

# PRELIMINARY & FINAL DRAINAGE PLAN

## CARRIAGE MEADOWS SOUTH AT LORSON RANCH FILING NO. 2

**DECEMBER, 2018  
REV. JUNE 18, 2019**

PUDSP-19-005

**Prepared for:**

Lorson, LLC  
212 N. Wahsatch Ave, Suite 301  
Colorado Springs, Colorado 80903  
(719) 635-3200

**Prepared by:**

Core Engineering Group, LLC  
15004 1<sup>ST</sup> Avenue South  
Burnsville, MN 55306  
(719) 570-1100

Project No. 100.046



## Summary of Comments on Microsoft Word - 100.046- PDR&FDR

Page: 1

Author: dsdrice Subject: text box Date: 8/23/2019 10:59:42 AM  
PUDSP-19-005

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:30:24 AM  
added

Author: dsdrice Subject: Text Box Date: 9/2/2019 11:47:14 PM  
[See comment letter.](#)

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:30:30 AM  
noted

Author: dsdrice Subject: EPC ENG Review Date: 9/2/2019 11:47:26 PM

[See comment letter.](#)

Engineering Review

09/02/2019 10:47:23 PM

dsdrice

JeffRice@elpasoco.com

(719) 520-7877

EPC Planning & Community  
Development Department

#### **Design Point 4**

Design point 4 is pipe flow for the proposed 24" RCP from Rubicon Trail to the existing CDOT type "D" inlet, and includes upstream flow from basins G1.1 through G1.5, and the combined peak flow at this location on the west side of Rubicon Trail was used to size the proposed 24" RCP at a minimum slope of 0.50%. Design point 4 contains 5.68 acres and generates a peak developed flow of 9.7cfs for the 5-year storm event and 21.3cfs for the 100-year storm event. These flows will be routed westerly via proposed 24" RCP at a minimum of 0.50% slope and is designed to handle the flow from this design point.

#### **Design Point 5**

Design point 5 is the pipe and overland flow from basins G1.1 through G1.5a, contains 6.69 acres and generates a peak developed flow of 11.5cfs for the 5-year storm event and 25.2cfs for the 100-year storm event. These flows will be routed westerly via an existing 24 RCP at 0.80% slope designed to handle the flow from this design point. Runoff then continues west to existing detention pond G1.7. The existing storm sewer has been designed to handle 14.9cfs/29.2cfs per the Carriage Meadows South Filing 1 FDR.

#### **Design Point 6**

Design point 6 includes upstream flow from basins G1.6 and G1.7, and the combined peak flow at this low point on the east side of Carriage Meadows Drive was used to verify the size and capacity of the existing 15' type "R" inlet. Design point 6 contains 2.75 acres and generates a peak developed flow of 6.2cfs for the 5-year storm event and 12.7cfs for the 100-year storm event. Inlet DP-6 is an existing 15' type "R" inlet. The 6.2cfs for the 5-year event requires a ponding depth of 0.43' (5.1") and the 12.7cfs for the 100-year event requires a ponding depth of 0.55' (6.6"). These flows will be routed westerly via existing 30" RCP at 0.80% slope, this pipe is designed to handle the flow from this design point. Runoff then continues to existing detention pond G1.7.

#### **Design Point 7**

Design point 7 is the total peak flow from this development, which includes basins G1.1 through G1.7, contains 9.44 acres and generates a peak developed flow of 17.2cfs for the 5-year storm event and 36.8cfs for the 100-year storm event. These flows will be routed westerly via existing 30" RCP at 0.80% slope, this pipe has been designed to handle these peak flows. Runoff then continues to existing detention pond G1.7. Pond G1.7 is only a detention pond constructed to reduce the flows from future commercial areas west of Carriage Meadows Drive. Runoff from this design point flows south through Pond G1.7 and into Pond G1/G2 which is a full spectrum detention pond including WQ treatment designed to treat all the developed runoff from this development. The existing storm sewer has been designed to handle 24.3cfs/46.5cfs per the Carriage Meadows South Filing 1 FDR.

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### **6.0 DETENTION AND WATER QUALITY POND**

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All Detention and water quality necessary for Carriage Meadows South Townhomes is provided in existing Detention (Pond G1/G2) constructed as part of Carriage Meadows South at Lorson Ranch Filing No. 1. Additional detention and water quality is not required at this time. See Appendix E for Excerpts from the FDR

[see comment on calculation sheet](#)

The total site area is 5.32 acres and is contained within the 96-acre tributary area of Detention Pond G1/G2. See Appendix E for pond watershed and spreadsheets.

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### **7.0 FOUR STEP PROCESS**

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The site has been developed to minimize wherever possible the rate of developed runoff that will leave the site and to provide water quality management for the runoff produced by the site as proposed on

[see comment on calculation sheet](#)

the development plan. The following four step process should be considered and incorporated into the storm water collection system and storage facilities where applicable.

Step 1: Employ Runoff Reduction Practices

Carriage Meadows South at Lorson Ranch Filing No. 2 has employed several methods of reducing runoff.

- The street configuration was laid out to minimize the length of streets. Many streets are straight and perpendicular resulting in lots with less wasted space.
- Open space tracts of land act as a buffer between lots and Jimmy Camp Creek
- Jimmy Camp Creek has a natural sand bottom and vegetated slopes has been preserved through this site
- All developed areas drain to WQ ponds.
- Lorson Ranch Metro District requires the townhome association to maintain landscaping
- Full Spectrum Detention Pond G1/G2 has been constructed to provide detention and water quality for this subdivision. The full spectrum detention pond mimics existing storm discharges

Step 2: Implement BMP's that Slowly Release the Water Quality Capture Volume

Treatment and slow release of the water quality capture volume (WQCV) is required. Carriage Meadows South at Lorson Ranch Filing No. 2 will utilize Pond G1/G2 which is a full spectrum stormwater detention pond which includes Water Quality Volume and a WQ outlet structure.

Step 3: Stabilize Drainageways

Jimmy Camp Creek is a major drainageway located east of this site. JCC has been stabilized per county criteria in 2006. The design included a natural sand channel bottom and armored sides.

Step 4: Implement Site Specific & Source Control BMP's

There are no potential sources of contaminants that could be introduced to the County's MS4. During construction the source control will be provided with the proper installation of erosion control BMPs to limit erosion and transport of sediment. Area disturbed by construction will be seeded and mulched. Cut and fill slopes will be reseeded, and the slopes equal to or greater than three-to-one will be protected with erosion control fabric. Silt fences will be placed at the bottom of re-vegetated and rough graded slopes. Inlet protection will be used around proposed inlets. In addition, temporary sediment basins will be constructed so runoff will be treated prior to discharge. Construction BMPs in the form of vehicle tracking control, sediment basins, concrete washout area, rock socks, buffers, and silt fences will be utilized to protect receiving waters.

**8.0 DRAINAGE AND BRIDGE FEES**

Carriage Meadows South Filing No. 2 is located within the Jimmy Camp Creek drainage basin which is currently a fee basin in El Paso County. Current El Paso County regulations require drainage and bridge fees to be paid for platting of land as part of the plat recordation process. Lorson Ranch Metro District will be constructing the major drainage infrastructure as part of the district improvements.

Carriage Meadows South Townhomes contains approximately 5.32 acres. The 5.32 acres has already paid drainage/bridge fees in 2017 as part of the Carriage Meadows South Filing No. 1 final plat. The following table provides a breakdown of the drainage fees that have been paid for this site.

See ECM I.7.2.

This should be "Step 4: Consider Need for Industrial and Commercial BMPs " Revise as appropriate.

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Author: dsdrice Subject: Callout Date: 8/23/2019 3:58:57 PM  
This should be "Step 4: Consider Need for Industrial and Commercial BMPs " Revise as appropriate.

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Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:43:13 AM  
steps revised per ECM I.7.2

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Author: dsdrice Subject: Cloud+ Date: 8/23/2019 3:59:54 PM  
See ECM I.7.2.

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Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:43:19 AM  
noted

The 2017 drainage fees are \$15,720, bridge fees are \$735 and Drainage Surety fees are \$7,000 per impervious acre and were calculated as follows:

were

**Table 8.1: 2017 Drainage/Bridge Fees Paid For This Site**

Type of Land Use	Total Area (ac)	Imperviousness	Drainage Fee	Bridge Fee	Surety Fee
Residential	5.32	65%	\$54,360	\$2,542	\$24,206
Total			\$54,360	\$2,542	\$24,206

**Table 8.2: Public Drainage Facility Costs (non-reimbursable)**

Item	Quantity	Unit	Unit Cost	Item Total
24" Storm	293	LF	\$40	\$11,720
Inlets	2	EA	\$3,0000	\$6,000
			Subtotal	\$17,720
			Eng/Cont 15%)	\$2,658
			Total Est. Cost	\$20,378

**Table 8.3: Private Drainage Facility Costs (non-reimbursable)**

Item	Quantity	Unit	Unit Cost	Item Total
12" PVC	490	LF	\$20	\$9,800.00
15" PVC	156	LF	\$25	\$3,900.00
Area Inlets	7	EA	\$150	\$1,050.00
			Subtotal	\$14,750.00
			Eng/Cont 15%)	\$2,212.50
			Total Est. Cost	\$16,960.50

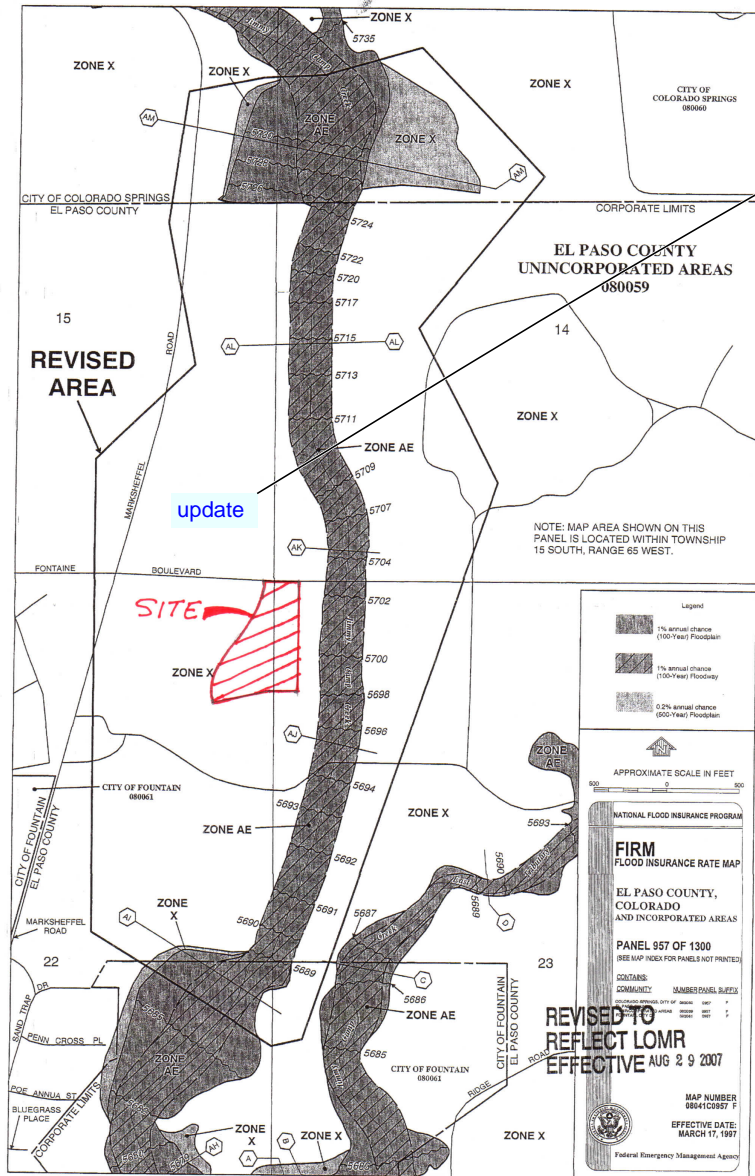
## 9.0 CONCLUSIONS

This drainage report has been prepared in accordance with the City of Colorado Springs/El Paso County Drainage Criteria Manual. The proposed development and drainage infrastructure will not cause adverse impacts to adjacent properties or properties located downstream. Several key aspects of the development discussed above are summarized as follows:

- Developed runoff will be conveyed via curb/gutter and storm sewer facilities
- Jimmy Camp Creek has been realigned within this study area

update

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:51:10 AM  
new firm inserted



Can this be adjusted? Is the outlet structure designed to be updated with each additional development?

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:52:11 AM  
 This would be hard to adjust since the outlet structure designed/built for full buildout.

### Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: \_\_\_\_\_  
 Basin ID: \_\_\_\_\_

**Example Zone Configuration (Retention Pond)**

Zone	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.06	2.577	Orifice Plate
Zone 2 (EURV)	4.54	6.236	Orifice Plate
Zone 3 (100-year)	9.88	9.918	Weir&Pipe (Restrict)
<b>Total</b>		<b>12.731</b>	

**User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)**

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft <sup>2</sup>
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

**Calculated Parameters for Underdrain**

**User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)**

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	1.5366-01	ft <sup>2</sup>
Depth at top of Zone using Orifice Plate =	4.54	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	18.20	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	22.12	sq. inches (use rectangular openings)	Elliptical Slot Area =	N/A	ft <sup>2</sup>

**Calculated Parameters for Plate**

**User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)**

Row	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.61	3.03					
Orifice Area (sq. inches)	22.12	22.12	22.12					

**User Input: Vertical Orifice (Circular or Rectangular)**

Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A	ft <sup>2</sup>
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

**Calculated Parameters for Vertical Orifice**

**User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)**

Zone 3 Weir	Not Selected	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H <sub>g</sub> =	Zone 3 Weir	Not Selected	feet
Overflow Weir Front Edge Height, H <sub>o</sub> =	4.15	N/A	10.14	5.82	N/A	feet
Overflow Weir Front Edge Length =	4.00	N/A	10.14	10.14	N/A	feet
Overflow Weir Slope =	6.00	N/A	H/V (enter zero for flat grate)	4.02	N/A	feet
Horiz. Length of Weir Sides =	10.00	N/A	feet	28.39	N/A	ft <sup>2</sup>
Overflow Grate Open Area % =	70%	N/A	% grate open area/total area	14.19	N/A	ft <sup>2</sup>
Debris Clogging % =	50%	N/A	%			

**Calculated Parameters for Overflow Weir**

**User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)**

Depth to Invert of Outlet Pipe =	0.20	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	7.07	N/A	ft <sup>2</sup>
Outlet Pipe Diameter =	36.00	N/A	inches	Outlet Orifice Centroid =	1.50	N/A	feet
Restrictor Plate Height Above Pipe Invert =	36.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

**Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate**

**User Input: Emergency Spillway (Rectangular or Trapezoidal)**

Spillway Invert Stage =	8.00	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth =	1.15	feet
Spillway Crest Length =	50.00	feet	Stage at Top of Freeboard =	10.15	feet
Spillway End Slopes =	4:00	H:V	Basin Area at Top of Freeboard =	3.37	acres
Freeboard above Max Water Surface =	1.00	feet			

**Routed Hydrograph Results**

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	0.53	1.07	1.16	1.44	1.68	1.92	2.16	2.42	0.00
One-Hour Rainfall Depth (in) =	2.577	8.814	6.842	8.912	10.804	13.017	14.962	17.363	0.000
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	2.577	8.806	6.841	8.905	10.803	13.008	14.953	17.352	#N/A
Predevelopment Unit Peak Flow, Q (cfs) =	0.00	0.00	0.01	0.03	0.11	0.29	0.44	0.65	0.00
Predevelopment Peak Q (cfs) =	0.0	0.0	0.5	1.0	1.2	1.5	1.8	2.1	0.0
Peak Inflow Q (cfs) =	31.0	103.1	80.7	104.4	125.7	150.4	171.9	199.4	#N/A
Peak Outflow Q (cfs) =	1.5	4.0	3.0	4.0	5.0	6.0	7.0	8.0	#N/A
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	4.0	4.0	4.0	4.0	4.0	#N/A
Structure Controlling Flow	Plate	Overflow Grate 1	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	#N/A
Max Velocity through Grate 1 (fps) =	N/A	0.01	N/A	0.0	0.2	0.7	1.1	1.8	#N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	#N/A
Time to Drain 87% of Inflow Volume (hours) =	38	62	57	62	63	61	61	60	#N/A
Time to Drain 99% of Inflow Volume (hours) =	40	67	61	67	69	69	69	69	#N/A
Maximum Ponding Depth (ft) =	1.93	4.29	3.60	4.32	4.80	5.25	5.58	5.91	#N/A
Area at Maximum Ponding Depth (acres) =	0.01	2.74	2.60	2.75	2.96	2.95	3.00	3.05	#N/A
Maximum Volume Stored (acre-ft) =	2.301	8.104	6.290	8.186	9.942	10.870	11.852	12.881	#N/A

Can this be adjusted? Is the outlet structure designed to be updated with each additional development?

Author: dsdrice Subject: Callout Date: 8/23/2019 4:19:40 PM

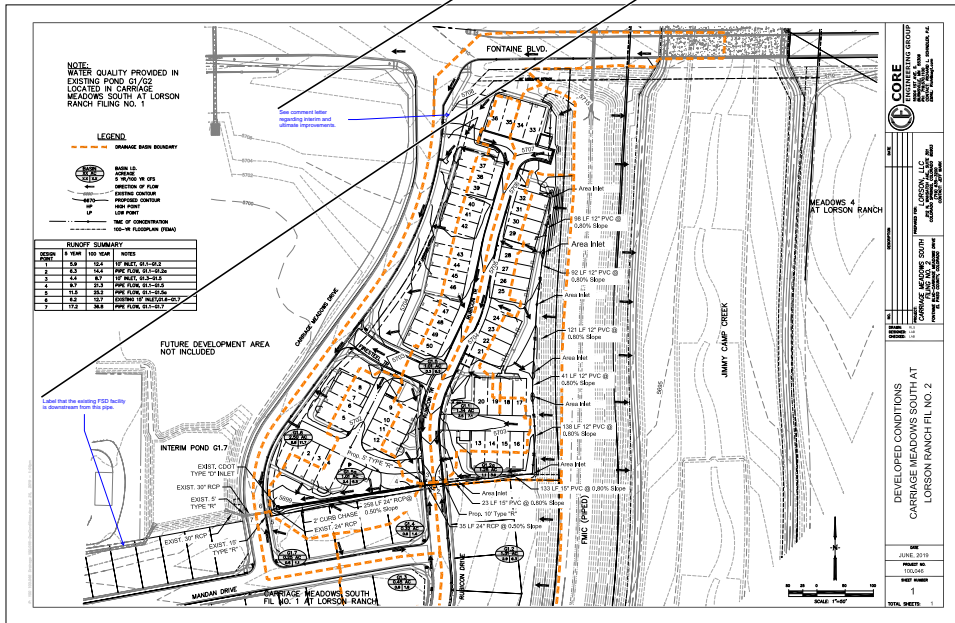
See comment letter regarding interim and ultimate improvements.

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:54:41 AM  
rt turn lane improvements shown

Author: dsdrice Subject: Callout Date: 9/2/2019 11:32:40 PM

Label that the existing FSD facility is downstream from this pipe.

Author: RSchindler Subject: Sticky Note Date: 10/14/2019 8:54:23 AM  
Label added






CONSTRUCTION PLANS  
FOR

## CARRIAGE MEADOW SOUTH AT LORSON RANCH FILING NO. 2

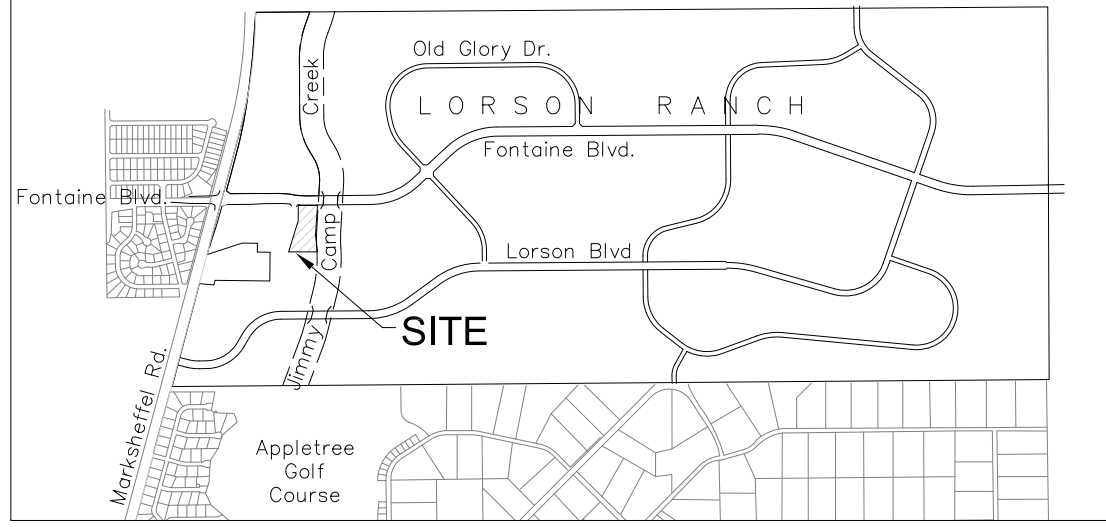
STREET/STORM SEWER AND UTILITY CONSTRUCTION PLANS

**Engineering Review**  
6032 20th Pl, Suite 410  
Colorado Springs, CO 80905  
(719) 584-8777  
Core Engineering Group  
Development Department



**Know what's below.  
Call before you dig.**  
CALL 2-BUSINESS DAYS IN ADVANCE  
BASIC 100 DIAL SCALE OR  
EXCAVATE FOR THE MARKING OF  
UNDERGROUND UTILITY SERVICES

SHEET INDEX	
SHEET NO.	SHEET DESCRIPTION
C0.1	COVER SHEET
C0.2 - C0.3	NOTES (GENERAL, UTILITY)
C0.4	TYPICAL SECTIONS
C0.1 - C0.2	HORIZONTAL CONTROL PLAN
C0.1	UTILITY SERVICE PLAN
C0.1	SEWER PLAN
C0.1 - C0.4	STREET/STORM SEWER PLAN AND PROFILES
C0.1 - C0.4	STORMWATER SEWER/WATERMAIN PLAN AND PROFILES
C0.1 - C0.3	STREET/STORM DETAILS
C0.1 - C0.2	SEWER/WATER DETAILS



VICINITY MAP  
NO SCALE

**WATER/SANITARY**  
WIDEFIELD WATER AND SANITATION DISTRICT  
8400 FONTAINE BLVD.  
COLORADO SPRINGS, CO 80925  
719-390-7111

**TELEPHONE**  
CENTEL  
7920 INDUSTRY ROAD  
COLORADO SPRINGS, CO 80939  
719-278-4651

**BASIS OF BEARING**  
BORINGS ARE SHOWN ON THE SOUTH LINE OF THE NORTH HALF OF SECTION 23, TOWNSHIP 16 SOUTH, RANGE 65 WEST OF THE 6TH PRINCIPAL MERIDIAN AS BEING SOUTH 89°42' WEST. THE EAST QUARTER CORNER OF SAID SECTION 23 IS A FOUND 2-1/2" ALUMINUM CAP MONUMENT AND THE WEST QUARTER CORNER OF SAID SECTION 23 IS A FOUND 2-1/2" ALUMINUM CAP MONUMENT.

**BENCHMARK**  
THE BENCHMARK IS LOCATED AT THE NORTHWEST CORNER OF FONTAINE BLVD AND COTTONWOOD GROVE DR. ELEVATION 5204.252 (N.G.D. 29)

**TRAFFIC CONTROL NOTE**  
THE CONTRACTOR SHALL PROVIDE ALL TRAFFIC CONTROL DEVICES AND MONITORING NECESSARY TO SAFELY COMPLETE THE WORK SHOWN IN THESE CONSTRUCTION DOCUMENTS IN CONFORMANCE WITH ALL CITY ORDINANCES. THE LOCATION OF TRAFFIC CONTROL DEVICES SHALL BE DETERMINED BY THE CONTRACTOR IN CONFORMANCE WITH ALL CITY ORDINANCES. ALL NECESSARY TRAFFIC CONTROL DEVICES SHALL BE PROVIDED SEPARATELY BUT IS INCLUDED IN THE COST OF THE PROJECT.

**SECURITY FIRE PROTECTION DISTRICT**  
400 SECURITY BOULEVARD  
SECURITY, CO 80911  
719-362-1121

**EL PASO COUNTY PLANNING AND COMMUNITY DEVELOPMENT**  
2880 INTERNATIONAL CIRCLE  
COLORADO SPRINGS, CO 80910  
719-520-6500

**WIDEFIELD WATER AND SANITATION DISTRICT WATER DESIGN APPROVAL**  
DATE \_\_\_\_\_ BY \_\_\_\_\_  
PROJECT NO. \_\_\_\_\_

**WIDEFIELD WATER AND SANITATION DISTRICT WASTEWATER DESIGN APPROVAL**  
DATE \_\_\_\_\_ BY \_\_\_\_\_  
PROJECT NO. \_\_\_\_\_

**IN CASE OF ERRORS OR OMISSIONS WITH THE WATER DESIGN AS SHOWN ON THIS DOCUMENT THE STANDARDS AS DEFINED IN THE "RULES AND REGULATIONS FOR INSTALLATION OF WATER MAINS AND SERVICES" SHALL RULE.**  
APPROVAL EXPIRES 180 DAYS FROM DESIGN APPROVAL

**PREPARED FOR:**  
LORSON LLC  
N. WARDEN AVE - SUITE 301  
COLORADO SPRINGS, CO 80903  
CONTACT: JEFF MARK

**PREPARED BY:**  
CORE ENGINEERING GROUP  
15004 1ST AVENUE S  
BIRMGHAM, AL 35256  
719-570-1100  
CONTACT: RICHARD L. SCHROEDER P.E.

**CONSTRUCTION APPROVAL**  
COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

**FILED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, DRAINAGE CRITERIA MANUALS VOLUME 1 AND 2, AND ENGINEERING CRITERIA MANUAL, AS AMENDED. IN ACCORDANCE WITH COM. SECTION 11.12, THESE CONSTRUCTION DOCUMENTS WILL BE VALID FOR CONSTRUCTION FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO COUNTY ENGINEER. IF CONSTRUCTION HAS NOT STARTED WITHIN THOSE TWO YEARS, THE PLANS WILL NEED TO BE RESUBMITTED FOR APPROVAL, INCLUDING PAYMENT OF REVIEW FEES AT THE PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION.**

**SEWER SHINE, COUNTY ENGINEER/COM ADMINISTRATOR** DATE \_\_\_\_\_  
CONDITIONS: \_\_\_\_\_

**ENGINEER'S APPROVAL**  
THESE DETAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECTION AND SUPERVISION. SAID PLANS AND SPECIFICATIONS HAVE BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR APPLICABLE MASTER DRAINAGE PLANS AND MASTER TRANSPORTATION PLANS. SAID PLANS AND SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR ROADWAY AND DRAINAGE FACILITIES ARE DESIGNED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARATION OF THESE DETAILED PLANS AND SPECIFICATIONS.

**RICHARD L. SCHROEDER, P.E. # 33997**  
FOR AND ON BEHALF OF CORE ENGINEERING GROUP

**PUDSP-19-005**

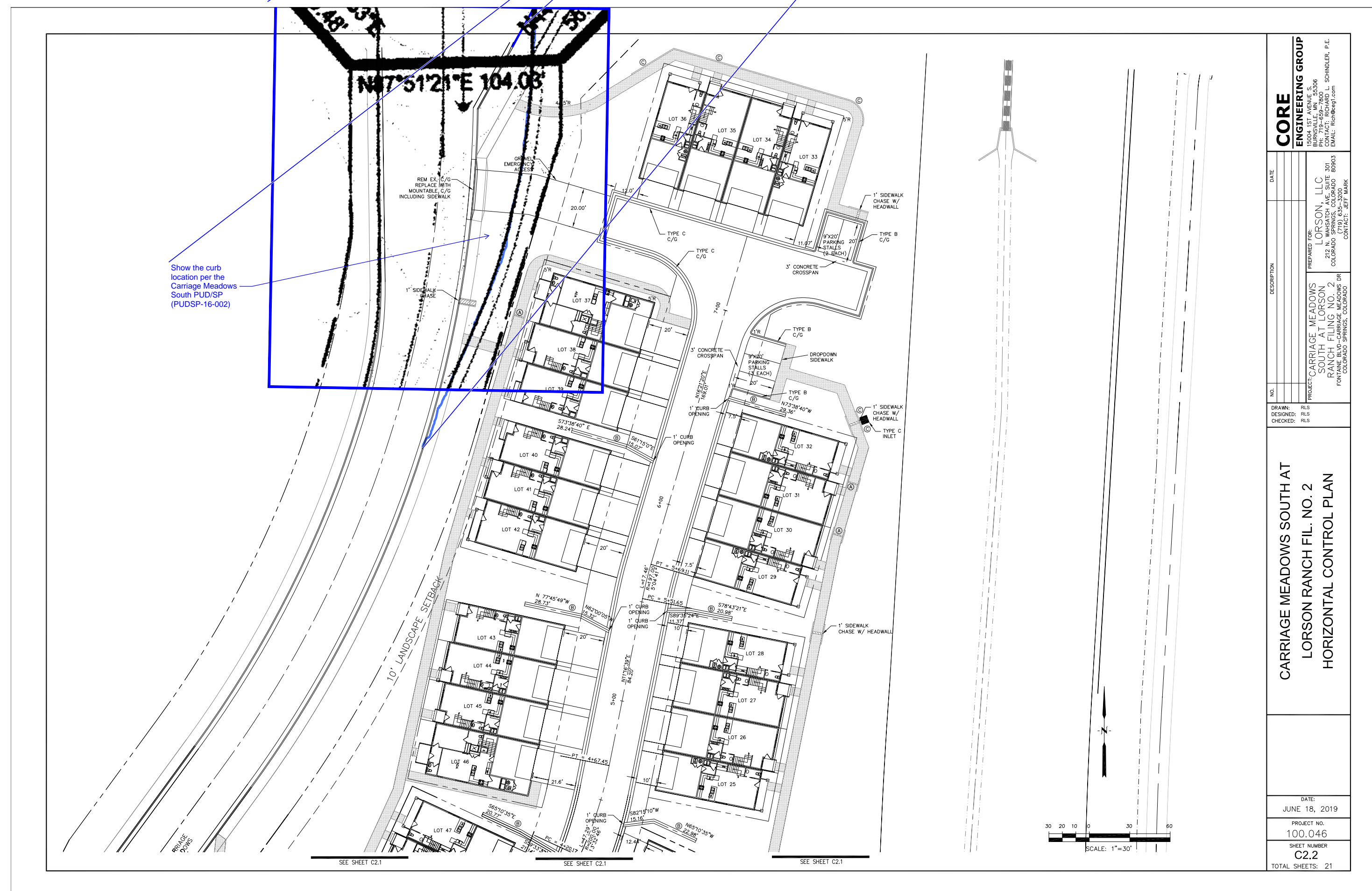
COVER SHEET  
STREET/STORM SEWER  
AND UTILITY PLANS

DATE: JUNE 18, 2019  
PROJECT NO.: 100.046  
SHEET NAME: C0.1  
TOTAL SHEETS: 21



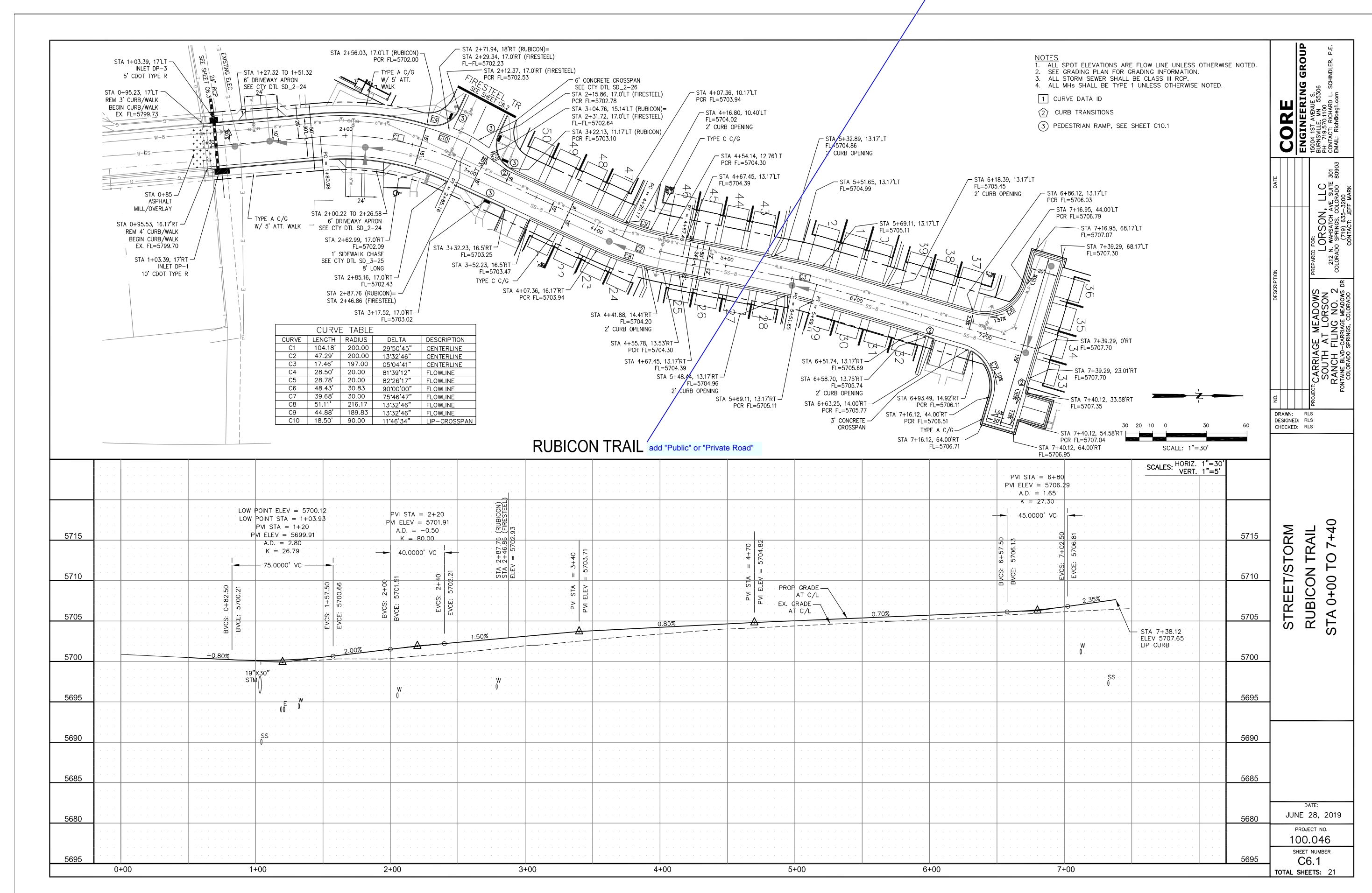
- Author: dhdice Subject: Line Date: 8/23/2019 9:40:43 AM
- Author: dhdice Subject: Snapshot Date: 8/30/2019 12:21:38 PM
- Author: dhdice Subject: Highlight Date: 8/30/2019 12:21:27 PM  
provide improvements
- Author: Richender Subject: Sticky Note Date: 10/14/2019 7:27:18 AM  
Please turn lane provided
- Author: dhdice Subject: Callout Date: 8/23/2019 9:47:14 AM
- Author: Richender Subject: Sticky Note Date: 10/14/2019 7:27:21 AM  
Please turn lane

Show the curb location per the Carriage Meadows South PUD/SP (PUDSP-16-002)









**NOTES**

1. SEE SPOT ELEVATIONS ARE FLOW LINE UNLESS OTHERWISE NOTED.
2. SEE GRADING PLAN FOR GRADING INFORMATION.
3. ALL TYPING SHALL BE CLASS II FONT.
4. ALL WPS SHALL BE TYPE 1 UNLESS OTHERWISE NOTED.

CURVE DATA ID  
 CURVE TRANSITIONS  
 PEDESTRIAN RAMP, SEE SHEET C101

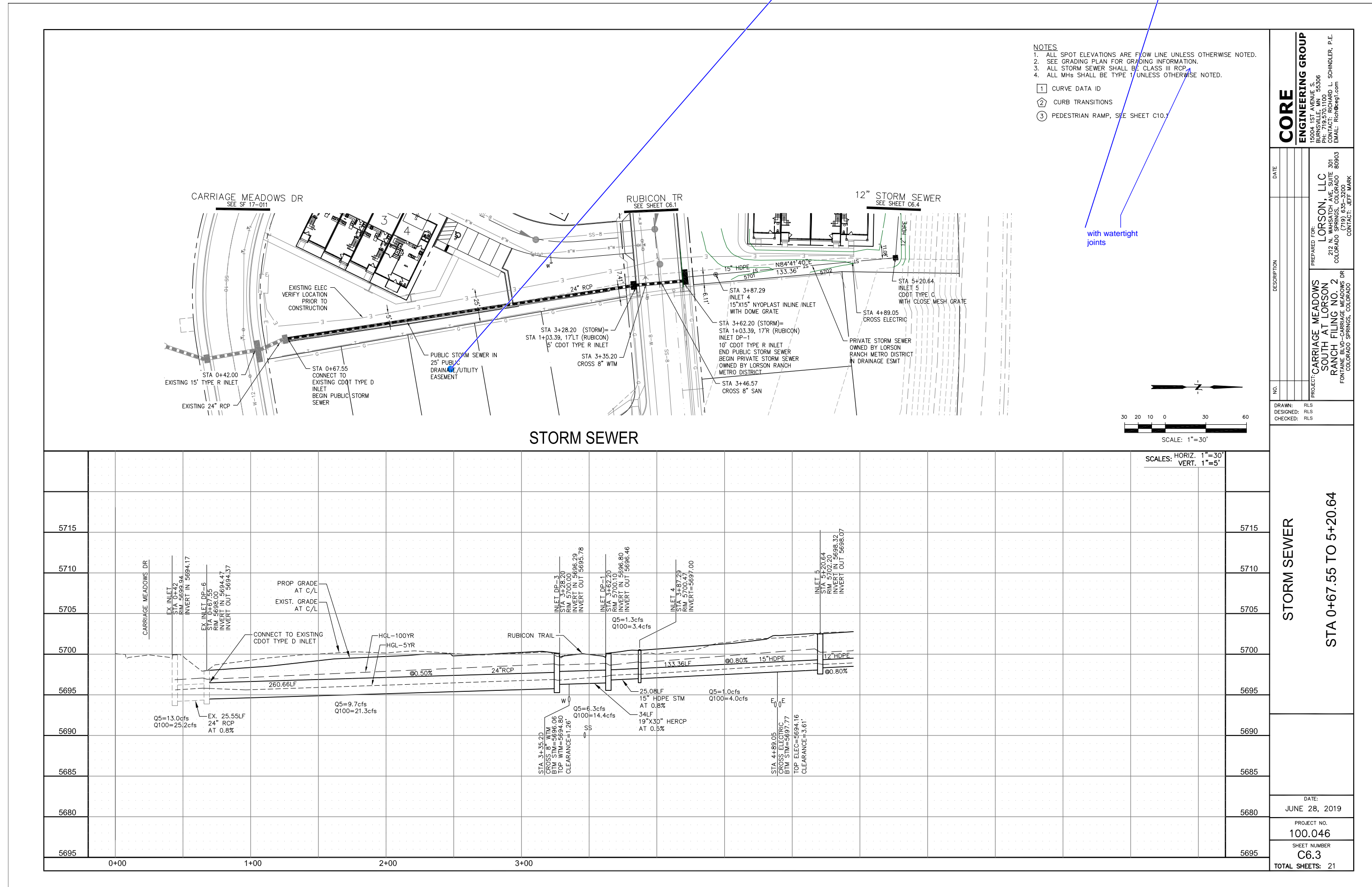
**SCALE**

HORIZ. 1"=30'  
 VERT. 1"=5'  
 SCALE: 1"=30'

SHEET: JUNE 28, 2019  
 PROJECT NO. 100.046  
 SHEET NUMBER: C6.1  
 TOTAL SHEETS: 21

















Author: dlabice Subject: Callout Date: 8/23/2019 10:49:49 AM  
See other comments regarding completion of Carriage Meadows Drive.  
Author: Kufender Subject: Sticky Note Date: 10/14/2019 9:17:11 AM  
11 km 10km  
Author: dlabice Date: 8/22/2019 4:16:31 PM

