, 12:06	2.8
FG32-G06	0.0402	69	01Jul2015, 12:06	2.8
G06	1.4421	507	01Jul2015, 12:54	1.6
FG08A	0.0750	116	01Jul2015, 12:06	2.6
FG08A-G05	0.0750	110	01Jul2015, 12:12	2.5
FG08B	0.0630	86	01Jul2015, 12:12	2.5
FG08B-G05	0.0630	84	01Jul2015, 12:12	2.5
FG09	0.0484	48	01Jul2015, 12:12	2.1
FG09-G05	0.0484	48	01Jul2015, 12:18	2.1
FG10B	0.0416	42	01Jul2015, 12:12	2.1
G05	0.2280	282	01Jul2015, 12:12	2.4
FG10A	0.0806	81	01Jul2015, 12:18	2.2
FG11	0.0625	75	01Jul2015, 12:18	2.7
FG13	0.0534	34	01Jul2015, 12:24	1.7
FG12	0.0328	50	01Jul2015, 12:12	2.8
POND D IN	0.4573	509	01Jul2015, 12:12	2.3
POND D	0.4573	134	01Jul2015, 13:00	1.9
POND D-G17	0.4573	134	01Jul2015, 13:00	1.9
FG15	0.0103	15	01Jul2015, 12:06	2.7
FG15-G17A	0.0103	15	01Jul2015, 12:12	2.7
G17A	0.4676	137	01Jul2015, 13:00	1.9
FG14	0.1000	98	01Jul2015, 12:18	2.4
G17	0.5676	196	01Jul2015, 12:30	2.0
G17-G18	0.5676	196	01Jul2015, 12:36	2.0
FG16	0.0791	133	01Jul2015, 12:06	2.7
G18	0.6467	240	01Jul2015, 12:24	2.1
G18-POND E	0.6467	240	01Jul2015, 12:24	2.1
FG31	0.0922	116	01Jul2015, 12:18	2.8
FG30	0.0389	73	01Jul2015, 12:06	2.8
FG30-PONDHS	0.0389	70	01Jul2015, 12:12	2.8
POND HS	0.1311	153	01Jul2015, 12:24	2.8
FG17a	0.0694	101	01Jul2015, 12:06	2.5
FG17a-POND E	0.0694	99	01Jul2015, 12:06	2.5
FG18	0.0644	56	01Jul2015, 12:24	2.3
FG18-POND E	0.0644	56	01Jul2015, 12:24	2.3
FG19	0.0527	84	01Jul2015, 12:06	2.9
FG17c	0.0313	31	01Jul2015, 12:06	1.6
FG17b	0.0214	39	01Jul2015, 12:06	2.8
POND E IN	1.0170	610	01Jul2015, 12:18	2.3
POND E	1.0170	242	01Jul2015, 13:30	1.8
H08	1.0170	205	01Jul2015, 13:30	1.6
H09	0.0000	37	01Jul2015, 13:30	n/a

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (100-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	TIME OF PEAK	TOTAL VOLUME Q100 (AC. FT.)
FG34	0.0600	34	01Jul2015, 12:18	1.4
G14	0.0600	34	01Jul2015, 12:18	1.4
G14-G15	0.0600	34	01Jul2015, 12:24	1.4
FG35	0.0344	20	01Jul2015, 12:24	1.5
G15	0.0944	53	01Jul2015, 12:24	1.4
G15-G08	0.0944	52	01Jul2015, 12:24	1.4
FG37	0.0797	41	01Jul2015, 12:18	1.3
FG36	0.0281	14	01Jul2015, 12:18	1.3
FG36-G08	0.0281	14	01Jul2015, 12:24	1.3
G08	0.2022	106	01Jul2015, 12:24	1.4

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
OS06	0.1313	52	01Jul2015, 12:12	6.5
G1a	0.1313	52	01Jul2015, 12:12	6.5
G1a-G2	0.1313	52	01Jul2015, 12:18	6.5
OS05	0.0578	26	01Jul2015, 12:12	2.9
OS05-G1	0.0578	25	01Jul2015, 12:12	2.9
FG01	0.0538	22	01Jul2015, 12:30	3.6
FG01-G1	0.0538	22	01Jul2015, 12:30	3.6
G1	0.1116	41	01Jul2015, 12:18	6.4
G1-G2	0.1116	41	01Jul2015, 12:18	6.4
FG02	0.0391	22	01Jul2015, 12:12	2.4
G2	0.2820	112	01Jul2015, 12:18	15.3
G2-G3	0.2820	109	01Jul2015, 12:24	15.2
FG03	0.0203	17	01Jul2015, 12:06	1.5
FG04	0.0172	16	01Jul2015, 12:00	1.3
G3	0.3195	123	01Jul2015, 12:18	17.9
G3-POND F	0.3195	121	01Jul2015, 12:18	17.9
FG06	0.0675	40	01Jul2015, 12:12	4.4
FG05	0.0580	33	01Jul2015, 12:24	4.6
OS07a	0.0170	9	01Jul2015, 12:12	1.0
OS07a-POND F	0.0170	9	01Jul2015, 12:18	0.9
POND F IN	0.4620	200	01Jul2015, 12:18	27.9
POND F	0.4620	125	01Jul2015, 12:42	26.2
POND F-G7	0.4620	124	01Jul2015, 12:48	26.0
FG21b	0.0170	20	01Jul2015, 12:12	2.0
FG21a	0.0072	4	01Jul2015, 12:06	0.4
FG21a-G7	0.0072	3	01Jul2015, 12:18	0.4
G7	0.4862	130	01Jul2015, 12:42	28.4
G7-G8	0.4862	130	01Jul2015, 12:48	28.4
FG22	0.1380	73	01Jul2015, 12:18	9.6
OS08	0.0406	25	01Jul2015, 12:12	2.6
OS08-G8	0.0406	24	01Jul2015, 12:12	2.6
FG23a	0.0216	15	01Jul2015, 12:12	1.6
OS07b	0.0156	10	01Jul2015, 12:06	0.9
OS07b-G7	0.0156	10	01Jul2015, 12:12	0.9
G8	0.7020	192	01Jul2015, 12:36	43.1
G8-G10	0.7020	191	01Jul2015, 12:42	42.9
OS09	0.1527	62	01Jul2015, 12:24	9.4
OS09-G10	0.1527	62	01Jul2015, 12:36	9.3
FG24	0.1373	76	01Jul2015, 12:18	9.9
G9	0.2900	125	01Jul2015, 12:30	19.2
G9-G10	0.2900	125.0	01Jul2015, 12:30	19.2
FG23b	0.0286	16	01Jul2015, 12:12	1.8
G10	1.0206	313	01Jul2015, 12:36	63.8
G10-G11	1.0206	311	01Jul2015, 12:36	63.6
FG23c	0.0122	9	01Jul2015, 12:06	0.9
G11	1.0328	314	01Jul2015, 12:36	64.5
FG25	0.1086	64	01Jul2015, 12:30	10.2
FG26	0.0863	58	01Jul2015, 12:18	7.1
FG26-POND G	0.0863	57	01Jul2015, 12:18	7.0
FG27	0.0500	40	01Jul2015, 12:18	4.8
FG28	0.0245	13	01Jul2015, 12:18	1.7

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
POND G IN	1.3022	459	01Jul2015, 12:30	88.2
POND G	1.3022	338	01Jul2015, 13:00	79.8
G12	1.3022	338	01Jul2015, 13:00	79.8
G12-G06	1.3022	337	01Jul2015, 13:00	79.2
FG29	0.0997	39	01Jul2015, 12:18	5.0
FG32	0.0402	57	01Jul2015, 12:06	4.8
FG32-G06	0.0402	54	01Jul2015, 12:06	4.8
G06	1.4421	357	01Jul2015, 13:00	89.0
FG08A	0.0750	90	01Jul2015, 12:06	7.9
FG08A-G05	0.0750	86	01Jul2015, 12:12	7.9
FG08B	0.0630	67	01Jul2015, 12:12	6.6
FG08B-G05	0.0630	65	01Jul2015, 12:12	6.6
FG09	0.0484	36	01Jul2015, 12:12	4.1
FG09-G05	0.0484	36	01Jul2015, 12:18	4.1
FG10B	0.0416	31	01Jul2015, 12:12	3.5
G05	0.2280	215	01Jul2015, 12:12	22.1
FG10A	0.0806	61	01Jul2015, 12:18	7.3
FG11	0.0625	59	01Jul2015, 12:18	7.0
FG13	0.0534	24	01Jul2015, 12:24	3.5
FG12	0.0328	40	01Jul2015, 12:12	3.9
POND D IN	0.4573	387	01Jul2015, 12:12	43.9
POND D	0.4573	91	01Jul2015, 13:06	34.7
POND D-G17	0.4573	91	01Jul2015, 13:06	34.6
FG15	0.0103	12	01Jul2015, 12:12	1.2
FG15-G17A	0.0103	12	01Jul2015, 12:12	1.2
G17A	0.4676	93	01Jul2015, 13:06	35.8
FG14	0.1000	74	01Jul2015, 12:18	9.6
G17	0.5676	132	01Jul2015, 12:36	45.4
G17-G18	0.5676	131	01Jul2015, 12:36	45.4
FG16	0.0791	104	01Jul2015, 12:06	9.0
G18	0.6467	178	01Jul2015, 12:12	54.4
G18-POND E	0.6467	176	01Jul2015, 12:12	54.4
FG31	0.0922	92	01Jul2015, 12:18	11.0
FG30	0.0389	57	01Jul2015, 12:06	4.7
FG30-PONDHS	0.0389	56	01Jul2015, 12:12	4.6
POND HS	0.1311	106	01Jul2015, 12:30	15.5
FG17a	0.0694	78	01Jul2015, 12:06	7.3
FG17a-POND E	0.0694	76	01Jul2015, 12:06	7.3
FG18	0.0644	42	01Jul2015, 12:24	5.9
FG18-POND E	0.0644	42	01Jul2015, 12:24	5.9
FG19	0.0527	66	01Jul2015, 12:06	6.4
FG17c	0.0313	22	01Jul2015, 12:06	2.0
FG17b	0.0214	31	01Jul2015, 12:06	2.6
POND E IN	1.0170	432	01Jul2015, 12:12	94.0
POND E	1.0170	153	01Jul2015, 13:54	70.7
H08	1.0170	137	01Jul2015, 13:54	62.1
H09	0.0000	16	01Jul2015, 13:54	8.5

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (50-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q50 (CFS)	TIME OF PEAK	TOTAL VOLUME Q50 (AC. FT.)
FG34	0.0600	23	01Jul2015, 12:18	3.2
G14	0.0600	23	01Jul2015, 12:18	3.2
G14-G15	0.0600	22	01Jul2015, 12:24	3.1
FG35	0.0344	13.4	01Jul2015, 12:24	2.0
G15	0.0944	35.6	01Jul2015, 12:24	5.1
G15-G08	0.0944	35	01Jul2015, 12:30	5.0
FG37	0.0797	27	01Jul2015, 12:24	4.0
FG36	0.0281	9	01Jul2015, 12:24	1.4
FG36-G08	0.0281	9	01Jul2015, 12:30	1.4
G08	0.2022	69	01Jul2015, 12:24	10.4

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
OS06	0.1313	30	01Jul2015, 12:18	4.3
G1a	0.1313	30	01Jul2015, 12:18	4.3
G1a-G2	0.1313	30	01Jul2015, 12:18	4.2
OS05	0.0578	15	01Jul2015, 12:12	1.9
OS05-G1	0.0578	15	01Jul2015, 12:12	1.9
FG01	0.0538	14	01Jul2015, 12:30	2.5
FG01-G1	0.0538	14	01Jul2015, 12:30	2.5
G1	0.1116	25	01Jul2015, 12:18	4.4
G1-G2	0.1116	25	01Jul2015, 12:24	4.3
FG02	0.0391	14	01Jul2015, 12:12	1.6
G2	0.2820	67	01Jul2015, 12:18	10.2
G2-G3	0.2820	66	01Jul2015, 12:24	10.1
FG03	0.0203	12	01Jul2015, 12:06	1.0
FG04	0.0172	11	01Jul2015, 12:00	0.9
G3	0.3195	74	01Jul2015, 12:24	12.0
G3-POND F	0.3195	74	01Jul2015, 12:24	12.0
FG06	0.0675	26	01Jul2015, 12:12	3.1
FG05	0.0580	23	01Jul2015, 12:24	3.3
OS07a	0.0170	6	01Jul2015, 12:12	0.6
OS07a-POND F	0.0170	6	01Jul2015, 12:24	0.6
POND F IN	0.4620	123	01Jul2015, 12:24	19.0
POND F	0.4620	64	01Jul2015, 12:54	17.6
POND F-G7	0.4620	63	01Jul2015, 13:00	17.5
FG21b	0.0170	16	01Jul2015, 12:12	1.5
FG21a	0.0072	2	01Jul2015, 12:06	0.2
FG21a-G7	0.0072	2	01Jul2015, 12:24	0.2
G7	0.4862	67	01Jul2015, 13:00	19.3
G7-G8	0.4862	67	01Jul2015, 13:00	19.2
FG22	0.1380	47	01Jul2015, 12:18	6.7
OS08	0.0406	16	01Jul2015, 12:12	1.8
OS08-G8	0.0406	15	01Jul2015, 12:18	1.8
FG23a	0.0216	10	01Jul2015, 12:12	1.1
OS07b	0.0156	6	01Jul2015, 12:06	0.6
OS07b-G7	0.0156	6	01Jul2015, 12:12	0.6
G8	0.7020	97	01Jul2015, 12:54	29.5
G8-G10	0.7020	96	01Jul2015, 12:54	29.3
OS09	0.1527	39	01Jul2015, 12:30	6.5
OS09-G10	0.1527	39	01Jul2015, 12:36	6.3
FG24	0.1373	50	01Jul2015, 12:18	7.0
G9	0.2900	81	01Jul2015, 12:30	13.3
G9-G10	0.2900	79.0	01Jul2015, 12:30	13.3
FG23b	0.0286	10	01Jul2015, 12:12	1.2
G10	1.0206	176	01Jul2015, 12:30	43.8
G10-G11	1.0206	174	01Jul2015, 12:30	43.6
FG23c	0.0122	6	01Jul2015, 12:12	0.6
G11	1.0328	177	01Jul2015, 12:30	44.2
FG25	0.1086	45.9	01Jul2015, 12:30	7.5
FG26	0.0863	40	01Jul2015, 12:18	5.1
FG26-POND G	0.0863	39	01Jul2015, 12:18	5.1
FG27	0.0500	29	01Jul2015, 12:18	3.6
FG28	0.0245	8	01Jul2015, 12:18	1.1

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
POND G IN	1.3022	288	01Jul2015, 12:30	61.5
POND G	1.3022	173	01Jul2015, 13:12	53.7
G12	1.3022	173	01Jul2015, 13:12	53.7
G12-G06	1.3022	173	01Jul2015, 13:18	53.2
FG29	0.0997	23	01Jul2015, 12:18	3.3
FG32	0.0402	44	01Jul2015, 12:06	3.7
FG32-G06	0.0402	41	01Jul2015, 12:06	3.7
G06	1.4421	184	01Jul2015, 13:18	60.1
FG08A	0.0750	66	01Jul2015, 12:06	5.9
FG08A-G05	0.0750	64	01Jul2015, 12:12	5.9
FG08B	0.0630	49	01Jul2015, 12:12	5.0
FG08B-G05	0.0630	48	01Jul2015, 12:12	4.9
FG09	0.0484	25	01Jul2015, 12:18	3.0
FG09-G05	0.0484	25	01Jul2015, 12:18	3.0
FG10B	0.0416	22	01Jul2015, 12:12	2.5
G05	0.2280	156	01Jul2015, 12:12	16.3
FG10A	0.0806	43	01Jul2015, 12:18	5.4
FG11	0.0625	44	01Jul2015, 12:18	5.2
FG13	0.0534	15	01Jul2015, 12:24	2.4
FG12	0.0328	30.2	01Jul2015, 12:12	3.0
POND D IN	0.4573	279.7	01Jul2015, 12:12	32.4
POND D	0.4573	50	01Jul2015, 13:24	24.3
POND D-G17	0.4573	50	01Jul2015, 13:24	24.3
FG15	0.0103	9	01Jul2015, 12:12	0.9
FG15-G17A	0.0103	9	01Jul2015, 12:12	0.9
G17A	0.4676	51	01Jul2015, 13:24	25.2
FG14	0.1000	53	01Jul2015, 12:18	7.1
G17	0.5676	75	01Jul2015, 12:24	32.3
G17-G18	0.5676	75	01Jul2015, 12:24	32.2
FG16	0.0791	78	01Jul2015, 12:06	6.8
G18	0.6467	128	01Jul2015, 12:12	39.1
G18-POND E	0.6467	126	01Jul2015, 12:12	39.0
FG31	0.0922	69	01Jul2015, 12:18	8.4
FG30	0.0389	44	01Jul2015, 12:06	3.6
FG30-PONDHS	0.0389	42	01Jul2015, 12:12	3.5
POND HS	0.1311	53	01Jul2015, 12:42	11.9
FG17a	0.0694	57	01Jul2015, 12:06	5.4
FG17a-POND E	0.0694	56	01Jul2015, 12:12	5.4
FG18	0.0644	30	01Jul2015, 12:24	4.3
FG18-POND E	0.0644	30	01Jul2015, 12:24	4.3
FG19	0.0527	50	01Jul2015, 12:06	4.9
FG17c	0.0313	14	01Jul2015, 12:06	1.4
FG17b	0.0214	24	01Jul2015, 12:06	1.9
POND E IN	1.0170	318	01Jul2015, 12:12	68.8
POND E	1.0170	80	01Jul2015, 14:36	46.5
H08	1.0170	72	01Jul2015, 14:36	39.8
H09	0.0000	8	01Jul2015, 14:36	6.7

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (25-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q25 (CFS)	TIME OF PEAK	TOTAL VOLUME Q25 (AC. FT.)
FG34	0.0600	13	01Jul2015, 12:24	2.1
G14	0.0600	13	01Jul2015, 12:24	2.1
G14-G15	0.0600	13	01Jul2015, 12:30	2.1
FG35	0.0344	8.3	01Jul2015, 12:24	1.3
G15	0.0944	21.3	01Jul2015, 12:30	3.4
G15-G08	0.0944	21	01Jul2015, 12:30	3.3
FG37	0.0797	16	01Jul2015, 12:24	2.6
FG36	0.0281	6	01Jul2015, 12:24	0.9
FG36-G08	0.0281	5	01Jul2015, 12:30	0.9
G08	0.2022	41	01Jul2015, 12:30	6.8

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
OS06	0.1313	12	01Jul2015, 12:18	2.2
G1a	0.1313	12	01Jul2015, 12:18	2.2
G1a-G2	0.1313	11	01Jul2015, 12:24	2.1
OS05	0.0578	5.6	01Jul2015, 12:12	1.0
OS05-G1	0.0578	5.5	01Jul2015, 12:18	1.0
FG01	0.0538	7.0	01Jul2015, 12:36	1.4
FG01-G1	0.0538	7.0	01Jul2015, 12:36	1.4
G1	0.1116	11	01Jul2015, 12:24	2.3
G1-G2	0.1116	11	01Jul2015, 12:30	2.3
FG02	0.0391	6.4	01Jul2015, 12:12	0.9
G2	0.2820	27	01Jul2015, 12:24	5.4
G2-G3	0.2820	27	01Jul2015, 12:30	5.3
FG03	0.0203	5.9	01Jul2015, 12:06	0.6
FG04	0.0172	5.8	01Jul2015, 12:06	0.5
G3	0.3195	31	01Jul2015, 12:30	6.4
G3-POND F	0.3195	31	01Jul2015, 12:30	6.4
FG06	0.0675	12	01Jul2015, 12:18	1.7
FG05	0.0580	12.2	01Jul2015, 12:24	2.0
OS07a	0.0170	2	01Jul2015, 12:12	0.3
OS07a-POND F	0.0170	2	01Jul2015, 12:30	0.3
POND F IN	0.4620	53.7	01Jul2015, 12:30	10.4
POND F	0.4620	17.0	01Jul2015, 13:48	9.6
POND F-G7	0.4620	17.0	01Jul2015, 13:54	9.5
FG21b	0.0170	10.2	01Jul2015, 12:12	1.0
FG21a	0.0072	1	01Jul2015, 12:06	0.1
FG21a-G7	0.0072	1	01Jul2015, 12:30	0.1
G7	0.4862	18	01Jul2015, 13:36	10.6
G7-G8	0.4862	18.3	01Jul2015, 13:42	10.6
FG22	0.1380	24.0	01Jul2015, 12:24	3.8
OS08	0.0406	7.7	01Jul2015, 12:12	1.0
OS08-G8	0.0406	8	01Jul2015, 12:18	1.0
FG23a	0.0216	5	01Jul2015, 12:12	0.7
OS07b	0.0156	3	01Jul2015, 12:06	0.3
OS07b-G7	0.0156	2	01Jul2015, 12:18	0.3
G8	0.7020	48	01Jul2015, 12:18	16.4
G8-G10	0.7020	47	01Jul2015, 12:24	16.3
OS09	0.1527	18	01Jul2015, 12:30	3.5
OS09-G10	0.1527	18.1	01Jul2015, 12:42	3.5
FG24	0.1373	26	01Jul2015, 12:24	4.0
G9	0.2900	38	01Jul2015, 12:36	7.5
G9-G10	0.2900	36.8	01Jul2015, 12:36	7.5
FG23b	0.0286	5	01Jul2015, 12:12	0.7
G10	1.0206	80	01Jul2015, 12:30	24.4
G10-G11	1.0206	80	01Jul2015, 12:36	24.4
FG23c	0.0122	3	01Jul2015, 12:12	0.3
G11	1.0328	81	01Jul2015, 12:36	24.7
FG25	0.1086	27.1	01Jul2015, 12:36	4.7
FG26	0.0863	22	01Jul2015, 12:18	3.0
FG26-POND G	0.0863	22	01Jul2015, 12:24	3.0
FG27	0.0500	17	01Jul2015, 12:18	2.3
FG28	0.0245	4	01Jul2015, 12:18	0.7

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
POND G IN	1.3022	145.9	01Jul2015, 12:30	35.3
POND G	1.3022	57	01Jul2015, 13:54	28.8
G12	1.3022	57	01Jul2015, 13:54	28.8
G12-G06	1.3022	57	01Jul2015, 14:00	28.6
FG29	0.0997	9	01Jul2015, 12:18	1.6
FG32	0.0402	29	01Jul2015, 12:06	2.5
FG32-G06	0.0402	27	01Jul2015, 12:06	2.4
G06	1.4421	61	01Jul2015, 13:54	32.7
FG08A	0.0750	41	01Jul2015, 12:06	3.8
FG08A-G05	0.0750	41	01Jul2015, 12:12	3.8
FG08B	0.0630	31	01Jul2015, 12:12	3.2
FG08B-G05	0.0630	29	01Jul2015, 12:18	3.2
FG09	0.0484	14	01Jul2015, 12:18	1.8
FG09-G05	0.0484	14	01Jul2015, 12:18	1.8
FG10B	0.0416	12.2	01Jul2015, 12:18	1.5
G05	0.2280	94	01Jul2015, 12:12	10.3
FG10A	0.0806	25	01Jul2015, 12:18	3.3
FG11	0.0625	28	01Jul2015, 12:18	3.4
FG13	0.0534	7	01Jul2015, 12:30	1.4
FG12	0.0328	19.9	01Jul2015, 12:12	2.0
POND D IN	0.4573	168.0	01Jul2015, 12:18	20.4
POND D	0.4573	19	01Jul2015, 14:24	14.8
POND D-G17	0.4573	19	01Jul2015, 14:24	14.8
FG15	0.0103	6	01Jul2015, 12:12	0.6
FG15-G17A	0.0103	6	01Jul2015, 12:12	0.6
G17A	0.4676	19	01Jul2015, 14:12	15.3
FG14	0.1000	32	01Jul2015, 12:24	4.5
G17	0.5676	43	01Jul2015, 12:24	19.8
G17-G18	0.5676	43	01Jul2015, 12:30	19.8
FG16	0.0791	50	01Jul2015, 12:06	4.5
G18	0.6467	79	01Jul2015, 12:12	24.3
G18-POND E	0.6467	78	01Jul2015, 12:12	24.3
FG31	0.0922	45	01Jul2015, 12:18	5.6
FG30	0.0389	29	01Jul2015, 12:06	2.4
FG30-PONDHS	0.0389	27	01Jul2015, 12:12	2.3
POND HS	0.1311	36	01Jul2015, 12:42	7.9
FG17a	0.0694	35	01Jul2015, 12:06	3.5
FG17a-POND E	0.0694	35.1	01Jul2015, 12:12	3.5
FG18	0.0644	18	01Jul2015, 12:24	2.7
FG18-POND E	0.0644	17	01Jul2015, 12:30	2.7
FG19	0.0527	33	01Jul2015, 12:12	3.3
FG17c	0.0313	7	01Jul2015, 12:06	0.8
FG17b	0.0214	16	01Jul2015, 12:06	1.3
POND E IN	1.0170	197	01Jul2015, 12:12	43.7
POND E	1.0170	30	01Jul2015, 17:36	24.4
H08	1.0170	24.1	01Jul2015, 17:36	19.1
H09	0.0000	6	01Jul2015, 17:36	5.3

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (10-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q10 (CFS)	TIME OF PEAK	TOTAL VOLUME Q10 (AC. FT.)
FG34	0.0600	5.5	01Jul2015, 12:24	1.1
G14	0.0600	5.5	01Jul2015, 12:24	1.1
G14-G15	0.0600	5.4	01Jul2015, 12:36	1.1
FG35	0.0344	3.5	01Jul2015, 12:30	0.7
G15	0.0944	8.7	01Jul2015, 12:36	1.8
G15-G08	0.0944	9	01Jul2015, 12:36	1.7
FG37	0.0797	6	01Jul2015, 12:24	1.3
FG36	0.0281	2	01Jul2015, 12:30	0.5
FG36-G08	0.0281	2	01Jul2015, 12:36	0.5
G08	0.2022	16	01Jul2015, 12:36	3.5

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
OS06	0.1313	3.8	01Jul2015, 12:24	1.1
G1a	0.1313	3.8	01Jul2015, 12:24	1.1
G1a-G2	0.1313	3.6	01Jul2015, 12:36	1.1
OS05	0.0578	1.8	01Jul2015, 12:18	0.5
OS05-G1	0.0578	1.7	01Jul2015, 12:24	0.5
FG01	0.0538	3.4	01Jul2015, 12:36	0.8
FG01-G1	0.0538	3.4	01Jul2015, 12:36	0.8
G1	0.1116	4.9	01Jul2015, 12:36	1.3
G1-G2	0.1116	4.8	01Jul2015, 12:36	1.3
FG02	0.0391	2.7	01Jul2015, 12:18	0.5
G2	0.2820	10	01Jul2015, 12:30	2.9
G2-G3	0.2820	10	01Jul2015, 12:42	2.9
FG03	0.0203	0.8	01Jul2015, 12:12	0.2
FG04	0.0172	3.1	01Jul2015, 12:06	0.3
G3	0.3195	11	01Jul2015, 12:36	3.3
G3-POND F	0.3195	11	01Jul2015, 12:42	3.3
FG06	0.0675	5.8	01Jul2015, 12:18	1.0
FG05	0.0580	6.7	01Jul2015, 12:30	1.2
OS07a	0.0170	0.9	01Jul2015, 12:12	0.2
OS07a-POND F	0.0170	0.9	01Jul2015, 12:36	0.2
POND F IN	0.4620	22.3	01Jul2015, 12:36	5.7
POND F	0.4620	8.5	01Jul2015, 14:06	5.2
POND F-G7	0.4620	8.5	01Jul2015, 14:12	5.1
FG21b	0.0170	7.0	01Jul2015, 12:12	0.7
FG21a	0.0072	0.3	01Jul2015, 12:12	0.1
FG21a-G7	0.0072	0.3	01Jul2015, 12:42	0.1
G7	0.4862	9	01Jul2015, 14:06	5.9
G7-G8	0.4862	9.2	01Jul2015, 14:12	5.9
FG22	0.1380	12.0	01Jul2015, 12:24	2.3
OS08	0.0406	3.4	01Jul2015, 12:12	0.6
OS08-G8	0.0406	3	01Jul2015, 12:18	0.6
FG23a	0.0216	3	01Jul2015, 12:18	0.4
OS07b	0.0156	1.0	01Jul2015, 12:12	0.2
OS07b-G7	0.0156	0.9	01Jul2015, 12:18	0.2
G8	0.7020	25	01Jul2015, 12:18	9.3
G8-G10	0.7020	24	01Jul2015, 12:24	9.3
OS09	0.1527	8	01Jul2015, 12:36	2.0
OS09-G10	0.1527	8.2	01Jul2015, 12:48	2.0
FG24	0.1373	13	01Jul2015, 12:24	2.5
G9	0.2900	17	01Jul2015, 12:48	4.4
G9-G10	0.2900	16.9	01Jul2015, 12:48	4.4
FG23b	0.0286	2	01Jul2015, 12:18	0.4
G10	1.0206	39	01Jul2015, 12:24	14.0
G10-G11	1.0206	38.9	01Jul2015, 12:30	13.9
FG23c	0.0122	1.5	01Jul2015, 12:12	0.2
G11	1.0328	39.7	01Jul2015, 12:30	14.1
FG25	0.1086	16.7	01Jul2015, 12:36	3.1
FG26	0.0863	12	01Jul2015, 12:24	1.9
FG26-POND G	0.0863	12	01Jul2015, 12:24	1.9
FG27	0.0500	11	01Jul2015, 12:18	1.5
FG28	0.0245	2	01Jul2015, 12:24	0.4

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
POND G IN	1.3022	78.7	01Jul2015, 12:30	21.0
POND G	1.3022	22	01Jul2015, 15:12	15.2
G12	1.3022	22	01Jul2015, 15:12	15.2
G12-G06	1.3022	22	01Jul2015, 15:18	15.0
FG29	0.0997	3	01Jul2015, 12:24	0.9
FG32	0.0402	20	01Jul2015, 12:06	1.7
FG32-G06	0.0402	18	01Jul2015, 12:12	1.7
G06	1.4421	24	01Jul2015, 15:18	17.5
FG08A	0.0750	27	01Jul2015, 12:06	2.6
FG08A-G05	0.0750	27	01Jul2015, 12:12	2.6
FG08B	0.0630	20.1	01Jul2015, 12:12	2.2
FG08B-G05	0.0630	19.5	01Jul2015, 12:18	2.2
FG09	0.0484	8.3	01Jul2015, 12:18	1.2
FG09-G05	0.0484	8	01Jul2015, 12:24	1.2
FG10B	0.0416	7.0	01Jul2015, 12:18	1.0
G05	0.2280	59	01Jul2015, 12:18	6.9
FG10A	0.0806	15	01Jul2015, 12:18	2.2
FG11	0.0625	19	01Jul2015, 12:18	2.4
FG13	0.0534	4	01Jul2015, 12:30	0.8
FG12	0.0328	13.7	01Jul2015, 12:12	1.4
POND D IN	0.4573	107.0	01Jul2015, 12:18	13.6
POND D	0.4573	12	01Jul2015, 14:36	9.2
POND D-G17	0.4573	12	01Jul2015, 14:42	9.2
FG15	0.0103	4	01Jul2015, 12:12	0.4
FG15-G17A	0.0103	4	01Jul2015, 12:12	0.4
G17A	0.4676	12	01Jul2015, 14:36	9.6
FG14	0.1000	20	01Jul2015, 12:24	3.0
G17	0.5676	25	01Jul2015, 12:24	12.5
G17-G18	0.5676	25	01Jul2015, 12:24	12.5
FG16	0.0791	34	01Jul2015, 12:06	3.1
G18	0.6467	51	01Jul2015, 12:12	15.7
G18-POND E	0.6467	50	01Jul2015, 12:12	15.6
FG31	0.0922	31	01Jul2015, 12:18	3.9
FG30	0.0389	20	01Jul2015, 12:06	1.7
FG30-PONDHS	0.0389	18	01Jul2015, 12:12	1.6
POND HS	0.1311	26	01Jul2015, 12:36	5.6
FG17a	0.0694	23	01Jul2015, 12:12	2.4
FG17a-POND E	0.0694	22.9	01Jul2015, 12:12	2.4
FG18	0.0644	11	01Jul2015, 12:30	1.8
FG18-POND E	0.0644	11	01Jul2015, 12:30	1.8
FG19	0.0527	23	01Jul2015, 12:12	2.3
FG17c	0.0313	3	01Jul2015, 12:12	0.4
FG17b	0.0214	11	01Jul2015, 12:06	0.9
POND E IN	1.0170	126	01Jul2015, 12:12	29.0
POND E	1.0170	16	01Jul2015, 20:00	13.1
H08	1.0170	11.8	01Jul2015, 20:00	9.4
H09	0.0000	4.1	01Jul2015, 20:00	3.7

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (5-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q5 (CFS)	TIME OF PEAK	TOTAL VOLUME Q5 (AC. FT.)
FG34	0.0600	2.0	01Jul2015, 12:30	0.6
G14	0.0600	2.0	01Jul2015, 12:30	0.6
G14-G15	0.0600	2.0	01Jul2015, 12:42	0.6
FG35	0.0344	1.5	01Jul2015, 12:30	0.4
G15	0.0944	3.3	01Jul2015, 12:42	0.9
G15-G08	0.0944	3.3	01Jul2015, 12:48	0.9
FG37	0.0797	2.0	01Jul2015, 12:36	0.7
FG36	0.0281	0.7	01Jul2015, 12:36	0.2
FG36-G08	0.0281	0.7	01Jul2015, 12:48	0.2
G08	0.2022	5.8	01Jul2015, 12:48	1.8

Highlighted green rows reference key design points (Typical all charts this section)

FUTURE MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
OS06	0.1313	0.5	01Jul2015, 13:30	0.3
G1a	0.1313	0.5	01Jul2015, 13:30	0.3
G1a-G2	0.1313	0.5	01Jul2015, 13:48	0.3
OS05	0.0578	0.2	01Jul2015, 13:24	0.2
OS05-G1	0.0578	0.2	01Jul2015, 13:30	0.2
FG01	0.0538	0.9	01Jul2015, 12:48	0.4
FG01-G1	0.0538	0.9	01Jul2015, 12:48	0.4
G1	0.1116	1.1	01Jul2015, 12:54	0.5
G1-G2	0.1116	1.1	01Jul2015, 13:00	0.5
FG02	0.0391	0.5	01Jul2015, 12:30	0.2
G2	0.2820	1.9	01Jul2015, 13:18	1.0
G2-G3	0.2820	1.9	01Jul2015, 13:30	1.0
FG03	0.0203	0.8	01Jul2015, 12:12	0.2
FG04	0.0172	0.9	01Jul2015, 12:06	0.1
G3	0.3195	2.4	01Jul2015, 13:24	1.3
G3-POND F	0.3195	2.4	01Jul2015, 13:30	1.3
FG06	0.0675	1.3	01Jul2015, 12:24	0.4
FG05	0.0580	2.4	01Jul2015, 12:30	0.6
OS07a	0.0170	0.1	01Jul2015, 12:48	0.1
OS07a-POND F	0.0170	0.1	01Jul2015, 13:30	0.1
POND F IN	0.4620	5.0	01Jul2015, 12:48	2.4
POND F	0.4620	2.4	01Jul2015, 15:48	2.0
POND F-G7	0.4620	2.4	01Jul2015, 16:00	2.0
FG21b	0.0170	4.0	01Jul2015, 12:12	0.4
FG21a	0.0072	0.0	01Jul2015, 13:06	0.0
FG21a-G7	0.0072	0.0	01Jul2015, 14:06	0.0
G7	0.4862	4.0	01Jul2015, 12:12	2.5
G7-G8	0.4862	3.9	01Jul2015, 12:12	2.5
FG22	0.1380	3.3	01Jul2015, 12:30	1.0
OS08	0.0406	0.7	01Jul2015, 12:24	0.2
OS08-G8	0.0406	0.7	01Jul2015, 12:30	0.2
FG23a	0.0216	0.8	01Jul2015, 12:18	0.2
OS07b	0.0156	0.1	01Jul2015, 12:48	0.1
OS07b-G7	0.0156	0.1	01Jul2015, 13:00	0.1
G8	0.7020	7.9	01Jul2015, 12:24	3.9
G8-G10	0.7020	7.9	01Jul2015, 12:30	3.9
OS09	0.1527	1.9	01Jul2015, 12:54	0.8
OS09-G10	0.1527	1.9	01Jul2015, 13:18	0.8
FG24	0.1373	4	01Jul2015, 12:30	1.1
G9	0.2900	4	01Jul2015, 13:12	1.9
G9-G10	0.2900	4.4	01Jul2015, 13:12	1.9
FG23b	0.0286	0	01Jul2015, 12:30	0.2
G10	1.0206	12.1	01Jul2015, 12:30	5.9
G10-G11	1.0206	12.1	01Jul2015, 12:36	5.9
FG23c	0.0122	0.4	01Jul2015, 12:18	0.1
G11	1.0328	12.3	01Jul2015, 12:36	6.0
FG25	0.1086	7.5	01Jul2015, 12:36	1.7
FG26	0.0863	5	01Jul2015, 12:24	1.0
FG26-POND G	0.0863	4.5	01Jul2015, 12:30	0.9
FG27	0.0500	5.0	01Jul2015, 12:24	0.8
FG28	0.0245	0.5	01Jul2015, 12:30	0.2

Highlighted green rows reference key design points (Typical all charts this section)

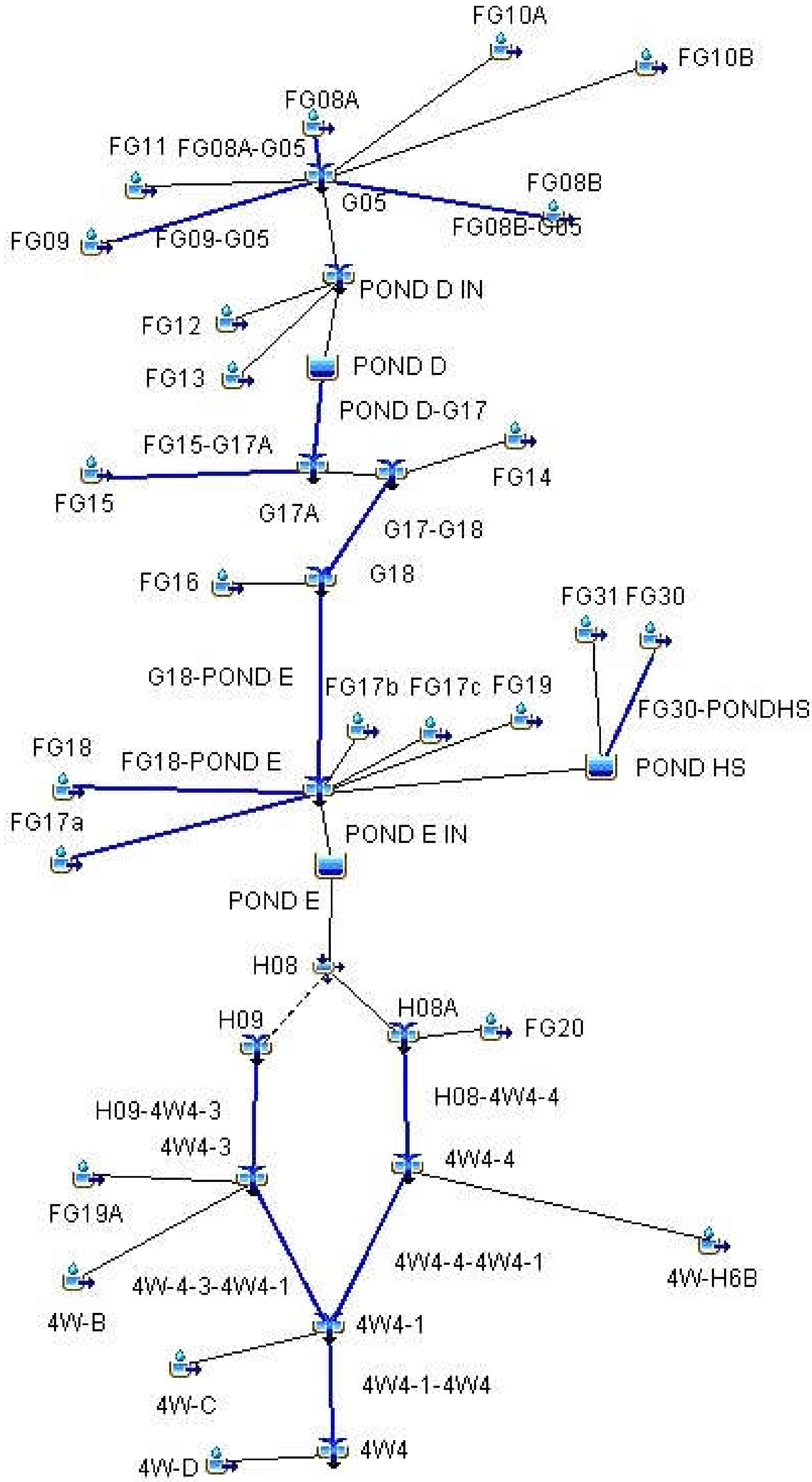
FUTURE MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
POND G IN	1.3022	28.5	01Jul2015, 12:30	9.5
POND G	1.3022	5	02Jul2015, 00:00	5.2
G12	1.3022	5	02Jul2015, 00:00	5.2
G12-G06	1.3022	5	02Jul2015, 00:00	5.1
FG29	0.0997	0.4	01Jul2015, 13:36	0.3
FG32	0.0402	11	01Jul2015, 12:06	1.0
FG32-G06	0.0402	11	01Jul2015, 12:12	1.0
G06	1.4421	11	01Jul2015, 12:12	6.4
FG08A	0.0750	13	01Jul2015, 12:12	1.5
FG08A-G05	0.0750	13.1	01Jul2015, 12:18	1.5
FG08B	0.0630	10.2	01Jul2015, 12:12	1.2
FG08B-G05	0.0630	10.0	01Jul2015, 12:18	1.2
FG09	0.0484	3.2	01Jul2015, 12:18	0.6
FG09-G05	0.0484	3	01Jul2015, 12:24	0.6
FG10B	0.0416	2.7	01Jul2015, 12:18	0.5
G05	0.2280	28.7	01Jul2015, 12:18	3.8
FG10A	0.0806	7	01Jul2015, 12:24	1.1
FG11	0.0625	9.8	01Jul2015, 12:18	1.4
FG13	0.0534	0.9	01Jul2015, 12:42	0.3
FG12	0.0328	7.8	01Jul2015, 12:12	0.8
POND D IN	0.4573	52.1	01Jul2015, 12:18	7.5
POND D	0.4573	4.3	01Jul2015, 18:00	4.0
POND D-G17	0.4573	4.3	01Jul2015, 18:06	4.0
FG15	0.0103	2	01Jul2015, 12:12	0.2
FG15-G17A	0.0103	2	01Jul2015, 12:12	0.2
G17A	0.4676	4	01Jul2015, 17:54	4.2
FG14	0.1000	9	01Jul2015, 12:24	1.6
G17	0.5676	12	01Jul2015, 12:24	5.8
G17-G18	0.5676	12	01Jul2015, 12:30	5.8
FG16	0.0791	18	01Jul2015, 12:06	1.9
G18	0.6467	26	01Jul2015, 12:12	7.7
G18-POND E	0.6467	25	01Jul2015, 12:12	7.6
FG31	0.0922	17	01Jul2015, 12:18	2.4
FG30	0.0389	11	01Jul2015, 12:06	1.0
FG30-PONDHS	0.0389	10.9	01Jul2015, 12:18	1.0
POND HS	0.1311	14.8	01Jul2015, 12:42	3.3
FG17a	0.0694	12	01Jul2015, 12:12	1.3
FG17a-POND E	0.0694	11.6	01Jul2015, 12:12	1.3
FG18	0.0644	4.7	01Jul2015, 12:30	0.9
FG18-POND E	0.0644	5	01Jul2015, 12:30	0.9
FG19	0.0527	13.1	01Jul2015, 12:12	1.4
FG17c	0.0313	0.5	01Jul2015, 12:18	0.2
FG17b	0.0214	6.1	01Jul2015, 12:06	0.6
POND E IN	1.0170	64.0	01Jul2015, 12:12	15.4
POND E	1.0170	6.6	02Jul2015, 00:00	5.9
H08	1.0170	4.1	02Jul2015, 00:00	3.6
H09	0.0000	2.4	02Jul2015, 00:00	2.3

Highlighted green rows reference key design points (Typical all charts this section)

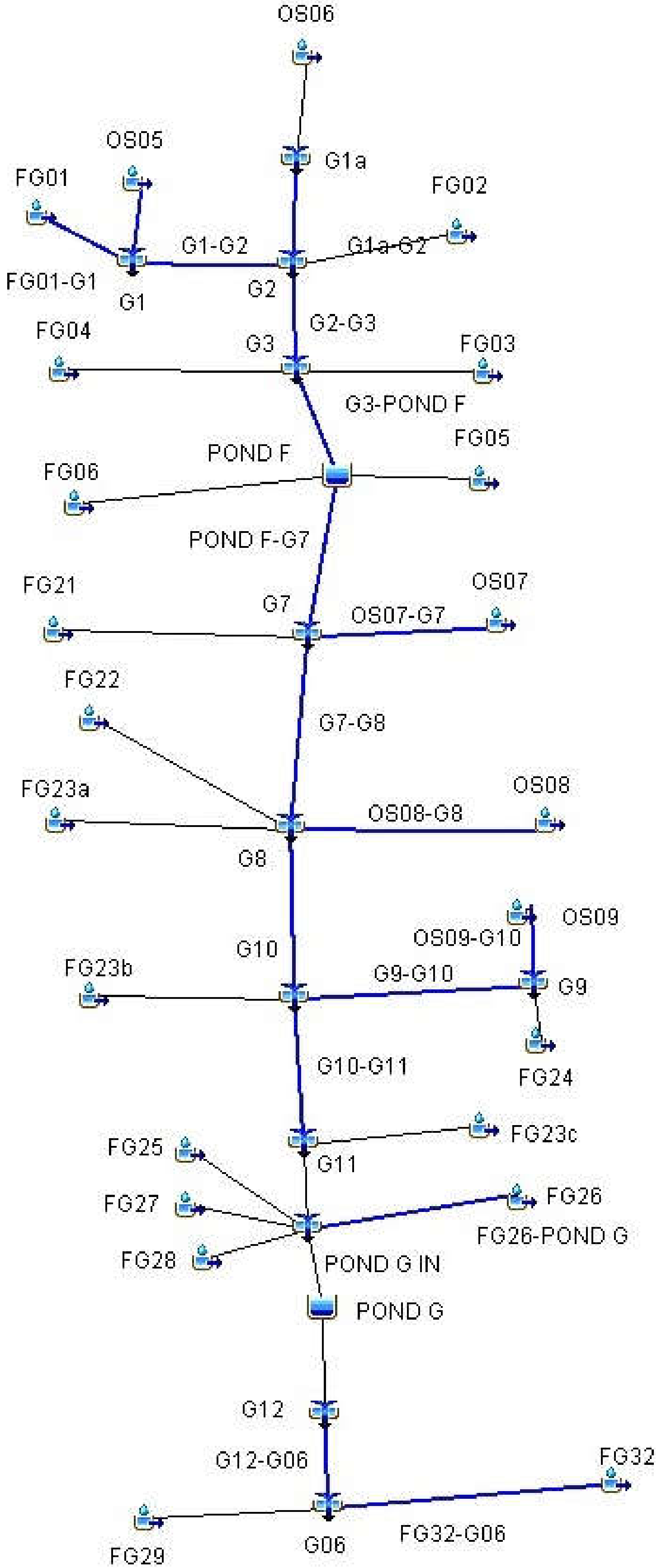
FUTURE MDDP (2-YEAR)				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q2 (CFS)	TIME OF PEAK	TOTAL VOLUME Q2 (AC. FT.)
FG34	0.0600	0.3	01Jul2015, 13:18	0.2
G14	0.0600	0.3	01Jul2015, 13:18	0.2
G14-G15	0.0600	0.3	01Jul2015, 13:48	0.2
FG35	0.0344	0.3	01Jul2015, 13:06	0.1
G15	0.0944	0.6	01Jul2015, 13:36	0.3
G15-G08	0.0944	0.6	01Jul2015, 13:48	0.3
FG37	0.0797	0.3	01Jul2015, 13:42	0.2
FG36	0.0281	0.1	01Jul2015, 13:42	0.1
FG36-G08	0.0281	0.1	01Jul2015, 14:00	0.1
G08	0.2022	1.0	01Jul2015, 13:48	0.6

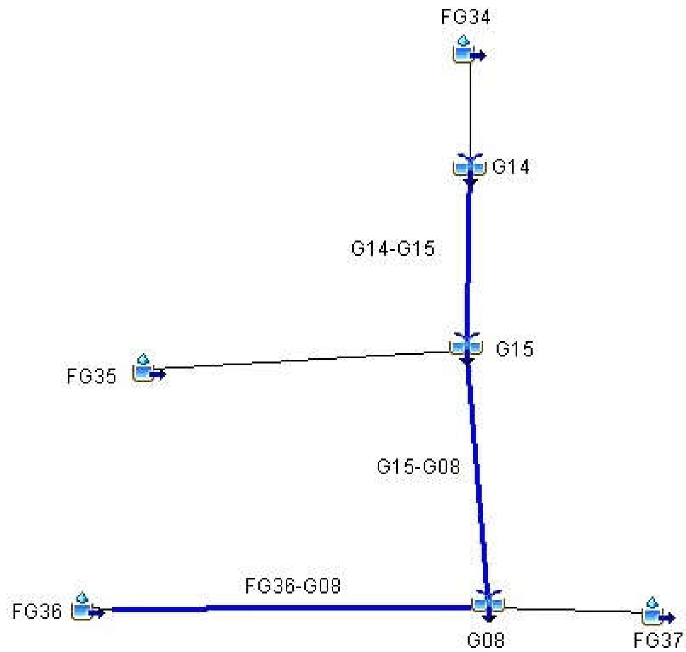
Highlighted green rows reference key design points (Typical all charts this section)

HAEGLER FUTURE CONDITIONS



GIECK FUTURE CONDITIONS




FG34
G14
G14-G15
G15
FG35
G15-G08
FG36-G08
FG36
G08
FG37

Appendix C - Detention Pond Information

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond D - Interim AS-BUILT

Geick Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	710
embankment elev =	7060
spillway length =	100
spillway elevation =	7058
100 year storage elev.=	7057.1
100 year storage vol.=	25.5
100 year discharge=	134
5 year storage elev.=	7053.9
5 year storage vol.=	7.4
5 year discharge=	12
WQCV storage vol.=	1.0
WQCV depth =	2.42
1/2 WQCV storage vol.=	0.50

Data for outlet pipe and grate:

Type			Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular		Orifice 1:	0.03	2.42		Area =	0.072	Elev to cl =	7050.21
Circular		Orifice 2:			8	Area =	0.349	Elev to cl =	7051.42
Rectangular		Orifice 3:	5	0.5		Area =	2.500	Elev to cl =	7053.35
None Selected		Orifice 4:				Area =	0.000	Elev to cl =	
Stand Pipe Dimensions									
Rec Grate		6	x	4.25	Elev =	7054.9			
Circ. Grate			dia.		Elev =				
Outlet Culvert Dimensions									
		Width (ft.)		Height (ft.)	Dia. (ft.)	Type			
Outlet Culvert			x		4	**Circular**			
Area		12.6		TOP					
Outlet I. E.		7048.1		7052.5					
Wall Thick.		5	in.						

50 year storage elev.=	7056.3
50 year discharge=	91
25 year storage elev.=	7055.6
25 year discharge=	50
10 year storage elev.=	7054.7
10 year discharge=	19
2 year storage elev.=	7053.2
2 year discharge=	4.3

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
7049	0	0	0.0	0.00	0.00			-								
7050	1	10705	0.2	0.1	0.12	-	-	0.2	-	-	-	-	13		0.2	0.15
7051	2	36676	0.8	0.5	0.67	-	-	0.3	-	-	-	-	33		0.3	0.31
7052	3	71989	1.7	1.2	1.91	-	-	0.5	1.3	-	-	-	60		1.8	1.8
7053	4	133440	3.1	2.4	4.27	-	-	0.6	2.1	-	-	-	90		2.7	2.7
7054	5	178828	4.1	3.6	7.86	-	-	0.7	2.7	9.7	-	-	119		13.1	13
7055	6	221269	5.1	4.6	12.45	-	-	0.8	3.2	15.5	-	1.4	139		21	21
7055.5	6.5	245509	5.6	2.7	15.13	-	-	0.8	3.4	17.7	-	20.2	148		42	42
7056	7	269749	6.2	5.6	18.08	-	-	0.8	3.6	20	-	50	157		74	74
7058	9	337508	7.7	13.9	32.03	-	-	1.0	4.3	26	-	216	188		188	188
7060	11	405520	9.3	31.0	49.09	-	848.5	1.1	4.9	31	-	277	214		214	1,063
						-	-	-	-	-	-	-			-	-

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E- FINAL INTERIM (TOTAL FLOWS)

Gieck Basin - El Paso County, Colorado

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974.5
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	242
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	16
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
WQCV depth =	1.9
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.75

50 year storage elev.=	6973.1
50 year storage vol.=	36.4
50 year discharge=	153
25 year storage elev.=	6972.5
25 year storage vol.=	30.0
25 year discharge=	80
10 year storage elev.=	6971.9
10 year storage vol.=	23.0
10 year discharge=	30
2 year storage elev.=	6970.6
2 year storage vol.=	10.9
2 year discharge=	6.6

STAGE		STORAGE				TOTAL DISCHARGE											
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)		PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft			1	2	3	4	**Rectangular**		1	2		
6967	0	1808	0.04	0.0	0.00			-	-	-	-	-	-	1.4	-	-	-
6967.5	0.5	16136.5	0.37	0.1	0.10	-	-	0.0	-	-	-	-	-	13	-	0.03	0.03
6968	1	30465	0.70	0.3	0.37	-	-	0.1	-	-	-	-	-	26	-	0.11	0.11
6968.5	1.5	81028.5	1.86	0.6	1.01	-	-	0.2	-	-	-	-	-	47	-	0.23	0.23
6969	2	131592	3.02	1.2	2.23	-	-	0.4	-	-	-	-	-	77	-	0.4	0.37
6969.5	2.5	201294.5	4.62	1.9	4.14	-	-	0.5	-	3.0	-	-	-	110	-	3.5	3.5
6970	3	270997	6.22	4.6	6.85	-	-	0.6	-	4	-	-	-	146	-	5	4.9
6970.5	3.5	329360	7.56	3.4	10.30	-	-	0.6	0.2	5	-	-	-	183	-	6	6.1
6970.75	3.75	358540.75	8.23	2.0	12.27			0.7	1.2	6	-	-	-	203	-	8	7.6
6971	4	387722	8.90	7.6	14.41	-	-	0.7	3.1	6	-	-	-	218	-	10	9.8
6971.25	4.25	408751	9.38	2.3	16.70	-	-	0.7	5.5	6	0.20	-	-	236	-	13	13
6971.5	4.5	429780	9.87	4.7	19.10	-	-	0.7	8	7	3.0	-	-	252	-	18	18
6971.75	4.75	450809	10.35	2.5	21.63	-	-	0.8	10	7	7.3	-	-	266	-	25	25
6972	5	471838	10.83	5.2	24.28	-	-	0.8	12	7	13	2.4	-	280	-	35	35
6972.25	5.25	482595.75	11.08	2.7	27.02	-	-	0.8	13	8	17	16	-	292	-	54	54
6972.5	5.5	493354	11.33	5.5	29.82	-	-	0.8	14	8	20	35	-	304	-	78	78
6973	6	514869	11.82	5.8	35.60	-	-	0.9	16	9	30	87	-	327	-	142	142
6973.25	6.25	518272	11.90	3.0	38.57	-	-	0.9	17	9	35	121	-	338	-	183	183
6973.5	6.5	521675	11.98	5.9	41.55	-	-	0.9	18	9	41	163	-	349	-	232	232
6974	7	528481	12.13	12.0	47.58	-	-	1.0	20	10	53	259	-	369	-	307	307
6976	9	553685	12.71	24.8	72.42	-	1,102	1.1	25	11	83	729	-	443	-	443	1,545

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E-INTERIM FUTURE (H08)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	205
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	12
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.8

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0248	1.65		Area =	0.041	Invert Elev =	6967.18
Rectangular	Orifice 2:	V	2	0.8		Area =	1.600	Invert Elev =	6970.40
Circular	Orifice 3:	H			10	Area =	0.545	Invert Elev =	6969.00
Rectangular	Orifice 4:	V	6	0.7		Area =	4.200	Invert Elev =	6971.20

Stand Pipe Dimensions

Rec Grate		11	x	7	Elev =	6971.90
Circ. Grate			dia.		Elev =	6971.90

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		3.5	**Circular**
Area	9.6		TOP		
Outlet I. E.	6966.8		6970.58		
Wall Thick.	4	in.			

50 year storage elev.=	6973.1
50 year discharge=	137
25 year storage elev.=	6972.5
25 year discharge=	72
10 year storage elev.=	6971.9
10 year discharge=	24
2 year storage elev.=	6970.6
2 year discharge=	4.1

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
6967	0	1808	0.04	0.0	0.0			-	-	-	-	-	0.91		-	-
6967.5	0.5	16136.5	0.37	0.1	0.1			0.0	-	-	-	-	8.0		0.01	0.01
6968	1	30465	0.70	0.3	0.4			0.1	-	-	-	-	18		0.06	0.06
6968.5	1.5	81028.5	1.86	0.6	1.0			0.1	-	-	-	-	30		0.11	0.11
6969	2	131592	3.02	1.2	2.2			0.2	-	-	-	-	52		0.2	0.2
6969.5	2.5	201294.5	4.62	1.9	4.1			0.2	-	1.9	-	-	75		2.1	2.1
6970	3	270997	6.22	4.6	6.9			0.3	-	2.6	-	-	97		2.9	2.9
6970.5	3.5	329359.5	7.56	3.4	10			0.3	0.2	3.2	-	-	122		3.7	3.7
6970.75	3.75	358540.75	8.23	2.0	12.3			0.3	1.2	3.5	-	-	135		5	5.0
6971	4	387722	8.90	7.6	14			0.3	2.8	3.7	-	-	146		7	6.8
6971.25	4.25	408751	9.38	2.3	17			0.4	4.7	3.9	0.2	-	157		9	9.2
6971.5	4.5	429780	9.87	4.7	19			0.4	6.4	4	3.0	-	167		14	14
6971.75	4.75	450809	10.35	2.5	22			0.4	7.5	4	7.3	-	176		20	20
6972	5	471838	10.83	5.2	24			0.4	8	5	13	2	185		29	29
6972.25	5.25	482595.75	11.08	2.7	27			0.4	9	5	17	16	193		47	47
6972.5	5.5	493354	11.33	5.5	30			0.4	10	5	20	35	201		70	70
6973	6	514869	11.82	5.8	36			0.4	11	5	24	87	217		128	128
6973.25	6.25	518272	11.90	3.0	39			0.5	12	5	26	118	224		162	162
6973.5	6.5	521675	11.98	5.9	42			0.5	13	6	28	152	231		199	199
6974	7	528481	12.13	12.0	48			0.5	14	6	32	228	244		244	244
6976	9	553685	12.71	24.8	72			0.6	18	7	43	623	291		291	291

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E-INTERIM FUTURE (H09)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974.5
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	37
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	4.1
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.75

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0248	1.65		Area =	0.041	Invert Elev =	6967.18
Rectangular	Orifice 2:	V	0.75	1		Area =	0.750	Invert Elev =	6970.75
Circular	Orifice 3:	H			8	Area =	0.349	Invert Elev =	6969.00
Rectangular	Orifice 4:	V	3.5	1.25		Area =	4.375	Invert Elev =	6971.75

Stand Pipe Dimensions

Rec Grate	4.25	3	Elev =	6973.00
Circ. Grate			Elev =	6973.00

Outlet Culvert Dimensions

	Width (ft.)	Height (ft.)	Dia. (ft.)	Type
Outlet Culvert			3.5	**Circular**
Area	9.6	TOP		
Outlet I. E.	6966.8	6970.7		
Wall Thick.	5 in.			

50 year storage elev.=	6973.1
50 year discharge=	16
25 year storage elev.=	6972.5
25 year discharge=	8.3
10 year storage elev.=	6971.9
10 year discharge=	5.9
2 year storage elev.=	6970.6
2 year discharge=	2.4

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
6967	0	1808	0.04	0.0	0.0			-	-	-	-	-	0.45		-	-
6967.5	0.5	16136.5	0.37	0.1	0.1			0.0	-	-	-	-	5.0		0.01	0.01
6968	1	30465	0.70	0.3	0.4			0.1	-	-	-	-	8.8		0.06	0.06
6968.5	1.5	81028.5	1.86	0.6	1.0			0.1	-	-	-	-	17		0.11	0.11
6969	2	131592	3.02	1.2	2.2			0.2	-	-	-	-	26		0.2	0.18
6969.5	2.5	201294.5	4.62	1.9	4.1			0.2	-	1.2	-	-	35		1.4	1.4
6970	3	270997	6.22	4.6	6.9			0.3	-	1.7	-	-	48		2.0	2.0
6970.5	3.5	329359.5	7.56	3.4	10.3			0.3	-	2.1	-	-	61		2.4	2.4
6970.75	3.75	358540.75	8.23	2.0	12.3			0.3	-	2.2	-	-	68		2.6	2.6
6971	4	387722	8.90	7.6	14.4			0.3	0.3	2.4	-	-	73		3.0	3.0
6971.25	4.25	408751	9.38	2.3	16.7			0.4	0.8	2.5	-	-	79		3.7	3.7
6971.5	4.5	429780	9.87	4.7	19.1			0.4	1.5	2.7	-	-	85		4.5	4.5
6971.75	4.75	450809	10.35	2.5	21.6			0.4	2.3	2.8	-	-	90		5.4	5.4
6972	5	471838	10.83	5.2	24.3			0.4	3.1	2.9	-	-	95		6.4	6.4
6972.25	5.25	482595.75	11.08	2.7	27.0			0.4	3.6	3.0	-	-	99		7.0	7.0
6972.5	5.5	493354	11.33	5.5	29.8			0.4	4.0	3.1	0.5	-	103		8	8.1
6973	6	514869	11.82	5.8	35.6			0.4	4.8	3.4	5.2	-	111		14	14
6973.25	6.25	518272	11.90	3.0	38.6			0.5	5.1	3.5	8.6	4	114		21	21
6973.5	6.5	521675	11.98	5.9	41.6			0.5	5.4	3.6	13	11	118		33	33
6974	7	528481	12.13	12.0	47.6			0.5	6.0	3.8	22	31	125		63	63
6976	9	553685	12.71	24.8	72.4			0.6	7.9	4.4	40	106	151		151	151

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond F INTERIM-Final

Geick Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	285
embankment elev =	7138.5
spillway length =	87
spillway elevation =	7137.5
100 year storage elev.=	7136.0
100 year storage vol.=	8.8
100 year discharge=	177
5 year storage elev.=	7131.2
5 year storage vol.=	1.8
5 year discharge=	8.0
WQCV storage elev.=	7129.1
WQCV storage vol.=	0.3
1/2 WQCV storage elev.=	7128.6
1/2 WQCV storage vol.=	0.15

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0131	1.25		Area =	0.016	Elev to cl =	7128.45
Rectangular	Orifice 2:	V	4	0.5		Area =	2.000	Elev to cl =	7130.75
Circular	Orifice 3:	H			8	Area =	0.349	Elev to cl =	7129.20
None Selected	Orifice 4:					Area =	0.000	Elev to cl =	

Stand Pipe Dimensions

Rec Grate	6	x	3	Elev =	7133
Circ. Grate		dia.		Elev =	7133

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		4	**Circular**
Area	12.6		TOP		
Outlet I. E.	7126.6		7131.0		
Wall Thick.	5	in.			

50 year storage elev.=	7134.9
50 year discharge=	121
25 year storage elev.=	7134.1
25 year discharge=	60
10 year storage elev.=	7132.7
10 year discharge=	17
2 year storage elev.=	7130.1
2 year discharge=	2.3

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
7127.7	0	0	0.00	0.00	0.00			-	-	-	-	-				
7128	0.3	2170	0.05	0.01	0.01	-	-	0.0	-	-	-	-	11		0.0	0.0
7129	1.3	17730	0.41	0.23	0.24	-	-	0.1	-	-	-	-	31		0.1	0.1
7130	2.3	33290	0.76	0.59	0.82	-	-	0.1	-	1.5	-	-	57		1.6	1.6
7131	3.3	39060	0.90	0.83	1.65	-	-	0.1	4.2	2.3	-	-	117		6.6	6.6
7132	4.3	44830	1.03	0.96	2.61	-	-	0.1	10.8	2.8	-	-	117		14	14
7133	5.3	55137.5	1.27	1.15	3.76	-	-	0.2	14.4	3.3	-	-	142		18	18
7134	6.3	65445	1.50	1.38	5.15	-	-	0.2	17.4	3.7	-	36	162		57	57
7135	7.3	79535	1.83	1.66	6.81	-	-	0.2	19.9	4.0	-	102	175		126	126
7136	8.3	93625	2.15	1.99	8.80	-	-	0.2	22.1	4.4	-	150	187		177	177
7137	9.3	111620	2.56	2.36	11.15	-	-	0.2	24.1	4.7	-	173	200		200	200
7138	10.3	129615	2.98	2.77	13.92	-	92.3	0.2	25.9	5.0	-	194	211		211	303
7138.5	10.8					-	261.0	0.3	26.8	5.1	-	203	211		-	261

Notes: 1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond G-FINAL INTERIM DESIGN (G12)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	500
embankment elev =	7034
spillway length =	130
spillway elevation =	7031.5
100 year storage elev.=	7030.0
100 year storage vol.=	24.1
100 year discharge=	399
5 year storage elev.=	7027.4
5 year storage vol.=	8.1
5 year discharge=	13
WQCV storage elev.=	7025.8
WQCV storage vol.=	0.9
1/2 WQCV storage elev.=	7024.9
1/2 WQCV storage vol.=	0.45

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0414	1.40		Area =	0.058	Elev to cl =	7024.00
Rectangular	Orifice 2:	V	8.5	1.1		Area =	9.350	Elev to cl =	7027.55
Circular	Orifice 3:	H			12	Area =	0.785	Elev to cl =	7025.40
Rectangular	Orifice 4:	V	4	0.6		Area =	2.400	Elev to cl =	7027.80
Rectangular	Orifice 5:	V	8.5	1.1		Area =	9.350	Elev to cl =	7027.55

Stand Pipe Dimensions

Rec Grate	20	x	8	Elev =	7028.10
Circ. Grate		dia.		Elev =	7028.10

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert	10	x	4		**Rectangular**
Area	40.0		TOP		
Outlet I. E.	7022.5		7027.50		
Wall Thick.	12	in.			

50 year storage elev.=	7029.4
50 year discharge=	237
25 year storage elev.=	7028.8
25 year discharge=	117
10 year storage elev.=	7028.0
10 year discharge=	35
2 year storage elev.=	7026.4
2 year discharge=	4.2

STAGE		STORAGE				DISCHARGE											
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)					GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft			1	2	3	4	5	Rectangular	1	2		
7023.3	0	0	0.00	0.0	0.00			-	-	-	-	-	-	12		-	-
7024	0.7	2232	0.05	0.0	0.02	-	-	0.1	-	-	-	-	-	51		0.1	0.07
7025	1.7	39917	0.92	0.5	0.50	-	-	0.3	-	-	-	-	-	111		0.3	0.28
7026	2.7	126469	2.90	1.9	2.41	-	-	0.4	-	2.9	-	-	-	184		3.3	3.3
7026.5	3.2	166675	3.83	3.6	4.06	-	-	0.4	-	4.0	-	-	-	224		4.4	4.4
7027	3.7	206880	4.75	2.1	6.20	-	-	0.5	-	4.8	-	-	-	268		5.3	5.3
7027.5	4.2	232032	5.33	4.6	8.64	-	-	0.5	9.0	5.5	-	9.0	-	304		15	15
7028	4.7	257183	5.90	5.3	11.53	-	-	0.6	25.5	6.1	4.2	25.5	-	337		36	36
7028.5	5.2	264196	6.07	5.7	14.33	-	-	0.6	43.9	6.7	9.7	43.9	27	373		88	88
7029	5.7	271209	6.23	6.1	17.59	-	-	0.6	54.2	7.2	12.7	54.2	92	406		167	167
7029.5	6.2	276106	6.34	11.7	20.30	-	-	0.7	70.5	7.7	17.1	70.5	179	436		275	275
7030	6.7	281003	6.45	9.4	23.72	-	-	0.7	77.3	8.1	19.0	77.3	283	464		388	388
7030.5	7.2	286003	6.57	6.5	26.75	-	-	0.7	77.3	8.5	19.0	77.3	402	491		491	491
7031	7.7	291002	6.68	6.6	30.28	-	-	0.7	83.6	8.9	20.7	83.6	533	516		516	516
7031.5	8.2	296443	6.81	6.7	33.44	-	-	0.8	89.5	9.3	22.2	89.5	677	540		540	540
7032	8.7	301883	6.93	3.4	36.87	137.9	137.9	0.8	95.0	9.7	23.7	95.0	832	563		563	701
7032.5	9.2	309236	7.10	7.0	40.39	390.0	390.0	0.8	100.2	10.1	25.1	100.2	997	586		586	976
7033	9.7	316589	7.27	3.6	44.0	716.5	716.5	0.8	105.1	10.4	26.4	105.1	1,171	607		607	1,323

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond D - Future AS-BUILT

Geick Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	710
embankment elev =	7060
spillway length =	100
spillway elevation =	7058
100 year storage elev.=	7057.1
100 year storage vol.=	25.5
100 year discharge=	134
5 year storage elev.=	7053.9
5 year storage vol.=	7.3
5 year discharge=	12
WQCV storage vol.=	1.0
WQCV depth =	2.42
1/2 WQCV storage vol.=	0.50

Data for outlet pipe and grate:

Type		Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	0.03	2.42		Area =	0.072	Elev to cl =	7050.21
Circular	Orifice 2:			8	Area =	0.349	Elev to cl =	7051.42
Rectangular	Orifice 3:	5	0.5		Area =	2.500	Elev to cl =	7053.35
None Selected	Orifice 4:				Area =	0.000	Elev to cl =	

Stand Pipe Dimensions

Rec Grate	6	x	4.25	Elev =	7054.9
Circ. Grate		dia.		Elev =	

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		4	**Circular**
Area	12.6		TOP		
Outlet I. E.	7048.1		7052.5		
Wall Thick.	5	in.			

50 year storage elev.=	7056.3
50 year discharge=	91
25 year storage elev.=	7055.6
25 year discharge=	50
10 year storage elev.=	7054.7
10 year discharge=	19
2 year storage elev.=	7053.2
2 year discharge=	4.3

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft			1	2	3	4	**Rectangular**	1	2		
7049	0	0	0.0	0.00	0.00			-								
7050	1	10705	0.2	0.1	0.12	-	-	0.2	-	-	-	-	13		0.2	0.15
7051	2	36676	0.8	0.5	0.67	-	-	0.3	-	-	-	-	33		0.3	0.31
7052	3	71989	1.7	1.2	1.91	-	-	0.5	1.3	-	-	-	60		1.8	1.8
7053	4	133440	3.1	2.4	4.27	-	-	0.6	2.1	-	-	-	90		2.7	2.7
7054	5	178828	4.1	3.6	7.86	-	-	0.7	2.7	9.7	-	-	119		13.1	13
7055	6	221269	5.1	4.6	12.45	-	-	0.8	3.2	15.5	-	1.4	139		21	21
7055.5	6.5	245509	5.6	2.7	15.13	-	-	0.8	3.4	17.7	-	20.2	148		42	42
7056	7	269749	6.2	5.6	18.08	-	-	0.8	3.6	20	-	50	157		74	74
7058	9	337508	7.7	13.9	32.03	-	-	1.0	4.3	26	-	216	188		188	188
7060	11	405520	9.3	31.0	49.09	-	848.5	1.1	4.9	31	-	277	214		214	1,063
						-	-	-	-	-	-	-			-	-

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E- FINAL FUTURE (TOTAL FLOWS)

Gieck Basin - El Paso County, Colorado

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974.5
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	242
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	16
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
WQCV depth =	1.9
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.75

50 year storage elev.=	6973.1
50 year storage vol.=	36.4
50 year discharge=	153
25 year storage elev.=	6972.5
25 year storage vol.=	30.0
25 year discharge=	80
10 year storage elev.=	6971.9
10 year storage vol.=	23.0
10 year discharge=	30
2 year storage elev.=	6970.6
2 year storage vol.=	10.9
2 year discharge=	6.6

STAGE		STORAGE				TOTAL DISCHARGE											
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)		PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft			1	2	3	4	Rectangular		1	2		
6967	0	1808	0.04	0.0	0.00			-	-	-	-	-	-	1.4	-	-	-
6967.5	0.5	16136.5	0.37	0.1	0.10	-	-	0.0	-	-	-	-	-	13	-	0.03	0.03
6968	1	30465	0.70	0.3	0.37	-	-	0.1	-	-	-	-	-	26	-	0.11	0.11
6968.5	1.5	81028.5	1.86	0.6	1.01	-	-	0.2	-	-	-	-	-	47	-	0.23	0.23
6969	2	131592	3.02	1.2	2.23	-	-	0.4	-	-	-	-	-	77	-	0.4	0.37
6969.5	2.5	201294.5	4.62	1.9	4.14	-	-	0.5	-	3.0	-	-	-	110	-	3.5	3.5
6970	3	270997	6.22	4.6	6.85	-	-	0.6	-	4	-	-	-	146	-	5	4.9
6970.5	3.5	329360	7.56	3.4	10.30	-	-	0.6	0.2	5	-	-	-	183	-	6	6.1
6970.75	3.75	358540.75	8.23	2.0	12.27			0.7	1.2	6	-	-	-	203	-	8	7.6
6971	4	387722	8.90	7.6	14.41	-	-	0.7	3.1	6	-	-	-	218	-	10	9.8
6971.25	4.25	408751	9.38	2.3	16.70	-	-	0.7	5.5	6	0.20	-	-	236	-	13	13
6971.5	4.5	429780	9.87	4.7	19.10	-	-	0.7	8	7	3.0	-	-	252	-	18	18
6971.75	4.75	450809	10.35	2.5	21.63	-	-	0.8	10	7	7.3	-	-	266	-	25	25
6972	5	471838	10.83	5.2	24.28	-	-	0.8	12	7	13	2.4	-	280	-	35	35
6972.25	5.25	482595.75	11.08	2.7	27.02	-	-	0.8	13	8	17	16	-	292	-	54	54
6972.5	5.5	493354	11.33	5.5	29.82	-	-	0.8	14	8	20	35	-	304	-	78	78
6973	6	514869	11.82	5.8	35.60	-	-	0.9	16	9	30	87	-	327	-	142	142
6973.25	6.25	518272	11.90	3.0	38.57	-	-	0.9	17	9	35	121	-	338	-	183	183
6973.5	6.5	521675	11.98	5.9	41.55	-	-	0.9	18	9	41	163	-	349	-	232	232
6974	7	528481	12.13	12.0	47.58	-	-	1.0	20	10	53	259	-	369	-	307	307
6976	9	553685	12.71	24.8	72.42	-	1,102	1.1	25	11	83	729	-	443	-	443	1,545

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E-FINAL FUTURE (H08)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	205
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	12
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.75

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0248	1.65		Area =	0.041	Invert Elev =	6967.18
Rectangular	Orifice 2:	V	2	0.8		Area =	1.600	Invert Elev =	6970.40
Circular	Orifice 3:	H			10	Area =	0.545	Invert Elev =	6969.00
Rectangular	Orifice 4:	V	6	0.7		Area =	4.200	Invert Elev =	6971.20

Stand Pipe Dimensions

Rec Grate		11	x	7	Elev =	6971.90
Circ. Grate			dia.		Elev =	6971.90

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		3.5	**Circular**
Area	9.6		TOP		
Outlet I. E.	6966.8		6970.58		
Wall Thick.	4	in.			

50 year storage elev.=	6973.1
50 year discharge=	137
25 year storage elev.=	6972.5
25 year discharge=	72
10 year storage elev.=	6971.9
10 year discharge=	24
2 year storage elev.=	6970.6
2 year discharge=	4.1

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
6967	0	1808	0.04	0.0	0.0			-	-	-	-	-	0.91		-	-
6967.5	0.5	16136.5	0.37	0.1	0.1			0.0	-	-	-	-	8.0		0.01	0.01
6968	1	30465	0.70	0.3	0.4			0.1	-	-	-	-	18		0.06	0.06
6968.5	1.5	81028.5	1.86	0.6	1.0			0.1	-	-	-	-	30		0.11	0.11
6969	2	131592	3.02	1.2	2.2			0.2	-	-	-	-	52		0.2	0.2
6969.5	2.5	201294.5	4.62	1.9	4.1			0.2	-	1.9	-	-	75		2.1	2.1
6970	3	270997	6.22	4.6	6.9			0.3	-	2.6	-	-	97		2.9	2.9
6970.5	3.5	329359.5	7.56	3.4	10			0.3	0.2	3.2	-	-	122		3.7	3.7
6970.75	3.75	358540.75	8.23	2.0	12.3			0.3	1.2	3.5	-	-	135		5	5.0
6971	4	387722	8.90	7.6	14			0.3	2.8	3.7	-	-	146		7	6.8
6971.25	4.25	408751	9.38	2.3	17			0.4	4.7	3.9	0.2	-	157		9	9.2
6971.5	4.5	429780	9.87	4.7	19			0.4	6.4	4	3.0	-	167		14	14
6971.75	4.75	450809	10.35	2.5	22			0.4	7.5	4	7.3	-	176		20	20
6972	5	471838	10.83	5.2	24			0.4	8	5	13	2	185		29	29
6972.25	5.25	482595.75	11.08	2.7	27			0.4	9	5	17	16	193		47	47
6972.5	5.5	493354	11.33	5.5	30			0.4	10	5	20	35	201		70	70
6973	6	514869	11.82	5.8	36			0.4	11	5	24	87	217		128	128
6973.25	6.25	518272	11.90	3.0	39			0.5	12	5	26	118	224		162	162
6973.5	6.5	521675	11.98	5.9	42			0.5	13	6	28	152	231		199	199
6974	7	528481	12.13	12.0	48			0.5	14	6	32	228	244		244	244
6976	9	553685	12.71	24.8	72			0.6	18	7	43	623	291		291	291

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Existing Detention Pond E-FINAL FUTURE (H09)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	1860
embankment elev =	6976
spillway length =	200
spillway elevation =	6974.5
100 year storage elev.=	6973.6
100 year storage vol.=	42.4
100 year discharge=	37
5 year storage elev.=	6971.4
5 year storage vol.=	18.0
5 year discharge=	4.1
WQCV storage elev.=	6968.9
WQCV storage vol.=	1.5
1/2 WQCV storage elev.=	6968.3
1/2 WQCV storage vol.=	0.8

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0248	1.65		Area =	0.041	Invert Elev =	6967.18
Rectangular	Orifice 2:	V	0.75	1		Area =	0.750	Invert Elev =	6970.75
Circular	Orifice 3:	H			8	Area =	0.349	Invert Elev =	6969.00
Rectangular	Orifice 4:	V	3.5	1.25		Area =	4.375	Invert Elev =	6971.75

Stand Pipe Dimensions

Rec Grate		4.25	x	3	Elev =	6973.00
Circ. Grate			dia.		Elev =	6973.00

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		3.5	**Circular**
Area	9.6		TOP		
Outlet I. E.	6966.8		6970.7		
Wall Thick.	5	in.			

50 year storage elev.=	6973.1
50 year discharge=	16
25 year storage elev.=	6972.5
25 year discharge=	8.3
10 year storage elev.=	6971.9
10 year discharge=	5.9
2 year storage elev.=	6970.6
2 year discharge=	2.4

STAGE		STORAGE				DISCHARGE										
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT	TOTAL
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2	OUTFLOW	FLOW
6967	0	1808	0.04	0.0	0.0			-	-	-	-	-	0.45		-	-
6967.5	0.5	16136.5	0.37	0.1	0.1			0.0	-	-	-	-	5.0		0.01	0.01
6968	1	30465	0.70	0.3	0.4			0.1	-	-	-	-	8.8		0.06	0.06
6968.5	1.5	81028.5	1.86	0.6	1.0			0.1	-	-	-	-	17		0.11	0.11
6969	2	131592	3.02	1.2	2.2			0.2	-	-	-	-	26		0.2	0.18
6969.5	2.5	201294.5	4.62	1.9	4.1			0.2	-	1.2	-	-	35		1.4	1.4
6970	3	270997	6.22	4.6	6.9			0.3	-	1.7	-	-	48		2.0	2.0
6970.5	3.5	329359.5	7.56	3.4	10.3			0.3	-	2.1	-	-	61		2.4	2.4
6970.75	3.75	358540.75	8.23	2.0	12.3			0.3	-	2.2	-	-	68		2.6	2.6
6971	4	387722	8.90	7.6	14.4			0.3	0.3	2.4	-	-	73		3.0	3.0
6971.25	4.25	408751	9.38	2.3	16.7			0.4	0.8	2.5	-	-	79		3.7	3.7
6971.5	4.5	429780	9.87	4.7	19.1			0.4	1.5	2.7	-	-	85		4.5	4.5
6971.75	4.75	450809	10.35	2.5	21.6			0.4	2.3	2.8	-	-	90		5.4	5.4
6972	5	471838	10.83	5.2	24.3			0.4	3.1	2.9	-	-	95		6.4	6.4
6972.25	5.25	482595.75	11.08	2.7	27.0			0.4	3.6	3.0	-	-	99		7.0	7.0
6972.5	5.5	493354	11.33	5.5	29.8			0.4	4.0	3.1	0.5	-	103		8	8.1
6973	6	514869	11.82	5.8	35.6			0.4	4.8	3.4	5.2	-	111		14	14
6973.25	6.25	518272	11.90	3.0	38.6			0.5	5.1	3.5	8.6	4	114		21	21
6973.5	6.5	521675	11.98	5.9	41.6			0.5	5.4	3.6	13	11	118		33	33
6974	7	528481	12.13	12.0	47.6			0.5	6.0	3.8	22	31	125		63	63
6976	9	553685	12.71	24.8	72.4			0.6	7.9	4.4	40	106	151		151	151

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond F-Final

Geick Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	285
embankment elev =	7138.5
spillway length =	87
spillway elevation =	7137.5
100 year storage elev.=	7136.0
100 year storage vol.=	8.8
100 year discharge=	177
5 year storage elev.=	7131.2
5 year storage vol.=	1.9
5 year discharge=	8.1
WQCV storage elev.=	7129.1
WQCV storage vol.=	0.3
1/2 WQCV storage elev.=	7128.6
1/2 WQCV storage vol.=	0.15

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0131	1.25		Area =	0.016	Elev to cl =	7128.45
Rectangular	Orifice 2:	V	4	0.5		Area =	2.000	Elev to cl =	7130.75
Circular	Orifice 3:	H			8	Area =	0.349	Elev to cl =	7129.20
None Selected	Orifice 4:					Area =	0.000	Elev to cl =	

Stand Pipe Dimensions

Rec Grate		6	x	3	Elev =	7133
Circ. Grate			dia.		Elev =	7133

50 year storage elev.=	7134.9
50 year discharge=	121
25 year storage elev.=	7134.1
25 year discharge=	61
10 year storage elev.=	7132.7
10 year discharge=	17
2 year storage elev.=	7130.1
2 year discharge=	2.3

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x		4	**Circular**
Area	12.6		TOP		
Outlet I. E.	7126.6		7131.0		
Wall Thick.	5	in.			

STAGE		STORAGE				DISCHARGE											
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	**Rectangular**	1	2		
7127.7	0	0	0.00	0.00	0.00			-	-	-	-	-				
7128	0.3	2170	0.05	0.01	0.01	-	-	0.0	-	-	-	-	11		0.0	0.0
7129	1.3	17730	0.41	0.23	0.24	-	-	0.1	-	-	-	-	31		0.1	0.1
7130	2.3	33290	0.76	0.59	0.82	-	-	0.1	-	1.5	-	-	57		1.6	1.6
7131	3.3	39060	0.90	0.83	1.65	-	-	0.1	4.2	2.3	-	-	117		6.6	6.6
7132	4.3	44830	1.03	0.96	2.61	-	-	0.1	10.8	2.8	-	-	117		14	14
7133	5.3	55137.5	1.27	1.15	3.76	-	-	0.2	14.4	3.3	-	-	142		18	18
7134	6.3	65445	1.50	1.38	5.15	-	-	0.2	17.4	3.7	-	36	162		57	57
7135	7.3	79535	1.83	1.66	6.81	-	-	0.2	19.9	4.0	-	102	175		126	126
7136	8.3	93625	2.15	1.99	8.80	-	-	0.2	22.1	4.4	-	150	187		177	177
7137	9.3	111620	2.56	2.36	11.15	-	-	0.2	24.1	4.7	-	173	200		200	200
7138	10.3	129615	2.98	2.77	13.92	-	92.3	0.2	25.9	5.0	-	194	211		211	303
7138.5	10.8					-	261.0	0.3	26.8	5.1	-	203	211		-	261

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond G-FINAL FUTURE DESIGN (G12)

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	500
embankment elev =	7033.5
spillway length =	130
spillway elevation =	7031.5
100 year storage elev.=	7030.3
100 year storage vol.=	25.3
100 year discharge=	478
5 year storage elev.=	7027.4
5 year storage vol.=	8.3
5 year discharge=	22
WQCV storage elev.=	7025.2
WQCV storage vol.=	0.9
1/2 WQCV storage elev.=	7024.9
1/2 WQCV storage vol.=	0.45

Data for outlet pipe and grate:

Type		H or V	Dimensions Width (ft.)	X Height (ft.)	Dia.(in)		(sqft)		
Rectangular	Orifice 1:	V	0.0263	1.90		Area =	0.050	Elev to cl =	7024.25
Rectangular	Orifice 2:	V	8.5	1.1		Area =	9.350	Elev to cl =	7027.55
Circular	Orifice 3:	H			12	Area =	0.785	Elev to cl =	7025.20
Rectangular	Orifice 4:	V	4	0.6		Area =	2.400	Elev to cl =	7027.80
Rectangular	Orifice 5:	V	8.5	1.1		Area =	9.350	Elev to cl =	7027.55

Stand Pipe Dimensions

Rec Grate	20	x	8	Elev =	7028.10
Circ. Grate		dia.		Elev =	7028.10

Outlet Culvert Dimensions

	Width (ft.)		Height (ft.)	Dia. (ft.)	Type
Outlet Culvert	10	x	4		**Rectangular**
Area	40.0		TOP		
Outlet I. E.	7022.5		7027.50		
Wall Thick.	12	in.			

50 year storage elev.=	7029.5
50 year discharge=	333
25 year storage elev.=	7028.7
25 year discharge=	170
10 year storage elev.=	7027.9
10 year discharge=	56
2 year storage elev.=	7026.8
2 year discharge=	5.1

STAGE		STORAGE				DISCHARGE												
ELEV	HEIGHT	AREA		VOLUME		TOP OF	SPILLWAY	ORIFICE (max outflow)					GRATE (max outflow)	PIPE		REALIZED CULVERT OUTFLOW	TOTAL FLOW
		sqft	acre	acft	cum acft	BANK		1	2	3	4	5	**Rectangular**	1	2		
7023.3	0	0	0.00	0.0	0.00			-	-	-	-	-	-	12		-	-
7024	0.7	2232	0.05	0.0	0.02	-	-	0.0	-	-	-	-	-	51		0.0	0.05
7025	1.7	39917	0.92	0.5	0.50	-	-	0.2	-	-	-	-	-	111		0.2	0.17
7026	2.7	126469	2.90	1.9	2.41	-	-	0.3	-	3.4	-	-	-	184		3.7	3.7
7026.5	3.2	166675	3.83	3.6	4.06	-	-	0.4	-	4.3	-	-	-	224		4.7	4.7
7027	3.7	206880	4.75	2.1	6.20	-	-	0.4	-	5.1	-	-	-	268		5.5	5.5
7027.5	4.2	232032	5.33	4.6	8.64	-	-	0.4	9.0	5.7	-	9.0	-	304		24	24
7028	4.7	257183	5.90	5.3	11.5	-	-	0.5	25.5	6.3	4.2	25.5	-	337		62	62
7028.5	5.2	264196	6.07	5.7	14.3	-	-	0.5	43.9	6.9	9.7	43.9	27	373		132	132
7029	5.7	271209	6.23	6.1	17.6	-	-	0.5	54.2	7.4	12.7	54.2	92	406		221	221
7029.5	6.2	276106	6.34	11.7	20.3	-	-	0.6	70.5	7.8	17.1	70.5	179	436		345	345
7030	6.7	281003	6.45	9.4	23.7	-	-	0.6	77.3	8.3	19.0	77.3	283	464		464	464
7030.5	7.2	286003	6.57	6.5	26.8	-	-	0.6	77.3	8.7	19.0	77.3	402	491		491	491
7031	7.7	291002	6.68	6.6	30.3	-	-	0.6	83.6	9.1	20.7	83.6	533	516		516	516
7031.5	8.2	296443	6.81	6.7	33.4	-	-	0.6	89.5	9.5	22.2	89.5	677	540		540	540
7032	8.7	301883	6.93	3.4	36.9	137.9	137.9	0.7	95.0	9.9	23.7	95.0	832	563		563	701
7032.5	9.2	309236	7.10	7.0	40.4	390.0	390.0	0.7	100.2	10.2	25.1	100.2	997	586		586	976
7033	9.7	316589	7.27	3.6	44.0	716.5	716.5	0.7	105.1	10.6	26.4	105.1	1,171	607		607	1,323

Notes:
1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. Q=CLH^1.5 (C=3.0)
2) Orifice flows are also from section 11.3.1. Q=CA(2gH)^.5 (C=.6)
3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow Q=(3PH^1.5)/F, Orifice Flow Q=4.815*AH^0.5)
4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

Per previous meetings. Replace with MH-Detention worksheet.

If you are inputting the inflow hydrograph into the MH-Detention software, make sure the inflow hydrograph values are provided.

Make sure to explain in the narrative that inflow hydrograph from the HEC-HMS is used as a user override and which hydrograph is used (future, interim or historic).

TURE POND G

Control Riser Calculations

TRIB
D

IMPE

DEPTH OF OUTLET 1.9

WQCV 0.11 inches

WQCV DESIGN VOL **0.9** ac-ft

K_{40} 0.36

AREA PER RISER[1] a 7.67 in^2

No. of Columns 1

No. of Holes 3 per column

Area per Hole 2.56 in^2

Hole size 1 3/4 in

Steel Plate Thickness 1/4 in

[1] AREA PER ROW PER RISER

Actual area per row per hole: 2.41 in^2

Actual area per riser: 7.2 in^2

Actual area per riser: 0.050 ft^2

TABLE SB-2							
Hole Dia (in)		Area per Row (in^2)					
Holes per Row		1	2	3	4	5	6
Min steel thickness		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/8	1.1250	0.99	1.99	2.98	3.98	4.97	5.96
1 1/4	1.2500	1.23	2.45	3.68	4.91	6.14	7.36
1 3/8	1.3750	1.48	2.97	4.45	5.94	7.42	8.91
1 1/2	1.5000	1.77	3.53	5.30	7.07	8.84	10.60
1 5/8	1.6250	2.07	4.15	6.22	8.30	10.37	12.44
1 3/4	1.7500	2.41	4.81	7.22	9.62	12.03	14.43
1 7/8	1.8750	2.76	5.52	8.28	11.04	13.81	16.57
2	2.0000	3.14	6.28	9.42	12.57	15.71	18.85

n = Number of columns of perforations

ROLLING HILLS RANCH PUD INTERIM CONDITION

Simulation Run: RHPUD-100 YR Reservoir: POND D

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	509(CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:12
Peak Outflow:	134 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 13:00
Total Inflow :	57.0 (AC-FT)	Peak Storage:	25.5 (AC-FT)
Total Outflow:	46.8 (AC-FT)	Peak Elevation:	7057.1 (FT)

Simulation Run: RHPUD-005 YR Reservoir: POND D

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	107 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	12 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 14:36
Total Inflow :	13.6 (AC-FT)	Peak Storage:	7.4 (AC-FT)
Total Outflow:	9.2 (AC-FT)	Peak Elevation:	7053.9 (FT)

Simulation Run: RHPUD-100 YR Reservoir: POND E

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	610 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	242 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 13:30
Total Inflow :	122.9 (AC-FT)	Peak Storage:	42.4 (AC-FT)
Total Outflow:	98.9 (AC-FT)	Peak Elevation:	6973.6 (FT)

Simulation Run: RHPUD-005 YR Reservoir: POND E

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	126 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:12
Peak Outflow:	16 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 20:00
Total Inflow :	29.0 (AC-FT)	Peak Storage:	18.0 (AC-FT)
Total Outflow:	13.1 (AC-FT)	Peak Elevation:	6971.4 (FT)

Simulation Run: RHPUD-100 YR Reservoir: POND F

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	242(CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	155 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 12:42
Total Inflow :	33.5 (AC-FT)	Peak Storage:	7.6 (AC-FT)
Total Outflow:	31.6 (AC-FT)	Peak Elevation:	7135.6 (FT)

Simulation Run: RHPUD-005 YR Reservoir: POND F

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	18 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:36
Peak Outflow:	6.5 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 14:24
Total Inflow :	4.8 (AC-FT)	Peak Storage:	1.5 (AC-FT)
Total Outflow:	4.3 (AC-FT)	Peak Elevation:	7131.0 (FT)

Simulation Run: RHPUD-100 YR Reservoir: POND G

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	504 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:36
Peak Outflow:	369 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 13:06
Total Inflow :	95.6 (AC-FT)	Peak Storage:	23.1 (AC-FT)
Total Outflow:	86.8 (AC-FT)	Peak Elevation:	7029.9 (FT)

Simulation Run: RHPUD-005 YR Reservoir: POND G

Start of Run:	01Jul2015, 00:00	Basin Model:	WW Grading
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	38 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:48
Peak Outflow:	12 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 18:18
Total Inflow :	14.2 (AC-FT)	Peak Storage:	7.8 (AC-FT)
Total Outflow:	8.6 (AC-FT)	Peak Elevation:	7025.8 (FT)

ROLLING HILLS RANCH PUD FUTURE CONDITION
Simulation Run: F-100 YR Reservoir: POND D

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	509(CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:12
Peak Outflow:	134 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 13:00
Total Inflow :	57.0 (AC-FT)	Peak Storage:	25.5 (AC-FT)
Total Outflow:	46.8 (AC-FT)	Peak Elevation:	7057.1 (FT)

Simulation Run: F-005 YR Reservoir: POND D

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:26:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	107 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	12 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 14:36
Total Inflow :	13.6 (AC-FT)	Peak Storage:	7.5 (AC-FT)
Total Outflow:	9.2 (AC-FT)	Peak Elevation:	7053.9 (FT)

Simulation Run: F-100 YR Reservoir: POND E

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	610 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	242 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 13:30
Total Inflow :	122.9 (AC-FT)	Peak Storage:	42.4 (AC-FT)
Total Outflow:	98.9 (AC-FT)	Peak Elevation:	6973.6 (FT)

Simulation Run: F-005 YR Reservoir: POND E

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:26:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	126 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:12
Peak Outflow:	16 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 20:00
Total Inflow :	29.0 (AC-FT)	Peak Storage:	18.0 (AC-FT)
Total Outflow:	13.1 (AC-FT)	Peak Elevation:	6971.4 (FT)

Simulation Run: F-100 YR Reservoir: POND F

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	256(CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:18
Peak Outflow:	164 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 12:42
Total Inflow :	35.3 (AC-FT)	Peak Storage:	8.0 (AC-FT)
Total Outflow:	33.4 (AC-FT)	Peak Elevation:	7135.8 (FT)

Simulation Run: F-005 YR Reservoir: POND F

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:26:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	19 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:36
Peak Outflow:	7.2 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 14:18
Total Inflow :	5.1 (AC-FT)	Peak Storage:	1.6 (AC-FT)
Total Outflow:	4.6 (AC-FT)	Peak Elevation:	7131.1 (FT)

Simulation Run: F-100 YR Reservoir: POND G

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 100YR
Compute Time:	14Mar2018 13:11:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	694 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:06
Peak Outflow:	479 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 12:32
Total Inflow :	119.4 (AC-FT)	Peak Storage:	25.4 (AC-FT)
Total Outflow:	110.2 (AC-FT)	Peak Elevation:	7030.3 (FT)

Simulation Run: F-005 YR Reservoir: POND G

Start of Run:	01Jul2015, 00:00	Basin Model:	Future SCS
End of Run:	02Jul2015, 00:00	Meteorologic Model:	SCS TYPE IIA 005YR
Compute Time:	14Mar2018 13:26:34	Control Specifications:	24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow:	73 (CFS)	Date/Time of Peak Inflow:	01Jul2015, 12:30
Peak Outflow:	21 (CFS)	Date/Time of Peak Outflow:	01Jul2015, 15:24
Total Inflow :	20.4 (AC-FT)	Peak Storage:	8.2 (AC-FT)
Total Outflow:	14.5 (AC-FT)	Peak Elevation:	7027.4 (FT)

Appendix D – Outlet Protection Design

Again, enter Figure HS-19a using the smaller d/D (or d/H) ratio to find the A/A_{full} ratio. Then,

$$A = \left(A/A_{full}\right)A_{full} \qquad \text{(HS-16c)}$$

Finally,

$$V = Q/A \qquad \text{(HS-16d)}$$

In which for Equations 16a through 16d above:

A_{full} = cross-sectional area of the pipe (ft^2)

A = area of the design flow in the end of the pipe (ft^2)

n = Manning's n for the pipe full depth

Q_{full} = pipe full discharge at its slope (cfs)

R = hydraulic radius of the pipe flowing full, ft [$R_{full} = D/4$ for circular pipes, $R_{full} = A_{full}/(2H + 2w)$ for rectangular pipes, where D = diameter of a circular conduit, H = height of a rectangular conduit, and w = width of a rectangular conduit (ft)]

S_o = longitudinal slope of the pipe (ft/ft)

V = design flow velocity at the pipe outlet (ft/sec)

V_{full} = flow velocity of the pipe flowing full (ft/sec)

3.4.3.2 Riprap Size

For the design velocity, use Figure HS-20c to find the size and type of the riprap to use in the scour protection basin downstream of the pipe outlet (i.e., B18, H, M or L). First, calculate the riprap sizing design parameter, P_d , namely,

$$P_d = \left(V^2 + gd\right)^{1/2} \qquad \text{(HS-16e)}$$

in which:

V = design flow velocity at pipe outlet (ft/sec)

g = acceleration due to gravity = 32.2 ft/sec^2

d = design depth of flow at pipe outlet (ft) necessary when the receiving or downstream channel may have little or no flow or tailwater at time when the pipe or culvert is in operation. Design criteria are provided in Figures HS-19a through HS-20c.

3.4.2 Objective

By providing a low tailwater basin at the end of a storm sewer conduit or culvert, the kinetic energy of the discharge is dissipated under controlled conditions without causing scour at the channel bottom. Photograph HS-12 shows a fairly large low tailwater basin.

3.4.3 Low Tailwater Basin Design

Low tailwater is defined as being equal to or less than ⅓ of the height of the storm sewer, that is:

$$y_t \le \frac{D}{3} \quad \text{or} \quad y_t \le \frac{H}{3}$$

in which:

y_t = tailwater depth at design

D = diameter of circular pipe (ft)

H = height of rectangular pipe (ft)

3.4.3.1 Finding Flow Depth and Velocity of Storm Sewer Outlet Pipe

The first step in the design of a scour protection basin at the outlet of a storm sewer is to find the depth and velocity of flow at the outlet. Pipe-full flow can be found using Manning's equation.

$$Q_{full} = \frac{1.49}{n} A_{full} \left(R_{full}\right)^{2/3} S_o^{1/2} \qquad \text{(HS-16a)}$$

Then and the pipe-full velocity can be found using the continuity equation.

$$V_{full} = Q_{full} / A_{full} \qquad \text{(HS-16a)}$$

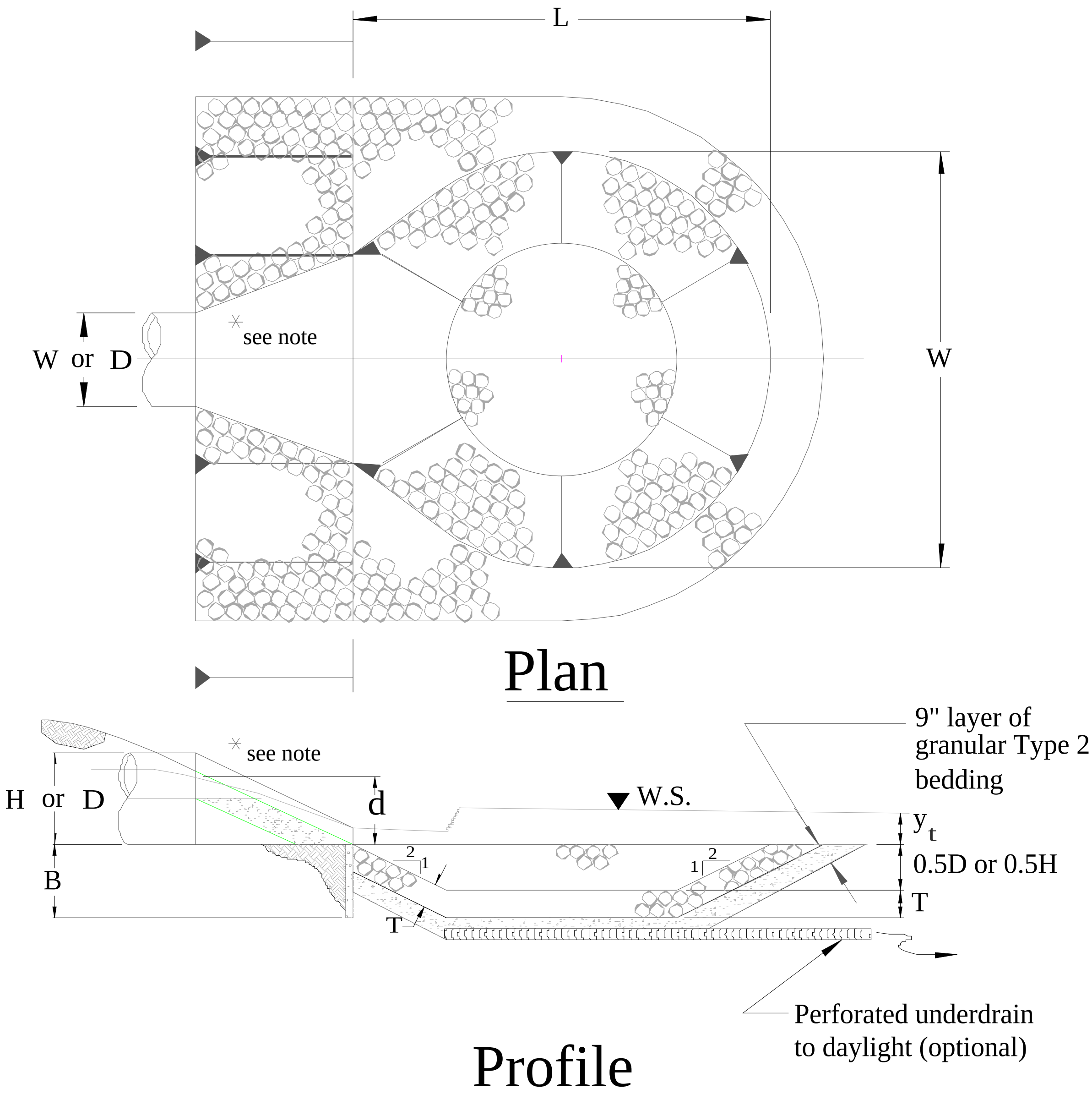
The normal depth of flow, d, and the velocity in a conduit can be found with the aid of Figure HS-20a and Figure HS-20b. Using the known design discharge, Q, and the calculated pipe-full discharge, Q_{full}, enter Figure HS-20a with the value of Q/Q_{full} and find d/D for a circular pipe of d/H for a rectangular pipe.

Compare the value of d/D (or d/H) with the one obtained from Figure HS-20b using the Froude parameter.

$$Q/D^{2.5} \quad \text{or} \quad Q/\left(wH^{1/5}\right) \qquad \text{(HS-16a)}$$

Choose the smaller of the two (d/D or d/H) ratios to calculate the flow depth at the end of the pipe.

$$d = D\left(d/D\right) \quad \text{or} \quad d = H\left(d/H\right) \qquad \text{(HS-16b)}$$



*Note: For rectangular conduits use a standard design for a headwall with wingwalls, paved bottom between the wingwalls, with an end cutoff wall extending to a minimum depth equal to B

Figure HS-19—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets— Low Tailwater Basin at Pipe Outlets
(Stevens and Urbonas 1996)

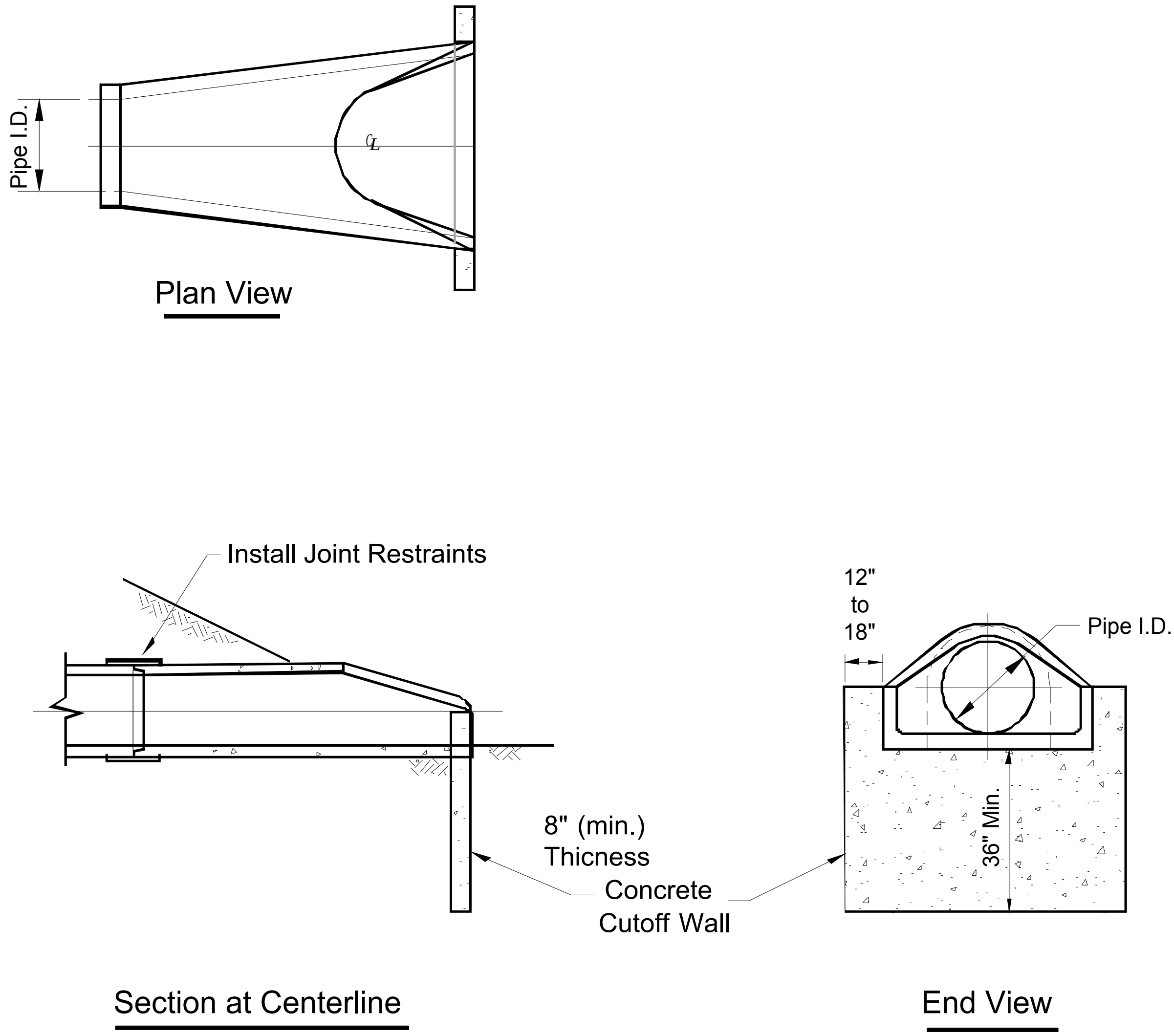


Figure HS-19a—Concrete Flared End Section with Cutoff Wall for all Pipe Outlets

Photograph HS-12—Upstream and downstream views of a low tailwater basin in Douglas County protecting downstream wetland area. Burying and revegetation of the rock would blend the structure better with the adjacent terrain.

When the riprap sizing design parameter indicates conditions that place the design above the Type H riprap line in Figure HS-20, use B18, or larger, grouted boulders. An alternative to a grouted boulder or loose riprap basin is to use the standard USBR Impact Basin VI or one of its modified versions, described earlier in this Chapter of the *Manual*.

After the riprap size has been selected, the minimum thickness of the riprap layer, T, in feet, in the basin is set at:

$$T = 1.75 D_{50} \qquad \text{(HS-17)}$$

in which:

D_{50} = the median size of the riprap (see Table HS-9.)

Table HS-9—Median (i.e., D_{50}) Size of District's Riprap/Boulder

Riprap Type	D_{50}—Median Rock Size (inches)
L	9
M	12
H	18
B18	18 (minimum dimension of grouted boulders)

3.4.3.3 Basin Length

The minimum length of the basin, L, in Figure HS-19, is defined as being the greater of the following:

for circular pipe: $$L = 4D \quad \text{or} \quad L = (D)^{1/2}\left(\frac{V}{2}\right) \qquad \text{(HS-18)}$$

for rectangular pipe: $L = 4H$ or $L = (H)^{1/2}\left(\frac{V}{2}\right)$ (HS-19)

in which:

L = basin length

H = height of rectangular conduit

V = design flow velocity at outlet

D = diameter of circular conduit

3.4.3.4 Basin Width

The minimum width, W, of the basin downstream of the pipe's flared end section is set as follows:

for circular pipes: $W = 4D$ (HS-20)

for rectangular pipe: $W = w + 4H$ (HS-21)

in which,

W = basin width (Figure HS-19)

D = diameter of circular conduit

w = width of rectangular conduit

3.4.3.5 Other Design Requirements

All slopes in the pre-shaped riprapped basin are 2H to 1V.

Provide pipe joint fasteners and a structural concrete cutoff wall at the end of the flared end section for a circular pipe or a headwall with wingwalls and a paved bottom between the walls, both with a cutoff wall that extends down to a depth of:

$B = \frac{D}{2} + T$ or $B = \frac{H}{2} + T$ (HS-22)

in which,

B = cutoff wall depth

D = diameter of circular conduit

T = Equation HS-17

The riprap must be extended up the outlet embankment's slope to the mid-pipe level.

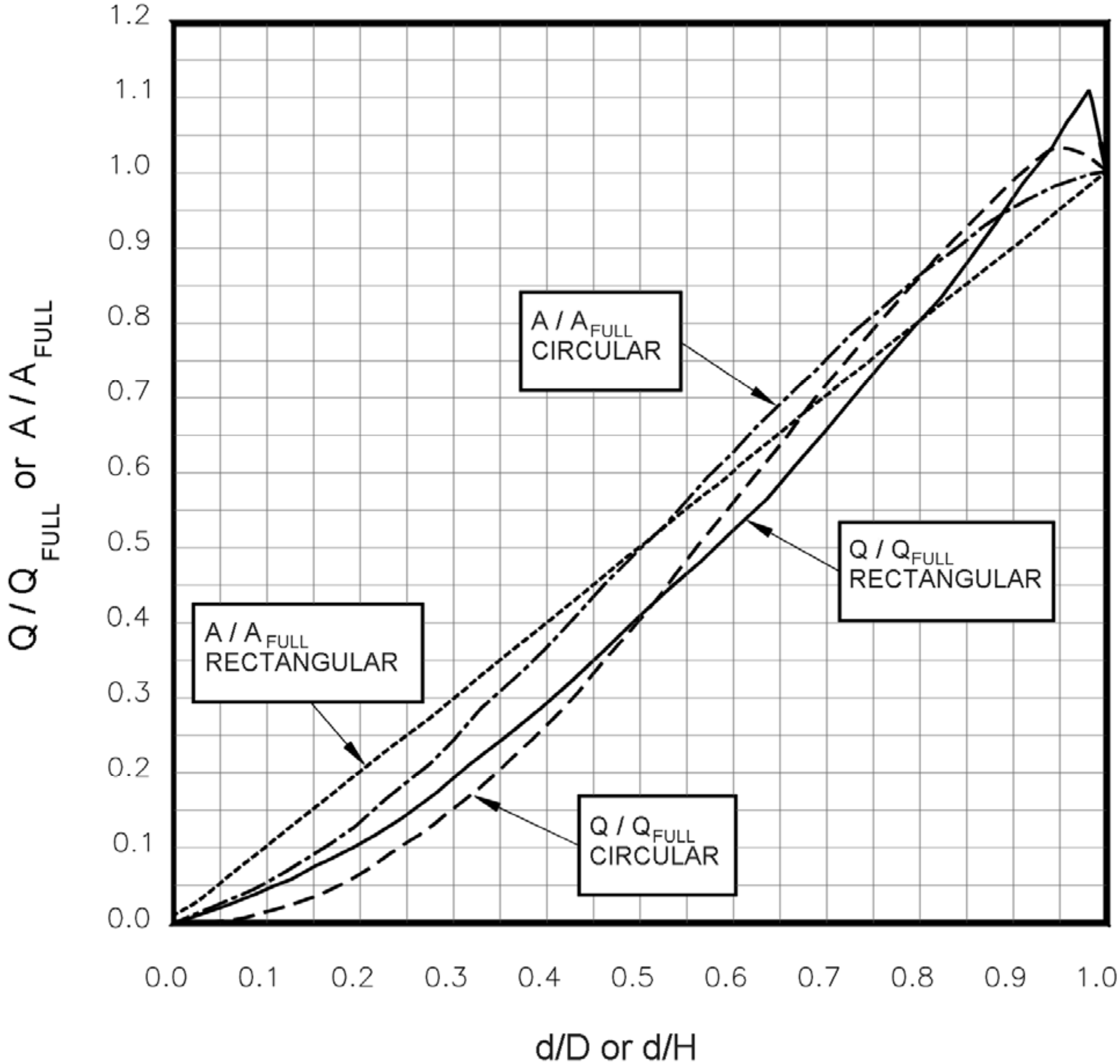


Figure HS-20a—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—
Discharge and Flow Area Relationships for Circular and Rectangular Pipes
(Ratios for Flow Based on Manning's *n* Varying With Depth)
(Stevens and Urbonas 1996)

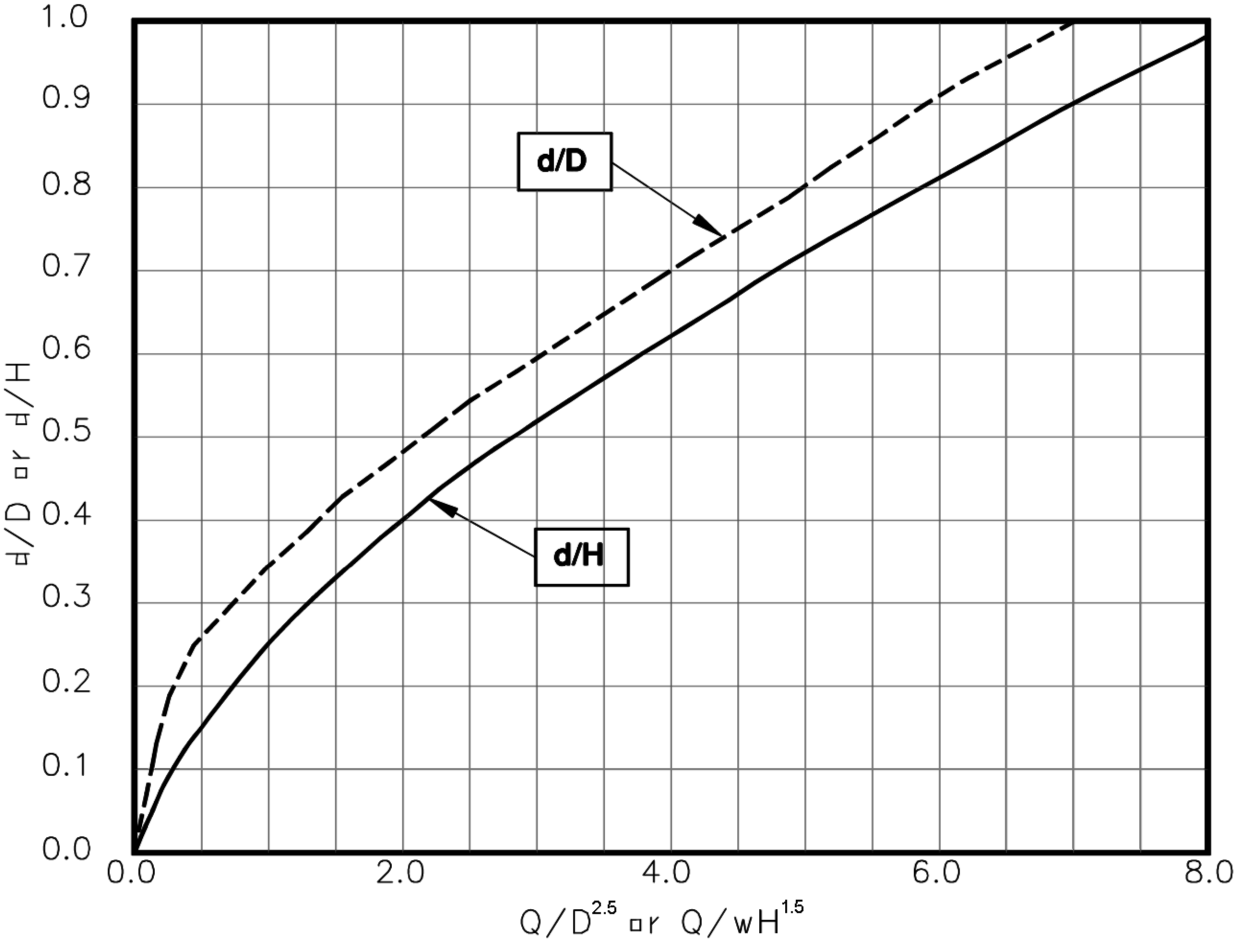


Figure HS-20b—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Brink Depth for Horizontal Pipe Outlets

(Stevens and Urbonas 1996)

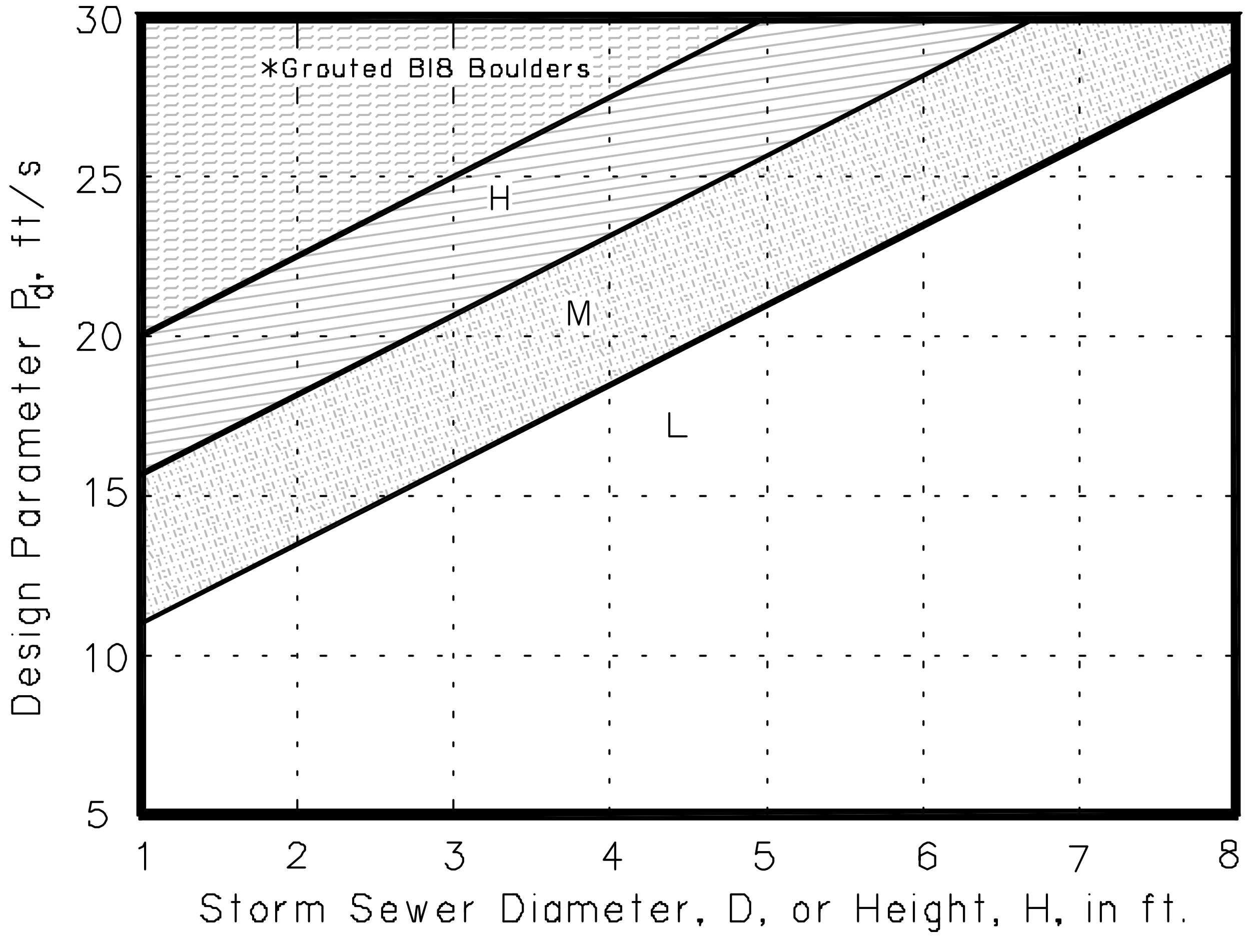


Figure HS-20c—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Riprap Selection Chart for Low Tailwater Basin at Pipe Outlet

(Stevens and Urbonas 1996)

RIP RAP PLUNGE POOL

Urban Drainage & Flood Control District Pipe Outlet Design

Low Tailwater Design ($y_t \leq D/3$)

OUTLET #	OS-2					
Outlet Size (D) :	42	in.	Discharge (q):	86	CFS	
Capacity (Q): (full flow)	144	CFS	Flow depth (d): (calculated)	26.3	in.	
Q_{full} =	144	CFS	q/Q_{full} =	0.60		
A_{full} =	9.6	SF				
V_{full} =	15.0	FPS	$Q/D^{2.5}$ =	3.8		

d/D	0.63	from HS-20a using q/Q_{full}
d/D	0.68	from HS-20b using $Q/D^{2.5}$

A' (A/A_{full})	0.63	from HS-20a using smaller d/D from above	Flow Area ($a = A' \times A_{full}$)	6.0	SF

Outlet Velocity (V = q/a) 14.3 FPS

$P_d = (V^2 + gd)^{1/2}$ = 17

RIP-RAP SIZE: M from HS-20c

d_{50}= 12 in $T = 1.75 x d_{50}$ 1.75 ft

Basin Length (L)	14.0 FT.	Cutoff Wall Depth (B=D/2+T)	3.5	FT
Basin Width (W)	14.0 FT.			

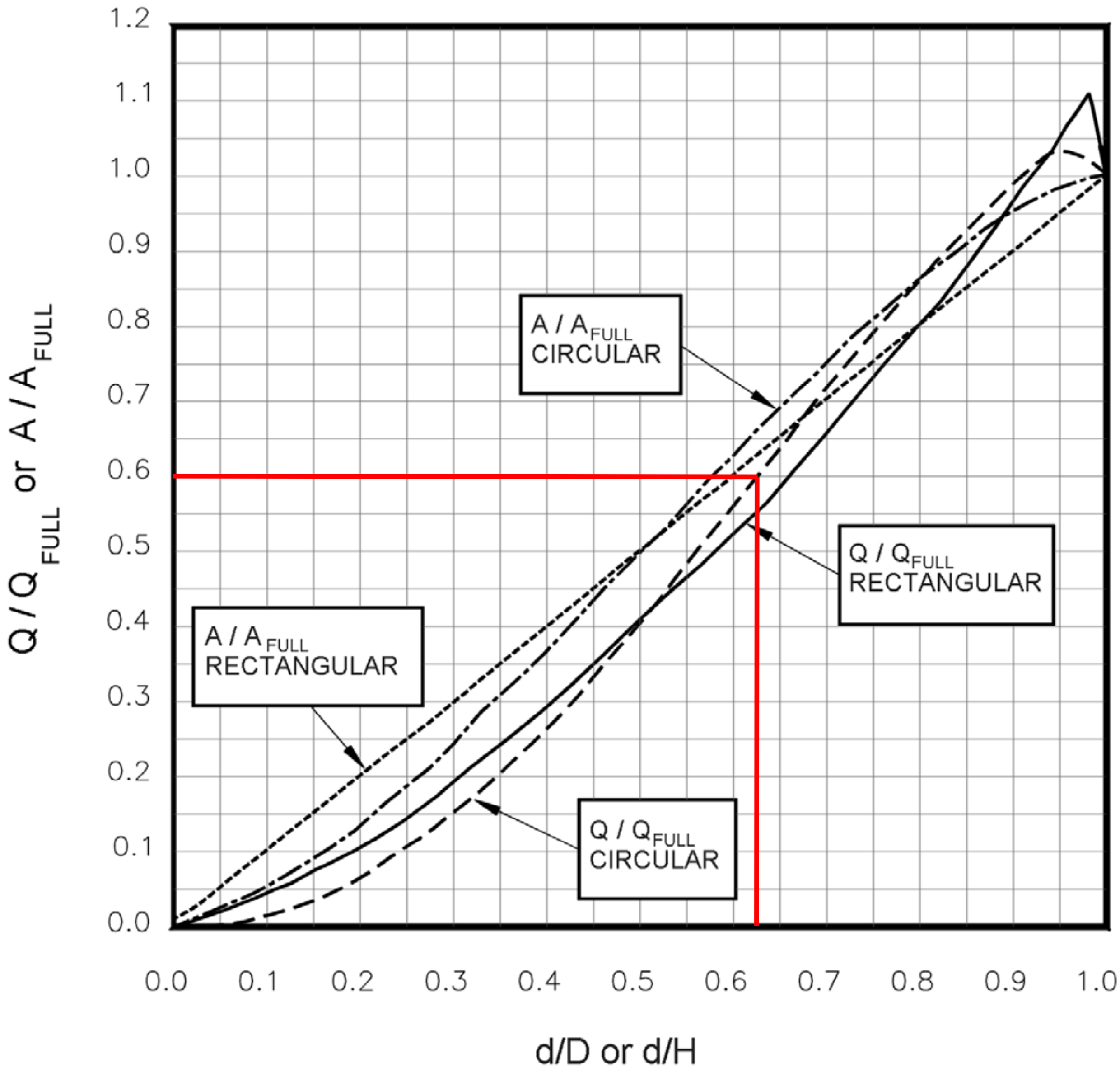


Figure HS-20a—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets— Discharge and Flow Area Relationships for Circular and Rectangular Pipes

(Ratios for Flow Based on Manning's n Varying With Depth)

(Stevens and Urbonas 1996)

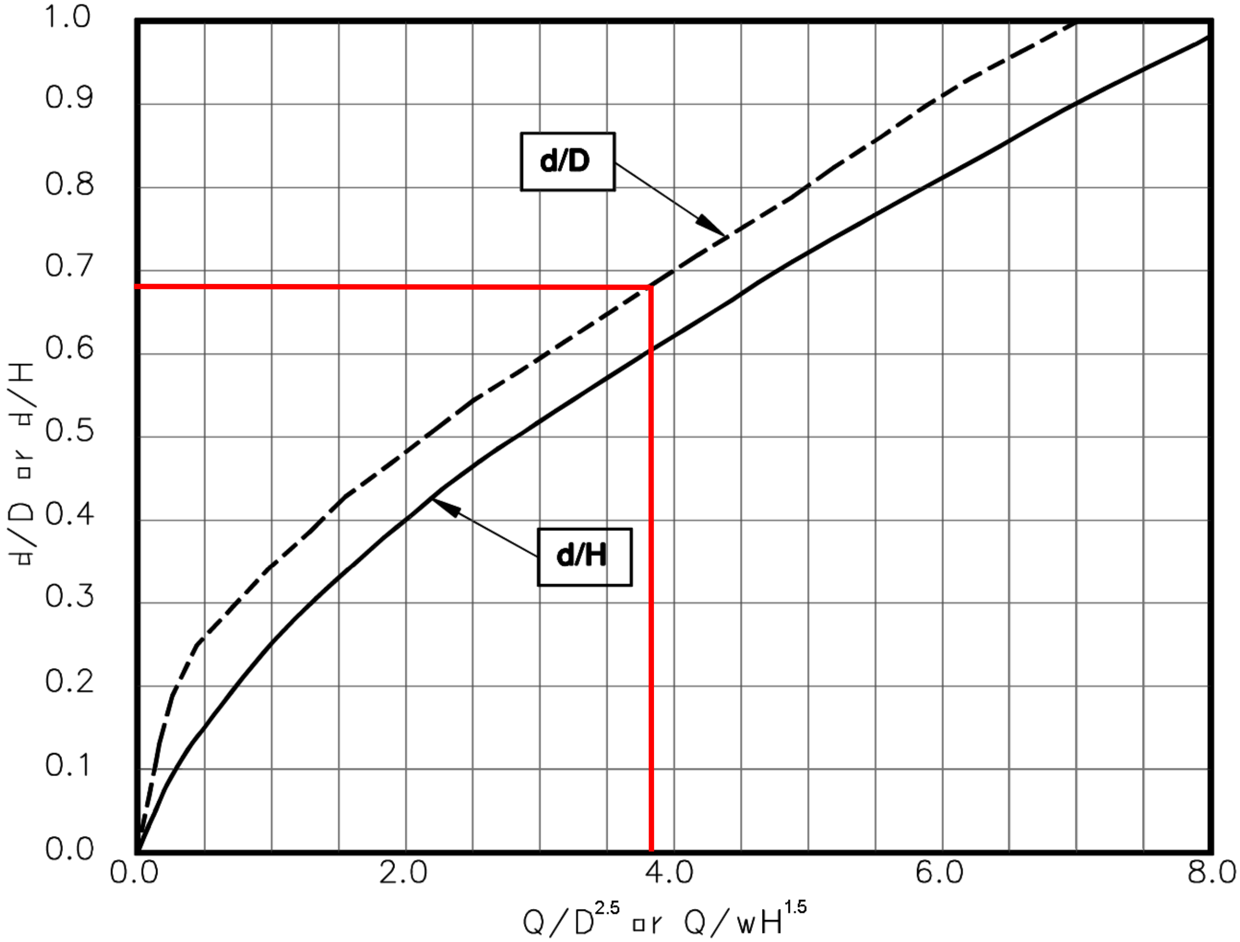


Figure HS-20b—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—
Brink Depth for Horizontal Pipe Outlets
(Stevens and Urbonas 1996)

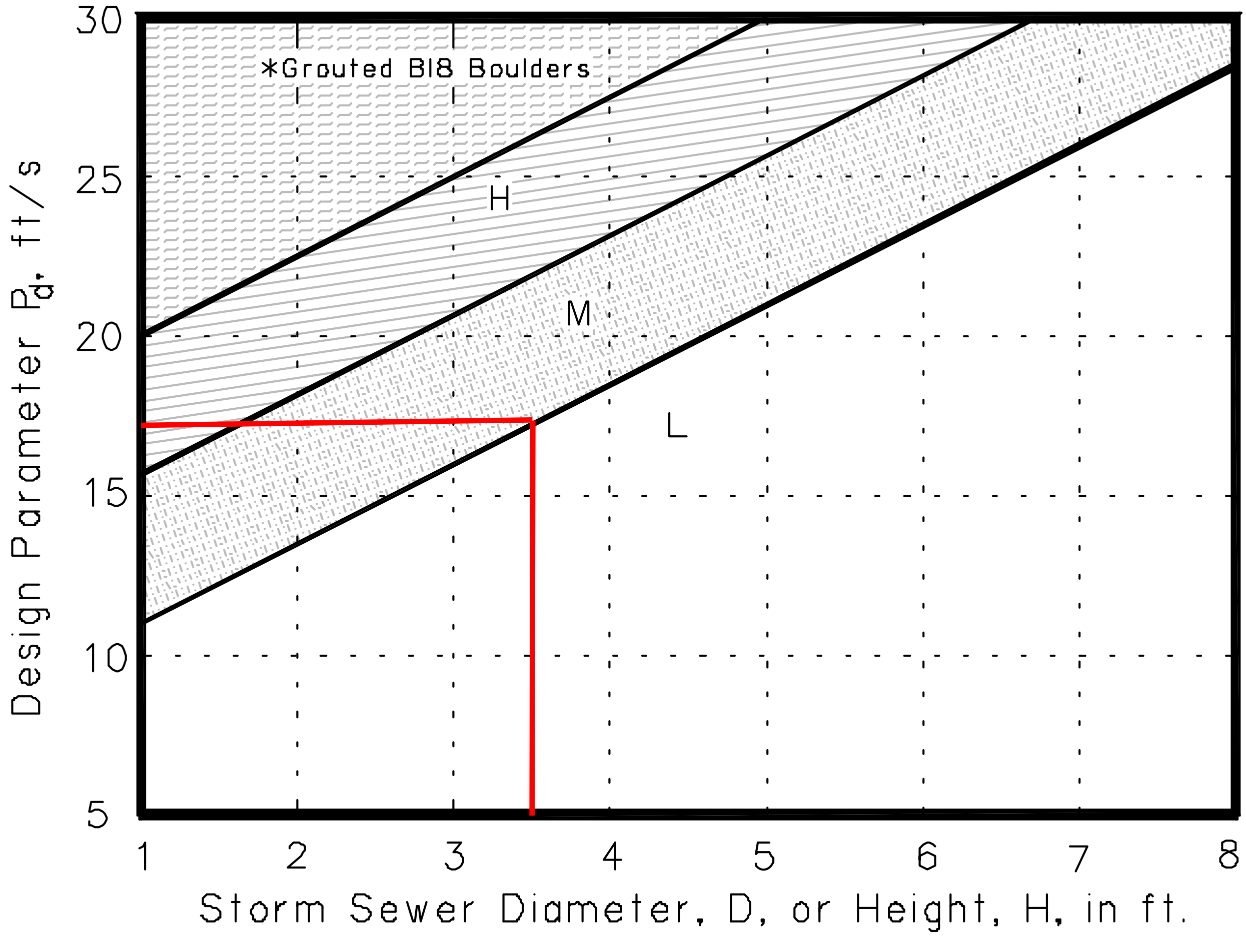


Figure HS-20c—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Riprap Selection Chart for Low Tailwater Basin at Pipe Outlet

(Stevens and Urbonas 1996)

RIP RAP PLUNGE POOL

Urban Drainage & Flood Control District Pipe Outlet Design

Low Tailwater Design ($y_t \le D/3$)

OUTLET # OS-5

Outlet Size (D) : 24 in.

Capacity (Q): 49 CFS
(full flow)

Discharge (q): 8.3 CFS

Flow depth (d): 8.4 in.
(calculated)

Q_{full} = 49 CFS

A_{full} = 3.1 SF

V_{full} = 15.6 FPS

q/Q_{full} = 0.17

$Q/D^{2.5}$ = 1.5

d/D 0.35 from HS-20a using q/Q_{full}

d/D 0.42 from HS-20b using $Q/D^{2.5}$

A' (A/A_{full}) 0.35 from HS-20a using smaller d/D from above

Flow Area ($a = A' \times A_{full}$) 1.1 SF

Outlet Velocity (V = q/a) 7.5 FPS

$P_d = (V^2 + gd)^{1/2}$ = 9

RIP-RAP SIZE: M from HS-20c

d_{50}= 12 in

$T = 1.75 x d_{50}$ 1.75 ft

Basin Length (L) 8.0 FT.

Basin Width (W) 8.0 FT.

Cutoff Wall Depth (B=D/2+T) 2.75 FT

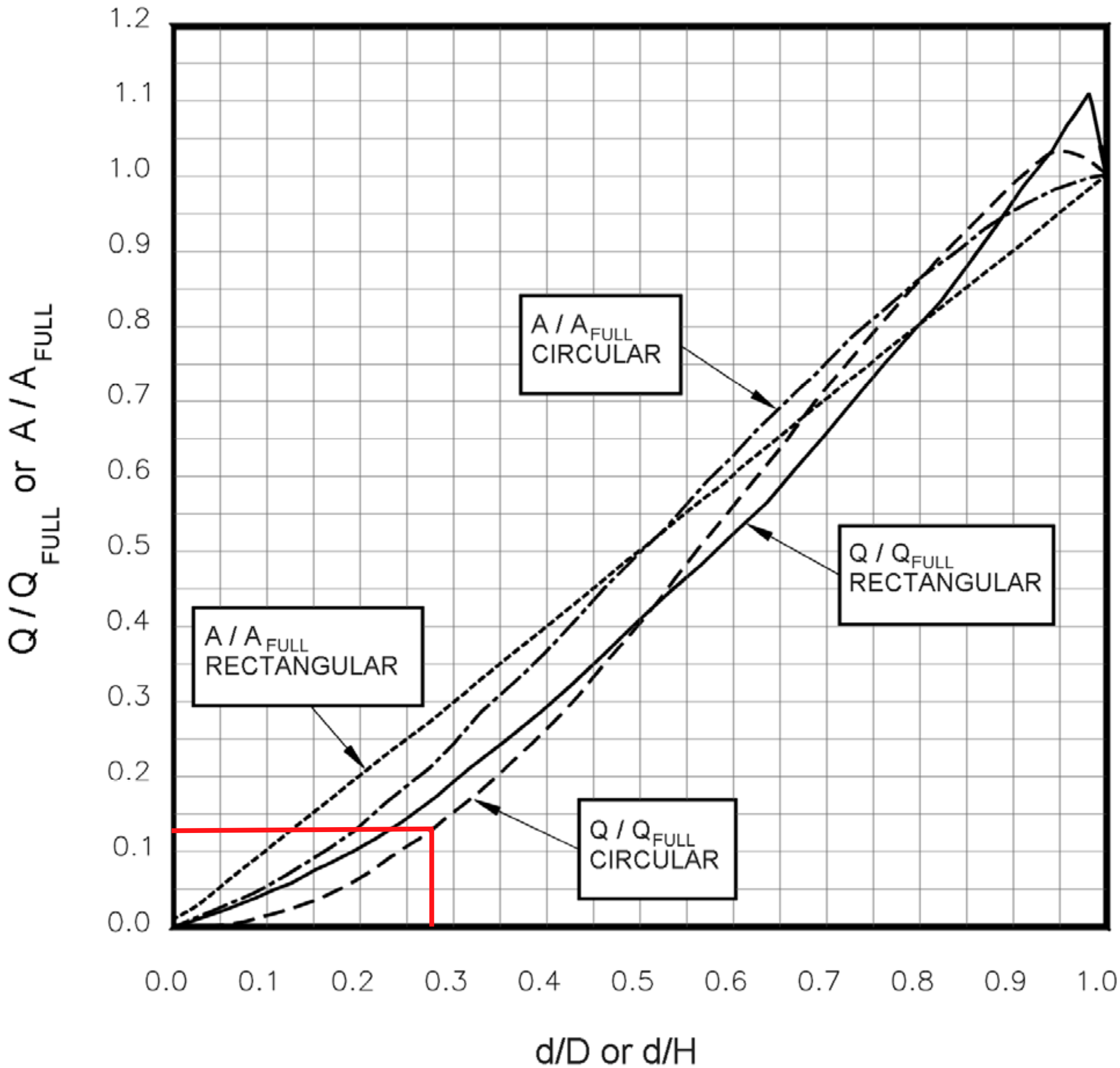


Figure HS-20a—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—Discharge and Flow Area Relationships for Circular and Rectangular Pipes
(Ratios for Flow Based on Manning's *n* Varying With Depth)
(Stevens and Urbonas 1996)

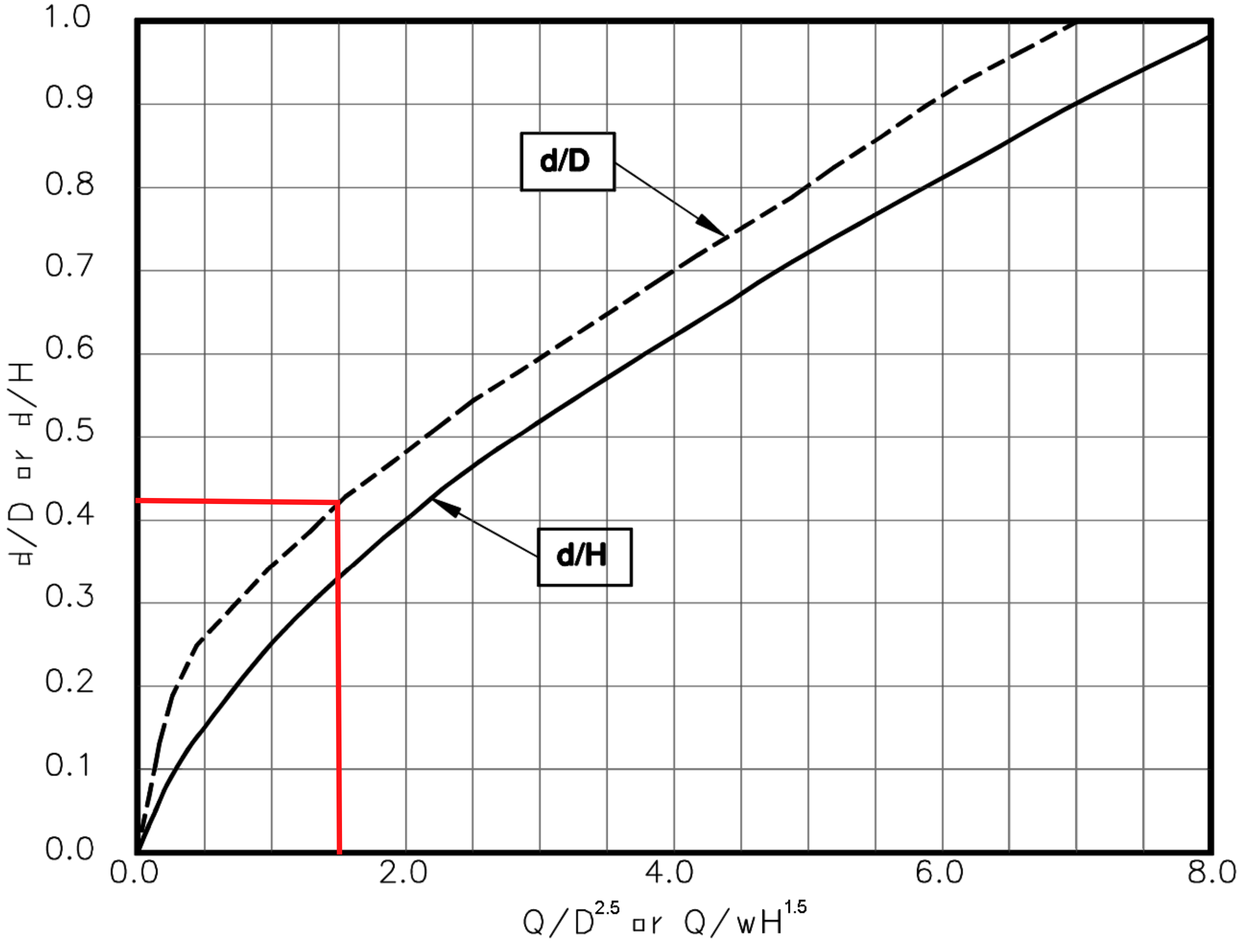


Figure HS-20b—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Brink Depth for Horizontal Pipe Outlets

(Stevens and Urbonas 1996)

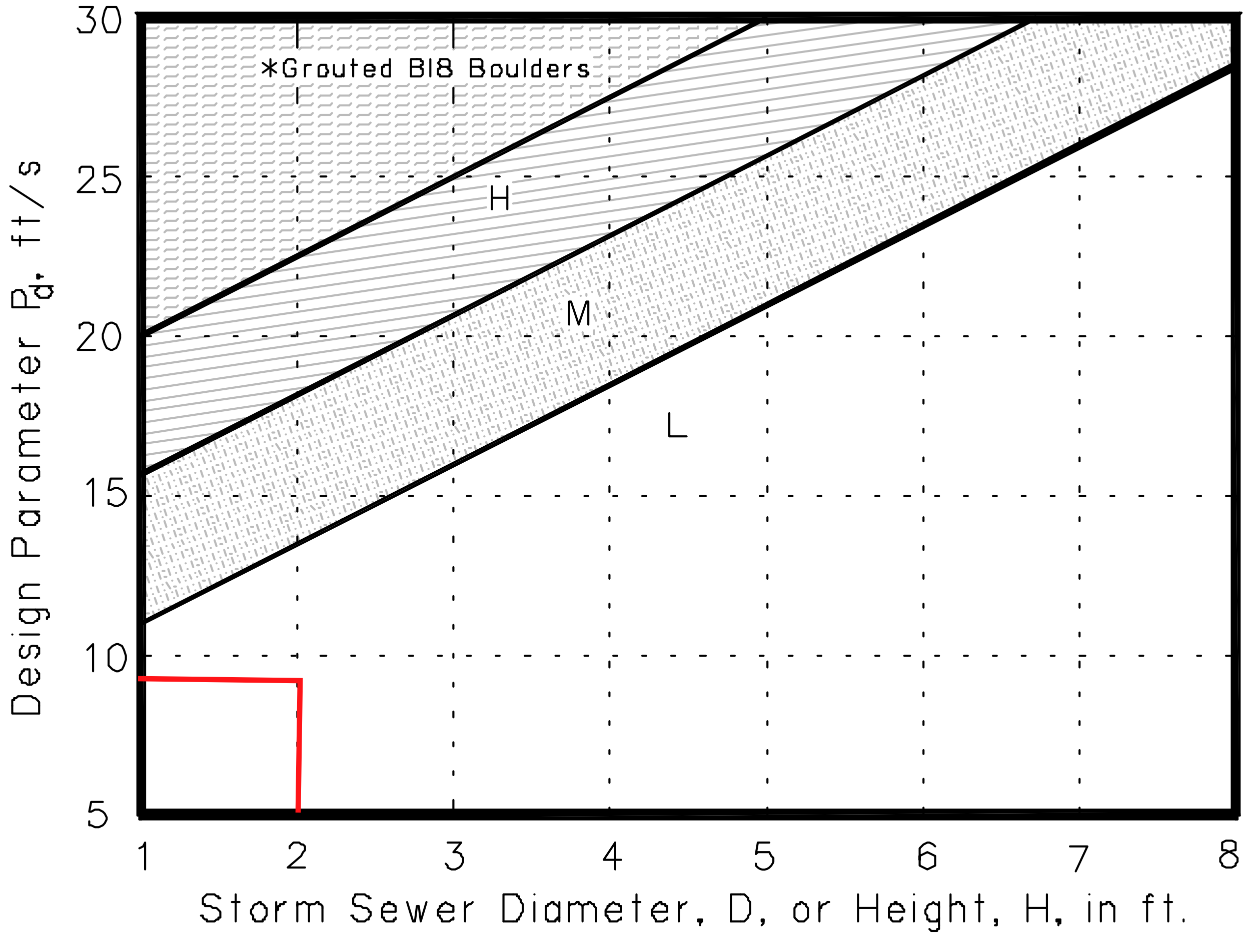


Figure HS-20c—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Riprap Selection Chart for Low Tailwater Basin at Pipe Outlet

(Stevens and Urbonas 1996)

RIP RAP PLUNGE POOL

Urban Drainage & Flood Control District Pipe Outlet Design

Low Tailwater Design ($y_t \leq H/3$)

OUTLET # POND G

Outlet Size (H) :	48	in.	Discharge (q):	478	CFS
Capacity (Q): (full flow)	479	CFS	Flow depth (d): (calculated)	39.1	in.
Retangular Width	10				

Q_{full} =	479 CFS	q/Q_{full} =	1.00
A_{full} =	40.0 SF		
V_{full} =	12.0 FPS	$Q/wH^{1.5}$ =	6.0

d/H	1.00	from HS-20a using Q/Q_{full}
d/H	0.82	from HS-20b using $Q/wH^{1.5}$

A' (A/A_{full})	0.82	from HS-20a using smaller d/D from above	Flow Area (a=A' x A_{full})	32.6	SF

Outlet Velocity (V = q/a) 14.7 FPS

$P_d = (V^2 + gd)^{1/2}$ = 18

RIP-RAP SIZE: M from HS-20c

d_{50}= 12 in $T=1.75xd_{50}$ 1.75 ft

Basin Length (L)	16.0 FT.	Cutoff Wall Depth (B=H/2+T)	3.75	FT
Basin Width (W)	26.0 FT.			

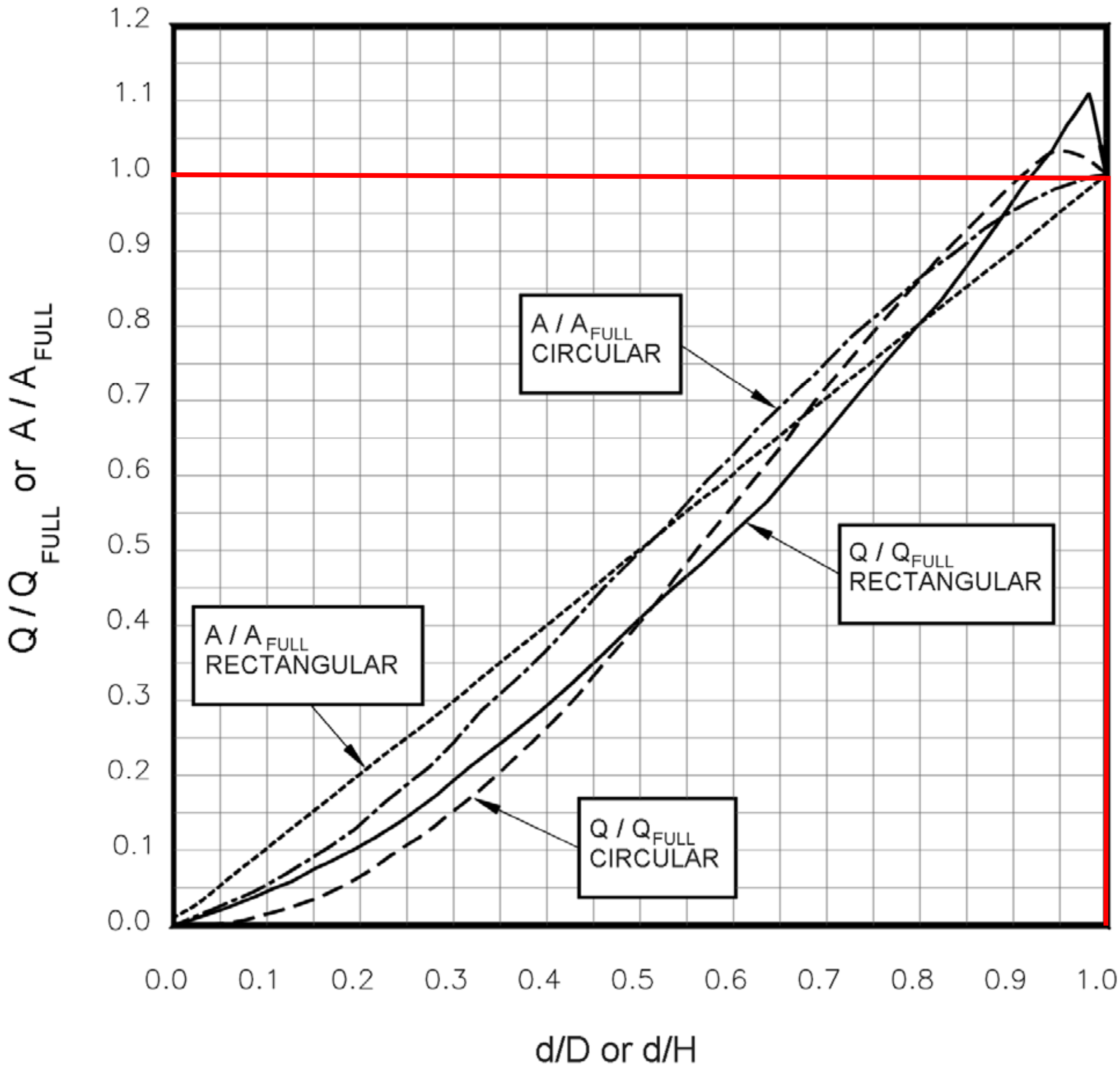


Figure HS-20a—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—
Discharge and Flow Area Relationships for Circular and Rectangular Pipes
(Ratios for Flow Based on Manning's *n* Varying With Depth)
(Stevens and Urbonas 1996)

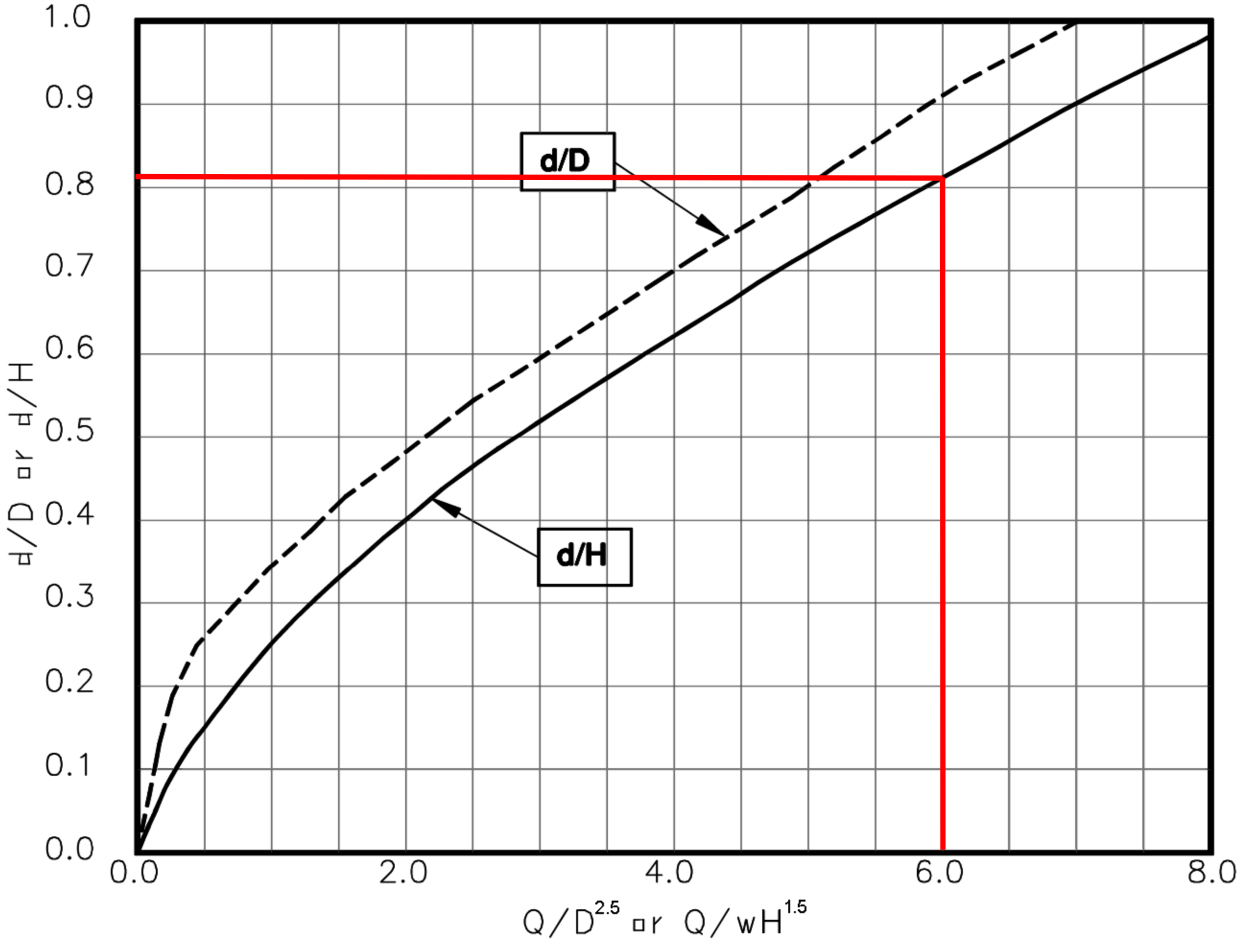


Figure HS-20b—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Brink Depth for Horizontal Pipe Outlets

(Stevens and Urbonas 1996)

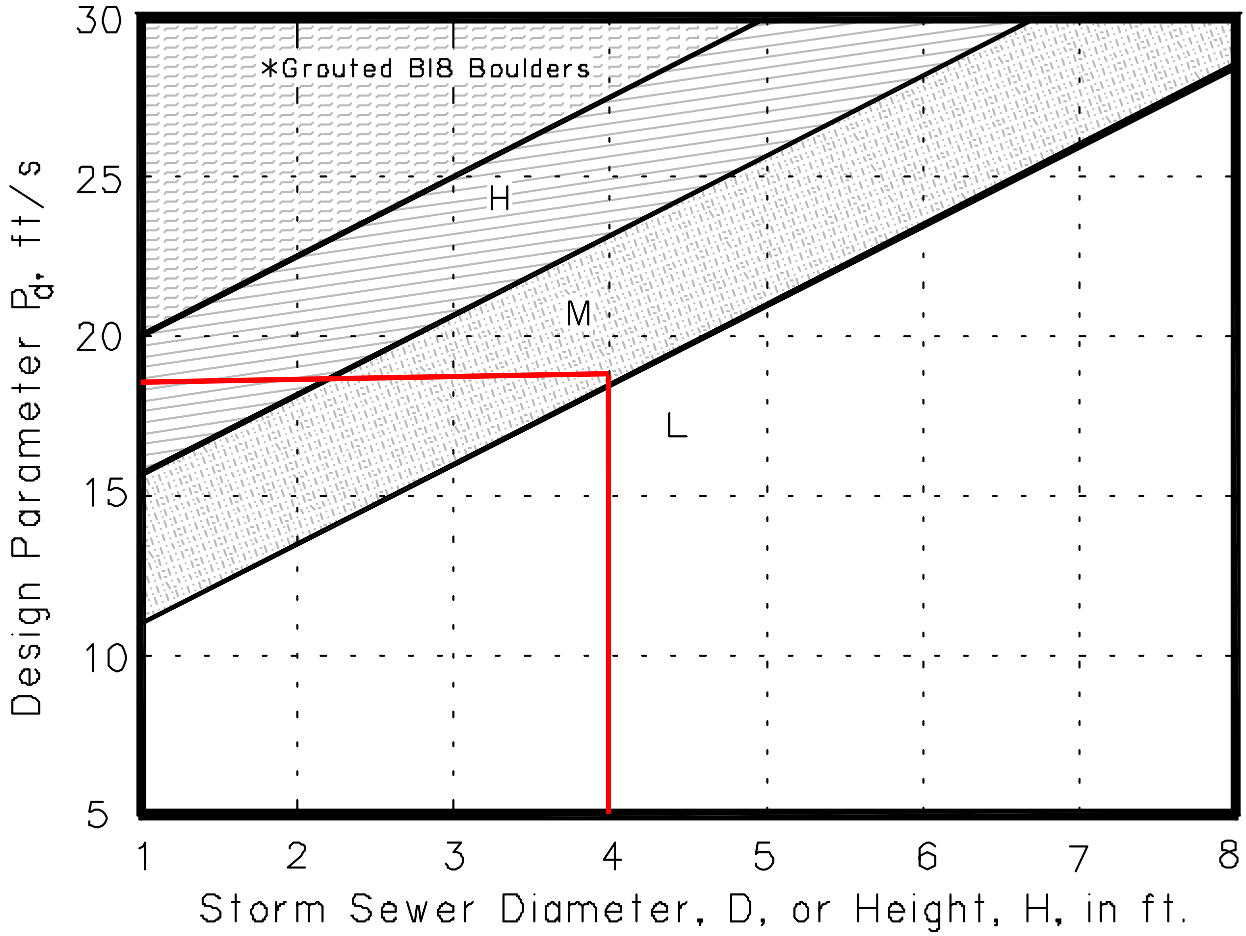


Figure HS-20c—Low Tailwater Riprap Basins for Storm Sewer Pipe Outlets—

Riprap Selection Chart for Low Tailwater Basin at Pipe Outlet

(Stevens and Urbonas 1996)

Appendix E – Temporary Sedimentation Ponds

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

TEMP POND 1

Tributary Area:	Required Volume	Depth at Outlet
29.5 ac.	1.2 ac-ft	6.7 ft.

Recommend simplifying the sub-basin in the rational to the tributary areas of the temp ponds

Area required per Row
1.0 in^2

WS Elev: 7075.2

No. of columns	Hole size
4	9/16 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7068.5	0	30	0.001	0.000	0.00
2	7069	0.5	1364	0.03	0.01	0.01
3	7070	1.5	4094	0.09	0.06	0.07
4	7071	2.5	6862	0.16	0.13	0.20
5	7072	3.5	8467	0.19	0.18	0.37
6	7073	4.5	10128	0.23	0.21	0.59
7	7074	5.5	11846	0.27	0.25	0.84
8	7075	6.5	13620	0.31	0.29	1.13
9	7076	7.5	15452	0.35	1.10	1.47

TABLE SB-2

Minimum steel thickness		1	2	3	**4**	5	6
		1/4	5/16	3/8	**3/8**	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	**0.99**	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

TEMP POND 2

Tributary Area:	Required Volume	Depth at Outlet
24.9 ac.	1.0 ac-ft	5.1 ft.

Area required per Row
0.9 in^2

WS Elev: 7036.6

No. of columns: 1

Hole size: 1 1/16 in

STAGE			STORAGE			
			AREA		VOLUME	
STAGE	ELEV	HEIGHT	sqft	acre	acft	cum acft
1	7031.5	0	10	0.000	0.000	0.00
2	7032	0.5	2660	0.06	0.02	0.02
3	7033	1.5	7570	0.17	0.12	0.13
4	7034	2.5	9179	0.21	0.19	0.33
5	7035	3.5	10861	0.25	0.23	0.56
6	7036	4.5	12614	0.29	0.27	0.82
7	7037	5.5	14438	0.33	0.31	1.13

TABLE SB-2

Minimum steel thickness		**1**	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	**0.89**	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

TEMP POND 3

Tributary Area:	Required Volume	Depth at Outlet
7.6 ac.	0.3 ac-ft	1.9 ft.

Area required per Row

0.8 in^2

WS Elev: 7063.4

No. of columns	Hole size
1	1 in

STAGE			STORAGE			
			AREA		VOLUME	
STAGE	ELEV	HEIGHT	sqft	acre	acft	cum acft
1	7061.5	0	10	0.000	0.000	0.00
2	7062	0.5	3807	0.09	0.02	0.02
3	7063	1.5	9900	0.23	0.16	0.18
4	7064	2.5	16892	0.39	0.31	0.49
5	7065	3.5	21531	0.49	0.44	0.93
6	7066	4.5	27137	0.62	0.56	1.49

TABLE SB-2

Minimum steel thickness		**1**	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	**0.79**	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

TEMP POND 4

Tributary Area:	Required Volume	Depth at Outlet
5.8 ac.	0.2 ac-ft	1.7 ft.

Area required per Row
0.8 in^2

WS Elev: 7057.2

No. of columns	Hole size
1	1 in

STAGE			STORAGE			
			AREA		VOLUME	
STAGE	ELEV	HEIGHT	sqft	acre	acft	cum acft
1	7055.5	0	10	0.000	0.000	0.00
2	7056	0.5	3763	0.09	0.02	0.02
3	7057	1.5	8129	0.19	0.14	0.16
4	7058	2.5	9414	0.22	0.20	0.36

TABLE SB-2							
Minimum steel thickness		**1**	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	**0.79**	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

TEMP POND 5

Tributary Area:	Required Volume	Depth at Outlet
4.5 ac.	0.2 ac-ft	2.2 ft.

Area required per Row
0.5 in^2

WS Elev: 7026.7

No. of columns	Hole size
2	9/16 in

STAGE			STORAGE			
			AREA		VOLUME	
STAGE	ELEV	HEIGHT	sqft	acre	acft	cum acft
1	7024.5	0	10	0.000	0.000	0.00
2	7025	0.5	785	0.02	0.00	0.00
3	7026	1.5	3560	0.08	0.05	0.05
4	7027	2.5	4630	0.11	0.09	0.15
5	7028	3.5	5785	0.13	0.21	0.27

TABLE SB-2							
Minimum steel thickness		1	**2**	3	4	5	6
		1/4	**5/16**	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	**0.50**	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

SEDIMENTATION SIZING

PERMANENT POND 6

Tributary Area:	Required Volume	Depth at Outlet
12.5 ac.	0.5 ac-ft	3.7 ft.

Area required per Row
0.6 in^2

WS Elev: 7094.6

No. of columns	Hole size
1	7/8 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7090.9	0	0	0.000	0.000	0.00
2	7091	0.1	44	0.00	0.00	0.00
3	7092	1.1	5600	0.13	0.06	0.06
4	7093	2.1	6835	0.16	0.14	0.21
5	7094	3.1	8170	0.19	0.17	0.38
6	7095	4.1	9606	0.22	0.20	0.58

TABLE SB-2

Minimum steel thickness		**1**	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	**0.60**	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

ROLLING HILLS RANCH GRADING

TEMPORARY SEDIMENTATION SIZING

LAMBERT POND

Tributary Area:	Required Volume	Depth at Outlet
53.5 ac.	2.2 ac-ft	8.7 ft.

Area required per Row

1.7 in^2

WS Elev: 7019.3

No. of columns	Hole size
3	13/16 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7010.6	0	10	0.000	0.000	0.00
2	7011	0.4	810	0.02	0.00	0.00
3	7012	1.4	3352	0.08	0.05	0.05
4	7013	2.4	6498	0.15	0.11	0.16
5	7014	3.4	9562	0.22	0.18	0.35
6	7015	4.4	11631	0.27	0.57	0.57
7	7016	5.4	13756	0.32	0.79	0.84
8	7017	6.4	15939	0.37	1.03	1.19
9	7018	7.4	18178	0.42	1.27	1.62
10	7019	8.4	20473	0.47	0.44	2.07
11	7020	9.4	22825	0.52	0.50	2.56

TABLE SB-2

Minimum steel thickness		1	2	**3**	4	5	6
		1/4	5/16	**3/8**	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	**0.44**	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	**1.56**	2.07	2.59	3.11

Appendix F – Regional Stormwater Quality Analysis

Several Regional Detention Facilities are located within the Meridian Ranch, all the detention facilities have Water Quality Capture Volume incorporated into the design and construction. The facilities are owned and maintained by the Meridian Service Metropolitan District under the jurisdiction and authority of El Paso County. The design and construction of the facilities meet the minimum standards of the County as outlined in the Drainage Criteria Manual and Engineering Criteria Manual. The WQCV found in each of the detention facilities was designed to provide water quality for 100 percent of the tributary area for the facility. Regional Facilities are designed and are intended as flood control and water quality as the primary use.

Below is the governing section from the ECM regarding the use of regional detention facilities with a WQCV component for reference:

Appendix I Stormwater Quality Policy and Procedures-revisions
I.7.1.C.5.

Applicable Development Site Draining to a Regional WQCV Facility The regional WQCV facility is designed to accept drainage from the Applicable development site. Stormwater from the site may discharge to a water of the state before being discharged to the regional WQCV facility. Before discharging to a water of the state, at least 20 percent of the upstream imperviousness of the applicable development site must be disconnected from the storm drainage system and drain through a receiving pervious area control measure comprising a footprint of at least 10 percent of the upstream disconnected impervious area of the applicable development site. The control measure must be designed in accordance with a design manual identified by the permittee. In addition,
The stream channel between the discharge point of the applicable development site and the regional WQCV facility must be stabilized. The regional WQCV facility must meet the following requirements:

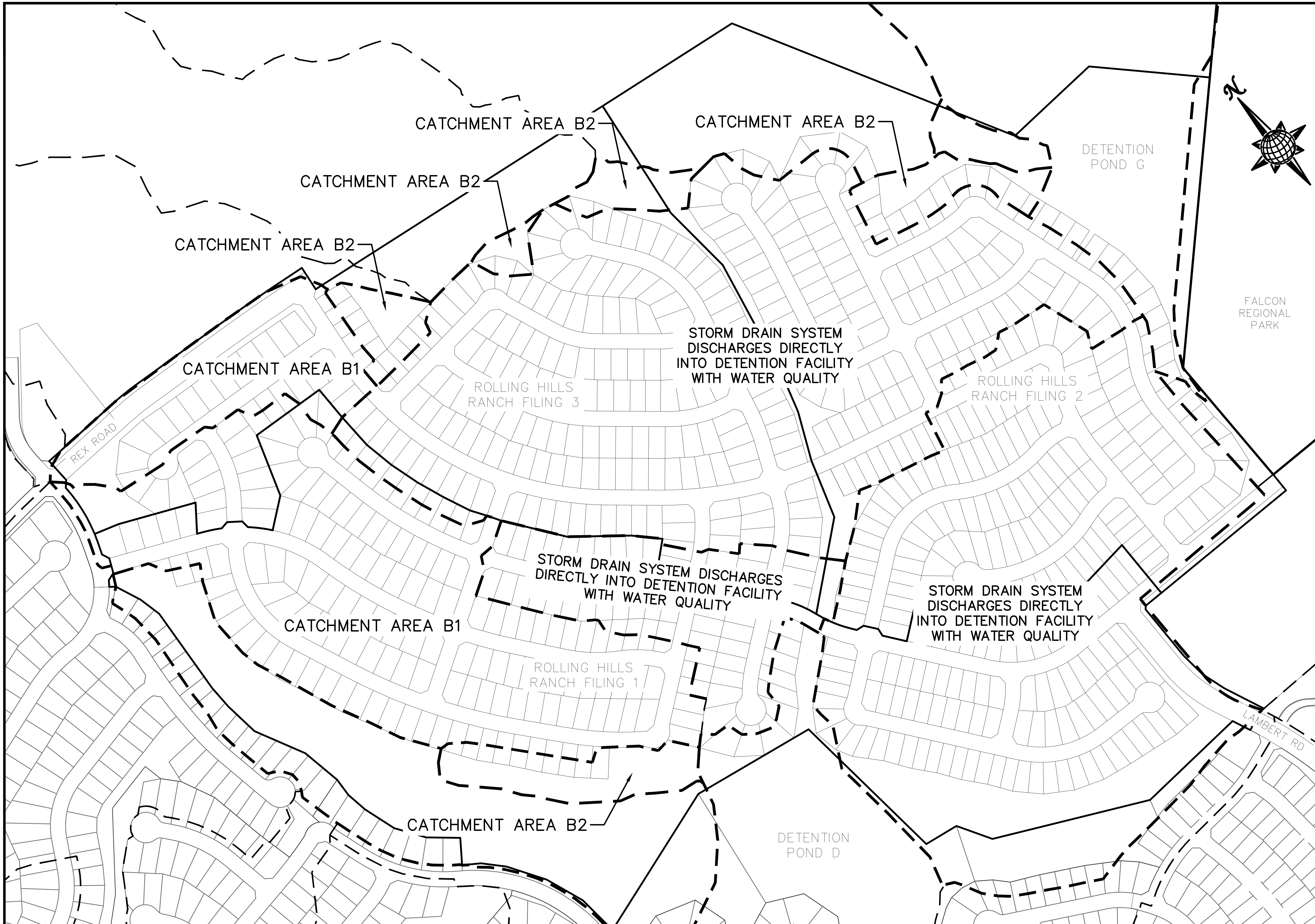
a. The regional WQCV facility must be implemented, functional, and maintained following good engineering, hydrologic and pollution control practices.
b. The regional WQCV facility must be designed and maintained for 100% WQCV for its entire drainage area.
c. The regional WQCV facility must have capacity to accommodate the drainage from the applicable development site.
d. The regional WQCV facility must be designed and built to comply with all assumptions for the development activities planned by the County within its drainage area, including the imperviousness of its drainage area and the applicable development site.
e. Evaluation of the minimum drain time shall be based on the pollutant removal mechanism and functionality of the facility. Consideration of drain time shall include maintaining vegetation necessary for operation of the facility (e.g., wetland vegetation).
f. The County shall require site plans and perform a site plan review consistent with the requirements of this ECM to ensure the regional WQCV facility and control measures for the applicable development site plans include:
    i. Design details for all structural control measures implemented to meet the requirements of Part I.E.4.
    ii. A narrative reference for all non-structural control measures for the site, if applicable. "Non-structural control measures” are control measures that are not

structural control measures and include, but are not limited to, control measures that preventor reduce pollutants being introduced to water or that prevent or reduce the generation of runoff or illicit discharges.

iii. Documentation of operation and maintenance procedures to ensure the long-term observation, maintenance, and operation of the control measures. The documentation shall include frequencies for routine inspections and maintenance activities.
iv. Documentation regarding easements or other legal means for access of the control measure sites for operation, maintenance, and inspection of control measures.
v. Confirmation that control measures meet the requirements of section I.7.C.
vi. Confirmation that site plans meet the requirements of County's Site plan review and approval requirements.

g. The regional WQCV facility must be subject to the County's authority consistent with requirements and actions for a Control Measure in accordance with a base design standard.
h. Regional Facilities must be designed and implemented with flood control or water quality as the primary use. Recreational ponds and reservoirs may not be considered Regional Facilities. Water bodies listed by name in surface water quality classifications and standards regulations (5CCR1002-32 through 5CCR1002-38) may not be considered regional facilities.

See the exhibits on the following pages for impacted areas, calculations and more information.



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REGIONAL WATER QUALITY

OVERALL MAP

ROLLING HILLS RANCH PUD

Drawn by	TAK
Checked by	-
Date	FEB 2020
Scale	N.T.S.
Sheet Number	1

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These are not required with this FDR since the project is for site grading only.

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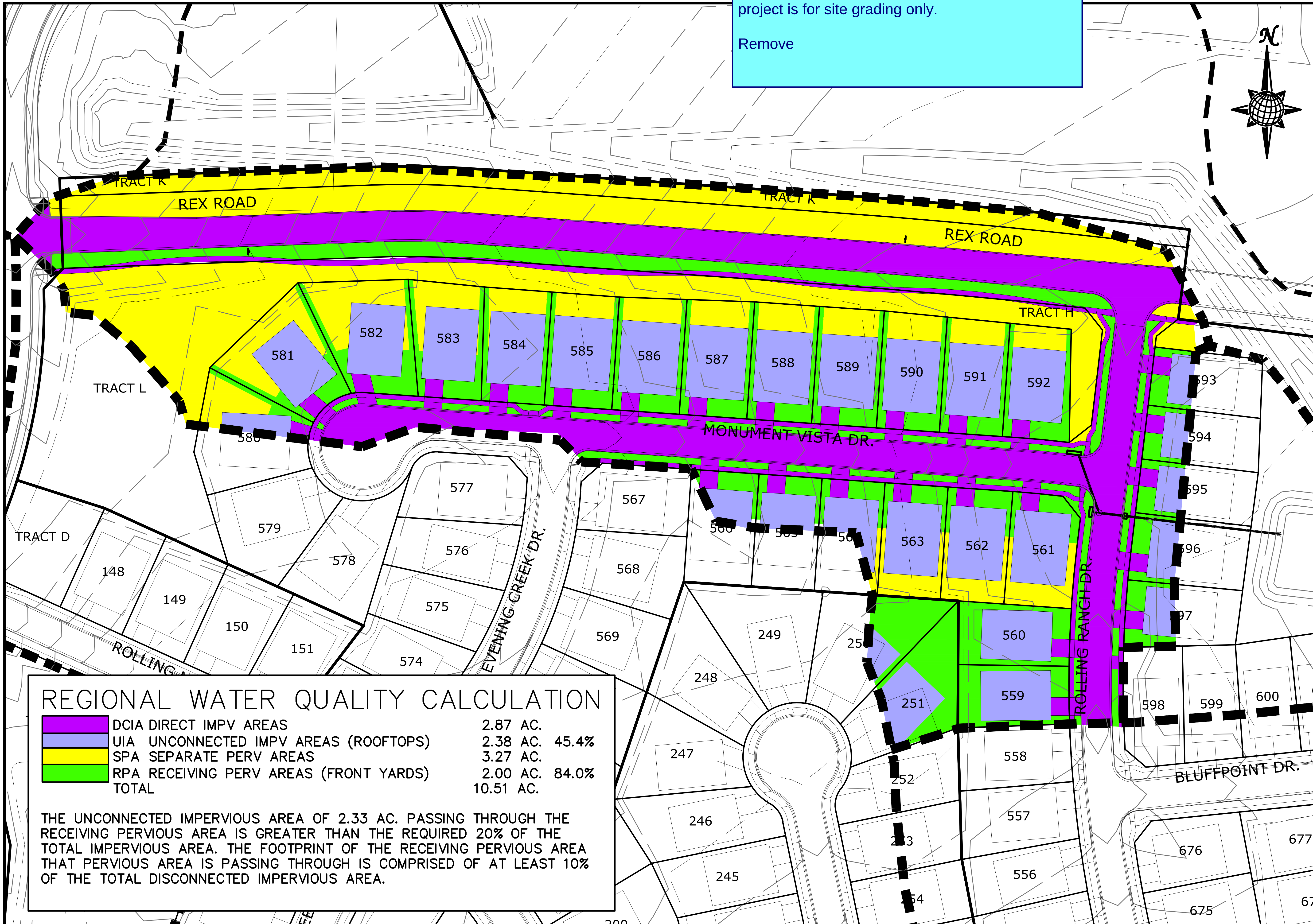

REGIONAL WATER QUALITY CALCULATION

DCIA DIRECT IMPV AREAS	2.87 AC.	
UIA UNCONNECTED IMPV AREAS (ROOFTOPS)	2.38 AC.	45.4%
SPA SEPARATE PERV AREAS	3.27 AC.	
RPA RECEIVING PERV AREAS (FRONT YARDS)	2.00 AC.	84.0%
TOTAL	10.51 AC.	

THE UNCONNECTED IMPERVIOUS AREA OF 2.33 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

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REGIONAL WATER QUALITY
CATCHMENT AREA A1
ROLLING HILLS RANCH PUD

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Scale: N.T.S.
Sheet Number: 2

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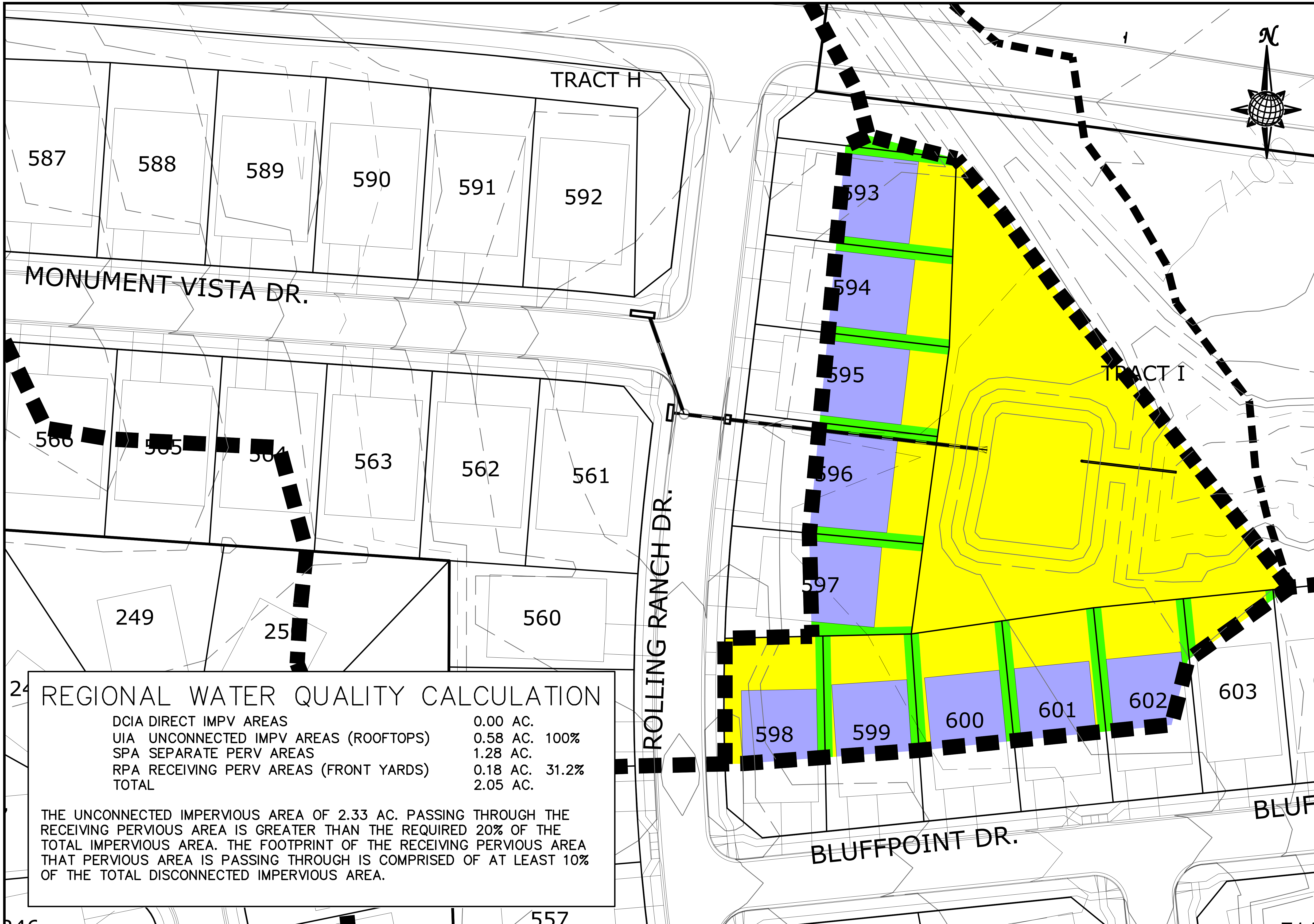

REGIONAL WATER QUALITY CALCULATION

DCIA DIRECT IMPV AREAS	0.00 AC.	
UIA UNCONNECTED IMPV AREAS (ROOFTOPS)	0.58 AC.	100%
SPA SEPARATE PERV AREAS	1.28 AC.	
RPA RECEIVING PERV AREAS (FRONT YARDS)	0.18 AC.	31.2%
TOTAL	2.05 AC.	

THE UNCONNECTED IMPERVIOUS AREA OF 2.33 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

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REGIONAL WATER QUALITY
CATCHMENT AREA A2
ROLLING HILLS RANCH PUD

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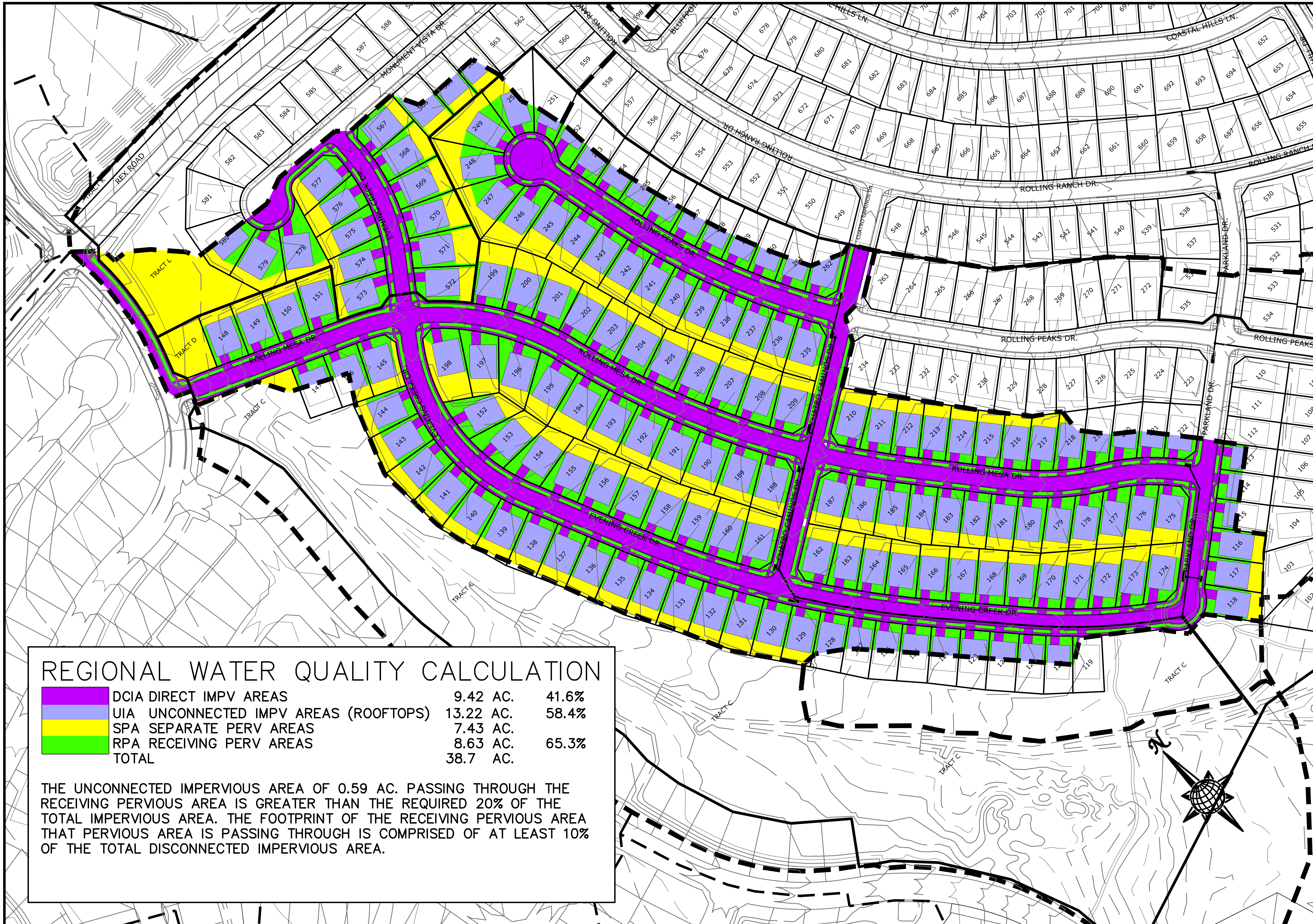
Sheet Number: 3



REGIONAL WATER QUALITY CALCULATION

DCIA	DIRECT IMPV AREAS	9.42 AC.	41.6%
UIA	UNCONNECTED IMPV AREAS (ROOFTOPS)	13.22 AC.	58.4%
SPA	SEPARATE PERV AREAS	7.43 AC.	
RPA	RECEIVING PERV AREAS	8.63 AC.	65.3%
	TOTAL	38.7 AC.	

THE UNCONNECTED IMPERVIOUS AREA OF 0.59 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

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REGIONAL WATER QUALITY
CATCHMENT AREA B1
ROLLING HILLS RANCH PUD

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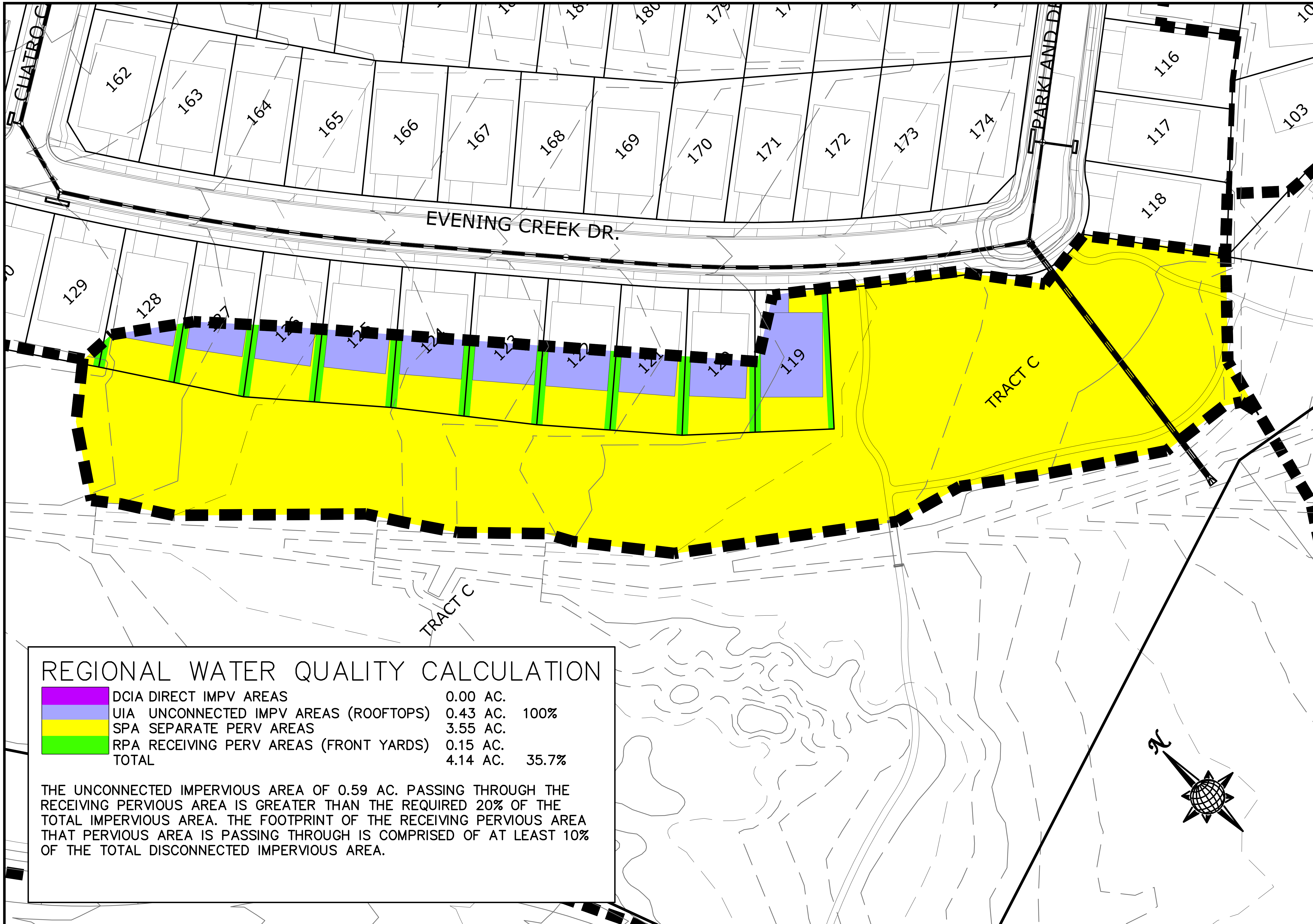

REGIONAL WATER QUALITY CALCULATION

DCIA	DIRECT IMPV AREAS	0.00 AC.	
UIA	UNCONNECTED IMPV AREAS (ROOFTOPS)	0.43 AC.	100%
SPA	SEPARATE PERV AREAS	3.55 AC.	
RPA	RECEIVING PERV AREAS (FRONT YARDS)	0.15 AC.	
	TOTAL	4.14 AC.	35.7%

THE UNCONNECTED IMPERVIOUS AREA OF 0.59 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

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REGIONAL WATER QUALITY
CATCHMENT AREA B2
ROLLING HILLS RANCH PUD

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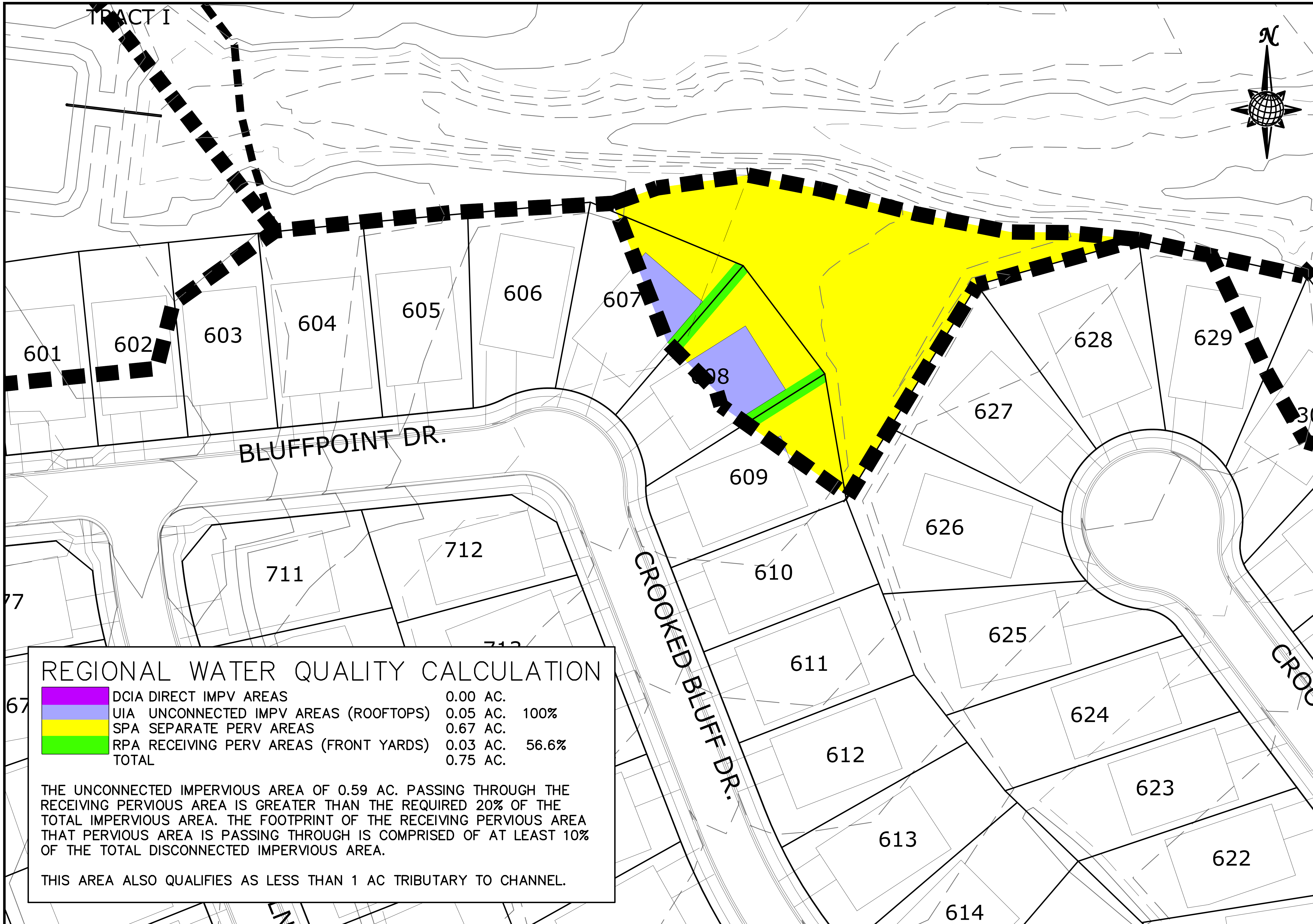


REGIONAL WATER QUALITY CALCULATION

DCIA	DIRECT IMPV AREAS	0.00 AC.	
UIA	UNCONNECTED IMPV AREAS (ROOFTOPS)	0.05 AC.	100%
SPA	SEPARATE PERV AREAS	0.67 AC.	
RPA	RECEIVING PERV AREAS (FRONT YARDS)	0.03 AC.	56.6%
	TOTAL	0.75 AC.	

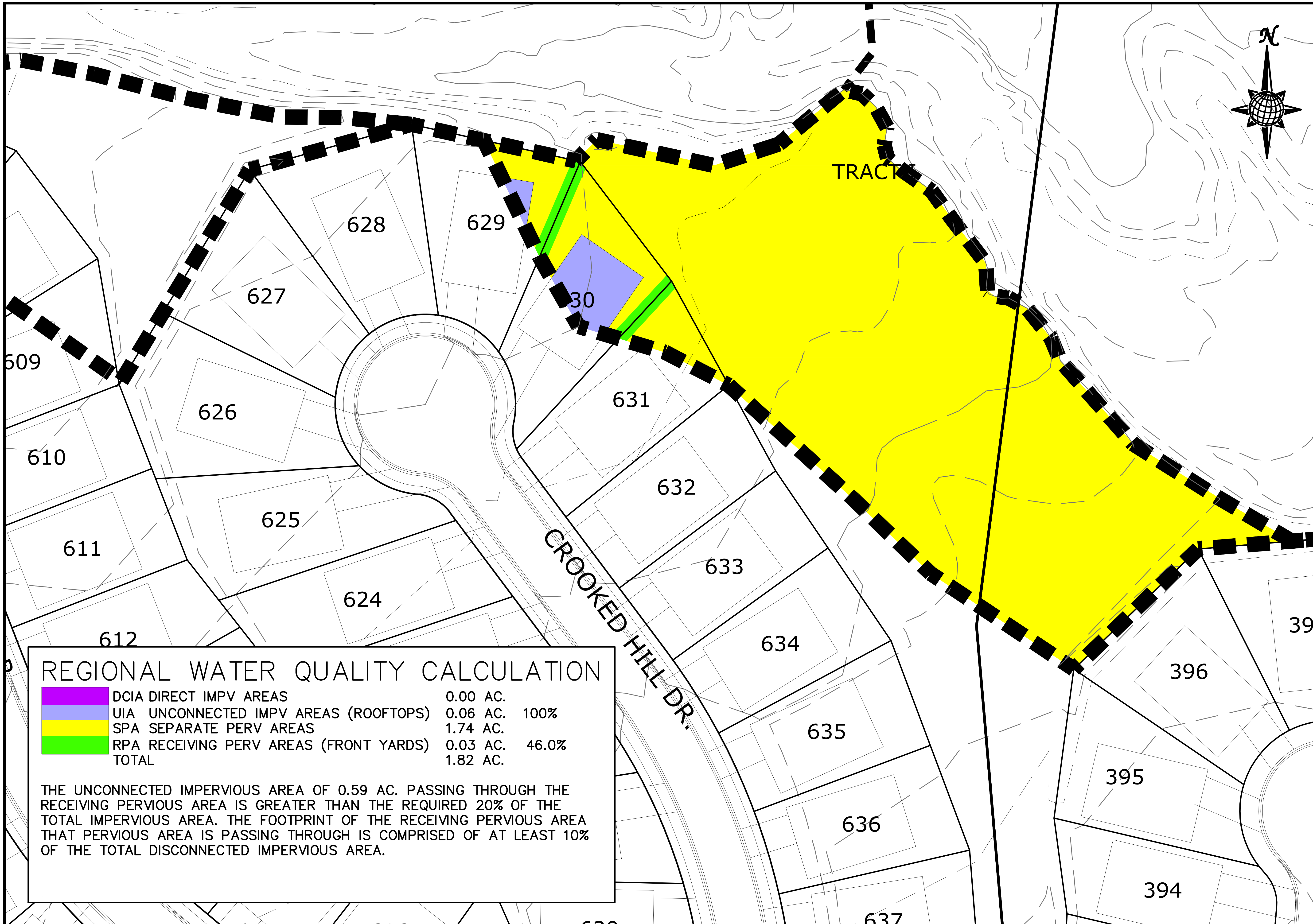
THE UNCONNECTED IMPERVIOUS AREA OF 0.59 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

THIS AREA ALSO QUALIFIES AS LESS THAN 1 AC TRIBUTARY TO CHANNEL.

TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
TELEPHONE: 719.495.7444
FAX: 719.495.3349

REGIONAL WATER QUALITY
CATCHMENT AREA D1
ROLLING HILLS RANCH PUD

Drawn by	TAK
Checked by	-
Date	FEB 2020
Scale	N.T.S.
Sheet Number	6



REGIONAL WATER QUALITY CALCULATION

DCIA	DIRECT IMPV AREAS	0.00 AC.	
UIA	UNCONNECTED IMPV AREAS (ROOFTOPS)	0.06 AC.	100%
SPA	SEPARATE PERV AREAS	1.74 AC.	
RPA	RECEIVING PERV AREAS (FRONT YARDS)	0.03 AC.	46.0%
	TOTAL	1.82 AC.	

THE UNCONNECTED IMPERVIOUS AREA OF 0.59 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

TECH CONTRACTORS
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FALCON, CO 80831
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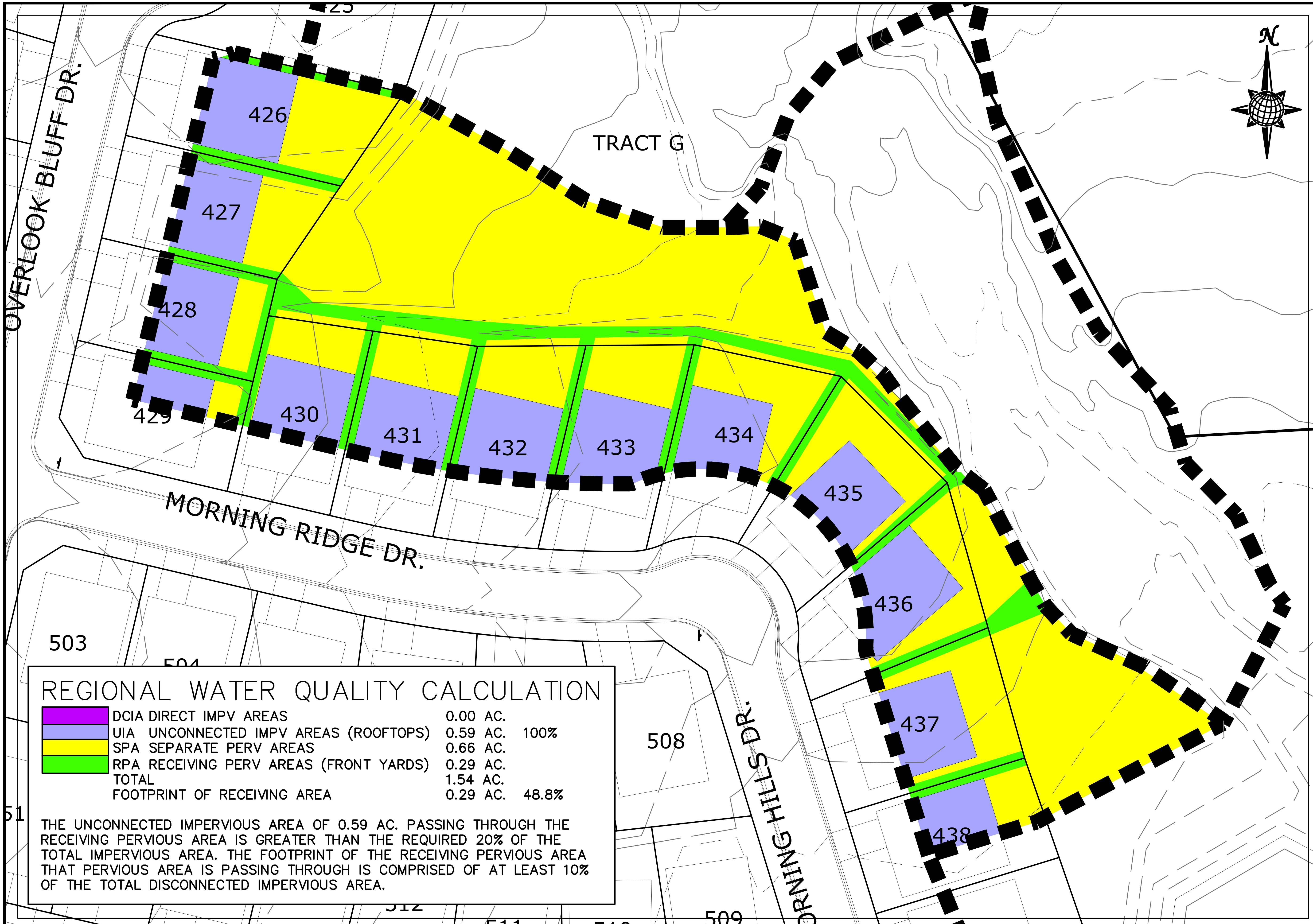
REGIONAL WATER QUALITY
CATCHMENT AREA D2
ROLLING HILLS RANCH PUD

Drawn by: TJK
Checked by: -
Date: FEB 2020
Scale: N.T.S.
Sheet Number: 7



REGIONAL WATER QUALITY CALCULATION

DCIA	DIRECT IMPV AREAS	0.00 AC.	
UIA	UNCONNECTED IMPV AREAS (ROOFTOPS)	0.59 AC.	100%
SPA	SEPARATE PERV AREAS	0.66 AC.	
RPA	RECEIVING PERV AREAS (FRONT YARDS)	0.29 AC.	
	TOTAL	1.54 AC.	
	FOOTPRINT OF RECEIVING AREA	0.29 AC.	48.8%

THE UNCONNECTED IMPERVIOUS AREA OF 0.59 AC. PASSING THROUGH THE RECEIVING PERVIOUS AREA IS GREATER THAN THE REQUIRED 20% OF THE TOTAL IMPERVIOUS AREA. THE FOOTPRINT OF THE RECEIVING PERVIOUS AREA THAT PERVIOUS AREA IS PASSING THROUGH IS COMPRISED OF AT LEAST 10% OF THE TOTAL DISCONNECTED IMPERVIOUS AREA.

TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
TELEPHONE: 719.495.7444
FAX: 719.495.3349

REGIONAL WATER QUALITY
CATCHMENT AREA D3
ROLLING HILLS RANCH PUD

Drawn by TAK
Checked by -
Date FEB 2020
Scale N.T.S.
Sheet Number 8

Appendix G – Soil Resource Report

United States Department of Agriculture

NRCS

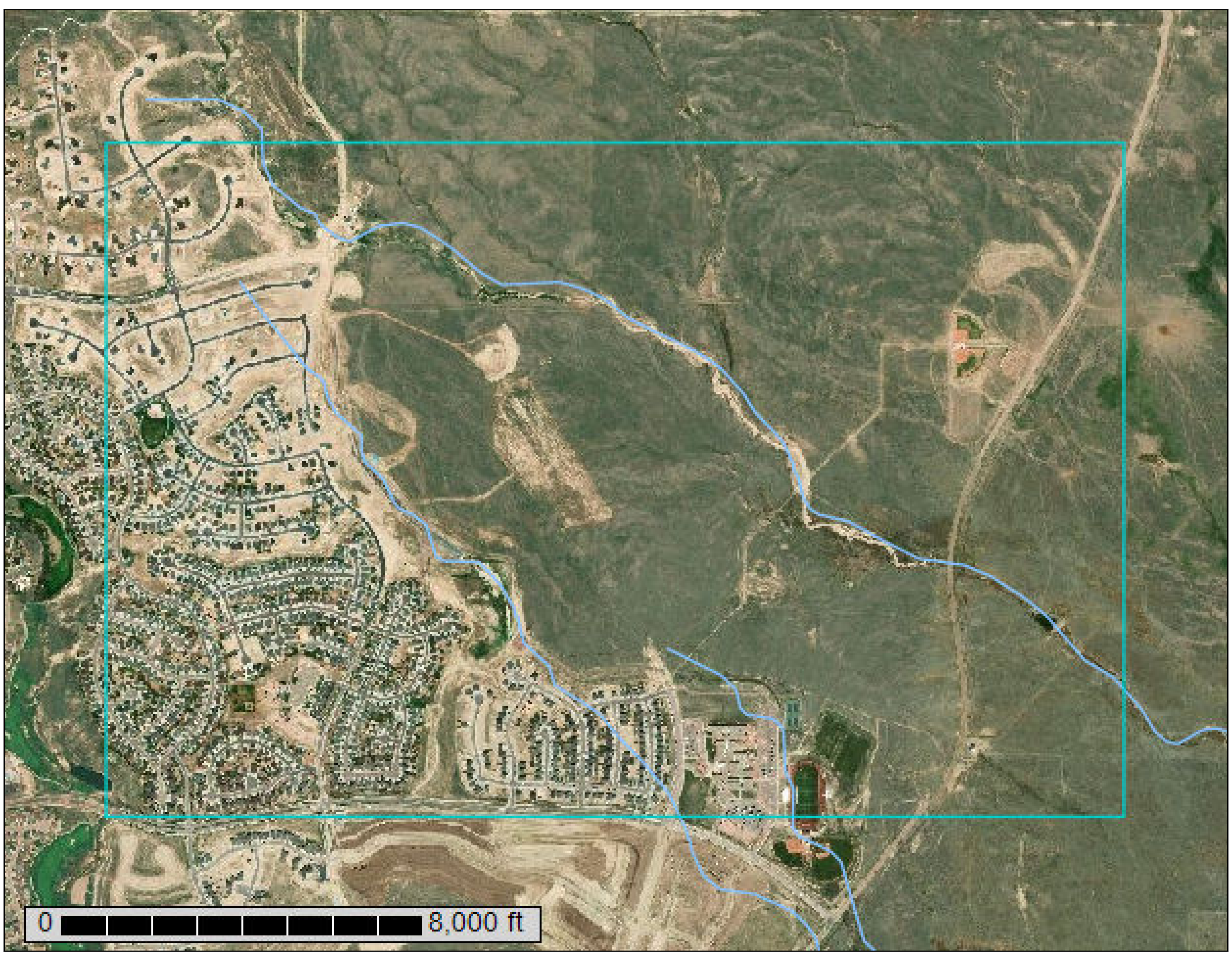
Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for

El Paso County Area, Colorado

ROLLING HILLS RANCH PUD



February 21, 2019

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Contents

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Soil Map

104° 35' 57" W
104° 33' 37" W
38° 59' 48" N
38° 58' 37" N
534900
535200
535500
535800
536100
536400
536700
537000
537300
537600
537900
4316500
4316200
4315900
4315600
4315300
4315000
4314700
4314400
83
19
Eastonville Rd
Map Scale: 1:15,400 if printed on A landscape (11" x 8.5") sheet.
Meters
0 200 400 800 1200
Feet
0 500 1000 2000 3000
N
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 16, Sep 10, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 7, 2016—Aug 17, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	387.7	31.2%
83	Stapleton sandy loam, 3 to 8 percent slopes	855.6	68.8%
Totals for Area of Interest		**1,243.3**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

19—Columbine gravelly sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367p
Elevation: 6,500 to 7,300 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 46 to 50 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Columbine and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Columbine

Setting

Landform: Flood plains, fan terraces, fans
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium

Typical profile

A - 0 to 14 inches: gravelly sandy loam
C - 14 to 60 inches: very gravelly loamy sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: A
Ecological site: Gravelly Foothill (R049BY214CO)
Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit:
Landform: Swales
Hydric soil rating: Yes

Pleasant

Percent of map unit:

Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

83—Stapleton sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369z
Elevation: 6,500 to 7,300 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 46 to 48 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Stapleton and similar soils: 80 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Stapleton

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy alluvium derived from arkose

Typical profile

A - 0 to 11 inches: sandy loam
Bw - 11 to 17 inches: gravelly sandy loam
C - 17 to 60 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 4.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B

Ecological site: Gravelly Foothill (R049BY214CO)
Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Fluvaquentic haplaquolls

Percent of map unit:
Landform: Swales
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

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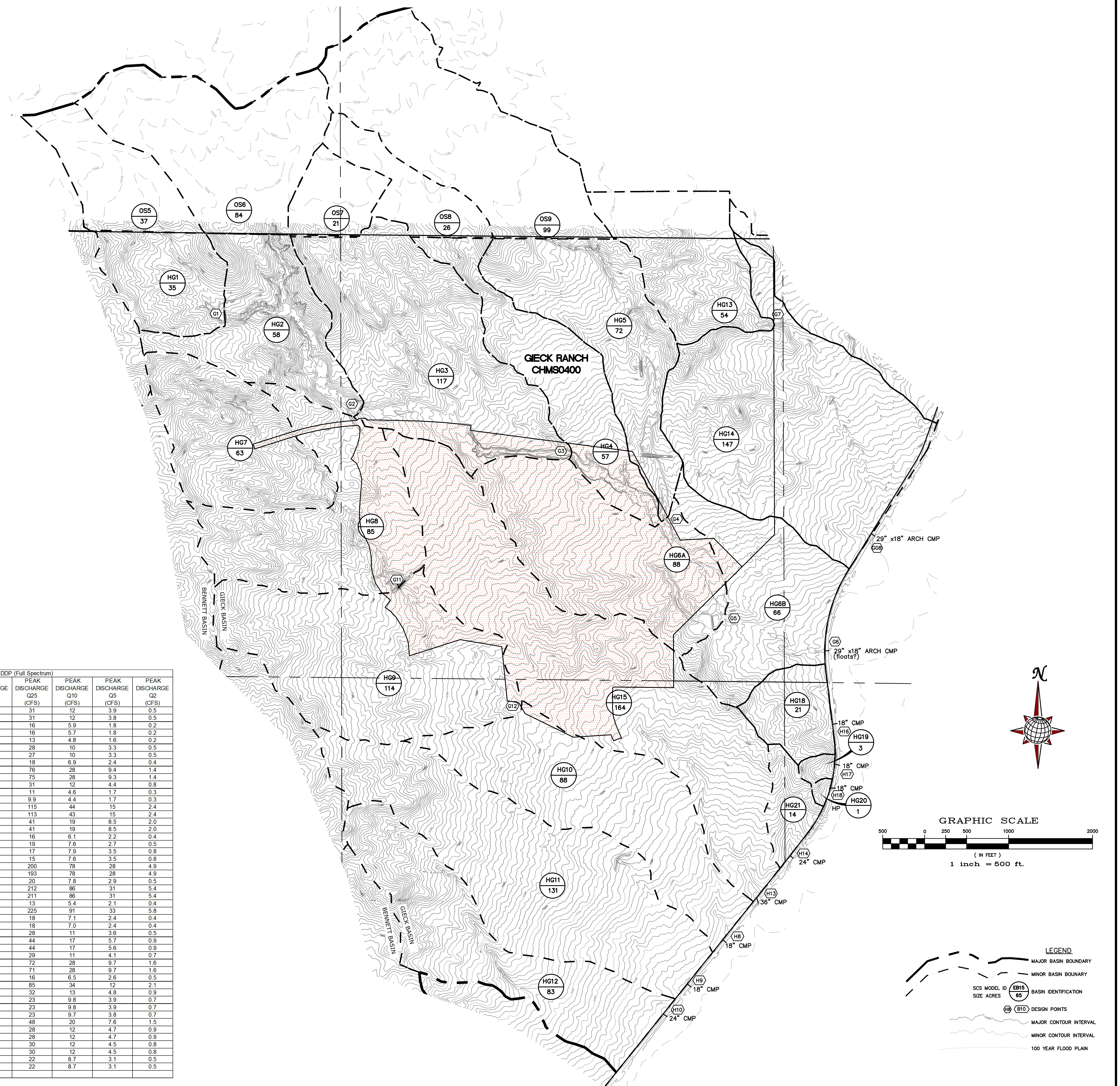
Appendix H – Drainage Maps

ROLLING HILLS RANCH
MERIDIAN RANCH



HISTORIC MDDP (Full Spectrum)

HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	81	53	31	12	3.9	0.5
OS06-G02	0.1313	79	52	31	12	3.8	0.5
OS05	0.0578	40	26	16	5.9	1.8	0.2
OS05-G01	0.0578	38	26	16	5.7	1.8	0.2
HG01	0.0547	33	21	13	4.8	1.6	0.2
G01	0.1125	71	47	28	10	3.3	0.5
G01-G02	0.1125	70	47	27	10	3.3	0.5
HG02	0.0906	46	30	18	6.9	2.4	0.4
G02	0.3344	194	129	76	28	9.4	1.4
G02-G03	0.3344	192	127	75	28	9.3	1.4
HG03	0.1828	79	51	31	12	4.4	0.8
OS07	0.0328	25	17	11	4.6	1.7	0.3
OS07-G03	0.0328	24	17	9.9	4.4	1.7	0.3
G03	0.55	295	195	115	44	15	2.4
G03-G04	0.55	286	192	113	43	15	2.4
OS09	0.1547	92	64	41	19	8.5	2.0
OS09-G04	0.1547	91	63	41	19	8.5	2.0
HG04	0.0891	40	27	16	6.1	2.2	0.4
HG05	0.1125	50	33	19	7.6	2.7	0.5
OS08	0.0406	36	25	17	7.9	3.5	0.8
OS08-G04	0.0406	34	24	15	7.6	3.5	0.8
G04	0.9469	502	336	200	78	28	4.9
G04-G05	0.9469	496	322	193	78	28	4.9
HG06A	0.1375	50	33	20	7.8	2.9	0.5
G05	1.0844	544	355	212	86	31	5.4
G05-G06	1.0844	530	353	211	86	31	5.4
HG06B	0.1031	34	22	13	5.4	2.1	0.4
G06	1.1875	561	375	225	91	33	5.8
HG07	0.0984	47	31	18	7.1	2.4	0.4
HG07-G11	0.0984	47	31	18	7.0	2.4	0.4
HG08	0.1328	73	48	28	11	3.6	0.5
G11	0.2312	115	75	44	17	5.7	0.9
G11-G12	0.2312	114	75	44	17	5.6	0.9
HG09	0.1781	73	48	29	11	4.1	0.7
G12	0.4093	187	122	72	28	9.7	1.6
G12-H08	0.4093	183	121	71	28	9.7	1.6
HG10	0.1375	39	26	16	6.5	2.6	0.5
H08	0.5468	216	142	85	34	12	2.1
HG14	0.2297	81	53	32	13	4.8	0.9
HG13	0.0844	55	37	23	9.8	3.9	0.7
G07	0.0844	55	37	23	9.8	3.9	0.7
G07-G08	0.0844	54	37	23	9.7	3.8	0.7
G08	0.3141	119	78	48	20	7.6	1.5
HG15	0.2563	70	46	28	12	4.7	0.9
H13	0.2563	70	46	28	12	4.7	0.9
HG11	0.2047	77	51	30	12	4.5	0.8
H09	0.2047	77	51	30	12	4.5	0.8
HG12	0.1297	57	38	22	8.7	3.1	0.5
H10	0.1297	57	38	22	8.7	3.1	0.5

TECH CONTRACTORS
11886 STAPLETON DRIVE
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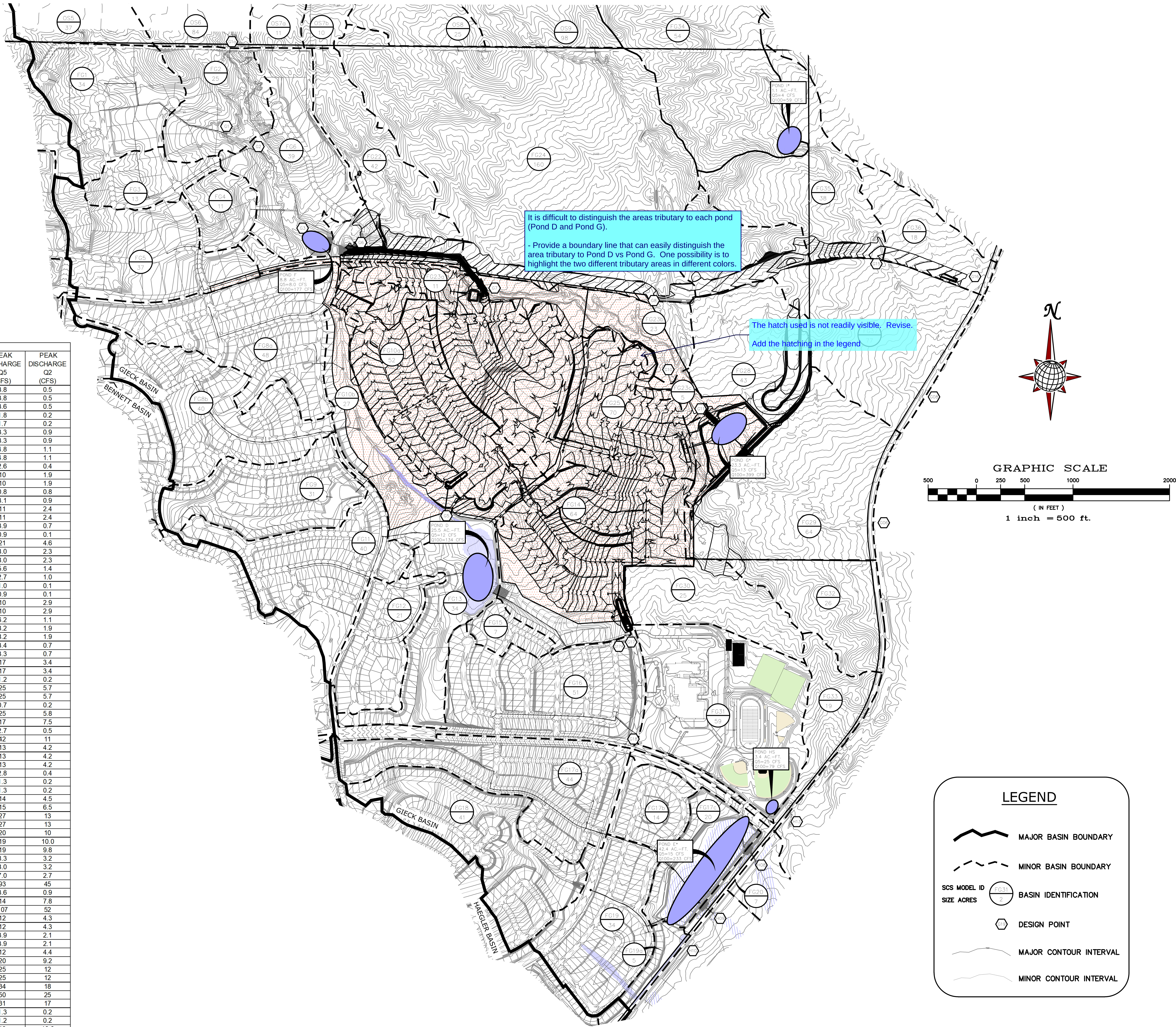
HISTORIC CONDITIONS - SCS MAP

FEB 2020

FIGURE 4

ROLLING HILL RANCH PUD
MERIDIAN RANCH



INTERIM MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	80	52	30	12	3.8	0.5
G1a	0.1313	80	52	30	12	3.8	0.5
G1a-G2	0.1313	79	52	30	11	3.6	0.5
OS05	0.0578	39	26	15	5.6	1.8	0.2
OS05-G1	0.0578	39	25	15	5.5	1.7	0.2
FG01	0.0531	31	22	14	6.9	3.3	0.9
FG01-G1	0.0531	31	22	14	6.9	3.3	0.9
G1	0.1109	61	41	25	11	4.8	1.1
G1-G2	0.1109	60	41	25	11	4.8	1.1
FG02	0.0391	32	22	14	6.2	2.6	0.4
G2	0.2813	166	112	67	27	10	1.9
G2-G3	0.2813	163	108	66	27	10	1.9
FG03	0.0203	24	17	12	5.9	0.8	0.8
FG04	0.0172	22	16	11	5.8	3.1	0.9
G3	0.3188	184	123	74	31	11	2.4
G3-POND F	0.3188	183	121	74	31	11	2.4
FG06	0.0658	46	32	20	9.1	3.9	0.7
OS07a-POND F	0.0170	13	9.0	5.7	2.4	0.9	0.1
POND F IN	0.4596	285	193	120	53	21	4.6
POND F	0.4596	177	121	60	17	8.0	2.3
POND F-G7	0.4596	177	120	60	17	8.0	2.3
FG22	0.0658	51	36	24	12	5.6	1.4
FG23a	0.0177	18	13	9.0	4.9	2.7	1.0
OS07b	0.0156	15	10	6.2	2.6	1.0	0.1
OS07b-G7	0.0156	13	9.2	5.4	2.3	0.9	0.1
G7	0.5587	215	145	72	22	10	2.9
G7-G10	0.5587	215	144	72	22	10	2.9
FG24	0.2503	110	73	43	17	6.2	1.1
OS09	0.1527	90	62	39	18	8.2	1.9
OS09-G10	0.1527	88	61	39	18	8.2	1.9
OS08	0.0397	35	24	16	7.5	3.4	0.7
OS08-G8	0.0397	34	24	15	7.2	3.3	0.7
G8	0.4427	227	154	94	41	17	3.4
G8-G10	0.4427	225	151	94	41	17	3.4
FG23b	0.0359	21	14	8.4	3.3	1.2	0.2
G10	1.0373	444	287	146	60	25	5.7
G10-G11	1.0373	442	285	145	60	25	5.7
FG23c	0.0070	5.8	4.2	2.8	1.4	0.7	0.2
G11	1.0443	445	287	146	61	25	5.8
FG25	0.1086	85	64	46	27	17	7.5
FG28	0.0673	38	26	16	6.7	2.7	0.5
POND G IN	1.2202	558	363	202	91	42	11
POND G	1.2202	399	237	117	35	13	4.2
G12	1.2202	399	237	117	35	13	4.2
G12-G06	1.2202	398	236	117	35	13	4.2
FG29	0.0997	60	39	23	8.7	2.8	0.4
FG32	0.0402	29	19	11	4.2	1.3	0.2
FG32-G06	0.0402	28	19	11	4.1	1.3	0.2
G06	1.3601	418	248	124	37	14	4.5
FG10A	0.0806	81	61	43	25	15	6.5
FG08A	0.0750	116	90	66	41	27	13
FG08A-G05	0.0750	110	86	64	41	27	13
FG08B	0.0630	86	67	49	31	20	10
FG08B-G05	0.0630	84	65	48	29	19	10.0
FG11	0.0625	75	59	44	28	19	9.8
FG09	0.0484	48	36	25	14	8.3	3.2
FG09-G05	0.0484	48	36	25	14	8.0	3.2
FG10B	0.0416	42	31	22	12	7.0	2.7
G05	0.3711	433	330	239	145	93	45
FG13	0.0534	34	24	15	7.5	3.6	0.9
FG12	0.0328	50	40	30	20	14	7.8
POND D IN	0.4573	509	387	280	168	107	52
POND D	0.4573	134	91	50	19	12	4.3
POND D-G17	0.4573	134	91	50	19	12	4.3
FG15	0.0103	15	12	9.0	5.8	3.9	2.1
FG15-G17A	0.0103	15	12	8.9	5.8	3.9	2.1
G17A	0.4676	137	93	51	19	12	4.4
FG14	0.1000	98	74	53	32	20	9.2
G17	0.5676	196	132	75	43	25	12
G17-G18	0.5676	196	131	75	43	25	12
FG16	0.0791	133	104	78	50	34	18
G18-POND E	0.6467	240	176	126	78	50	25
FG31	0.0922	116	92	69	45	31	17
FG30	0.0389	30	20	12	4.3	1.3	0.2
FG30-PONDHS	0.0389	28	19	11	4.2	1.2	0.2
POND HS	0.1311	112	63	40	28	19	10.0
FG17a	0.0694	101	78	57	35	23	12
FG17a-POND E	0.0694	99	76	56	35	23	12
FG18	0.0644	56	42	30	18	11	4.7
FG18-POND E	0.0644	56	42	30	17	11	4.6
FG19	0.0527	84	66	50	33	23	13
FG17c	0.0313	31	22	14	6.5	2.8	0.5
FG17b	0.0214	39	31	24	16	11	6.1
POND E IN	1.0170	552	423	308	190	123	63
POND E	1.0170	233	144	74	28	15	6.1
H08	1.0170	200	130	66	22	11	3.7
H09	0.0000	34	14	7.9	5.7	3.9	2.4
FG34	0.0836	48	32	19	7.0	2.3	0.3
G14	0.0836	48	32	19	7.0	2.3	0.3
G14-G15	0.0836	48	32	18	7.0	2.3	0.3
FG35	0.0586	19	13	7.4	3.0	1.2	0.2
G15	0.1422	60	39	23	8.9	3.1	0.5
G15-G08	0.1422	59	39	23	8.8	3.1	0.5
FG37	0.1203	44	29	17	6.8	2.5	0.4
FG36	0.0281	16	10	6	2.4	0.9	0.1
FG36-G08	0.0281	15	10	6	2.4	0.8	0.1
G08	0.2906	114	75	44	17.6	6.4	1.1

*NOTE: PRELIMINARY STORAGE VOLUMES AND OUTFLOW QUANTITIES HAVE BEEN PROVIDED FOR EACH OF THE FUTURE DETENTION FACILITIES LOCATED WITHIN THE DEVELOPMENT. THE ACTUAL STORAGE VOLUMES AND DISCHARGE RATES WILL BE DETERMINED UPON A COMPLETE ANALYSIS FOR EACH DETENTION FACILITY PRIOR TO CONSTRUCTION. THE VALUES GIVEN FOR DISCHARGE AND VOLUME ARE ESTIMATES FOR PLANNING PURPOSES ONLY.

INTERIM CONDITIONS - SCS MAP

TECH CONTRACTORS
11886 STAPLETON DR.
FALCON, CO 80831
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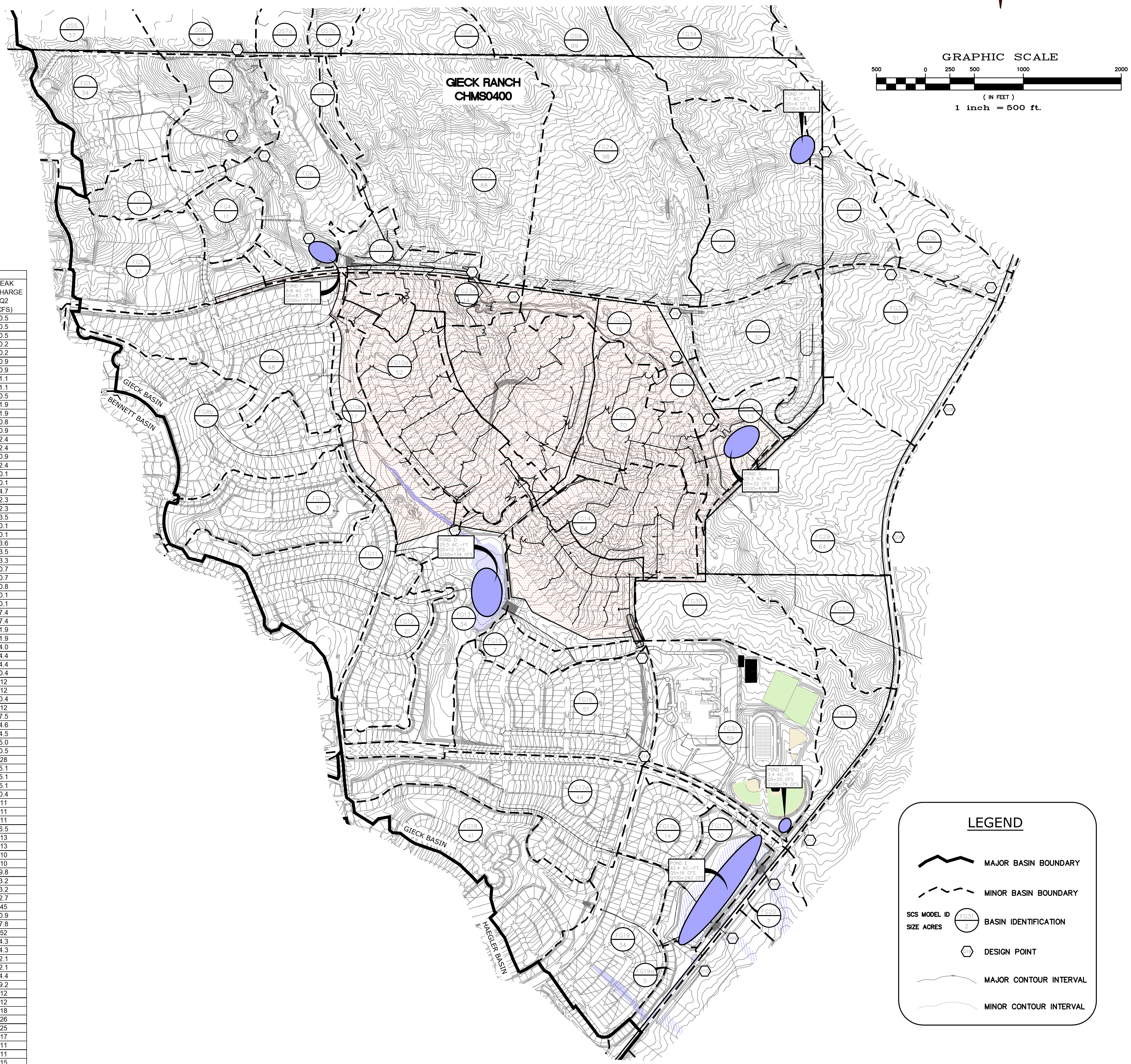
FEB 2020

FIGURE 5

ROLLING HILL RANCH PUD
MERIDIAN RANCH



FUTURE MDDP (Full Spectrum)							
	DRAINAGE AREA (SQ. MI.)	PEAK DISCHARGE Q100 (CFS)	PEAK DISCHARGE Q50 (CFS)	PEAK DISCHARGE Q25 (CFS)	PEAK DISCHARGE Q10 (CFS)	PEAK DISCHARGE Q5 (CFS)	PEAK DISCHARGE Q2 (CFS)
OS06	0.1313	80	52	30	12	3.8	0.5
G1a	0.1313	80	52	30	12	3.8	0.5
G1a-G2	0.1313	79	52	30	11	3.6	0.5
OS05	0.0578	39	26	15	5.6	1.8	0.2
OS05-G1	0.0578	39	25	15	5.5	1.7	0.2
FG01	0.0538	31	22	14	7.0	3.4	0.9
FG01-G1	0.0538	31	22	14	6.9	3.4	0.9
G1	0.1116	61	41	25	11	4.9	1.1
G1-G2	0.1116	61	41	25	11	4.8	1.1
FG02	0.0391	32	22	14	6.4	2.7	0.5
G2	0.2820	167	112	67	27	10	1.9
G2-G3	0.2820	163	109	66	27	10	1.9
FG03	0.0203	24	17	12	5.9	0.8	0.8
FG04	0.0172	22	16	11	5.8	3.1	0.9
G3	0.3195	185	123	74	31	11	2.4
G3-POND F	0.3195	183	121	74	31	11	2.4
FG06	0.0608	49	34	22	10	4.6	0.9
FG05	0.0580	45	33	23	12	6.7	2.4
OS07a	0.0170	14	9.2	5.7	2.5	0.9	0.1
OS07a-POND F	0.0170	13	9.0	5.7	2.4	0.9	0.1
POND F IN	0.4553	286	194	120	52	22	4.7
POND F	0.4553	177	121	61	17	8.1	2.3
POND F-G7	0.4553	177	120	60	17	8.1	2.3
FG21b	0.0170	25	20	15	9.6	6.5	3.5
FG21a	0.0072	7.2	5.0	3.2	1.4	0.5	0.1
FG21a-G7	0.0072	6.8	4.9	2.7	1.4	0.5	0.1
G7	0.4795	186	126	64	18	8.8	3.6
G7-G8	0.4795	185	126	64	18	8.8	3.5
FG22	0.1380	102	73	47	24	12	3.3
OS08	0.0406	35	25	16	7.7	3.4	0.7
OS08-G8	0.0406	34	24	15	7.5	3.4	0.7
FG23a	0.0216	21	15	10	5.2	2.7	0.8
OS07b	0.0156	15	10	6.2	2.6	1.0	0.1
OS07b-G7	0.0156	14	10	6.0	2.4	0.9	0.1
G8	0.6953	291	186	95	47	24	7.4
G8-G10	0.6953	288	186	94	46	24	7.4
OS09	0.1527	90	62	39	18	8.2	1.9
OS09-G10	0.1527	88	62	39	18	8.2	1.9
FG24	0.1373	105	76	50	26	13	4.0
G9	0.2900	180	125	81	38	17	4.4
G9-G10	0.2900	178	125	79	37	17	4.4
FG23b	0.0286	23	16	10	4.6	2.0	0.4
G10	1.0139	478	307	174	80	39	12
G10-G11	1.0139	474	305	173	80	38	12
FG23c	0.0122	12	8.7	5.7	3.0	1.5	0.4
G11	1.0261	479	308	176	81	39	12
FG25	0.1086	85	64	46	27	17	7.5
FG26	0.0863	78	58	40	22	12	4.6
FG26-POND G	0.0863	77	57	39	22	12	4.5
FG27	0.0500	52	40	29	17	11	5.0
FG28	0.0245	18	13	8.5	4.1	2.0	0.5
POND G IN	1.2955	684	454	287	145	78	28
POND G	1.2955	478	333	170	56	22	5.1
G12	1.2955	478	333	170	56	22	5.1
G12-G06	1.2955	478	332	170	56	22	5.1
FG29	0.0997	60	39	23	8.7	2.8	0.4
FG32	0.0402	72	57	44	29	20	11
FG32-G06	0.0402	69	54	41	27	18	11
G06	1.4354	506	352	181	61	24	11
FG10A	0.0806	81	61	43	25	15	6.5
FG08A	0.0750	116	90	66	41	27	13
FG08A-G05	0.0750	110	86	64	41	27	13
FG08B	0.0630	86	67	49	31	20	10
FG08B-G05	0.0630	84	65	48	29	19	10
FG11	0.0625	75	59	44	28	19	9.8
FG09	0.0484	48	36	25	14	8.3	3.2
FG09-G05	0.0484	48	36	25	14	8.0	3.2
FG10B	0.0416	42	31	22	12	7.0	2.7
G05	0.3711	433	330	239	145	93	45
FG13	0.0534	34	24	15	7.5	3.6	0.9
FG12	0.0328	50	40	30	20	14	7.8
POND D IN	0.4573	509	387	280	168	107	52
POND D	0.4573	134	91	50	19	12	4.3
POND D-G17	0.4573	134	91	50	19	12	4.3
FG15	0.0103	15	12	9.0	5.8	3.9	2.1
FG15-G17A	0.0103	15	12	8.9	5.8	3.9	2.1
G17A	0.4676	137	93	51	19	12	4.4
FG14	0.1000	98	74	53	32	20	9.2
G17	0.5676	196	132	75	43	25	12
G17-G18	0.5676	196	131	75	43	25	12
FG16	0.0791	133	104	78	50	34	18
G18	0.6467	240	178	128	79	51	26
G18-POND E	0.6467	240	176	126	78	50	25
FG31	0.0922	116	92	69	45	31	17
FG30	0.0389	73	57	44	29	20	11
FG30-PONDHS	0.0389	70	56	42	27	18	11
POND HS	0.1311	153	106	53	36	26	15
FG17a	0.0694	101	78	57	35	23	12
FG17a-POND E	0.0694	99	76	56	35	23	12
FG18	0.0644	56	42	30	18	11	4.7
FG18-POND E	0.0644	56	42	30	17	11	4.6
FG19	0.0527	84	66	50	33	23	13
FG17c	0.0313	31	22	14	6.5	2.8	0.5
FG17b	0.0214	39	31	24	16	11	6.1
POND E IN	1.0170	610	432	318	197	126	64
POND E	1.0170	242	153	80	30	16	6.6
H08	1.0170	205	137	72	24	12	4.1
H09	0.0000	37	16	8.3	5.9	4.1	2.4
FG34	0.0600	34	23	13	5.5	2.0	0.3
G14	0.0600	34	23	13	5.5	2.0	0.3
G14-G15	0.0600	34	22	13	5.4	2.0	0.3
FG35	0.0344	20	13	8.3	3.5	1.5	0.3
G15	0.0944	53	36	21	8.7	3.3	0.6
G15-G08	0.0944	52	35	21	8.7	3.3	0.6
FG37	0.0797	41	27	16	6.0	2.0	0.3
FG36	0.0281	14	9.4	5.5	2.1	0.7	0.1
FG36-G08	0.0281	14	9.3	5.4	2.1	0.7	0.1
G08	0.2022	106	69	41	16	5.8	1.0

*NOTE: PRELIMINARY STORAGE VOLUMES AND OUTFLOW QUANTITIES HAVE BEEN PROVIDED FOR EACH OF THE FUTURE DETENTION FACILITIES LOCATED WITHIN THE DEVELOPMENT. THE ACTUAL STORAGE VOLUMES AND DISCHARGE RATES WILL BE DETERMINED UPON A COMPLETE ANALYSIS FOR EACH DETENTION FACILITY PRIOR TO CONSTRUCTION. THE VALUES GIVEN FOR DISCHARGE AND VOLUME ARE ESTIMATES FOR PLANNING PURPOSES ONLY.

DEVELOPED CONDITIONS - SCS MAP

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TELEPHONE: 719.495.7444

FEB 2020

FIGURE 5

NOTE:

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

BENCH MARK:

INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)

ELEVATION = 6874.00

GRAPHIC SCALE

100 0 50 100 200 400

(IN FEET)

1 inch = 100 ft.

N

G01 — BASIN DESIGNATION

SUB-WATERSHED DESIGNATION

MINOR/MAJOR STORM COEFFICIENT

BASIN AREA IN ACRES

61 — DESIGN POINT DESIGNATION

MAJOR BASIN BOUNDARY

SUB-BASIN BOUNDARY

6130 — EXISTING CONTOUR

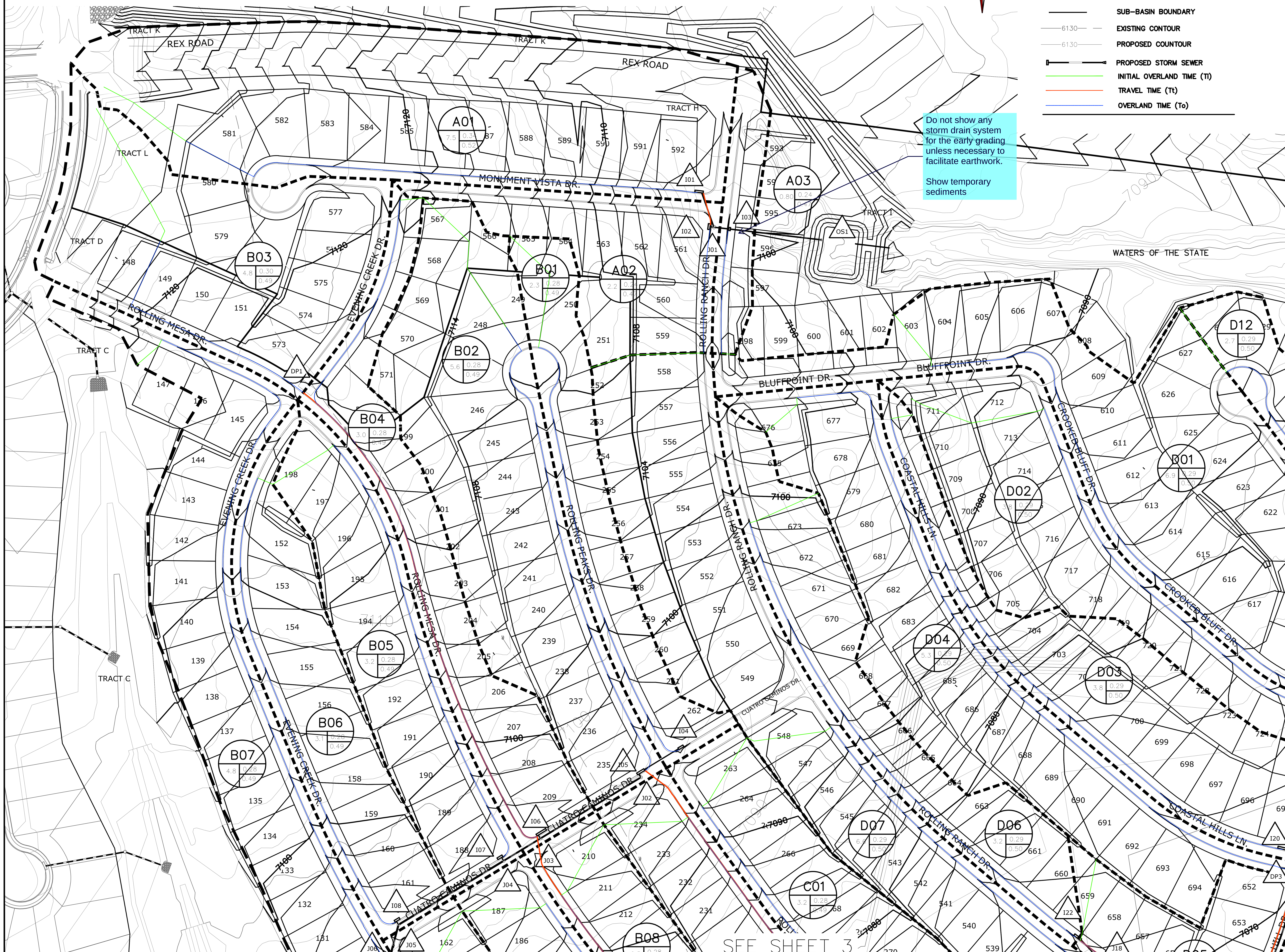
6130 — PROPOSED COUNTOUR

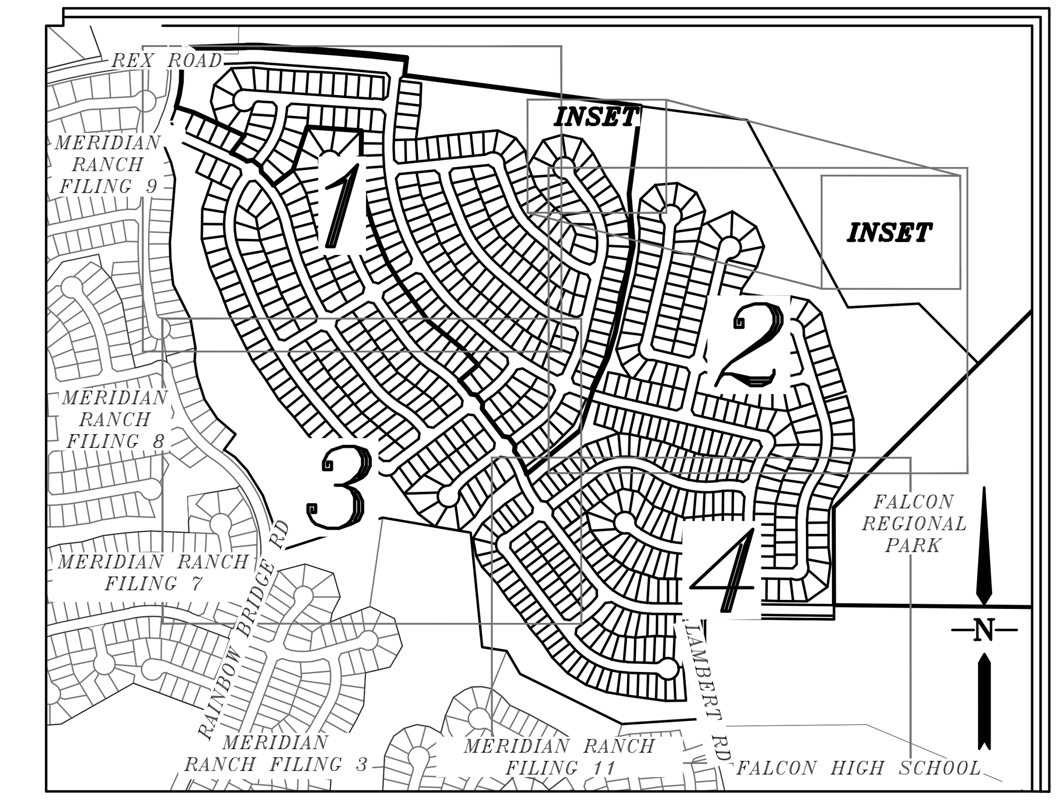
PROPOSED STORM SEWER

INITIAL OVERLAND TIME (Ti)

TRAVEL TIME (Tt)

OVERLAND TIME (To)





INDEX MAP

N.T.S.

SEE INSET SHEET 2

SEE SHEET 2

DP	BASIN	AREA (AC)	Q(5) (CFS)	Q(100) (CFS)	INLET	Q(5) (CFS)	Q(100) (CFS)	PIPE
OS5	OS1	4.40	1.3	9	PR Type C	1.2	8.4	24 " RCP
I01	A01	7.52	8.6	22	PR 15 ' FORCED SUMP	8.6	14	18 " RCP
I02	A02	2.20	2.0	14	PR 10 ' SUMP	2.0	14	18 " RCP
J01						10.5	28	24 " RCP
I03	A03	0.79	1.0	3.1	PR 5 ' SUMP	11.3	30	24 " RCP
I04	B01	2.29	2.2	6.4	PR 10 ' FORCED SUMP	2.2	6.4	18 " RCP
I05	B02	5.63	5.2	15	PR 15 ' FORCED SUMP	7.3	20	24 " RCP
J02						7.3	20	24 " RCP
DP1	B03	4.81	5.2	14				
I06	B04	3.03	7.1	20	PR 20 ' FORCED SUMP	7.1	17	18 " RCP
J03						13.9	36	30 " RCP
I07	B05	3.22	3.1	9.1	PR 10 ' FORCED SUMP	3.1	9.1	18 " RCP
J04						16.6	44	36 " RCP
I08	B06	3.13	3.3	9.9	PR 10 ' FORCED SUMP	3.3	9.9	18 " RCP
J05						19.0	51	36 " RCP
I09	B07	4.76	4.3	13	PR 20 ' FLOW-BY	3.7	9.2	18 " RCP
J06						22.4	59	36 " RCP
J07						22.0	58	36 " RCP
I10	B08	2.54	2.5	8.3	PR 10 ' FORCED SUMP	2.5	8.3	18 " RCP
J08						2.5	8.2	18 " RCP
I11	B09	2.74	2.6	7.7	PR 10 ' SUMP	2.6	7.7	18 " RCP
I12	B10 & B11	6.38	8.5	27	PR 20 ' SUMP	5.4	18	18 " RCP
J09						10.1	32	24 " RCP
J10						30.6	87	42 " RCP
I13	C01	3.15	3.1	9.0	PR 10 ' FORCED SUMP	3.1	9.0	24 " RCP
I14	C02	3.54	3.4	10	PR 15 ' FORCED SUMP	3.4	10	24 " RCP
J11						6.0	18	24 " RCP
I15	C03	1.33	1.4	4.0	PR 5 ' FORCED SUMP	1.4	4.0	18 " RCP
I16	C04	3.10	3.2	9.4	PR 5 ' FORCED SUMP	3.2	6.3	18 " RCP
J12						9.5	26	30 " RCP
I17	C05	0.58	0.6	1.8	PR 5 ' SUMP	0.6	1.8	18 " RCP
I18	C06	1.03	1.0	6.0	PR 5 ' SUMP	1.0	6.0	18 " RCP
J13						10.8	32	36 " RCP
CB1	C07	0.88	0.9	2.5	PR Type C	0.9	2.5	18 " RCP
J14						11.4	34	36 " RCP
I19	D01	6.87	6.8	19	PR 15 ' FORCED SUMP	6.8	14	18 " RCP
J15						6.8	14	24 " RCP
DP2	D02	3.83	3.8	16				
I20	D03	3.84	7.3	21	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J16						13.8	30	30 " RCP
DP3	D04	5.30	5.0	17				
I21	D05	2.00	6.9	24	PR 15 ' FORCED SUMP	6.9	14	24 " RCP
J17						19.6	41	30 " RCP
I22	D06	3.20	3.2	9	PR 15 ' FLOW-BY	2.6	6.2	18 " RCP
I23	D07	6.59	6.9	20	PR 10 ' FORCED SUMP	6.9	9.9	18 " RCP
J18						9.1	16	24 " RCP
I24	D08	1.64	1.8	13	PR 10 ' FORCED SUMP	1.8	9.9	24 " RCP
J19						29.2	64	42 " RCP
CB2	D09	1.63	1.2	3.4	PR Type C	1.2	3.4	18 " RCP
I25	D10	0.81	0.9	5.4	PR 10 ' SUMP	2.0	8.4	18 " RCP
ES1	D11	4.16	2.4	6.9	" FES	2.4	6.9	18 " RCP
I26	D12	2.67	2.4	16	PR 20 ' SUMP	4.3	19	24 " RCP
J20						29.2	71	42 " RCP
I27	D13	1.79	2.2	5.8	PR 15 ' FLOW-BY	1.7	3.9	18 " RCP
J21						29.2	72	42 " RCP
I28	D14	6.45	6.3	18	PR 10 ' FORCED SUMP	6.3	9.9	18 " RCP
I29	D15	6.35	6.2	18	PR 10 ' FORCED SUMP	6.2	9.9	18 " RCP
J22						12.5	20	24 " RCP
J23						34.9	84	48 " RCP
I30	D16	4.02	4.2	17	PR 10 ' FORCED SUMP	4.2	9.9	18 " RCP
J24						37.4	90	48 " RCP
J25						37.4	90	48 " RCP
I31	D17	5.13	5.3	27	PR 15 ' SUMP	5.3	24	30 " RCP
J26						41.0	107	48 " RCP
I32	D18	3.13	3.0	10	PR 15 ' SUMP	43.0	114	54 " RCP
I33	E01	5.38	6.2	17	PR 20 ' FORCED SUMP	6.2	17	18 " RCP
J27						6.2	17	18 " RCP
I34	E02	6.48	7.3	19	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J28						13.7	35	30 " RCP
I35	E03	5.82	6.5	17	PR 15 ' FORCED SUMP	6.5	14	18 " RCP
J29						19.7	47	36 " RCP
I36	E04	3.14	3.9	13	PR 15 ' FORCED SUMP	3.9	13	18 " RCP
I37	E05	2.55	2.7	8.7	PR 15 ' FLOW-BY	2.3	6.0	18 " RCP
J30						23.9	61	36 " RCP
J31						23.8	61	36 " RCP
I38	E06	1.27	1.6	4.2	PR 5 ' FORCED SUMP	1.6	4.2	18 " RCP
J32						1.5	4.1	18 " RCP
I39	E07	2.05	2.5	6.7	PR 15 ' FLOW-BY	2.0	4.5	18 " RCP
J33						3.5	8.5	18 " RCP
I40	E08	4.17	4.8	13	PR 10 ' FORCED SUMP	4.8	9.9	18 " RCP
J34						8.0	18	24 " RCP
J35						8.0	18	24 " RCP
I41	E09	5.44	6.2	18	PR 15 ' FORCED SUMP	6.2	14	24 " RCP
J36						35.2	86	42 " RCP
I42	E10	6.98	7.0	19	PR 20 ' SUMP	7.0	19	24 " RCP
J37						41.1	102	48 " RCP
CB3	E11	13.04	6.3	18	PR Type C	6.3	18	18 " RCP
I43	E12	1.60	3.6	9.2	PR 20 ' FLOW-BY	3.2	7.1	18 " RCP
J38						52.5	132	54 " RCP
EI1	E13	6.02	8.2	19	EX 15 ' FORCED SUMP	8.2	13	18 " RCP

No.	Revisions	Date	Init.	Appr.	Date
-		-	-	-	-

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MERIDIAN RANCH

RATIONAL DRAINAGE MAP
PRELIMINARY DRAINAGE REPORT
ROLLING HILLS RANGE

Drawn by TAL
Checked by -
Date FEB 2020

Scale 1" = 100'

1 of 4

NOTE:

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BENCH MARK:

INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)

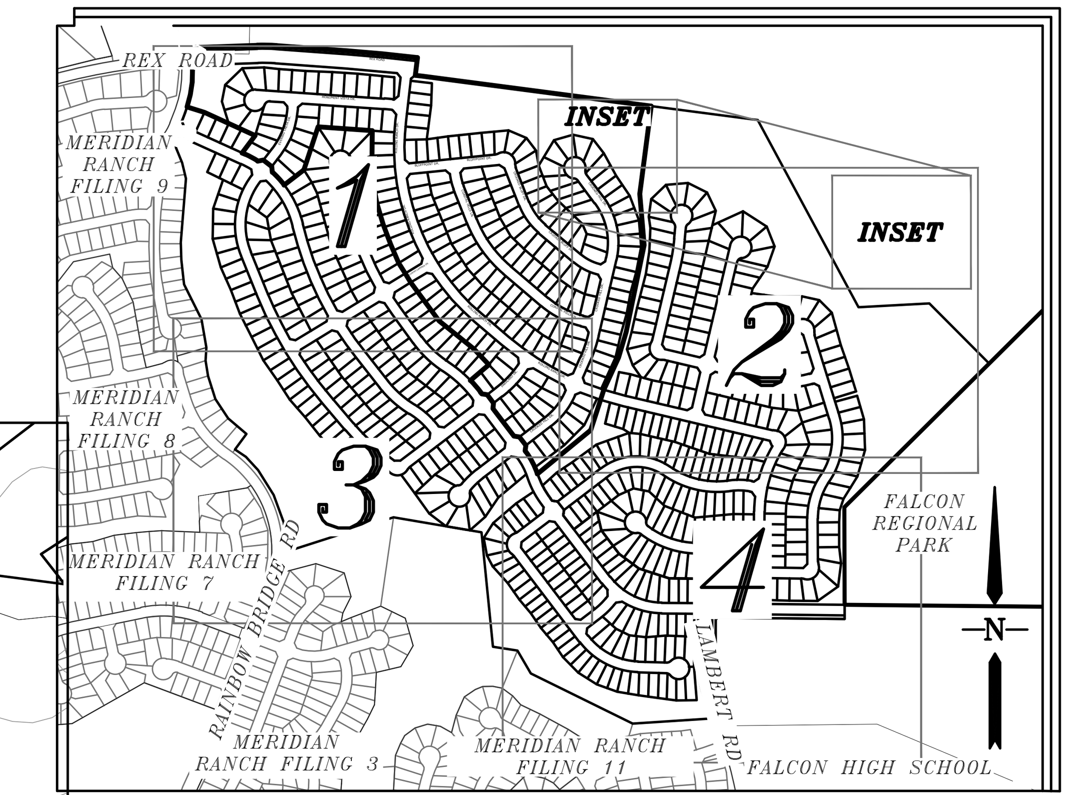
ELEVATION = 6874.00

GRAPHIC SCALE

100 0 50 100 200 400

(IN FEET)

1 inch = 100 ft.



INDEX MAP

N.T.S.

DP	BASIN	AREA (AC)	Q(5) (CFS)	Q(100) (CFS)	INLET	Q(5) (CFS)	Q(100) (CFS)	PIPE
OS5	OS1	4.40	1.3	9	PR Type C	1.2	8.4	24 " RCP
I01	A01	7.52	8.6	22	PR 15 ' FORCED SUMP	8.6	14	18 " RCP
I02	A02	2.20	2.0	14	PR 10 ' SUMP	2.0	14	18 " RCP
J01						10.5	28	24 " RCP
I03	A03	0.79	1.0	3.1	PR 5 ' SUMP	11.3	30	24 " RCP
I04	B01	2.29	2.2	6.4	PR 10 ' FORCED SUMP	2.2	6.4	18 " RCP
I05	B02	5.63	5.2	15	PR 15 ' FORCED SUMP	7.3	20	24 " RCP
J02						7.3	20	24 " RCP
DP1	B03	4.81	5.2	14				
I06	B04	3.03	7.1	20	PR 20 ' FORCED SUMP	7.1	17	18 " RCP
J03						13.9	36	30 " RCP
I07	B05	3.22	3.1	9.1	PR 10 ' FORCED SUMP	3.1	9.1	18 " RCP
J04						16.6	44	36 " RCP
I08	B06	3.13	3.3	9.9	PR 10 ' FORCED SUMP	3.3	9.9	18 " RCP
J05						19.0	51	36 " RCP
I09	B07	4.76	4.3	13	PR 20 ' FLOW-BY	3.7	9.2	18 " RCP
J06						22.4	59	36 " RCP
J07						22.0	58	36 " RCP
I10	B08	2.54	2.5	8.3	PR 10 ' FORCED SUMP	2.5	8.3	18 " RCP
J08						2.5	8.2	18 " RCP
I11	B09	2.74	2.6	7.7	PR 10 ' SUMP	2.6	7.7	18 " RCP
I12	B10 & B11	6.38	8.5	27	PR 20 ' SUMP	5.4	18	18 " RCP
J09						10.1	32	24 " RCP
J10						30.6	87	42 " RCP
I13	C01	3.15	3.1	9.0	PR 10 ' FORCED SUMP	3.1	9.0	24 " RCP
I14	C02	3.54	3.4	10	PR 15 ' FORCED SUMP	3.4	10	24 " RCP
J11						6.0	18	24 " RCP
I15	C03	1.33	1.4	4.0	PR 5 ' FORCED SUMP	1.4	4.0	18 " RCP
I16	C04	3.10	3.2	9.4	PR 5 ' FORCED SUMP	3.2	6.3	18 " RCP
J12						9.5	28	30 " RCP
I17	C05	0.58	0.6	1.8	PR 5 ' SUMP	0.6	1.8	18 " RCP
I18	C06	1.03	1.0	6.0	PR 5 ' SUMP	1.0	6.0	18 " RCP
J13						10.8	32	36 " RCP
CB1	C07	0.88	0.9	2.5	PR Type C	0.9	2.5	18 " RCP
J14						11.4	34	36 " RCP
I19	D01	6.87	6.8	19	PR 15 ' FORCED SUMP	6.8	14	18 " RCP
J15						6.8	14	24 " RCP
DP2	D02	3.83	3.8	16				
I20	D03	3.84	7.3	21	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J16						13.8	30	30 " RCP
DP3	D04	5.30	5.0	17				
I21	D05	2.00	6.9	24	PR 15 ' FORCED SUMP	6.9	14	24 " RCP
J17						19.6	41	30 " RCP
I22	D06	3.20	3.2	9	PR 15 ' FLOW-BY	2.6	6.2	18 " RCP
I23	D07	6.59	6.9	20	PR 10 ' FORCED SUMP	6.9	9.9	18 " RCP
J18						9.1	16	24 " RCP
I24	D08	1.64	1.8	13	PR 10 ' FORCED SUMP	1.8	9.9	24 " RCP
J19						29.2	64	42 " RCP
CB2	D09	1.63	1.2	3.4	PR Type C	1.2	3.4	18 " RCP
I25	D10	0.81	0.9	5.4	PR 10 ' SUMP	2.0	8.4	18 " RCP
ES1	D11	4.16	2.4	6.9	" FES	2.4	6.9	18 " RCP
I26	D12	2.67	2.4	16	PR 20 ' SUMP	4.3	19	24 " RCP
J20						29.2	71	42 " RCP
I27	D13	1.79	2.2	5.8	PR 15 ' FLOW-BY	1.7	3.9	18 " RCP
J21						29.2	72	42 " RCP
I28	D14	6.45	6.3	18	PR 10 ' FORCED SUMP	6.3	9.9	18 " RCP
I29	D15	6.35	6.2	18	PR 10 ' FORCED SUMP	6.2	9.9	18 " RCP
J22						12.5	20	24 " RCP
J23						34.9	84	48 " RCP
I30	D16	4.02	4.2	17	PR 10 ' FORCED SUMP	4.2	9.9	18 " RCP
J24						37.4	90	48 " RCP
J25						37.4	90	48 " RCP
I31	D17	5.13	5.3	27	PR 15 ' SUMP	5.3	24	30 " RCP
J26						41.0	107	48 " RCP
I32	D18	3.13	3.0	10	PR 15 ' SUMP	43.0	114	54 " RCP
I33	E01	5.38	6.2	17	PR 20 ' FORCED SUMP	6.2	17	18 " RCP
J27						6.2	17	18 " RCP
I34	E02	6.48	7.3	19	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J28						13.7	35	30 " RCP
I35	E03	5.82	6.5	17	PR 15 ' FORCED SUMP	6.5	14	18 " RCP
J29						19.7	47	36 " RCP
I36	E04	3.14	3.9	13	PR 15 ' FORCED SUMP	3.9	13	18 " RCP
I37	E05	2.55	2.7	8.7	PR 15 ' FLOW-BY	2.3	6.0	18 " RCP
J30						23.9	61	36 " RCP
J31						23.8	61	36 " RCP
I38	E06	1.27	1.6	4.2	PR 5 ' FORCED SUMP	1.6	4.2	18 " RCP
J32						1.5	4.1	18 " RCP
I39	E07	2.05	2.5	6.7	PR 15 ' FLOW-BY	2.0	4.5	18 " RCP
J33						3.5	8.5	18 " RCP
I40	E08	4.17	4.8	13	PR 10 ' FORCED SUMP	4.8	9.9	18 " RCP
J34						8.0	18	24 " RCP
J35						8.0	18	24 " RCP
I41	E09	5.44	6.2	18	PR 15 ' FORCED SUMP	6.2	14	24 " RCP
J36						35.2	86	42 " RCP
I42	E10	6.98	7.0	19	PR 20 ' SUMP	7.0	19	24 " RCP
J37						41.1	102	48 " RCP
CB3	E11	13.04	6.3	18	PR Type C	6.3	18	18 " RCP
I43	E12	1.60	3.6	9.2	PR 20 ' FLOW-BY	3.2	7.1	18 " RCP
J38						52.5	132	54 " RCP
EI1	E13	6.02	8.2	19	EX 15 ' FORCED SUMP	8.2	13	18 " RCP

G01 — BASIN DESIGNATION

SUB-WATERSHED DESIGNATION

MINOR/MAJOR STORM COEFFICIENT

BASIN AREA IN ACRES

61 — DESIGN POINT DESIGNATION

MAJOR BASIN BOUNDARY

SUB-BASIN BOUNDARY

6130 — EXISTING CONTOUR

6130 — PROPOSED COUNTOUR

PROPOSED STORM SEWER

INITIAL OVERLAND TIME (Ti)

TRAVEL TIME (Tt)

OVERLAND TIME (To)

No.	Revisions	Date	Init.	Appr.	Date
-		-	-	-	

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MERIDIAN RANCH

RATIONAL DRAINAGE MAP
PRELIMINARY DRAINAGE REPORT
ROLLING HILLS RANGE

Drawn by TAL

Checked by -

Date FEB 2020

Scale 1" = 100'

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INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)

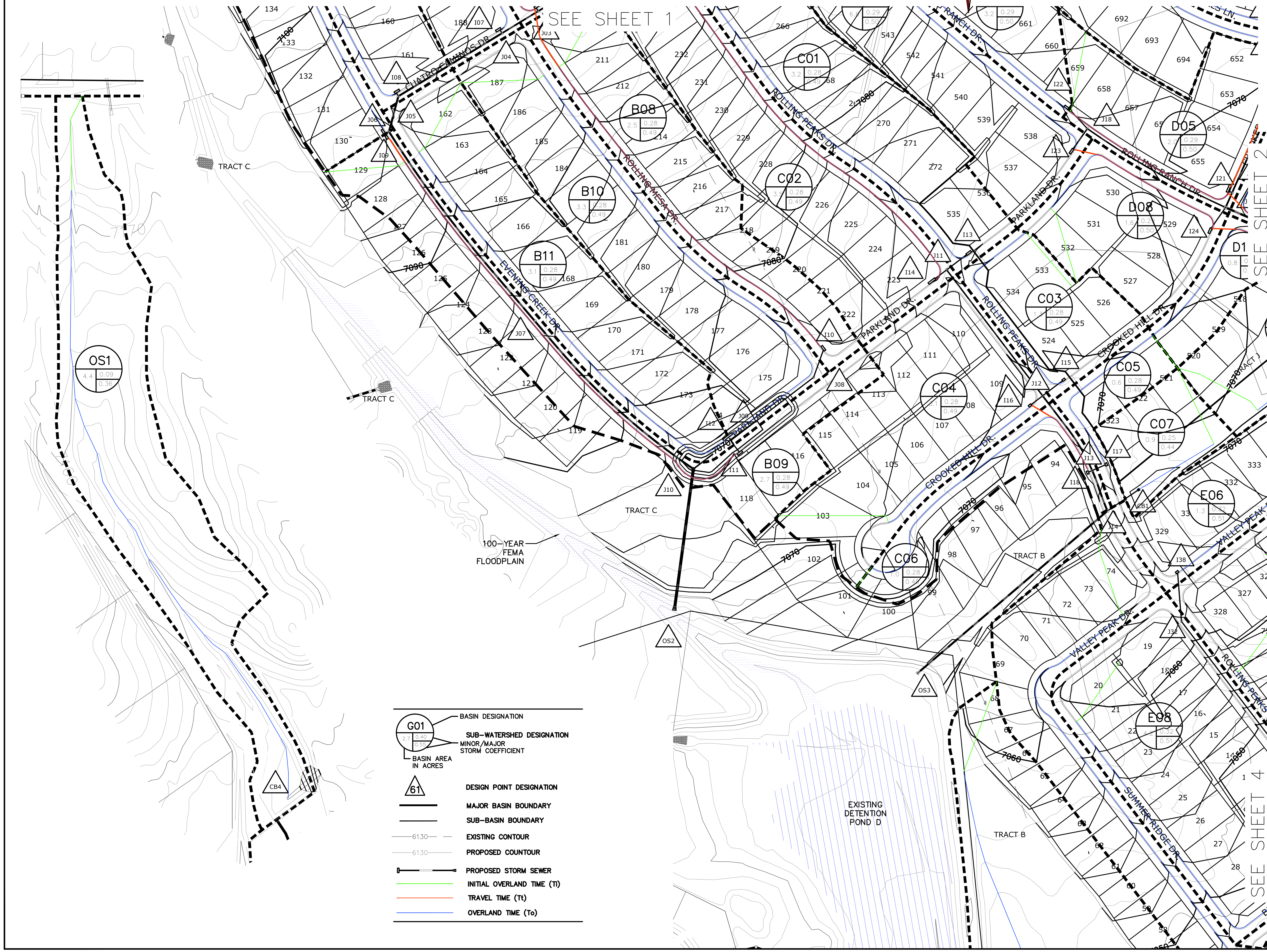
ELEVATION = 6874.00

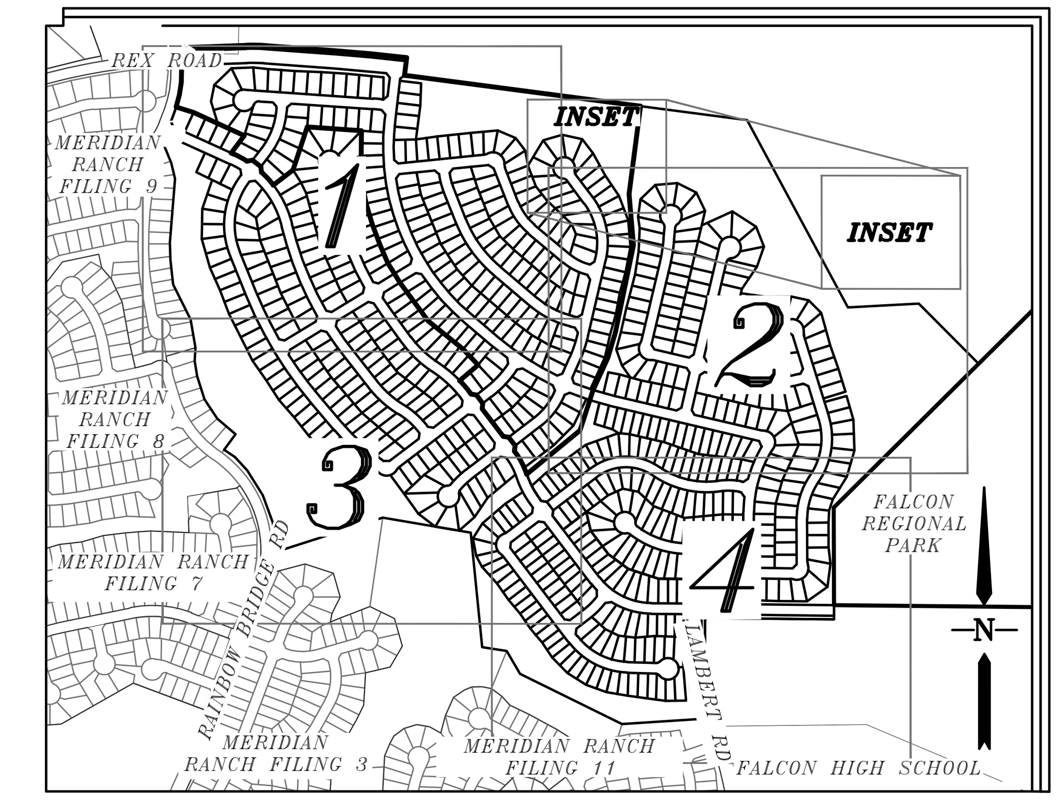
GRAPHIC SCALE

100 0 50 100 200 400

(IN FEET)

1 inch = 100 ft.





INDEX MAP

N.T.S.

DP	BASIN	AREA (AC)	Q(5) (CFS)	Q(100) (CFS)	INLET	Q(5) (CFS)	Q(100) (CFS)	PIPE
OS5	OS1	4.40	1.3	9	PR Type C	1.2	8.4	24" RCP
I01	A01	7.52	8.6	22	PR 15' FORCED SUMP	8.6	14	18" RCP
I02	A02	2.20	2.0	14	PR 10' SUMP	2.0	14	18" RCP
J01						10.5	28	24" RCP
I03	A03	0.79	1.0	3.1	PR 5' SUMP	11.3	30	24" RCP
I04	B01	2.29	2.2	6.4	PR 10' FORCED SUMP	2.2	6.4	18" RCP
I05	B02	5.63	5.2	15	PR 15' FORCED SUMP	7.3	20	24" RCP
J02						7.3	20	24" RCP
DP1	B03	4.81	5.2	14				
I06	B04	3.03	7.1	20	PR 20' FORCED SUMP	7.1	17	18" RCP
J03						13.9	36	30" RCP
I07	B05	3.22	3.1	9.1	PR 10' FORCED SUMP	3.1	9.1	18" RCP
J04						16.6	44	36" RCP
I08	B06	3.13	3.3	9.9	PR 10' FORCED SUMP	3.3	9.9	18" RCP
J05						19.0	51	36" RCP
I09	B07	4.76	4.3	13	PR 20' FLOW-BY	3.7	9.2	18" RCP
J06						22.4	59	36" RCP
J07						22.0	58	36" RCP
I10	B08	2.54	2.5	8.3	PR 10' FORCED SUMP	2.5	8.3	18" RCP
J08						2.5	8.2	18" RCP
I11	B09	2.74	2.6	7.7	PR 10' SUMP	2.6	7.7	18" RCP
I12	B10 & B11	6.38	8.5	27	PR 20' SUMP	5.4	18	18" RCP
J09						10.1	32	24" RCP
J10						30.6	87	42" RCP
I13	C01	3.15	3.1	9.0	PR 10' FORCED SUMP	3.1	9.0	24" RCP
I14	C02	3.54	3.4	10	PR 15' FORCED SUMP	3.4	10	24" RCP
J11						6.0	18	24" RCP
I15	C03	1.33	1.4	4.0	PR 5' FORCED SUMP	1.4	4.0	18" RCP
I16	C04	3.10	3.2	9.4	PR 5' FORCED SUMP	3.2	6.3	18" RCP
J12						9.5	26	30" RCP
I17	C05	0.58	0.6	1.8	PR 5' SUMP	0.6	1.8	18" RCP
I18	C06	1.03	1.0	6.0	PR 5' SUMP	1.0	6.0	18" RCP
J13						10.8	32	36" RCP
CB1	C07	0.88	0.9	2.5	PR Type C	0.9	2.5	18" RCP
J14						11.4	34	36" RCP
I19	D01	6.87	6.8	19	PR 15' FORCED SUMP	6.8	14	18" RCP
J15						6.8	14	24" RCP
DP2	D02	3.83	3.8	16				
I20	D03	3.84	7.3	21	PR 20' FORCED SUMP	7.3	17	18" RCP
J16						13.8	30	30" RCP
DP3	D04	5.30	5.0	17				
I21	D05	2.00	6.9	24	PR 15' FORCED SUMP	6.9	14	24" RCP
J17						19.6	41	30" RCP
I22	D06	3.20	3.2	9	PR 15' FLOW-BY	2.6	6.2	18" RCP
I23	D07	6.59	6.9	20	PR 10' FORCED SUMP	6.9	9.9	18" RCP
J18						9.1	16	24" RCP
I24	D08	1.64	1.8	13	PR 10' FORCED SUMP	1.8	9.9	24" RCP
J19						29.2	64	42" RCP
CB2	D09	1.63	1.2	3.4	PR Type C	1.2	3.4	18" RCP
I25	D10	0.81	0.9	5.4	PR 10' SUMP	2.0	8.4	18" RCP
ES1	D11	4.16	2.4	6.9	" FES	2.4	6.9	18" RCP
I26	D12	2.67	2.4	16	PR 20' SUMP	4.3	19	24" RCP
J20						29.2	71	42" RCP
I27	D13	1.79	2.2	5.8	PR 15' FLOW-BY	1.7	3.9	18" RCP
J21						29.2	72	42" RCP
I28	D14	6.45	6.3	18	PR 10' FORCED SUMP	6.3	9.9	18" RCP
I29	D15	6.35	6.2	18	PR 10' FORCED SUMP	6.2	9.9	18" RCP
J22						12.5	20	24" RCP
J23						34.9	84	48" RCP
I30	D16	4.02	4.2	17	PR 10' FORCED SUMP	4.2	9.9	18" RCP
J24						37.4	90	48" RCP
J25						37.4	90	48" RCP
I31	D17	5.13	5.3	27	PR 15' SUMP	5.3	24	30" RCP
J26						41.0	107	48" RCP
I32	D18	3.13	3.0	10	PR 15' SUMP	43.0	114	54" RCP
I33	E01	5.38	6.2	17	PR 20' FORCED SUMP	6.2	17	18" RCP
J27						6.2	17	18" RCP
I34	E02	6.48	7.3	19	PR 20' FORCED SUMP	7.3	17	18" RCP
J28						13.7	35	30" RCP
I35	E03	5.82	6.5	17	PR 15' FORCED SUMP	6.5	14	18" RCP
J29						19.7	47	36" RCP
I36	E04	3.14	3.9	13	PR 15' FORCED SUMP	3.9	13	18" RCP
I37	E05	2.55	2.7	8.7	PR 15' FLOW-BY	2.3	6.0	18" RCP
J30						23.9	61	36" RCP
J31						23.8	61	36" RCP
I38	E06	1.27	1.6	4.2	PR 5' FORCED SUMP	1.6	4.2	18" RCP
J32						1.5	4.1	18" RCP
I39	E07	2.05	2.5	6.7	PR 15' FLOW-BY	2.0	4.5	18" RCP
J33						3.5	8.5	18" RCP
I40	E08	4.17	4.8	13	PR 10' FORCED SUMP	4.8	9.9	18" RCP
J34						8.0	18	24" RCP
J35						8.0	18	24" RCP
I41	E09	5.44	6.2	18	PR 15' FORCED SUMP	6.2	14	24" RCP
J36						35.2	86	42" RCP
I42	E10	6.98	7.0	19	PR 20' SUMP	7.0	19	24" RCP
J37						41.1	102	48" RCP
CB3	E11	13.04	6.3	18	PR Type C	6.3	18	18" RCP
I43	E12	1.60	3.6	9.2	PR 20' FLOW-BY	3.2	7.1	18" RCP
J38						52.5	132	54" RCP
EI1	E13	6.02	8.2	19	EX 15' FORCED SUMP	8.2	13	18" RCP

No.	Revisions	Date	Init.	Appr.	Date
-		-	-	-	-

TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
TELEPHONE: 719.495.7444
FAX: 719.495.3349

MERIDIAN RANCH

RATIONAL DRAINAGE MAP
PRELIMINARY DRAINAGE REPORT
ROLLING HILLS RANGE

Drawn by TAL
Checked by -
Date FEB 2020
Scale 1" = 100'
3 of 4

NOTE:

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

BENCH MARK:

INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)

ELEVATION = 6874.00

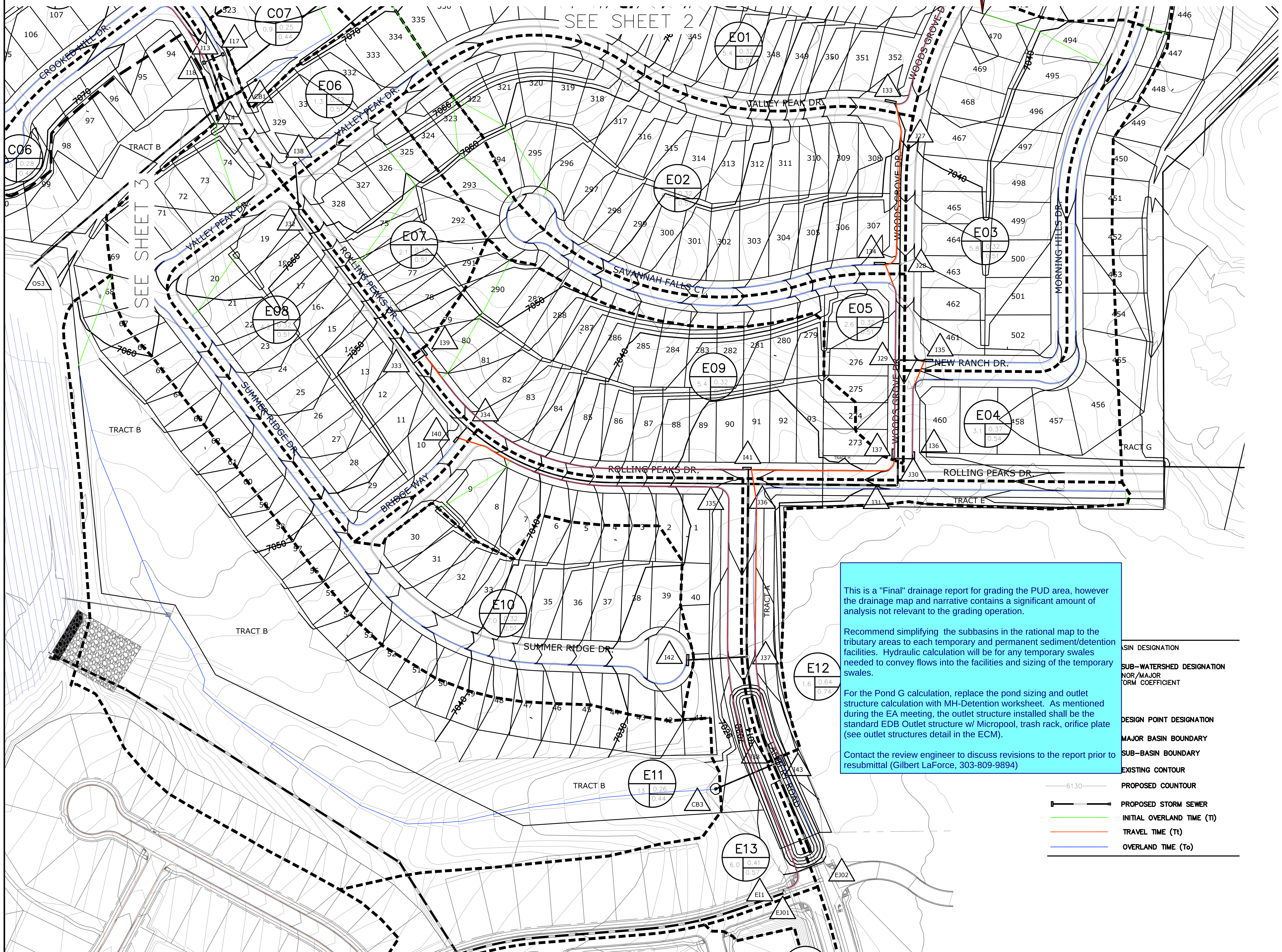
GRAPHIC SCALE

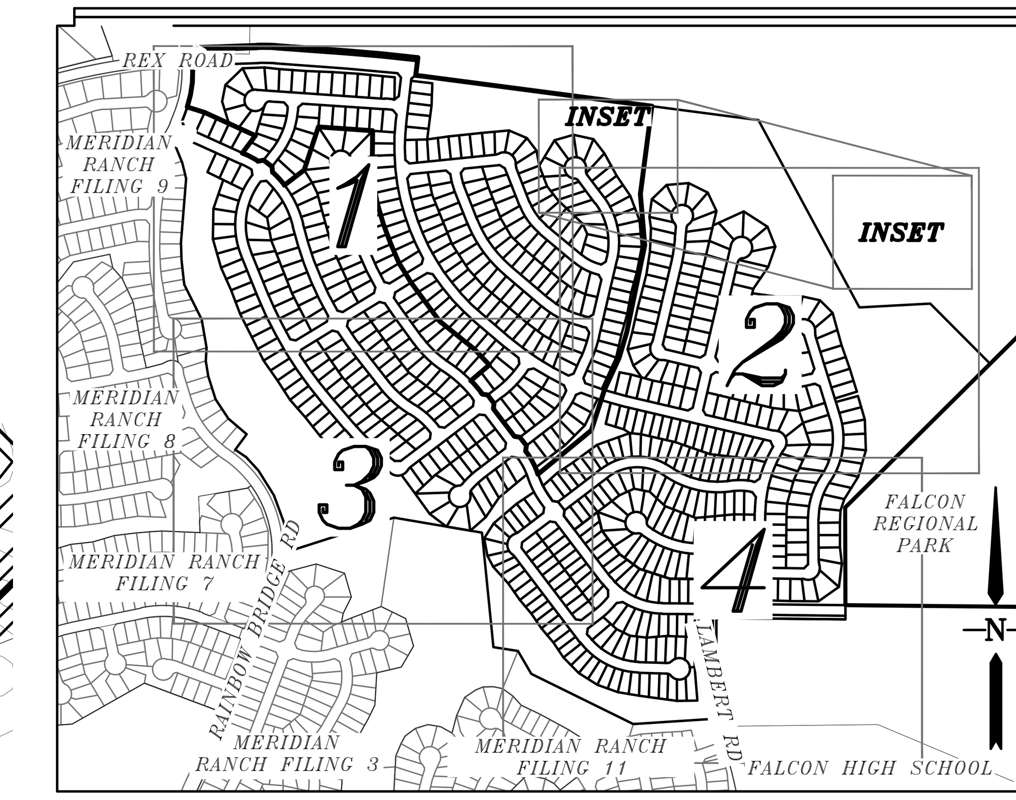
100 0 50 100 200 400

(IN FEET)

1 inch = 100 ft.

N





INDEX MAP

N.T.S.

This is a "Final" drainage report for grading the PUD area, however the drainage map and narrative contains a significant amount of analysis not relevant to the grading operation.

Recommend simplifying the subbasins in the rational map to the tributary areas to each temporary and permanent sediment/detention facilities. Hydraulic calculation will be for any temporary swales needed to convey flows into the facilities and sizing of the temporary swales.

For the Pond G calculation, replace the pond sizing and outlet structure calculation with MH-Detention worksheet. As mentioned during the EA meeting, the outlet structure installed shall be the standard EDB Outlet structure w/ Micropool, trash rack, orifice plate (see outlet structures detail in the ECM).

Contact the review engineer to discuss revisions to the report prior to resubmittal (Gilbert LaForce, 303-809-9894)

SIN DESIGNATION

SUB-WATERSHED DESIGNATION
NOR/MAJOR
ORM COEFFICIENT

DESIGN POINT DESIGNATION

MAJOR BASIN BOUNDARY

SUB-BASIN BOUNDARY

EXISTING CONTOUR

6130 PROPOSED CONTOUR

PROPOSED STORM SEWER

INITIAL OVERLAND TIME (Ti)

TRAVEL TIME (Tt)

OVERLAND TIME (To)

DP	BASIN	AREA (AC)	Q(5) (CFS)	Q(100) (CFS)	INLET	Q(5) (CFS)	Q(100) (CFS)	PIPE
OS5	OS1	4.40	1.3	9	PR Type C	1.2	8.4	24 " RCP
I01	A01	7.52	8.6	22	PR 15 ' FORCED SUMP	8.6	14	18 " RCP
I02	A02	2.20	2.0	14	PR 10 ' SUMP	2.0	14	18 " RCP
J01						10.5	28	24 " RCP
I03	A03	0.79	1.0	3.1	PR 5 ' SUMP	11.3	30	24 " RCP
I04	B01	2.29	2.2	6.4	PR 10 ' FORCED SUMP	2.2	6.4	18 " RCP
I05	B02	5.63	5.2	15	PR 15 ' FORCED SUMP	7.3	20	24 " RCP
J02						7.3	20	24 " RCP
DP1	B03	4.81	5.2	14				
I06	B04	3.03	7.1	20	PR 20 ' FORCED SUMP	7.1	17	18 " RCP
J03						13.9	36	30 " RCP
I07	B05	3.22	3.1	9.1	PR 10 ' FORCED SUMP	3.1	9.1	18 " RCP
J04						16.6	44	36 " RCP
I08	B06	3.13	3.3	9.9	PR 10 ' FORCED SUMP	3.3	9.9	18 " RCP
J05						19.0	51	36 " RCP
I09	B07	4.76	4.3	13	PR 20 ' FLOW-BY	3.7	9.2	18 " RCP
J06						22.4	59	36 " RCP
J07						22.0	58	36 " RCP
I10	B08	2.54	2.5	8.3	PR 10 ' FORCED SUMP	2.5	8.3	18 " RCP
J08						2.5	8.2	18 " RCP
I11	B09	2.74	2.6	7.7	PR 10 ' SUMP	2.6	7.7	18 " RCP
I12	B10 & B11	6.38	8.5	27	PR 20 ' SUMP	5.4	18	18 " RCP
J09						10.1	32	24 " RCP
J10						30.6	87	42 " RCP
I13	C01	3.15	3.1	9.0	PR 10 ' FORCED SUMP	3.1	9.0	24 " RCP
I14	C02	3.54	3.4	10	PR 15 ' FORCED SUMP	3.4	10	24 " RCP
J11						6.0	18	24 " RCP
I15	C03	1.33	1.4	4.0	PR 5 ' FORCED SUMP	1.4	4.0	18 " RCP
I16	C04	3.10	3.2	9.4	PR 5 ' FORCED SUMP	3.2	6.3	18 " RCP
J12						9.5	26	30 " RCP
I17	C05	0.58	0.6	1.8	PR 5 ' SUMP	0.6	1.8	18 " RCP
I18	C06	1.03	1.0	6.0	PR 5 ' SUMP	1.0	6.0	18 " RCP
J13						10.8	32	36 " RCP
CB1	C07	0.88	0.9	2.5	PR Type C	0.9	2.5	18 " RCP
J14						11.4	34	36 " RCP
I19	D01	6.87	6.8	19	PR 15 ' FORCED SUMP	6.8	14	18 " RCP
J15						6.8	14	24 " RCP
DP2	D02	3.83	3.8	16				
I20	D03	3.84	7.3	21	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J16						13.8	30	30 " RCP
DP3	D04	5.30	5.0	17				
I21	D05	2.00	6.9	24	PR 15 ' FORCED SUMP	6.9	14	24 " RCP
J17						19.6	41	30 " RCP
I22	D06	3.20	3.2	9	PR 15 ' FLOW-BY	2.6	6.2	18 " RCP
I23	D07	6.59	6.9	20	PR 10 ' FORCED SUMP	6.9	9.9	18 " RCP
J18						9.1	16	24 " RCP
I24	D08	1.64	1.8	13	PR 10 ' FORCED SUMP	1.8	9.9	24 " RCP
J19						29.2	64	42 " RCP
CB2	D09	1.63	1.2	3.4	PR Type C	1.2	3.4	18 " RCP
I25	D10	0.81	0.9	5.4	PR 10 ' SUMP	2.0	8.4	18 " RCP
ES1	D11	4.16	2.4	6.9	" FES	2.4	6.9	18 " RCP
I26	D12	2.67	2.4	16	PR 20 ' SUMP	4.3	19	24 " RCP
J20						29.2	71	42 " RCP
I27	D13	1.79	2.2	5.8	PR 15 ' FLOW-BY	1.7	3.9	18 " RCP
J21						29.2	72	42 " RCP
I28	D14	6.45	6.3	18	PR 10 ' FORCED SUMP	6.3	9.9	18 " RCP
I29	D15	6.35	6.2	18	PR 10 ' FORCED SUMP	6.2	9.9	18 " RCP
J22						12.5	20	24 " RCP
J23						34.9	84	48 " RCP
I30	D16	4.02	4.2	17	PR 10 ' FORCED SUMP	4.2	9.9	18 " RCP
J24						37.4	90	48 " RCP
J25						37.4	90	48 " RCP
I31	D17	5.13	5.3	27	PR 15 ' SUMP	5.3	24	30 " RCP
J26						41.0	107	48 " RCP
I32	D18	3.13	3.0	10	PR 15 ' SUMP	43.0	114	54 " RCP
I33	E01	5.38	6.2	17	PR 20 ' FORCED SUMP	6.2	17	18 " RCP
J27						6.2	17	18 " RCP
I34	E02	6.48	7.3	19	PR 20 ' FORCED SUMP	7.3	17	18 " RCP
J28						13.7	35	30 " RCP
I35	E03	5.82	6.5	17	PR 15 ' FORCED SUMP	6.5	14	18 " RCP
J29						19.7	47	36 " RCP
I36	E04	3.14	3.9	13	PR 15 ' FORCED SUMP	3.9	13	18 " RCP
I37	E05	2.55	2.7	8.7	PR 15 ' FLOW-BY	2.3	6.0	18 " RCP
J30						23.9	61	36 " RCP
J31						23.8	61	36 " RCP
I38	E06	1.27	1.6	4.2	PR 5 ' FORCED SUMP	1.6	4.2	18 " RCP
J32						1.5	4.1	18 " RCP
I39	E07	2.05	2.5	6.7	PR 15 ' FLOW-BY	2.0	4.5	18 " RCP
J33						3.5	8.5	18 " RCP
I40	E08	4.17	4.8	13	PR 10 ' FORCED SUMP	4.8	9.9	18 " RCP
J34						8.0	18	24 " RCP
J35						8.0	18	24 " RCP
I41	E09	5.44	6.2	18	PR 15 ' FORCED SUMP	6.2	14	24 " RCP
J36						35.2	86	42 " RCP
I42	E10	6.98	7.0	19	PR 20 ' SUMP	7.0	19	24 " RCP
J37						41.1	102	48 " RCP
CB3	E11	13.04	6.3	18	PR Type C	6.3	18	18 " RCP
I43	E12	1.60	3.6	9.2	PR 20 ' FLOW-BY	3.2	7.1	18 " RCP
J38						52.5	132	54 " RCP
EI1	E13	6.02	8.2	19	EX 15 ' FORCED SUMP	8.2	13	18 " RCP

No.	Revisions	Date	Init.	Appr.	Date

TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
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MERIDIAN RANCH

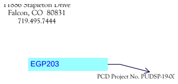
RATIONAL DRAINAGE MAP
PRELIMINARY DRAINAGE REPORT
ROLLING HILLS RANGE

Drawn by TAL
Checked by -
Date FEB 2020

Scale 1" = 100'

FDR_V1.pdf Markup Summary

dsdlaforce (18)



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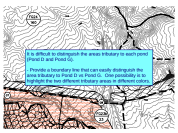
EGP203



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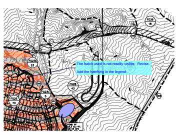
Page Label: 188
Author: dsdlaforce
Date: 3/18/2020 3:19:19 PM
Status:
Color:
Layer:
Space:

It is difficult to distinguish the areas tributary to each pond (Pond D and Pond G).

- Provide a boundary line that can easily distinguish the area tributary to Pond D vs Pond G. One possibility is to highlight the two different tributary areas in different colors.



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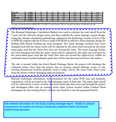
The hatch used is not readily visible. Revise.

Add the hatching in the legend



Subject: Cloud+
Page Label: 19
Author: dsdlaforce
Date: 3/18/2020 3:30:06 PM
Status:
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Layer:
Space:

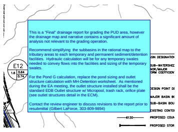
Not relevant information for the Early Grading drainage report. Modify to analyze conveyance via swales and sizing of temporary sediment basins during the earthmoving operation

---

Subject: Callout
Page Label: 190
Author: dsdlaforce
Date: 3/18/2020 3:30:52 PM
Status:
Color:
Layer:
Space:

Do not show any storm drain system for the early grading unless necessary to facilitate earthwork.

Show temporary sediments



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Page Label: 193
Author: dsdlaforce
Date: 3/18/2020 4:09:11 PM
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Color:
Layer:
Space:

This is a "Final" drainage report for grading the PUD area, however the drainage map and narrative contains a significant amount of analysis not relevant to the grading operation.

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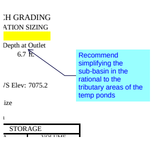
For the Pond G calculation, replace the pond sizing and outlet structure calculation with MH-Detention worksheet. As mentioned during the EA meeting, the outlet structure installed shall be the standard EDB Outlet structure w/ Micropool, trash rack, orifice plate (see outlet structures detail in the ECM).

Contact the review engineer to discuss revisions to the report prior to resubmittal (Gilbert LaForce, 303-809-9894)



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Author: dsdlaforce
Date: 3/19/2020 2:47:55 PM
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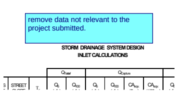
Space:

Recommend simplifying the sub-basin in the rational to the tributary areas of the temp ponds

---



Subject: Text Box
Page Label: 54
Author: dsdlaforce
Date: 3/19/2020 3:23:02 PM
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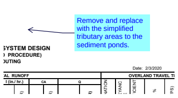
Color:
Layer:
Space:

remove data not relevant to the project submitted.



Subject: Callout
Page Label: 52
Author: dsdlaforce
Date: 3/19/2020 3:23:22 PM
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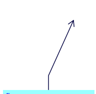
Remove and replace with the simplified tributary areas to the sediment ponds.



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Author: dsdlaforce
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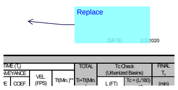

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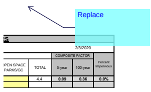
Replace



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Space:

Replace

Subject: Callout
Page Label: 46
Author: dsdlaforce
Date: 3/19/2020 3:25:08 PM
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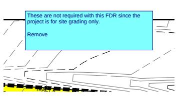
Color:
Layer:
Space:

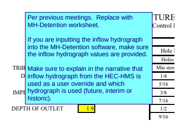
Update inserts.



Subject: Text Box
Page Label: 161
Author: dsdlaforce
Date: 3/19/2020 8:43:14 AM
Status:
Color:
Layer:
Space:

These are not required with this FDR since the project is for site grading only.

Remove

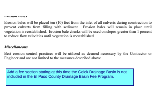


Subject: Text Box
Page Label: 120
Author: dsdlaforce
Date: 3/19/2020 8:51:09 AM
Status:
Color:
Layer:
Space:

Per previous meetings. Replace with MH-Detention worksheet.

If you are inputting the inflow hydrograph into the MH-Detention software, make sure the inflow hydrograph values are provided.

Make sure to explain in the narrative that inflow hydrograph from the HEC-HMS is used as a user override and which hydrograph is used (future, interim or historic).



Subject: Text Box
Page Label: 41
Author: dsdlaforce
Date: 3/19/2020 8:55:00 AM
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Color:
Layer:
Space:

Add a fee section stating at this time the Geick Drainage Basin is not included in the El Paso County Drainage Basin Fee Program.

Subject: Accepted
Page Label: 36
Author: dsdlaforce
Date: 3/19/2020 8:57:04 AM
Status:
Color:
Layer:
Space:

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Page Label: 37
Author: dsdlaforce
Date: 3/19/2020 8:57:19 AM
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