OCTOBER 2020

Prepared For:

NES, INC.

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Prepared By:

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Please feel free to email/call me to set-up a meeting if you'd like to discuss any of the comments provided.

TNE Job No. 2015.00 County Job No. SKP-20-003

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APPENDIX

VICINITY MAP
S.C.S. SOILS MAP
FEMA FIRM MAP
HYDROLOGIC CALCULATIONS
PAGES FROM HISTORIC REPORTS
DRAINAGE MAP

CERTIFICATION STATEMENT:

Engineers Statement

This attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

master plan of the drainage basin. I accepacts, errors or omissions on my part in pro	t responsibility for any liability caused by any negligen eparing this report.
Dane Frank, P.E. 50207	Seal
Developers Statements I, Phillip Buford and Mary Jean Berg Buf the requirements specified in this drainag	ord, the developer have read and will comply with all or e report and plan.
Phillip Buford and Mary Jean Berg Bufor Business Name	<u>d</u>
By: Title: Address:	
El Paso County Approval:	
Filed in accordance with the requirements County Engineering Criteria Manual and	of the Drainage Criteria Manual, Volumes 1 & 2, El Paso Land Development Code as amended.
Jennifer Irvine, County Engineer / ECM Administrator	Date
Conditions:	

PURPOSE

The purpose of this Master Development Drainage Plan (MDDP) is to identify major drainageways; ponding/detention areas; locations of culverts, bridges, and open channels; and drainage areas which are tributary to the proposed development.

DBPS

The site lies within the Bennett Ranch Drainage Basin and is covered by the Bennett Ranch Pilot Project Drainage Basin Planning Study, dated November, 2001.

GENERAL DESCRIPTION

This MDDP for "LOT 1177 WOODMEN HILLS FILING #10" is an analysis of approximately 31.29 acres located in Section 32, Township 12 South, Range 64 West of the Sixth Principal Meridian, City of Colorado Springs, CO. The site is at 8507 Eastonville Road, on the east corner of the intersection of Judge Orr Road and Eastonville Road. This lot is planned for commercial development.

The site is bounded on the west by Eastonville Road, on the south by Judge Orr Road, on the east by an unplatted parcel (undeveloped), and on the north by Lot 1179 Woodmen Hills Filing # 10 (residential).

The site has previously been studied in the following reports:

- "Master Development Drainage Plan, Bennett Ranch, El Paso County, Colorado", by URS, dated August 11, 2000
- "Preliminary Drainage & Erosion Control Report For Woodmen Hills Subdivision Filing 10, El Paso County, Colorado", by URS, dated September 6, 2000
- "Final Drainage & Erosion Control Report For Woodmen Hills Subdivision Filing 10, El Paso County, Colorado", by URS, dated April 3, 2001

The August 11, 2000 MDDP shows the area of the site as a portion of a basin, with little else in details. The September 6, 2000 PDR shows the area of the site as a portion of a basin with a pond on the site. The April 3, 2001 FDR shows the area of the site as a portion of a basin, with a pond on the site, and four storm pipes discharging into the swale on the west side of the site.

Soils in the study area are shown as mapped by the S.C.S. in the "Soils Survey of El Paso County Area" (see appendix). Soils for this project are 100% Columbine gravelly sandy loam 19 (HSG A).

The site lies within the Bennett Ranch Drainage Basin and runoff ultimately flows into the West Fork of Squirrel Creek.

The study area consists of undeveloped land that has existing vegetation consisting of established native grasses and some shrubs/trees in the existing drainage channels. The site drains from north to south overland, with drainage channels on three sides, and into the Bennett Ranch Drainageway with average slopes of \sim 4%.

Please indicate in the

narrative what these combined flows are.

EXISTING DRAINAGE CONDITIONS

There are three existing offsite basins that surface drain onto the site, and the site itself is composed of three basins that drains from north to south. There are also four storm sewers that discharge into the swale on the west side of the site. The Bennett Ranch Drainageway runs along the east side of the site, and a swale runs along the west and south sides of the site. The combined flows from the site, drainage channel, and swale leave the site at the southeast corner.

Offsite Basin OS-Z's 0.48 acres consists of a landscaping area on an adjacent developed parcel that flows onto the site. Runoff ($Q_5 = 0.4$ cfs, $Q_{100} = 1.5$ cfs) channel flows southwest into the swale on Basin EX-A.

Offsite Basin OS-Y's 3.84 acres consists of residential land (single house). Runoff ($Q_5 = 2.8$ cfs, $Q_{100} = 10.7$ cfs) sheet/channel flows south onto Basin EX-B and then continues south overland and along a path that loosely resembles a swale.

Offsite Basin OS-X's 0.93 acres consists of a strip of land between the curb and the property line.

Please provide excerpts of the portions of this report that describes the flow that is conveyed to these 4 storm sewers on this page. Otherwise describe in Runoff $(Q_5 = 0 \not A \text{ cfs}, Q_{100} = 2.9 \text{ cfs})$ sheet flows east or north into the swale in Paragraph and that are impacting your site.

follows the channel south or east.

There are four storm sewers that discharge into the swale on the west side of the site and Basin EX-These storm sewer networks and the basins they drain are shown and described in the maps/pages from the Woodmen Hills Subdivision Filing 10 FDR.

Runoff ($Q_5 = 0.8 \text{ cfs}$, $Q_{100} = 4.8 \text{ cfs}$) from Basin EX-A's 6.30 acres sheet flows into a swale and then channel flows along the west and south sides of the site. Design Point A is located near the southeast corner of the site where the swale flows into the Bennett Ranch Drainageway.

Runoff ($Q_5 = 5.2$ cfs, $Q_{100} = 34.1$ cfs) from Basin EX-B's 21.1 acres sheet flows south across undeveloped land and into the swale in Basin EX-A. Design Point B is located in the southeast corner of the basin.

Runoff ($Q_5 = 0.7$ cfs, $Q_{100} = 4.5$ cfs) from Basin EX-C's 3.91 acres is the portion of Bennett Ranch Drainageway on the site. The drainageway flows from north to south on the eastern edge of the site. Design Point C is located at the southeast corner of the site where the drainageway leaves the site and passes under Judge Orr Road in a culvert.

The Bennett Ranch Drainageway flowlines extends at least 3.4 miles upstream from the site, with multiple forks. The tributary area is primarily residential subdivision and golf course land. There appear to be multiple ponds along the flowlines, with the nearest pond approximately 0.3 mile upstream of the site. The flows entering the site in the Bennett Ranch Drainageway are roughly $Q_5 =$ 420 cfs and $Q_{100} = 1,860$ cfs per the Bennett Ranch DBPS. The portion of the drainageway near the site is vegetated (grasses mostly) and roughly trapezoidal. There are riprap structures and some areas are covered with riprap (such as at culvert ends). Water from the drainageway eventually reaches the Arkansas River, with two ponds approximately one mile downstream of the site.

Wetlands have been identified in the drainage channels on three sides of the site. The areas are shown on the drainage maps (see appendix).

Please identify whether the installed channel meets the requirements laid out in the DBPS. Also please identify what DBPS improvements have been installed and if what is installed is different, please verify whether it is adequate/sufficient and any changes/improvements that are anticipated to be needed.

culverts at the Judge Orr Road crossing. The proposed channel changes in the DBPS include a new channel cross section and check structures (roughly four on the site). The DBPS shows new check structures every 234 feet, which is a different spacing than the check structures currently built. Based on the existing riprap check dams in the channel, the new channel has been in place since at least 2006. The Judge Orr Road culvert crossing was improved between 2017 and 2019, with the new culvert being double 12'x8' concrete box culverts. Based on this, the DBPS channel improvements on or adjacent to the site appear to have been previously completed. It is not known if all of the recommended improvements in the DBPS were installed.

Please identify if the installed box culvert is what was indicated in the DBPS

PROPOSED DRAINAGE CONDITIONS

The site is planned for commercial development. In the proposed condition the swale and drainageway onsite will remain largely unchanged, and the central portion of the site will be developed. There are currently no specific plans for development of the site. A possible layout for interior roads has been shown on the proposed drainage map, but this is only for a visual aid of how the site could be developed. A swale is proposed along the north property line to divert offsite runoff east to the drainageway, rather than having it flow across the entire site. A possible location and rough footprint for a full spectrum detention stormwater pond has also been shown near the southeast corner of the site. It is expected that the general runoff patterns for the developed site will continue to direct runoff to the southeast corner of the site.

Offsite Basin OS-Z's 0.48 acres consists of a landscaping area on an adjacent developed parcel that flows onto the site. Runoff ($Q_5 = 0.4$ cfs, $Q_{100} = 1.5$ cfs) channel flows southwest into the swale on Basin EX-A.

Offsite Basin OS-Y's 3.84 acres consists of residential land (single house). Runoff ($Q_5 = 2.8$ cfs, $Q_{100} = 10.7$ cfs) sheet/channel flows south onto Basin PR-1 and will then be diverted east in a swale to the Bennett Ranch Drainageway. The diversion swale may require armoring in steep sections and energy dissipation at the outfall will likely be needed.

Offsite Basin OS-X's 0.93 acres consists of a strip of land between the curb and the property line. Runoff ($Q_5 = 0.4$ cfs, $Q_{100} = 2.9$ cfs) sheet flows east or north into the swale in Basin EX-A and then

Please elaborate on your discussion of the existing swale. What is the width of this swale, slope, V or trapezoidal, etc. Was this swale/channel designed/sized to convey these offsite flows and this sites flows?

follows the channel south or east. Also please provide the flows (Q) that this swale is receiving from offsite areas in your narrative and also provide the cumulative flow in this basin(basin EX-A).

Basin EX-A will remain the same in the proposed condition. Runoff ($Q_5 = 0.8$ cfs, $Q_{100} = 4.8$ cfs)

from Basin EX-A's 6/30 acres sheet flows into a swale and then channel flows along the west and

south sides of the site. Design Point A is located near the southeast corner of the site where the

Please provide discussion on this discrepancy. It appears swale flows into the Bennett Ranch Drainageway are indicating that the installed culvert does not have sufficient capacity. The EPC memo indicates a LOMR as the source of their flows. Is this flow more accurate than

The existing swale on the west and s

the DBPS. Please analyze/discuss in your narrative. vegetated and appears to be in good condition. Hydraulic analysis will be required to determine the

capacity of this swale, which will be included in the final drainage report determine which flow (Q) is accurate or the full analysis of this Q may be determined at the preliminary/final drainage report. If you choose the latter, please state in your

Basin EX-C will remain the same in the

narrative that this will be further analyzed with the from Basin EX-C's 3.91 acres is the prelim/final-drainage reportin Discussion regarding the

drainageway flows from north to south on the eastern edge of the site. Design Point C is located at

the southeast corner of the site where the drainageway leaves the site and passes under Judge Orr Road in a culvert. Per the County's May 2017 drainage memo, this culvert is designed to convey the future 100 year storm event ($Q_{100} = 1,073$ cfs). This is roughly 58% of the developed 100 year flow in the Bennett Ranch DBPS. This culvert was designed to convey the $Q_{100} = 1,073$ cfs with two feet of internal freeboard.

Please include "and detention"

Basin PR-1's 21.1 acres consists of the central portion of the site. A percent impervious of 95% was assumed for this basin in the developed condition. Runoff ($Q_5 = 89.2$ cfs, $Q_{100} \neq 177.3$ cfs) is expected to flow south to Design Point 1. It is likely a stormwater treatment facility will be constructed near Design Point 1.

The southeast corner of the site is the low point of the site, so it's the most likely location for an onsite stormwater treatment facility. Based on basin PR-1 runoff a stormwater pond was roughly sized to have a footprint of 70,000 square feet (shown as 200'x350' on the drainage map). This

assumes the entire basin will be treated at a single location.

Please discuss the anticipated outfall of the pond as concentrated flow will outfall to either of the drainageways. Does the existing box culvert meet the definition of a suitable The Bennett Ranch Drainageway currently receives runoif from the site and carries it south. There

are multiple existing ponds downstream of the site and the Bennett Ranch DBPS proposed many

such as? Please identify what has been installed.

downstream facilities (check structures and culverts primarily). Some of the proposed facilities have been installed, but it is not known if all of them have been installed. Since the County drainage criteria will require runoff leaving the site be treated to release at predevelopment levels, development of the site is not expected to add any additional flows to the drainageway.

Any new roads on the site are expected to cross the exis and clearly identify the anticipated drainage problems and solutions.

It is not clear what those problems are from the text above. Please elaborate and clearly identify the anticipated drainage problems and solutions. This can be summarized in your conclusion.

No drainage problems are anticipated for the proposed development, other than these discussed above.

The Bennett Ranch DBPS appears to assume a future land use for the site area of 4DU/AC and 40 % impervious (Figure 2-4).

Since the site is commercial with 95% impervious there may not be many venues for reducing the runoff but there are some that could assumed 95% imperviousnesse implemented such as maximizing the vegetated/landscape areas along roadways or throughout lots to minimize directly connected impervious areas, providing LID's etc. The intent is to In an effort to protect receividentifyegeneral ways forreduce-runoff in this commercial diverse impacts of urbanization" this site was analyzed in the following manner:

- 1. Reduce Runoff- As no details of the proposed development have been provided and the site plan has only possible development features shown, it is not known if/how runoff would be reduced.

 based on your analysis of the existing drainageways is it anticipated that stabilization would be needed? you may
- 2. Stabilize Drainageways—As no also discuss any anticipated development to the led and hydraulic calculations are not swale/drainageway in this section. Please revise this section accordingly.

 stabilization will be necessary for this development.
- 3. Provide Water Quality Capture Volume (WQCV)- A possible location for an extended detention basin has been shown on the proposed drainage map, that could provide WQCV. As no details of the proposed development have been provided, it is not currently known if this location will be used or what form of water quality treatment will be used.
- 4. Consider Need for Industrial and Commercial BMPs- The proposed development is for commercial land. As no details of the proposed development have been provided, it is not currently known if commercial BMPs will be warranted, or what form they could take.

Please remove the highlighted sentence. It is understood that details of the development are not known but a possible solution for the required permanent water quality for a development such as this is the EDB that you have mentioned. Identifying that and/or other methods for water quality will suffice.

Provide general examples of the commercial BMPs that may be needed for a commercial development with 95% impervious.

Connecting a new roadway may not raise objection but how will the floodplain be mitigated HYDROLOGIC CALCULATIO so that it is not a problem for the new roadway? Is it anticipated that the drainageway/swale will Hydrologic calculations were performed to contain the flood language Design Criteria

Manual - Volumes 1 & 2, latest edificiently having the floodplain on a lot would water limit the development of that lot. The floodplain runoff anticipated from design stor being within the pond location can create als.

problems. Discuss the problems the floodplain can cause to the development and provide solutions.

FLOODPLAIN STATEMENT

A portion of this site is within a designated F.E.M.A. floodplain, as determined by Flood Insurance Rate Map No. 08041C0544 G dated December 7, 2018 (see appendix). The floodplain is shown on the drainage maps.

such as?

A new road will very likely be built in the floodplain and there may be other development in the floodplain. The existing Judge Orr Road is already in the mapped floodplain, so connecting a new road to it in the floodplain is not expected to raise significant objections. regulatory/administrative requirements are expected. Other types of development in the floodplain could raise major issues, but these issues would depend on the details of what is done. Development in the floodplain may require a LOMR or CLOMR be prepared.

DRAINAGE FEES

the intent of DCM 4.2, is to identify anticipated drainage problems associated with the development

It is expected that the County will require drainage fees be paid when this site is platted for commercial use.

SUMMARY

The site is planned for commercial development. The concepts presented in this MDDP are preliminary in nature and will need to be refined in the future final drainage report(s). The existing drainageway that crosses the site already carries runoff from the site.

PREPARED BY:

TERRA NOVA ENGINEERING, INC.

Dane Frank, P.E. Project Engineer Jobs/2015.00/Drainage/201500 MDDP.doc

BIBLIOGRAPHY

"El Paso County Drainage Criteria Manual-Volumes 1 & 2", latest edition

"El Paso County Board Resolution No 15-042" (Adoption of Chapter 6 and Section 3.2.1 Chapter 13 of the City of Colorado Springs Drainage Criteria Manual dated May 2014, Hydrology and Full Spectrum Detention)

"Bennett Ranch Pilot Project Drainage Basin Planning Study", by Stormwater & Environmental Consultants, Inc., dated November, 2001

"Master Development Drainage Plan, Bennett Ranch, El Paso County, Colorado", by URS, dated August 11, 2000

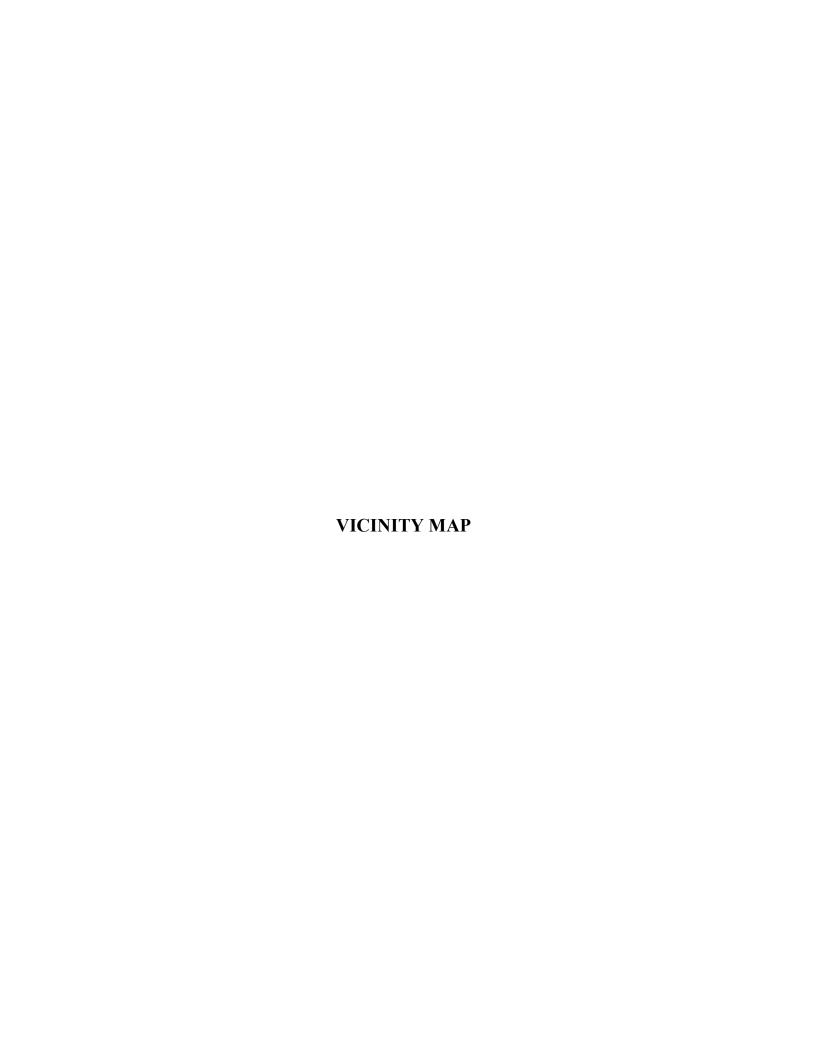
"Preliminary Drainage & Erosion Control Report For Woodmen Hills Subdivision Filing 10, El Paso County, Colorado", by URS, dated September 6, 2000

"Final Drainage & Erosion Control Report For Woodmen Hills Subdivision Filing 10, El Paso County, Colorado", by URS, dated April 3, 2001

"Memo: Judge Orr Road Culvert at Bennett Ranch Creek – Drainage, CDOT Project #: ER C040-047, SA 21422, El Paso County Project #: 152626", by El Paso County Department of Public Works, dated May 2017

SCS Soils Map for El Paso County

FEMA Floodplain Map



El Paso County - Community: Property Search

Schedule Number: 4232302003

8507 Eastonville Road Vicinity Map

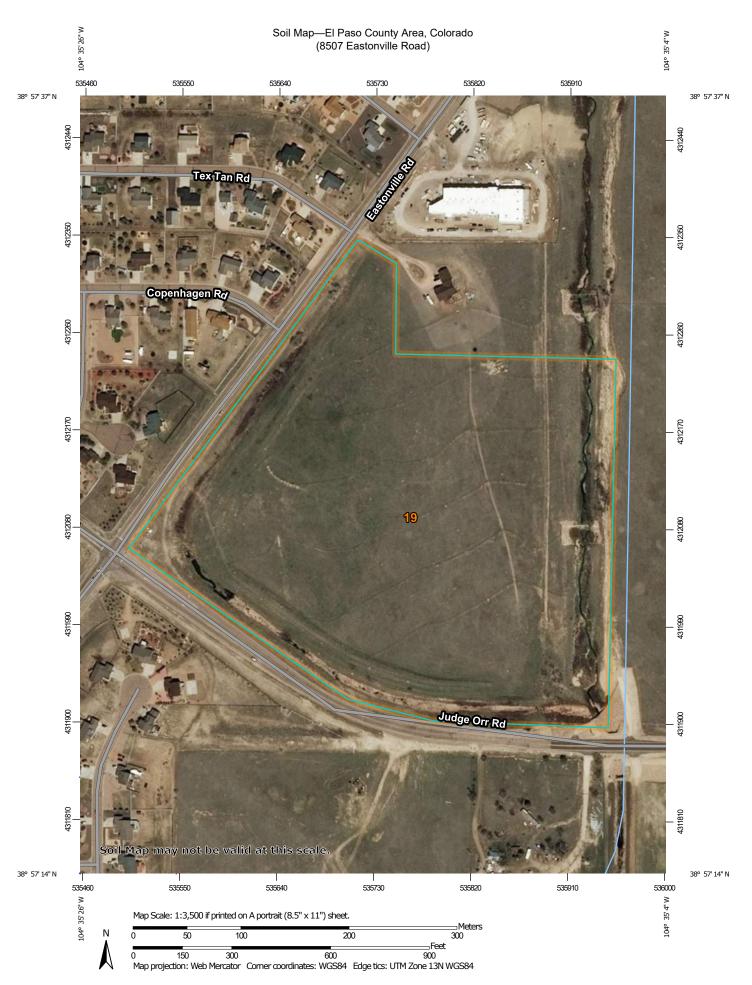


North is up ^^^

1 of 2







MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

CLIAD

Spoil Area

Stony Spot

Very Stony Spot

Wet Spot

Other

Special Line Features

Water Features

Δ

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 17, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI							
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	31.9	100.0%							
Totals for Area of Interest		31.9	100.0%							

El Paso County Area, Colorado

19—Columbine gravelly sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367p Elevation: 6,500 to 7,300 feet

Mean annual precipitation: 14 to 16 inches Mean annual air temperature: 46 to 50 degrees F

Frost-free period: 125 to 145 days

Farmland classification: Not prime farmland

Map Unit Composition

Columbine and similar soils: 97 percent

Minor components: 3 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Columbine

Setting

Landform: Flood plains, fan terraces, fans

Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium

Typical profile

A - 0 to 14 inches: gravelly sandy loam
C - 14 to 60 inches: very gravelly loamy sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High to

very high (5.95 to 19.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: A

Ecological site: Gravelly Foothill (R049BY214CO)

Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit: 1 percent

Landform: Swales Hydric soil rating: Yes

Other soils

Percent of map unit: 1 percent Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 17, Sep 13, 2019



NOTES TO USERS

is map is for use in administering the National Flood Insurance Program, to the season of the unces of small size. The community map repository should be consessible updated or additional flood hazard information.

To obtain more acticled information in areas where Sase Broot Beautions (SFE) and soften General pure when elementary as well seen for the control of the set of the control of the con

Coastal Base Flood Elevations shown on this map apply only landward of 0.0° North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal food elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or oodplain management purposes when they are higher than the elevations shown or his FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on \$6od control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTN) zone 13. The horizontal datum was NADOS, GR650 spherod, 1990 production of Filter for adjustment in the production of Filter for adjustment intelligent control and officences in map features across jurisdictions range years and effect the accuracy of the FiRM.

Flood developes on his map are referenced to the Morth American Vertical Date und 1988 (MANDSIS). These flood develorins must be compared to structure or ground elevations referenced to the same vertical datum. For information reparting conversion between the National Geodetic Vertical Datum of 1929 and the North http://www.nga.noias.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench mai shown on this map, please contact the Information Services Branch of the Natio Seodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by EI Pass County, Cotorado Springs Utilities, City of Fountain, Bureau of Land Management National Oceanic and Amospheric Administration, United States Geological Survey and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations hen those shown on the previous FRM for this jurisdiction. However, the previous FRM for this jurisdiction, the previous FRM for the production of the producti

porate limits shown on this map are based on the best data available at the time sublication. Because changes due to annexations or de-ennexations may have urred after this map was published, map users should contact appropriate invariety officials to workly current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a licitating of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMX) 1-477-396-2627 for information on available products associated with this product of the service of the

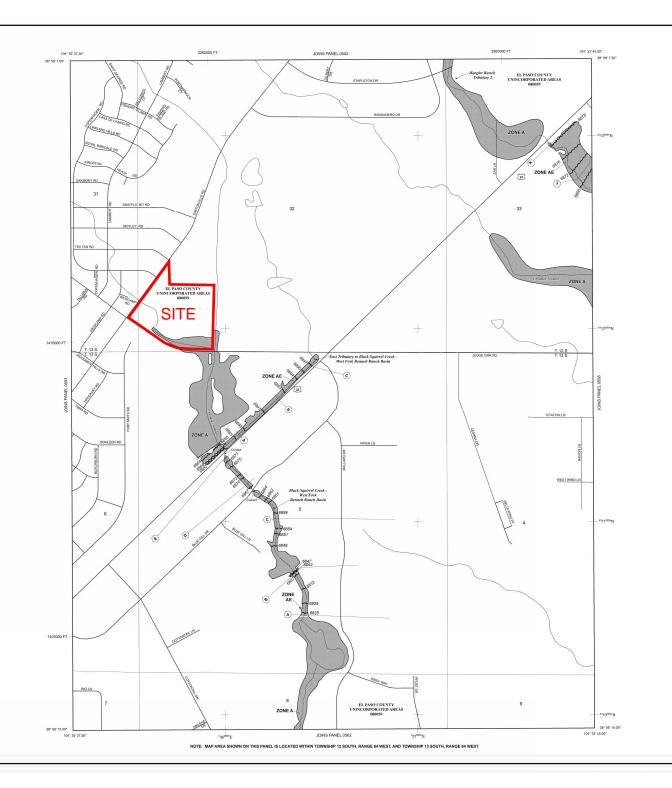
If you have questions about this map or questions concerning the National Floo insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2827) of sixt the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table Flooding Source REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEM).





LEGEND SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Heards Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Heard Richde Zones A, AE, AR, AQ, AR, AR9, V, and VE. The Base Flood Elevation is the water surface cleanation of the 1% annual chance flood. ZONE A No Base Flood Elevations determined. ZONE AE Base Flood Elevations determined. ZONE AF Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined. Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined. Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently described. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood. Coastal flood zone with velocity hazard (wave action); no Base Flood Bevolors determined. ZONE VE Coestal flood zone with velocity hazard (wave action); Base Flood Bevetions determined. FLOODWAY AREAS IN ZONE AE The floodway is the channel of a stream plus any adjacent floodplain areas that must be lept free of encreachment so that the 1% annual chance flood can be carried without unchannel increases in flood healths. Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood. OTHER AREAS Areas determined to be outside the 0.2% annual chance floodplain ZONE D Areas in which flood hazards are undetermined, but possible. COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS OTHERWISE PROTECTED AREAS (OPAs) Zone D Boundary CRRS and ORA houndary

ZONE V

////

ZONE X

ZONE X

~~ 513 ~~

M1.5

(A)—(A) 23----23 97° 07' 30.00° 32" 22" 30.00° Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)

Base Flood Elevation line and value; elevation in feet* Base Rood Elevation value where uniform within zone; elevation in feet*

rican Vertical Datum of 1988 (NAVD 88)

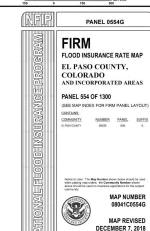
1000-meter Universal Transverse Mercator grid ticks, zone 13 4275000mgN 6000000 FT

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP MARCH 17, 1997

For community map revision history prior to countywide mapping, refer to the Com Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood insurance Program at 1-800-638-6520.







Federal Emergency Management Agency



LOT 1177 WOODMEN HILLS FILING #10 AREA RUNOFF COEFFICIENT (C) SUMMARY

EXISTING

		DEVELOPED			UNDEVELOPED			WEIG	GHTED	WEIGHTED CA	
BASIN	TOTAL AREA (Acres)	AREA (Acres)	C5	C100	AREA (Acres)	C5	C100	C5	C100	CA5	CA100
OS-Z	0.48	0.48	0.20	0.44	0.00	0.09	0.36	0.20	0.44	0.10	0.21
OS-Y	3.84	3.84	0.20	0.44	0.00	0.09	0.36	0.20	0.44	0.77	1.69
OS-X	0.93	0.93	0.09	0.36	0.00	0.09	0.36	0.09	0.36	0.08	0.33
EX-A	6.30	0.00	0.09	0.36	6.30	0.09	0.36	0.09	0.36	0.57	2.27
EX-B	21.1	0.00	0.09	0.36	21.10	0.09	0.36	0.09	0.36	1.90	7.60
EX-C	3.91	0.00	0.90	0.96	3.91	0.09	0.36	0.09	0.36	0.35	1.41

DEVELOPED

		DEVELOPED			UN	UNDEVELOPED			GHTED	WEIGHTED CA	
BASIN	TOTAL AREA	AREA	C5	C100	AREA	C5	C100	C5	C100	CA5	CA100
	(Acres)	(Acres)			(Acres)						
OS-Z	0.48	0.48	0.20	0.44	0.00	0.09	0.36	0.20	0.44	0.10	0.21
OS-Y	3.84	3.84	0.20	0.44	0.00	0.09	0.36	0.20	0.44	0.77	1.69
OS-X	0.93	0.93	0.09	0.36	0.00	0.09	0.36	0.09	0.36	0.08	0.33
EX-A	6.30	0.00	0.09	0.36	6.30	0.09	0.36	0.09	0.36	0.57	2.27
PR-1	21.1	21.10	0.81	0.88	0.00	0.09	0.36	0.81	0.88	17.09	18.57
EX-C	3.91	0.00	0.90	0.96	3.91	0.09	0.36	0.09	0.36	0.35	1.41

Calculated by: DLF
Date: 5/6/2020

Checked by:

LOT 1177 WOODMEN HILLS FILING #10 RUNOFF SUMMARY

EXISTING

		WEIGHTED		OVERLAND			STREET / CHANNEL FLOW			T_{C}	INTE	VSITY	TOTA	L FLOWS		
BASIN	AREA TOTAL	C_5	C ₁₀₀	C_5	Length	Slope	T_t	Length	Slope	Velocity	T_t	TOTAL	I_5	I ₁₀₀	\mathbf{Q}_{5}	\mathbf{Q}_{100}
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
OS-Z	0.48	0.20	0.44	0.20	100	0.04	10.3	0	4.0%	1.0	0.0	10.3	4.0	7.0	0.4	1.5
OS-Y	3.84	0.20	0.44	0.20	100	0.02	12.9	0	2.0%	0.7	0.0	12.9	3.7	6.3	2.8	10.7
OS-X	0.93	0.09	0.36	0.09	15	0.02	5.6	0	2.0%	0.7	0.0	5.6	4.9	8.7	0.4	2.9
EX-A	6.30	0.09	0.36	0.09	50	0.21	4.7	2300	1.0%	0.5	76.7	81.4	1.3	2.1	0.8	4.8
EX-B	21.10	0.09	0.36	0.09	300	0.02	25.1	0	2.0%	0.7	0.0	25.1	2.7	4.5	5.2	34.1
EX-C	3.91	0.09	0.36	0.09	75	0.08	8.0	1100	1.0%	0.5	36.7	44.6	2.0	3.2	0.7	4.5

DEVELOPED

			WEIGHTED		OVERLAND		STREET / CHANNEL FLOW			T_{C}	INTE	VSITY	TOTA	L FLOWS		
BASIN	AREA TOTAL	C_5	C ₁₀₀	C_5	Length	Slope	T_t	Length	Slope	Velocity	T_t	TOTAL	I_5	I_{100}	Q_5	Q_{100}
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
OS-Z	0.48	0.20	0.44	0.20	100	0.04	10.3	0	4.0%	1.0	0.0	10.3	4.0	7.0	0.4	1.5
OS-Y	3.84	0.20	0.44	0.20	100	0.02	12.9	0	2.0%	0.7	0.0	12.9	3.7	6.3	2.8	10.7
OS-X	0.93	0.09	0.36	0.09	15	0.02	5.6	0	2.0%	0.7	0.0	5.6	4.9	8.7	0.4	2.9
EX-A	6.30	0.09	0.36	0.09	50	0.21	4.7	2300	1.0%	0.5	76.7	81.4	1.3	2.1	0.8	4.8
PR-1	21.10	0.81	0.88	0.81	100	0.02	4.2	0	2.0%	0.7	0.0	4.2	5.2	9.5	89.2	177.3
EX-C	3.91	0.09	0.36	0.09	75	0.08	8.0	1100	1.0%	0.5	36.7	44.6	2.0	3.2	0.7	4.5

Calculated by: DLF
Date: 5/6/2020
Checked by:

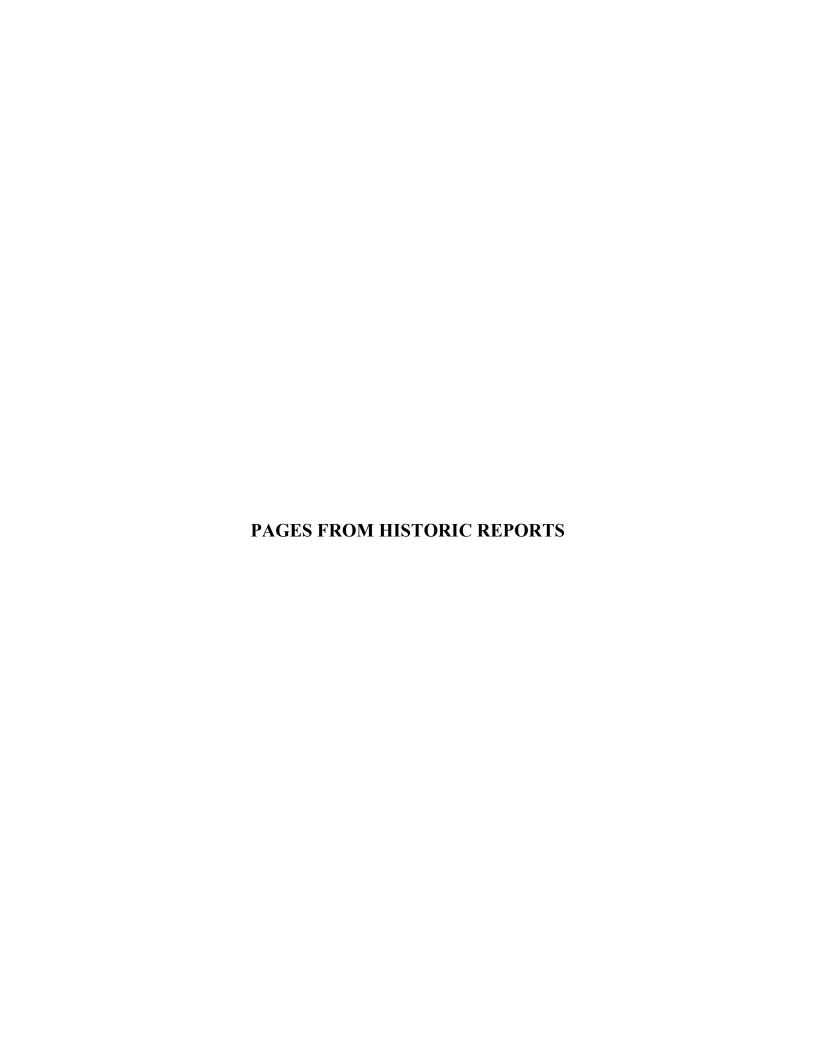
LOT 1177 WOODMEN HILLS FILING #10 SURFACE ROUTING SUMMARY

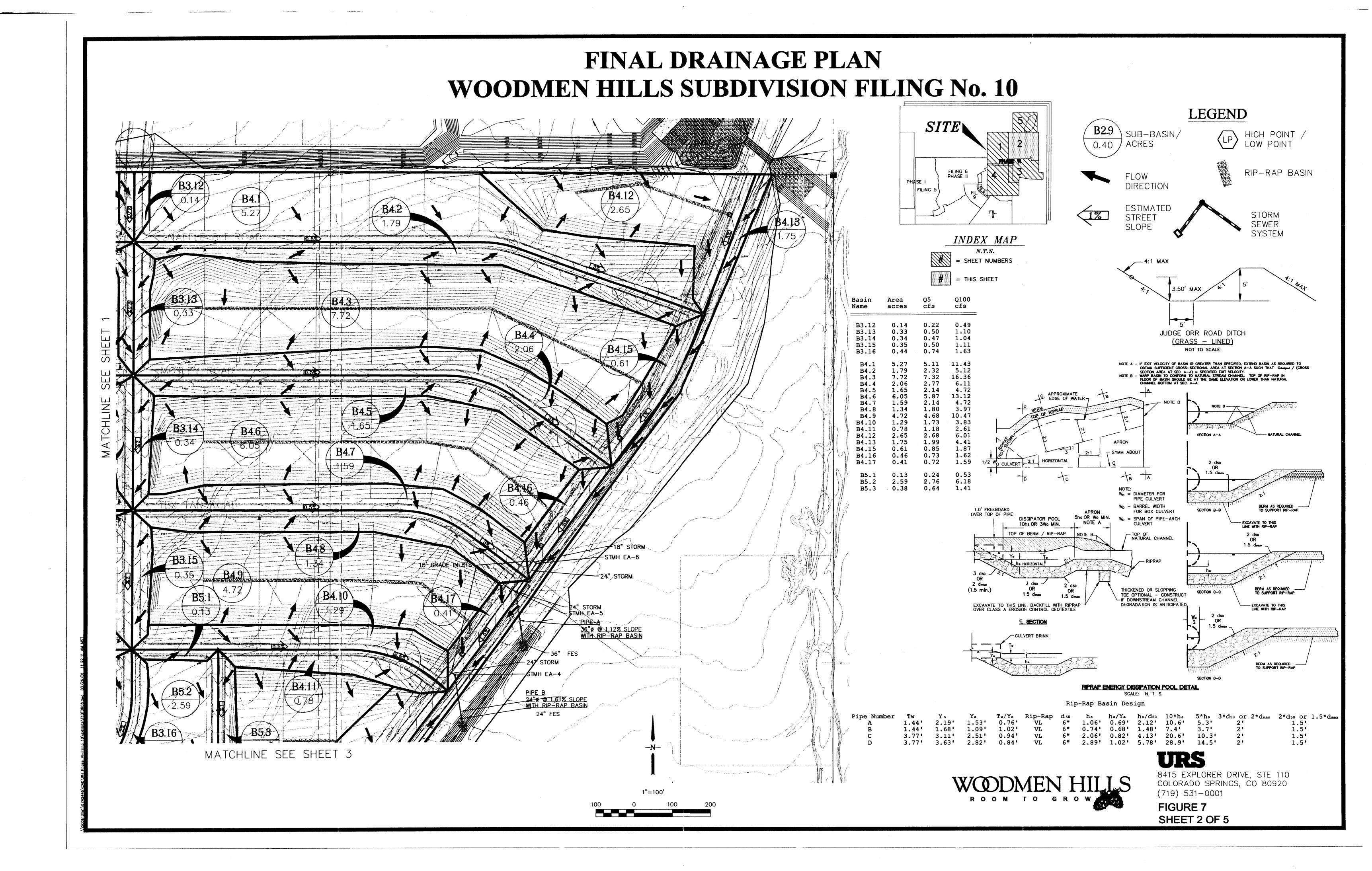
			Flor	w (cfs)
Design Point(s)	Contributing Basins	Area (ac)	Q 5	Q 100
Z	OS-Z	0.48	0.4	1.5
Y	OS-Y	3.84	2.8	10.7
X	OS-X	0.93	0.4	2.9
\boldsymbol{A}	OS-Z, OS-X, EX-A, PR-1	22.5	90.8	186.5
1	PR-1	21.1	89.2	177.3
C	ALL + Drainageway Flow	-	~514	~2,062

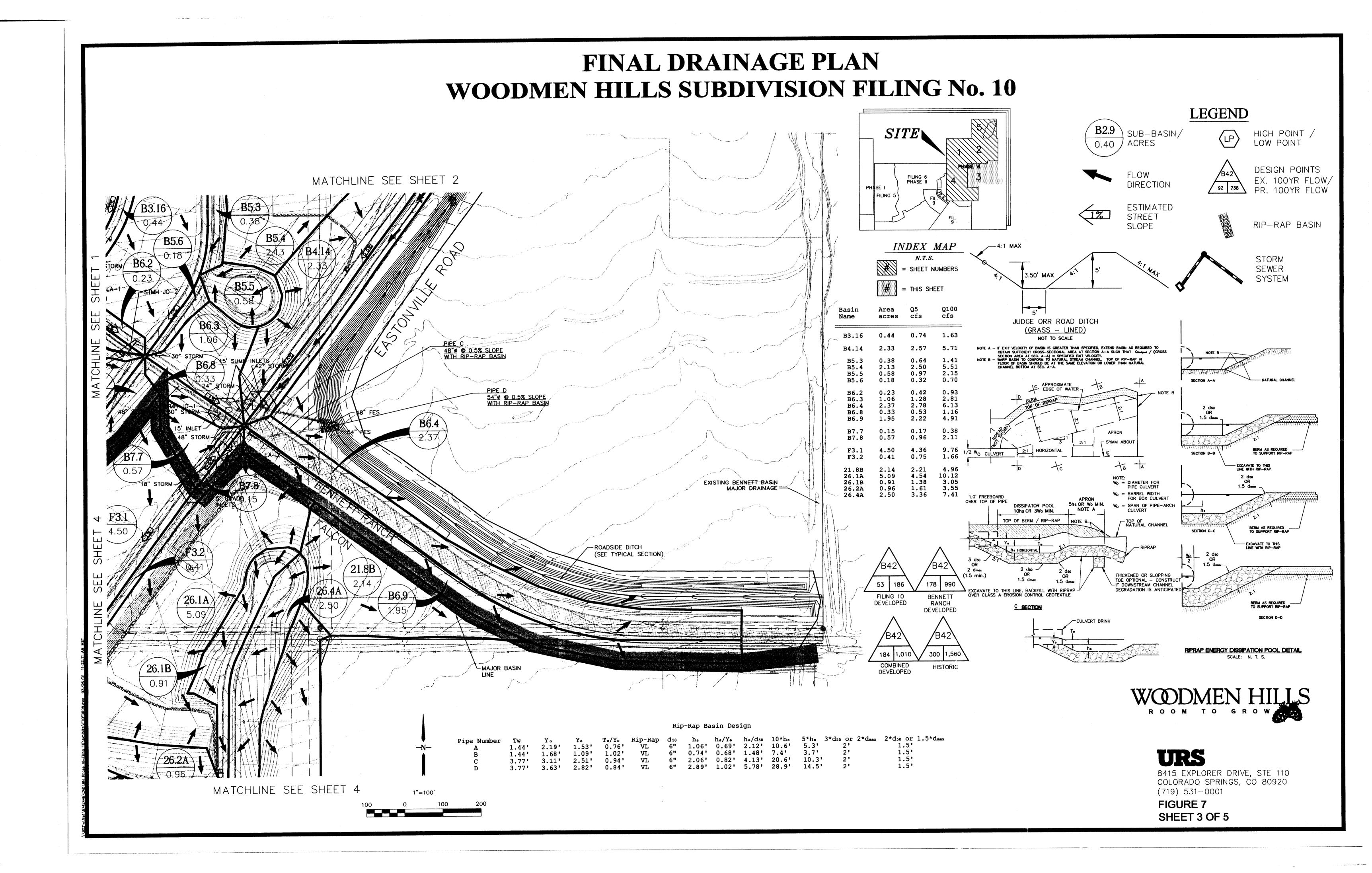
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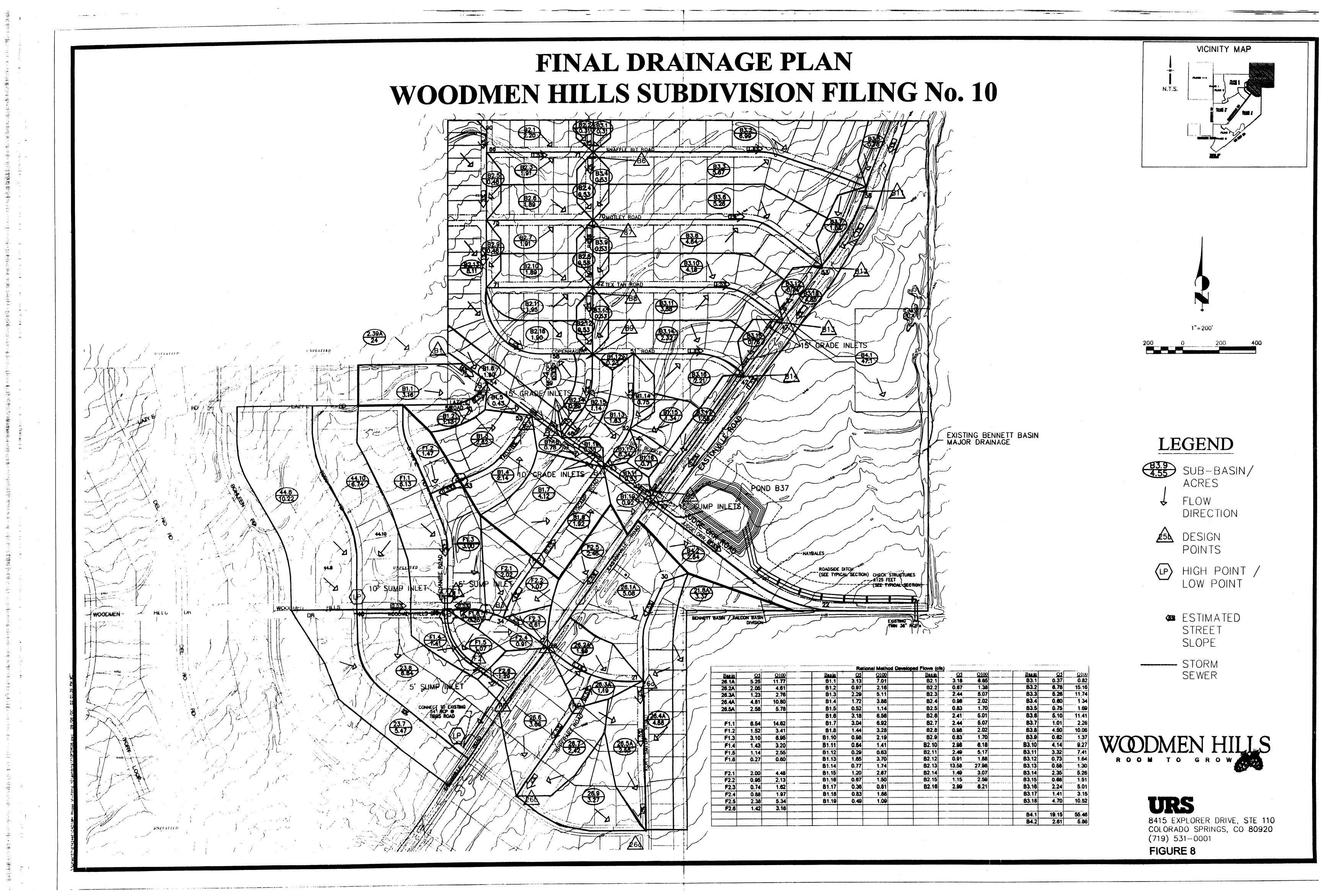
Date: _____5/6/2020

Checked by:











NOTES

1. A PORTION OF THE SITE IS WITHIN A 100 YEAR FEMA FLOOD PLAIN. 2. OFFSITE BASIN OS—X IS A STRIP OF LAND BETWEEN THE CURB AND PROPERTY LINE. THIS BASIN DRAINS INTO THE ADJACENT SWALE FOR ITS ENTIRE LENGTH.

LOT 1177 WOODMEN HILLS FILING #10

EL PASO COUNTY, CO

EXISTING DRAINAGE MAP

OCTOBER 2020

<u>DRAINAGE</u>	<u> SUMMAF</u>	<u> </u>

DECLON		, , , ,	FLOW				
DESIGN POINT	BASIN TRIBUTARY	AREA (ACRES)	5 YR (cfs)	100 YR (cfs)			
Z	OS-Z	0.48	0.4	1.5			
Y	OS-Y	3.84	2.8	10.7			
X	OS-X	0.93	0.4	2.9			
Α	EX-A	6.30	0.8	4.8			
В	EX-B	21.1	5.2	34.1			
С	EX-C	3.91	0.7	4.5			

LEGEND



BASIN BOUNDARY

ROAD AND DITCH FLOW DIRECTION GROUND SURFACE FLOW DIRECTION EXISTING CONTOURS - MINOR

EXISTING CONTOURS - MAJOR PROPOSED

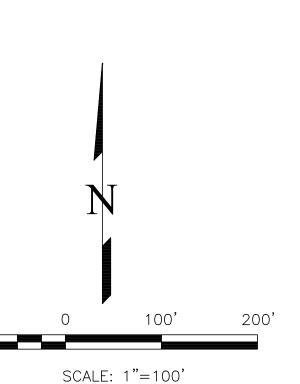
PR ΕX

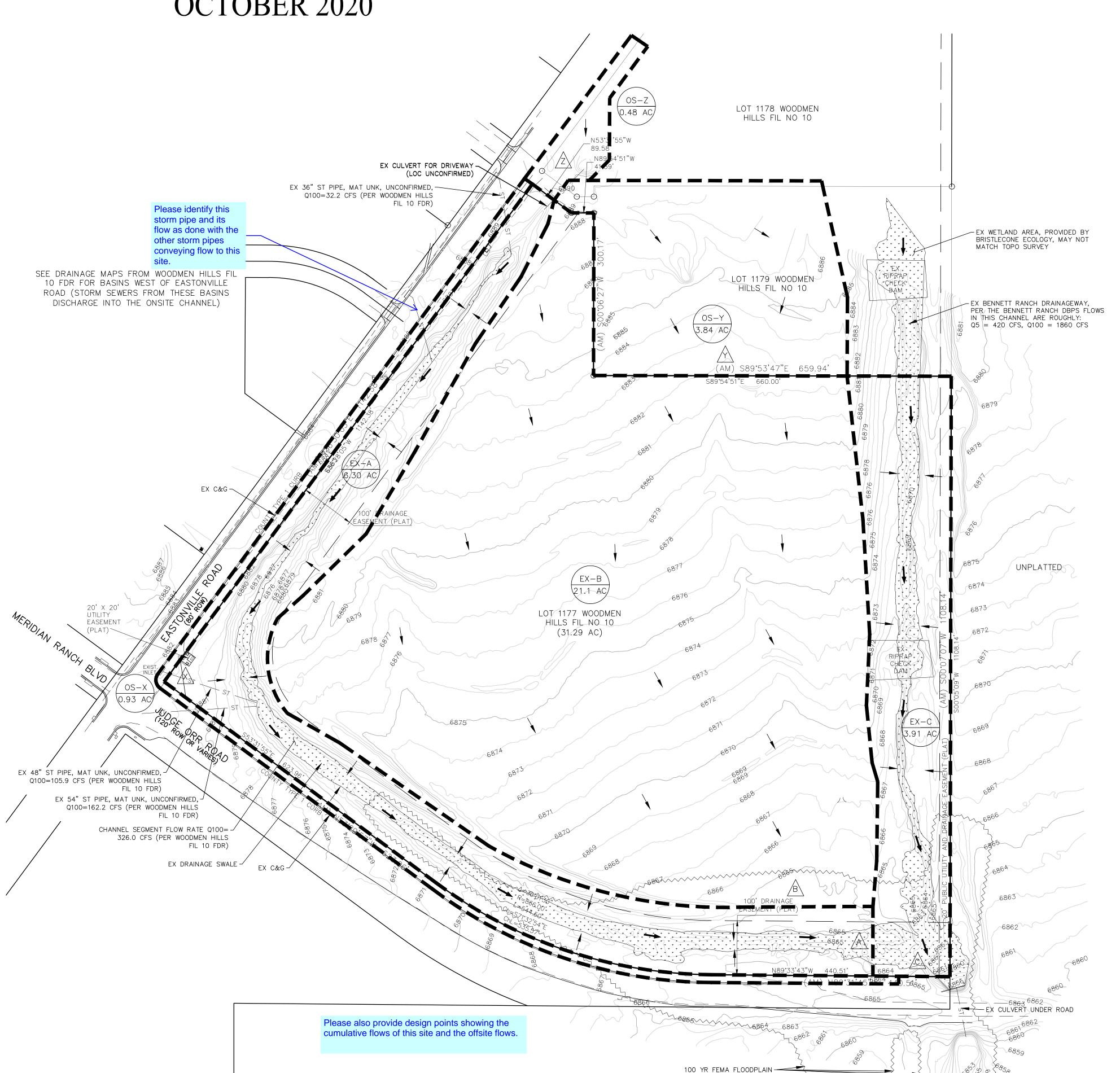
EXISTING

---- SETBACK LINE UNDERGROUND GAS LINE ----- UNDERGROUND ELECTRIC LINE ----- x ----- BARBED WIRE FENCE

— st — st — st — STORM SEWER LINE

SANITARY SEWER MANHOLE WATER VALVE FIRE HYDRANT





PLAT NAME

LOT 1177 WOODMEN HILLS FIL NO 10

ESIGNED BY DLF RAWN BY DLF

HECKED BY LD

-SCALE N/A

OB NO. 2015.00 ATE ISSUED 10/30/

-SCALE AS NOTEI

HEET NO. 1 OF

NOTES 1. A PORTION OF THE SITE IS WITHIN A 100 YEAR FEMA FLOOD PLAIN. 2. OFFSITE BASIN OS-X IS A STRIP OF LAND BETWEEN THE CURB AND PROPERTY LINE. THIS BASIN

DRAINS INTO THE ADJACENT SWALE FOR ITS ENTIRE LENGTH.

LOT 1177 WOODMEN HILLS FILING #10 EL PASO COUNTY, CO

PLAT NAME

LOT 1177 WOODMEN HILLS FIL NO 10

SIGNED BY DLF

RAWN BY DLF

HECKED BY LD

-SCALE N/A

DB NO. 1973.00

Is this your ultimate calculated

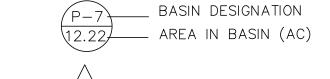
ATE ISSUED 10/30/

SCALE AS NOTEI

DRAINAGE SUMMARY

DECION	D.A. O.I.N.I.	4554	Fl	_OW
DESIGN POINT	BASIN TRIBUTARY	AREA (ACRES)	5 YR (cfs)	100 YR (cfs)
Z	OS-Z	0.48	0.4	1.5
Y	OS-Y	3.84	2.8	10.7
X	OS-X	0.93	0.4	2.9
А	EX-A	6.30	0.8	4.8
1	PR-1	21.1	89.2	177.3
С	EX-C	3.91	0.7	4.5





DESIGN POINT

BASIN BOUNDARY

ROAD AND DITCH FLOW DIRECTION GROUND SURFACE FLOW DIRECTION EXISTING CONTOURS - MINOR

EXISTING CONTOURS - MAJOR

ΕX

EXISTING

PROPOSED

— ug — UNDERGROUND GAS LINE ——— UNDERGROUND ELECTRIC LINE BARBED WIRE FENCE

— st — st — STORM SEWER LINE SANITARY SEWER MANHOLE

WATER VALVE

FIRE HYDRANT

