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Final Drainage Report

**Misfit Crew
Estates**

Project No. 61160

October 16, 2024

PCD File No. SF2417

Final Drainage Report

for

Misfit Crew Estates

Project No. 61160

October 16, 2024

prepared for

Mark McDonald

5775 Mountain Shadow View
Colorado Springs, CO 80908

prepared by

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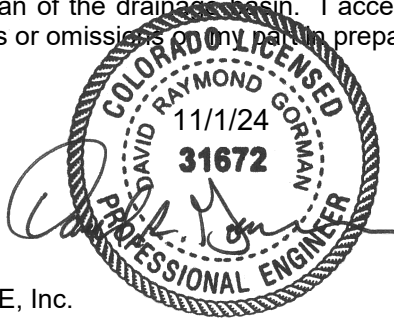
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Statements and Acknowledgments

Engineer's Statement

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.



David R. Gorman, P.E.
Colorado No. 31672
For and on Behalf of MVE, Inc.

Developer's Statement

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Mark E. McDonald

Mark McDonald
Owner
5775 Mountain Shadow View
Colorado Springs, CO 80908

11/01/2024

Date

El Paso County

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Joshua Palmer, P.E.
County Engineer / ECM Administrator

Date

Conditions:

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Final Drainage Report

The purpose of this Final Drainage Report is to identify drainage patterns and quantities within and affecting the proposed Misfit Crew Estates site. The development project is a residential subdivision with 5.0± acre lots. The report will identify specific solutions to drainage concerns on-site and off-site resulting from the proposed project. The report and included maps present results of hydrologic and drainage facilities analyses. The report will discuss the recommended drainage improvements to the site and identify drainage requirements relative to the proposed project. This report has been prepared and submitted in accordance with the requirements of the El Paso County development approval process. An Appendix is included with this report with pertinent calculations and graphs used in the drainage analyses and design.

1 General Location and Description

1.1 Location

The proposed Misfit Crew Estates site is located within the east half of Section 24, Township 11 South, Range 66 West of the 6th principal meridian in El Paso County, Colorado. The 36.05± acre site is situated on the north side of Hodgen Road, at the end of Mountain Shadow View, a private gravel drive, ending at the east edge of the site. The parcel (Zone RR-5) contains a single family residence and out buildings. The El Paso County Assessor's Schedule Number for the site is 6124000013. A **Vicinity Map** is included in the **Appendix**. The site is located in El Paso County's East Cherry Creek Drainage Basin.

1.2 Description of Property

The Misfit Crew Estates site 36.05± acres and is zoned RR-5 (Residential Rural (5 Acres)). The property contains a single-family residence with an existing gravel driveway and several out buildings. The proposed Misfit Crew Estates includes 3 rural residential lots.

The ground cover, which is in fair condition, consists of native grasses. The tree coverage is sporadic and located only around the residence.

The existing site topography slopes to the southeast with grades that range from 2% to 10%.

There are two major drainage ways in the Misfit Crew Estates site. For the north 4± acres, all storm runoff flows drain east off of the site. For the central 21± acres, all storm runoff flows drain southeast through a livestock pond and then southeast off of the site. For the south 11± acres, all storm runoff flows drain southeast off of the site. There is no storm drain system in the surrounding area. The site is located in El Paso County's East Cherry Creek Drainage Basin. The flows from the site flow southeast and eventually enter a tributary of East Cherry Creek.

According to the National Resource Conservation Service, there are two (2) soil types in the Misfit Crew Estates site. Peyton sandy loam (map unit 67) makes up about 78% of the soil on the site. The soil is deep and well drained. Permeability is moderate, surface runoff is medium, and the hazard of erosion is moderate. Peyton sandy loam is classified as being part of Hydrologic Soil Group B.

The other soil type located on the site is Cruckton sandy loam (map unit 21) which makes up the remaining 22% of the soil on the site. The soil is deep and well drained. Permeability is moderately rapid, surface runoff is slow to medium, and the hazard of erosion is moderate. Cruckton sandy loam is classified as being part of Hydrologic Soil Group B.

A portion of the Soil Map and data tables from the National Cooperative Soil Survey and relevant Official Soil Series Descriptions (OSD) are included in the **Appendix**.^{1 2}

2 Drainage Basins and Sub-Basins

2.1 Major Basin Descriptions

The Misfit Crew Estates site is located in the East Cherry Creek Drainage Basin (CYCY0200).

The current Flood Insurance Study of the region includes Flood Insurance Rate Maps (FIRM), effective on December 7, 2018.³ The proposed subdivision is included in the Community Panel Numbered 08041C0305 G of the Flood Insurance Rate Maps for the El Paso County. No part of the site is shown to be included in a 100-year flood hazard area as determined by FEMA. A portion of the current FEMA Flood Insurance Rate Maps with the site delineated is included in the **Appendix**.

2.2 Sub-Basin Description

The existing and developed drainage patterns of the Misfit Crew Estates project are described by three (3) on-site drainage basins and one (1) offsite basin along with two (2) points of off-site flows as detailed in the Final Drainage Report for Longview Estates. All of these basins are previously undisturbed or developed to a degree as described below. All existing basin delineations and data are depicted on the attached **Drainage Map**.

2.2.1 Existing / Developed Drainage Patterns (Off-Site)

Existing off-site Design Point DP8 is located west of the site at the north edge, being the pond outfall from an adjacent detention pond in Longview Estates, the outfall drains east onto the site. This flow enters the onsite sub-basin A1 and continues through the site.

Existing off-site sub-basin OS-B1 represents the off-site that combine with sub-basin B2 to travel southeast through the site to an existing livestock pond, then continues southeast off the site.

Existing off-site Design Point DP10 is located west of the site at the south edge, being the pond outfall from an adjacent detention pond in Longview Estates, the outfall drains east onto the site. This flow enters the onsite sub-basin C1 and continues through the site to a depression along the north side of Hodgen Road where it enters an existing 42" RCP and goes under Hodgen Road.

3 Drainage Design Criteria

3.1 Development Criteria Reference

This Final Drainage Report for Misfit Crew Estates has been prepared according to the report guidelines presented in the latest edition of *El Paso County Drainage Criteria Manual* (DCM)⁴. The County has also adopted portions of the City of Colorado Springs Drainage Criteria Manual Volumes 1 and 2, especially concerning the calculation of rainfall runoff flow rates.^{5 6} The hydrologic analysis is based on a collection of data from the DCM, the NRCS Web Soil Survey⁷, and existing topographic data by Polaris.

1 WSS
 2 OSD
 3 FIRM
 4 DCM Section 4.3 and Section 4.4
 5 CS DCM Vol 1
 6 CS DCM Vol 2
 7 WSS

3.2 Previous Drainage Studies

Longview Estates to the west was studied and the Final Drainage Report for Longview Estates by M.V.E., Inc, dated October 30, 2002 was used in the preparation of this report. Developed drainage basins from Longview Estates drain directly onto this site and the flows stated in the drainage report were used as off-site flows. A copy of this reports Drainage Map is included in the **Appendix**.

The existing 42" RCP under Hodgen Road, that is the nearest downstream facility to this site, was designed in the Final Drainage Report for Hodgen Road (State Highway 83 to Black Forest Road) by URS, dated December 8, 2009. Excerpts from this report and the culvert construction drawings are included in the **Appendix**.

3.3 Hydrologic Criteria

For this Final Drainage Report, the Rational Method as described in the *Drainage Criteria Manual* has been used for all Storm Runoff calculations, as the development and all sub-basins are less than 130 acres in area. "Colorado Springs Rainfall Intensity Duration Frequency" curves, Figure 6-5 in the DCM, was used to obtain the design rainfall values; a copy is included in the **Appendix**. The "Overland (Initial) Flow Equation" (Eq. 6-8) in the DCM, and Manning's equation with estimated depths were used in time of concentration calculations. "Runoff Coefficients for Rational Method", Table 6-6 in the DCM, was utilized as a guide in estimating runoff coefficient and Percent Impervious values; a copy is included in the **Appendix**. Peak runoff discharges were calculated for each drainage sub-basin for both the 5-year storm event and the 100-year storm event with the Rational Method formula, (Eq. 6-5) in the DCM.⁸

4 Drainage Facility Design

4.1 General Concept

The intent of the drainage concept presented in this Final Drainage Report is to allow for the development of the three lots while maintaining the existing drainage patterns on the site. Major and minor storm flows will continue to be safely conveyed through the site and downstream.

The existing and proposed drainage hydrologic conditions are described in more detail below. Input data and results for all calculations are included in the **Appendix**. A Drainage map for the hydrology are also included in the **Appendix**.

4.2 Existing / Developed Hydrologic Conditions

The Misfit Crew Estates site includes four (4) sub-basins, three (3) on site and one (1) off site as well as two (2) design points taken from the Final Drainage Report for Longview Estates. The site generally drains southeast. The sub-basins are described in more detail below.

Sub-basin A1, located at the very north end of the site, is 3.89± acres in area and accepts the flows from off-site Design Point DP8. Sub-basin A1 contains meadow/pasture and a portion of the roof of a barn, and is expected to remain as is. Peak storm runoff rates are $Q_5 = 1.0$ cfs and $Q_{100} = 7.0$ cfs (existing flows) with no change to the developed flows which drain east to DP1. These flows continue to drain generally via sheet flow to the east into the adjacent properties. The combined peak storm runoff rates flowing to DP1 are $Q_5 = 7.3$ cfs and $Q_{100} = 21.7$ cfs (existing / developed flows) which flow east through adjacent properties. There is no change to this sub-basin from existing to developed.

Existing offsite sub-basin OS-B1, located on the central west side of the site, is 1.22± acres in area. Sub-basin OS-B1 contains a meadow/pasture area of an existing developed residential lot in Longview Estates. Peak storm runoff rates are $Q_5 = 0.3$ cfs and $Q_{100} = 2.3$ cfs (existing / developed flows) which drain on-site to the southeast. These flows continue southeast through sub-basin B2.

8 DCM

Sub-basin B2 located in approximately the middle half of the site, is 20.92± acres in area. Sub-basin B2 contains a single family residence, gravel drive, several out buildings and a livestock pond. This sub-basin is expected to remain as is. Peak storm runoff rates are $Q_5 = 4.8$ cfs and $Q_{100} = 33.4$ cfs (existing / developed flows) which drain southeast into the livestock pond where it collects and infiltrates. No clearly defined spillway or outlet exist for this structure. The State Dam Safety Engineer has directed that any future grading or improvements to this structure shall include the construction of an adequate spillway and outlet. There are no planned improvements to the pond at this time. The combined peak storm runoff rates from OS-B1 and sub-basin B2 flowing to DP2 are $Q_5 = 5.0$ cfs and $Q_{100} = 34.9$ cfs (existing / developed flows) which flow southeast to the edge of proposed lot 1 where a new driveway will be constructed with an 18" CMP culvert under the drive. The culvert is sized to accept the 5 year flows and the 100 year flows will overtop the driveway by 25 CFS (under 3" in depth).

Sub-basin C1, located at south end of the site, is 11.01± acres in area and accepts the flows from off-site Design Point DP10. Sub-basin C1 currently contains meadow/pasture and is the proposed location of lots 2 and 3. Sub-basin C1's peak storm runoff rates are $Q_5 = 2.6$ cfs and $Q_{100} = 19.0$ cfs (existing flows) and $Q_5 = 3.3$ cfs and $Q_{100} = 20.8$ cfs (developed flows). The developed flows assume a land use of 5 Acre lots with a percent imperviousness of 7.0%. A no-build / drainage easement is proposed for the existing swale through sub-basin C1. The swale is well vegetated and shows no signs of erosion. Calculations to determine the depth of flows and velocity of this swale are included in the **Appendix**. These flows, combined with those from off-site DP10 drain southeast to DP3, which flows southeast via an existing swale onto the adjacent property to a localized depression containing an existing 42" RCP that flows under Hodgen Road. Calculations for the existing swale and the 42" RCP are included in the Appendix. The combined peak storm runoff rates flowing to DP3 are $Q_5 = 24.7$ cfs and $Q_{100} = 72.3$ cfs (existing flows) and $Q_5 = 25.4$ cfs and $Q_{100} = 74.1$ cfs (developed flows). This is an increase of $Q_5 = 1.7$ cfs (2.75%) and $Q_{100} = 1.8$ cfs (2.43%).

The existing 42" RCP lying approximately 250 feet downstream of DP3 was designed using 100 year discharge of 100.9 cfs. This culvert was recalculated using 102.7 cfs (the increase in flows from existing to developed) and found to be capable of handling the minor increase in flows from this development. The culvert appears to be in stable condition with no signs of erosion at the downstream end. Outlet protection was provided and is in satisfactory condition. Excerpts from the URS drainage report showing the culvert calculations along with updated calculations for this culvert are included in the **Appendix**.

4.3 Erosion Control

There is no public infrastructure construction or overlot grading associated with this subdivision. Any required control measures (CM's) for the individual lot home construction will be handled on the BESQCP for each lot at time of building permit. The velocity of the swale at DP3 (100 yr) is 4.07 FPS and the velocity of the swale at developed basin C1 is 4.48 FPS. These velocities are under the 7 FPS discussed in the associated soils and geology report and will not need stabilization.

4.4 Four Step Process

The El Paso County Engineering Criteria Manual (Appendix I, Section I.7.2) requires the consideration of a "Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long term source controls". The Four Step Process is incorporated in this project and the elements are discussed below.

The entire site consists of 5-acre single family residential lots which are excluded from Post Construction Stormwater Management requirements by ECM 1.7.1.B.5 due to the low development density as 5-acre lots. There is a 10' wide public roadway being dedicated to El Paso County. The site is not subject to Post Construction Stormwater Treatment requirements.

1) Runoff Reduction Practices are employed in this project. Impervious surfaces have been reduced as much as practically possible. There is only minimal concrete or other hard surfaces proposed.

Minimized Directly Connected Impervious Areas (MDCIA) is employed on the project because runoff passes through an open space meadow area before leaving the site.

2) There are no drainage paths on the site that are required to be stabilized as they are well vegetated with no visual erosion.

3) The project contains no potentially hazardous uses. The site is exempted from the use of WQCV CMs by ECM 1.7.1.B.5 by virtue of the large lot rural residential nature of the site having percent imperviousness of less than 10%.

4) The rural residential lot is not anticipated to contain storage of potentially harmful substances or use of potentially harmful substances. No site specific or other source control CMs are required.

5 Drainage and Bridge Fees

The subdivision is located within the East Cherry Creek Drainage Basin, El Paso Basin Number CYCY0200, which which has no DBPS. There are no fees associated with this basin.

6 Conclusion

This Final Drainage Report presents existing and proposed drainage conditions for the proposed Misfit Crew Estates project. The development will have negligible and inconsequential effects on the existing site drainage and drainage conditions downstream. The site is exempted from the use of WQCV CMs by ECM 1.7.1.B.5 by virtue of the large lot rural residential nature of the site having percent imperviousness of less than 10%. The entire site is consists of 5-acre single family residential lots which are excluded from Post Construction Stormwater Management requirements due to the low development density as 5-acre lots. The site is not subject to Post Construction Stormwater Treatment requirements. With such a negligible increase in stormwater flows from the site, detention will not be necessary for the proposed development and will not be provided. The proposed project will not, with respect to stormwater runoff, negatively impact the adjacent properties and downstream properties.

References

NRCS Web Soil Survey. United States Department of Agriculture, Natural Resources Conservation Service ("<http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx>", accessed March, 2018).

NRCS Official Soil Series Descriptions. United States Department of Agriculture, Natural Resources Conservation Service ("<http://soils.usda.gov/technical/classification/osd/index.html>", accessed March, 2018).

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| Appendices

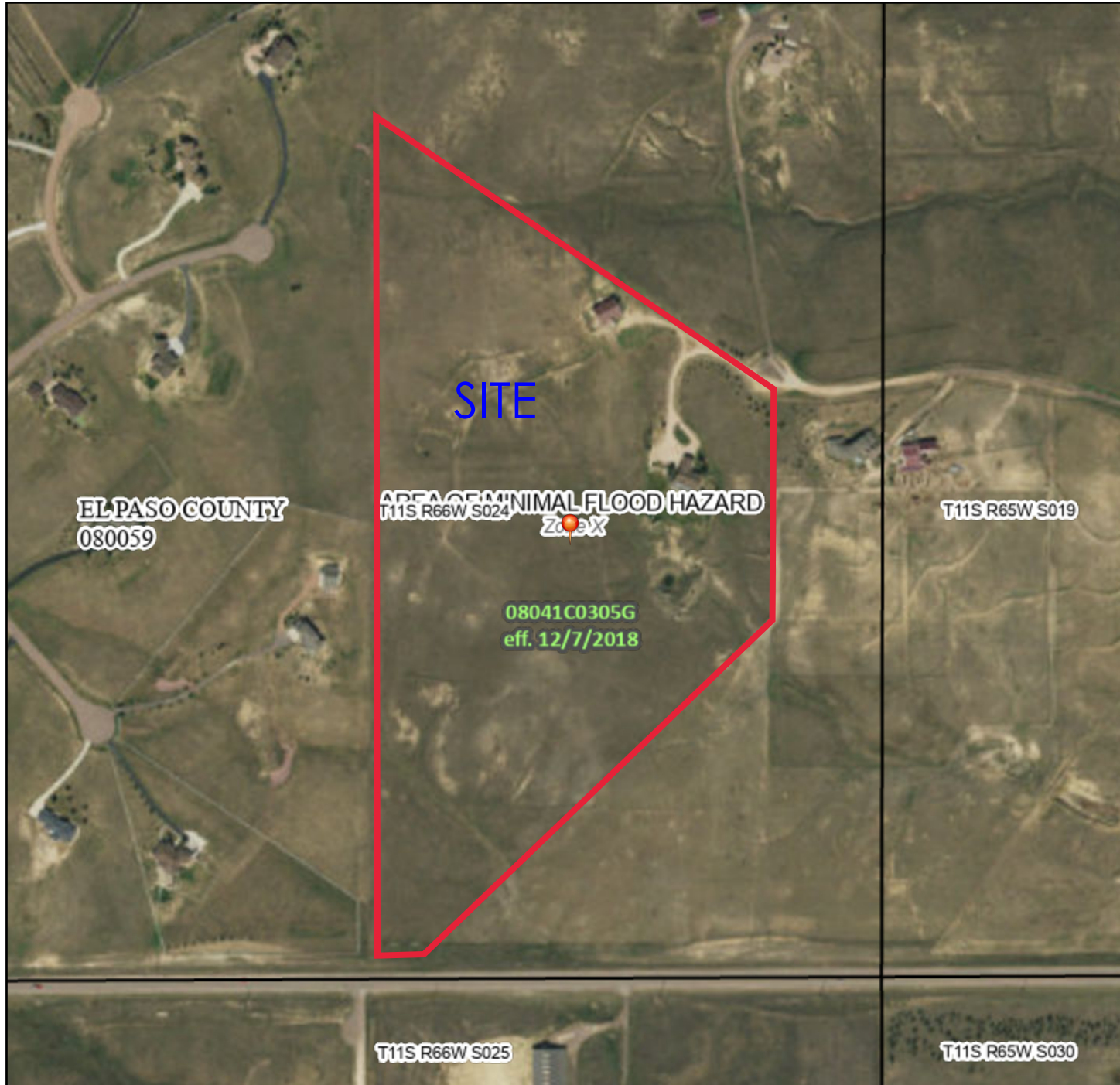
1 General Maps and Supporting Data

- Vicinity Map
- Portions of Flood Insurance Rate Map
- NRCS Soil Map and Tables
- SCS Soil Type Descriptions
- Hydrologic Soil Group Map and Tables
- Livestock Watertank Application

National Flood Hazard Layer FIRMMette



104°43'39"W 39°4'41"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D

OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall

OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Profile Baseline
	Hydrographic Feature	

MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 5/25/2021 at 5:59 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Custom Soil Resource Report for El Paso County Area, Colorado



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

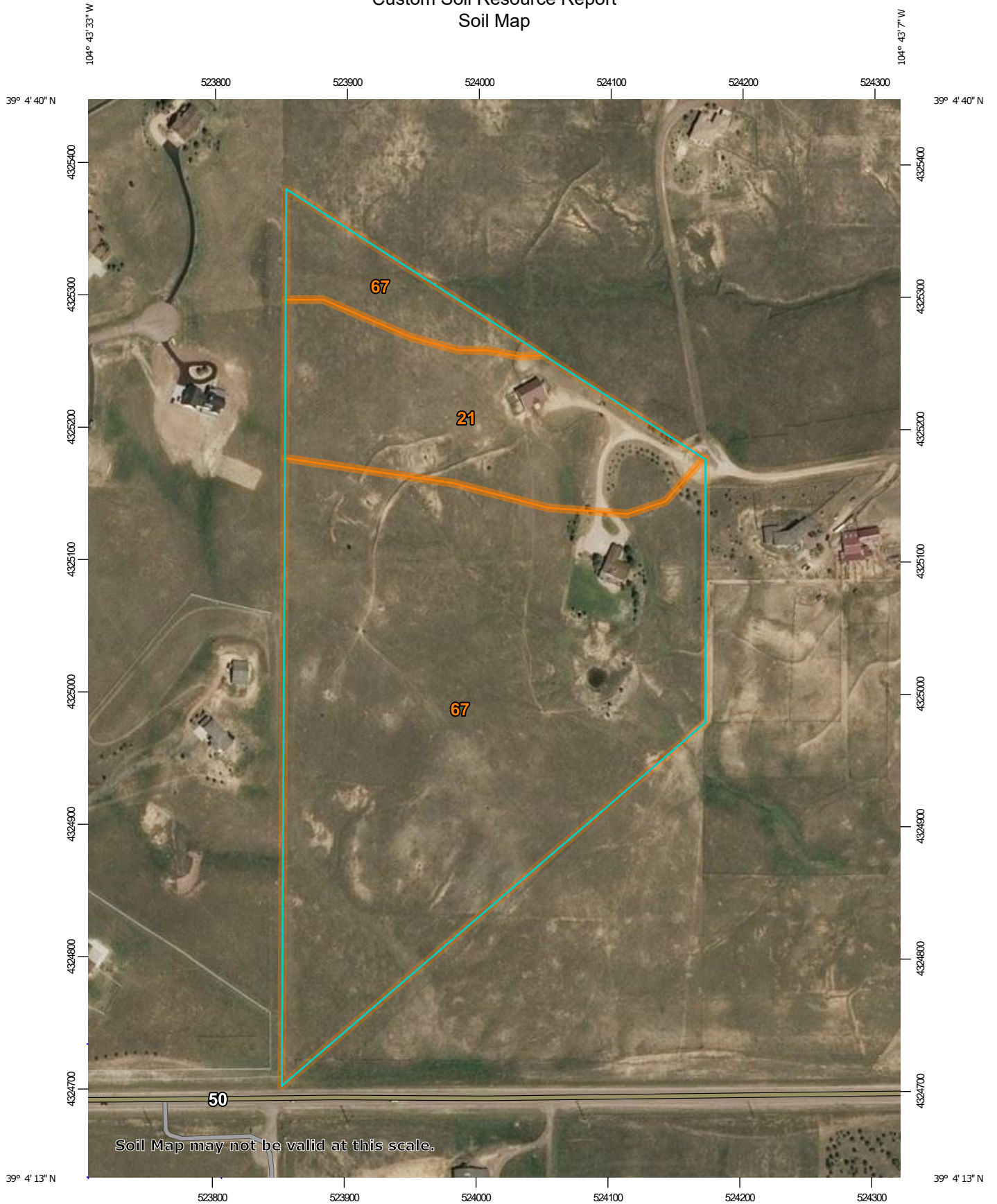
Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

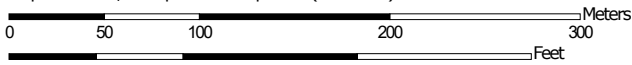
Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map




Map Scale: 1:3,970 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)


Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit


 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot


 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
21	Cruckton sandy loam, 1 to 9 percent slopes	7.6	21.9%
67	Peyton sandy loam, 5 to 9 percent slopes	27.1	78.1%
Totals for Area of Interest		34.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

21—Cruckton sandy loam, 1 to 9 percent slopes

Map Unit Setting

National map unit symbol: 367s
Elevation: 7,200 to 7,600 feet
Mean annual precipitation: 16 to 18 inches
Mean annual air temperature: 42 to 46 degrees F
Frost-free period: 110 to 120 days
Farmland classification: Not prime farmland

Map Unit Composition

Cruckton and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cruckton

Setting

Landform: Flats, hills
Landform position (three-dimensional): Side slope, talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from arkose

Typical profile

A - 0 to 11 inches: sandy loam
Bt - 11 to 28 inches: sandy loam
C - 28 to 60 inches: loamy coarse sand

Properties and qualities

Slope: 1 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
(0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: R049XB216CO - Sandy Divide
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

67—Peyton sandy loam, 5 to 9 percent slopes

Map Unit Setting

National map unit symbol: 369d
Elevation: 6,800 to 7,600 feet
Mean annual air temperature: 43 to 45 degrees F
Frost-free period: 115 to 125 days
Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: R049XB216CO - Sandy Divide
Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:

Landform: Depressions

Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

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Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

The main limitations of this complex for urban development are high frost action potential, moderate shrink-swell potential, the presence of stones and rock outcrop, and steep slopes. This complex requires special site or building designs because of the shrink-swell potential. Special designs are also needed when building on the steeper slopes. The roads and streets must have adequate cut-slope grade and be provided with drains to control surface runoff and keep soil losses to a minimum. Frost action is also a potential problem for the design of roads and streets. Capability subclass VIIe.

21—Cruckton sandy loam, 1 to 9 percent slopes. This deep, well drained soil formed in arkosic sandy loam deposits on uplands. Elevation ranges from 7,200 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 115 days.

Typically, the surface layer is dark grayish brown sandy loam about 4 inches thick. The subsoil is grayish brown sandy loam about 24 inches thick. The substratum is pale brown loamy coarse sand.

Included with this soil in mapping are small areas of Peyton sandy loam, 1 to 5 percent slopes, Peyton sandy loam, 5 to 9 percent slopes, and Pring coarse sandy loam, 3 to 8 percent slopes.

Permeability of this Cruckton soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate to high. Surface runoff is slow to medium, and the hazard of erosion is moderate. In places runoff from snowmelt in spring causes rills and small gullies to form in cultivated fields.

Most of this soil is in native grass that is used for grazing livestock. A small acreage on some of the more gentle slopes is used for small grain and corn for silage.

Native vegetation is mainly mountain muhly, bluestem, mountain brome, needleandthread, and blue grama. The soil is subject to invasion by Kentucky bluegrass and Gambel oak. Noticeable forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Proper location of livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings are generally suited to this soil. Soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for use as homesites. Special design of roads and streets is needed because of frost action. Installation of drains helps to control surface runoff and keeps soil losses to a minimum. Capability subclass VIe.

22—Cushman loam, 1 to 5 percent slopes. This moderately deep, well drained soil formed in calcareous loamy material derived from weakly consolidated beds of sandstone and shale on uplands. Elevation ranges from 6,000 to 6,500 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is grayish brown loam about 5 inches thick. The subsoil is brown sandy clay loam about 18 inches thick. The substratum is grayish brown fine sandy loam about 7 inches thick. Interbedded sandstone and shale are at a depth of 30 inches.

Included with this soil in mapping are small areas of Bresser sandy loam, 0 to 3 percent slopes; Kutch clay loam, 3 to 5 percent slopes; and Louviers silty clay loam, 3 to 18 percent slopes.

Permeability of this Cushman soil is moderate. Effective rooting depth is 20 to 40 inches. Available water capacity is low to moderate. Surface runoff is medium, and the hazard of erosion is moderate.

This soil is suitable for cultivation and is generally used for this purpose. Winter wheat is the main crop, and this is followed by a year of summer fallow because of limited precipitation. Feed grains such as millet and sorghum may be substituted for wheat in some years. Crop residue management and minimum tillage are the practices needed to protect this soil. The soil has a high producing potential, but production is reduced by low precipitation.

This soil is well suited to the production of native vegetation suitable for grazing. Native vegetation consists of western wheatgrass, needlegrasses, big bluestem, side-oats grama, blue grama, and native bluegrasses.

If the range has deteriorated, blue grama, junegrass, and native bluegrasses increase. Sleepygrass and annuals replace these grasses if the range is seriously deteriorated. Proper range management and proper location of livestock watering facilities are essential to help maintain the more desirable plants on this soil. Seeding is a good practice if the range is in poor condition. Seeding of the native vegetation is desirable, but the range can also be seeded with tame species of grasses such as Nordan crested wheatgrass, Russian wildrye, pubescent wheatgrass, or intermediate wheatgrass.

Windbreaks and environmental plantings are generally not suited to this soil. Onsite investigation is needed to determine if plantings are feasible.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing areas for nesting and escape cover. For pheasant, the provision of undisturbed nesting cover is

support a load and potential frost action on roads and streets. Roads and buildings can be designed to overcome these limitations. Capability subclass IVe.

67—Peyton sandy loam, 5 to 9 percent slopes. This deep, noncalcareous, well drained soil formed in alluvium and residuum derived from weathered arkosic sedimentary rock on uplands. Elevation ranges from 6,800 to 7,600 feet.

Typically, the surface layer is grayish brown sandy loam about 12 inches thick. The subsoil, about 23 inches thick, is pale brown sandy clay loam in the upper 13 inches and pale brown sandy loam in the lower 10 inches. The substratum is pale brown sandy loam to a depth of 60 inches.

Included with this soil in mapping are small areas of Holderness loam, 5 to 8 percent slopes; Pring coarse sandy loam, 3 to 8 percent slopes; and Tomah-Crowfoot loamy sands, 3 to 8 percent slopes.

Permeability of this soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium, and the hazard of erosion is moderate. Gullies and rills are common.

Most of the acreage of this Peyton soil is used as rangeland. Some areas are used for wheat and oats. Stubble mulching or other crop residue management practices are needed to control water erosion. Wildlife habitat is also an important use.

This soil is well suited to the production of native vegetation suitable for grazing. The native vegetation is mainly mountain muhly, bluestem, mountain brome, needleandthread, and blue grama. This soil is subject to invasion by Kentucky bluegrass and Gambel oak. Minor amounts of forbs such as hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat are in the stand.

Proper location of livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings generally are suited to this soil. Soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be necessary when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for homesites. The main limitation is the limited ability to support a load and potential frost action. Buildings and roads can be designed to overcome these limitations. Capability subclass IVe.

68—Peyton-Pring complex, 3 to 8 percent slopes. These gently sloping to moderately sloping soils are on valley side slopes and on uplands. Elevation ranges from 6,800 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

The Peyton soil makes up about 40 percent of the complex, the Pring soil about 30 percent, and other soils about 30 percent.

Included with these soils in mapping are areas of Holderness loam, 1 to 5 percent slopes; Holderness loam, 5 to 8 percent slopes; and Tomah-Crowfoot loamy sands, 3 to 8 percent slopes. In some places arkosic beds of sandstone and shale are at a depth of 0 to 40 inches.

The Peyton soil is commonly on the less sloping part of the landscape. It is deep, noncalcareous, and well drained. It formed in alluvium and residuum derived from weathered arkosic sedimentary rock. Typically, the surface layer is grayish brown sandy loam about 12 inches thick. The subsoil, about 23 inches thick, is pale brown sandy clay loam in the upper 13 inches and pale brown sandy loam in the lower 10 inches. The substratum is pale brown sandy loam to a depth of 60 inches or more.

Permeability of the Peyton soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium, and the hazard of erosion is moderate.

The Pring soil is deep, noncalcareous, and well drained. It formed in sandy sediment derived from weathered arkosic sedimentary rock. Typically, the surface layer is dark grayish brown coarse sandy loam about 4 inches thick. The substratum is dark grayish brown coarse sandy loam about 10 inches thick over pale brown gravelly sandy loam that extends to a depth of 60 inches or more.

Permeability of the Pring soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate.

These soils are used as rangeland, for wildlife habitat, and for homesites.

These soils are well suited to the production of native vegetation suitable for grazing. The dominant native species are mountain muhly, bluestem, needleandthread, and blue grama. These soils are subject to invasion of Kentucky bluegrass and Gambel oak. Common forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Properly locating livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings generally are suited to these soils. Soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good

2965

2965

Wilbur Thompson

STATE OF COLORADO

DEPARTMENT OF WATER RESOURCES

Office of State Engineer

Standard Plans, Drawings

and

SPECIFICATIONS

Including

RULES AND REGULATIONS

Pertaining to

THE FILING OF APPLICATIONS

for

THE APPROVAL

of

LIVESTOCK WATER TANKS

PURSUANT TO H.B. No. 750

SESSION LAWS OF 1941

DENVER, COLORADO, MAY 1, 1941



APPLICATION FOR APPROVAL OF LIVESTOCK WATER TANK

This application and Statement is made in conformity with provisions of the Livestock Water Tank Act of Colorado.

This application must be accompanied by a filing fee of one dollar, payable to the State Engineer of Colorado.

2965

Name of Owner Wilbur Thompson Motor Route # 3 Colorado Springs Colo. P. O. Address

Location of Tank SW 1/4 SE 1/4 Section 24, Township 11 So, Range 66 W. 6th PM

Name of water course on which tank is located None

Is water course normally dry Yes

Approximate area of drainage basin above tank 60 acres.

Nature of vegetative cover over drainage basin above tank Grassland

Character of topography of drainage basin (steep, medium or flat) Medium

Character of surface formation of drainage basin (rock, rocky soil, or soil) Soil

Approximate elevation of drainage basin above sea level 7800 feet.

Is water course subject to floods at times Yes

Height of top of dam above bottom of water course 10.0 feet.

Height of bottom of spillway above bottom of water course 7.0 feet.

Approximate capacity of tank 1.0 0/0 acre feet.

Location of spillway with respect to dam Around end

Bottom width of spillway at narrowest point 30.0 feet.

Distance of lower end of spillway below dam 50.0 feet.

Kind of formations in which spillway is located (rock, shale, clay, earth or mixture of soil and rock) Earth

Width of top of dam 8.0 feet.

Length of top of dam 141.0 feet.

Slope of upstream face of dam 3:1

Slope of downstream face of dam 2:1

Nature of riprap or other protection to be placed over water face of dam To be seeded to grass

Is the reservoir to be provided with an outlet pipe No.

If so, give kind and size of pipe

Give location by section, township and range, and size of every other stock tank now constructed in drainage basin in which this tank will be located No other dams in this drainage area

NOTE—Remainder of statements to be furnished by State Engineer's office.

Date of receipt of application by State Engineer NOV 29 1946, 19

Date of notice from applicant of completion of tank Oct. 25, 19 46.

Tank or site inspected by _____, 19

Recommendation of Inspector _____

Date of return of plans and specifications to applicant for correction or revision _____

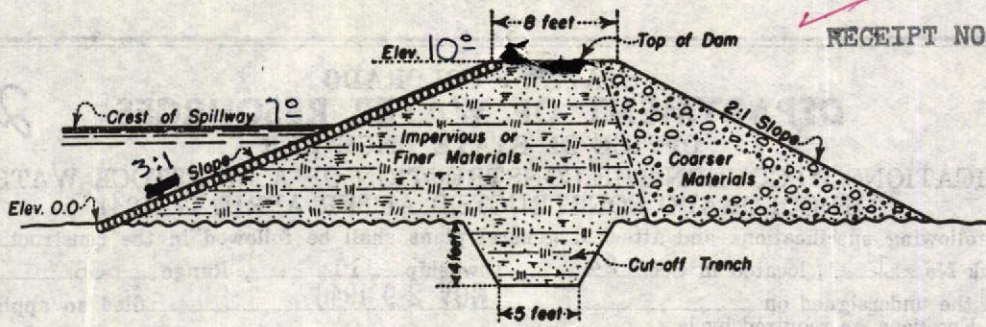
Reasons therefor _____

Filing Fee Paid NOV 29 1946, 19

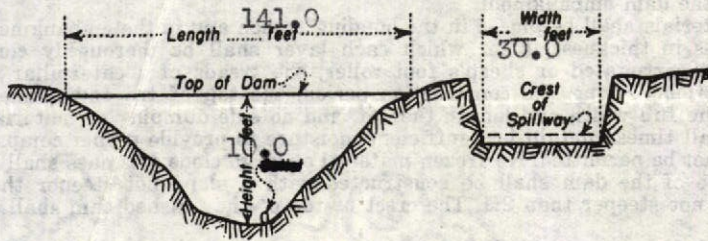
Application approved this 28th day of AUGUST, 1947.

Number assigned this stock tank is _____

By G.A. Brundage State Engineer Deputy

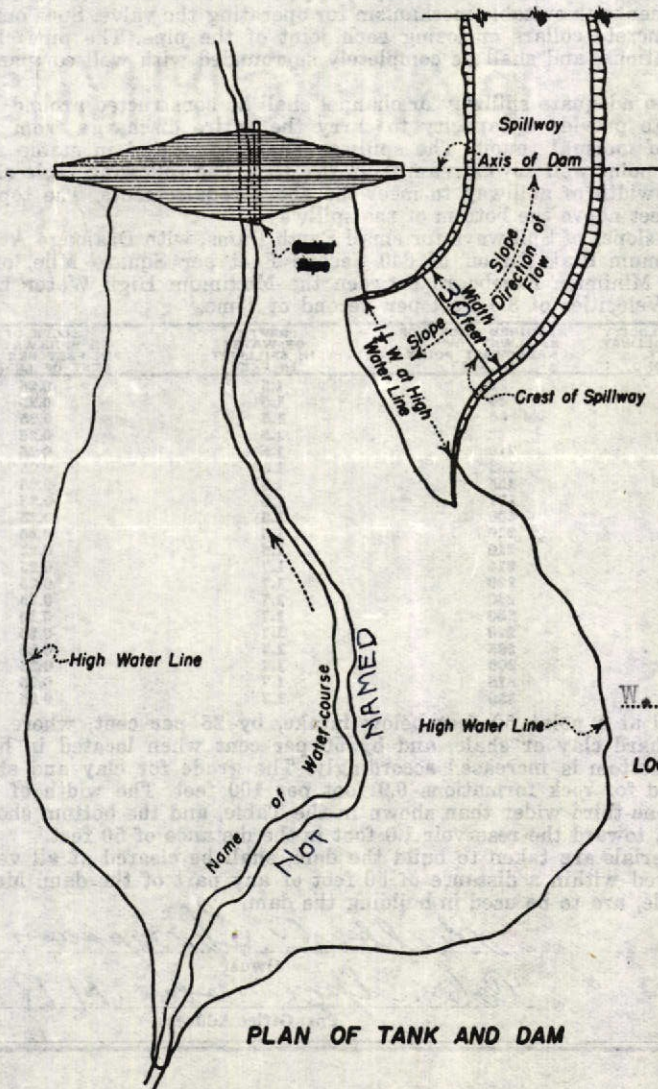


MAXIMUM CROSS-SECTION OF DAM



CROSS-SECTION OF DAM SITE AND SPILLWAY

Show length and height of dam and width of spillway on drawing



PLAN OF TANK AND DAM

RECEIPT NO. 2965

STATEMENT BY OWNER

Know all men by these presents: That the undersigned Wilbur Thompson whose postoffice address is M. Rt. 3 Colo. Spgs. has caused to be located this Stock Water Tank, the essential features of which are shown by this map and plans, which together with the accompanying application and statements are hereby filed with the State Engineer pursuant to the provisions of law.

First: Height of dam above bottom of water-course is 10.0 feet.

Second: Height of spillway above bottom of water-course is 7.0 feet.

Third: Total capacity of said Stock Water Tank is 10 acre feet.

Fourth: The source of supply for said Stock Water Tank is (name of stream) None

Fifth: Filing of this map and accompanying statements with the State Engineer was made on the 29th day of November, 1947.

Wilbur C. Thompson
Owner

MAP AND PLANS

FOR

W. Thompson STOCK WATER TANK AND DAM

LOCATED IN SECTION 24 TWP. 11⁵ RANGE 66 W OF 6th P.M.

El Paso COUNTY

DRAINAGE AREA ABOVE DAM 60 ACRES

Approved this 28th day of AUGUST, 1947

State Engineer
By [Signature]
Deputy

2965

STATE OF COLORADO
DEPARTMENT OF WATER RESOURCES
OFFICE OF STATE ENGINEER

**SPECIFICATIONS TO GOVERN THE CONSTRUCTION OF A LIVESTOCK WATER TANK
 IN COLORADO CONSTRUCTED AFTER APRIL 17, 1941**

The following specifications and attached general plans shall be followed in the construction of stock water tank No. 1, located in Sec. 24, Township 11, Range 66, for which the undersigned on NOV 29 1946 filed an application with the State Engineer, as required by law.

Preparation of Foundation for Dam—All vegetable matter of every description, including roots to a depth of two feet, shall be removed from the entire area upon which the dam will rest, following which the top six-inch layer of soil, together with boggy or unstable materials shall be removed and deposited outside the toes of the dam. The banks of the stream channel shall be dressed to a slope of about 1½:1. A bonding trench, with sloping sides and a bottom width of not less than 5 feet and depth of 4 feet, shall then be excavated beneath the center line of the dam the full length thereof, which trench shall be refilled with the most impervious materials available. The foundation of the dam shall then be lightly plowed lengthwise of the dam, to provide proper contact between the foundation and the dam embankment.

Placing of Dam Embankment—The materials shall be placed in the bonding trench and in the embankment of the dam in layers not exceeding 6 inches in thickness, after which each layer shall be thoroughly compacted by a heavily loaded disc cultivator, a corrugated or sheep's foot roller, the treads of a caterpillar or trucks, or by livestock used in the construction. During the construction period, the top of the embankment shall be maintained as a horizontal plane the full width and length thereof, and no side dumping of materials shall be permitted. The materials shall at all times contain just sufficient moisture to provide proper compaction. Puddling of material with water shall not be permitted. No frozen material or large clods or stones shall be incorporated in the dam. The upstream face of the dam shall be constructed with a slope not steeper than 2½:1, and the downstream face on a slope not steeper than 2:1. The crest or top of the finished dam shall be not less than 8 feet in width.

The upstream two-thirds of the dam shall be constructed of the most impervious materials, such as clay loam, or a mixture of clay and sand, and the downstream one third of more pervious material, such as sand or gravel. The upstream face of the dam shall be adequately protected against wave action by stone riprap, or other suitable materials.

Outlet—Should the state engineer so require, there shall be located beneath the dam a galvanized, corrugated steel pipe of No. 14 gauge and not less than 8 inches in diameter, equipped with a suitable control valve attached to the upstream end of the pipe, together with suitable mechanism for operating the valve. Such outlet pipe, when required, shall be provided with concrete collars enclosing each joint of the pipe. The pipe shall be placed in a trench bottomed in stable formations, and shall be completely surrounded with well compacted impervious materials.

Spillway—For the protection of the dam, an adequate spillway or channel shall be constructed around one or both ends of the dam, of sufficient width to provide a capacity to carry the entire discharge from the drainage basin above the dam during periods of unusual runoff. The spillway shall be located in stable formations not easily eroded, and shall extend to a point well downstream from the dam. The following table shall be used to determine the necessary depth and width of spillway to meet the above requirements. The top of the dam at all points shall be not less than 4 feet above the bottom of the spillway.

Table Showing Required Freeboard, Widths and slopes of Spillways for small Earth Dams, with Drainage Areas above the Same as Shown, Based upon a maximum Peak Runoff of 640 Second Feet per Square Mile, or 1 Second Foot per Acre, with an Allowance of a Minimum Freeboard between the Maximum High Water Line and Top of Dam, of 2.3 Feet, and Maximum Velocities of 3.5 Feet per Second of Time.

AREA OF DRAINAGE BASIN ABOVE DAM IN ACRES	PEAK RUNOFF IN CU. FT. PER SECOND	ASSUMED VELOCITY THROUGH SPILLWAY IN FEET PER SECOND	REQUIRED WIDTH OF SPILLWAY "W" AT NARROWEST POINT IN FEET	DEPTH OF WATER IN SPILLWAY IN FEET	SLOPE OF SPILLWAY IN FEET PER 100 FEET OF LENGTH
100	100	3.0	22	1.5	0.25
200	200	3.0	44	1.5	0.25
300	300	3.0	66	1.5	0.25
400	400	3.0	88	1.5	0.25
500	500	3.0	110	1.5	0.25
600	600	3.0	133	1.5	0.25
700	700	3.0	155	1.5	0.25
800	800	3.0	177	1.5	0.25
900	900	3.0	200	1.5	0.25
1000	1000	3.0	220	1.5	0.25
1100	1100	3.0	240	1.6	0.25
1200	1200	3.2	260	1.7	0.25
1300	1300	3.3	280	1.7	0.25
1400	1400	3.4	300	1.7	0.25
1500	1500	3.4	320	1.7	0.25
1600	1600	3.5	340	1.7	0.25
1700	1700	3.5	360	1.7	0.25
1800	1800	3.5	380	1.7	0.25
1900	1900	3.5	400	1.7	0.25
2000	2000	3.5	420	1.7	0.25

The above spillway widths may be reduced at a point 50 feet below intake, by 25 per cent, where the spillway is located the full length thereof in hard clay or shale, and by 50 per cent when located in hard rock formations, if the slope or grade of the bottom is increased accordingly. The grade for clay and shale formations should be 0.30 foot per 100 feet, and for rock formations 0.9 foot per 100 feet. The width of the entrance to the spillway must in all cases be one-third wider than shown in the Table, and the bottom should slope from the lower end of the funnel section, toward the reservoir 1.0 foot in the distance of 50 feet.

Borrow Pits—Borrow pits, from which materials are taken to build the dam, shall be cleared of all vegetable matter, and no material shall be borrowed within a distance of 50 feet of any part of the dam. Materials excavated from the spillway, when suitable, are to be used in building the dam.

Date X Nov. 25 1946
 NOV 29 1946

X Milton C. Thompson
 Owner
Colo. Spgs Colo. M R #3
 Post Office Address

2 Hydrologic Calculations

Runoff Coefficients and Percent Imperviousness Table 6-6

Colorado Springs Rainfall Intensity Duration Frequency Table 6-5

Hydrologic Calculations Summary Form SF-1 for Existing & Developed Conditions

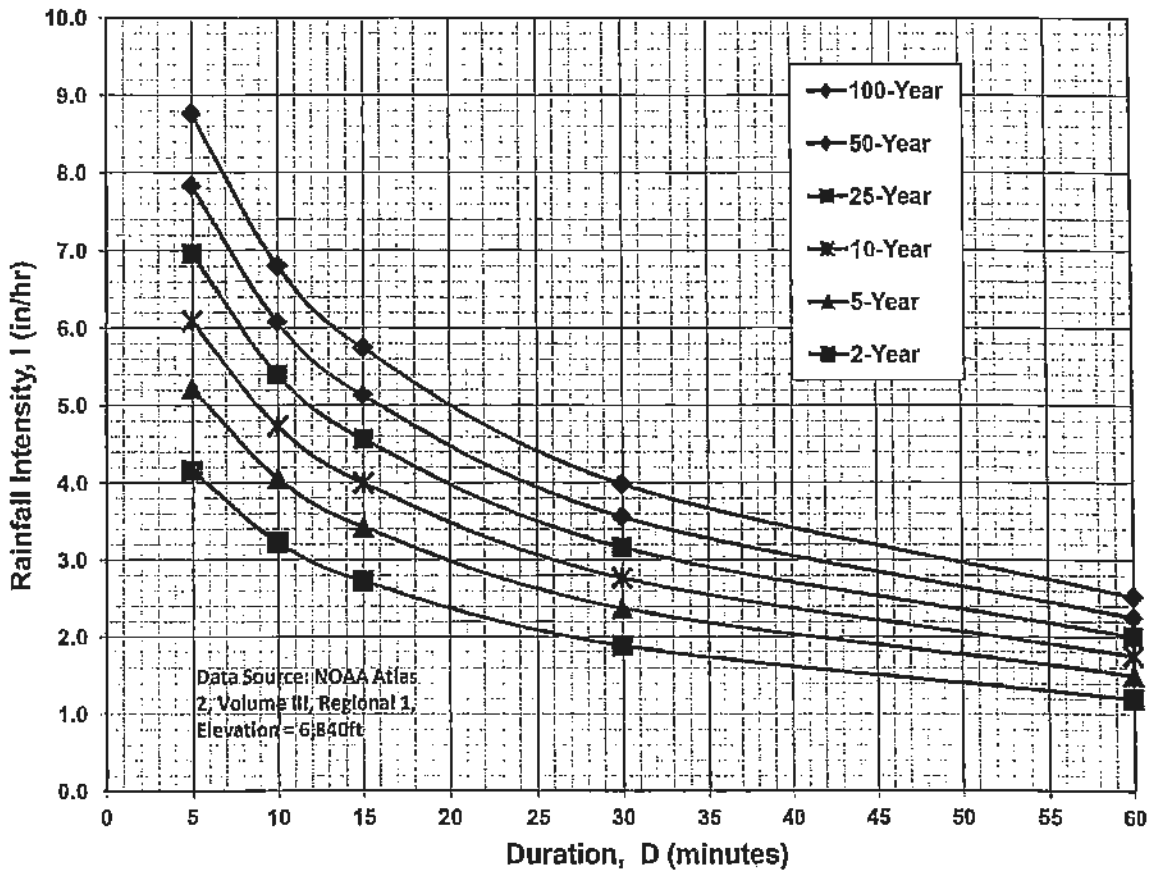
Hydrologic Calculations Summary 5-yr Form SF-2 for Existing & Developed Conditions

Hydrologic Calculations Summary 100-yr Form SF-2 for Existing & Developed Conditions

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries													
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks													
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs													
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns													
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

Job No.: **61160**
 Project: **Misfit Crew Estates**
 Design Storm: **5-Year Storm (20% Probability)**
 Jurisdiction: **DCM**

Date: **9/27/2024 13:28**
 Calcs By: **TJW**
 Checked By: _____

Sub-Basin and Combined Flows (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C5	Direct Runoff				Combined Runoff				Streetflow			Pipe Flow				Travel Time		
				t _c (min)	CA (Acres)	I5 (in/hr)	Q5 (cfs)	t _c (min)	CA (Acres)	I5 (in/hr)	Q5 (cfs)	Slope (%)	Length (ft)	Q (cfs)	Q (cfs)	Slope (%)	Mnngs n	Length (ft)	D _{Pipe} (in)	Length (ft)	v _{0sc} (ft/s)
DP1	A1	3.89	0.08	20.4	0.32	3.06	0.99	20.4	0.32	3.06	7.3										
		3.89	0.08																		
DP2	OS-B1	1.22	0.08	18.8	0.10	3.19	0.31	26.7	1.87	2.66	5.0										
	B2	20.92	0.08	26.1	1.78	2.69	4.77														
EX-DP3	EX-C1	11.01	0.08	22.1	0.88	2.94	2.59	22.1	0.88	2.94	24.7										
PP-DP3	PP-C1	11.01	0.10	21.8	1.10	2.96	3.26	21.8	1.10	2.96	25.4										

DCM: $I = C1 * \ln(tc) + C2$
 C1: 1.5
 C1: 7.583

Job No.: **61160**
 Project: **Misfit Crew Estates**
 Design Storm: **100-Year Storm (1% Probability)**
 Jurisdiction: **DCM**

Date: **9/27/2024 13:28**
 Calcs By: **TJW**
 Checked By: _____

Sub-Basin and Combined Flows (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C100	Direct Runoff				Combined Runoff				Streetflow			Pipe Flow				Travel Time		
				t _c (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	t _c (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	Slope (%)	Length (ft)	Q (cfs)	Q (cfs)	Slope (%)	Mnngs n	Length (ft)	D _{Pipe} (in)	Length (ft)	v _{0sc} (ft/s)
DP1	A1	3.89	0.35	20.4	1.37	5.13	7.03	20.4	1.37	5.13	21.7										
		3.89	0.35																		
DP2	OS-B1	1.22	0.35	18.8	0.43	5.35	2.28	26.7	7.82	4.46	34.9										
	B2	20.92	0.35	26.1	7.40	4.51	33.36														
EX-DP3	EX-C1	11.01	0.35	22.1	3.86	4.94	19.04	22.1	3.86	4.94	72.3										
PP-DP3	PP-C1	11.01	0.38	21.8	4.19	4.97	20.82	21.8	4.19	4.97	74.1										

DCM: $I = C1 * \ln(tc) + C2$
 C1: 2.52
 C1: 12.735

Sub-Basin Ex-A1 Runoff Calculations

Job No.: 61160
 Project: Misfit Crew Estates
 Jurisdiction: DCM
 Runoff Coefficient: Surface Type

Date: 9/27/2024 13:28
 Calcs by: TJW
 Checked by: _____
 Soil Type: B
 Urbanization: Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	168,736	3.87	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved			0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	789	0.02	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	169,525	3.89	0.02	0.08	0.15	0.25	0.30	0.35	0.4%

169525

Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max,Overland}$	300 ft	C_v	7		
	L (ft)	ΔZ_0 (ft)	S_0 (ft/ft)	v (ft/s)	t (min)	t_{Alt} (min)
Total	500	26	-	-	-	-
Initial Time	300	16	0.053	-	18.3	N/A DCM Eq. 6-8
Shallow Channel	200	10	0.050	1.6	2.1	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				t_c	20.4 min.	

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.44	3.06	3.57	4.08	4.59	5.13
Runoff (cfs)	0.2	1.0	2.1	4.0	5.4	7.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.2	1.0	2.1	4.0	5.4	7.0

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Combined Sub-Basin Runoff Calculations - DP1

Includes Basins A1

Job No.:	61160	Date:	9/27/2024 13:28
Project:	Misfit Crew Estates	Calcs by:	TJW
Jurisdiction	DCM	Checked by:	
Runoff Coefficient	Surface Type	Soil Type	B
		Urbanization	Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	168,736	3.87	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved	-	0.00	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	789	0.02	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	169,525	3.89	0.02	0.08	0.15	0.25	0.30	0.35	0.4%

Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ_0 (ft)	Q_i (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	A1	-	500	26	-	-	-	-	20.4
Channelized-1									
Channelized-2									
Channelized-3									
Total			500	26					
								t_c (min)	20.4

Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas Off-Site DP8

Q_{Minor} 6.3 (cfs) - 5-year Storm

Q_{Major} 14.7 (cfs) - 100-year Storm

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.44	3.06	3.57	4.08	4.59	5.13
Site Runoff (cfs)	0.22	0.99	2.12	4.01	5.40	7.03
OffSite Runoff (cfs)	-	6.30	-	-	-	14.70
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	7.3	-	-	-	21.7

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

Sub-Basin OS-B1 Runoff Calculations

Job No.: 61160
 Project: Misfit Crew Estates
 Jurisdiction: DCM
 Runoff Coefficient: Surface Type

Date: 9/27/2024 13:28
 Calcs by: TJW
 Checked by: _____
 Soil Type: B
 Urbanization: Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	53,170	1.22	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved			0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs			0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	53,170	1.22	0.02	0.08	0.15	0.25	0.30	0.35	0.0%

Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max,Overland}$	ΔZ_0 (ft)	S_0 (ft/ft)	v (ft/s)	t (min)	t_{Alt} (min)
Total	300	15	-	-	-	-
Initial Time	300	15	0.050	-	18.8	N/A DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				t_c	18.8 min.	

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.55	3.19	3.72	4.25	4.78	5.35
Runoff (cfs)	0.1	0.3	0.7	1.3	1.8	2.3
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.1	0.3	0.7	1.3	1.8	2.3

DCM: $I = C1 * \ln(tc) + C2$

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Sub-Basin B2 Runoff Calculations

Job No.: 61160
 Project: Misfit Crew Estates
 Jurisdiction: DCM
 Runoff Coefficient: Surface Type

Date: 9/27/2024 13:28
 Calcs by: TJW
 Checked by: _____
 Soil Type: B
 Urbanization: Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	904,717	20.77	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved	1,000	0.02	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	5,621	0.13	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	911,338	20.92	0.03	0.08	0.15	0.25	0.30	0.35	0.7%

911338

Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max,Overland}$	ΔZ_0 (ft)	S_0 (ft/ft)	v (ft/s)	t (min)	t_{Alt} (min)
Total	1,200	78	-	-	-	-
Initial Time	300	17	0.057	-	17.9	N/A DCM Eq. 6-8
Shallow Channel	900	61	0.068	1.8	8.2	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				t_c	26.1 min.	

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.15	2.69	3.14	3.58	4.03	4.51
Runoff (cfs)	1.1	4.8	10.1	19.0	25.6	33.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.1	4.8	10.1	19.0	25.6	33.4

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Combined Sub-Basin Runoff Calculations - DP2

Includes Basins OS-B1 B2

Job No.:	61160	Date:	9/27/2024 13:28
Project:	Misfit Crew Estates	Calcs by:	TJW
Jurisdiction	DCM	Checked by:	
Runoff Coefficient	Surface Type	Soil Type	B
		Urbanization	Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	957,887	21.99	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved	1,000	0.02	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	5,621	0.13	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	964,508	22.14	0.02	0.08	0.15	0.25	0.30	0.35	0.6%

Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ_0 (ft)	Q_i (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	OS-B1	-	300	15	-	-	-	-	18.8
Channelized-1	Trap Ditch	2	1,200	78	2	2	2	2.5	8.0
Channelized-2									
Channelized-3									
Total			1,500	93					

2 = Natural, Winding, minimal vegetation/shallow grass

t_c (min) **26.7**

Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas

Q_{Minor}	(cfs) - 5-year Storm
Q_{Major}	(cfs) - 100-year Storm

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.13	2.66	3.10	3.54	3.98	4.46
Site Runoff (cfs)	1.17	4.98	10.58	19.90	26.78	34.87
OffSite Runoff (cfs)	-	0.00	-	-	-	0.00
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	5.0	-	-	-	34.9

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

Sub-Basin EX-C1 Runoff Calculations

Job No.: 61160
 Project: Misfit Crew Estates
 Jurisdiction: DCM
 Runoff Coefficient: Surface Type

Date: 9/27/2024 13:28
 Calcs by: TJW
 Checked by: _____
 Soil Type: B
 Urbanization: Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	479,797	11.01	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved			0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs			0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	479,797	11.01	0.02	0.08	0.15	0.25	0.30	0.35	0.0%

Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max,Overland}$	ΔZ_0 (ft)	S_0 (ft/ft)	v (ft/s)	t (min)	t_{Alt} (min)
Total	1,110	54	-	-	-	-
Initial Time	300	26	0.087	-	15.6	N/A DCM Eq. 6-8
Shallow Channel	410	18	0.044	1.5	4.7	- DCM Eq. 6-9
Channelized	400	10	0.025	3.8	1.8	- Trap Ditch
				t_c	22.1 min.	

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.35	2.94	3.43	3.92	4.41	4.94
Runoff (cfs)	0.5	2.6	5.7	10.8	14.6	19.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.5	2.6	5.7	10.8	14.6	19.0

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Sub-Basin PP-C1 Runoff Calculations

Job No.: 61160
 Project: Misfit Crew Estates
 Jurisdiction: DCM
 Runoff Coefficient: Surface Type

Date: 9/27/2024 13:28
 Calcs by: TJW
 Checked by: _____
 Soil Type: B
 Urbanization: Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow			0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved			0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs			0.71	0.73	0.75	0.78	0.8	0.81	90%
5 Acre	479,797	11.01	0.06	0.1	0.2	0.29	0.34	0.38	7%
Combined	479,797	11.01	0.06	0.10	0.20	0.29	0.34	0.38	7.0%

Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max,Overland}$	ΔZ_0 (ft)	S_0 (ft/ft)	v (ft/s)	t (min)	t_{Alt} (min)
	300	54	-	-	-	-
Total	1,110	54	-	-	-	-
Initial Time	300	26	0.087	-	15.3	N/A DCM Eq. 6-8
Shallow Channel	410	18	0.044	1.5	4.7	- DCM Eq. 6-9
Channelized	400	10	0.025	3.8	1.8	- Trap Ditch
				t_c	21.8 min.	

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.37	2.96	3.46	3.95	4.45	4.97
Runoff (cfs)	1.6	3.3	7.6	12.6	16.6	20.8
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.6	3.3	7.6	12.6	16.6	20.8

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Combined Sub-Basin Runoff Calculations - EX-DP3

Includes Basins EX-C1

Job No.:	61160	Date:	9/27/2024 13:28
Project:	Misfit Crew Estates	Calcs by:	TJW
Jurisdiction	DCM	Checked by:	
Runoff Coefficient	Surface Type	Soil Type	B
		Urbanization	Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	479,797	11.01	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved	-	0.00	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	-	0.00	0.71	0.73	0.75	0.78	0.8	0.81	90%
5 Acre	-	0.00	0.06	0.1	0.2	0.29	0.34	0.38	7%
Combined	479,797	11.01	0.02	0.08	0.15	0.25	0.30	0.35	0.0%

Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ_0 (ft)	Q_i (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	EX-C1	-	1,110	54	-	-	-	-	22.1
Channelized-1									
Channelized-2									
Channelized-3									
Total			1,110	54					
							t_c (min)		22.1

Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas Off-Site DP10

Q_{Minor} 22.1 (cfs) - 5-year Storm

Q_{Major} 53.3 (cfs) - 100-year Storm

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.35	2.94	3.43	3.92	4.41	4.94
Site Runoff (cfs)	0.52	2.59	5.67	10.80	14.59	19.04
OffSite Runoff (cfs)	-	22.10	-	-	-	53.30
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	24.7	-	-	-	72.3

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

Combined Sub-Basin Runoff Calculations - PP-DP3

Includes Basins PP-C1

Job No.:	61160	Date:	9/27/2024 13:28
Project:	Misfit Crew Estates	Calcs by:	TJW
Jurisdiction	DCM	Checked by:	
Runoff Coefficient	Surface Type	Soil Type	B
		Urbanization	Non-Urban

Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	-	0.00	0.02	0.08	0.15	0.25	0.3	0.35	0%
Paved	-	0.00	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	-	0.00	0.71	0.73	0.75	0.78	0.8	0.81	90%
5 Acre	479,797	11.01	0.06	0.1	0.2	0.29	0.34	0.38	7%
Combined	479,797	11.01	0.06	0.10	0.20	0.29	0.34	0.38	7.0%

Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ_0 (ft)	Q_i (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	PP-C1	-	1,110	54	-	-	-	-	21.8
Channelized-1									
Channelized-2									
Channelized-3									
Total			1,110	54					
								t_c (min)	21.8

Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas Off-Site DP10

Q_{Minor} 22.1 (cfs) - 5-year Storm

Q_{Major} 53.3 (cfs) - 100-year Storm

Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.37	2.96	3.46	3.95	4.45	4.97
Site Runoff (cfs)	1.57	3.26	7.62	12.62	16.65	20.82
OffSite Runoff (cfs)	-	22.10	-	-	-	53.30
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	25.4	-	-	-	74.1

DCM: $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

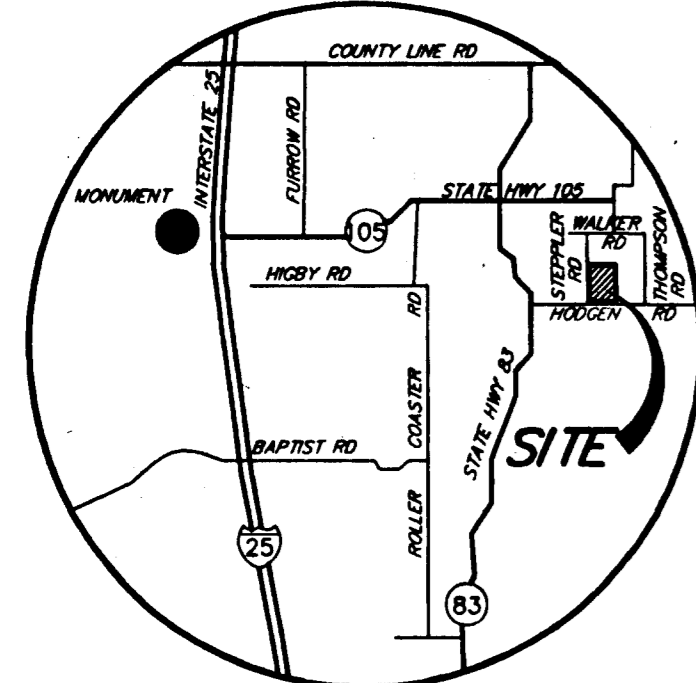
Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

3 Hydraulic Calculations

Drainage Map for Longview Estates
Excerpts from URS Drainage Report
Culvert Calculations
Swale (Channel) Calculations

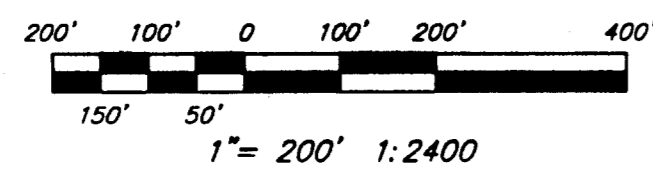
DRAINAGE MAP



VICINITY MAP
NTS



SCALE



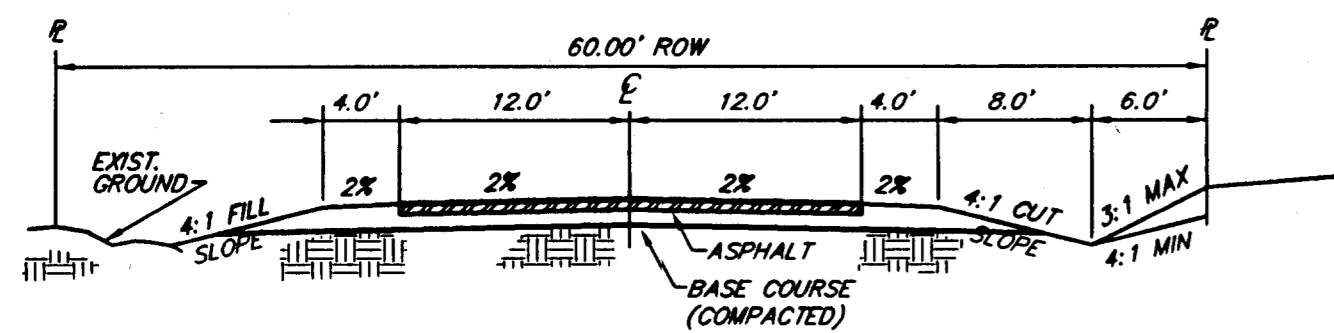
LEGEND

- DRAINAGE BASIN BOUNDARY
- DRAINAGE BASIN DESIGNATION
- DRAINAGE BASIN AREA
- DESIGN POINT DESIGNATION
- FLOW DIRECTION
- Tc PATHWAY
- PROPOSED CULVERT
- EXISTING INDEX CONTOUR
- EXISTING INTERMEDIATE CONTOUR
- PROPOSED INDEX CONTOUR
- PROPOSED INTERMEDIATE CONTOUR

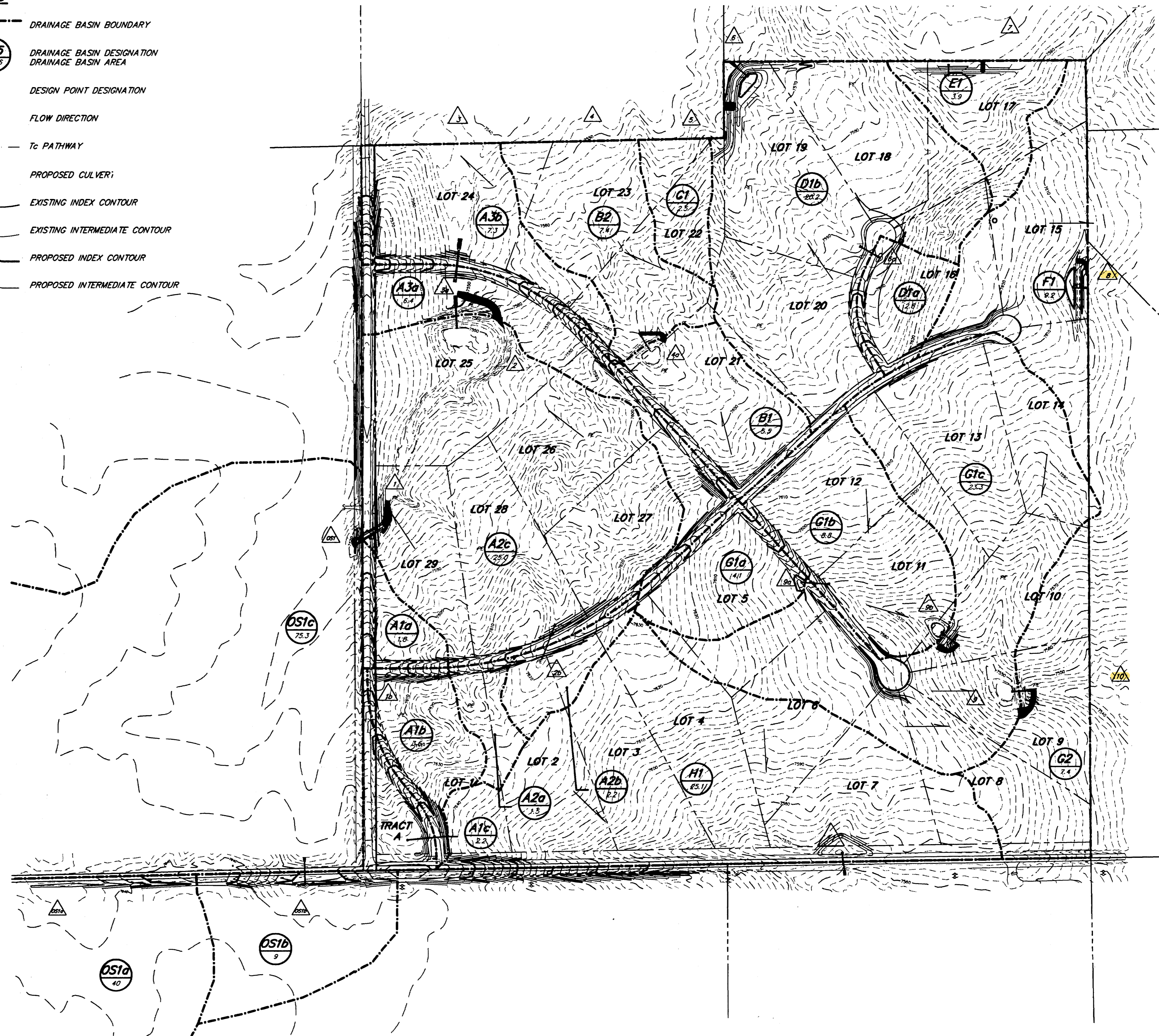
DESIGN POINT HYDROLOGIC DATA										
DP DESIGN POINT	EXISTING			DP DESIGN POINT	DEVELOPED			WITH DETENTION		STRUCTURE
	DRAINAGE AREA (AC)	Q5 (CFS)	Q100 (CFS)		DRAINAGE AREA (AC)	Q5 (CFS)	Q100 (CFS)	Q5 (CFS)	Q100 (CFS)	
OS1a	40±	---	59±	OS1a	40±	---	59±	---	---	
OS1b	9±	---	17±	OS1b	9±	---	17±	---	---	24" CULVERT
OS1	124.3	82.7	197.6	OS1	126.5	84.4	201.6	---	---	(2) 60" CULVERTS
1	132.0	88.2	211.2	1b	5.1	3.3	12.4	3.2	12.0	24" CULVERT
2	160.7	106.9	256.9	1	133.4	88.3	214.7	---	---	
3	172.3	114.9	276.6	2b	2.2	2.1	5.0	1.8	3.8	18" CULVERT
4	14.1	9.6	23.6	2	160.6	111.1	267.7	54.7	129.5	DETENTION POND
5	2.5	1.8	4.5	3a	167.0	54.7	130.4	51.8	125.8	60" CULVERT
6	23.7	16.3	40.0	3	174.3	51.8	125.8	---	---	
7	3.9	2.8	7.0	4a	5.5	5.0	11.7	2.6	6.2	DETENTION POND
8	8.5	6.2	15.2	4	12.9	8.5	19.7	---	---	
9	37.0	25.8	63.2	5	2.5	2.5	5.9	---	---	
10	44.4	31.2	76.4	6a	2.8	2.7	6.3	2.7	6.2	24" CULVERT
11	25.1	17.3	42.3	6	23.0	21.9	51.4	16.1	38.8	DETENTION POND
				7	3.9	3.9	9.2	1.6	2.8	DETENTION POND
				8	9.2	9.0	21.1	6.3	14.7	DETENTION POND
				9a	4.1	4.0	9.5	3.9	9.2	24" CULVERT
				9b	12.9	12.4	29.8	4.9	25.2	DETENTION POND
				9	36.2	24.7	70.7	18.7	45.7	DETENTION POND
				10	43.6	22.1	53.3	---	---	
				11	25.1	22.8	53.6	17.9	34.5	30" CULVERT

NOTES

- INDIVIDUAL LOT OWNERS WILL BE RESPONSIBLE FOR CONSTRUCTING POSITIVE DRAINAGE AWAY FROM ALL STRUCTURES.
- INDIVIDUAL LOT OWNERS WILL BE RESPONSIBLE FOR PLACING DRIVEWAY CULVERTS. EL PASO COUNTY D.O.T. IS REQUIRED TO BE CONTACTED FOR A DRIVEWAY PERMIT. MINIMUM DRIVEWAY CULVERTS SIZE SHALL BE 18" DIAMETER.



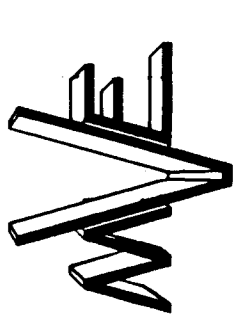
TYPICAL ROAD SECTION
NTS



DEVELOPED CONDITION

PROJECT: LONGVIEW ESTATES

TITLE: DRAINAGE MAP - DEVELOPED CONDITION



MONUMENT VALLEY
ENGINEERS INC.
ENGINEERS & SURVEYORS
1903 LELARAY STREET
COLORADO SPRINGS, COLORADO 80909
PHONE (719) 635-5736

DISK NO.	
XREFS	
Exbase	
Subdiv	
Legend	
PROJ. NO.	60703
DRAWN:	BDK
ENGINEER:	CCC
CHECKED:	
SCALE:	1" = 200'
DATE:	10-31-2002
REVISIONS:	
NO.:	DATE:
ITEM:	

SHEET
2 OF 2
DRAWING NO.
60703010

URS

***FINAL DRAINAGE REPORT
Hodgen Road
State Highway 83 to Black Forest Road***

December 8, 2009

Submitted to



El Paso County Public Services Department – Transportation Division
&
Pikes Peak Rural Transportation Authority
Prepared by

URS

URS Corporation
9960 Federal Drive, Suite 300
Colorado Springs, CO 80921

URS Project No. 21711554

Culvert Analysis Report

P Cu 121+51

Analysis Component			
Storm Event	Design	Discharge	100.90 cfs

Peak Discharge Method: User-Specified			
Design Discharge	100.90 cfs	Check Discharge	49.60 cfs

Tailwater properties: Trapezoidal Channel

Tailwater conditions for Design Storm.			
Discharge	100.90 cfs	Bottom Elevation	7,519.00 ft
Depth	0.45 ft	Velocity	4.11 ft/s

Name	Description	Discharge	HW Elev.	Velocity
Culvert-1	1-42 inch Circular	100.90 cfs	7,528.27 ft	15.62 ft/s
Weir	Not Considered	N/A	N/A	N/A

Culvert Analysis Report

P Cu 121+51

Component: Culvert-1

Culvert Summary			
Computed Headwater Elevation	7,528.27 ft	Discharge	100.90 cfs
Inlet Control HW Elev.	7,528.27 ft	Tailwater Elevation	7,519.45 ft
Outlet Control HW Elev.	7,528.17 ft	Control Type	Inlet Control
Headwater Depth/Height	1.58		
Grades			
Upstream Invert	7,522.73 ft	Downstream Invert	7,519.24 ft
Length	164.00 ft	Constructed Slope	0.021280 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	2.23 ft
Slope Type	Steep	Normal Depth	2.13 ft
Flow Regime	Supercritical	Critical Depth	3.08 ft
Velocity Downstream	15.62 ft/s	Critical Slope	0.009054 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.50 ft
Section Size	42 inch	Rise	3.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	7,528.17 ft	Upstream Velocity Head	1.97 ft
Ke	0.20	Entrance Loss	0.39 ft
Inlet Control Properties			
Inlet Control HW Elev.	7,528.27 ft	Flow Control	Submerged
Inlet Type	Beveled ring, 33.7° bevels	Area Full	9.6 ft ²
K	0.00180	HDS 5 Chart	3
M	2.50000	HDS 5 Scale	B
C	0.02430	Equation Form	1
Y	0.83000		

Oversight / NHS

FHWA REGION VIII OVERSIGHT? NO YES

NATIONAL HIGHWAY SYSTEM? NO YES

EL PASO COUNTY PUBLIC SERVICES DEPARTMENT TRANSPORTATION DIVISION

PLAN OF PROPOSED

HODGEN ROAD CORRIDOR IMPROVEMENTS PHASE 1 - SH 83 TO BLACK FOREST ROAD

CONSTRUCTION BID PLANS SEPTEMBER 1, 2010

Related Projects:

P. E. UNDER PROJECT:
PROJECT NUMBER:
PROJECT CODE:

R.O.W. Projects:
R.O.W. Project Description



EL PASO COUNTY PROJECT #75173



PPRTA PROJECT #06-00005

BEGIN PROJECT
HODGEN ROAD
STA 10+32.56

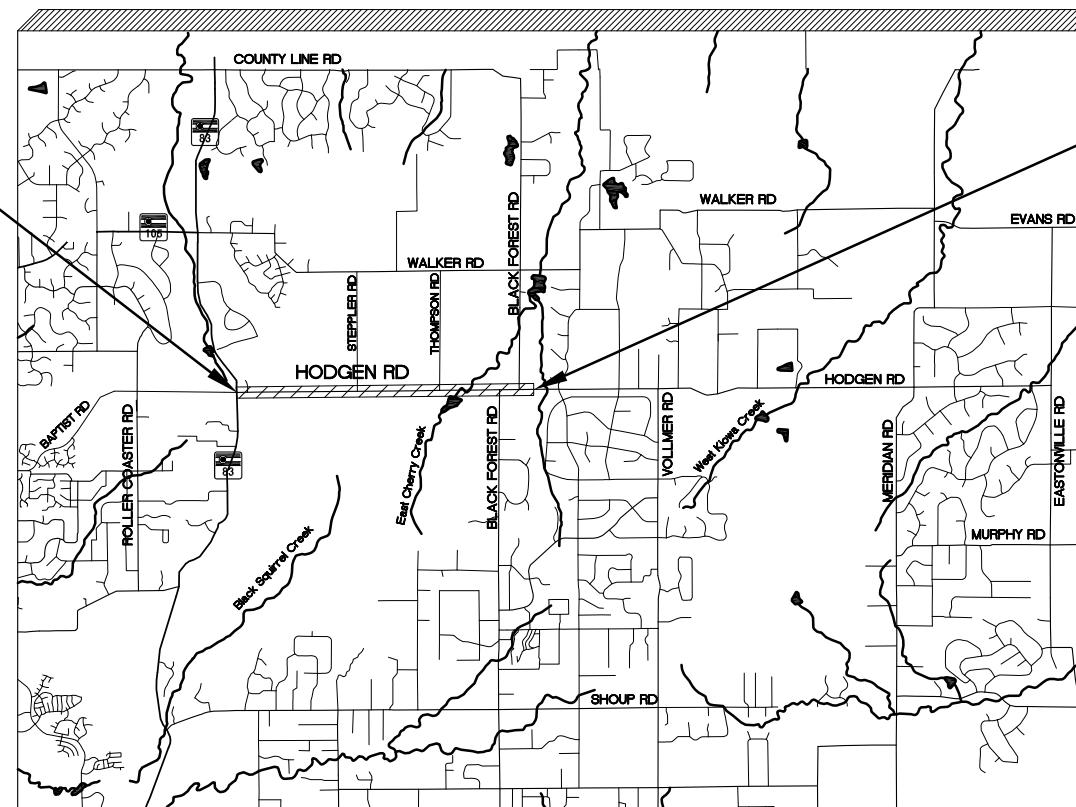
END PROJECT
HODGEN ROAD
STA 210+50.00

TABULATION OF LENGTH

BEGIN PROJECT STA 10+32.56	
END PROJECT STA 210+50.00	20,017.44 FT (3.79 MI)

DESIGN DATA

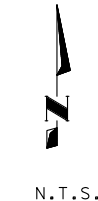
DESIGN SPEED	60 MPH
MINIMUM RADIUS	1505'
MAXIMUM SUPERELEVATION RATE	4%
MAXIMUM GRADE	8%



LOCATION MAP

INDEX OF SHEETS

1	TITLE SHEET	
2	STANDARD PLANS LIST	
3	GENERAL NOTES	
4	SUMMARY OF QUANTITIES	
5	to 12	TABULATION OF QUANTITIES
13	KEY MAP & LEGEND	
14	to 20	TYPICAL SECTIONS
21	to 22	GEOMETRIC LAYOUT
23	to 27	SURVEY CONTROL DIAGRAM
28	SURVEY TABULATION	
29	to 43	REMOVALS AND RESET
44	to 59	UTILITY PLAN
60	to 60	POTHOLE INFO
61	UTILITY CONFLICTS	
62	to 76	ROADWAY PLAN & PROFILE
77	to 80	APPROACH ROAD PLAN & PROFILE
81	to 86	DRIVEWAY PLAN & PROFILE
87	to 90	SWMP AND EROSION CONTROL NOTES
91	to 105	DRAINAGE AND EROSION CONTROL PLAN
106	to 112	DRAINAGE PROFILES AND DETAILS
113	to 116	CONSTRUCTION PHASING
117	to 131	SIGNING AND STRIPING PLAN
132	to 133	RETAINING WALL
134	to 137	EXPLORATORY BORINGS



As-Constructed

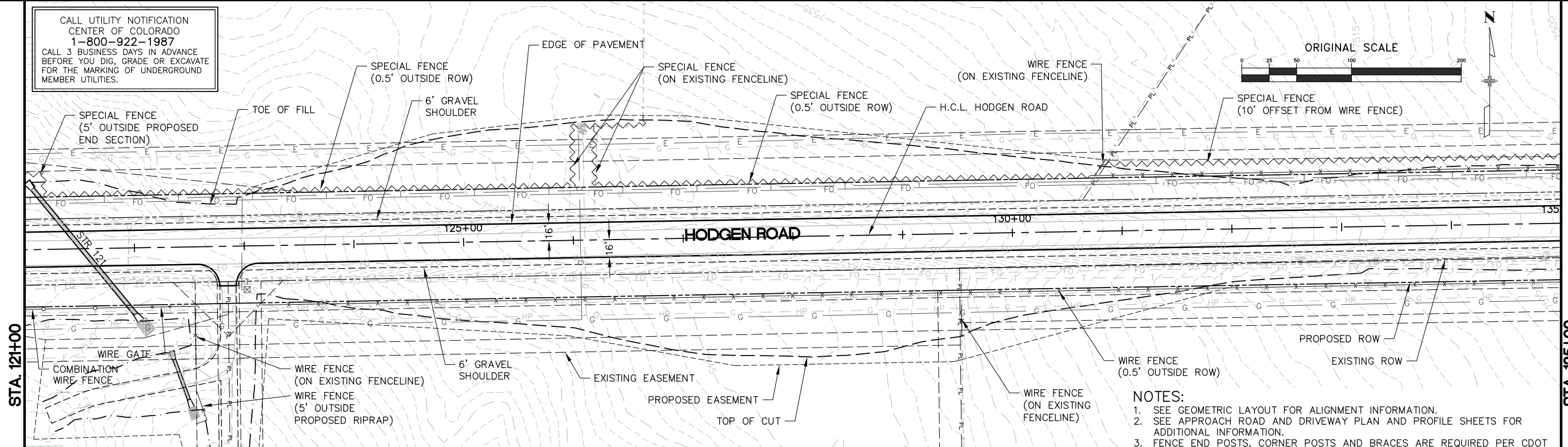
Ted J. J...
WILSON & COMPANY

1-6-12
DATE

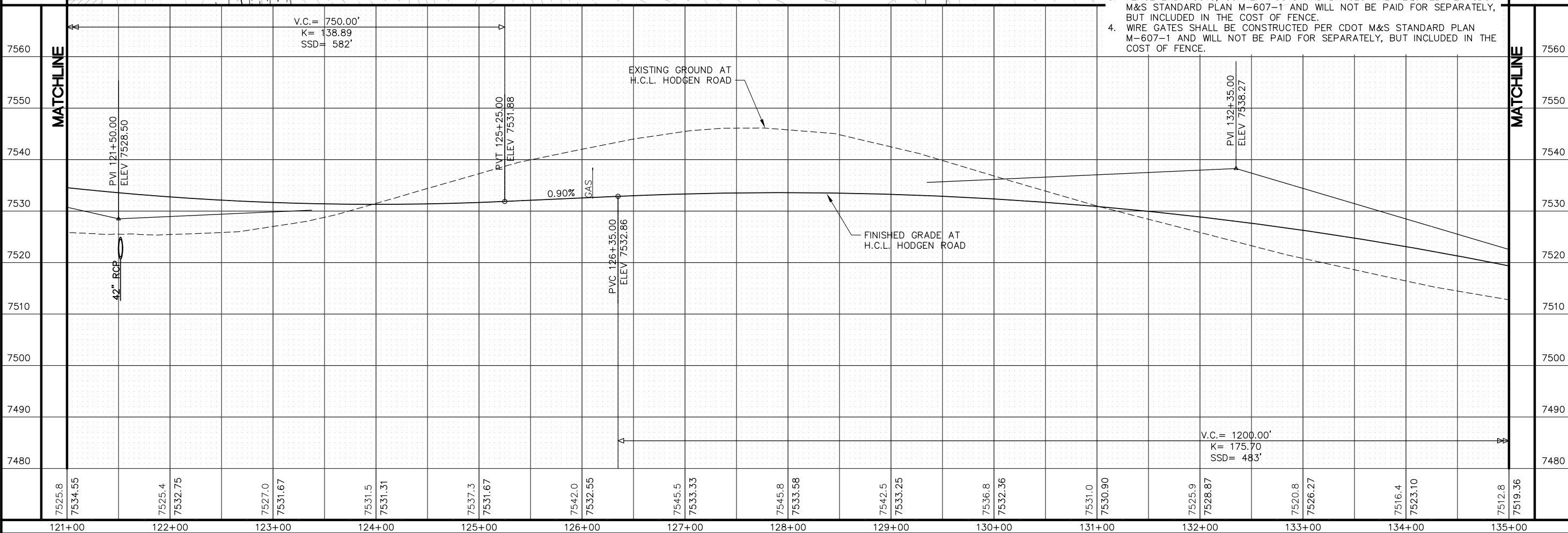


9960 FEDERAL DRIVE, SUITE 300
COLORADO SPRINGS, CO. 80921
PHONE (719) 531-0001
FAX (719) 531-0007

CALL UTILITY NOTIFICATION
 CENTER OF COLORADO
 1-800-922-1987
 CALL 3 BUSINESS DAYS IN ADVANCE
 BEFORE YOU DIG, GRADE OR EXCAVATE
 FOR THE MARKING OF UNDERGROUND
 MEMBER UTILITIES.



- NOTES:**
- SEE GEOMETRIC LAYOUT FOR ALIGNMENT INFORMATION.
 - SEE APPROACH ROAD AND DRIVEWAY PLAN AND PROFILE SHEETS FOR ADDITIONAL INFORMATION.
 - FENCE END POSTS, CORNER POSTS AND BRACES ARE REQUIRED PER CDOT M&S STANDARD PLAN M-607-1 AND WILL NOT BE PAID FOR SEPARATELY, BUT INCLUDED IN THE COST OF FENCE.
 - WIRE GATES SHALL BE CONSTRUCTED PER CDOT M&S STANDARD PLAN M-607-1 AND WILL NOT BE PAID FOR SEPARATELY, BUT INCLUDED IN THE COST OF FENCE.



Computer File Information

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Last Modification Date: 31-Aug-10 Operator: MPR
Mapped Path: p:\roadway\sheets
Drawing File Name: rdp-Hodgen.dwg
Acad. Ver. 2006 11x17 Scale: 1"=100'H: 20'V Units: Feet

Index of Revisions

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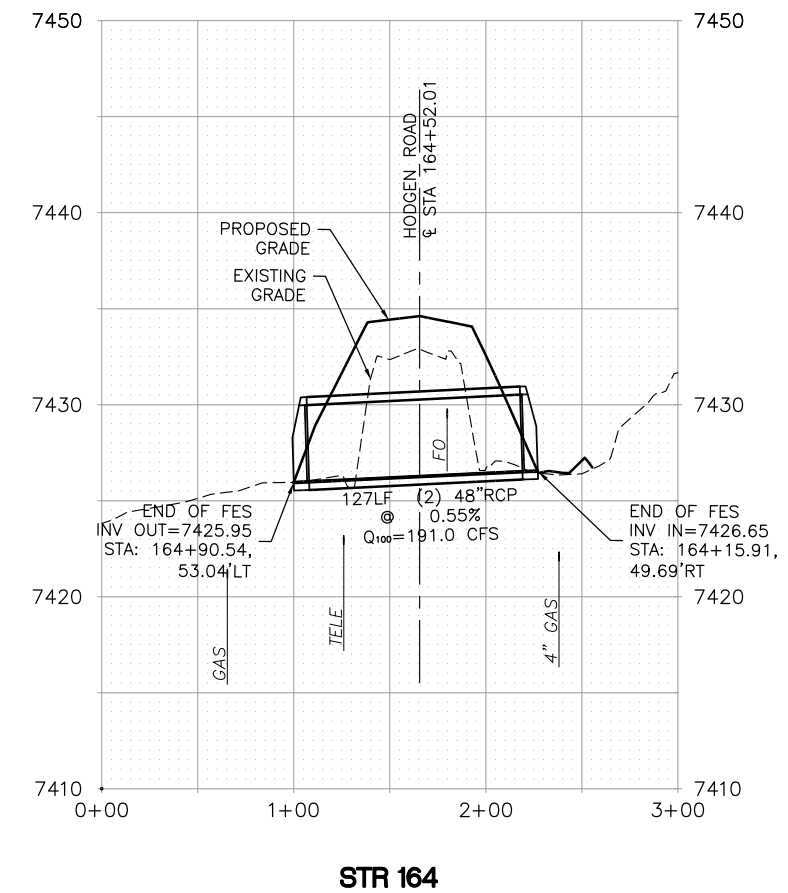
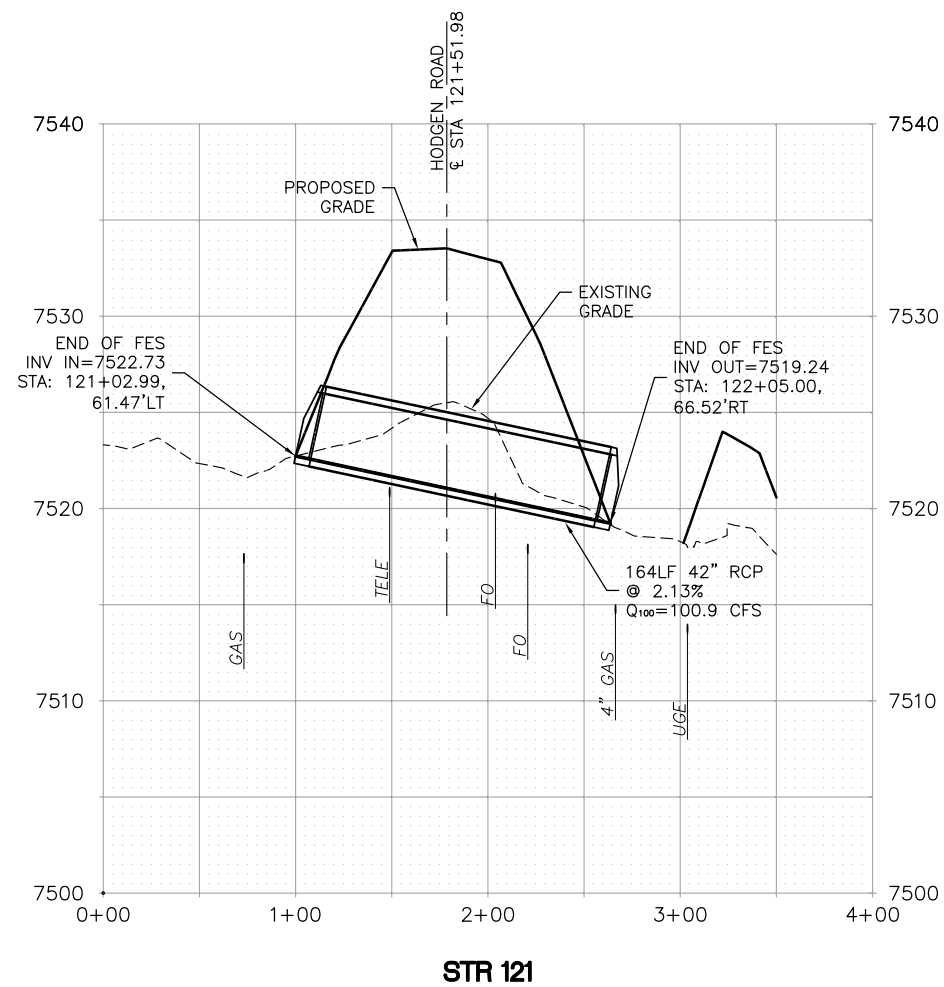
EPC #75173

PPRTA #06-00005

9960 FEDERAL DRIVE
 SUITE 300
 COLORADO SPRINGS, CO 80921
 (719) 531-0001

HODGEN ROAD CORRIDOR IMPROVEMENTS	JN: 21711554
ROADWAY PLAN AND PROFILE	Roadway: 9 of 15
STA. 121+00 TO STA. 135+00	Sheet Number 70

P:\Roadway\Sheets\rdpp-Hodgen.dwg, 121-135(9), 8/31/2010 12:55:41 PM, michael_ryan



NOTE:

- PIPES LENGTHS LISTED INCLUDE LENGTH OF FLARED END SECTIONS.

Computer File Information

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Acad. Ver. 2006 11x17 Scale: 1"=100H, 10'V Units: Feet	(R-)

Index of Revisions



EPC #75173



PPRTA #06-00005



9960 FEDERAL DRIVE
SUITE 300
COLORADO SPRINGS, CO 80921
(719) 531-0001

HODGEN ROAD CORRIDOR IMPROVEMENTS

DRAINAGE PROFILES

STA. 121+00 TO STA. 165+00

JN: 21711554

Drain. Profiles: 4 of 6

Sheet Number 109

Culvert Report

Sub-Basin B2 Driveway Culvert - 5 yr

Invert Elev Dn (ft)	=	100.00
Pipe Length (ft)	=	30.00
Slope (%)	=	2.00
Invert Elev Up (ft)	=	100.60
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.023
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Mitered to slope (C)
Coeff. K,M,c,Y,k	=	0.021, 1.33, 0.0463, 0.75, 0.7

Embankment

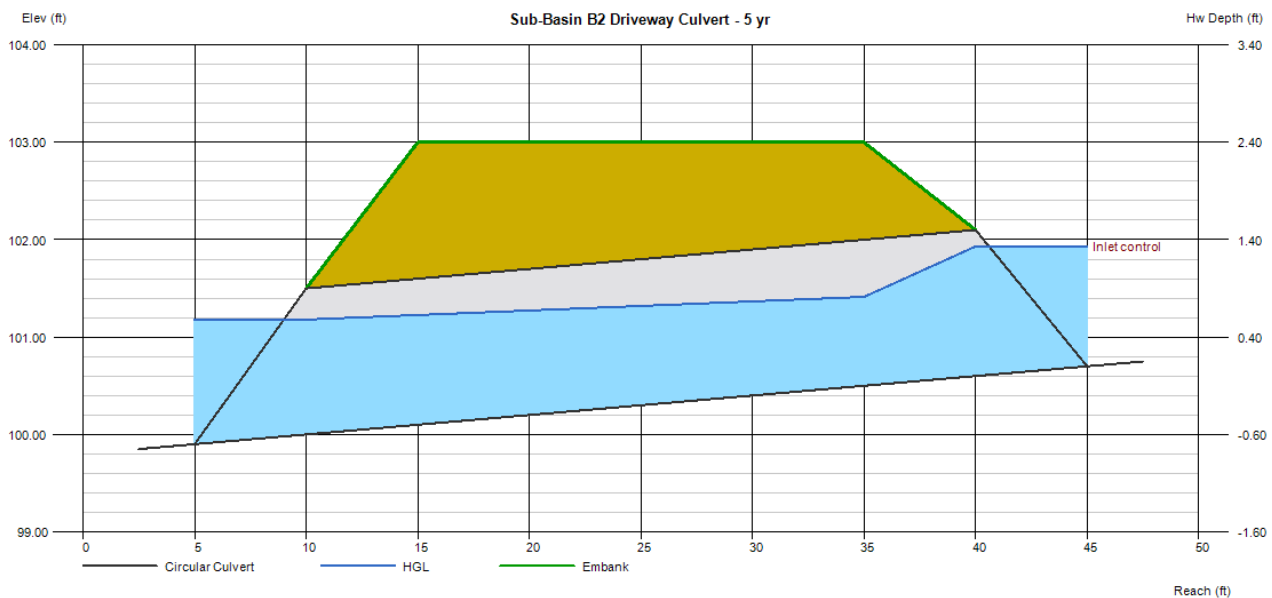
Top Elevation (ft)	=	103.00
Top Width (ft)	=	20.00
Crest Width (ft)	=	70.00

Calculations

Qmin (cfs)	=	5.00
Qmax (cfs)	=	5.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	5.00
Qpipe (cfs)	=	5.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	3.35
Veloc Up (ft/s)	=	4.77
HGL Dn (ft)	=	101.18
HGL Up (ft)	=	101.46
Hw Elev (ft)	=	101.93
Hw/D (ft)	=	0.89
Flow Regime	=	Inlet Control



Culvert Report

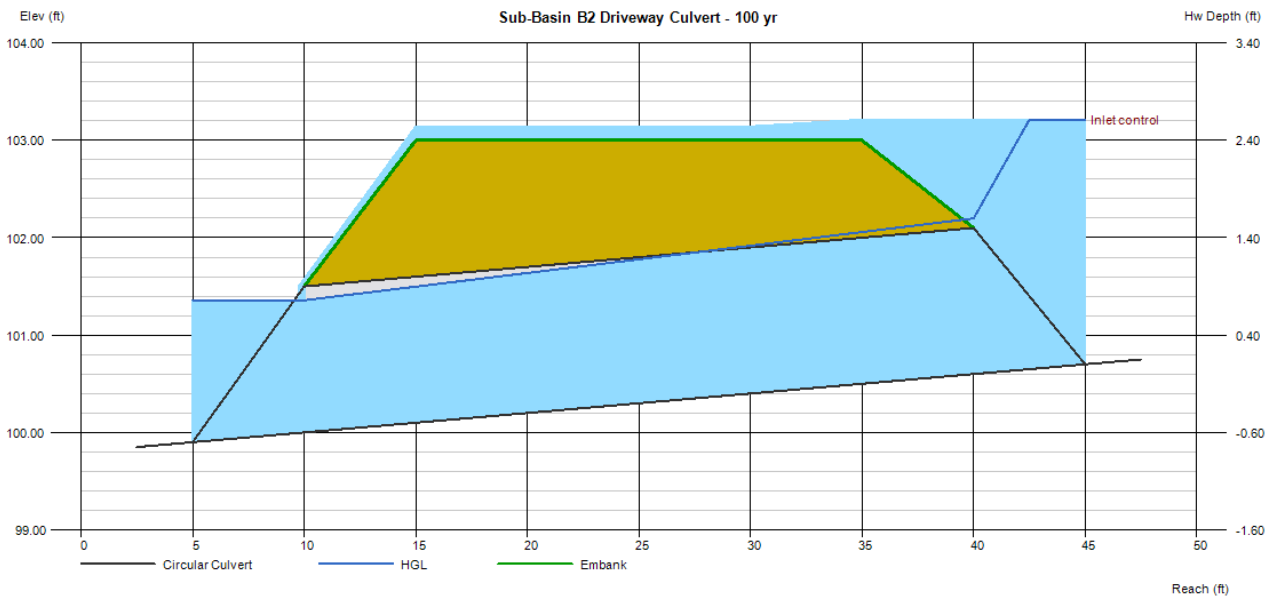
Sub-Basin B2 Driveway Culvert - 100 yr

Invert Elev Dn (ft)	=	100.00
Pipe Length (ft)	=	30.00
Slope (%)	=	2.00
Invert Elev Up (ft)	=	100.60
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.023
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Mitered to slope (C)
Coeff. K,M,c,Y,k	=	0.021, 1.33, 0.0463, 0.75, 0.7

Embankment	
Top Elevation (ft)	= 103.00
Top Width (ft)	= 20.00
Crest Width (ft)	= 70.00

Calculations	
Qmin (cfs)	= 34.90
Qmax (cfs)	= 34.90
Tailwater Elev (ft)	= (dc+D)/2

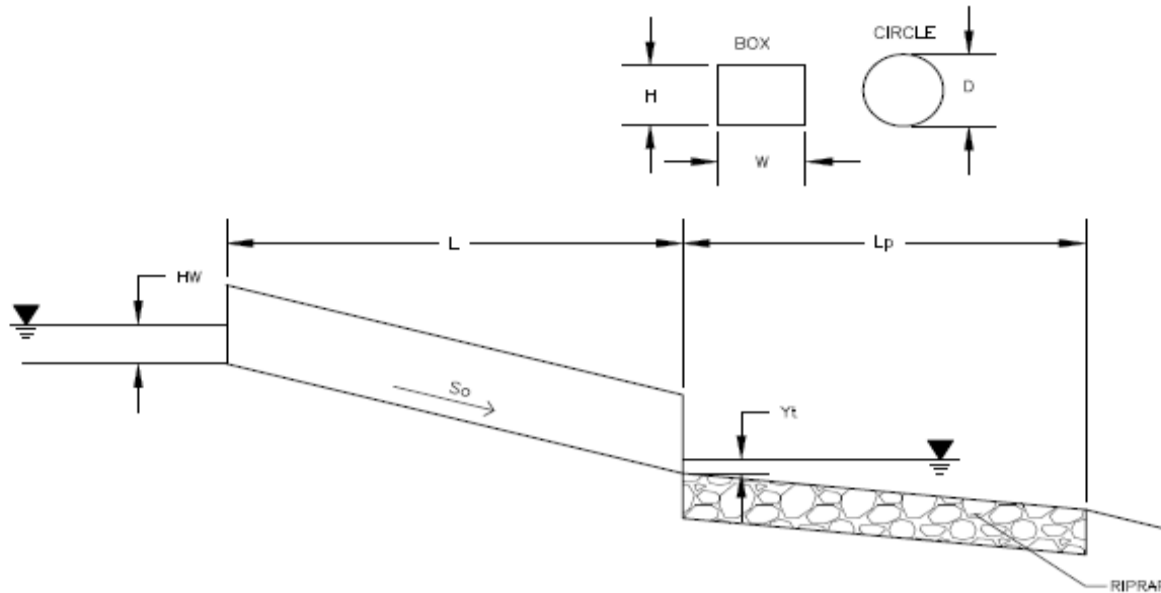
Highlighted	
Qtotal (cfs)	= 34.90
Qpipe (cfs)	= 9.94
Qovertop (cfs)	= 24.96
Veloc Dn (ft/s)	= 5.91
Veloc Up (ft/s)	= 5.63
HGL Dn (ft)	= 101.36
HGL Up (ft)	= 102.20
Hw Elev (ft)	= 103.21
Hw/D (ft)	= 1.74
Flow Regime	= Inlet Control



Determination of Culvert Headwater and Outlet Protection

Project: **61160 - Misfit Crew Estates**

Basin ID: **Driveway Culvert Outlet Protection - 9.94 cfs 100 yr Pipe Flow**



Soil Type:

Choose One:

Sandy

Non-Sandy

Supercritical Flow! Using Da to calculate protection type.

Design Information (Input):	
Design Discharge	Q = <input style="width: 100px;" type="text" value="9.94"/> cfs
Circular Culvert:	
Barrel Diameter in Inches	D = <input style="width: 100px;" type="text" value="18"/> inches
Inlet Edge Type (Choose from pull-down list)	<input style="width: 150px;" type="text" value="1.5 : 1 Beveled Edge"/> ▼
Box Culvert:	OR
Barrel Height (Rise) in Feet	Height (Rise) = <input style="width: 100px;" type="text"/>
Barrel Width (Span) in Feet	Width (Span) = <input style="width: 100px;" type="text"/>
Inlet Edge Type (Choose from pull-down list)	<input style="width: 150px;" type="text"/> ▼
Number of Barrels	No = <input style="width: 100px;" type="text" value="1"/>
Inlet Elevation	Elev IN = <input style="width: 100px;" type="text" value="100.6"/> ft
Outlet Elevation OR Slope	Elev OUT = <input style="width: 100px;" type="text" value="2"/> ft
Culvert Length	L = <input style="width: 100px;" type="text" value="30"/> ft
Manning's Roughness	n = <input style="width: 100px;" type="text" value="0.023"/>
Bend Loss Coefficient	k _b = <input style="width: 100px;" type="text" value="0"/>
Exit Loss Coefficient	k _x = <input style="width: 100px;" type="text" value="1"/>
Tailwater Surface Elevation	Elev Y _t = <input style="width: 100px;" type="text"/>
Max Allowable Channel Velocity	V = <input style="width: 100px;" type="text" value="5"/> ft/s
Required Protection (Output):	
Tailwater Surface Height	Y _t = <input style="width: 100px;" type="text" value="0.60"/> ft
Flow Area at Max Channel Velocity	A _t = <input style="width: 100px;" type="text" value="1.99"/> ft ²
Culvert Cross Sectional Area Available	A = <input style="width: 100px;" type="text" value="1.77"/> ft ²
Entrance Loss Coefficient	k _e = <input style="width: 100px;" type="text" value="0.20"/>
Friction Loss Coefficient	k _f = <input style="width: 100px;" type="text" value="1.70"/>
Sum of All Losses Coefficients	k _s = <input style="width: 100px;" type="text" value="2.90"/> ft
Culvert Normal Depth	Y _n = <input style="width: 100px;" type="text" value="0.31"/> ft
Culvert Critical Depth	Y _c = <input style="width: 100px;" type="text" value="1.22"/> ft
Tailwater Depth for Design	d = <input style="width: 100px;" type="text" value="1.36"/> ft
Adjusted Diameter OR Adjusted Rise	D _a = <input style="width: 100px;" type="text" value="0.90"/> ft
Expansion Factor	1/(2*tan(θ)) = <input style="width: 100px;" type="text" value="6.57"/>
Flow/Diameter ^{2.5} OR Flow/(Span * Rise ^{1.5})	Q/D ^{2.5} = <input style="width: 100px;" type="text" value="3.61"/> ft ^{0.5} /s
Froude Number	Fr = <input style="width: 100px;" type="text" value="14.48"/> Supercritical!
Tailwater/Adjusted Diameter OR Tailwater/Adjusted Rise	Y _t /D = <input style="width: 100px;" type="text" value="0.66"/>
Inlet Control Headwater	HW _i = <input style="width: 100px;" type="text" value="0.87"/> ft
Outlet Control Headwater	HW _o = <input style="width: 100px;" type="text" value="-95.82"/>
Design Headwater Elevation	HW = <input style="width: 100px;" type="text" value="101.47"/> ft
Headwater/Diameter OR Headwater/Rise Ratio	HW/D = <input style="width: 100px;" type="text" value="0.58"/>
Minimum Theoretical Riprap Size	d ₅₀ = <input style="width: 100px;" type="text" value="5"/> in
Nominal Riprap Size	d ₅₀ = <input style="width: 100px;" type="text" value="6"/> in
UDFCD Riprap Type	Type = <input style="width: 100px;" type="text" value="VL"/>
Length of Protection	L_p = <input style="width: 100px;" type="text" value="12"/> ft
Width of Protection	T = <input style="width: 100px;" type="text" value="4"/> ft

Culvert Report

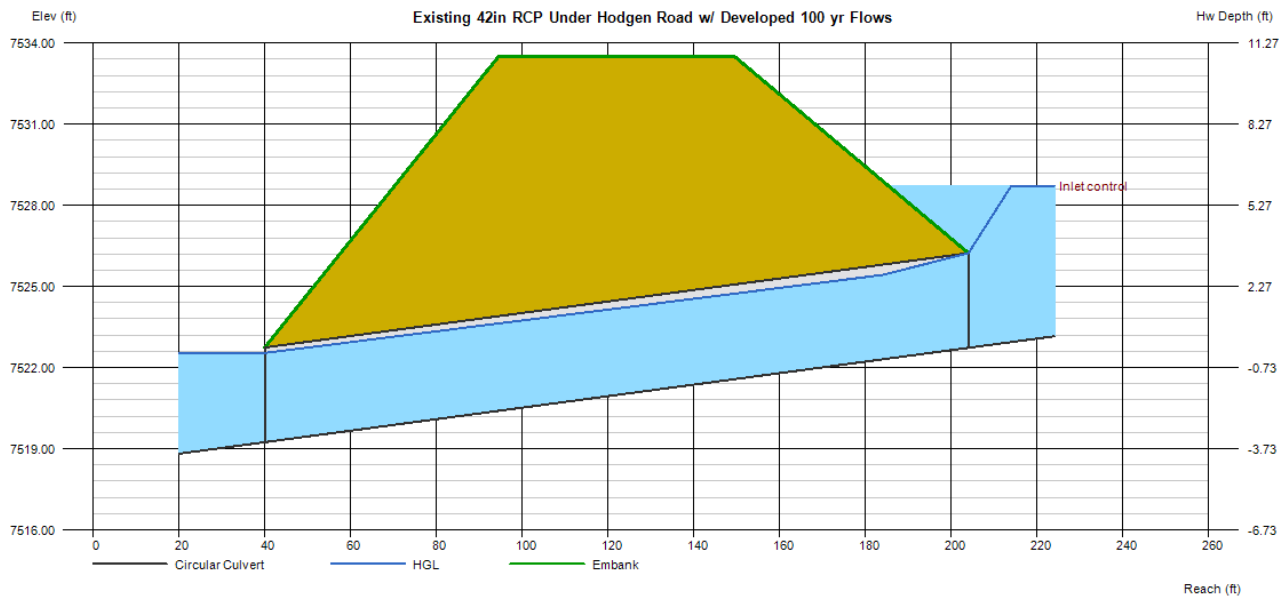
Existing 42in RCP Under Hodgen Road w/ Developed 100 yr Flows

Invert Elev Dn (ft)	= 7519.24
Pipe Length (ft)	= 164.00
Slope (%)	= 2.13
Invert Elev Up (ft)	= 7522.73
Rise (in)	= 42.0
Shape	= Circular
Span (in)	= 42.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Groove end projecting (C)
Coeff. K,M,c,Y,k	= 0.0045, 2, 0.0317, 0.69, 0.2

Embankment	
Top Elevation (ft)	= 7533.50
Top Width (ft)	= 55.00
Crest Width (ft)	= 0.00

Calculations	
Qmin (cfs)	= 102.70
Qmax (cfs)	= 102.70
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 102.70
Qpipe (cfs)	= 102.70
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 10.93
Veloc Up (ft/s)	= 11.41
HGL Dn (ft)	= 7522.54
HGL Up (ft)	= 7525.83
Hw Elev (ft)	= 7528.72
Hw/D (ft)	= 1.71
Flow Regime	= Inlet Control



Channel Report

Developed Basin C1 Channel 100 yr - 74.1 cfs

Trapezoidal

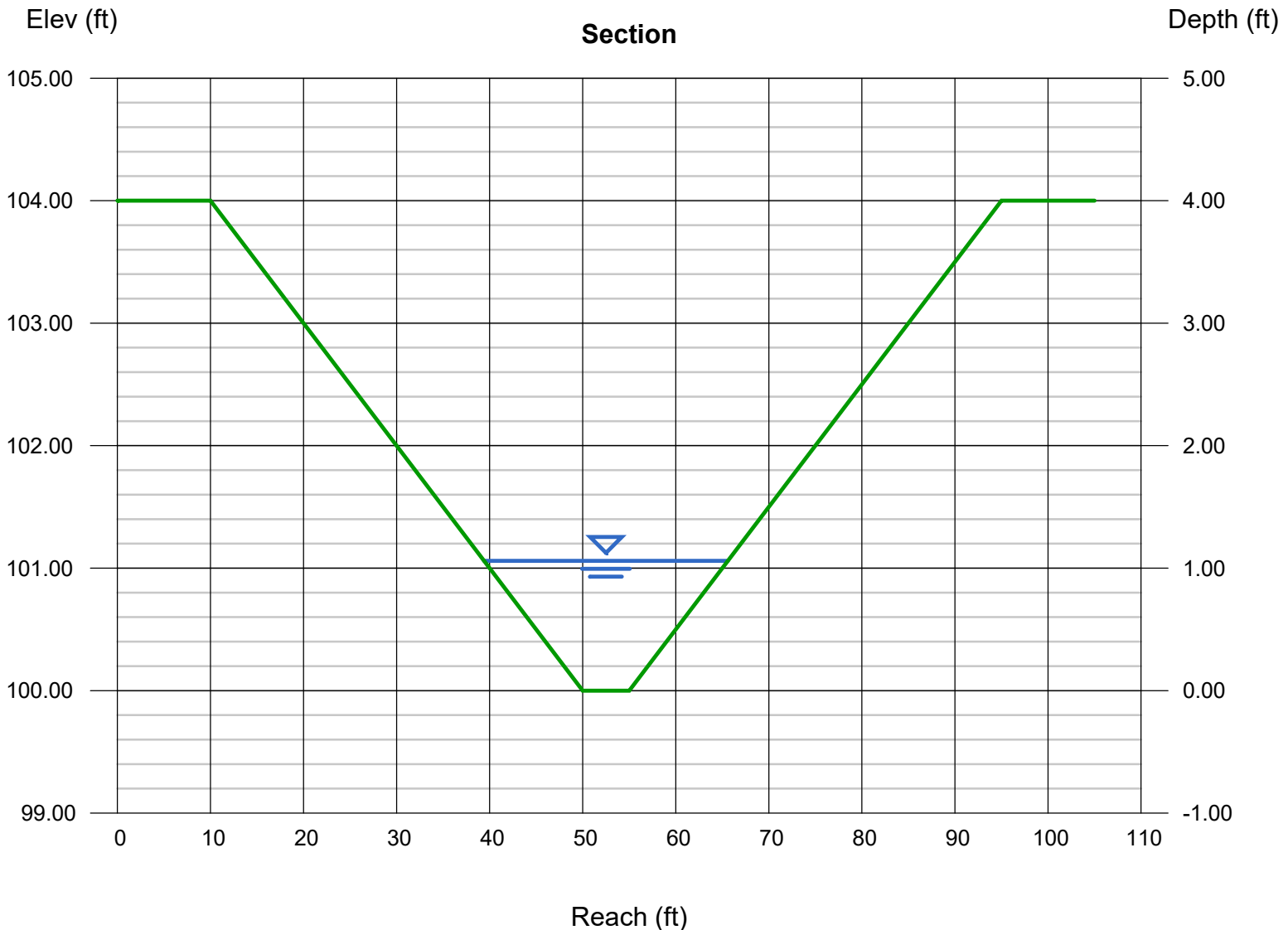
Bottom Width (ft)	= 5.00
Side Slopes (z:1)	= 10.00, 10.00
Total Depth (ft)	= 4.00
Invert Elev (ft)	= 100.00
Slope (%)	= 3.50
N-Value	= 0.045

Highlighted

Depth (ft)	= 1.06
Q (cfs)	= 74.10
Area (sqft)	= 16.54
Velocity (ft/s)	= 4.48
Wetted Perim (ft)	= 26.31
Crit Depth, Yc (ft)	= 1.06
Top Width (ft)	= 26.20
EGL (ft)	= 1.37

Calculations

Compute by:	Known Q
Known Q (cfs)	= 74.10



Channel Report

Downstream of DP3 100 yr - 74.1 cfs

Trapezoidal

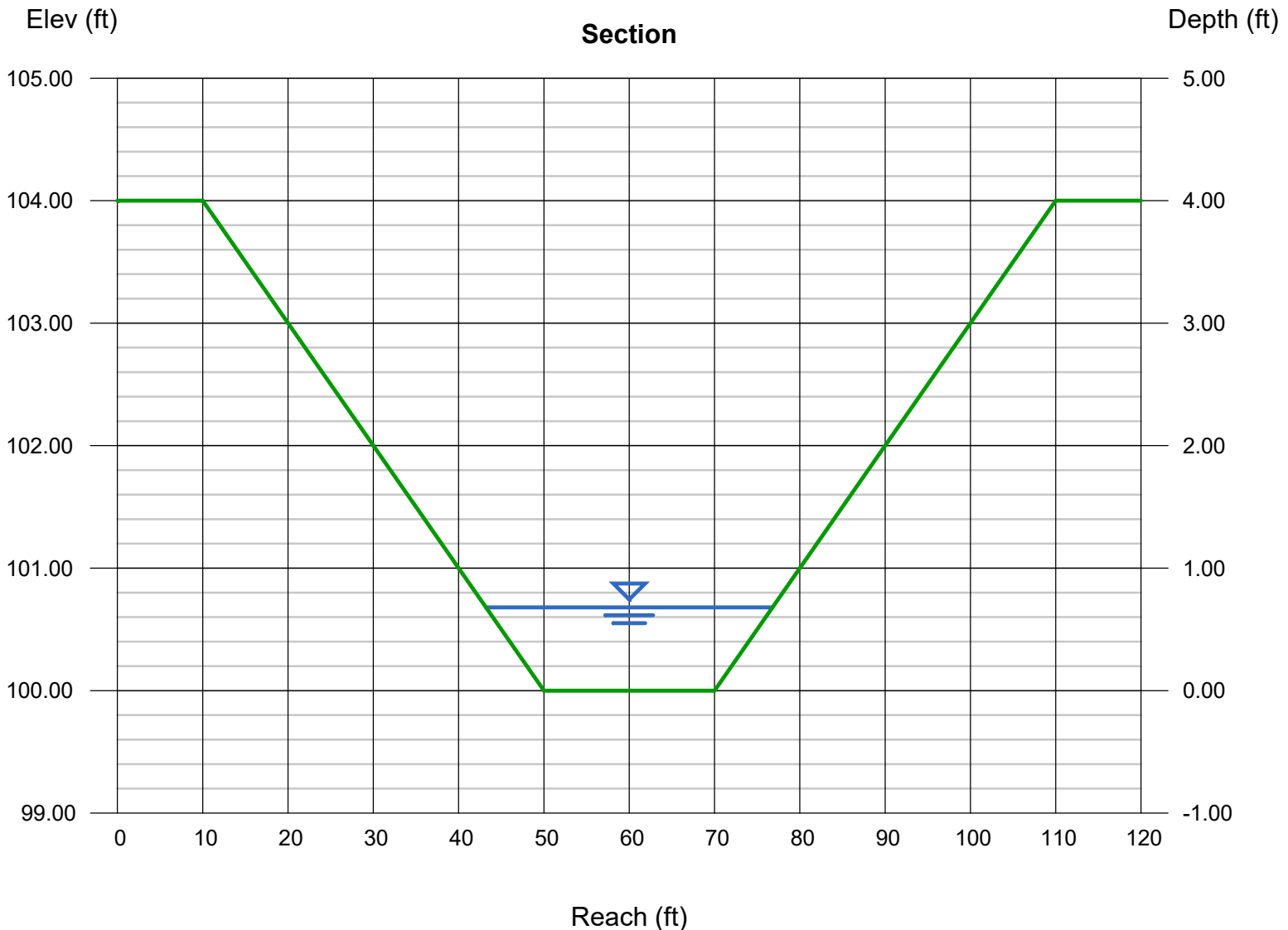
Bottom Width (ft)	= 20.00
Side Slopes (z:1)	= 10.00, 10.00
Total Depth (ft)	= 4.00
Invert Elev (ft)	= 100.00
Slope (%)	= 3.50
N-Value	= 0.045

Highlighted

Depth (ft)	= 0.68
Q (cfs)	= 74.10
Area (sqft)	= 18.22
Velocity (ft/s)	= 4.07
Wetted Perim (ft)	= 33.67
Crit Depth, Yc (ft)	= 0.67
Top Width (ft)	= 33.60
EGL (ft)	= 0.94

Calculations

Compute by:	Known Q
Known Q (cfs)	= 74.10



4 Report Maps

Existing Condition Hydraulic Analysis Map (Map Pocket)
Proposed Condition Hydraulic Analysis Map (Map Pocket)



LEGEND

PROPERTY LINE
 EASEMENT LINE
 LOT LINE

EXISTING

INDEX CONTOUR
 INTERMEDIATE CONTOUR

PROPOSED

INDEX CONTOUR
 INTERMEDIATE CONTOUR

BASIN BOUNDARY

FLOW AMOUNTS
 SLOPE DIRECTION AND GRADE

BASIN LABEL
 AREA IN ACRES
 PERCENT IMPERVIOUS

DESIGN POINT

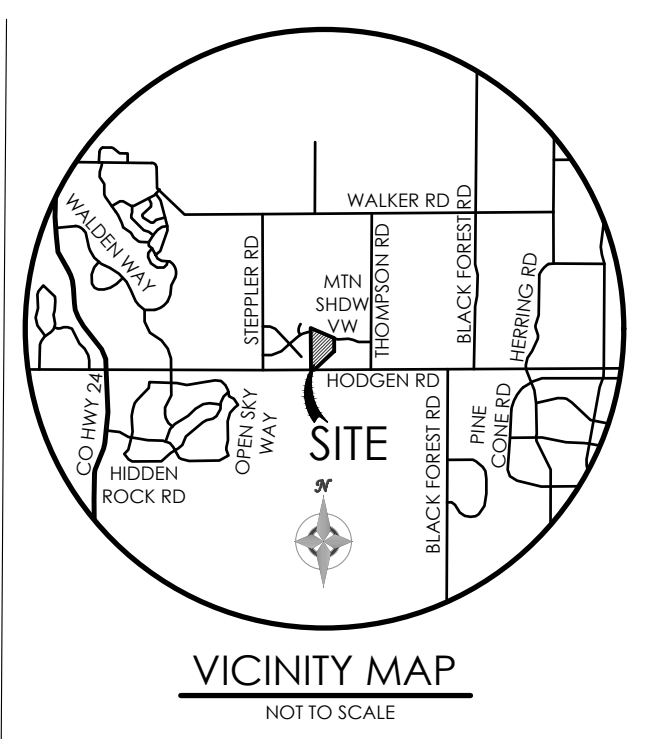
TIME OF CONCENTRATION
 FLOW DIRECTION

FLOODPLAIN STATEMENT

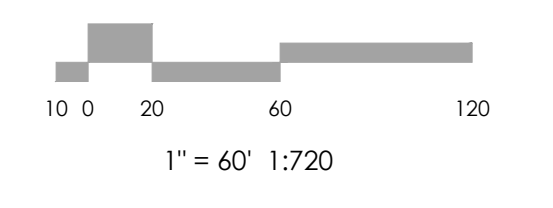
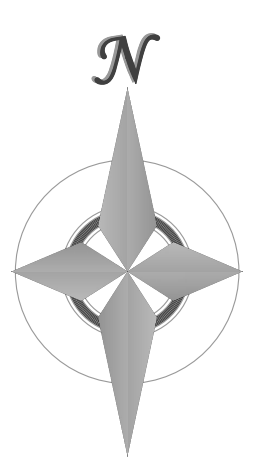
ACCORDING TO FEMA FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 08041CO285 G, DATED DECEMBER 7, 2018, THE PROPERTY IS LOCATED IN ZONE "X". (AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN).

EXISTING DRAINAGE SUMMARY TABLE

DESIGN POINTS	INCLUDED BASINS	AREA (AC)	Tc (MIN.)	RUNOFF		METHOD
				Q5 (CFS)	Q100 (CFS)	
	EX-A1	3.89	20.4	1.0	7.0	RATIONAL
DP1	LV DP8, EX-A1			7.3	21.7	RATIONAL
	OS-B1	1.22	18.8	0.3	2.3	RATIONAL
	B2	20.92	26.1	4.8	33.4	RATIONAL
DP2	OS-B1, B2	22.14	26.7	5.0	34.9	RATIONAL
	EX-C1	11.01	22.1	2.6	19.0	RATIONAL
DP3	LV DP10, EX-C1			24.7	72.3	RATIONAL



BENCHMARK



MVE, INC.
 ENGINEERS & SURVEYORS

1903 Idamay Street, Suite 200 Colorado Springs CO 80909 719.635.5736

REVISIONS

DESIGNED BY _____
 DRAWN BY _____
 CHECKED BY _____
 AS-BUILT BY _____
 CHECKED BY _____

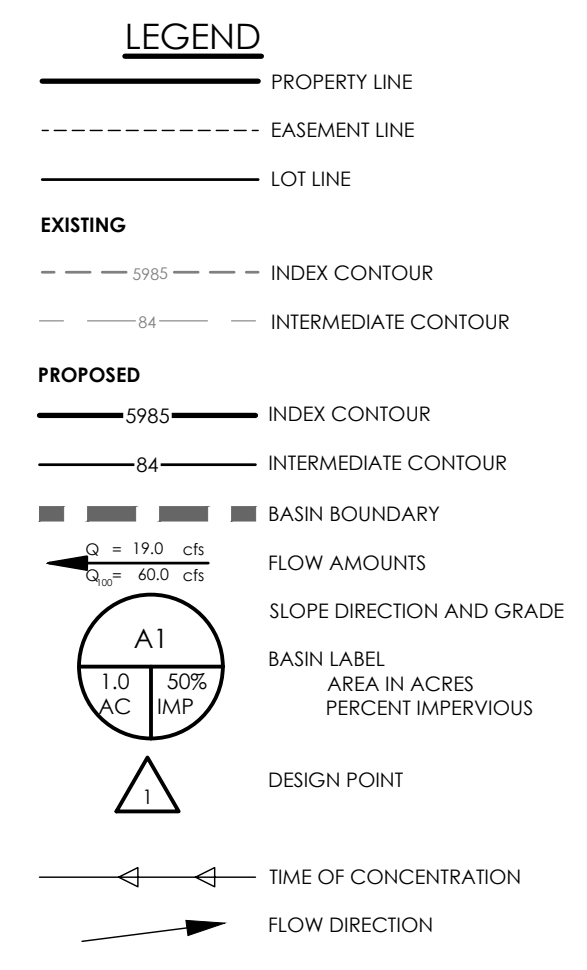
MIISFIT CREW
 ESTATES

DRAINAGE REPORT

EX. DRAINAGE MAP

MVE PROJECT 61160
 MVE DRAWING DRAIN-PP

MAY 21, 2024
SHEET 1 OF 1

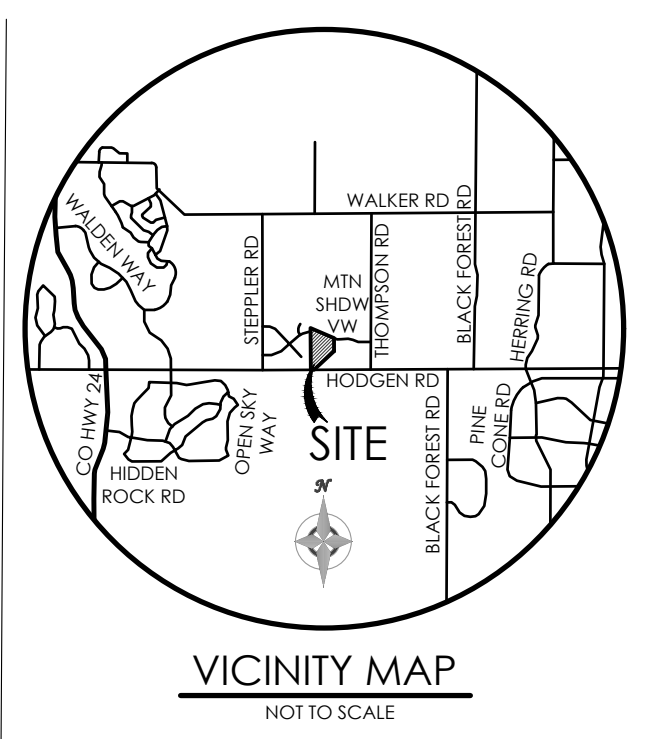


FLOODPLAIN STATEMENT

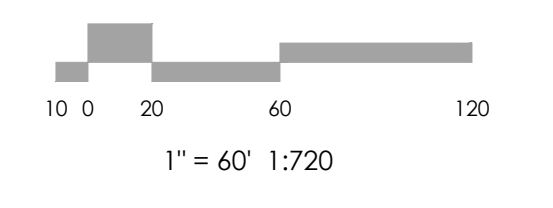
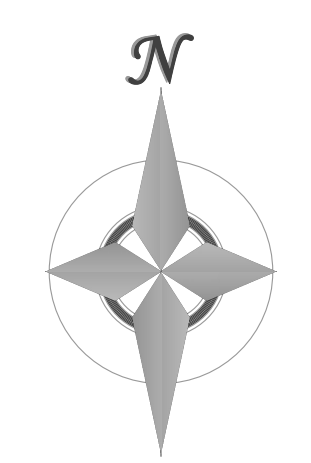
ACCORDING TO FEMA FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 08041CO285 G, DATED DECEMBER 7, 2018, THE PROPERTY IS LOCATED IN ZONE "X". (AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN).

PROPOSED DRAINAGE SUMMARY TABLE

DESIGN POINTS	INCLUDED BASINS	AREA (AC)	Tc (MIN.)	RUNOFF		METHOD
				Q5 (CFS)	Q100 (CFS)	
	EX-A1	3.89	20.4	1.0	7.0	RATIONAL
DP1	LV DP8, EX-A1			7.3	21.7	RATIONAL
	OS-B1	1.22	18.8	0.3	2.3	RATIONAL
	B2	20.92	26.1	4.8	33.4	RATIONAL
DP2	OS-B1, B2	22.14	26.7	5.0	34.9	RATIONAL
	PP-C1	11.01	21.8	3.3	20.8	RATIONAL
DP3	LV DP10, PP-C1			25.4	74.1	RATIONAL



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ENGINEERS & SURVEYORS

1903 Idamay Street, Suite 200 Colorado Springs CO 80909 719.635.5736

REVISIONS

DESIGNED BY _____
DRAWN BY _____
CHECKED BY _____
AS-BUILTS BY _____
CHECKED BY _____

MIISFIT CREW
ESTATES

DRAINAGE REPORT

PROP. DRAINAGE MAP

MVE PROJECT 61160
MVE DRAWING DRAIN-PP

MAY 21, 2024
SHEET 1 OF 1