

CHRIS TEAM SUBDIVISION

FINAL DRAINAGE REPORT

ALL TERRAIN ENGINEERING PROJECT NO: 24019 AUGUST 2024

PREPARED FOR:

CHRIS TEAM LIVING TRUST

CONTACT: CHRISTINE TSCHAMLER

PREPARED BY:

ALL TERRAIN ENGINEERING LLC

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ENGINEER'S STATEMENT

The attacked drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the city/county for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent accts, errors or omissions on my part in preparing this report.

Nicholas Q. Jokerst, PE	Date	_	
State of Colorado No. 59273			
For and on behalf of All Terrai	in Engineering LLC		
DEVELOPER'S STATEMENT			
CHRIS TEAM LIVING TRUST to specified in this drainage rep	•	read and will com	ply with all of the require
Business Name			
Ву:			
Title:			
Address:			
EL PASO COUNTY ONLY			
Filed in accordance with Sec	tion 51.1 of the El	Paso Land Develor	oment Code as amended
		·	
Director of Public Works Conditions:		Date	



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I. General Purpose, Location & Description

a. Purpose & Project Description

The purpose of the Final Drainage Report (FDR) for the CHRIS TEAM SUBDIVISION is to describe the site's onsite and offsite drainage patterns, existing and proposed storm infrastructure, and to safely route developed stormwater to adequate outfalls.

b. Location

CHRIS TEAM SUBDIVISION, referred to as 'the site' herein, is in a portion of the northeast quarter of Section 14, Township 11 South, Range 65 West of the 6th P.M., El Paso County, Colorado. The site is bound by unplatted land to the north, west and east, Gerth Subdivision and Beierle Minor Subdivision (R1) to the south, and Hendriks Subdivision to the north. A vicinity map is presented in Appendix A.

c. Description of Property

The site is approximately 19.18 acres and includes a single residence. Existing vegetation consists of native grasses and dense forest. There will be no land disturbance or site improvements associated with this report. The site is currently unplatted and zoned RR-5. The intention of the project is to plat a minor subdivision of three (3) 5+ acre lots. At this time, no additional development will occur on the property.

In general, the site slopes northeasterly. Onsite elevations range from 7415' - 7450' with slopes ranging 1 – 20%. Per a NRCS soil survey, the site is made up of Type B Elbeth sandy loam. An existing drainage tributary bisects the site and conveys the site's stormwater. Per the Land Survey Plat, an underground telecommunication line runs along the site's northern boundary. Two existing 12" PVC culverts discharge onsite along the western property line. An existing drainage map is presented in Appendix F.

d. Floodplain Statement

Based on FEMA Firm map 08041C0310G dated December 7, 2018, the site is Zone X, which are areas determined to be outside the 0.2% annual chance flood.

II. Drainage Basins

a. Major Basin Description

The site is located within the West Kiowa Creek Drainage Basin. West Kiowa Creek is an unstudied drainage basin and a Drainage Basin Planning Study has not been completed.

b. Existing Subbasin Description

The existing site's drainage patterns are relatively uniform. A tributary bisects the site and conveys stormwater to a low point in the northeast corner of the site. See below for existing basin descriptions:

Basin A is 19.20 acres of dense forest, dirt roads and a single residence. The basin is formed by existing road benches along the site's north, west and eastern boundaries. Offsite culverts convey stormwater along the road bench to the north. Along the site's western boundary, two 12" PVC culverts discharge stormwater into



the onsite tributary. An additional survey would be required to fully quantify the flow from these culverts. However, the existing condition of the culverts and outfall is stable. This historic drainage pattern will remain unchanged. Stormwater flows from Basin A ($Q_5 = 5.0$ cfs $Q_{100} = 24.4$ cfs) collect in a low point at DP2. The low point is densely vegetated and stable. Site photos of the culvert outfall and site low point are presented in Appendix E.

Basin B is 0.49 acres of dirt roads and meadow. Stormwater from this basin ($Q_5 = 0.3$ cfs $Q_{100} = 1.3$ cfs) is collected in a roadside ditch and conveyed to DP3 where it overtops Black Squirrel Road. Additional flow enters this basin from offsite areas to the north. An additional survey of offsite, private property would be required to fully analyze. However, this basin will remain unchanged and follow historic drainage patterns. The existing roadside ditch and culverts will not be affected.

Basin C is 0.26 acres of dirt road. Stormwater from this basin (Q_5 = 0.5 cfs Q_{100} = 1.1 cfs) is collected in a roadside ditch and conveyed to DP1 where two 12" PVC culverts convey the flow to the onsite tributary. Additional flow enters this basin from offsite areas to the west. An additional survey of offsite, private property would be required to fully analyze. However, this basin will remain unchanged and follow historic drainage patterns. The existing roadside ditch and culverts will not be affected.

c. Proposed Subbasin Description

The project will not be performing any site improvements nor disturbing land. Drainage basins will remain unchanged.

III. Drainage Design Criteria

a. Development Criteria Reference

The drainage analysis, proposed storm sewer system, and proposed private, full spectrum water quality and detention pond follow the criteria from the "Drainage Criteria Manual County of El Paso, Colorado" Volumes 1 and 2," as amended.

b. Hydrologic Criteria

Hydrologic data was obtained from the NOAA Atlas 14 for the site area. Onsite drainage analysis included the 5-year storm (minor event) and 100-year storm (major event) using 1-hr duration rainfall depths from NOAA Atlas 14. Runoff was calculated per EPCDCM Chapter 5 – Storm Runoff Method of Analysis.

d. Hydraulic Criteria

Hydraulic criteria for channel analysis was obtained from EPCDCM Chapter 10 - Open Channels and Structures.

Outfall tailwater conditions into detention facilities are based on the hydraulic grade line in the pond at the time of concentration of the tributary basin. The hydraulic grade line is adapted from the UD-Detention Drain Time v. Ponding Depth graph presented in Appendix D.



IV. Drainage Facility Design

a. General Concept

The site will remain in its existing condition. No stormwater improvements will be made in conjunction with this FDR.

b. Water Quality & Detention

The site will not require water quality and detention. The site is comprised of 5+ acre lots with a total impervious of 7% and is excluded from permanent stormwater quality control measures per the Large Lot Single Family Sites exclusion in Appendix I of the EPC DCM. A PBMP Applicability map is presented in Appendix F.

c. Major Drainageways

There are no major drainageways that traverse the site. The existing onsite drainage tributary is stable and will not receive an increase in flows from this project.

d. Operations & Maintenance

An Operations and Maintenance Manual will not be required as no stormwater facilities are proposed.

e. Grading & Erosion Control Plan

A Grading and Erosion Control plan is not required as no land disturbance will occur with this project.

f. Four Step Method

Step 1 – Reducing Runoff Volumes: Existing roof drains route across landscape areas whenever possible to promote infiltration. In addition, a vegetated drainage tributary captures and conveys stormwater to the historic outfall at the northeast corner of the site. An IRF spreadsheet is presented in Appendix B.

Step 2 – Treat and slowly release the WQCV: The site is exempt from permanent water quality per the Large Lot Single Family Site exclusion in Appendix I of the EPC DCM.

Step 3 – Stabilize stream channels: All new and re-development projects are required to construct or participate in the funding of channel stabilization measures. Drainage basin fees paid, at the time of platting, go towards channel stabilization with the drainage basin. However, the site is within the West Kiowa Creek Drainage Basin which does not have established basin or bridge fees.

Step 4 – Consider the need for source controls: No industrial or commercial uses are proposed within this development and therefore no source controls are proposed.

g. Drainage Basin & Bridge Fees

The site is within the West Kiowa Creek Drainage Basin which does not have established basin or bridge fees. Therefore, no drainage fees will be paid at time of platting.



h. Engineer's Opinion of Probable Cost

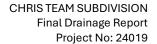
An OPC will not be provided as there are no improvements associated with this FDR.

V. Summary

CHRIS TEAM SUBDIVISION remains consistent with pre-development drainage conditions. The proposed development will not adversely affect downstream stormwater infrastructure or surrounding developments. This report is in accordance with the latest El Paso County Drainage criteria.

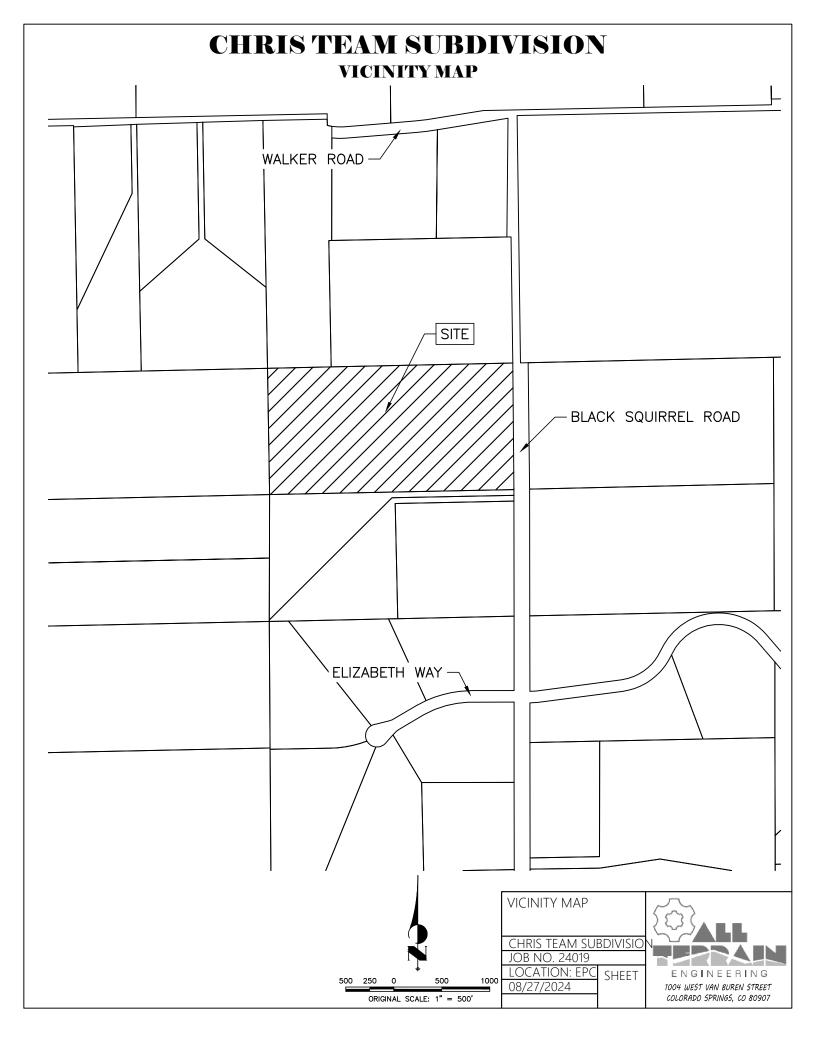
VI. References

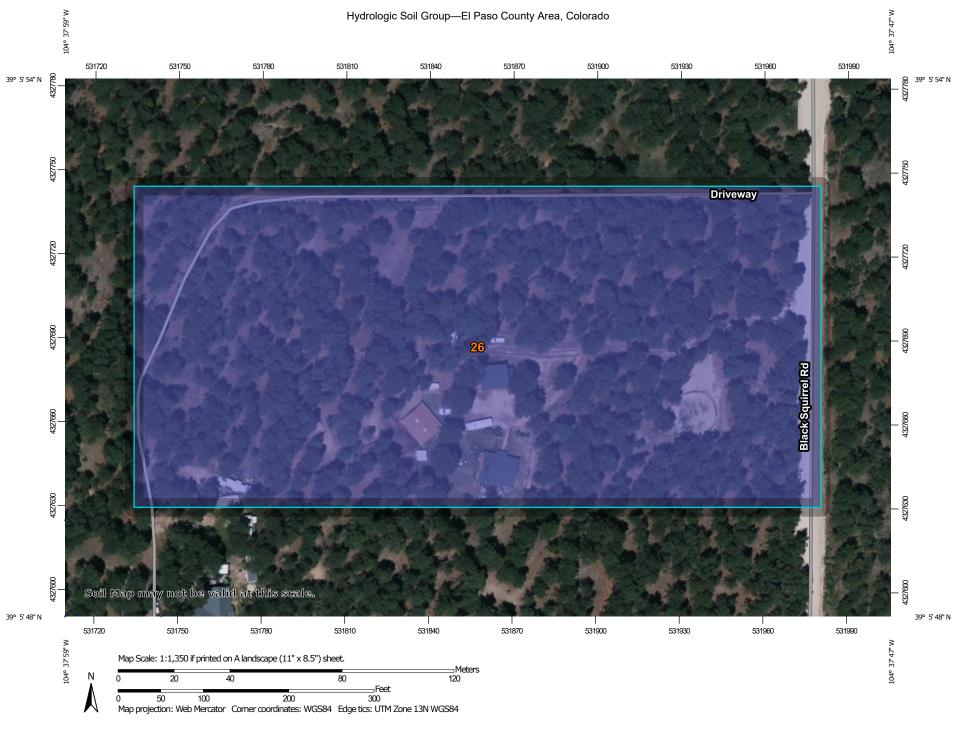
- 1. El Paso County Drainage Criteria Manual, latest revision.
- 2. Urban Storm Drainage Criteria Manual, Mile High Flood District, March 2024.





APPENDIX A – VICINITY MAP, FEMA MAP, NRCS WEB SOIL SURVEY & NOAA ATLAS 14





MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:24.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 21, Aug 24, 2023 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Jun 9, 2021—Jun 12. 2021 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
26	Elbeth sandy loam, 8 to 15 percent slopes	В	7.0	100.0%
Totals for Area of Intere	st		7.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

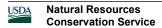
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified



National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD **HAZARD AREAS** Regulatory Floodway 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X **Future Conditions 1% Annual** Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - - - Channel, Culvert, or Storm Sewer **GENERAL** STRUCTURES | LILLI Levee, Dike, or Floodwall 20.2 Cross Sections with 1% Annual Chance 17.5 Water Surface Elevation **Coastal Transect** ₩ 513 W Base Flood Elevation Line (BFE) Limit of Study Jurisdiction Boundary **Coastal Transect Baseline** OTHER **Profile Baseline FEATURES** Hydrographic Feature Digital Data Available No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

point selected by the user and does not represent

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 8/27/2024 at 11:59 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.





NOAA Atlas 14, Volume 8, Version 2 Location name: Colorado Springs, Colorado, USA* Latitude: 39.0977°, Longitude: -104.6314° Elevation: 7471 ft**



* source: ESRI Maps ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

				Average	recurrence	interval (ye	ars)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.240 (0.188-0.306)	0.291 (0.228-0.371)	0.379 (0.296-0.485)	0.456 (0.355-0.587)	0.570 (0.431-0.764)	0.662 (0.489-0.898)	0.760 (0.542-1.05)	0.864 (0.592-1.23)	1.01 (0.665-1.47)	1.12 (0.720-1.65
10-min	0.351 (0.276-0.448)	0.425 (0.334-0.544)	0.554 (0.433-0.710)	0.668 (0.519-0.860)	0.834 (0.631-1.12)	0.970 (0.716-1.32)	1.11 (0.794-1.54)	1.26 (0.866-1.80)	1.48 (0.973-2.15)	1.64 (1.05-2.42)
15-min	0.428 (0.336-0.547)	0.519 (0.407-0.663)	0.676 (0.529-0.866)	0.815 (0.633-1.05)	1.02 (0.770-1.36)	1.18 (0.873-1.60)	1.36 (0.969-1.88)	1.54 (1.06-2.19)	1.80 (1.19-2.63)	2.00 (1.28-2.95)
30-min	0.607 (0.477-0.775)	0.736 (0.577-0.940)	0.957 (0.749-1.23)	1.15 (0.896-1.48)	1.44 (1.09-1.92)	1.67 (1.23-2.26)	1.91 (1.36-2.65)	2.17 (1.48-3.08)	2.52 (1.66-3.68)	2.81 (1.80-4.14)
60-min	0.768 (0.603-0.981)	0.922 (0.724-1.18)	1.20 (0.935-1.53)	1.44 (1.12-1.86)	1.81 (1.38-2.44)	2.12 (1.57-2.88)	2.45 (1.75-3.40)	2.80 (1.92-3.99)	3.30 (2.18-4.82)	3.70 (2.37-5.45)
2-hr	0.928 (0.735-1.18)	1.11 (0.877-1.40)	1.43 (1.13-1.82)	1.73 (1.36-2.21)	2.19 (1.68-2.93)	2.57 (1.92-3.48)	2.98 (2.15-4.13)	3.43 (2.38-4.87)	4.07 (2.72-5.92)	4.59 (2.97-6.72)
3-hr	1.01 (0.805-1.28)	1.20 (0.953-1.51)	1.55 (1.22-1.95)	1.87 (1.47-2.38)	2.38 (1.84-3.18)	2.81 (2.11-3.80)	3.29 (2.39-4.54)	3.81 (2.66-5.39)	4.56 (3.06-6.62)	5.17 (3.36-7.54)
6-hr	1.18 (0.941-1.46)	1.38 (1.10-1.72)	1.76 (1.41-2.21)	2.14 (1.70-2.69)	2.73 (2.13-3.64)	3.24 (2.46-4.36)	3.81 (2.80-5.24)	4.44 (3.13-6.26)	5.36 (3.64-7.75)	6.12 (4.02-8.88)
12-hr	1.37 (1.11-1.70)	1.60 (1.29-1.98)	2.04 (1.64-2.54)	2.47 (1.97-3.07)	3.14 (2.47-4.14)	3.72 (2.85-4.95)	4.36 (3.23-5.95)	5.08 (3.61-7.10)	6.12 (4.19-8.77)	6.97 (4.62-10.0)
24-hr	1.61 (1.31-1.97)	1.88 (1.53-2.31)	2.40 (1.94-2.94)	2.88 (2.32-3.55)	3.62 (2.86-4.72)	4.26 (3.28-5.60)	4.95 (3.69-6.68)	5.72 (4.10-7.91)	6.81 (4.70-9.69)	7.71 (5.16-11.0)
2-day	1.88 (1.54-2.27)	2.22 (1.81-2.69)	2.82 (2.30-3.43)	3.37 (2.74-4.12)	4.20 (3.33-5.38)	4.88 (3.78-6.34)	5.62 (4.22-7.49)	6.42 (4.63-8.79)	7.55 (5.25-10.6)	8.46 (5.71-12.0)
3-day	2.05 (1.69-2.47)	2.43 (2.00-2.93)	3.10 (2.54-3.75)	3.70 (3.02-4.50)	4.59 (3.66-5.84)	5.32 (4.14-6.86)	6.10 (4.59-8.07)	6.94 (5.02-9.44)	8.11 (5.66-11.4)	9.04 (6.14-12.8)
4-day	2.20 (1.82-2.64)	2.60 (2.15-3.13)	3.31 (2.72-3.99)	3.94 (3.22-4.77)	4.87 (3.89-6.17)	5.63 (4.39-7.23)	6.44 (4.86-8.48)	7.30 (5.31-9.90)	8.51 (5.97-11.9)	9.48 (6.46-13.4)
7-day	2.60 (2.16-3.10)	3.03 (2.51-3.61)	3.77 (3.12-4.50)	4.43 (3.65-5.32)	5.41 (4.35-6.80)	6.22 (4.88-7.92)	7.08 (5.39-9.26)	8.00 (5.86-10.8)	9.29 (6.56-12.9)	10.3 (7.09-14.5)
10-day	2.97 (2.48-3.52)	3.42 (2.85-4.05)	4.20 (3.49-5.00)	4.90 (4.05-5.86)	5.93 (4.79-7.41)	6.78 (5.35-8.59)	7.68 (5.87-10.0)	8.63 (6.35-11.6)	9.98 (7.08-13.8)	11.0 (7.63-15.5)
20-day	3.99 (3.36-4.68)	4.60 (3.86-5.40)	5.61 (4.70-6.61)	6.48 (5.40-7.67)	7.71 (6.25-9.49)	8.69 (6.90-10.9)	9.70 (7.46-12.5)	10.7 (7.96-14.2)	12.2 (8.70-16.6)	13.3 (9.26-18.5)
30-day	4.81 (4.06-5.61)	5.55 (4.69-6.48)	6.76 (5.70-7.92)	7.77 (6.51-9.15)	9.17 (7.44-11.2)	10.2 (8.15-12.7)	11.3 (8.74-14.4)	12.4 (9.23-16.3)	13.9 (9.96-18.8)	15.0 (10.5-20.7)
45-day	5.80 (4.93-6.73)	6.70 (5.68-7.77)	8.13 (6.88-9.46)	9.29 (7.82-10.9)	10.9 (8.83-13.1)	12.0 (9.60-14.8)	13.2 (10.2-16.6)	14.3 (10.7-18.7)	15.8 (11.4-21.2)	16.8 (11.9-23.2)
60-day	6.63 (5.65-7.65)	7.63 (6.49-8.82)	9.22 (7.82-10.7)	10.5 (8.85-12.2)	12.2 (9.91-14.6)	13.4 (10.7-16.3)	14.6 (11.3-18.3)	15.7	17.2	18.2

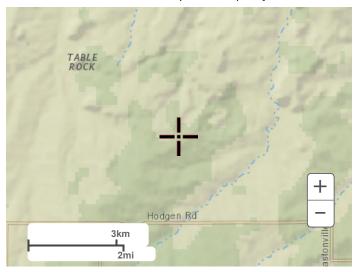
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

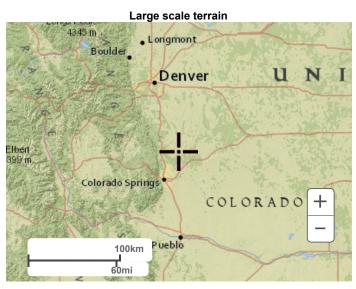
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

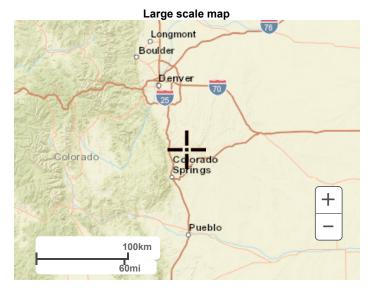
Please refer to NOAA Atlas 14 document for more information.

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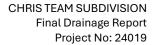
PF graphical







Large scale aerial





APPENDIX B - HYDROLOGIC CALCULATIONS

Subdivision:

Location:

Project Name:

Black Squirrel Road

El Paso County

Black Squirrel Road

Project Number: 24019 Calculated By: NQJ

Checked By:

Date: 8/27/2024

	EX DRA	INAGE CA	LCS - BA	ASIN SUI	MMARY	TABLE	
Tributary Sub-basin	Area (acres)	Percent Impervious	C ₅	C ₁₀₀	t _c (min)	Q₅ (cfs)	Q ₁₀₀ (cfs)
Α	19.20	9%	0.14	0.39	43.2	5.0	24.4
В	0.49	44%	0.37	0.55	23.9	0.3	1.3
С	0.26	80%	0.59	0.70	15.2	0.5	1.1

DESIGN PO	INT SUMM	ARY TABLE
DP#	Q_{5-YR}	Q _{100-YR}
1	0.5	1.1
2	5.4	25.0
3	0.3	1.3

COMPOSITE % IMPERVIOUS CALCULATIONS - EXISTING CONDITIONS

Subdivision: Black Squirrel Road
Location: El Paso County

Project Name: Black Squirrel Road

Project No.: 24019.00

Calculated By: NQJ

Checked By:

Date: 8/27/24

			Grave	l Drives			Roc	ofs			For	est		Weighter	I C ₅ & C ₁₀₀	Basins Total
Basin ID	Total Area C ₅ C ₁₀₀ Area (ac) % Imp.					C ₅	Cana	Area (ac)	% Imp.	C _E	C ₁₀₀	Area (ac)	% Imp.	Weighted	Weighted %	
Dasiii 1D	(ac)	-5	C ₁₀₀	Aica (ac)	70 mp.	05	C ₁₀₀	Aica (ac)	γο imp.		C 100	Aica (ac)	70 IIII p.	C ₅	C ₁₀₀	lmp.
Α	19.20	0.59	0.70	1.32	80.0%	0.73	0.81	0.33	90.0%	0.09	0.36	17.55	0.0%	0.14	0.39	7.0%
В	0.49	0.59	0.70	0.27	80.0%	0.73	0.81	0.00	90.0%	0.09	0.36	0.22	0.0%	0.37	0.55	44.0%
С	0.26	0.59	0.70	0.26	80.0%	0.73	0.81	0.00	90.0%	0.09	0.36	0.00	0.0%	0.59	0.70	80.0%
Total	19.95															8.9%

STANDARD FORM SF-2 - EXISTING CONDITIONS TIME OF CONCENTRATION

Subdivision: Black Squirrel Road Location: El Paso County

Project Name: Black Squirrel Road

Project No.: 24019.00

Calculated By: NQJ

Checked By:

Date: 8/27/24

		SUB-BASI	N		INITI	AL/OVER	RLAND		TI	RAVEL TIM	E					
		DATA				(T _i)				(T _t)			(U	FINAL		
BASIN	D.A.	Hydrologic	Weighted	Impervious	L	S _o	t _i	L _t	S_t	K	VEL.	t _t	COMP. t _c	TOTAL	Urbanized t_c	t _c
ID	(ac)	Soils Group	C ₅	(%)	(ft)	(ft) (%) (min)		(ft)	(ft) (%) (ft/s		(ft/s)	(min)	(min)	LENGTH (ft)	(min)	(min)
Α	19.20	В	0.14	8.9%	100	5.0%	10.2	1540	1.8%	2.5	0.3	76.5	86.8	1640.0	43.2	43.2
В	0.49	В	0.37	44%	17	2.0%	4.4	1058	4.7%	2.5	0.5	32.6	37.0	1075.0	23.9	23.9
С	0.26	В	0.59	80%	11	2.0%	2.4	635	3.5%	2.5	0.5	22.6	25.1	646.0	15.2	15.2

NOTES:

Where:

$$t_c = t_i + t_t$$

 t_c = computed time of concentration (minutes)

 t_i = overland (initial) flow time (minutes)

 t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

 t_i = overland (initial) flow time (minutes)

 C_5 = runoff coefficient for 5-year frequency (from Table 6-4)

 L_i = length of overland flow (ft)

 S_0 = average slope along the overland flow path (ft/ft).

Equation 6-4!6-17i) +
$$\frac{L_t}{60(14i+9)\sqrt{S_t}}$$

Equation 6-5

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Tuble 0 2. Title 5 Cont	cyanice metors, ix
Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Where:

 t_t = channelized flow time (travel time, min)

 L_t = waterway length (ft)

So = waterway slope (ft/ft)

 V_t = travel time velocity (ft/sec) = K $\sqrt{S_0}$

K = NRCS conveyance factor (see Table 6-2).

 t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.

 L_t = length of channelized flow path (ft)

i = imperviousness (expressed as a decimal)

 $S_t =$ slope of the channelized flow path (ft/ft).

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

STANDARD FORM SF-3 - EXISTING CONDITIONS STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

	Project Name: Black Squirrel Road	
Subdivision: Black Squirrel Road	Project No.: 24019.00	
Location: El Paso County	Calculated By: NQJ	
Design Storm: 5-Year	Checked By:	

Date: 8/27/24

				DIR	ECT RU	NOFF			TC)TAL	RUNO	FF		STREE	Т		PI	PE		TRAV	EL TII	ME	
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	/ (in/hr)	Q (cfs)	tc (min)	C*A (ac)	/ (in/hr)	Q (cfs)	Q _{street} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	$t_{ m t}$ (min)	REMARKS
	2	А	19.20	0.14	43.2	2.60	1.94	5.0															BASIN A HISTORIC FLOW, CONVEYED IN EXISTING CHANNEL TO DP2
	3	В	0.26	0.37	23.9	0.09	2.82	0.3															BASIN B HISTORIC FLOW, CONVEYED IN ROADSIDE DITCHES/CULVERTS TO DP3
	1	С	0.26	0.59	15.2	0.15	3.50	0.5				•											BASIN C HISTORIC FLOW TO EX 12" PVC CULVERTS @ DP1, FLOWS ONSITE TO DP2
Notos	2								58.4	2.75	1.48	5.4											TOTAL FLOW AT DP2

Notes: Street and Pipe C^*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3 - EXISTING CONDITIONS STORM DRAINAGE SYSTEM DESIGN

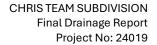
(RATIONAL METHOD PROCEDURE)

		Project Name:	Black Squirrel Road
Subdivision: B	Black Squirrel Road	Project No.:	24019.00
Location: El	I Paso County	Calculated By:	NQJ
Design Storm: 10	00-Year	Checked By:	
		Date:	8/27/24

				DIRE	CT RUI	NOFF			T	OTAL RU	NOFF		STRE	ΕT		PIPE TRAVEL TIME		ME					
STREET	Design Point	Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	/ (in/hr)	Q (cfs)	tc (min)	C*A (ac)	/ (in/hr) O (cfs)		C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _t (min)	REMARKS	
	2	А	19.20	0.39	43.2	7.51	3.25	24.4														BASIN A HISTORIC FLOW, CONVEYED IN EXISTING CHANNEL TO DP2	
	3	В	0.49	0.55	23.9	0.27	4.74	1.3														BASIN B HISTORIC FLOW, CONVEYED IN ROADSIDE DITCHES/CULVERTS TO DP3	
	1	С	0.26	0.70	15.2	0.18	5.88	1.1														BASIN C HISTORIC FLOW TO EX 12" PVC CULVERTS @ DP1, FLOWS ONSITE TO DP2	
	2				-				43.2	7.69	.25 25	.0										TOTAL FLOW AT DP2	

Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

			Des	ign Proced	lure Form:	Runoff Redu	ction					
				UD-BMP (\	Version 3.07, Ma	arch 2018)						Sheet 1 of 1
Designer:	NQJ										-	
Company:		ENGINEERING	•								-	
Date:	August 29, 20										-	
Project:		BLACK SQUIRE	REL ROAD								-	
Location:	EL PASO COL	JNTY									-	
												
SITE INFORMATION (Use	er Input in Blu	ıe Cells)										
		Rainfall Depth	0.60	inches								
Depth of Average Ru	noff Producing	g Storm, d ₆ =	0.43	inches (for W	atersheds Ou	tside of the Denv	er Region, Fig	gure 3-1 in US	SDCM Vol. 3)			
Area Tuna	SPA	UIA:RPA										
Area Type Area ID	1-SPA	1-UIA:RPA										
Downstream Design Point ID	1	1			 					 		
Downstream BMP Type	None	None										
DCIA (ft ²)												
UIA (ft²)		14,209										
RPA (ft²)		28,000										
SPA (ft²)	80,000											
HSG A (%)	0%	0%										
HSG B (%)	100%	100%										
HSG C/D (%)	0%	0%		<u> </u>	<u> </u>							
Average Slope of RPA (ft/ft)		0.040		ļ!	 				 	-	ļ'	├ ──┤ '
UIA:RPA Interface Width (ft)		75.00		<u> </u>	L		I		<u> </u>		<u> </u>	l
CALCULATED RUNOFF	RESULTS											
Area ID	1-SPA	1-UIA:RPA				Ī						
UIA:RPA Area (ft²)		42,209										
L / W Ratio		7.50										
UIA / Area		0.3366										
Runoff (in)	0.00	0.00			<u> </u>						ļ	
Runoff (ft ³)	0	0 592							 	-	ļ	
Runoff Reduction (ft 3)	4000	592				J			L	L		<u> </u>
CALCULATED WQCV RE	SULTS											
Area ID		1-UIA:RPA				Ī						
WQCV (ft ³)	0	592										
WQCV Reduction (ft ³)	0	592										
WQCV Reduction (%)	0%	100%										
Untreated WQCV (ft 3)	0	0			L				<u> </u>	L		
041 0111 4750 0501011 5	ONE DECL	TO /										
CALCULATED DESIGN P Downstream Design Point ID	1	1S (sums res	uits from all	columns with	the same D	ownstream Des	ign Point ID)		г			
DCIA (ft ²)	0	0										
UIA (ft²)	14,209	14,209										
RPA (ft²)	28,000	28,000		 							 	
SPA (ft²)	80,000	80,000									,	
Total Area (ft ²)	122,209	122,209										
Total Impervious Area (ft ²)	14,209	14,209										
WQCV (ft ³)		592										
WQCV Reduction (ft ³)		592			<u> </u>					ļ	<u> </u>	ļI
WQCV Reduction (%)	100%	100%		<u> </u>	 		——		 		<u> </u>	
Untreated WQCV (ft 3)	0	0			<u> </u>				<u> </u>		<u> </u>	
CALCULATED SITE RES	III TS (eume i	roculte from a	ıll columne i	n workshoot)								
Total Area (ft ²)		Tesuits iroin a	iii colullilis i	ii worksneet)								
Total Impervious Area (ft ²)		1										
WQCV (ft ³)												
WQCV Reduction (ft ³)												
WQCV Reduction (%)												
Untreated WQCV (ft 3)	0											





APPENDIX C – HYDRAULIC CALCULATIONS

Channel Report

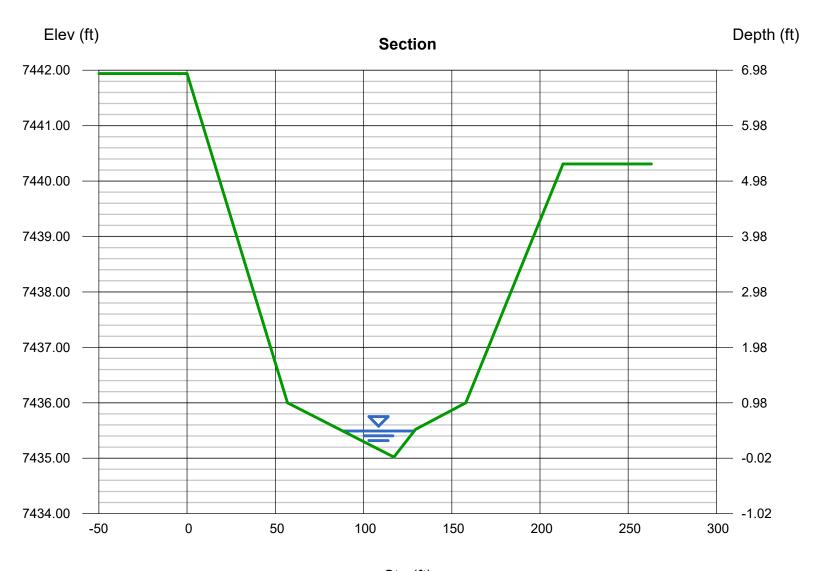
Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Aug 29 2024

CHANNEL SECTION 1 (Q100 = 25.0 cfs)

User-defined		Highlighted	
Invert Elev (ft)	= 7435.02	Depth (ft)	= 0.47
Slope (%)	= 2.00	Q (cfs)	= 25.00
N-Value	= 0.030	Area (sqft)	= 9.51
		Velocity (ft/s)	= 2.63
Calculations		Wetted Perim (ft)	= 40.45
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.47
Known Q (cfs)	= 25.00	Top Width (ft)	= 40.44
, ,		EGL (ft)	= 0.58

(Sta, EI, n)-(Sta, EI, n)... (0.00, 7441.94)-(56.94, 7436.00, 0.030)-(117.21, 7435.02, 0.030)-(129.46, 7435.52, 0.030)-(157.89, 7436.00, 0.030)-(213.09, 7440.31, 0.030)



Sta (ft)

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Aug 29 2024

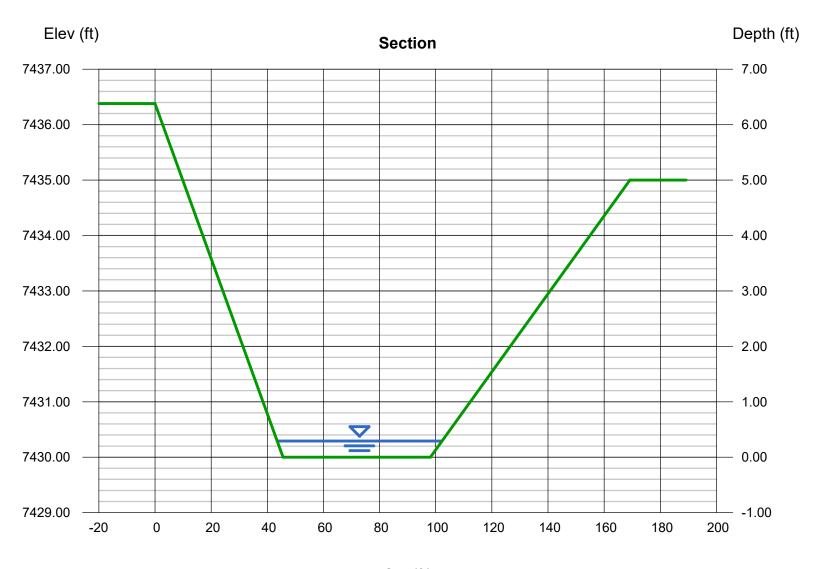
= 0.33

CHANNEL SECTION 2 (Q100 = 25.0 cfs)

User-defined		Highlighted	
Invert Elev (ft)	= 7430.00	Depth (ft)	= 0.29
Slope (%)	= 0.60	Q (cfs)	= 25.00
N-Value	= 0.030	Area (sqft)	= 16.13
		Velocity (ft/s)	= 1.55
Calculations		Wetted Perim (ft)	= 58.73
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.19
Known Q (cfs)	= 25.00	Top Width (ft)	= 58.69

EGL (ft)

(Sta, EI, n)-(Sta, EI, n)... (0.00, 7436.38)-(45.58, 7430.00, 0.030)-(98.08, 7430.00, 0.030)-(169.15, 7435.00, 0.030)



Channel Report

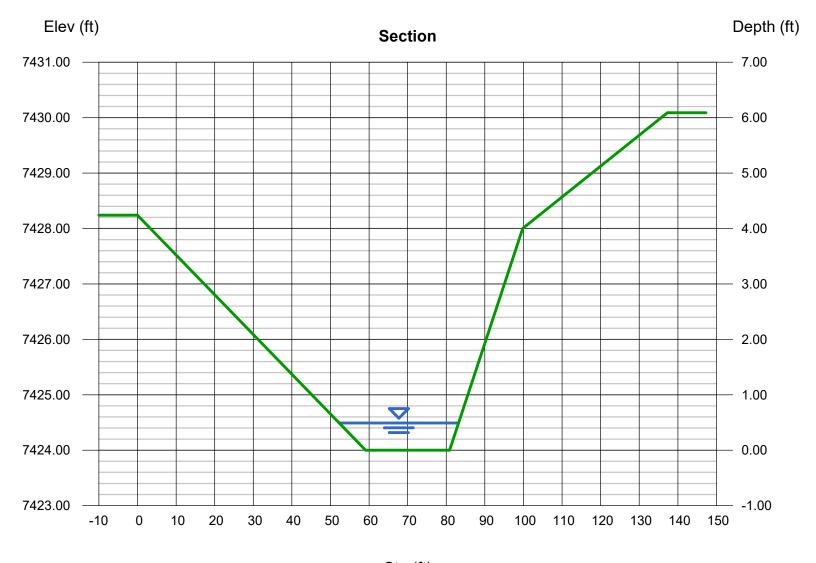
Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

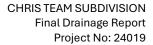
Thursday, Aug 29 2024

CHANNEL SECTION 3 (Q100 = 25.0 cfs)

User-defined		Highlighted	
Invert Elev (ft)	= 7424.00	Depth (ft)	= 0.49
Slope (%)	= 0.49	Q (cfs)	= 25.00
N-Value	= 0.030	Area (sqft)	= 12.91
		Velocity (ft/s)	= 1.94
Calculations		Wetted Perim (ft)	= 30.98
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.33
Known Q (cfs)	= 25.00	Top Width (ft)	= 30.92
		EGL (ft)	= 0.55

(Sta, EI, n)-(Sta, EI, n)... (0.00, 7428.24)-(59.11, 7424.00, 0.030)-(80.87, 7424.00, 0.030)-(99.81, 7428.00, 0.030)-(137.27, 7430.09, 0.030)







APPENDIX D – WATER QUALITY & DETENTION

Post Construction Stormwater Management Applicability Evaluation Form

This form is to be used by the Engineer of Record to evaluate applicable construction activities to determine if the activities are eligible for an exclusion to permanent stormwater quality management requirements. Additionally Part III of the form is used to identify and document which allowable control measure design standard is used for the structure.

Part I. Project Information							
1. Project Name:							
2. El Paso County Project #: 3. ESQCP #:							
4. Project Location:	Project Location in MS4 Permit Area (Y or N):						
5. Project Description:							
If project is located within the El Paso County MS4 Permit Area, please provide copy of this completed form to the Stormwater Quality Coordinator for reporting purposes; and save completed form with project file.							

Part II. Exclusion Evaluation: Determine if Post-Construction Stormwater Management exclusion criteria are met. Note: Questions A thru K directly correlate to the MS4 permit Part I.E.4.a.i (A) thru (K). If Yes, to any of the following questions, then mark Not Applicable in Part III, Question 2.						
Questions	Yes	No	Not Applicable	Notes:		
A. Is this project a "Pavement Management Site" as defined in Permit Part I E.4.a.i.(A)?				This exclusion applies to "roadways" only. Areas used primarily for parking or access to parking are not included.		
B. Is the project "Excluded Roadway Development"?						
 Does the site add less than 1 acre of paved area per mile? 						
 Does the site add 8.25 feet or less of paved width at any location to the existing roadway? 						
C. Does the project increase the width of the existing roadway by less than 2 times the existing width?				For redevelopment of existing roadways, only the area of the existing roadway is excluded from post-construction requirements when the site does not increase the width by two times or more. This exclusion only excludes the original roadway area it does NOT apply to entire project.		
D. Is the project considered an aboveground and Underground Utilities activity?				Activity can NOT permanently alter the terrain, ground cover or drainage patterns from those present prior to the activity		
E. Is the project considered a "Large Lot Single-Family Site"?				Must be a single-residential lot or agricultural zoned land, \geq 2.5 acres per dwelling and total lot impervious area < 10 percent.		

2019 Page **1** of **3**

Questions (cont'd)	Yes	No	Not	Notes
			Applicable	
F. Do Non-Residential or Non-Commercial Infiltration Conditions exist? Post-development surface conditions do not result in concentrated stormwater flow or surface water discharge during an 80 th percentile stormwater runoff event.				Exclusion does not apply to residential or commercial sites for buildings. A site specific study is required and must show: rainfall and soil conditions; allowable slopes; surface conditions; and ratios of imperviousness area to pervious area.
G. Is the project land disturbance to Undeveloped Land where undeveloped land remains undeveloped following the activity?				Project must be on land with no human made structures such as buildings or pavement.
H. Is the project a Stream Stabilization Site?				Standalone stream stabilization projects are excluded.
I. Is the project a bike or pedestrian trail?				Bike lanes for roadways are not included in this exclusion, but may qualify if part of larger roadway activity is excluded in A, B or C above.
J. Is the project Oil and Gas Exploration?				Activities and facilities associated with oil and gas exploration are excluded.
K. Is the project in a County Growth Area?				Note, El Paso County does not apply this exclusion. All Applicable Construction Activity in El Paso County must comply the Post-Construction Stormwater Management criteria.

Part III. Post Construction (Permanent) Stormwater Control Determination		
Questions	Yes	No
1. Is project an Applicable Construction Activity?		
2. Do any of the Exclusions (A-K in Part II) apply?		

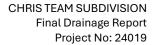
If the project is an Applicable Construction Activity and no Exclusions apply then Post-Construction (Permanent) Stormwater Management is required.

Complete the applicable sections of Part IV below and then coordinate signatures for form and place in project file.

If the project is not an Applicable Construction Activity, or Exclusion(s) apply then Post-Construction (Permanent) Stormwater Management is NOT required. Coordinate signatures for form and place in project file.

2019 Page **2** of **3**

Pa	rt IV: Onsite PWQ Requirements, Documentation and Considerations	Yes	No
1.	Check which Design Standard(s) the project will utilize. Standards align with Control		
	Measure Requirements identified in permit Part I.E.4.a.iv.		
	Water Quality Capture Volume (WQCV) Standard		
	Pollutant Removal/80% Total Suspended Solids Removal (TSS)		
	Runoff Reduction Standard	X	
	Applicable Development Site Draining to a Regional WQCV Control Measure		
E.	Applicable Development Site Draining to a Regional WQCV Facility		
F.			
	Previous Permit Term Standard		
2.	Will any of the project permanent stormwater control measure(s) be maintained by another MS4?		
	If Yes, you must obtain a structure specific maintenance agreement with the other MS4 prior to advertisement.		
3.	Will any of the project permanent stormwater control measures be maintained by a private entity or quasi-governmental agency (e.g. HOA or Special District, respectively)? If Yes, a Private Detention Basin/Stormwater Quality Best Management Practice		
	Maintenance Agreement and Easement must be recorded with the El Paso County Clerk and Recorder.		
Pai	rt V Notes (attach an additional sheet if you need more space)		
			The state of the s
	Project design is complete to include the project design, construction plans, drainage	report,	
	specifications, and maintenance and access agreements as required. The engineering	, drainage	
	considerations and information used to complete these documents is complete, true,	and accurat	te
	to the best of my belief and knowledge is the best of my belief and knowledge is the best of my belief and knowledge.		
	Mil (h) + PE0059273 2:	08/29/2024	
	Signature and Stamp of Engineer of Records 2	Date	
	Post-Construction Stormwater Management Shiplication Form has been reviewed and	the project	•
	design, construction plans, drainage reportures size ations, and maintenance and acce	ess agreeme	ents
	as required, have been reviewed for compliance with the Post Construction Stormwat		
	Management process and MS4 Permit requirements.		
	Cignature of El Dage County Project Francisco		
	Signature of El Paso County Project Engineer	Date	





APPENDIX E - REFERENCE MATERIAL



Figure 1: Looking SW from Black Squirrel Road to the site low point



Figure 2: Same location at Figure 1, looking NE across Black Squirrel Road





Figure 3: From Black Squirrel Road, looking towards low point

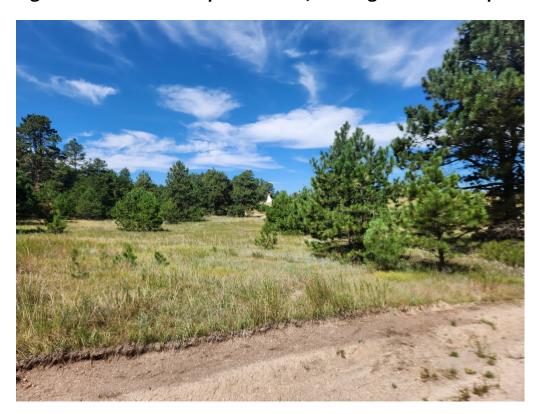


Figure 4: From dirt road along north PL, looking upstream of low point





Figure 5: From dirt road along west PL, looking down drainage tributary



Figure 6: Same location as Figure 5, looking west to offsite/upstream portion of tributary





Figure 7: Double 12" PVC culverts, downstream



Figure 8: Double 12" PVC culverts, upstream



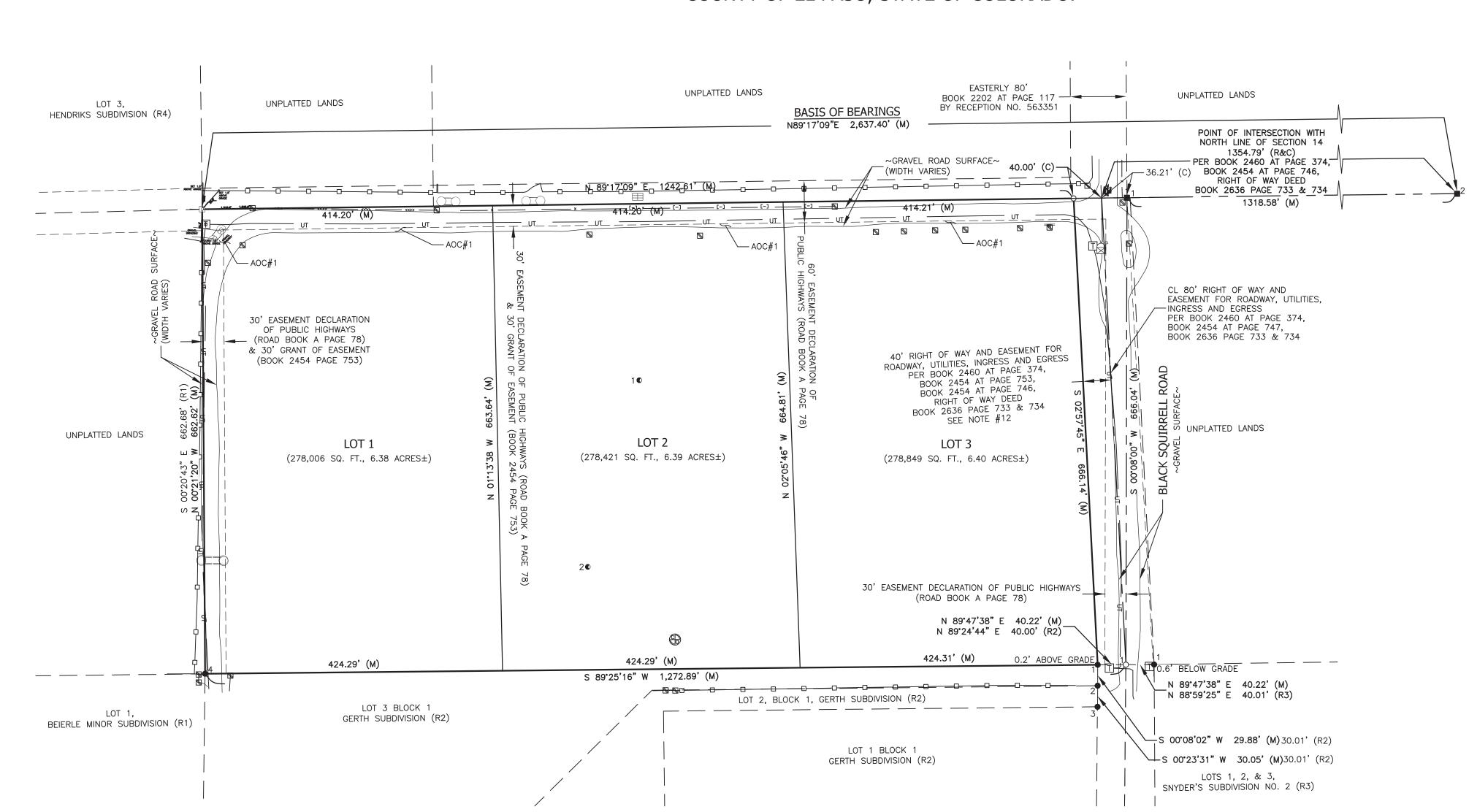


Figure 9: Drainage tributary immediately downstream of 12" PVC culverts



TEAM CHRIS SUBDIVISION

BEING A PART OF THE NORTHEAST QUARTER OF SECTION 14, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH PRINCIPAL MERIDIAN, COUNTY OF EL PASO, STATE OF COLORADO.



LEGEND

- 1 FOUND NO. 5 REBAR AS NOTED
- FOUND NO. 4 REBAR WITH 1"

 YELLOW PLASTIC CAP, PLS 15686,
- FOUND NO. 4 REBAR WITH 1"
 3 YELLOW PLASTIC CAP REMNANTS,
- 0.2' ABOVE GRADE

 FOUND NO. 5 REBAR WITH 1-1/4"
- 4 ORANGE PLASTIC CAP, PLS 38141,
- 0.6' BELOW GRADE N 1/16 SEC 14 T11S R65W
- 1 FOUND NO. 6 REBAR WITH 2-1/2" ALUM CAP MARKED E1/16 S11&S14 1997 PLS 4842, FLUSH WITH GRADE
- NE 1/4 SEC 14 T11S R65W
- FOUND NO. 6 REBAR WITH 2-1/2" ALUM CAP MARKED 1/4 S11&S14 1997 PLS 4842, FLUSH WITH GRADE
- N 1/4 SEC. 14 T11S R65W. FOUND NO. 6 REBAR,

 REHABILITATED WITH 2-1/2" ALUM CAP, T11S R65W 1/4
 S13|S14 2024 PLS 38759, FLUSH WITH GRADE
- FOUND NO. 5 REBAR WITH 1-1/4" O PURPLE PLASTIC CAP, PLS 38759,
- FOUND NO. 5 REBAR WITH 1-1/2"
 10 ALUMINUM CAP, PLS 38759,
- 0.5' BELOW GRADE

FLISH WITH GRADE

- (R) RECORD VALUE
- (R1) RECORD VALUE (BEIERLE MINOR SUBDIVISION) RECEPTION NO. 216713868
- (R2) RECORD VALUE (GERTH SUBDIVISION) PLAT BOOK X-3 AT PAGE 178
- (R3) RECORD VALUE (SNYDER'S SUBDIVISION NO.2) RECEPTION NO. 1490259
- (R4) RECORD VALUE (HENDRICKS SUBDIVISION) RECEPTION NO. 1178523
- (M) MEASURED VALUE
 (AOC#_) AREA OF CONCERRN
- BREAK SYMBOL
- 1 HEADSTONE2 BRICK GRILL
- STORM CULVERT INLET

 STORM DRAIN INLET
- SANITARY SEWER CLEANOUT
- FENCE POST
- MAILBOX CLUSTER

 UT UNDERGROUND TELEPHONE LINE
- WROUGHT−IRON FENCE

 BARABED WIRE FENCE R
- BARABED WIRE FENCE REMNANTS

 WIRE MESH FENCE
- × BARBED—WIRE FENCE

AREA(S) OF CONCERN

(AOC#1): Portions of gravel road lies southerly and easterly of said easement, as graphically depicted on this Land Survey Plat, causing an area of concern.

SURVEYOR'S NOTES

- 1. NOTICE: According to Colorado law you must commence any legal action based upon any defect in this survey within three years after you first discover such defect. In no event may any action based upon any defect in this survey be commenced more than ten years from the date of the certification shown hereon.
- 2. Any person who knowingly removes, alters or defaces any public land survey monument or land boundary monument or accessory commits a class 2 misdemeanor pursuant to the Colorado Revised Statute 18—4—508.
- 3. The lineal units used in this drawing are U.S. Survey Feet.
- 4. The fieldwork for this survey was completed on May 28, 2024.
- 5. The overall subject parcel contains a net calculated area of 835,271 square feet (19.18 acres) of land, more or less.
- 6. This survey does not constitute a title search by Apex Land Surveying and Mapping, LLC. to determine ownership or easements of record. For information regarding easements, rights—of—way and title of record, Apex Land Surveying and Mapping, LLC. relied upon Title Commitment order number RND55116760, with an effective date of 05/24/2024 @ 5:00 P.M. as provided by Land Title Guaranty Company & Old Republic National Title Insurance Company..
- 7. Bearings are based on a portion of the North line of Section 14, T11S, R65W of the Ute P.M., monumented on the west end with a found No. 6 rebar, rehabilitated with 2-1/2" aluminum cap, T11S R65W 1/4 S13|S14 2024 PLS 38759, flush with grade, and on the east end with a found No. 6 rebar with 2-1/2" aluminum cap marked 1/4 S11|S14 1997 PLS 4842, flush with grade and is assumed to bear N89*17'09"E a measured distance of 2,637.40 feet.
- 8. Any underground or above ground utilities shown hereon have been located from field survey information. Apex Land Surveying and Mapping, LLC. does not guarantee said underground utilities to be shown in their exact location and that said underground utilities are shown in their entirety. Apex Land Surveying and Mapping, LLC. did not physically enter any manholes or inlets to verify size and material. Where additional or more detailed information is required, the client is advised that excavation may be necessary.
- 9. Site Benchmark: Set 60D nail (Elevation=7459.74' NAVD88).
- 10. The purpose of this survey is to determine boundary lines of subject parcel for future
- 11. Exeption No 13 in title commitment stipulates terms, conditions, provisions, burdens and obligations as set forth in right of way recorded July 09, 1967 under Reception No. 563351 under Book 2202 at Page 117. Said right of way and easement for roadway, utilities, ingress and egress purposes over and across the East 80 feet of that part of the west half of the Southeast quarter of Section 11 in Township 11 South, Range 65 West of the 6th P.M., as graphically depicted on thie Land Survey Plat.
- 12. Right Of Way Deed per Book 2636 at Page 733 by Reception No. 30371 grants, bargain, sell, and convey the said 80' Strip (40' on either side of centerline) to El Paso County as graphically depicted on this Land Survey Plat. POINT OF INTERSECTION WITH NORTH LINE OF SECTION 14, a distance of 1354.79' (R&C) lands within field measured evidence of intersection of Black Squirrel Road (Gravel road) and private road (gravel road). This document is listed as an "EX" in the vesting deed (Warranty Deed by Reception No. 218044100).
- 13. Abbreviated Legal Description in vesting Warranty Deed by Reception No. 218044100 Has an address listed as 6275 Montabor Dr, Colorado Springs CO 80918. The address listed in this document is the address for Chris team Living trust, not the physical address of subject parcel.
- 14. Exception No. 19—Grant of right of way to mountain view electric association, inc. over a portion of subject property as recorded June 5, 2001 under reception No. 201075608. The evidence in this description in this document does not touch the subject parcel.
- 15. Exception No. 20—Grant of right of way to mountain view electric association, inc. over a portion of subject property as recorded October 2, 2012 under Reception No. 212115628. The evidence in this description does not touch the subject parcel.
- 16. Exception No. 22—Easement granted to public service company of Colorado, for utility, and incidental purposes, by instrument recorded april 21, 1964, in book 2007 at page 850. The evidence in this description does not touch the subject parcel.



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APEX Land Sur

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SHEET 2 OF 2



APPENDIX F - DRAINAGE MAPS

