

PRELIMINARY/FINAL DRAINAGE REPORT FOR HIGH PLAINS FILING NO. 1

July 2018

Prepared for:

Savage Development, Inc.
835 Diamond Rim Drive
Colorado Springs, CO 80921

Drainage report comments will be provided with the final plat application (PCD File No SF1824)

Prepared By:


321 W. Henrietta Ave, Suite A
Woodland Park, CO 80863
719-426-2124

SP-18-003
SF-18-024

PCD FILE NO's: SP-18-XXX
SF-18-XXX

PRELIMINARY/FINAL DRAINAGE REPORT
HIGH PLAINS FILING NO. 1

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according the criteria established for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Certification Statement:

This report and plan for the preliminary and final drainage design for the HIGH PLAINS FILING NO. 1 was prepared by me (or under my direct supervision) in accordance with the provisions of City of Colorado Springs/El Paso County Drainage Criteria Manual Volumes 1 and 2 Drainage Design and Technical Criteria for the owners thereof. I understand that El Paso County does not and will not assume liability for drainage facilities designed by others.

David L. Mijares, Colorado PE #40510
For and on behalf of Catamount Engineering

Date

Developer's Statement:

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

Savage Development, Inc. hereby certifies that the drainage facilities for HIGH PLAINS FILING NO. 1 shall be constructed according to the design presented in this report. I understand that El Paso County does not and will not assume liability for the drainage facilities designed and or certified by my engineer and that the El Paso County reviews drainage plans pursuant to Colorado Revised Statutes, Title 30, Article 28; but cannot, on behalf of HIGH PLAINS FILING NO. 1, guarantee that final drainage design review will absolve Savage Development, Inc. and/or their successors and/or assigns of future liability for improper design. I further understand that approval of the final plat does not imply approval of my engineer's drainage design.

Savage Development, Inc.
Business Name

By: Jordan Savage

Title: President

Address: 835 Diamond Rim Drive

Colorado Springs, CO 80921

El Paso County:

Filed in accordance with the requirements of the El Paso County land Development Code and the Drainage Criteria manual Volumes 1 and 2, and the El Paso County Engineering Criteria Manual, latest revision.

Jennifer Irvine, PE
County Engineer/ECM Administrator

Date

Conditions:

PRELIMINARY/FINAL DRAINAGE REPORT for HIGH PLAINS FILING NO. 1

PURPOSE

The purpose of this drainage report is to identify existing drainage patterns, quantify developed storm water runoff, and establish outfall scenarios from the proposed development.

GENERAL LOCATION AND DESCRIPTION

The subject 38.49 acres consists of unplatted land to be developed into 7 rural residential lots (RR-5 zoning) located within the SE ¼ of Section 19, Township 11 South, Range 65 West of the 6th principal meridian in unincorporated El Paso County. The parcel is bounded to the north by unplatted land zoned RR-5, to the east and west by platted RR-5 residential lots, and to the south by Hodgen Road.

The parcel contains an unnamed tributary of the east fork of East Cherry Creek that flows from a dual culvert crossing of Hodgen Road at the southern limits of the parcel to the northeast and exits the parcel along the easterly property line. The site drains directly to the reach of Cherry Creek at slopes between 4% and 25%.

Existing soils on the site consist of Peyton sandy loam, hydrologic soil group B (51%), and Peyton -Pring complex, hydrologic soil group B (49%) as determined by the Natural Resources Conservation Service Web Soil Survey. The site is located within the East Cherry Creek Basin.

The site is sparsely vegetated with native grasses. Some volunteer shrubs and trees are evident within the existing drainage. A swale along the south edge of the project running from west to east and outfalls to the unnamed tributary of East Cherry Creek. The site lies within the East Cherry Creek Basin.

Existing soils on the site consist of Peyton sandy loam, hydrologic soil group B (51%), and Peyton -Pring complex, hydrologic soil group B (49%) as determined by the Natural Resources Conservation Service Web Soil Survey. Hydrologic Group B soils were used in analysis.

A portion of the site lies within an F.E.M.A. designated zone 'A' (unstudied) floodplain per FIRM 08041C0325 F, effective March 17, 1997. A LOMR is in process to develop base flood elevations for the reach and has been included in the appendix. Analysis of the floodplain indicates significant reduction in effective zone 'A' (unstudied) floodplain. The area currently identified as Zone 'A' (unstudied) has been included in a no build easement to be dedicated to El Paso County with plat recordation.

EXISTING DRAINAGE CONDITIONS

No existing studies on the site or overall basin have been identified. The parcel contains two unnamed tributaries to the Cherry Creek Basin. The westerly reach identified as design point SS3 ($Q_{100}=153$ cfs) enters the westerly boundary of the property within an unimproved swale and conveys flows to a confluence with the southerly unnamed tributary within the property. The southerly reach identified as design point SS2 ($Q_{100}=295$ cfs) enters the property through a dual 48" crossing of Hodgen Road installed by El Paso County. No hydrologic or hydraulic analysis was available for the crossing information. Combined flows are conveyed through the property northeasterly to the easterly property boundary (design point SS1, $Q_{100}=357$ cfs). USGS Streamstats modeling developed for the LOMR submittal was utilized in obtaining approximate flows within the reaches.

Basin E1 (22.00 Acres, $Q_2=0.8$ cfs, $Q_5=2.8$ cfs, $Q_{10}=6.2$ cfs, $Q_{25}=10.9$ cfs, $Q_{50}=14.6$ cfs, and $Q_{100}=18.9$ cfs) consists of that portion tributary to the westerly lot line of the parcel and sheetflow directly to the unnamed reach of East Cherry Creek within the parcel.

Basin E2 (5.46 Acres, $Q_2=0.3$ cfs, $Q_5=1.3$ cfs, $Q_{10}=2.8$ cfs, $Q_{25}=4.8$ cfs, $Q_{50}=6.5$ cfs, and $Q_{100}=8.4$ cfs) consists of that portion tributary to the northerly lot line of the parcel and sheetflow directly to the unnamed reach of East Cherry Creek within the parcel.

Basin E3 (1.62 Acres, $Q_2=1.0$ cfs, $Q_5=1.5$ cfs, $Q_{10}=2.2$ cfs, $Q_{25}=3.1$ cfs, $Q_{50}=3.8$ cfs, and $Q_{100}=4.6$ cfs) consists of that portion tributary to the southerly lot line of the parcel west of the channel and sheetflow directly to the unnamed reach of East Cherry Creek within the parcel.

Basin E4 (3.53 Acres, $Q_2=0.9$ cfs, $Q_5=1.6$ cfs, $Q_{10}=2.8$ cfs, $Q_{25}=4.3$ cfs, $Q_{50}=5.5$ cfs, and $Q_{100}=6.9$ cfs) consists of that portion tributary to the southerly lot line of the parcel east of the channel and sheetflow directly to the unnamed reach of East Cherry Creek within the parcel.

Basin E5 (38.49 Acres, $Q_2=2.4$ cfs, $Q_5=9.1$ cfs, $Q_{10}=20.0$ cfs, $Q_{25}=34.9$ cfs, $Q_{50}=46.8$ cfs, and $Q_{100}=60.8$ cfs) consists of the majority of the development parcel which sheetflow directly to the reach of East Cherry Creek within the parcel.

DEVELOPED DRAINAGE BASINS

The majority of the area within basins was modeled as 1-acre residential. Areas identified as no-build were modeled as agricultural land. Roadways and shoulders were modeled as pavement and gravel roadways where proposed.

Basin A1 (5.91 Acres, $Q_2=2.9$ cfs, $Q_5=5.1$ cfs, $Q_{10}=7.3$ cfs, $Q_{25}=10.3$ cfs, $Q_{50}=12.9$ cfs, and $Q_{100}=15.6$ cfs) represents the northwesterly portion of proposed residential lots and the central cul-de-sac. Runoff generated within the basin will sheet flow to the roadside ditch adjacent to the proposed cul-de-sac and be conveyed to a lowpoint at a common lot line within the cul-de-sac bulb at Design Point 3. Flows from Design Point 3 will be conveyed in a swale directly to the reach of East Cherry Creek.

Revise. Flowrates listed do not match calculations

Basin A2 (0.88 Acres, $Q_2=2.5$ cfs, $Q_5=3.1$ cfs, $Q_{10}=3.7$ cfs, $Q_{25}=10.3$ cfs, $Q_{50}=12.9$ cfs, and $Q_{100}=15.6$ cfs) consists of the westerly half of the proposed north-south roadway. The roadway was modeled assuming ultimate construction to the northerly property line rather than the interim condition of termination at connection with the cul-de-sac connection to allow for appropriate southerly culvert analysis. Sheet flow from the roadway is conveyed south to the proposed culvert triple 36" culvert crossing at Design Point 1. Design point 1 ($Q_{100}=173.9$ cfs) represents the confluence of Basins A2, Basin E1, and Stream Stats Design Point SS3. Flows are conveyed in a 3.5' deep, 5' bottom width channel with a 1% longitudinal slope to the reach of East Cherry Creek.

Basin A3 (3.48 Acres, $Q_2=0.7$ cfs, $Q_5=1.5$ cfs, $Q_{10}=2.6$ cfs, $Q_{25}=4.0$ cfs, $Q_{50}=5.2$ cfs, and $Q_{100}=6.5$ cfs) consists of the southeasterly portion of the residential lots directly tributary to the existing Hodgen Roadside ditch. Combined flows from Basin A-3 and existing Basin E3 are conveyed within the existing roadside ditch directly to the Reach of the East Fork of Cherry Creek at Design Point 2 ($Q_2=2.5$ cfs, $Q_5=3.4$ cfs, $Q_{10}=4.3$ cfs, $Q_{25}=5.5$ cfs, $Q_{50}=6.4$ cfs, and $Q_{100}=7.5$ cfs).

Basin A4 (28.21 Acres, $Q_2=6.8$ cfs, $Q_5=15.0$ cfs, $Q_{10}=24.9$ cfs, $Q_{25}=38.0$ cfs, $Q_{50}=49.3$ cfs, and $Q_{100}=61.4$ cfs) consists of residential, no-build, and roadway areas in the center of the property directly tributary to the Reach of the East Cherry Creek.

Does overtopping occur during the 100yr event?

See Appendix for Calculations.

The flowrate shown on the proposed and existing drainage map are equal for DP SS1. Include a narrative explaining/stating the insignificant increase in discharge rate between the existing and developed condition.

In conjunction with the above comment, provide a narrative justifying why no detention is required.

WATER QUALITY/4-STEP PROCEDURE

The development addresses Low Impact Development strategies primarily through the utilization of large impervious areas and utilization of landscape swales receiving runoff generated within impervious roadways. Impervious areas generated within home sites will flow across pervious disconnected areas prior to offsite discharge.

A Grading, Erosion Control, and Stormwater Quality Plan and narrative have been submitted concurrently for the development and will be subject to county approval prior to any soil disturbance. The erosion control plan included specific source control BMP's as well defined overall site management practices for the construction period.

List each of the 4 step (ECM Appendix I.7.2, pg I-21). Below each step provide a narrative how that specific step was implemented/considered in the design process.

Categorically state permanent WQ facility is not required. See ECM I.7.1.B, pg I-18

COST ESTIMATE

Private Improvements Non-reimbursable

The proposed culverts should be public non-reimbursable improvements.

36" RCP	156 LF	@\$	75/LF	\$	11,700
36" FES	6 EA	@\$	350/EA	\$	2,100
30" RCP	85 LF	@\$	75/LF	\$	6,375
30" FES	2 EA	@\$	350/EA	\$	700
Rip Rap Outfall	3 EA	@\$	500/EA	\$	1,500
SUBTOTAL				\$	22,375
15% CONTINGENCY				\$	3,356
TOTAL				\$	25,731

DRAINAGE FEE CALCULATION

The development proposes to plat 38.49 acres within El Paso County, all contained within the East Cherry Creek Drainage Basin. The East Cherry Creek Drainage Basin has not been studied and no drainage or bridge fees have been adopted.

DRAINAGE METHODOLOGY

This drainage report was prepared in accordance to the criteria established in the City of Colorado Springs/El Paso County Drainage Criteria Manual Volumes 1 and 2, as revised May 2014.

The rational method for drainage basin study areas of less than 100 acres was utilized in the on-site analysis. For the Rational Method, flows were calculated for the 2, 5, 10, 25, 50, and 100-year recurrence intervals. The average runoff coefficients, 'C' values, are taken from Table 6-6 and the Intensity-Duration-Frequency curves are taken from Figure 6-5 of the City Drainage Criteria Manual. Time of concentration for overland flow and storm drain or gutter flow are calculated per Section 3.2 of the City Drainage Criteria Manual. Calculations for the Rational Method are shown in the Appendix of this report.

StreamStats version 4 (USGS) was utilized in development of hydrology for off-site basins in floodplain development for FEMA submittal. HEC-RAS version 5.0.1 was utilized in channel and existing culvert modeling developing base flood elevations refining the existing Zone 'A' unstudied floodplain within the development.

SUMMARY

The High Plains Filing No. 1 project consists of large lot development with minor increases in impervious areas consistent with surrounding rural development. The development proposes no development and a setback approach in regards to the reach of the East Cherry Creek drainage within the parcel. A no-build easement has been established outside of the limits of the existing jurisdictional zone 'A' unstudied 100-YR floodplain. A LOMR is in process developing base flood elevations through the reach. Development of the parcel is in conformance of current El Paso County criteria and will not adversely affect downstream properties or facilities.

REFERENCES:

City of Colorado Springs Engineering Division Drainage Criteria Manual Volumes 1 and 2, revised May 2014

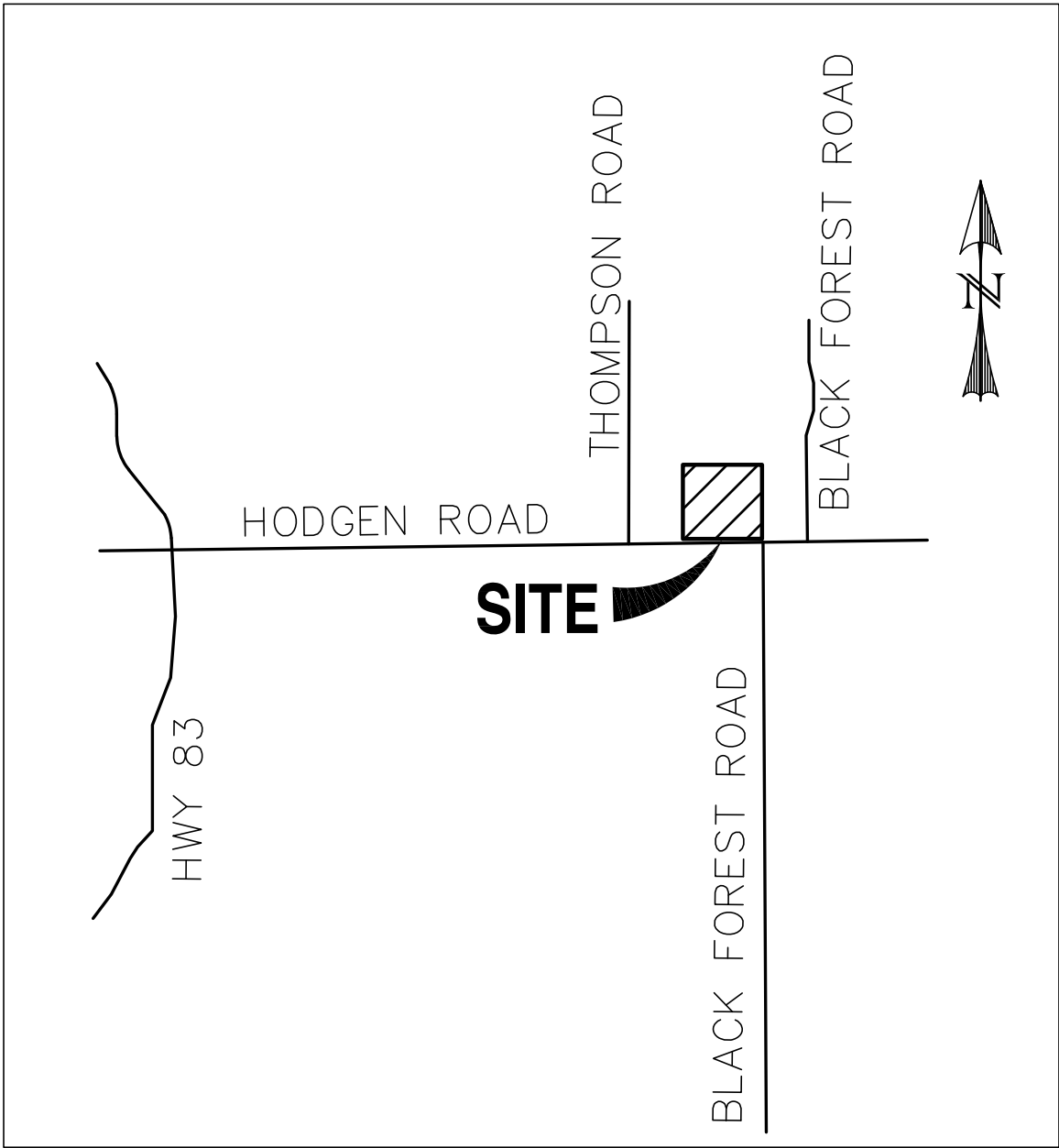
“Drainage Study Rockin’ Four-ESE Subdivision El Paso County, Colorado”, prepared by E.L.B. & Asso. Inc., dated April 24, 1980.

“LOMR Case # 18-08-072”, prepared by Catamount Engineering, DRAFT

Flood Insurance rate map 08041C0325 F

Natural Resources Conservation Service Web Soil Survey

APPENDIX



VICINITY MAP

SCALE: N.T.S.



APPROXIMATE SCALE IN FEET
2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY,
COLORADO AND
INCORPORATED AREAS

PANEL 325 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

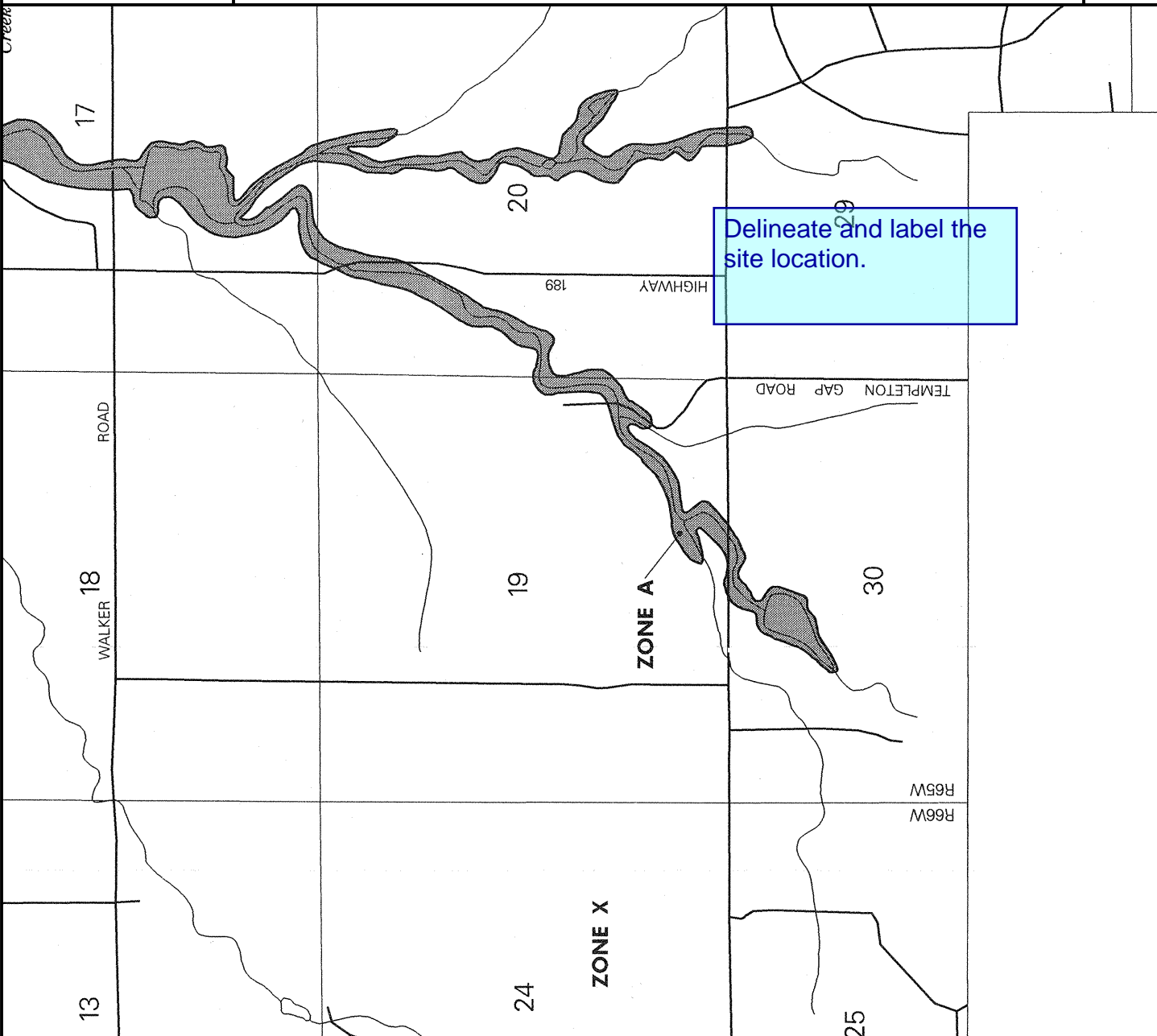
CONTAINS:
COMMUNITY
EL PASO COUNTY
UNINCORPORATED AREAS
NUMBER PANEL SUFFIX
080059 0325 F

MAP NUMBER
08041C0325 F
EFFECTIVE DATE:
MARCH 17, 1997

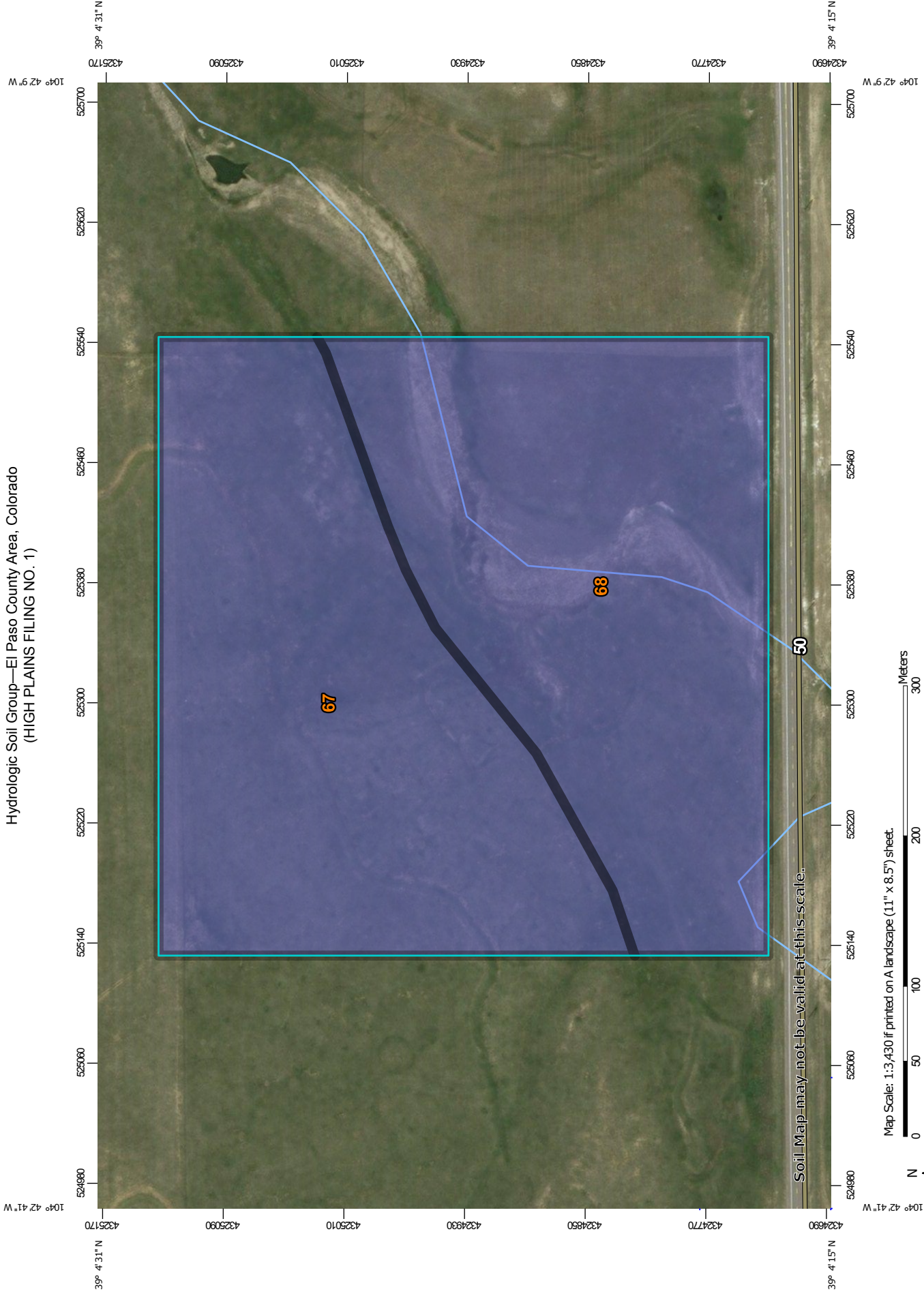


Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps, check the FEMA Flood Map Store at www.msc.fema.gov



Hydrologic Soil Group—El Paso County Area, Colorado
(HIGH PLAINS FILING NO. 1)



Map Scale: 1:3,430 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



MAP LEGEND

Area of Interest (AOI)
 Area of Interest (AOI)

Soils

Soil Rating Polygons

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Lines

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Points

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2016—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
67	Peyton sandy loam, 5 to 9 percent slopes	B	20.9	50.6%
68	Peyton-Pring complex, 3 to 8 percent slopes	B	20.4	49.4%
Totals for Area of Interest			41.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

EXISTING HYDROLOGY

BASIN	AREA TOTAL (Acres)	CONVEYANCE TC										TT TOTAL (min)	INTENSITY						TOTAL FLOWS											
		C ₂	C ₅	C ₁₀	C ₂₅	C ₅₀	C ₁₀₀	Length (ft)	Height (ft)	TI (min)	Length (ft)		Height (ft)	TC (min)	Velocity (fps)	Slope (%)	C _v	I ₂ (in/hr)	I ₅ (in/hr)	I ₁₀ (in/hr)	I ₂₅ (in/hr)	I ₅₀ (in/hr)	I ₁₀₀ (in/hr)	Q ₂ (c.f.s.)	Q ₅ (c.f.s.)	Q ₁₀ (c.f.s.)	Q ₂₅ (c.f.s.)	Q ₅₀ (c.f.s.)	Q ₁₀₀ (c.f.s.)	
E1 AGRICULTURE	22.00	0.03	0.09	0.17	0.26	0.31	0.36	200	8	17.3	1597	24	5	1.5%	0.6	43.4	60.7	1.1	1.4	1.7	1.9	2.1	2.4	0.8	2.8	6.2	10.9	14.6	18.9	
E2 AGRICULTURE	5.46	0.03	0.09	0.17	0.26	0.31	0.36	200	12	15.1	834	35	5	4.2%	1.0	13.6	28.7	2.0	2.5	3.0	3.4	3.8	4.3	0.3	1.3	2.8	4.8	6.5	8.4	
E3 AGRICULTURE ROADWAY	1.62 1.25 0.37	0.23 0.03 0.89	0.28 0.09 0.90	0.34 0.17 0.92	0.42 0.26 0.94	0.46 0.31 0.95	0.50 0.36 0.96	100	3	11.0	186	3	5	1.6%	0.6	4.9	15.9	15.9	2.7	3.4	4.0	4.6	5.2	5.8	1.0	1.5	2.2	3.1	3.8	4.6
E4 AGRICULTURE ROADWAY	3.53 3.20 0.33	0.11 0.03 0.89	0.17 0.09 0.90	0.24 0.17 0.92	0.32 0.26 0.94	0.37 0.31 0.95	0.42 0.36 0.96	200	7	16.7	610	43	5	7.0%	1.3	7.7	24.4	24.4	2.2	2.8	3.3	3.7	4.2	4.7	0.9	1.6	2.8	4.3	5.5	6.9
E5 AGRICULTURE	38.49	0.03	0.09	0.17	0.26	0.31	0.36	200	9	16.6	790	47	5	5.9%	1.2	10.8	27.4	27.4	2.1	2.6	3.1	3.5	3.9	4.4	2.4	9.1	20.0	34.9	46.8	60.8

Calculated by: DLM
Date: 7/16/2018

DESIGN POINT	AREA TOTAL (Acres)	WEIGHTED						TT TOTAL (min)	INTENSITY						TOTAL FLOWS					
		C ₂	C ₅	C ₁₀	C ₂₅	C ₅₀	C ₁₀₀		I ₂ (in/hr)	I ₅ (in/hr)	I ₁₀ (in/hr)	I ₂₅ (in/hr)	I ₅₀ (in/hr)	I ₁₀₀ (in/hr)	Q ₂ (c.f.s.)	Q ₅ (c.f.s.)	Q ₁₀ (c.f.s.)	Q ₂₅ (c.f.s.)	Q ₅₀ (c.f.s.)	Q ₁₀₀ (c.f.s.)
		0.00	0.00	0.00	0.00	0.00	0.00		#####	#####	#####	#####	#####	#####	#####	#####	#####	#####	#####	#####
DP-1 BASIN OS-A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0	#####	#####	#####	#####	#####	#####	#####	#####	#####	#####	#####	#####
DP-2 LOMR																				592.0
DP-3 BASIN OS-B	23.62	0.04	0.10	0.18	0.27	0.32	0.37	60.7	1.1	1.4	1.7	1.9	2.1	2.4	1.2	3.5	7.1	12.1	16.1	20.8
BASIN OS-B	22.00	0.03	0.09	0.17	0.26	0.31	0.36													
BASIN EX-B	1.62	0.23	0.28	0.34	0.42	0.46	0.50													

Update: Subbasin and DP designation does not match the existing drainage map.

PROPOSED HYDROLOGY

BASIN	AREA TOTAL (Acres)	CONVEYANCE TC										INTENSITY						TOTAL FLOWS												
		C ₂	C ₅	C ₁₀	C ₂₅	C ₅₀	C ₁₀₀	Length (ft)	Height (ft)	TI (min)	Length (ft)	Height (ft)	TC (min)	Velocity (fps)	Slope (%)	TC (min)	TOTAL (min)	I ₂ (in/hr)	I ₅ (in/hr)	I ₁₀ (in/hr)	I ₂₅ (in/hr)	I ₅₀ (in/hr)	I ₁₀₀ (in/hr)	Q ₂ (c.f.s.)	Q ₅ (c.f.s.)	Q ₁₀ (c.f.s.)	Q ₂₅ (c.f.s.)	Q ₅₀ (c.f.s.)	Q ₁₀₀ (c.f.s.)	
E1 AGRICULTURE	22.00	0.03	0.09	0.17	0.26	0.31	0.36	200	8	17.3	1597	24	5	1.5%	0.6	43.4	60.7	1.1	1.4	1.7	1.9	2.1	2.4	0.8	2.8	6.2	10.9	14.6	18.9	
E2 AGRICULTURE	5.46	0.03	0.09	0.17	0.26	0.31	0.36	200	12	15.1	834	35	5	4.2%	1.0	13.6	28.7	2.0	2.5	3.0	3.4	3.8	4.3	0.3	1.3	2.8	4.8	6.5	8.4	
E3 AGRICULTURE ROADWAY	1.62	0.23	0.28	0.34	0.42	0.46	0.50	100	3	11.0	186	3	5	1.6%	0.6	4.9	15.9	2.7	3.4	4.0	4.6	5.2	5.8	1.0	1.5	2.2	3.1	3.8	4.6	
E4 AGRICULTURE ROADWAY	3.53	0.11	0.17	0.24	0.32	0.37	0.42	200	7	16.7	610	43	5	7.0%	1.3	7.7	24.4	2.2	2.8	3.3	3.7	4.2	4.7	0.9	1.6	2.8	4.3	5.5	6.9	
A1 RESIDENTIAL ROADWAY	5.91	0.19	0.27	0.33	0.41	0.45	0.49	100	4	10.4	740	35	7	4.7%	1.5	8.1	18.5	2.6	3.2	3.7	4.3	4.8	5.4	2.9	5.1	7.3	10.3	12.9	15.6	
A2 ROADWAY	0.88	0.89	0.90	0.92	0.94	0.95	0.96	40	0.7	2.0	1053	36	10	3.4%	1.8	9.5	11.5	3.1	3.9	4.6	5.2	5.9	6.6	2.5	3.1	3.7	4.3	4.9	5.6	
A3 RESIDENTIAL NO BUILD	3.48	0.09	0.16	0.23	0.32	0.37	0.41	100	3	12.7	509	9	5	1.8%	0.7	12.8	25.5	2.2	2.7	3.2	3.6	4.1	4.6	0.7	1.5	2.6	4.0	5.2	6.5	
A4 RESIDENTIAL NO BUILD ROADWAY	28.21	0.10	0.17	0.24	0.32	0.37	0.42	100	6	10.0	693	40	5	5.8%	1.2	9.6	19.6	2.5	3.1	3.6	4.2	4.7	5.2	6.8	15.0	24.9	38.0	49.3	61.4	
	0.27	0.89	0.90	0.92	0.94	0.95	0.96																							

Calculated by: DLM

Date: 7/16/2018

DESIGN POINT	AREA TOTAL (Acres)	WEIGHTED						TT TOTAL (min)	INTENSITY						TOTAL FLOWS						
		C ₂	C ₅	C ₁₀	C ₂₅	C ₅₀	C ₁₀₀		I ₂ (in/hr)	I ₅ (in/hr)	I ₁₀ (in/hr)	I ₂₅ (in/hr)	I ₅₀ (in/hr)	I ₁₀₀ (in/hr)	Q ₂ (c.f.s.)	Q ₅ (c.f.s.)	Q ₁₀ (c.f.s.)	Q ₂₅ (c.f.s.)	Q ₅₀ (c.f.s.)	Q ₁₀₀ (c.f.s.)	
DP-1 BASIN E1 BASIN A2 DP-SS3	22.88 22.00 0.88	0.06 0.03 0.89	0.12 0.09 0.90	0.20 0.17 0.92	0.29 0.26 0.94	0.33 0.31 0.95	0.38 0.36 0.96	60.7	1.1	1.4	1.7	1.9	2.1	2.4							173.9
DP-2 BASIN E3 BASIN A3	2.50 1.62 0.88	0.46 0.23 0.89	0.50 0.28 0.90	0.55 0.34 0.92	0.60 0.42 0.94	0.63 0.46 0.95	0.66 0.50 0.96	25.5	2.2	2.7	3.2	3.6	4.1	4.6	2.5	3.4	4.3	5.5	6.4	7.5	
DP-3 BASIN A1	5.91 5.91	0.19 0.19	0.27 0.27	0.33 0.33	0.41 0.41	0.45 0.45	0.49 0.49	18.5	2.6	3.2	3.7	4.3	4.8	5.4	2.9	5.1	7.3	10.3	12.9	15.6	

Include DP SS1.



321 W. HENRIETTA AVE SUITE A
PO BOX 221
WOODLAND PARK, CO 80866

SAVAGE SUBDIVISION: L.O.M.R. REQUEST – PROJECT NARRATIVE

The Savage Subdivision development project is a one filing subdivision located in the northern portion of El Paso County, Colorado. This subdivision is located entirely within the NFIP FIRM Map for El Paso County, Colorado and Incorporated Areas, panel 325 of 1300, map number 08041C0325F, with an effective date of March 17, 1997.

The project is bisected by an unnamed tributary of the East Fork of Cherry Creek and it is in this section of the East Fork of Cherry Creek that this LOMR relates. The current zone designation for this reach is Zone A with no existing FIS model. It is our intent to establish Base Flood Elevations for this portion of the unnamed tributary of the East Fork of Cherry Creek.

Documents supporting this application include hydrologic analysis calculations using the Regional Regression methodology equations within the USGS StreamStats computer program. Hydraulic analysis calculations were performed using the USACE HEC-RAS computer program. A certified topographic work map has been included showing the subdivision with 1-foot contours generated from field work conducted by a land surveyor. This map shows the current Zone A boundary as well as the proposed floodplain boundary with Base Flood Elevations.

Hydrologic electronic data files from the USGS StreamStats program as well as hydraulic data files from the HEC-RAS model have been included with this submittal.

This application is exempt from a review fee as it is establishing Base Flood Elevations for an area where no current data exists. This reach of the unnamed tributary of the East Fork of Cherry Creek is well established and no changes are being made to the existing channel, as such, sediment transport was not considered in this analysis.

Please direct any questions concerning this application to Catamount Engineering.

Dave Mijares, P.E.
Catamount Engineering
321 W. Henrietta Ave. Suite 'A'
P.O. Box 221
Woodland Park, CO 80866
(719) 426-2124
david@catamounteng.com

U.S. DEPARTMENT OF HOMELAND SECURITY
 FEDERAL EMERGENCY MANAGEMENT AGENCY
OVERVIEW & CONCURRENCE FORM

*O.M.B No. 1660-0016
 Expires February 28, 2014*

PAPERWORK BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 1 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless it displays a valid OMB control number. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Department of Homeland Security, Federal Emergency Management Agency, 1800 South Bell Street, Arlington, VA 20958-3005, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

PRIVACY ACT STATEMENT

AUTHORITY: The National Flood Insurance Act of 1968, Public Law 90-448, as amended by the Flood Disaster Protection Act of 1973, Public Law 93-234.

PRINCIPAL PURPOSE(S): This information is being collected for the purpose of determining an applicant's eligibility to request changes to National Flood Insurance Program (NFIP) Flood Insurance Rate Maps (FIRM).

ROUTINE USE(S): The information on this form may be disclosed as generally permitted under 5 U.S.C § 552a(b) of the Privacy Act of 1974, as amended. This includes using this information as necessary and authorized by the routine uses published in DHS/FEMA/NFIP/LOMA-1 National Flood Insurance Program (NFIP); Letter of Map Amendment (LOMA) February 15, 2006, 71 FR 7990.

DISCLOSURE: The disclosure of information on this form is voluntary; however, failure to provide the information requested may delay or prevent FEMA from processing a determination regarding a requested change to a (NFIP) Flood Insurance Rate Maps (FIRM).

A. REQUESTED RESPONSE FROM DHS-FEMA

This request is for a (check one):

- CLOMR: A letter from DHS-FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60, 65 & 72).
- LOMR: A letter from DHS-FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See 44 CFR Ch. 1, Parts 60, 65 & 72)

B. OVERVIEW

1. The NFIP map panel(s) affected for all impacted communities is (are):

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
Example: 480301	City of Katy	TX	48473C	0005D	02/08/83
480287	Harris County	TX	48201C	0220G	09/28/90
080059	El Paso County - Unincorporated Areas	CO	08041C	0325F	03/17/97

2. a. Flooding Source: East Cherry Creek

- b. Types of Flooding: Riverine Coastal Shallow Flooding (e.g., Zones AO and AH)
 Alluvial fan Lakes Other (Attach Description)

3. Project Name/Identifier: Savage Subdivision

4. FEMA zone designations affected: A (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)

5. Basis for Request and Type of Revision:

a. The basis for this revision request is (check all that apply)

- Physical Change Improved Methodology/Data Regulatory Floodway Revision Base Map Changes
 Coastal Analysis Hydraulic Analysis Hydrologic Analysis Corrections
 Weir-Dam Changes Levee Certification Alluvial Fan Analysis Natural Changes
 New Topographic Data Other (Attach Description)

Note: A photograph and narrative description of the area of concern is not required, but is very helpful during review.

b. The area of revision encompasses the following structures (check all that apply)

- Structures:
- | | | |
|---|--|---|
| <input type="checkbox"/> Channelization | <input type="checkbox"/> Levee/Floodwall | <input checked="" type="checkbox"/> Bridge/Culvert |
| <input type="checkbox"/> Dam | <input type="checkbox"/> Fill | <input type="checkbox"/> Other (Attach Description) |

6. Documentation of ESA compliance is submitted (required to initiate CLOMR review). Please refer to the instructions for more information.

C. REVIEW FEE

Has the review fee for the appropriate request category been included? Yes Fee amount: \$_____

No, Attach Explanation

Please see the DHS-FEMA Web site at http://www.fema.gov/plan/prevent/fhm/frm_fees.shtm for Fee Amounts and Exemptions.

D. SIGNATURE

All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Name: Jordan Savage		Company: Savage Development, Inc.	
Mailing Address: 1125 Diamond Rim Drive Colorado Springs, Colorado 80921		Daytime Telephone No.: (719) 649-5266	Fax No.:
		E-Mail Address: jsavage@goodwinknight.com	
Signature of Requester (required):		Date:	

As the community official responsible for floodplain management, I hereby acknowledge that we have received and reviewed this Letter of Map Revision (LOMR) or conditional LOMR request. Based upon the community's review, we find the completed or proposed project meets or is designed to meet all of the community floodplain management requirements, including the requirements for when fill is placed in the regulatory floodway, and that all necessary Federal, State, and local permits have been, or in the case of a conditional LOMR, will be obtained. For Conditional LOMR requests, the applicant has documented Endangered Species Act (ESA) compliance to FEMA prior to FEMA's review of the Conditional LOMR application. For LOMR requests, I acknowledge that compliance with Sections 9 and 10 of the ESA has been achieved independently of FEMA's process. For actions authorized, funded, or being carried out by Federal or State agencies, documentation from the agency showing its compliance with Section 7(a)(2) of the ESA will be submitted. In addition, we have determined that the land and any existing or proposed structures to be removed from the SFHA are or will be reasonably safe from flooding as defined in 44CFR 65.2(c), and that we have available upon request by FEMA, all analyses and documentation used to make this determination.

Community Official's Name and Title: Keith Curtis - Floodplain Administrator		Community Name: El Paso County	
Mailing Address: 2880 International Circle Colorado Springs, Colorado 80910		Daytime Telephone No.: (719) 327-2898	Fax No.: N/A
		E-Mail Address: keith@pprbd.org	
Community Official's Signature (required):		Date:	

CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER AND/OR LAND SURVEYOR

This certification is to be signed and sealed by a licensed land surveyor, registered professional engineer, or architect authorized by law to certify elevation information data, hydrologic and hydraulic analysis, and any other supporting information as per NFIP regulations paragraph 65.2(b) and as described in the MT-2 Forms Instructions. All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

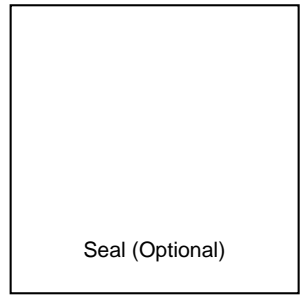
Certifier's Name: David Mijares		License No.: 40510	Expiration Date: October 31, 2019
Company Name: Catamount Engineering		Telephone No.: (719) 426-2124	Fax No.: N/A
Signature:	Date:	E-Mail Address: david@catamounteng.com	

Ensure the forms that are appropriate to your revision request are included in your submittal.

Form Name and (Number)

Required if ...

- | | |
|---|---|
| <input checked="" type="checkbox"/> Riverine Hydrology and Hydraulics Form (Form 2) | New or revised discharges or water-surface elevations |
| <input type="checkbox"/> Riverine Structures Form (Form 3) | Channel is modified, addition/revision of bridge/culverts, addition/revision of levee/floodwall, addition/revision of dam |
| <input type="checkbox"/> Coastal Analysis Form (Form 4) | New or revised coastal elevations |
| <input type="checkbox"/> Coastal Structures Form (Form 5) | Addition/revision of coastal structure |
| <input type="checkbox"/> Alluvial Fan Flooding Form (Form 6) | Flood control measures on alluvial fans |



U.S. DEPARTMENT OF HOMELAND SECURITY
 FEDERAL EMERGENCY MANAGEMENT AGENCY
RIVERINE HYDROLOGY & HYDRAULICS FORM

*O.M.B No. 1660-0016
 Expires February 28, 2014*

PAPERWORK BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 3.5 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Department of Homeland Security, Federal Emergency Management Agency, 1800 South Bell Street, Arlington VA 20958-3005, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

PRIVACY ACT STATEMENT

AUTHORITY: The National Flood Insurance Act of 1968, Public Law 90-448, as amended by the Flood Disaster Protection Act of 1973, Public Law 93-234.

PRINCIPAL PURPOSE(S): This information is being collected for the purpose of determining an applicant's eligibility to request changes to National Flood Insurance Program (NFIP) Flood Insurance Rate Maps (FIRM).

ROUTINE USE(S): The information on this form may be disclosed as generally permitted under 5 U.S.C § 552a(b) of the Privacy Act of 1974, as amended. This includes using this information as necessary and authorized by the routine uses published in DHS/FEMA/NFIP/LOMA-1 National Flood Insurance Program (NFIP); Letter of Map Amendment (LOMA) February 15, 2006, 71 FR 7990.

DISCLOSURE: The disclosure of information on this form is voluntary; however, failure to provide the information requested may delay or prevent FEMA from processing a determination regarding a requested change to a NFIP Flood Insurance Rate Maps (FIRM).

Flooding Source: East Cherry Creek _____

Note: Fill out one form for each flooding source studied

A. HYDROLOGY

1. Reason for New Hydrologic Analysis (check all that apply)

- Not revised (skip to section B)
 No existing analysis
 Improved data
 Alternative methodology
 Proposed Conditions (CLOMR)
 Changed physical condition of watershed

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	Effective/FIS (cfs)	Revised (cfs)
----------	-------------------------	---------------------	---------------

3. Methodology for New Hydrologic Analysis (check all that apply)

- Statistical Analysis of Gage Records
 Precipitation/Runoff Model → Specify Model: _____
 Regional Regression Equations
 Other (please attach description)

Please enclose all relevant models in digital format, maps, computations (including computation of parameters), and documentation to support the new analysis.

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Is the hydrology for the revised flooding source(s) affected by sediment transport? Yes No

If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation..

B. HYDRAULICS

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit*	<u>1564' North of the Hodgen Rd. Centerline</u>	<u>0+00</u>	_____	<u>7413.90</u>
Upstream Limit*	<u>916' South of the Hodgen Road Centerline</u>	<u>24+80</u>	_____	<u>7438.21</u>

*Proposed/Revised elevations must tie-into the Effective elevations within 0.5 foot at the downstream and upstream limits of revision.

2. Hydraulic Method/Model Used: HEC-RAS v5.03

3. Pre-Submittal Review of Hydraulic Models*
 DHS-FEMA has developed two review programs, CHECK-2 and CHECK-RAS, to aid in the review of HEC-2 and HEC-RAS hydraulic models, respectively. We recommend that you review your HEC-2 and HEC-RAS models with CHECK-2 and CHECK-RAS.

4. **Models Submitted**

	<u>Natural Run</u>		<u>Floodway Run</u>		<u>Datum</u>
	File Name:	Plan Name:	File Name:	Plan Name:	
Duplicate Effective Model*	_____	_____	_____	_____	_____
Corrected Effective Model*	_____	_____	_____	_____	_____
Existing or Pre-Project Conditions Model	_____	_____	_____	_____	_____
Revised or Post-Project Conditions Model	File Name: <u>Savage</u>	Plan Name: <u>100Yr Subcritical</u>	_____	_____	<u>NAVD88</u>
Other - (attach description)	_____	_____	_____	_____	_____

* For details, refer to the corresponding section of the instructions.

Digital Models Submitted? (Required)

C. MAPPING REQUIREMENTS

A **certified topographic work map** must be submitted showing the following information (where applicable): the boundaries of the effective, existing, and proposed conditions 1%-annual-chance floodplain (for approximate Zone A revisions) or the boundaries of the 1%- and 0.2%-annual-chance floodplains and regulatory floodway (for detailed Zone AE, AO, and AH revisions); location and alignment of all cross sections with stationing control indicated; stream, road, and other alignments (e.g., dams, levees, etc.); current community easements and boundaries; boundaries of the requester's property; certification of a registered professional engineer registered in the subject State; location and description of reference marks; and the referenced vertical datum (NGVD, NAVD, etc.).

Digital Mapping (GIS/CADD) Data Submitted (preferred)

Topographic Information: Field survey

Source: Barron Land Date: January 22, 2018

Accuracy: 1' Contour Interval

Note that the boundaries of the existing or proposed conditions floodplains and regulatory floodway to be shown on the revised FIRM and/or FBFM must tie-in with the effective floodplain and regulatory floodway boundaries. Please attach a **copy of the effective FIRM and/or FBFM**, at the same scale as the original, annotated to show the boundaries of the revised 1%-and 0.2%-annual-chance floodplains and regulatory floodway that tie-in with the boundaries of the effective 1%-and 0.2%-annual-chance floodplain and regulatory floodway at the upstream and downstream limits of the area on revision.

Annotated FIRM and/or FBFM (Required)

D. COMMON REGULATORY REQUIREMENTS*

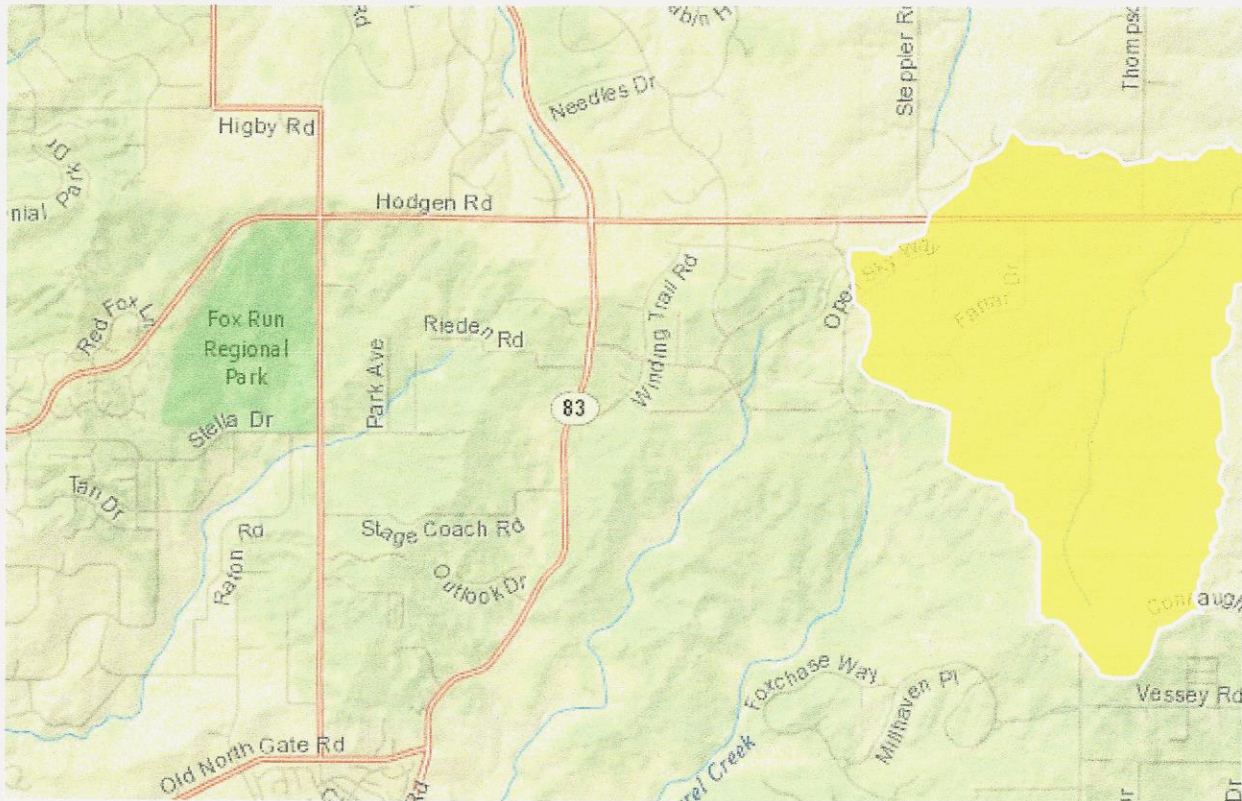
1. For LOMR/CLOMR requests, do Base Flood Elevations (BFEs) increase? Yes No
- a. For CLOMR requests, if either of the following is true, please submit **evidence of compliance with Section 65.12 of the NFIP regulations**:
- The proposed project encroaches upon a regulatory floodway and would result in increases above 0.00 foot compared to pre-project conditions.
 - The proposed project encroaches upon a SFHA with or without BFEs established and would result in increases above 1.00 foot compared to pre-project conditions.
- b. Does this LOMR request cause increase in the BFE and/or SFHA compared with the effective BFEs and/or SFHA? Yes No
If Yes, please attach **proof of property owner notification and acceptance (if available)**. Elements of and examples of property owner notifications can be found in the MT-2 Form 2 Instructions.
2. Does the request involve the placement or proposed placement of fill? Yes No
- If Yes, the community must be able to certify that the area to be removed from the special flood hazard area, to include any structures or proposed structures, meets all of the standards of the local floodplain ordinances, and is reasonably safe from flooding in accordance with the NFIP regulations set forth at 44 CFR 60.3(A)(3), 65.5(a)(4), and 65.6(a)(14). Please see the MT-2 instructions for more information.
3. For LOMR requests, is the regulatory floodway being revised? Yes No
- If Yes, attach **evidence of regulatory floodway revision notification**. As per Paragraph 65.7(b)(1) of the NFIP Regulations, notification is required for requests involving revisions to the regulatory floodway. (Not required for revisions to approximate 1%-annual-chance floodplains [studied Zone A designation] unless a regulatory floodway is being established. Elements and examples of regulatory floodway revision notification can be found in the MT-2 Form 2 Instructions.)
4. For CLOMR requests, please submit documentation to FEMA and the community to show that you have complied with Sections 9 and 10 of the Endangered Species Act (ESA).

For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA. Please see the MT-2 instructions for more detail.

* Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.

SAVAGE PROJECT - StreamStats Report 1

Region ID: CO
 Workspace ID: CO20180218171329262000
 Clicked Point (Latitude, Longitude): 39.07488, -104.70275
 Time: 2018-02-18 10:13:45 -0700



Design Point Above Minor Trib TGap

Basin Characteristics

Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	2.96	square miles
16H100Y	6-hour precipitation that is expected to occur on average once in 100 years	3.84	inches
STATSCLAY	Percentage of clay soils from STATSGO	16.3	percent
OUTLETELEV	Elevation of the stream outlet in thousands of feet above NAVD88.	7416	feet
124H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	4.99	inches

Parameter Code	Parameter Description	Value	Unit
RCN	Runoff-curve number as defined by NRCS (http://policy.nrcs.usda.gov/OpenNonWebContent.aspx?content=17758.wba)	60.68	
TOC	Time of concentration in hours	3.6	
RUNCO_CO	Soil runoff coefficient as defined by Verdin and Gross (2017)	0.25	
LFPLENGTH	Length of longest flow path	3.41	miles
BSLDEM10M	Mean basin slope computed from 10 m DEM	6.42	percent
CSL1085LFP	Change in elevation divided by length between points 10 and 85 percent of distance along the longest flow path to the basin divide, LFP from 2D grid	68	feet per mi
EL7500	Percent of area above 7500 ft	83	percent
ELEV	Mean Basin Elevation	7557	feet
ELEVMAX	Maximum basin elevation	7690	feet
I24H2Y	Maximum 24-hour precipitation that occurs on average once in 2 years - Equivalent to precipitation intensity index	1.92	inches
I6H2Y	Maximum 6-hour precipitation that occurs on average once in 2 years	1.38	
LAT_OUT	Latitude of Basin Outlet	4325125	degrees
LC11BARE	Percentage of barren from NLCD 2011 class 31	0	
LC11CRPHAY	Percentage of cultivated crops and hay, classes 81 and 82, from NLCD 2011	0	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	3.1	percent
LC11FOREST	Percentage of forest from NLCD 2011 classes 41-43	2.1	percent
LC11GRASS	Percent of area covered by grassland/herbaceous using 2011 NLCD	76.1	
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	3.1	percent
LC11SHRUB	Percent of area covered by shrubland using 2011 NLCD	18.7	
LC11SNOIC	Percent snow and ice from NLCD 2011 class 12	0	
LC11WATER	Percent of open water, class 11, from NLCD 2011	0	
LC11WETLND	Percentage of wetlands, classes 90 and 95, from NLCD 2011	0	
LONG_OUT	Longitude of Basin Outlet	525705	degrees
MINBELEV	Minimum basin elevation	7410	feet
PRECIP	Mean Annual Precipitation	20.83	inches
SSURGOA	Percentage of area of Hydrologic Soil Type A from SSURGO	0	percent
SSURGOB	Percentage of area of Hydrologic Soil Type B from SSURGO	91.2	percent
SSURGOC	Percentage of area of Hydrologic Soil Type C from SSURGO	7.1	percent
SSURGOD	Percentage of area of Hydrologic Soil Type D from SSURGO	1.65	percent
STORNHD	Percent storage (wetlands and waterbodies) determined from 1:24K NHD	0.9	percent

General Disclaimers

Upstream regulation was checked for this watershed.

Peak-Flow Statistics Parameters [Foothills Region Peak Flow 2016 5099]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	2.96	square miles	0.6	2850
16H100Y	6 Hour 100 Year Precipitation	3.84	inches	2.38	4.89
STATSCLAY	STATSGO Percentage of Clay Soils	16.3	percent	9.87	37.5
OUTLETELEV	Elevation of Gage	7416	feet	4290	8270

Peak-Flow Statistics Flow Report [Foothills Region Peak Flow 2016 5099]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

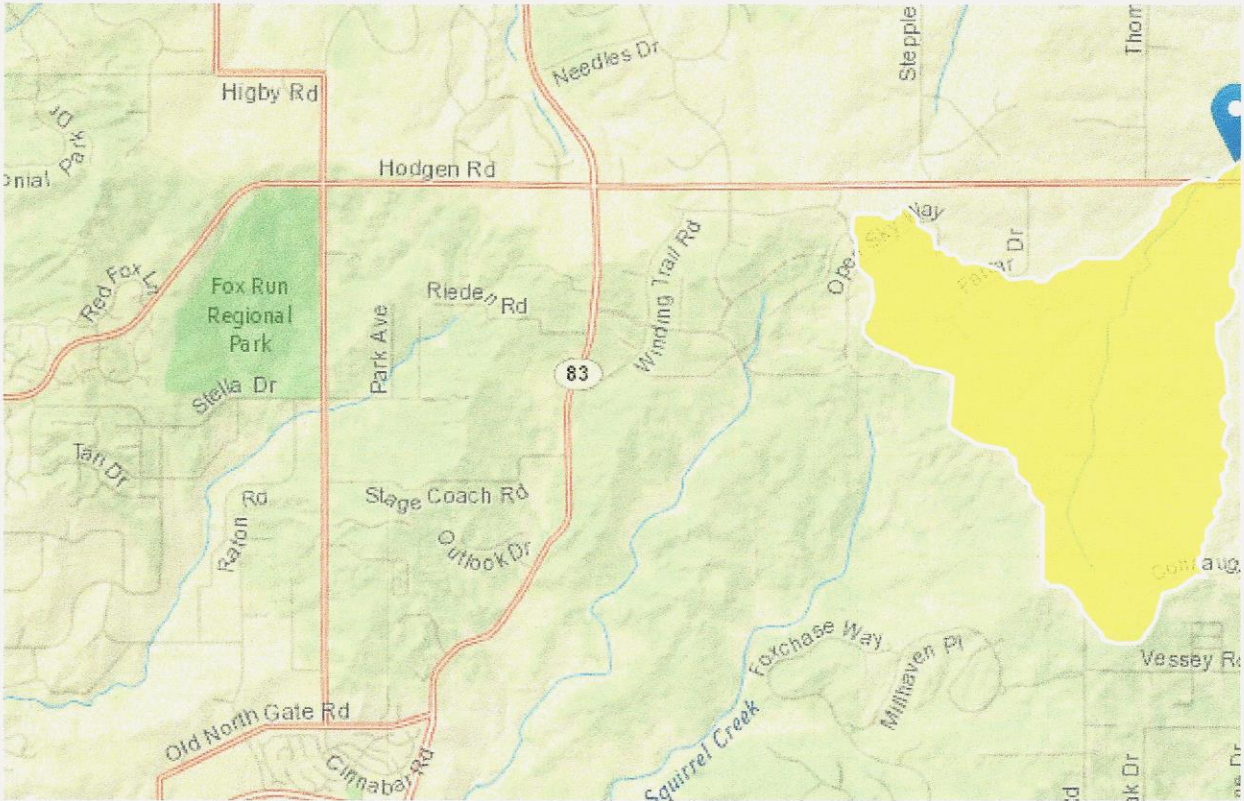
Statistic	Value	Unit	SEp
2 Year Peak Flood	22	ft ³ /s	117
5 Year Peak Flood	61.8	ft ³ /s	87
10 Year Peak Flood	104	ft ³ /s	80
25 Year Peak Flood	181	ft ³ /s	80
50 Year Peak Flood	257	ft ³ /s	83
100 Year Peak Flood	357	ft ³ /s	88
200 Year Peak Flood	474	ft ³ /s	94
500 Year Peak Flood	661	ft ³ /s	104

Peak-Flow Statistics Citations

Kohn, M.S., Stevens, M.R., Harden, T.M., Godaire, J.E., Klinger, R.E., and Mommandi, A., 2016, Paleoflood investigations to improve peak-streamflow regional-regression equations for natural streamflow in eastern Colorado, 2015: U.S. Geological Survey Scientific Investigations Report 2016-5099, 58 p. (<http://dx.doi.org/10.3133/sir20165099>)

SAVAGE PROJECT - StreamStats Report2

Region ID: CO
 Workspace ID: C020180218174250826000
 Clicked Point (Latitude, Longitude): 39.07232, -104.70650
 Time: 2018-02-18 10:43:06 -0700



Design Point Above Minor West Trib

Basin Characteristics

Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	2.14	square miles
16H100Y	6-hour precipitation that is expected to occur on average once in 100 years	3.84	inches
STATSCLAY	Percentage of clay soils from STATSGO	16.3	percent
OUTLETELEV	Elevation of the stream outlet in thousands of feet above NAVD88.	7428	feet
BSLDEM10M	Mean basin slope computed from 10 m DEM	6.5	percent

Parameter Code	Parameter Description	Value	Unit
CSL1085LFP	Change in elevation divided by length between points 10 and 85 percent of distance along the longest flow path to the basin divide, LFP from 2D grid	68.1	feet per mi
EL7500	Percent of area above 7500 ft	88	percent
ELEV	Mean Basin Elevation	7564	feet
ELEVMAX	Maximum basin elevation	7690	feet
I24H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	5	inches
I24H2Y	Maximum 24-hour precipitation that occurs on average once in 2 years - Equivalent to precipitation intensity index	1.92	inches
I6H2Y	Maximum 6-hour precipitation that occurs on average once in 2 years	1.38	
LAT_OUT	Latitude of Basin Outlet	4324845	degrees
LC11BARE	Percentage of barren from NLCD 2011 class 31	0	
LC11CRPHAY	Percentage of cultivated crops and hay, classes 81 and 82, from NLCD 2011	0	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	1.7	percent
LC11FOREST	Percentage of forest from NLCD 2011 classes 41-43	3	percent
LC11GRASS	Percent of area covered by grassland/herbaceous using 2011 NLCD	73.4	
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	1.7	percent
LC11SHRUB	Percent of area covered by shrubland using 2011 NLCD	22	
LC11SNOIC	Percent snow and ice from NLCD 2011 class 12	0	
LC11WATER	Percent of open water, class 11, from NLCD 2011	0	
LC11WETLND	Percentage of wetlands, classes 90 and 95, from NLCD 2011	0	
LFPLENGTH	Length of longest flow path	3.09	miles
LONG_OUT	Longitude of Basin Outlet	525385	degrees
MINBELEV	Minimum basin elevation	7430	feet
PRECIP	Mean Annual Precipitation	20.73	inches
RCN	Runoff-curve number as defined by NRCS (http://policy.nrcs.usda.gov/OpenNonWebContent.aspx?content=17758.wba)	61.09	
RUNCO_CO	Soil runoff coefficient as defined by Verdin and Gross (2017)	0.25	
SSURGOA	Percentage of area of Hydrologic Soil Type A from SSURGO	0	percent
SSURGOB	Percentage of area of Hydrologic Soil Type B from SSURGO	92	percent
SSURGOC	Percentage of area of Hydrologic Soil Type C from SSURGO	6.04	percent
SSURGOD	Percentage of area of Hydrologic Soil Type D from SSURGO	1.93	percent
STORNHD	Percent storage (wetlands and waterbodies) determined from 1:24K NHD	1.1	percent

Parameter Code	Parameter Description	Value	Unit
TOC	Time of concentration in hours	3.27	

Peak-Flow Statistics Parameters [Foothills Region Peak Flow 2016 5099]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	2.14	square miles	0.6	2850
16H100Y	6 Hour 100 Year Precipitation	3.84	inches	2.38	4.89
STATSCLAY	STATSGO Percentage of Clay Soils	16.3	percent	9.87	37.5
OUTLETELEV	Elevation of Gage	7428	feet	4290	8270

Peak-Flow Statistics Flow Report [Foothills Region Peak Flow 2016 5099]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

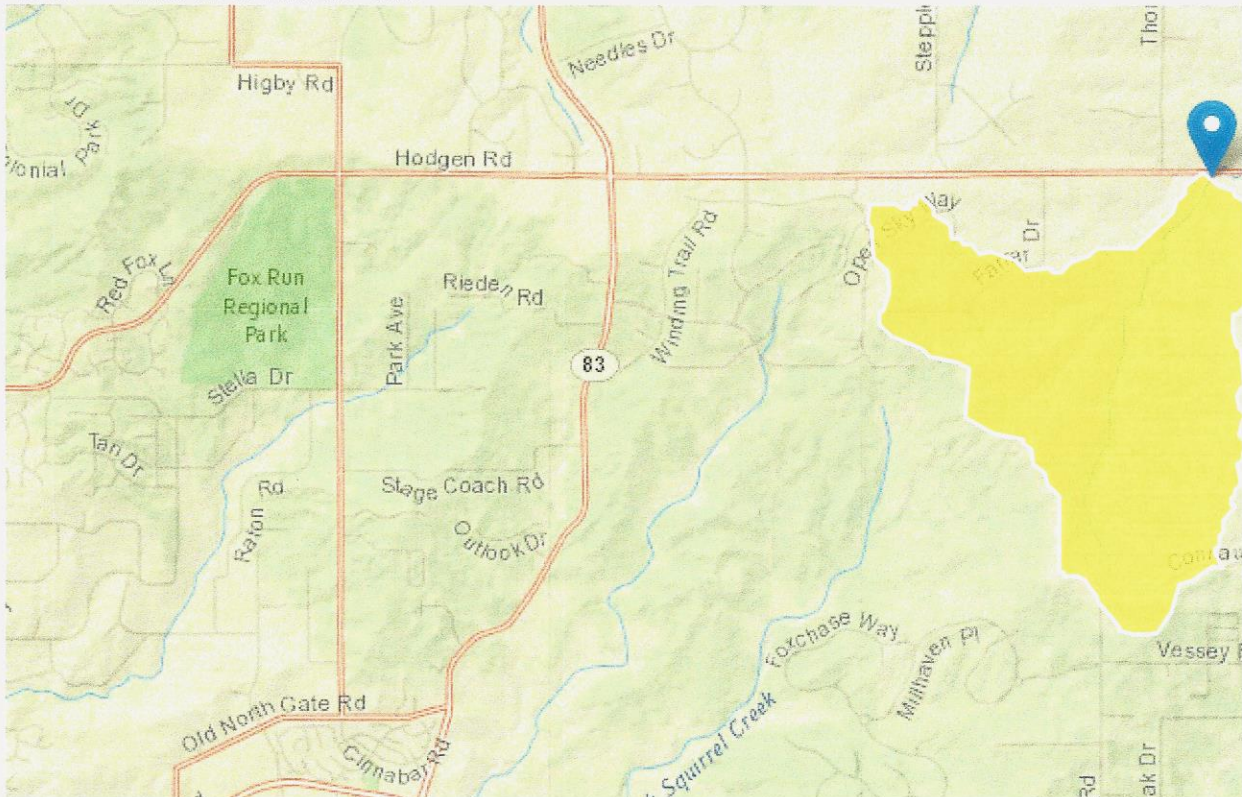
Statistic	Value	Unit	SEp
2 Year Peak Flood	17.9	ft ³ /s	117
5 Year Peak Flood	50.7	ft ³ /s	87
10 Year Peak Flood	85.9	ft ³ /s	80
25 Year Peak Flood	149	ft ³ /s	80
50 Year Peak Flood	212	ft ³ /s	83
100 Year Peak Flood	295	ft ³ /s	88
200 Year Peak Flood	391	ft ³ /s	94
500 Year Peak Flood	547	ft ³ /s	104

Peak-Flow Statistics Citations

Kohn, M.S., Stevens, M.R., Harden, T.M., Godaire, J.E., Klinger, R.E., and Mommandi, A., 2016, Paleoflood investigations to improve peak-streamflow regional-regression equations for natural streamflow in eastern Colorado, 2015: U.S. Geological Survey Scientific Investigations Report 2016-5099, 58 p. (<http://dx.doi.org/10.3133/sir20165099>)

SAVAGE PROJECT - StreamStats Report3

Region ID: CO
 Workspace ID: C020180218175120980000
 Clicked Point (Latitude, Longitude): 39.07058, -104.71005
 Time: 2018-02-18 10:51:39 -0700



Design Point Above Hodgen

Basin Characteristics			
Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	2.07	square miles
16H100Y	6-hour precipitation that is expected to occur on average once in 100 years	3.84	inches
STATSCLAY	Percentage of clay soils from STATSGO	16.3	percent
OUTLETELEV	Elevation of the stream outlet in thousands of feet above NAVD88.	7445	feet
BSLDEM10M	Mean basin slope computed from 10 m DEM	6.52	percent

Parameter Code	Parameter Description	Value	Unit
CSL1085LFP	Change in elevation divided by length between points 10 and 85 percent of distance along the longest flow path to the basin divide, LFP from 2D grid	73.1	feet per mi
EL7500	Percent of area above 7500 ft	90	percent
ELEV	Mean Basin Elevation	7568	feet
ELEVMAX	Maximum basin elevation	7690	feet
I24H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	5	inches
I24H2Y	Maximum 24-hour precipitation that occurs on average once in 2 years - Equivalent to precipitation intensity index	1.92	inches
I6H2Y	Maximum 6-hour precipitation that occurs on average once in 2 years	1.38	
LAT_OUT	Latitude of Basin Outlet	4324655	degrees
LC11BARE	Percentage of barren from NLCD 2011 class 31	0	
LC11CRPHAY	Percentage of cultivated crops and hay, classes 81 and 82, from NLCD 2011	0	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	1.1	percent
LC11FOREST	Percentage of forest from NLCD 2011 classes 41-43	3.1	percent
LC11GRASS	Percent of area covered by grassland/herbaceous using 2011 NLCD	74	
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	1.2	percent
LC11SHRUB	Percent of area covered by shrubland using 2011 NLCD	21.7	
LC11SNOIC	Percent snow and ice from NLCD 2011 class 12	0	
LC11WATER	Percent of open water, class 11, from NLCD 2011	0	
LC11WETLND	Percentage of wetlands, classes 90 and 95, from NLCD 2011	0	
LFPLENGTH	Length of longest flow path	2.79	miles
LONG_OUT	Longitude of Basin Outlet	525085	degrees
MINBELEV	Minimum basin elevation	7440	feet
PRECIP	Mean Annual Precipitation	20.73	inches
RCN	Runoff-curve number as defined by NRCS (http://policy.nrcs.usda.gov/OpenNonWebContent.aspx?content=17758.wba)	61.04	
RUNCO_CO	Soil runoff coefficient as defined by Verdin and Gross (2017)	0.25	
SSURGOA	Percentage of area of Hydrologic Soil Type A from SSURGO	0	percent
SSURGOB	Percentage of area of Hydrologic Soil Type B from SSURGO	92.2	percent
SSURGOC	Percentage of area of Hydrologic Soil Type C from SSURGO	6.25	percent
SSURGOD	Percentage of area of Hydrologic Soil Type D from SSURGO	1.5	percent
STORNHD	Percent storage (wetlands and waterbodies) determined from 1:24K NHD	1.2	percent

Parameter Code	Parameter Description	Value	Unit
TOC	Time of concentration in hours	3.01	

Peak-Flow Statistics Parameters [Foothills Region Peak Flow 2016 5099]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	2.07	square miles	0.6	2850
I6H100Y	6 Hour 100 Year Precipitation	3.84	inches	2.38	4.89
STATSCLAY	STATSGO Percentage of Clay Soils	16.3	percent	9.87	37.5
OUTLETELEV	Elevation of Gage	7445	feet	4290	8270

Peak-Flow Statistics Flow Report [Foothills Region Peak Flow 2016 5099]

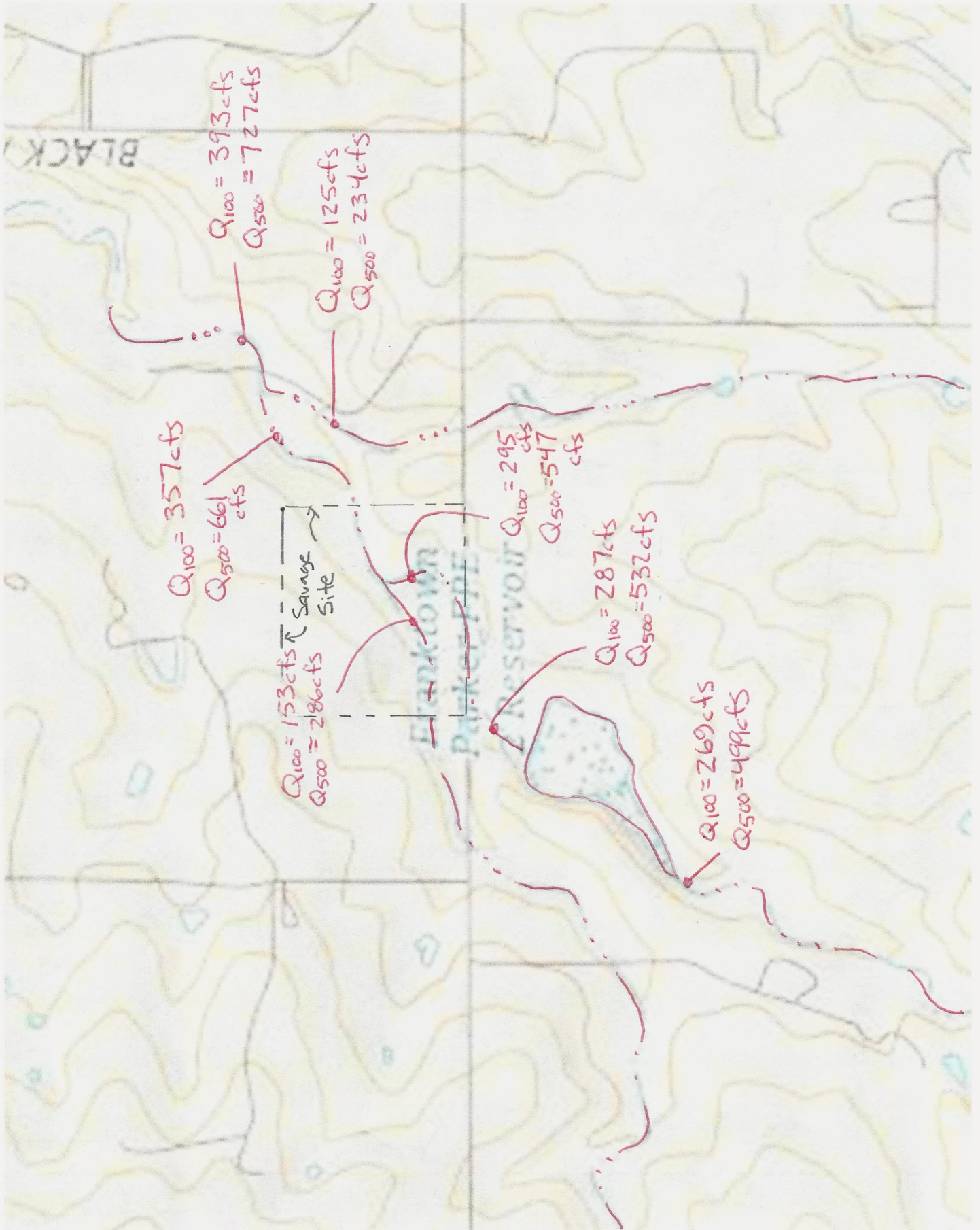
PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	SEp
2 Year Peak Flood	17.4	ft ³ /s	117
5 Year Peak Flood	49.4	ft ³ /s	87
10 Year Peak Flood	83.6	ft ³ /s	80
25 Year Peak Flood	145	ft ³ /s	80
50 Year Peak Flood	207	ft ³ /s	83
100 Year Peak Flood	287	ft ³ /s	88
200 Year Peak Flood	381	ft ³ /s	94
500 Year Peak Flood	532	ft ³ /s	104

Peak-Flow Statistics Citations

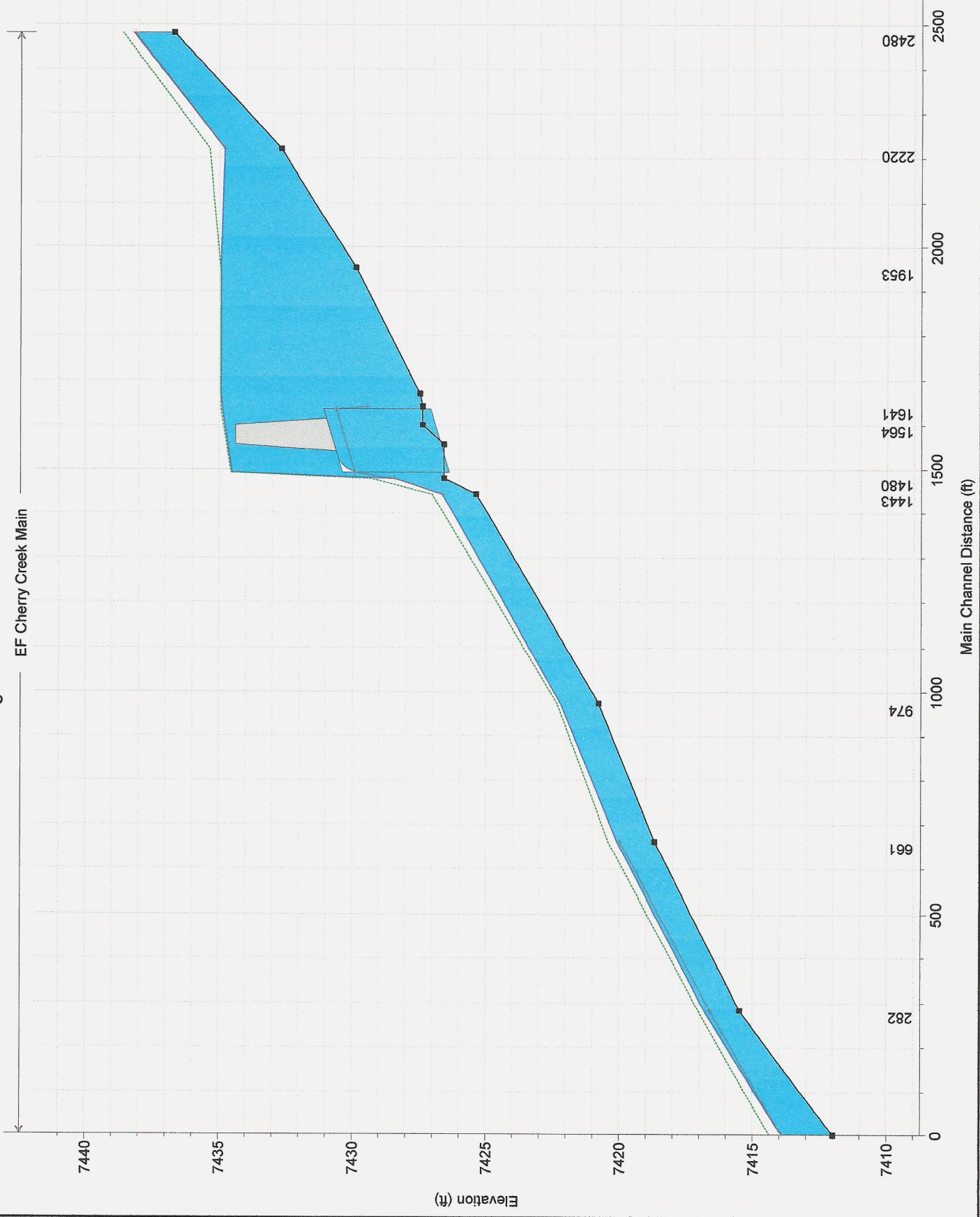
Kohn, M.S., Stevens, M.R., Harden, T.M., Godaire, J.E., Klinger, R.E., and Mommandi, A., 2016, Paleoflood investigations to improve peak-streamflow regional-regression equations for natural streamflow in eastern Colorado, 2015: U.S. Geological Survey Scientific Investigations Report 2016-5099, 58 p. (<http://dx.doi.org/10.3133/sir20165099>)

North
1" = 800'



Savage Plan: 100Yr Subcritical 3/25/2018
 EF Cherry Creek Main

Legend	
EG PF 1	(Dashed line)
WS PF 1	(Solid blue line)
Crit PF 1	(Dotted line)
Ground	(Black line with square markers)



1 in Horiz. = 300 ft 1 in Vert. = 5 ft

HEC-RAS Plan: 100Yr sub River: EF Cherry Creek Reach: Main Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Main	2480	PF 1	287.00	7436.70	7438.21	7438.15	7438.61	0.011766	5.11	56.19	60.48	0.93
Main	2220	PF 1	287.00	7432.70	7434.81	7434.81	7435.37	0.013130	6.03	47.62	43.24	1.01
Main	1953	PF 1	287.00	7429.90	7434.94		7434.95	0.000035	0.78	588.61	301.20	0.07
Main	1669	PF 1	287.00	7427.50	7434.93		7434.94	0.000014	0.64	552.90	136.25	0.04
Main	1641	PF 1	287.00	7427.40	7434.87	7429.44	7434.93	0.000114	1.99	144.55	195.32	0.13
Main	1564		Culvert									
Main	1480	PF 1	287.00	7426.60	7428.44	7428.44	7429.35	0.010743	7.62	37.67	148.77	1.00
Main	1443	PF 1	295.00	7425.40	7426.65	7426.65	7427.05	0.014357	5.04	58.55	74.76	1.00
Main	974	PF 1	295.00	7420.80	7422.21		7422.34	0.003321	2.93	100.89	97.83	0.51
Main	661	PF 1	357.00	7418.70	7420.09	7419.98	7420.42	0.012088	4.57	78.03	101.03	0.92
Main	282	PF 1	357.00	7415.50	7416.81	7416.59	7417.03	0.006782	3.77	94.57	106.09	0.70
Main	0	PF 1	357.00	7412.00	7413.90	7413.90	7414.35	0.013968	5.39	66.28	74.89	1.01

Plan: 100Yr sub EF Cherry Creek Main RS: 2480 Profile: PF 1

E.G. Elev (ft)	7438.61	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.41	Wt. n-Val.		0.030	
W.S. Elev (ft)	7438.21	Reach Len. (ft)	305.00	260.00	132.00
Crit W.S. (ft)	7438.15	Flow Area (sq ft)		56.19	
E.G. Slope (ft/ft)	0.011766	Area (sq ft)		56.19	
Q Total (cfs)	287.00	Flow (cfs)		287.00	
Top Width (ft)	60.48	Top Width (ft)		60.48	
Vel Total (ft/s)	5.11	Avg. Vel. (ft/s)		5.11	
Max Chl Dpth (ft)	1.51	Hydr. Depth (ft)		0.93	
Conv. Total (cfs)	2645.9	Conv. (cfs)		2645.9	
Length Wtd. (ft)	260.00	Wetted Per. (ft)		60.61	
Min Ch El (ft)	7436.70	Shear (lb/sq ft)		0.68	
Alpha	1.00	Stream Power (lb/ft s)		3.48	
Frctn Loss (ft)	3.23	Cum Volume (acre-ft)	1.49	6.71	0.99
C & E Loss (ft)	0.02	Cum SA (acres)	1.21	4.51	0.97

Plan: 100Yr sub EF Cherry Creek Main RS: 2220 Profile: PF 1

E.G. Elev (ft)	7435.37	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.56	Wt. n-Val.		0.030	
W.S. Elev (ft)	7434.81	Reach Len. (ft)	249.00	267.00	235.00
Crit W.S. (ft)	7434.81	Flow Area (sq ft)		47.62	
E.G. Slope (ft/ft)	0.013130	Area (sq ft)		47.62	
Q Total (cfs)	287.00	Flow (cfs)		287.00	
Top Width (ft)	43.24	Top Width (ft)		43.24	
Vel Total (ft/s)	6.03	Avg. Vel. (ft/s)		6.03	
Max Chl Dpth (ft)	2.11	Hydr. Depth (ft)		1.10	
Conv. Total (cfs)	2504.7	Conv. (cfs)		2504.7	
Length Wtd. (ft)	264.11	Wetted Per. (ft)		43.51	
Min Ch El (ft)	7432.70	Shear (lb/sq ft)		0.90	
Alpha	1.00	Stream Power (lb/ft s)		5.41	
Frctn Loss (ft)	0.03	Cum Volume (acre-ft)	1.49	6.40	0.99
C & E Loss (ft)	0.17	Cum SA (acres)	1.21	4.20	0.97

Plan: 100Yr sub EF Cherry Creek Main RS: 1953 Profile: PF 1

E.G. Elev (ft)	7434.95	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.01	Wt. n-Val.	0.045	0.030	0.045
W.S. Elev (ft)	7434.94	Reach Len. (ft)	213.00	284.00	321.00
Crit W.S. (ft)		Flow Area (sq ft)	220.79	271.19	96.63
E.G. Slope (ft/ft)	0.000035	Area (sq ft)	220.79	271.19	96.63
Q Total (cfs)	287.00	Flow (cfs)	57.10	210.24	19.66
Top Width (ft)	301.20	Top Width (ft)	146.09	63.40	91.71
Vel Total (ft/s)	0.49	Avg. Vel. (ft/s)	0.26	0.78	0.20
Max Chl Dpth (ft)	5.04	Hydr. Depth (ft)	1.51	4.28	1.05
Conv. Total (cfs)	48194.0	Conv. (cfs)	9588.9	35303.4	3301.7
Length Wtd. (ft)	276.05	Wetted Per. (ft)	146.38	63.65	91.80
Min Ch El (ft)	7429.90	Shear (lb/sq ft)	0.00	0.01	0.00
Alpha	1.92	Stream Power (lb/ft s)	0.00	0.01	0.00
Frctn Loss (ft)	0.01	Cum Volume (acre-ft)	0.86	5.43	0.73
C & E Loss (ft)	0.00	Cum SA (acres)	0.79	3.87	0.72

Plan: 100Yr sub EF Cherry Creek Main RS: 1669 Profile: PF 1

E.G. Elev (ft)	7434.94	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.01	Wt. n-Val.	0.045	0.030	0.045
W.S. Elev (ft)	7434.93	Reach Len. (ft)	23.00	28.00	62.00
Crit W.S. (ft)		Flow Area (sq ft)	104.14	400.16	48.60
E.G. Slope (ft/ft)	0.000014	Area (sq ft)	104.14	400.16	48.60
Q Total (cfs)	287.00	Flow (cfs)	21.63	257.33	8.04
Top Width (ft)	136.25	Top Width (ft)	46.00	59.90	30.35
Vel Total (ft/s)	0.52	Avg. Vel. (ft/s)	0.21	0.64	0.17

Plan: 100Yr sub EF Cherry Creek Main RS: 1669 Profile: PF 1 (Continued)

Max Chl Dpth (ft)	7.43	Hydr. Depth (ft)	2.26	6.68	1.60
Conv. Total (cfs)	77909.3	Conv. (cfs)	5872.4	69853.9	2183.1
Length Wtd. (ft)	28.29	Wetted Per. (ft)	46.67	60.48	30.63
Min Ch El (ft)	7427.50	Shear (lb/sq ft)	0.00	0.01	0.00
Alpha	1.39	Stream Power (lb/ft s)	0.00	0.00	0.00
Frctn Loss (ft)	0.00	Cum Volume (acre-ft)	0.06	3.24	0.19
C & E Loss (ft)	0.01	Cum SA (acres)	0.32	3.47	0.27

Plan: 100Yr sub EF Cherry Creek Main RS: 1641 Profile: PF 1

E.G. Elev (ft)	7434.93	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.06	Wt. n-Val.		0.030	
W.S. Elev (ft)	7434.87	Reach Len. (ft)	202.00	161.00	164.00
Crit W.S. (ft)	7429.44	Flow Area (sq ft)		144.55	
E.G. Slope (ft/ft)	0.000114	Area (sq ft)	92.06	387.48	221.85
Q Total (cfs)	287.00	Flow (cfs)		287.00	
Top Width (ft)	195.32	Top Width (ft)	49.40	57.90	88.02
Vel Total (ft/s)	1.99	Avg. Vel. (ft/s)		1.99	
Max Chl Dpth (ft)	7.47	Hydr. Depth (ft)		7.30	
Conv. Total (cfs)	26922.6	Conv. (cfs)		26922.6	
Length Wtd. (ft)	161.00	Wetted Per. (ft)		19.82	
Min Ch El (ft)	7427.40	Shear (lb/sq ft)		0.05	
Alpha	1.00	Stream Power (lb/ft s)		0.10	
Frctn Loss (ft)		Cum Volume (acre-ft)	0.01	2.98	0.00
C & E Loss (ft)		Cum SA (acres)	0.29	3.43	0.19

Plan: 100Yr sub EF Cherry Creek Main RS: 1480 Profile: PF 1

E.G. Elev (ft)	7429.35	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.90	Wt. n-Val.		0.030	
W.S. Elev (ft)	7428.44	Reach Len. (ft)	23.00	37.00	62.00
Crit W.S. (ft)	7428.44	Flow Area (sq ft)		37.67	
E.G. Slope (ft/ft)	0.010743	Area (sq ft)	38.63	117.45	2.22
Q Total (cfs)	287.00	Flow (cfs)		287.00	
Top Width (ft)	148.77	Top Width (ft)	68.91	74.59	5.27
Vel Total (ft/s)	7.62	Avg. Vel. (ft/s)		7.62	
Max Chl Dpth (ft)	1.84	Hydr. Depth (ft)		1.81	
Conv. Total (cfs)	2769.0	Conv. (cfs)		2769.0	
Length Wtd. (ft)	37.00	Wetted Per. (ft)		20.84	
Min Ch El (ft)	7426.60	Shear (lb/sq ft)		1.21	
Alpha	1.00	Stream Power (lb/ft s)		9.24	
Frctn Loss (ft)	0.46	Cum Volume (acre-ft)	0.01	2.85	0.00
C & E Loss (ft)	0.15	Cum SA (acres)	0.02	3.19	0.01

Plan: 100Yr sub EF Cherry Creek Main RS: 1443 Profile: PF 1

E.G. Elev (ft)	7427.05	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.39	Wt. n-Val.		0.030	
W.S. Elev (ft)	7426.65	Reach Len. (ft)	268.00	469.00	453.00
Crit W.S. (ft)	7426.65	Flow Area (sq ft)		58.55	
E.G. Slope (ft/ft)	0.014357	Area (sq ft)		58.55	
Q Total (cfs)	295.00	Flow (cfs)		295.00	
Top Width (ft)	74.76	Top Width (ft)		74.76	
Vel Total (ft/s)	5.04	Avg. Vel. (ft/s)		5.04	
Max Chl Dpth (ft)	1.25	Hydr. Depth (ft)		0.78	
Conv. Total (cfs)	2462.0	Conv. (cfs)		2462.0	
Length Wtd. (ft)	469.00	Wetted Per. (ft)		74.86	
Min Ch El (ft)	7425.40	Shear (lb/sq ft)		0.70	
Alpha	1.00	Stream Power (lb/ft s)		3.53	
Frctn Loss (ft)	2.84	Cum Volume (acre-ft)	0.00	2.77	0.00
C & E Loss (ft)	0.08	Cum SA (acres)	0.00	3.12	0.01

Plan: 100Yr sub EF Cherry Creek Main RS: 974 Profile: PF 1

E.G. Elev (ft)	7422.34	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.13	Wt. n-Val.		0.030	0.045
W.S. Elev (ft)	7422.21	Reach Len. (ft)	278.00	313.00	173.00
Crit W.S. (ft)		Flow Area (sq ft)		100.81	0.08
E.G. Slope (ft/ft)	0.003321	Area (sq ft)		100.81	0.08
Q Total (cfs)	295.00	Flow (cfs)		294.97	0.03
Top Width (ft)	97.83	Top Width (ft)		97.04	0.79
Vel Total (ft/s)	2.92	Avg. Vel. (ft/s)		2.93	0.42
Max Chl Dpth (ft)	1.41	Hydr. Depth (ft)		1.04	0.11
Conv. Total (cfs)	5118.7	Conv. (cfs)		5118.1	0.6
Length Wtd. (ft)	312.99	Wetted Per. (ft)		97.14	0.82
Min Ch El (ft)	7420.80	Shear (lb/sq ft)		0.22	0.02
Alpha	1.00	Stream Power (lb/ft s)		0.63	0.01
Frctn Loss (ft)	1.90	Cum Volume (acre-ft)	0.00	1.91	0.00
C & E Loss (ft)	0.02	Cum SA (acres)	0.00	2.20	0.00

Plan: 100Yr sub EF Cherry Creek Main RS: 661 Profile: PF 1

E.G. Elev (ft)	7420.42	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.33	Wt. n-Val.		0.030	
W.S. Elev (ft)	7420.09	Reach Len. (ft)	390.00	379.00	189.00
Crit W.S. (ft)	7419.98	Flow Area (sq ft)		78.03	
E.G. Slope (ft/ft)	0.012088	Area (sq ft)		78.03	
Q Total (cfs)	357.00	Flow (cfs)		357.00	
Top Width (ft)	101.03	Top Width (ft)		101.03	
Vel Total (ft/s)	4.57	Avg. Vel. (ft/s)		4.57	
Max Chl Dpth (ft)	1.39	Hydr. Depth (ft)		0.77	
Conv. Total (cfs)	3247.1	Conv. (cfs)		3247.1	
Length Wtd. (ft)	379.00	Wetted Per. (ft)		101.34	
Min Ch El (ft)	7418.70	Shear (lb/sq ft)		0.58	
Alpha	1.00	Stream Power (lb/ft s)		2.66	
Frctn Loss (ft)	3.36	Cum Volume (acre-ft)	0.00	1.27	
C & E Loss (ft)	0.03	Cum SA (acres)	0.00	1.49	

Plan: 100Yr sub EF Cherry Creek Main RS: 282 Profile: PF 1

E.G. Elev (ft)	7417.03	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.22	Wt. n-Val.		0.030	
W.S. Elev (ft)	7416.81	Reach Len. (ft)	294.00	282.00	256.00
Crit W.S. (ft)	7416.59	Flow Area (sq ft)		94.57	
E.G. Slope (ft/ft)	0.006782	Area (sq ft)	0.00	94.57	
Q Total (cfs)	357.00	Flow (cfs)		357.00	
Top Width (ft)	106.09	Top Width (ft)	0.03	106.06	
Vel Total (ft/s)	3.77	Avg. Vel. (ft/s)		3.77	
Max Chl Dpth (ft)	1.31	Hydr. Depth (ft)		0.89	
Conv. Total (cfs)	4334.9	Conv. (cfs)		4334.9	
Length Wtd. (ft)	282.00	Wetted Per. (ft)		106.24	
Min Ch El (ft)	7415.50	Shear (lb/sq ft)		0.38	
Alpha	1.00	Stream Power (lb/ft s)		1.42	
Frctn Loss (ft)	2.66	Cum Volume (acre-ft)	0.00	0.52	
C & E Loss (ft)	0.02	Cum SA (acres)	0.00	0.59	

Plan: 100Yr sub EF Cherry Creek Main RS: 0 Profile: PF 1

E.G. Elev (ft)	7414.35	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.45	Wt. n-Val.		0.030	
W.S. Elev (ft)	7413.90	Reach Len. (ft)			
Crit W.S. (ft)	7413.90	Flow Area (sq ft)		66.28	
E.G. Slope (ft/ft)	0.013968	Area (sq ft)		66.28	
Q Total (cfs)	357.00	Flow (cfs)		357.00	
Top Width (ft)	74.89	Top Width (ft)		74.89	
Vel Total (ft/s)	5.39	Avg. Vel. (ft/s)		5.39	

Plan: 100Yr sub EF Cherry Creek Main RS: 0 Profile: PF 1 (Continued)

Max Chl Dpth (ft)	1.90	Hydr. Depth (ft)		0.89	
Conv. Total (cfs)	3020.6	Conv. (cfs)		3020.6	
Length Wtd. (ft)		Wetted Per. (ft)		75.09	
Min Ch El (ft)	7412.00	Shear (lb/sq ft)		0.77	
Alpha	1.00	Stream Power (lb/ft s)		4.15	
Frctn Loss (ft)		Cum Volume (acre-ft)			
C & E Loss (ft)		Cum SA (acres)			

Plan: 100Yr sub EF Cherry Creek Main RS: 1564 Culv Group: 48inch Dual Profile: PF 1

Q Culv Group (cfs)	281.57	Culv Full Len (ft)	114.75
# Barrels	2	Culv Vel US (ft/s)	11.20
Q Barrel (cfs)	140.78	Culv Vel DS (ft/s)	12.03
E.G. US. (ft)	7434.93	Culv Inv El Up (ft)	7427.10
W.S. US. (ft)	7434.87	Culv Inv El Dn (ft)	7426.40
E.G. DS (ft)	7429.35	Culv Frctn Ls (ft)	1.47
W.S. DS (ft)	7428.44	Culv Exit Loss (ft)	2.82
Delta EG (ft)	5.59	Culv Entr Loss (ft)	0.98
Delta WS (ft)	6.43	Q Weir (cfs)	5.43
E.G. IC (ft)	7433.86	Weir Sta Lft (ft)	170.13
E.G. OC (ft)	7434.93	Weir Sta Rgt (ft)	189.93
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	7431.10	Weir Max Depth (ft)	0.26
Culv WS Outlet (ft)	7429.92	Weir Avg Depth (ft)	0.22
Culv Nml Depth (ft)	4.00	Weir Flow Area (sq ft)	4.42
Culv Crt Depth (ft)	3.52	Min El Weir Flow (ft)	7434.68



APPROXIMATE SCALE IN FEET
 2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

**FIRM
 FLOOD INSURANCE RATE MAP
 EL PASO COUNTY,
 COLORADO AND
 INCORPORATED AREAS**

PANEL 325 OF 1300

(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:
 COMMUNITY

NUMBER PANEL SUFFIX

EL PASO COUNTY UNINCORPORATED AREAS 080569 0325 F

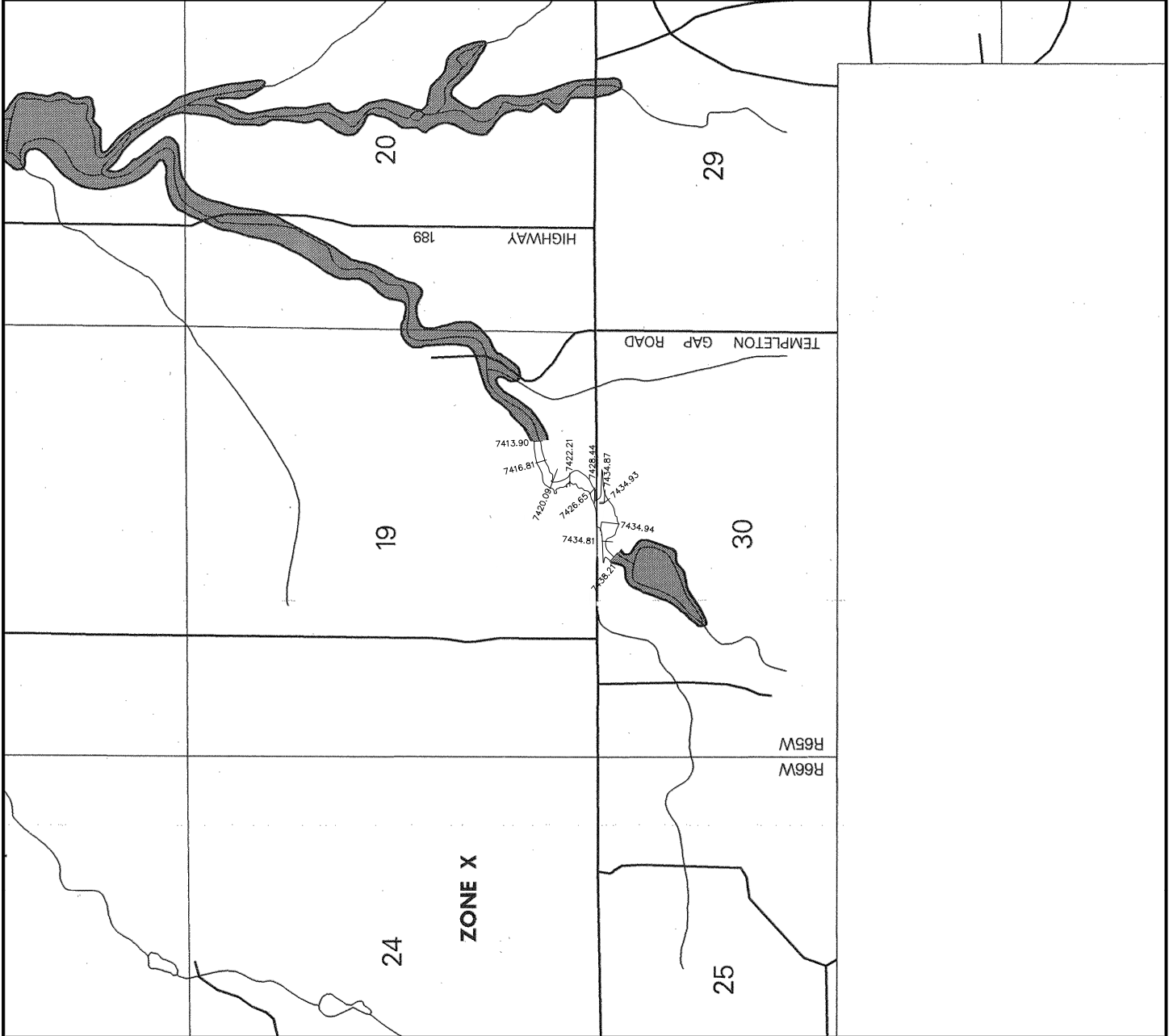
**MAP NUMBER
 08041C0325 F**

**EFFECTIVE DATE:
 MARCH 17, 1997**



Federal Emergency Management Agency

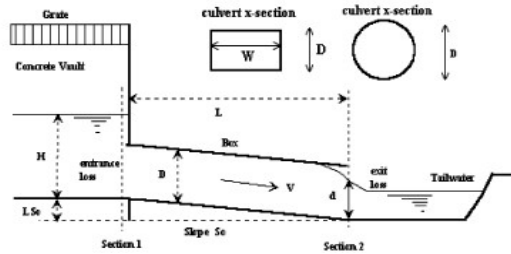
This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov



HYDRAULIC CALCULATIONS

CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: **High Plains Filing no. 1**
 Basin ID: **Design Point 1**
 Status: _____



Design Information (Input):

Circular Culvert: Barrel Diameter in Inches
 Inlet Edge Type (choose from pull-down list)

D = inches
 Grooved End Projection

OR:

Box Culvert: Barrel Height (Rise) in Feet
 Barrel Width (Span) in Feet
 Inlet Edge Type (choose from pull-down list)

Height (Rise) =
 Width (Span) =
 Square Edge w/ 30-78 deg. Flared Wingwall

Number of Barrels
 Inlet Elevation at Culvert Invert
 Outlet Elevation at Culvert Invert OR Slope of Culvert (ft v./ft h.)
 Culvert Length in Feet
 Manning's Roughness
 Bend Loss Coefficient
 Exit Loss Coefficient

No =
 Inlet Elev = ft. elev.
 Outlet Elev = ft. elev.
 L = ft.
 n =
 K_b =
 K_x =

Design Information (calculated):

Entrance Loss Coefficient
 Friction Loss Coefficient
 Sum of All Loss Coefficients
 Orifice Inlet Condition Coefficient
 Minimum Energy Condition Coefficient

K_e =
 K_f =
 K_s =
 C_d =
 KE_{low} =

1. Culvert P&P is missing in the construction drawings. Culvert calculation will be reviewed with the resubmittal.
2. Submit the outlet protection calculation
3. Provide the Hw/D. Per DCM Hw/D must be less than 1.5.
4. Provide the culvert calculation for the proposed 30" RCP.

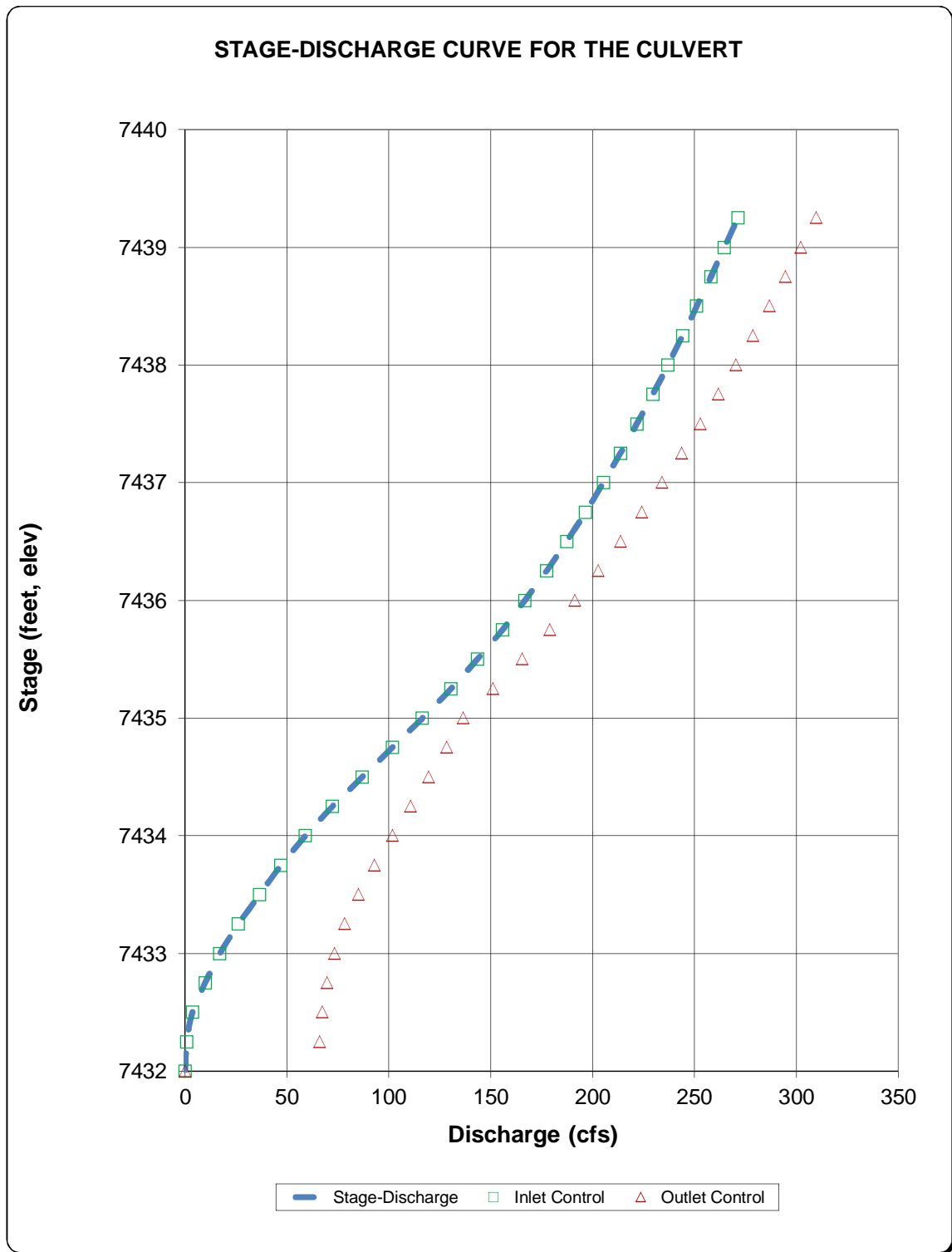
Calculations of Culvert Capacity (output):

Water Surface Elevation (ft., linked)	Tailwater Surface Elevation ft	Culvert Inlet-Control Flowrate cfs	Culvert Outlet-Control Flowrate cfs	Controlling Culvert Flowrate cfs (output)	Inlet Equation Used:	Flow Control Used
7432.00		0.00	0.00	0.00	No Flow (WS < inlet)	N/A
7432.25		0.90	65.95	0.90	Min. Energy Eqn.	INLET
7432.50		3.60	67.32	3.60	Min. Energy Eqn.	INLET
7432.75		9.90	69.78	9.90	Min. Energy Eqn.	INLET
7433.00		17.10	73.33	17.10	Min. Energy Eqn.	INLET
7433.25		26.10	78.38	26.10	Min. Energy Eqn.	INLET
7433.50		36.60	84.94	36.60	Min. Energy Eqn.	INLET
7433.75		47.10	92.99	47.10	Regression Eqn.	INLET
7434.00		59.10	101.73	59.10	Regression Eqn.	INLET
7434.25		72.30	110.74	72.30	Regression Eqn.	INLET
7434.50		87.00	119.62	87.00	Regression Eqn.	INLET
7434.75		101.70	128.36	101.70	Regression Eqn.	INLET
7435.00		116.40	136.55	116.40	Regression Eqn.	INLET
7435.25		130.50	151.16	130.50	Regression Eqn.	INLET
7435.50		143.40	165.50	143.40	Regression Eqn.	INLET
7435.75		155.70	178.88	155.70	Regression Eqn.	INLET
7436.00		166.80	191.17	166.80	Regression Eqn.	INLET
7436.25		177.30	202.78	177.30	Regression Eqn.	INLET
7436.50		187.20	213.70	187.20	Regression Eqn.	INLET
7436.75		196.50	224.22	196.50	Regression Eqn.	INLET
7437.00		205.20	234.05	205.20	Regression Eqn.	INLET
7437.25		213.60	243.61	213.60	Regression Eqn.	INLET
7437.50		221.70	252.89	221.70	Regression Eqn.	INLET
7437.75		229.50	261.77	229.50	Regression Eqn.	INLET
7438.00		237.00	270.37	237.00	Regression Eqn.	INLET
7438.25		244.20	278.70	244.20	Regression Eqn.	INLET
7438.50		251.10	286.76	251.10	Regression Eqn.	INLET
7438.75		258.00	294.54	258.00	Regression Eqn.	INLET
7439.00		264.60	302.19	264.60	Regression Eqn.	INLET
7439.25		271.20	309.70	271.20	Regression Eqn.	INLET

Processing Time: 00.93 Seconds

CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

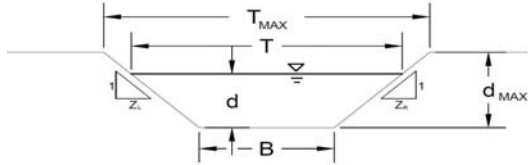
Project: High Plains Filing no. 1
Basin ID: Design Point 1



AREA INLET IN A SWALE

Enter Your Project Name Here

Swale DP-1



This worksheet uses the NRCS vegetative retardance method to determine Manning's n.
For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

NRCS Vegetal Retardance (A, B, C, D, or E) A, B, C, D or E
Manning's n (Leave cell D16 blank to manually enter an n value) n =
Channel Invert Slope S₀ = ft/ft
Bottom Width B = ft
Left Side Slope Z1 = ft/ft
Right Side Slope Z2 = ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V _{MAX})	Max Froude No. (F _{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
 Non-Cohesive
 Cohesive
 Paved

	Minor Storm	Major Storm	
T _{MAX} =	<input type="text" value="22.00"/>	<input type="text" value="30.00"/>	feet
d _{MAX} =	<input type="text" value="2.00"/>	<input type="text" value="2.50"/>	feet

Maximum Channel Capacity Based On Allowable Top Width

	Minor Storm	Major Storm	
T _{MAX} =	<input type="text" value="22.00"/>	<input type="text" value="30.00"/>	ft
d =	<input type="text" value="2.13"/>	<input type="text" value="3.13"/>	ft
A =	<input type="text" value="28.69"/>	<input type="text" value="54.69"/>	sq ft
P =	<input type="text" value="22.52"/>	<input type="text" value="30.77"/>	ft
R =	<input type="text" value="1.27"/>	<input type="text" value="1.78"/>	ft
n =	<input type="text" value="0.037"/>	<input type="text" value="0.033"/>	
V =	<input type="text" value="4.74"/>	<input type="text" value="6.70"/>	fps
VR =	<input type="text" value="6.03"/>	<input type="text" value="11.91"/>	ft ² /s
D =	<input type="text" value="1.30"/>	<input type="text" value="1.82"/>	ft
Fr =	<input type="text" value="0.73"/>	<input type="text" value="0.87"/>	
Q _T =	<input type="text" value="135.9"/>	<input type="text" value="366.6"/>	cfs

Max. Flow Based On Allowable Top Width

Maximum Channel Capacity Based On Allowable Water Depth

	Minor Storm	Major Storm	
d _{MAX} =	<input type="text" value="2.00"/>	<input type="text" value="2.50"/>	feet
T =	<input type="text" value="21.00"/>	<input type="text" value="25.00"/>	feet
A =	<input type="text" value="26.00"/>	<input type="text" value="37.50"/>	square feet
P =	<input type="text" value="21.49"/>	<input type="text" value="25.62"/>	feet
R =	<input type="text" value="1.21"/>	<input type="text" value="1.46"/>	feet
n =	<input type="text" value="0.038"/>	<input type="text" value="0.035"/>	
V =	<input type="text" value="4.41"/>	<input type="text" value="5.50"/>	fps
VR =	<input type="text" value="5.34"/>	<input type="text" value="8.05"/>	ft ² /s
D =	<input type="text" value="1.24"/>	<input type="text" value="1.50"/>	feet
Fr =	<input type="text" value="0.70"/>	<input type="text" value="0.79"/>	
Q _d =	<input type="text" value="114.8"/>	<input type="text" value="206.1"/>	cfs

Max. Flow Based On Allowable Water Depth

Allowable Channel Capacity Based On Channel Geometry

	Minor Storm	Major Storm	
Q _{allow} =	<input type="text" value="114.8"/>	<input type="text" value="206.1"/>	cfs
d _{allow} =	<input type="text" value="2.00"/>	<input type="text" value="2.50"/>	ft

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	<input type="text" value="50.0"/>	<input type="text" value="173.9"/>	cfs
Water Depth	<input type="text" value="1.50"/>	<input type="text" value="2.34"/>	feet

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

AREA INLET IN A SWALE

Enter Your Project Name Here

Swale DP-1

Inlet Design Information (Input)

Type of Inlet

Inlet Type =

Angle of Inclined Grate (must be <= 30 degrees)

Width of Grate

Length of Grate

Open Area Ratio

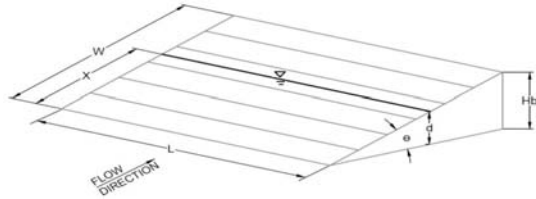
Height of Inclined Grate

Clogging Factor

Grate Discharge Coefficient

Orifice Coefficient

Weir Coefficient



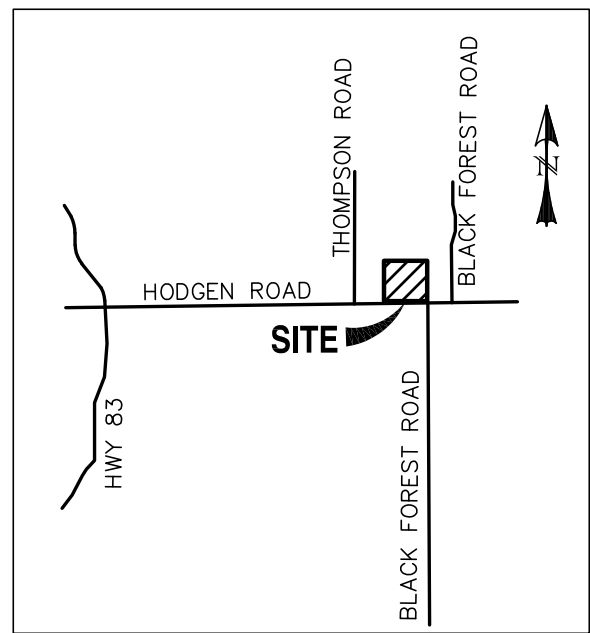
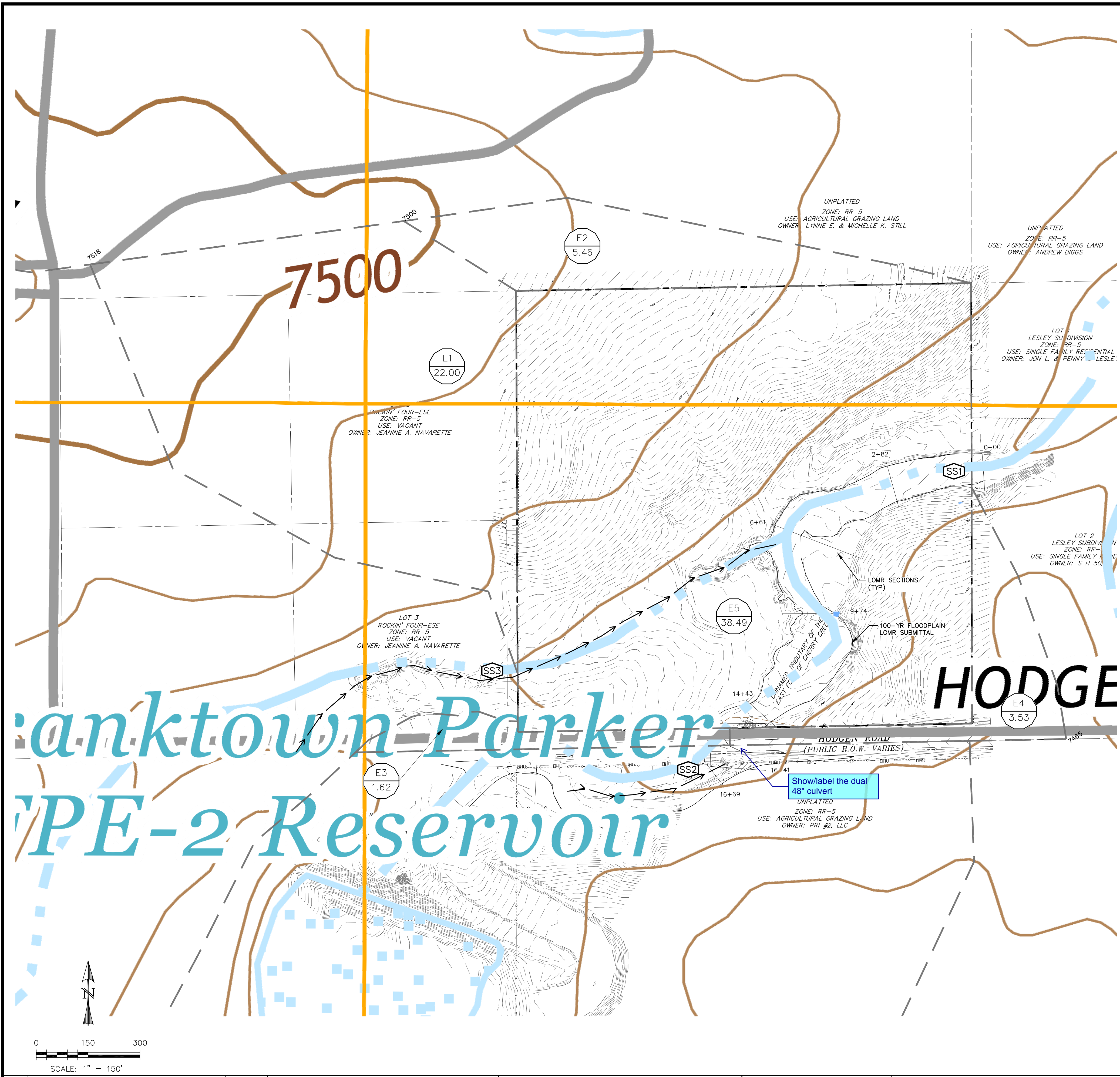
θ = degrees
 W = feet
 L = feet
 A_{RATIO} =
 H_B = feet
 C_1 =
 C_d =
 C_o =
 C_w =

Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)

Total Inlet Interception Capacity (assumes clogged condition)

	MINOR	MAJOR	
d =	<input type="text"/>	<input type="text"/>	
Q_a =	<input type="text"/>	<input type="text"/>	cfs
Bypassed Flow, Q_b =	<input type="text"/>	<input type="text"/>	cfs
Capture Percentage = Q_a/Q_o = C%	<input type="text"/>	<input type="text"/>	%

DRAINAGE MAPS



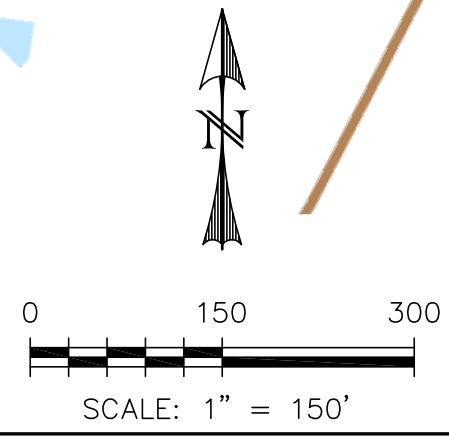
VICINITY MAP
SCALE: N.T.S.

EXISTING DRAINAGE BASINS							
BASIN	AREA (ACRES)	Q2 (CFS)	Q5 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
E1	22.00	0.8	2.8	6.2	10.9	14.6	18.9
E2	5.46	0.3	1.3	2.8	4.8	6.5	8.4
E3	1.62	1.0	1.5	2.2	3.1	3.8	4.6
E4	3.53	0.9	1.6	2.8	4.3	5.5	6.9
E5	38.49	2.4	9.1	20.0	34.9	46.8	60.8

EXISTING DESIGN POINTS		
DESIGN POINT	Q100 (CFS)	Q500 (CFS)
SS1	357	661.0
SS2	295.0	547.0
SS3	153.0	286.0

**Franktown Parker
PE-2 Reservoir**

Show/label the dual
48" culvert



LEGEND

EXISTING	(E)
PROPOSED	(P)
BOUNDARY	---
RIGHT-OF-WAY	----
LOT LINE	-----
EASEMENT	- - - - -
(E) CONTOUR, INDEX	--- 6820 ---
(E) CONTOUR	----- 6820 -----
(P) CONTOUR, INDEX	--- 6820 ---
(P) CONTOUR	----- 6820 -----

REV.	DESCRIPTION	DATE



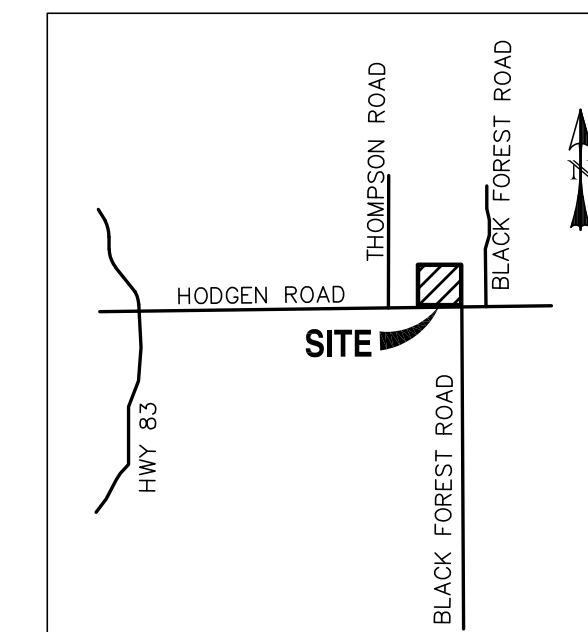
PREPARED FOR:
SAVAGE DEVELOPMENT INC
1125 DIAMOND RIM DR.
COLORADO SPRINGS, CO 80921



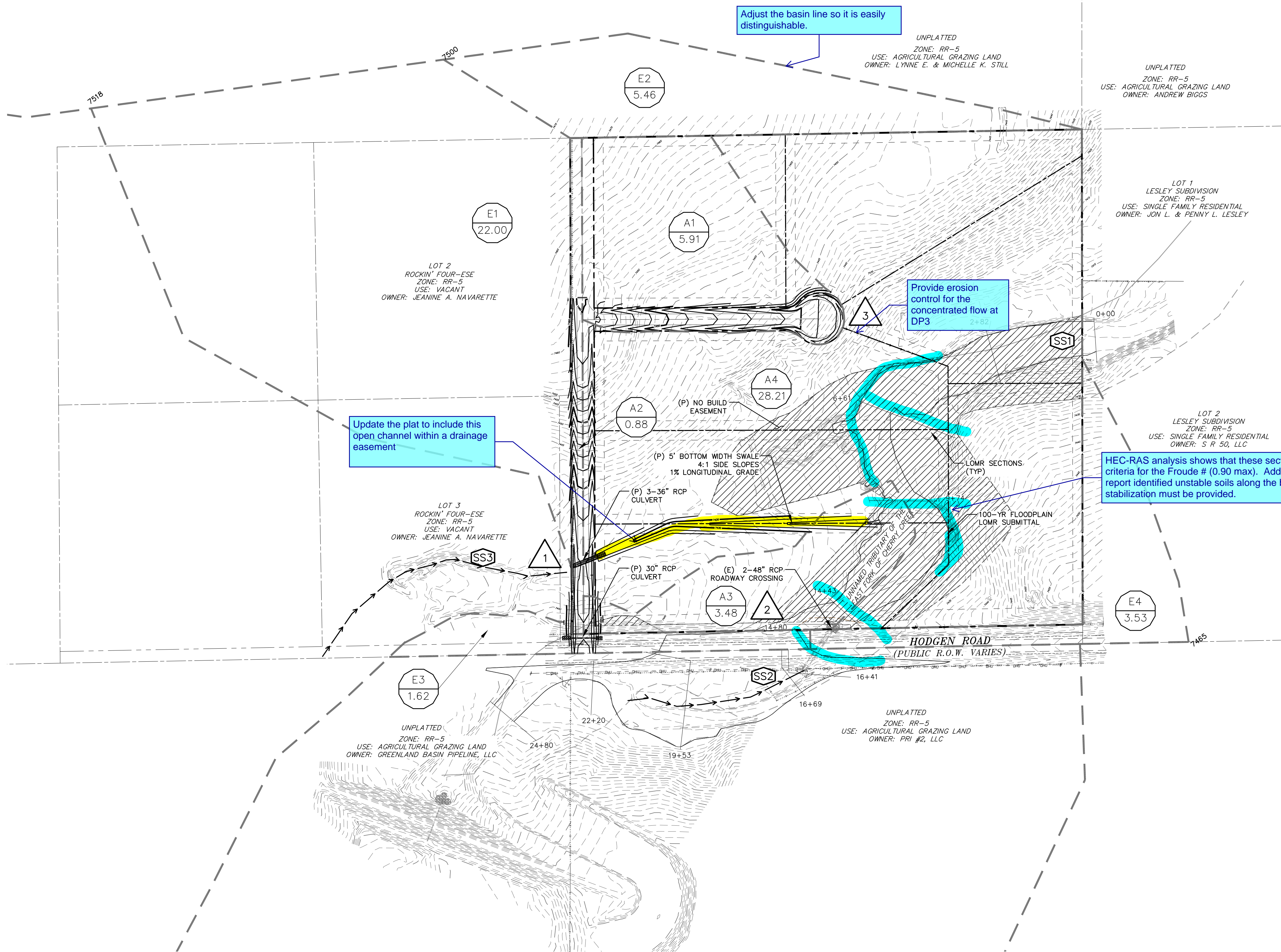
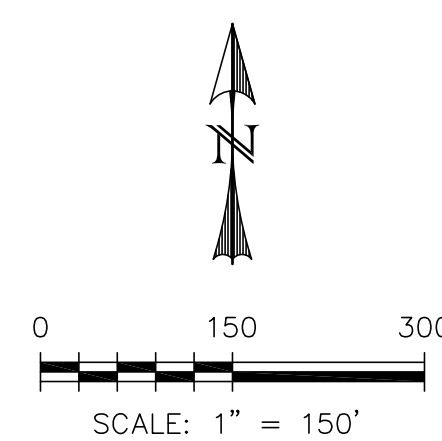
HIGH PLAINS FILING NO. 1

EXISTING CONDITIONS

DESIGNED BY: DLM	DRAWN BY:
SCALE: 1"=150'	DATE: 07/09/18
JOB NUMBER: 17-135	SHEET: 1 OF 1



VICINITY MAP
SCALE: N.T.S.



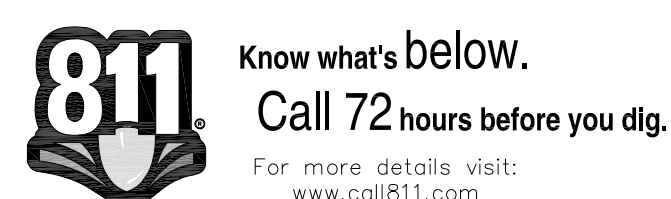
PROPOSED DRAINAGE BASINS							
BASIN	AREA (ACRES)	Q2 (CFS)	Q5 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
E1	22.00	0.8	2.8	6.2	10.9	14.6	18.9
E2	5.46	0.3	1.3	2.8	4.8	6.5	8.4
E3	1.62	1.0	1.5	2.2	3.1	3.8	4.6
E4	3.53	0.9	1.6	2.8	4.3	5.5	6.9
A1	5.91	2.9	5.1	7.3	10.3	12.9	15.6
A2	0.88	2.5	3.1	3.7	4.3	4.9	5.6
A3	3.48	0.7	1.5	2.6	4.0	5.2	6.5
A4	28.21	6.8	15.0	24.9	38.0	49.3	61.4

PROPOSED DESIGN POINTS							
DESIGN POINT	Q2 (CFS)	Q5 (CFS)	Q10 (CFS)	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)	Q500 (CFS)
SS1						357.0	661.0
SS2						295.0	547.0
SS3						153.0	286.0
1						173.9	
2	2.5	3.4	4.3	5.5	6.4	7.5	
3	2.9	5.1	7.3	10.3	12.9	15.6	

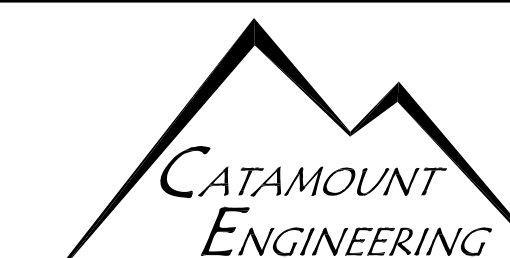
LEGEND

- EXISTING (E)
- PROPOSED (P)
- BOUNDARY
- RIGHT-OF-WAY
- LOT LINE
- EASEMENT
- (E) CONTOUR, INDEX
- (E) CONTOUR
- (P) CONTOUR, INDEX
- (P) CONTOUR

REV.	DESCRIPTION	DATE



PREPARED FOR:
SAVAGE DEVELOPMENT INC
1125 DIAMOND RIM DR.
COLORADO SPRINGS, CO 80921



PO BOX 221 WOODLAND PARK, CO 80866 (719) 435-2124

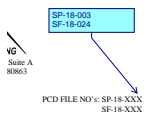
HIGH PLAINS FILING NO. 1

PROPOSED CONDITIONS

DESIGNED BY: DLM	DRAWN BY:
SCALE: 1"=150'	DATE: 07/09/18
JOB NUMBER	SHEET
17-135	1 OF 1

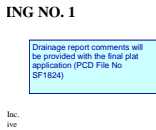
Markup Summary

dsdlaforce (22)



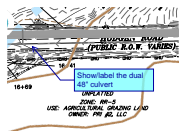
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Page Label: 1
Author: dsdlaforce
Date: 8/13/2018 4:18:44 PM
Color: ■

SP-18-003
SF-18-024



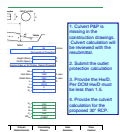
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Page Label: 1
Author: dsdlaforce
Date: 8/14/2018 9:10:49 AM
Color: ■

Drainage report comments will be provided with the final plat application (PCD File No SF1824)



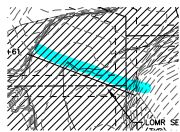
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Author: dsdlaforce
Date: 8/27/2018 5:19:55 PM
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Show/label the dual 48" culvert



Subject: Text Box
Page Label: 48
Author: dsdlaforce
Date: 8/28/2018 1:54:57 PM
Color: ■

1. Culvert P&P is missing in the construction drawings. Culvert calculation will be reviewed with the resubmittal.
2. Submit the outlet protection calculation
3. Provide the Hw/D. Per DCM Hw/D must be less than 1.5.
4. Provide the culvert calculation for the proposed 30" RCP.



Subject: Highlight
Page Label: 54
Author: dsdlaforce
Date: 8/28/2018 4:08:39 PM
Color: ■



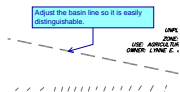
Subject: Callout
Page Label: 54
Author: dsdlaforce
Date: 8/28/2018 4:19:17 PM
Color: ■

HEC-RAS analysis shows that these sections exceed the criteria for the Froude # (0.90 max). Additionally the soils report identified unstable soils along the banks. Channel stabilization must be provided.



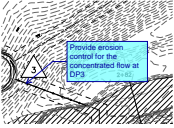
Subject: Callout
Page Label: 5
Author: dsdlaforce
Date: 8/28/2018 4:49:55 PM
Color: ■

Does overtopping occur during the 100yr event?



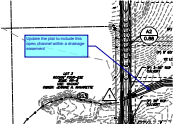
Subject: Callout
Page Label: 54
Author: dsdlaforce
Date: 8/28/2018 8:31:26 AM
Color: ■

Adjust the basin line so it is easily distinguishable.



Subject: Callout
Page Label: 54
Author: dsdlaforce
Date: 8/28/2018 8:33:16 AM
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Provide erosion control for the concentrated flow at DP3



Subject: Callout
Page Label: 54
Author: dsdlaforce
Date: 8/28/2018 8:36:32 AM
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Update the plat to include this open channel within a drainage easement

PROPOSED	EXISTING	LENGTH	WIDTH	DEPTH	VELOCITY	DISCHARGE	STATUS
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S
Proposed	Existing	100.0	4.0	1.0	1.5	100.0	S

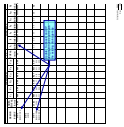
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The proposed culverts should be public non-reimbursable improvements.



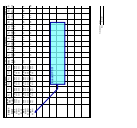
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Delineate and label the site location.



Subject: Callout
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Author: dsdlaforce
Date: 8/28/2018 9:06:34 AM
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Update. Subbasin and DP designation does not match the existing drainage map.



Subject: Callout
Page Label: 21
Author: dsdlaforce
Date: 8/28/2018 9:12:02 AM
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Include DP SS1.

EXISTING	PROPOSED	LENGTH	WIDTH	DEPTH	VELOCITY	DISCHARGE	STATUS
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S
Existing	Proposed	100.0	4.0	1.0	1.5	100.0	S

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Author: dsdlaforce
Date: 8/28/2018 9:42:03 AM
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The flowrate shown on the proposed and existing drainage map are equal for DP SS1. Include a narrative explaining/stating the insignificant increase in discharge rate between the existing and developed condition.

In conjunction with the above comment, provide a narrative justifying why no detention is required.

STEP	DESCRIPTION	STATUS
1	Step 1: Determine the design flow rate and peak discharge rate.	Completed
2	Step 2: Determine the design detention volume and detention time.	Completed
3	Step 3: Determine the design detention structure and detention facility.	Completed
4	Step 4: Determine the design detention structure and detention facility.	Completed

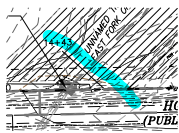
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List each of the 4 step (ECM Appendix I.7.2, pg I-21). Below each step provide a narrative how that specific step was implemented/considered in the design process.

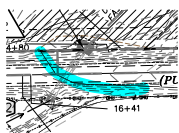
Categorically state permanent WQ facility is not required. See ECM I.7.1.B, pg I-18



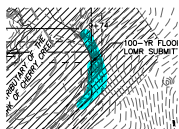
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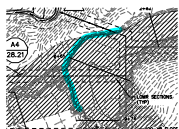
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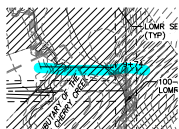
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Daniel Torres (4)

25=**10.3** cfs
 h-south roa

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Author: Daniel Torres
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10.3

50=**12.9** cfs
 ay. The roa

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Author: Daniel Torres
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12.9

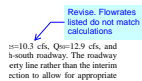
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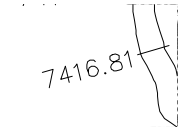
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Revise. Flowrates listed do not match calculations



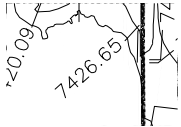
AutoCAD SHX Text (11)



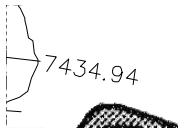
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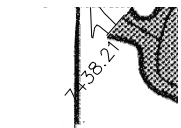
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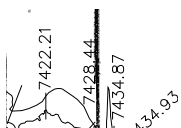
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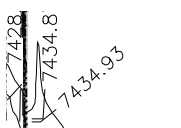
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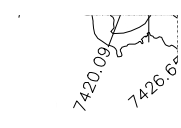
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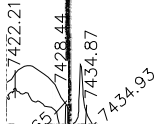
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