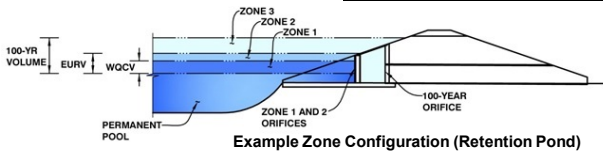


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.07 (June 2025)

Project: Honor Charter Academy

Basin ID: School Pond



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.37	0.128	Orifice Plate
Zone 2 (EURV)	2.25	0.299	Orifice Plate
Zone 3 (100-year)	2.83	0.252	Weir&Pipe (Circular)
Total (all zones)		0.679	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration SCM)

Underdrain Orifice Invert Depth = N/A ft (distance below the filtration media surface)
Underdrain Orifice Diameter = N/A inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = N/A ft²
Underdrain Orifice Centroid = N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = 2.63 ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = N/A inches
Orifice Plate: Orifice Area per Row = N/A sq. inches

TAIT RESPONSE:
THE ORIFICE CENTROID ELEVATION HAS BEEN ADJUSTED ON THE PLANS. NO CHANGES WERE MADE TO THESE CALCULATIONS.

Elevation call out and dimensions conflict in CDs

Calculated Parameters for Orifice Plate
Orifice Plate Centroid = N/A feet
Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00 ✓	0.25 ✓	0.50 ✓	0.90 ✓	1.90 X			
Orifice Area (sq. inches)	0.31 ✓	0.44 ✓	0.44 ✓	0.79 ✓	12.57 ✓			

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = N/A inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = N/A ft²
Vertical Orifice Centroid = N/A feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Overflow Weir Front Edge Height, H_o = 2.63 ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = 6.00 feet
Overflow Weir Grate Slope = 0.00 H:V
Horiz. Length of Weir Sides = 6.00 feet
Overflow Grate Type = Close Mesh Grate
Debris Clogging % = 50%

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_t = 2.63 feet
Overflow Weir Slope Length = 6.00 feet
Grate Open Area / 100-yr Orifice Area = 16.11
Overflow Grate Open Area w/o Debris = 28.48 ft²
Overflow Grate Open Area w/ Debris = 14.24 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 0.00 ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter = 18.00 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = 1.77 ft²
Outlet Orifice Centroid = 0.75 feet
Half-Central Angle of Restrictor Plate on Pipe = N/A radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 2.85 ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = 20.00 feet
Spillway End Slopes = 4.00 H:V
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = 0.37 feet
Stage at Top of Freeboard = 4.22 feet
Basin Area at Top of Freeboard = 0.52 acres
Basin Volume at Top of Freeboard = 1.18 acre-ft

Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.68
One-Hour Rainfall Depth (in) =	N/A	N/A	0.314	0.417	0.500	0.631	0.759	0.922	1.600
CUHP Runoff Volume (acre-ft) =	N/A	N/A	0.314	0.417	0.500	0.631	0.759	0.922	1.600
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.1	0.3	0.4	1.6	2.9	4.5	11.1
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.02	0.04	0.05	0.21	0.39	0.61	1.49
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A	0.02	0.04	0.05	0.21	0.39	0.61	1.49
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	4.4	5.9	7.0	9.5	11.8	14.7	25.5
Peak Inflow Q (cfs) =	N/A	N/A	0.1	0.3	0.4	1.6	2.9	4.5	11.1
Peak Outflow Q (cfs) =	N/A	N/A	0.1	0.3	0.4	1.6	2.9	4.5	11.1
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.1	1.0	0.3	0.6	1.0	1.5
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	0.0	0.1	0.4
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	40	70	68	71	72	74	73	71	64
Time to Drain 99% of Inflow Volume (hours) =	43	76	73	77	79	81	81	80	76
Maximum Ponding Depth (ft) =	1.38	2.25	1.91	2.13	2.31	2.59	2.73	2.84	3.04
Area at Maximum Ponding Depth (acres) =	0.26	0.40	0.36	0.39	0.41	0.44	0.45	0.46	0.47
Maximum Volume Stored (acre-ft) =	0.131	0.430	0.297	0.383	0.454	0.572	0.630	0.684	0.777