

Info Only: Comments from Service Engineering are in blue text.

CHRIS TEAM SUBDIVISION

FINAL DRAINAGE REPORT

ALL TERRAIN ENGINEERING PROJECT NO: 24019

AUGUST 2024

PREPARED FOR:

CHRIS TEAM LIVING TRUST

CONTACT: CHRISTINE TSCHAMLER

SF2426

PREPARED BY:

ALL TERRAIN ENGINEERING LLC

CONTACT: NICHOLAS Q. JOKERST

NJOKERST@ALLTERRAINENG.COM

(530) 391-7635

It appears that the analysis in the written narrative and maps/aerials is mixed between the Chris Team Subdivision property and parcel 5114006001 to the south. Based on aerials and site visit there is no existing driveway or structures on the applicant's property.

ENGINEER'S STATEMENT

| The attacked dr | Please use the current County signature block: v direction and supervision and are correct |
|-------------------|--|
| to the best of my | rknowledge and belief. Said drainage report has been prepared according to the criteria |
| established by t | |
| the drainage ba | <u>Design Engineer's Statement:</u> |
| omissions on m | The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report. |
| Nicholas Q. Joke | part in preparing this report. |
| State of Colorac | [Name, P.E. #] Date |
| For and on beha | |
| | <u>Owner/Developer's Statement:</u> |
| DEVELOPER'S | I, the owner/developer have read and will comply with <u>all of</u> the requirements specified in this drainage report and plan |
| CHRIS TEAM LIN | |
| specified in this | [Name, Title] Date [Business Name] [Address] |
| Business Name | <u>El Paso County:</u> |
| Ву: | Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended. |
| Title: | |
| Address: | Joshua Palmer, P.E. Date County Engineer / ECM Administrator |
| | Conditions: |
| EL PASO COUN | |

Filed in accordance with Section 51.1 of the El Paso Land Development Code as amended.

| Director of Public Works |
|--------------------------|
| Conditions: |



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I. General Purpose, Location & Description

a. Purpose & Project Description

The purpose of the Final Drainage Report (FDR) for the CHRIS TEAM SUBDIVISION is to describe the site's onsite and offsite drainage patterns, existing and proposed storm infrastructure, and to safely route developed stormwater to adequate outfalls.

b. Location

CHRIS TEAM SUBDIVISION, referred to as 'the site' herein, is in a portion of the northeast quarter of Section 14, Township 11 South, Range 65 West of the 6th P.M., El Paso County, Colorado. The site is bound by unplatted land to the north, west and east, Gerth Subdivision and Beierle Minor Subdivision (R1) to the south, and Hendriks Subdivision to the north. A vicinity map is presented in Appendix A.

c. Description of Property

The site is approximately 19.18 acres and includes a single residence. Existing vegetation consists of native grasses and dense forest. There will be no land disturbance or site improvements associated with this report. The site is currently unplatted and zoned RR-5. The intention of the project is to plat a minor subdivision of three (3) 5+ acre lots. At this time, no additional development will occur on the property.

In general, the site slopes northeasterly. Onsite elevations range from 7415' - 7450' with slopes ranging 1 – 20%. Per a NRCS soil survey, the site is made up of Type B Elbeth sandy loam. An existing drainage tributary bisects the site and conveys the site's stormwater. Per the Land Survey Plat, an underground telecommunication line runs along the site's northern boundary. Two existing 12" PVC culverts discharge onsite along the western property line. An existing drainage map is presented in Appendix F.

d. Floodplain Statement

Based on FEMA Firm map 08041C0310G dated December 7, 2018, the site is Zone X, which are areas determined to be outside the 0.2% annual chance flood.

II. Drainage Basins

a. Major Basin Description

The site is located within the West Kiowa Creek Drainage Basin. West Kiowa Creek is an unstudied drainage basin and a Drainage Basin Planning Study has not been completed.

b. Existing Subbasin Description

The existing site's drainage patterns are relatively uniform. A tributary bisects the site and conveys stormwater to a low point in the northeast corner of the site. See below for existing basin descriptions: Per site visit, there is currently no improvements on site

Basin A is 19.20 acres of dense forest, dirt roads and a single residence. The basin is formed by existing road benches along the site's north, west and eastern boundaries. Offsite culverts convey stormwater along the road bench to the north. Along the site's western boundary, two 12" PVC culverts discharge stormwater into



Note: The culverts should be cleaned out from sediment buildup during construction.

CHRIS If additional survey is not available, something such as streamstats could be used to identify the offsite flow

the onsite tributary. An additional survey would be required to fully quantify the flow from these culverts. However, the existing condition of the culverts and outfall is stable. This historic drainage pattern will remain unchanged. Stormwater flows from Basin A ($Q_5 = 5.0 \text{ cfs } Q_{100} = 24.4 \text{ cfs}$) collect in a low point at DP2. The low point is densely vegetated and stable. Site photos of the culvert outfall and site low point are presented in Appendix E. Include a discussion of the existing 12" culvert at the NW corner of the site, as shown on the drainage map

Discuss what happens to flows from this basin. How do they cross the roads and where do they do downstream?

Basin B is 0.49 acres of dirt roads and meadow. Stormwater from this basin ($Q_5 = 0.3 \text{ cfs}$ they do downstream? collected in a roadside ditch and conveyed to DP3 where it overtops Black Squirrel Road. Additional flow enters this basin from offsite areas to the north. An additional survey of offsite, private property would be required to fully analyze. However, this basin will remain unchanged and follow historic drainage patterns. The existing roadside ditch and culverts will not be affected.

Basin C is 0.26 acres of dirt road. Stormwater from this basin ($Q_5 = 0.5 \text{ cfs } Q_{100} = 1.1 \text{ cfs}$) is collected in a roadside ditch and conveyed to DP1 where two 12" PVC culverts convey the flow to the onsite tributary. Additional flow enters this basin from offsite areas to the west. An additional survey of offsite, private property would be required to fully analyze. However, this basin will remain unchanged and follow historic drainage patterns. The existing roadside ditch and culverts will not be affected. Provide a basin and description for offsite

flows from the south of the project site C. Proposed Suppasin Description

The project w Will still need to assume an increased unchanged. imperviousness for future home and driveway and what those increase in flows will be.

III. Drainage Design Criteria

a. Development Criteria Reference

The drainage analysis, proposed storm sewer system, and proposed private, full spectrum water quality and detention pond follow the criteria from the "Drainage Criteria Manual County of El Paso, Colorado" Volumes 1 and 2," as amended.

b. Hydrologic Criteria

Hydrologic data was obtained from the NOAA Atlas 14 for the site area. Onsite drainage analysis included the 5-year storm (minor event) and 100-year storm (major event) using 1-hr duration rainfall depths from NOAA Atlas 14. Runoff was calculated per EPCDCM Chapter 5 – Storm Runoff Method of Analysis.

d. Hydraulic Criteria

Hydraulic criteria for channel analysis was obtained from EPCDCM Chapter 10 - Open Channels and Structures.

Outfall tailwater conditions into detention facilities are based on the hydraulic grade line in the pond at the time of concentration of the tributary basin. The hydraulic grade line is adapted from the UD-Detention Drain Time v. Ponding Depth graph presented in Appendix D.

There is no pond and there was no graph provided in appendix D.

describe the ability of the ditch along the west side of Black Squirrel Road to handle the flows from the site and whether improvements will be needed.



IV. Drainage Facility Design

a. General Concept

The site will remain in its existing condition. No stormwater improvements will be made in conjunction with this FDR.

b. Water Quality & Detention

The site will not require water quality and detention. The site is comprised of 5+ acre lots with a total impervious of 7% and is excluded from permanent stormwater quality control measures per the Large Lot Single Family Sites exclusion in Appendix I of the EPC DCM. A PBMP Applicability map is presented in Appendix F. Appendix I does not relinquish detention

requirements. Please address increased flows with

future home and driveway construction. See

c. Major Drainag appendix for additional comments.

There are no major drainageways that traverse the site. The existing onsite drainage tributary is stable and will not receive an increase in flows from this project. Major channel appears to be cutting

through the middle of the site.

d. Operations & Maintenance

An Operations and Maintenance Manual will not be required as no stormwater facilities are proposed. Who will maintain drainage channel?

e. Grading & Erosion Control Plan

A Grading and Erosion Control plan is not required as no land disturbance will occur with this project.

f. Four Step Method

Step 1 – Reducing Runoff Volumes: Existing roof drains route across landscape areas whenever possible to promote infiltration. In addition, a vegetated drainage tributary captures and conveys stormwater to the historic outfall at the northeast corner of the site. An IRF spreadsheet is presented in Appendix B.

Step 2 – Treat and slowly release the WQCV: The site is exempt from permanent water quality per the Large Lot Single Family Site exclusion in Appendix I of the EPC DCM.

Step 3 – Stabilize stream channels: All new and re-development projects are required to construct or participate in the funding of channel stabilization measures. Drainage basin fees paid, at the time of platting, go towards channel stabilization with the drainage basin. However, the site is within the West Kiowa Creek Drainage Basin which does not have established basin or bridge fees.

Step 4 – Consider the need Bor source controls: No industrial or commercial uses are proposed within this development and therefore no source controls are proposed.

g. Drainage Basin & Bridge Fees

The site is within the West Kiowa Creek Drainage Basin which does not have established basin or bridge fees. Therefore, no drainage fees will be paid at time of platting.



h. Engineer's Opinion of Probable Cost

An OPC will not be provided as there are no improvements associated with this FDR.

V. Summary

CHRIS TEAM SUBDIVISION remains consistent with pre-development drainage conditions. The proposed development will not adversely affect downstream stormwater infrastructure or surrounding developments. This report is in accordance with the latest El Paso County Drainage criteria.

VI. References

Please include date

- 1. El Paso County Drainage Criteria Manual, latest revision.
- 2. Urban Storm Drainage Criteria Manual, Mile High Flood District, March 2024.



CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX A – VICINITY MAP, FEMA MAP, NRCS WEB SOIL SURVEY & NOAA ATLAS 14





Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
|---------------------------|--|--------|--------------|----------------|
| 26 | Elbeth sandy loam, 8 to 15 percent slopes | В | 7.0 | 100.0% |
| Totals for Area of Intere | st | | 7.0 | 100.0% |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

USDA

National Flood Hazard Layer FIRMette



Legend



Basemap Imagery Source: USGS National Map 2023

Precipitation Frequency Data Server



Location name: Colorado Springs, Colorado, USA* Latitude: 39.0977°, Longitude: -104.6314° Elevation: 7471 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

NOAA Atlas 14, Volume 8, Version 2

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

| PDS- | based po | int precip | itation fre | quency es | stimates v | vith 90% c | confidenc | e interva | als (in ind | ches) ¹ | | | | | |
|----------|---|---|----------------------------|----------------------------|----------------------------|----------------------------|----------------------------|----------------------------|----------------------------|----------------------------|--|--|--|--|--|
| Duration | Average recurrence interval (years) 1 2 5 10 25 50 100 200 500 1000 | | | | | | | | | | | | | | |
| Duration | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 | | | | | |
| 5-min | 0.240 | 0.240 0.291 0.379 0.456 | | 0.570 | 0.662 | 0.760 | 0.864 | 1.01 | 1.12 | | | | | | |
| | (0.188-0.306) | (0.228-0.371) (0.296-0.485) (0.355-0.587) | | (0.431-0.764) | (0.489-0.898) | (0.542-1.05) | (0.592-1.23) | (0.665-1.47) | (0.720-1.65) | | | | | | |
| 10-min | 0.351 | 0.425 | 0.554 | 0.668 | 0.834 | 0.970 | 1.11 | 1.26 | 1.48 | 1.64 | | | | | |
| | (0.276-0.448) | (0.334-0.544) | (0.433-0.710) | (0.519-0.860) | (0.631-1.12) | (0.716-1.32) | (0.794-1.54) | (0.866-1.80) | (0.973-2.15) | (1.05-2.42) | | | | | |
| 15-min | 0.428 | 0.519 | 0.676 | 0.815 | 1.02 | 1.18 | 1.36 | 1.54 | 1.80 | 2.00 | | | | | |
| | (0.336-0.547) | (0.407-0.663) | (0.529-0.866) | (0.633-1.05) | (0.770-1.36) | (0.873-1.60) | (0.969-1.88) | (1.06-2.19) | (1.19-2.63) | (1.28-2.95) | | | | | |
| 30-min | 0.607 | 0.736 | 0.957 | 1.15 | 1.44 | 1.67 | 1.91 | 2.17 | 2.52 | 2.81 | | | | | |
| | (0.477-0.775) | (0.577-0.940) | (0.749-1.23) | (0.896-1.48) | (1.09-1.92) | (1.23-2.26) | (1.36-2.65) | (1.48-3.08) | (1.66-3.68) | (1.80-4.14) | | | | | |
| 60-min | 0.768 | 0.922 | 1.20 | 1.44 | 1.81 | 2.12 | 2.45 | 2.80 | 3.30 | 3.70 | | | | | |
| | (0.603-0.981) | (0.724-1.18) | (0.935-1.53) | (1.12-1.86) | (1.38-2.44) | (1.57-2.88) | (1.75-3.40) | (1.92-3.99) | (2.18-4.82) | (2.37-5.45) | | | | | |
| 2-hr | 0.928 | 1.11 | 1.43 | 1.73 | 2.19 | 2.57 | 2.98 | 3.43 | 4.07 | 4.59 | | | | | |
| | (0.735-1.18) | (0.877-1.40) | (1.13-1.82) | (1.36-2.21) | (1.68-2.93) | (1.92-3.48) | (2.15-4.13) | (2.38-4.87) | (2.72-5.92) | (2.97-6.72) | | | | | |
| 3-hr | 1.01 (0.805-1.28) | 1.20 (0.953-1.51) | 1.55 (1.22-1.95) | 1.87 (1.47-2.38) | 2.38 (1.84-3.18) | 2.81 (2.11-3.80) | 3.29 (2.39-4.54) | 3.81 (2.66-5.39) | 4.56 (3.06-6.62) | 5.17 (3.36-7.54) | | | | | |
| 6-hr | 1.181.38(0.941-1.46)(1.10-1.72) | | 1.76 (1.41-2.21) | 2.14 (1.70-2.69) | 2.73 (2.13-3.64) | 3.24 (2.46-4.36) | 3.81 (2.80-5.24) | 4.44 (3.13-6.26) | 5.36 (3.64-7.75) | 6.12 (4.02-8.88) | | | | | |
| 12-hr | 1.37 | 1.60 | 2.04 | 2.47 | 3.14 | 3.72 | 4.36 | 5.08 | 6.12 | 6.97 | | | | | |
| | (1.11-1.70) | (1.29-1.98) | (1.64-2.54) | (1.97-3.07) | (2.47-4.14) | (2.85-4.95) | (3.23-5.95) | (3.61-7.10) | (4.19-8.77) | (4.62-10.0) | | | | | |
| 24-hr | 1.61 | 1.88 | 2.40 | 2.88 | 3.62 | 4.26 | 4.95 | 5.72 | 6.81 | 7.71 | | | | | |
| | (1.31-1.97) | (1.53-2.31) | (1.94-2.94) | (2.32-3.55) | (2.86-4.72) | (3.28-5.60) | (3.69-6.68) | (4.10-7.91) | (4.70-9.69) | (5.16-11.0) | | | | | |
| 2-day | 1.88 (1.54-2.27) | 2.22 (1.81-2.69) | 2.82 (2.30-3.43) | 3.37 (2.74-4.12) | 4.20 (3.33-5.38) | 4.88 (3.78-6.34) | 5.62 (4.22-7.49) | 6.42 (4.63-8.79) | 7.55 (5.25-10.6) | 8.46 (5.71-12.0) | | | | | |
| 3-day | 2.05 (1.69-2.47) | 2.43 (2.00-2.93) | 3.10 (2.54-3.75) | 3.70 (3.02-4.50) | 4.59 (3.66-5.84) | 5.32 (4.14-6.86) | 6.10 (4.59-8.07) | 6.94 (5.02-9.44) | 8.11 (5.66-11.4) | 9.04 (6.14-12.8) | | | | | |
| 4-day | 2.20 | 2.60 | 3.31 | 3.94 | 4.87 | 5.63 | 6.44 | 7.30 | 8.51 | 9.48 | | | | | |
| | (1.82-2.64) | (2.15-3.13) | (2.72-3.99) | (3.22-4.77) | (3.89-6.17) | (4.39-7.23) | (4.86-8.48) | (5.31-9.90) | (5.97-11.9) | (6.46-13.4) | | | | | |
| 7-day | 2.60 | 3.03 | 3.77 | 4.43 | 5.41 | 6.22 | 7.08 | 8.00 | 9.29 | 10.3 | | | | | |
| | (2.16-3.10) | (2.51-3.61) | (3.12-4.50) | (3.65-5.32) | (4.35-6.80) | (4.88-7.92) | (5.39-9.26) | (5.86-10.8) | (6.56-12.9) | (7.09-14.5) | | | | | |
| 10-day | 2.97 (2.48-3.52) | 3.42 (2.85-4.05) | 4.20 (3.49-5.00) | 4.90 (4.05-5.86) | 5.93 (4.79-7.41) | 6.78 (5.35-8.59) | 7.68 (5.87-10.0) | 8.63 (6.35-11.6) | 9.98 (7.08-13.8) | 11.0 (7.63-15.5) | | | | | |
| 20-day | 3.99 (3.36-4.68) | 4.60 (3.86-5.40) | 5.61 (4.70-6.61) | 6.48 (5.40-7.67) | 7.71 (6.25-9.49) | 8.69 (6.90-10.9) | 9.70 (7.46-12.5) | 10.7 (7.96-14.2) | 12.2 (8.70-16.6) | 13.3 (9.26-18.5) | | | | | |
| 30-day | 4.81 (4.06-5.61) | 5.55 (4.69-6.48) | 6.76 (5.70-7.92) | 7.77 (6.51-9.15) | 9.17 (7.44-11.2) | 10.2 (8.15-12.7) | 11.3 (8.74-14.4) | 12.4 (9.23-16.3) | 13.9 (9.96-18.8) | 15.0 (10.5-20.7) | | | | | |
| 45-day | 5.80 | 6.70 | 8.13 | 9.29 | 10.9 | 12.0 | 13.2 | 14.3 | 15.8 | 16.8 | | | | | |
| | (4.93-6.73) | (5.68-7.77) | (6.88-9.46) | (7.82-10.9) | (8.83-13.1) | (9.60-14.8) | (10.2-16.6) | (10.7-18.7) | (11.4-21.2) | (11.9-23.2) | | | | | |
| 60-day | 6.63 (5.65-7.65) | 7.63 (6.49-8.82) | 9.22 (7.82-10.7) | 10.5 (8.85-12.2) | 12.2 (9.91-14.6) | 13.4 (10.7-16.3) | 14.6 (11.3-18.3) | 15.7 (11.8-20.4) | 17.2 (12.4-23.0) | 18.2 (12.9-25.0) | | | | | |

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical



Large scale terrain





Large scale aerial



CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX B – HYDROLOGIC CALCULATIONS

| Subdivision: | Black Squirrel | Black Squirrel Road | | | | | | | | |
|-----------------|---------------------|---------------------|--|--|--|--|--|--|--|--|
| Location: | El Paso Count | El Paso County | | | | | | | | |
| Project Name: | Black Squirrel Road | | | | | | | | | |
| Project Number: | 24019 | | | | | | | | | |
| Calculated By: | NQJ | | | | | | | | | |
| Checked By: | | | | | | | | | | |
| Date: | 8/27/2024 | | | | | | | | | |
| | | | | | | | | | | |

| | EX DRAINAGE CALCS - BASIN SUMMARY TABLE | | | | | | | | | | | | | | |
|-----------|---|---------|------|------|----------------|-----|-------------------------|--|--|--|--|--|--|--|--|
| Tributary | Area | Percent | | | t _c | Q₅ | Q ₁₀₀ | | | | | | | | |
| Sub-basin | basin (acres) Impervious C ₅ C ₁₀₀ (min) (cfs) (cfs | | | | | | | | | | | | | | |
| А | 19.20 | 9% | 0.14 | 0.39 | 43.2 | 5.0 | 24.4 | | | | | | | | |
| В | 0.49 | 44% | 0.37 | 0.55 | 23.9 | 0.3 | 1.3 | | | | | | | | |
| С | 0.26 | 80% | 0.59 | 0.70 | 15.2 | 0.5 | 1.1 | | | | | | | | |

| DESIGN POINT SUMMARY TABLE | | | | | | | | | | | | | |
|----------------------------|-------------------|----------------------------|--|--|--|--|--|--|--|--|--|--|--|
| DP# | Q _{5-YR} | Q _{100-YR} | | | | | | | | | | | |
| 1 | 1 0.5 1.1 | | | | | | | | | | | | |
| 2 | 5.4 | 25.0 | | | | | | | | | | | |
| 3 | 0.3 | 1.3 | | | | | | | | | | | |

COMPOSITE % IMPERVIOUS CALCULATIONS - EXISTING CONDITIONS

| Subdivision: | Black Squir | rel Road | | | | Project Name: Black Squirrel Road | | | | | | | | | | | | | |
|--------------|--|----------|------|-----------|--|--|------|-----------|--------|------|------|-----------|--------|-----------|---------------------|------------|--|--|--|
| Location: | El Paso Cou | unty | | | | Project No.: 24019.00 | | | | | | | | | | | | | |
| | Calculated By: NQJ | | | | | | | | | | | | | | | | | | |
| | | | | | No ex | No existing imperviousness Checked By: | | | | | | | | | | | | | |
| | Date: 8/27/24 | | | | | | | | | | | | | | | | | | |
| | | | | | obser | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |
| | Gravel Drives Roofs Forest Weighted C. & C | | | | | | | | | | | | | | Basins Total | | | | |
| Basin ID | Total Area | C. | Cran | Area (ac) | % Imn | C. | Com | Area (ac) | % Imn | C. | Cran | Area (ac) | % Imn | Treightee | - e5 a e100 | Weighted % | | | |
| Busin iB | (ac) | | 0100 | | ,, iiip. | 05 | 0100 | Alea (ae) | 70 mp. | •, | 0100 | | 70 mp. | C₅ | C ₁₀₀ | Imp. | | | |
| А | 19.20 | 0.59 | 0.70 | 1.32 | 80.0% | 0.73 | 0.81 | 0.33 | 90.0% | 0.09 | 0.36 | 17.55 | 0.0% | 0.14 | 0.39 | 7.0% | | | |
| В | 0.49 | 0.59 | 0.70 | 0.27 | 80.0% | 0.73 | 0.81 | 0.00 | 90.0% | 0.09 | 0.36 | 0.22 | 0.0% | 0.37 | 0.55 | 44.0% | | | |
| C | 0.26 | 0.59 | 0.70 | 0.26 | 80.0% 0.73 0.81 0.00 90.0% 0.09 0.36 0.00 0.0% 0.59 0.70 | | | | | | | | | | | | | | |
| Total | 19.95 | | | | | | | | | | | | | | | 8.9% | | | |

| Need to provide proposed conditions |
|--|
| hydrology calculations. Assume a 10% |
| imperviousness for 5 acre+ lots or |
| show/estimate house footprint, driveway, |
| etc. |

STANDARD FORM SF-2 - EXISTING CONDITIONS TIME OF CONCENTRATION

| Sub | division: | Black Squirr | el Road | | | | | | Proje | t Name: | Black Squi | rrel Road | | | | | |
|-----------------------|--|---------------------------------------|----------|----------------|-----------------|---|-------------------------------------|------------------------------------|-----------------------|-----------------|---------------|----------------|---------------------------|------------------|--------------------------|----------------|--|
| L | ocation: | El Paso Cou | nty | | | | | | Pro | oject No.: | 24019.00 | | | | - | | |
| | | | N | ote: Rural a | reas can | have | | | Calcu | lated By: | NQJ | | | | - | | |
| | | | ar | n initial leng | th of up t | to | | | Che | cked By: | | | | | - | | |
| | | | 30 | 00 feet | | | | | | Date: | 8/27/24 | | | | - | | |
| | | | | | | | | | | | | | | | | | |
| | SUB-BASIN INITIAL/OVERLAND TRAVEL TIME tc CHECK | | | | | | | | | | | | | | | | |
| | DATA (T _i) (T _t) (URBANIZED BA | | | | | | | | | | | | | | SINS) | FINAL | |
| BASIN | D.A. | Hydrologic | Weighted | Impervious | L | S _o | t, | L _t | S _t | К | VEL. | t _t | COMP. t _c | TOTAL | Urbanized t _c | t _c | |
| ID | (ac) | Soils Group | C₅ | (%) | (ft) | (%) | (min) | (ft) | (%) | | (ft/s) | (min) | (min) | LENGTH (ft) | (min) | (min) | |
| А | 19.20 | В | 0.14 | 8.9% | 100 | 5.0% | 10.2 | 1540 | 1.8% | 2.5 | 0.3 | 76.5 | 86.8 | 1640.0 | 43.2 | 43.2 | |
| В | 0.49 | В | 0.37 | 44% | 17 | 17 2.0% 4.4 1058 4.7% 2.5 0.5 32.6 37.0 1075.0 23 | | | | | | | | | | | |
| С | C 0.26 B 0.59 80% 11 2.0% 2.4 635 3.5% 2.5 0.5 22.6 25.1 646.0 | | | | | | | | | | | | | | 15.2 | 15.2 | |
| NOTES: | | | | | | | | | | | | | | | | | |
| $t_c =$ | $t_i + t_t$ | | | | Eq | $t = \frac{0.395}{1000000000000000000000000000000000000$ | $(1.1-C_5)\sqrt{L_i}$ | | | | Equation 6- | 3 | Tabl | e 6-2. NRCS Conv | veyance factors, K | | |
| Where: | | | | | | -1 | S ₀ ^{0.33} | | | | -1 | | I ype of Land Heavy me | adow | 2.5 | | |
| | | - F | | | Where | e: | | | | | | | Tillage/ | field | 5 | | |
| $l_c = co$ | inputed time | of concentration (i | minutes) | | | $t_i = overlan$ | nd (initial) flow t | ime (minutes) | | | | | Short pasture | and lawns | 7 | | |
| $t_i = ov$ | erland (initia | flow time (minut) | tes) | | | $C_5 = runofi$ | f coefficient for 5 | 5-year frequenc | y (from Table 6 | 5-4) | | | Nearly bare | ground | 10 | | |
| $t_t = ch$ | annelized flo | w time (minutes). | | | | $S_o = average$ | ge slope along the | e overland flow | path (ft/ft). | | | | Grassed wa | aterway | 15 | | |
| $t_{i} = -$ | | L_t | | Ea | uation 6-4 ?6 - | -17 <i>i</i>)+ | | | | | Equation 6-5 | Pave | d areas and shall | low paved swales | 20 | | |
| ' 6 | $50K\sqrt{S_o}$ | $60V_t$ | | | | 60(14 | $4i + 9)\sqrt{S_t}$ | | | | • | | | | | | |
| Where: | | | | | 2 | | | | | | | | | | | | |
| $t_t = cha$ | nnelized flow | time (travel time, n | nin) | | t. | = minimum | time of concentr | ration for first d | lesion point w | en less than t- | from Equation | 6-1 | | | | | |
| $L_t = wa$ So = wa | terway length | n (ft) (ft/ft) | | | L | t = length of 0 | channelized flow | v path (ft) | kesign point wi | | Hom Equation | 0-1. | | | | | |
| $V_t = \text{tra}$ | vel time velo | city (ft/sec) = $K\sqrt{S_o}$ | 6 2) | | i S | = impervious = slope of th | ness (expressed e channelized fl | as a decimal) low path (ft/ft). | | | | | | | | | |
| K = NK | CS conveyan | ice factor (see Table | 0-2). | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | |

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

| | | | | | | | | | | | | | STV | | | | | -3 - F | | | : | דוחאר | |
|---|---|---------|---------|-----------|-----------|---------|-----------|----------|---------|------|------|-----|-----|--|----|-------|---------|--------|--------|--------|-------|-------|---|
| | | | | | | | | | | | | | 317 | | | | | -3 - L | | | | | 10113 |
| | | | | | | | | | | | | | | | | | | AGE | | | 105 | GN | |
| | | | | | | | | | | | | | | | (R | ATION | NAL IVI | ETHO | DPRO | JCEDU | JRE) | | |
| | | | | | | | | | | | | | | | | Pro | oject N | Name: | Blac | k Squi | irrel | Road | |
| Subdivision | : Black | Squirr | el Road | I | | | | | | | | | | | | | Proje | ct No. | : 2401 | 19.00 | | | |
| Location: El Paso County Calculated By: NQJ | | | | | | | | | | | | | | | | | | | | | | | |
| Design Storm: 5-Year Checked By: | | | | | | | | | | | | | | | | | | | | | | | |
| | Date: 8/27/24 | | | | | | | | | | | | | | | | | | | | | | |
| | DIRECT RUNOFF TOTAL RUNOFF STREET PIPE TRAVEL TIME | | | | | | | | | | | | | | | | | | | | | | |
| | DIRECT RUNOFF TOTAL RUNOFF STREET PIPE TRAVEL TIME | | | | | | | | | | | | | | | | | | | | | | |
| STREET | esign Point asin ID krea (Ac) c (min) c (min) | | | | | | | | | | | | | | | | | | | | | | |
| | 2 | А | 19.20 | 0.14 | 43.2 | 2.60 | 1.94 | 5.0 | | | | | | | | | | | | | | | BASIN A HISTORIC FLOW, CONVEYED IN EXISTING CHANNEL TO DP2 |
| | 3 | В | 0.26 | 0.37 | 23.9 | 0.09 | 2.82 | 0.3 | | | | | | | | | | | | | | | BASIN B HISTORIC FLOW, CONVEYED IN ROADSIDE DITCHES/CULVERTS TO DP3 |
| | 1 | с | 0.26 | 0.59 | 15.2 | 0.15 | 3.50 | 0.5 | | | | | | | | | | | | | | | BASIN C HISTORIC FLOW TO EX 12" PVC CULVERTS @ DP1, FLOWS ONSITE TO DP2 |
| | 2 | 1 | | | | | | | 58.4 | 2.75 | 1.48 | 5.4 | | | | | | | | | | | TOTAL FLOW AT DP2 |
| Notes: Street and Pipe C* | *A value | es ared | etermin | ned by O/ | /iusing t | he cato | hment's i | ntensity | / value | | | | | | | | | | | | | | |

DP 1 should also account for off site flow being conveyed through the culverts.

| | | | | | | | | | | | | | ST | AND | ARD STO | D FOI DRM (RATI | RM S DRAI ONAL | NAG METH | - EXI E SYS HOD PI | STIN STEM ROCEI | IG CO DESI DURE) | OND GN | ITIONS |
|--|--------------|----------|-----------|---------------|-------------|----------|-----------|----------|----------|----------|-----------|---------|---------------------------|----------|------------|-------------------------|----------------------|--------------------|--------------------------|-----------------------|------------------------|-------------|---|
| | | | | | | | | | | | | | | | | Pro | ject N | ame: | Black | k Squi | rel Ro | ad | |
| Subdivision | : Black | Squirr | el Roac | 1 | | | | | | | | _ | | | | I | Projec | t No. | : 2401 | 9.00 | | | |
| Location | El Pas | o Cou | nty | | | | | | | | | _ | | | | Cal | culate | d By: | NQJ | | | | |
| Design Storm: | : 100-Y | ear | | | | | | | | | | - | | | | С | hecke | d By: | | | | | |
| | | | | | | | | | | | | | | | | | 1 | Date: | 8/27 | /24 | | | |
| | T | 1 | | DIDE | | NOFE | | | т | 0741 | | | | TOFF | - | 1 | DI | DE | | TDAY | /51 70 | | |
| | | | 1 | DIRE | | NOFF | | | | | RUNU | FF | 2 | TREE | | | PI | PE | 1 | TRA | | VIE | |
| STREET | Design Point | Basin ID | Area (ac) | Runoff Coeff. | t_c (min) | C*A (ac) | / (in/hr) | Q (cfs) | tc (min) | C*A (ac) | / (in/hr) | Q (cfs) | Q _{street} (cfs) | C*A (ac) | Slope (%) | Q _{pipe} (cfs) | C*A (ac) | Slope (%) | Pipe Size (inches) | Length (ft) | Velocity (fps) | t_t (min) | REMARKS |
| | 2 | А | 19.20 | 0.39 | 43.2 | 7.51 | 3.25 | 24.4 | | | | | | | | | | | | | | | BASIN A HISTORIC FLOW, CONVEYED IN EXISTING CHANNEL TO DP2 |
| | 3 | в | 0.49 | 0.55 | 23.9 | 0.27 | 4.74 | 1.3 | | | | | | | | | | | | | | | BASIN B HISTORIC FLOW, CONVEYED IN ROADSIDE DITCHES/CULVERTS TO DP3 |
| | 1 | С | 0.26 | 0.70 | 15.2 | 0.18 | 5.88 | 1.1 | | | | | | | | | | | | | | | BASIN C HISTORIC FLOW TO EX 12" PVC CULVERTS @ DP1, FLOWS ONSITE TO DP2 |
| | 2 | | | | | | | | 43.2 | 7.69 | 3.25 | 25.0 | | | | | | | | | | | TOTAL FLOW AT DP2 |
| Notes: Street and Pipe C ³ | *A value | es are o | determi | ned by | Q/i usii | ng the o | catchme | ent's in | tensity | / value. | | | - | | | - | | | | | | | |

| Design Procedure Form: Runoff Reduction | | | | | | | | | | | | |
|--|--|---------------------------------|-----------------|--------------|-----------------|---------------|----------------|---|---|---|---|--|
| | | | | UD-BMP (| Version 3.07, M | arch 2018) | | | | | | Sheet 1 of 1 |
| Designer: | NQJ | | | | | | | | | | | |
| Company: | ALL TERRAIN | | G | | | | | | | | - | |
| Date: | August 29, 20 | ugust 29, 2024 | | | | | | | | | | |
| Project: | 18412-18440 | 18412-18440 BLACK SQUIRREL ROAD | | | | | | | | | | |
| Location: | Location: EL PASO COUNTY | | | | | | | | | | | |
| | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| SITE INFORMATION (Use | er Input in Blu | ue Cells) | | - | | | | | | | | |
| | WQCV Rainfall Depth 0.60 inches | | | | | | | | | | | |
| Depth of Average Ru | Depth of Average Runoff Producing Storm, d ₆ = 0.43 linches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3) | | | | | | | | | | | |
| Area Tura | CD A | | | | | | | | | | | |
| Area Type | SPA 1 CDA | | | | | | | | | | | |
| Area ID | 1-5PA | 1-UIA:RPA | | | | | | | | | | |
| Downstream Design Point ID | 1 Nore | 1 None | | | | | | | | | | |
| Downstream BMP Type | None | None | | - | | | | | | | | |
| DCIA (π) | | | | | | | | | | | | |
| UIA (IL) | | 28,000 | | - | 1 | | | | | | | |
| RFA (II) | 80.000 | 20,000 | | | | | | | | | | —————————————————————————————————————— |
| | 0% | 0% | | | | | | | | | | —————————————————————————————————————— |
| HSG B (%) | 100% | 100% | | | | | | | | | | |
| HSG C/D (%) | 0% | 0% | | | | | | | | | | |
| Average Slope of RPA (ft/ft) | | 0.040 | | 1 | 1 | | | | | | | |
| UIA:RPA Interface Width (ft) | | 75.00 | | | | | | | | | | |
| | | | | | L | | 1 | 1 | 1 | | 1 | |
| | | | | | | | | | | | | |
| CALCULATED RUNOFF | RESULTS | | | | | | | | | | | |
| Area ID | 1-SPA | 1-UIA:RPA | | | | | | | | | | |
| UIA:RPA Area (ft ²) | | 42,209 | | | | | | | | | | |
| L / W Ratio | | 7.50 | | | | | | | | | | |
| UIA / Area | | 0.3366 | | | | | | | | | | |
| Runoff (in) | 0.00 | 0.00 | | | | | | | | | | |
| Runoff (ft ³) | 0 | 0 | | | | | | | | | | |
| Runoff Reduction (ft ³) | 4000 | 592 | | | | | | | | | | |
| | | | | | | | | | | | | |
| CALCULATED WQCV RE | SULTS | | | | | 1 | | | | I | | |
| Area ID | 1-SPA | 1-UIA:RPA | | | | | | | | | | |
| WQCV (ft ³) | 0 | 592 | | | | | | | | | | |
| WQCV Reduction (ft ³) | 0 | 592 | | - | - | | | | | | | |
| WQCV Reduction (%) | 0% | 100% | | - | - | | | | | | | |
| Untreated WQCV (ft°) | 0 | 0 | | | | | | | | | | |
| | | TS (sums rea | ulte from oll | columne wit | h the same D | ownetream Dee | ion Point ID) | | | | | |
| Downstream Design Point ID | | | unto n'Offi all | Columns wit | n die same D | ownsueam Des | ign Follit ID) | | | | | |
| | 0 | 0 | | | | | | | | | | |
| | 14 209 | 14 209 | | | | | | | | | | |
| BPA (#2) | 28 000 | 28,000 | | | | | | | | | | |
| SPA (#2) | 80,000 | 80,000 | - | t | t | | | | | | | |
| Total Area (ft ²) | 122.209 | 122.209 | - | t | t | | | | | | | |
| Total Impervious Area (ft ²) | 14,209 | 14,209 | | 1 | 1 | İ | | | | | | |
| WQCV (ft ³) | 592 | 592 | | 1 | 1 | l | | | | | | |
| WQCV Reduction (ft ³) | 592 | 592 | | İ | İ | | | | | | | |
| WQCV Reduction (%) | 100% | 100% | | l I | 1 | | | | | | | |
| Untreated WQCV (ft 3) | 0 | 0 | | | | | | | | | | |
| | <u> </u> | | | | | | | | | | | |
| CALCULATED SITE RES | ULTS (sums | results from a | all columns i | n worksheet) | | | | | | | | |
| Total Area (ft ²) | 244,418 |] | | | | | | | | | | |
| Total Impervious Area (ft ²) | 28,418 |] | | | | | | | | | | |
| WQCV (ft ³) | 592 |] | | | | | | | | | | |
| WQCV Reduction (ft ³) | 592 |] | | | | | | | | | | |
| WQCV Reduction (%) | 100% |] | | | | | | | | | | |
| Untreated WQCV (ft ³) | 0 | J | | | | | | | | | | |
| | | | | | | | | | | | | |
| | | | | | | | | | | | | |



CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX C – HYDRAULIC CALCULATIONS

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Aug 29 2024

CHANNEL SECTION 1 (Q100 = 25.0 cfs)

| User-defined | | Highlighted | |
|------------------|-----------|---------------------|---------|
| Invert Elev (ft) | = 7435.02 | Depth (ft) | = 0.47 |
| Slope (%) | = 2.00 | Q (cfs) | = 25.00 |
| N-Value | = 0.030 | Area (sqft) | = 9.51 |
| | | Velocity (ft/s) | = 2.63 |
| Calculations | | Wetted Perim (ft) | = 40.45 |
| Compute by: | Known Q | Crit Depth, Yc (ft) | = 0.47 |
| Known Q (cfs) | = 25.00 | Top Width (ft) | = 40.44 |
| . , | | EGL (ft) | = 0.58 |

(Sta, El, n)-(Sta, El, n)... (0.00, 7441.94)-(56.94, 7436.00, 0.030)-(117.21, 7435.02, 0.030)-(129.46, 7435.52, 0.030)-(157.89, 7436.00, 0.030)-(213.09, 7440.31, 0.030)



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Aug 29 2024

CHANNEL SECTION 2 (Q100 = 25.0 cfs)

| User-defined | | Highlighted | |
|------------------|-----------|---------------------|---------|
| Invert Elev (ft) | = 7430.00 | Depth (ft) | = 0.29 |
| Slope (%) | = 0.60 | Q (cfs) | = 25.00 |
| N-Value | = 0.030 | Area (sqft) | = 16.13 |
| | | Velocity (ft/s) | = 1.55 |
| Calculations | | Wetted Perim (ft) | = 58.73 |
| Compute by: | Known Q | Crit Depth, Yc (ft) | = 0.19 |
| Known Q (cfs) | = 25.00 | Top Width (ft) | = 58.69 |
| | | EGL (ft) | = 0.33 |

(Sta, El, n)-(Sta, El, n)... (0.00, 7436.38)-(45.58, 7430.00, 0.030)-(98.08, 7430.00, 0.030)-(169.15, 7435.00, 0.030)



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, Aug 29 2024

CHANNEL SECTION 3 (Q100 = 25.0 cfs)

| User-defined | | Highlighted | |
|------------------|-----------|---------------------|---------|
| Invert Elev (ft) | = 7424.00 | Depth (ft) | = 0.49 |
| Slope (%) | = 0.49 | Q (cfs) | = 25.00 |
| N-Value | = 0.030 | Area (sqft) | = 12.91 |
| | | Velocity (ft/s) | = 1.94 |
| Calculations | | Wetted Perim (ft) | = 30.98 |
| Compute by: | Known Q | Crit Depth, Yc (ft) | = 0.33 |
| Known Q (cfs) | = 25.00 | Top Width (ft) | = 30.92 |
| | | EGL (ft) | = 0.55 |

(Sta, El, n)-(Sta, El, n)... (0.00, 7428.24)-(59.11, 7424.00, 0.030)-(80.87, 7424.00, 0.030)-(99.81, 7428.00, 0.030)-(137.27, 7430.09, 0.030)





CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX D – WATER QUALITY & DETENTION

Post Construction Stormwater Management Applicability Evaluation Form

This form is to be used by the Engineer of Record to evaluate applicable construction activities to determine if the activities are eligible for an exclusion to permanent stormwater quality management requirements. Additionally Part III of the form is used to identify and document which allowable control measure design standard is used for the structure.

| Part I. Project Information | | | | | |
|---|---|--|--|--|--|
| 1. Project Name: | | | | | |
| 2. El Paso County Project #: | 3. ESQCP #: | | | | |
| 4. Project Location: | Project Location in MS4 Permit Area (Y or N): | | | | |
| 5. Project Description: | | | | | |
| If project is located within the EI Paso County MS4 Permit Area, please provide copy of this completed form | | | | | |

to the Stormwater Quality Coordinator for reporting purposes; and save completed form with project file.

Part II. Exclusion Evaluation: Determine if Post-Construction Stormwater Management exclusion criteria are met. Note: Questions A thru K directly correlate to the MS4 permit Part I.E.4.a.i (A) thru (K). If Yes, to any of the following questions, then mark Not Applicable in Part III, Question 2. Yes No Not Notes: Ouestions Applicable A. Is this project a "Pavement This exclusion applies to "roadways" only. Areas used primarily for Management Site" as defined in Permit Part I E.4.a.i.(A)? parking or access to parking are not included. B. Is the project "Excluded Roadway Development"? • Does the site add less than 1 acre of paved area per mile? • Does the site add 8.25 feet or less of paved width at any location to the existing roadway? C. Does the project increase the width of For redevelopment of existing the existing roadway by less than 2 times roadways, only the area of the the existing width? existing roadway is excluded from post-construction requirements when the site does not increase the width by two times or more. This exclusion only excludes the original roadway area it does NOT apply to entire project. D. Is the project considered an Activity can NOT permanently alter aboveground and Underground Utilities the terrain, ground cover or drainage patterns from those activity? present prior to the activity Must be a single-residential lot or E. Is the project considered a "Large Lot Single-Family Site"? agricultural zoned land, > 2.5 acres per dwelling and total lot impervious area < 10 percent.

| Questions (cont'd) | Yes | No | Not Applicable | Notes |
|--|-----|----|-------------------|---|
| F. Do Non-Residential or Non-Commercial Infiltration Conditions exist? Post-development surface conditions do not result in concentrated stormwater flow or surface water discharge during an 80 th percentile stormwater runoff event. | | | | Exclusion does not apply to residential or commercial sites for buildings. A site specific study is required and must show: rainfall and soil conditions; allowable slopes; surface conditions; and ratios of imperviousness area to pervious area. |
| G. Is the project land disturbance to Undeveloped Land where undeveloped land remains undeveloped following the activity? | | | | Project must be on land with no human made structures such as buildings or pavement. |
| H. Is the project a Stream Stabilization Site? | | | | Standalone stream stabilization projects are excluded. |
| I. Is the project a bike or pedestrian trail? | | | | Bike lanes for roadways are not included in this exclusion, but may qualify if part of larger roadway activity is excluded in A, B or C above. |
| J. Is the project Oil and Gas Exploration? | | | | Activities and facilities associated with oil and gas exploration are excluded. |
| K. Is the project in a County Growth Area? | | | | Note, El Paso County does not apply this exclusion. All Applicable Construction Activity in El Paso County must comply the Post-Construction Stormwater Management criteria. |

| Part III. Post Construction (Permanent) Stormwater Control Determination | | | | | | |
|---|-----|----|--|--|--|--|
| Questions | Yes | No | | | | |
| 1. Is project an Applicable Construction Activity? | | | | | | |
| 2. Do any of the Exclusions (A-K in Part II) apply? | | | | | | |
| If the project is an Applicable Construction Activity and no Exclusions apply then Post-Construction | | | | | | |
| (Permanent) Stormwater Management is required. | | | | | | |
| Complete the applicable sections of Part IV below and then coordinate signatures for form and place in | | | | | | |
| project file. | | | | | | |
| If the project is not an Applicable Construction Activity, or Exclusion(s) apply then Post-Construction | | | | | | |

(Permanent) Stormwater Management is NOT required. Coordinate signatures for form and place in project file.

| Pa | rt IV: Onsite PWQ Requirements, Documentation and Considerations | Yes | No |
|----|---|-----|----|
| 1. | Check which Design Standard(s) the project will utilize. Standards align with Control Measure Requirements identified in permit Part I.E.4.a.iv. | | |
| Α. | Water Quality Capture Volume (WQCV) Standard | | |
| Β. | Pollutant Removal/80% Total Suspended Solids Removal (TSS) | | |
| С. | Runoff Reduction Standard | Х | |
| D. | Applicable Development Site Draining to a Regional WQCV Control Measure | | |
| E. | Applicable Development Site Draining to a Regional WQCV Facility | | |
| F. | Constrained Redevelopment Sites Standard | | |
| G. | Previous Permit Term Standard | | |
| 2. | Will any of the project permanent stormwater control measure(s) be maintained by another MS4? | | |
| | If Yes, you must obtain a structure specific maintenance agreement with the other MS4 prior to advertisement. | | |
| 3. | Will any of the project permanent stormwater control measures be maintained by a private entity or quasi-governmental agency (e.g. HOA or Special District, respectively)? | | |
| | If Yes, a Private Detention Basin/Stormwater Quality Best Management Practice Maintenance Agreement and Easement must be recorded with the El Paso County Clerk and Recorder. | | |

Part V Notes (attach an additional sheet if you need more space)

Project design is complete to include the project design, construction plans, drainage report, specifications, and maintenance and access agreements as required. The engineering, drainage considerations and information used to complete these documents is complete, true, and accurate to the best of my belief and knowledge.

E005927

08/29/2024

Date

Post-Construction Stormwater Managements in the project design, construction plans, drainage report the project as required, have been reviewed for compliance with the Post Construction Stormwater Management process and MS4 Permit requirements.

Signature of El Paso County Project Engineer

Signature and Stamp of Engineer of Red

Date



CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX E – REFERENCE MATERIAL





Figure 1: Looking SW from Black Squirrel Road to the site low point

Figure 2: Same location at Figure 1, looking NE across Black Squirrel Road







Figure 3: From Black Squirrel Road, looking towards low point

Figure 4: From dirt road along north PL, looking upstream of low point





Figure 5: From dirt road along west PL, looking down drainage tributary



Figure 6: Same location as Figure 5, looking west to offsite/upstream portion of tributary





Figure 7: Double 12" PVC culverts, downstream



Figure 8: Double 12" PVC culverts, upstream





Figure 9: Drainage tributary immediately downstream of 12" PVC culverts





AREA(S) OF CONCERN

(AOC#1): Portions of gravel road lies southerly and easterly of said easement, as graphically depicted on this Land Survey Plat, causing an area of concern.

SURVEYOR'S NOTES

1. NOTICE: According to Colorado law you must commence any legal action based upon any defect in this survey within three years after you first discover such defect. In no event may any action based upon any defect in this survey be commenced more than ten years from the date of the certification shown hereon.

2. Any person who knowingly removes, alters or defaces any public land survey monument or land boundary monument or accessory commits a class 2 misdemeanor pursuant to the Colorado Revised Statute 18-4-508.

3. The lineal units used in this drawing are U.S. Survey Feet.

4. The fieldwork for this survey was completed on May 28, 2024.

5. The overall subject parcel contains a net calculated area of 835,271 square feet (19.18 acres) of land, more or less.

6. This survey does not constitute a title search by Apex Land Surveying and Mapping, LLC. to determine ownership or easements of record. For information regarding easements, rights-of-way and title of record, Apex Land Surveying and Mapping, LLC. relied upon Title Commitment order number RND55116760, with an effective date of 05/24/2024 @ 5:00 P.M. as provided by Land Title Guaranty Company & Old Republic National Title Insurance Company..

7. Bearings are based on a portion of the North line of Section 14, T11S, R65W of the Ute P.M., monumented on the west end with a found No. 6 rebar, rehabilitated with 2-1/2" aluminum cap, T11S R65W 1/4 S13|S14 2024 PLS 38759, flush with grade, and on the east end with a found No. 6 rebar with 2-1/2" aluminum cap marked 1/4S11|S14 1997 PLS 4842, flush with grade and is assumed to bear N89°17'09"E a measured distance of 2,637.40 feet.

8. Any underground or above ground utilities shown hereon have been located from field survey information. Apex Land Surveying and Mapping, LLC. does not guarantee said underground utilities to be shown in their exact location and that said underground utilities are shown in their entirety. Apex Land Surveying and Mapping, LLC. did not physically enter any manholes or inlets to verify size and material. Where additional or more detailed information is required, the client is advised that excavation may be necessary.

9. Site Benchmark: Set 60D nail (Elevation=7459.74' NAVD88).

10. The purpose of this survey is to determine boundary lines of subject parcel for future minor subdivision.

11. Exeption No 13 in title commitment stipulates terms, conditions, provisions, burdens and obligations as set forth in right of way recorded July 09, 1967 under Reception No. 563351 under Book 2202 at Page 117. Said right of way and easement for roadway, utilities, ingress and egress purposes over and across the East 80 feet of that part of the west half of the Southeast quarter of Section 11 in Township 11 South, Range 65 West of the 6th P.M., as graphically depicted on thie Land Survey Plat.

12. Right Of Way Deed per Book 2636 at Page 733 by Reception No. 30371 grants, bargain, sell, and convey the said 80' Strip (40' on either side of centerline) to El Paso County as graphically depicted on this Land Survey Plat. POINT OF INTERSECTION WITH NORTH LINE OF SECTION 14, a distance of 1354.79' (R&C) lands within field measured evidence of intersection of Black Squirrel Road (Gravel road) and private road (gravel road). This document is listed as an "EX" in the vesting deed (Warranty Deed by Reception No. 218044100).

13. Abbreviated Legal Description in vesting Warranty Deed by Reception No. 218044100 Has an address listed as 6275 Montabor Dr, Colorado Springs CO 80918. The address listed in this document is the address for Chris team Living trust, not the physical address of subject parcel.

14. Exception No. 19—Grant of right of way to mountain view electric association, inc. over a portion of subject property as recorded June 5, 2001 under reception No. 201075608. The evidence in this description in this document does not touch the subject parcel.

15. Exception No. 20-Grant of right of way to mountain view electric association, inc. over a portion of subject property as recorded October 2, 2012 under Reception No. 212115628. The evidence in this description does not touch the subject parcel.

16. Exception No. 22-Easement granted to public service company of Colorado, for utility, and incidental purposes, by instrument recorded april 21, 1964, in book 2007 at page 850. The evidence in this description does not touch the subject parcel.



| | August 20, 2 | | | | |
|--------|--------------|----------------|--------------|----|--------------------------|
| No. | | Remarks | Date | Ву | |
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| | | | | | |
| | | | | | LAND SURVEYING AND MAPPI |
| Field: | TJM/DDR | Drawn: TJM/DDR | Checked: DDR | · | PROJECT No.: 24032 |

APEX Land Surveying and Mapping LLC

5855 Lehman Drive, Suite 102 Colorado Springs, CO 80918 Phone: 719-318-0377 E-mail: info@apexsurveyor.com Website: www.apexsurveyor.com SURVEYING AND MAPPING LLC

SHEET 2 OF 2



CHRIS TEAM SUBDIVISION Final Drainage Report Project No: 24019

APPENDIX F – DRAINAGE MAPS



CHRIS TEAM SUBDIVISION

| EX DRAINAGE CALCS - BASIN SUMMARY TABLE | | | | | | | |
|---|---------|------------|-----------------------|-------------------------|----------------|-------|-------------------------|
| Tributary | Area | Percent | | | t _c | Q₅ | Q ₁₀₀ |
| Sub-basin | (acres) | Impervious | C ₅ | C ₁₀₀ | (min) | (cfs) | (cfs) |
| Α | 19.20 | 9% | 0.14 | 0.39 | 43.2 | 5.0 | 24.4 |
| В | 0.49 | 44% | 0.37 | 0.55 | 23.9 | 0.3 | 1.3 |
| С | 0.26 | 80% | 0.59 | 0.70 | 15.2 | 0.5 | 1.1 |

| DESIGN POINT SUMMARY TABLE | | | | | | | |
|----------------------------|-------------------|----------------------------|--|--|--|--|--|
| DP# | Q _{5-YR} | Q _{100-YR} | | | | | |
| 1 | 0.5 | 1.1 | | | | | |
| 2 | 5.4 | 25.0 | | | | | |
| 3 | 0.3 | 1.3 | | | | | |



CHRIS TEAM SUBDIVISION



CHRIS TEAM SUBDIVISION

v1_Drainage Report - Final.pdf Markup Summary

| Arrow (2) | | |
|--|--|--|
| | Subject: Arrow Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 3:27:43 PM Status: Color: Layer: Space: | |
| | Subject: Arrow Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 3:28:00 PM Status: Color: Layer: Space: | |
| Callout (16) | | |
| In pre-onversion trainage conditions, ine proponsion to information infrastructure or aurounding develope i County Dranage criteria. Please include date vision of the please include date vision. Please include date vision. | Subject: Callout Page Label: 7 Author: HaoVo Date: 11/1/2024 8:31:27 AM Status: Color: Color: Color: Space: | Please include date |
| | Subject: Callout Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:12:55 AM Status: Color: Layer: Space: | These flows need to account for the runoff from the property to the west. The downstream flows will need to be updated once this one is corrected. |
| THIS RESW SOIT | Subject: Callout Page Label: 13 Author: Joseph Sandstrom Date: 11/4/2024 8:04:19 AM Status: Color: Layer: Space: | Site |
| | Subject: Callout Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:07:53 AM Status: Color: Layer: Space: | Add flows |

Subject: Callout Note: The culverts should be cleaned out from Page Label: 5 sediment buildup during construction. Author: Joseph Sandstrom Date: 11/4/2024 4:26:06 PM Status: Color: Layer: Space: Subject: Callout Note: Rural areas can have an initial length of up Page Label: 12 to 300 feet Author: CDurham Date: 11/4/2024 1:10:11 PM Status: Color: Layer: Space: _____ Subject: Callout DP 1 should also account for off site flow being Page Label: 13 conveyed through the culverts. Author: CDurham Date: 11/4/2024 1:14:45 PM Status: Color: Layer: Space: Subject: Callout Per site visit, there is currently no improvements Page Label: 4 on site. Author: CDurham Date: 11/4/2024 4:24:42 PM Status: Color: Layer: Space: Subject: Callout If additional survey is not available, something Page Label: 5 such as streamstats could be used to identify the Author: CDurham offsite flow Date: 11/4/2024 2:29:44 PM Status: Color: Layer: Space: Subject: Callout Please expand the view to show contours for Page Label: [1] Ex Drn Map offsite flows entering from the West and South. Author: Joseph Sandstrom Date: 11/4/2024 3:27:10 PM Status: Color: Layer: Space:



Cloud (2)

| | Subject: Cloud Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 3:27:17 PM Status: Color: Layer: Space: | |
|---|---|---|
| | Subject: Cloud Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 3:27:26 PM Status: Color: Layer: Space: | |
| Cloud+ (1) | | |
| TINON TO THE THE THE THE THE THE THE THE THE THE | Subject: Cloud+ Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 3:59:08 PM Status: Color: Layer: Space: | Indicate how flows exit site and show where flows go downstream |
| Highlight (4) | | |
| | | |
| A 2005 | Subject: Highlight Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:07:16 AM Status: Color: Layer: Space: | |
| .20 Q5 Q100 | Subject: Highlight Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:07:17 AM Status: Color: Layer: Space: | |
| 0.49 Q5 B | Subject: Highlight Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:07:22 AM Status: Color: Layer: Space: | |



Subject: Highlight Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 8:07:25 AM Status: Color: Layer: Space:

Image (1)

Subject: Image Page Label: 2 Author: Joseph Sandstrom Date: 10/30/2024 3:00:04 PM Status: Color: Layer: Space:

Polygon (9)



Subject: Polygon Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 12:58:56 PM Status: Color: Layer: Space:



Subject: Polygon Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 12:59:12 PM Status: Color: Layer: Space:



Subject: Polygon Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 12:59:59 PM Status: Color: Layer: Space:



Subject: Polygon Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 1:00:19 PM Status: Color: Layer: Space:





Subject: PolyLine Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 4:05:26 PM Status: Color: Layer: Space:

| | Subject: PolyLine Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 4:05:15 PM Status: Color: Layer: Space: | |
|---|--|--|
| | Subject: PolyLine Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 4:04:46 PM Status: Color: Layer: Space: | |
| 7464 7464 7464 7462 74 74 | Subject: PolyLine Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 4:05:11 PM Status: Color: Layer: Space: | |
| 7416 | Subject: PolyLine Page Label: [1] Ex Drn Map Author: CDurham Date: 11/4/2024 4:05:31 PM Status: Color: Layer: Space: | |
| Text Box (20) | | |
| Vision, Comments from Branko Exploring as in the fact. | Subject: Text Box Page Label: 1 Author: HaoVo Date: 10/28/2024 12:55:12 PM Status: Color: | Info Only: Comments from Service Engineering are in blue text. |
| SF2426 | Space: Subject: Text Box Page Label: 1 Author: HaoVo Date: 10/28/2024 12:56:55 PM Status: Color: Layer: Space: | SF2426 |

Subject: Text Box Page Label: 2 Author: Joseph Sandstrom Date: 10/30/2024 3:00:26 PM Status: Color: Layer: Space: Subject: Text Box Please use the current County signature block: Page Label: 2 Author: Joseph Sandstrom Date: 10/30/2024 2:56:54 PM Status: Color: Layer: Space: Subject: Text Box It appears that the analysis in the written narrative Page Label: 1 and maps/aerials is mixed between the Chris Author: Joseph Sandstrom Team Subdivision property and parcel Date: 11/4/2024 7:47:54 AM 5114006001 to the south. Based on aerials and Status: site visit there is no existing driveway or structures Color: on the applicant's property. Layer: Space: Subject: Text Box Not project site Page Label: 10 Author: Joseph Sandstrom Date: 11/4/2024 8:04:46 AM Status: Color: Layer: Space: Subject: Text Box No existing driveway or buildings observed on this Page Label: [1] Ex Drn Map property. Driveway and building footprints appears Author: Joseph Sandstrom to match the property to the south. Date: 11/4/2024 8:06:49 AM Status: Color: Layer: Space: Subject: Text Box No existing imperviousness observed on the site. Page Label: 11 Author: Joseph Sandstrom Date: 11/4/2024 8:14:29 AM Status: Color: Layer: Space:

| More sub hered need to be encounts accurately advinces the sets. | Subject: Text Box Page Label: [1] Ex Drn Map Author: Joseph Sandstrom Date: 11/4/2024 10:27:02 AM Status: Color: Layer: Space: | More sub basins need to be shown to accurately delineate the site. |
|---|--|--|
| New ILIA | Subject: Text Box Page Label: 11 Author: CDurham Date: 11/4/2024 1:45:04 PM Status: Color: Layer: Space: | Need to provide proposed conditions hydrology calculations. Assume a 10% imperviousness for 5 acre+ lots or show/estimate house footprint, driveway, etc. |
| And 0.2 X More a data and the analysis of the Market term of the match de that the data any used to 20% where the 12 MV Additional three waters that have been utilities and the three states of the states of the states of the term of the market states of the states of the states of the states of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the states of the any of the states of the any of the states of the any of the states of the any of the states of the any o | Subject: Text Box Page Label: 5 Author: CDurham Date: 11/4/2024 2:30:19 PM Status: Color: Layer: Space: | Provide a basin and description for offsite flows from the south of the project site |
| <text><text><text></text></text></text> | Subject: Text Box Page Label: 5 Author: CDurham Date: 11/4/2024 3:34:43 PM Status: Color: Layer: Space: | Include a discussion of the existing 12" culvert at the NW corner of the site, as shown on the drainage map |
| : grade line in the ne is adspect from ¹ . There is noor and there was no graph provided in appendix D. 3 | Subject: Text Box Page Label: 5 Author: CDurham Date: 11/4/2024 3:35:42 PM Status: Color: Layer: Space: | There is no pond and there was no graph provided in appendix D. |
| v frost News coherts. Integration will remain Iow point an DP2. The low "Diffection shaft Reports to Porton point and the second shaft Reports to Porton point and the second shaft Report Porton Shaft Reports and Merris do and the property would be fortic dramage patterns. | Subject: Text Box Page Label: 5 Author: CDurham Date: 11/4/2024 3:49:31 PM Status: Color: Layer: Space: | Discuss what happens to flows from this basin. How do they cross the roads and where do they do downstream? |

| the vend the required to help analyse. Nearest, the base of partners. The search produced dist and culores will and Subbasen Description of the search and search and search and the search and search and search and many search and search and search and many search and search and search and many search and se | Subject: Text Box Page Label: 5 Author: CDurham Date: 11/4/2024 3:50:56 PM Status: Color: Layer: Space: | Will still need to assume an increased imperviousness for future home and driveway and what those increase in flows will be. |
|---|---|--|
| A A A A A A A A A A A A A A A A A A A | Subject: Text Box Page Label: 6 Author: CDurham Date: 11/4/2024 3:54:01 PM Status: Color: Layer: Space: | Appendix I does not relinquish detention requirements. Please address increased flows with future home and driveway construction. See appendix for additional comments. |
| ue bio site. The entiting costle dishape tributary is at (ed | Subject: Text Box Page Label: 6 Author: CDurham Date: 11/4/2024 3:54:27 PM Status: Color: Layer: Space: | Major channel appears to be cutting through the middle of the site. |
| Inst the state. The watching cristle drainage tribuitary is a reject. | Subject: Text Box Page Label: 6 Author: CDurham Date: 11/4/2024 3:54:52 PM Status: Color: Layer: Space: | Who will maintain drainage channel? |
| ATED LASS | Subject: Text Box Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 4:09:20 PM Status: Color: Layer: Space: | See comments on previous sheet. Also Proposed map will need to assume an increased imperviousness for proposed/future homes. |
| Non-Name Constrained and account of addressed SUMMARY TABLE Opposite and account of solution t 0/0 0/0 1 0/0 t 0/0 0/0 1 0/0 1 0/0 t 0/0 0/0 1 0/0 0/0 1 0/0 | Subject: Text Box Page Label: [1] Pr Drn Map Author: CDurham Date: 11/4/2024 4:10:29 PM Status: Color: Layer: Space: | Update tables based on proposed calculations |