

# Final Drainage Report

# Ellicott School Addition - 2 Buildings

Project No. 61183



Add PCD File # PPR-22-50

### **Final Drainage Report**

for

**Ellicott School Addition - 2 Buildings** 

Project No. 61183

### September 1, 2022

prepared for

#### Ellicott School District No 22 322 S Ellicott Hwy

Calhan, CO 80808 719.683.2700

prepared by

MVE, Inc. 1903 Lelaray Street, Suite 200 Colorado Springs, CO 80909 719.635.5736

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61183-Final Drainage Report.odt



#### **Engineer's Statement**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

David R. Gorman, P.E. For and on Behalf of MVE, Inc. Colorado No. 31672

Date

#### **Developer's Statement**

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

School District Board of Education President, Ellicott School District No 22 Date

#### El Paso County

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Joshua Palmer, PE Interim County Engineer / ECM Administrator Date

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# Final Drainage Report

The purpose of this Final Drainage Report is to identify drainage patterns and quantities within and affecting the proposed Ellicott School Addition - 2 Buildings site. The report will "identify specific solutions to problems on-site and off-site resulting from the proposed project. The report and included maps present results of hydrologic and drainage facilities analyses. The report will discuss the recommended drainage improvements to the site and identify drainage requirements relative to the proposed project. This report has been prepared and submitted in accordance with the requirements of the El Paso County development approval process. An Appendix is included with this report with pertinent calculations and graphs used in the drainage analyses and design. The scope of this report does not include the entirety of the site but is limited to the regions of the site where grading and construction will be occurring.

#### **1** General Location and Description

#### 1.1 Location

The proposed Ellicott School Addition - 2 Buildings site is located within the southwest quarter of Section 18, Township 14 South, Range 62 West of the 6th Principal Meridian, El Paso County, Colorado. The 28.51± acre site is situated north of Handle Road and east of South Ellicott Highway in El Paso County. The site contains an elementary school building that uses the address of 399 S Ellicott Highway, Calhan, CO 80808 and El Paso County Assessor's Schedule Number 2418000019. A **Vicinity Map** is included in the **Appendix**.

The south edge of the site is adjacent to Handle Road, and to the south of Handle road is an unplatted lot containing 109.82 acres, is owned by Bailey Teresa D and is zoned Agricultural (A-35) with El Paso County Assessor's Schedule Number 2400000220. An unplatted lot containing 40.45 acres, is owned by Jerry R Sales and Kathy A Sales, zoned Agricultural (A-35) is adjacent to the east of the site with El Paso County Assessor's Schedule Number 2418000013.

The western edge of the site is adjacent to South Ellicott highway, and adjacent to the highway on the west are two unplatted lots owned by Ellicott School District No. 22 and zoned Agricultural (A-35). The northern parcel to the east has El Paso County Assessor's Schedule Number 3413000024, containing 29.3 acres, and the southern parcel has El Paso County Assessor's Schedule Number 3413000025, containing 10.7 acres.

An unplatted lot containing 30 acres, is owned by Schubert Beverly J Living Trust and zoned Agricultural (A-35) is adjacent to the north of the site with El Paso County Assessor's Schedule Number 2418000006. Unplatted lot, containing 5.35 acres, is owned by El Paso County School District No 22 is zoned Agricultural (A-35), is also adjacent to the north of the site with El Paso County Assessor's Schedule Number 2418000018.

A tributary to Black Squirrel Creek, flows northwest to southeast, approximately 1,200 feet southwest of the site. The site is located in El Paso County's Ellicott Consolidated Major Drainage Basin.

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#### 1.2 Description of Property

The Ellicott School Addition - 2 Buildings site is zoned Agricultural - 35 Acres (A-35). The property is the location of an elementary school with existing elementary school building, a superintendent office building, and various ancillary buildings as well as gravel drives, parking lots and grass covered ball fields. There are existing 12" CMP culverts below the driveways connecting the site to the roadway at 5 locations along the edge of the site.

The site is covered with native prairie grasses and weeds in average condition having approximately 80% ground coverage with sparse trees and shrubs scattered. The existing site topography slopes easterly and southerly towards Handle Road with grades that range from 1% to 10% with areas in the northern and eastern portions with slopes that reach 22%. The main access to this project is from the existing public South Ellicott Highway. There is one (1) access point directly in front of Ellicott Sr High School at 375 S Ellicott Hwy as well as another access directly in front of Ellicott Elementary School at 399 S Ellicott Hwy.

The lowest point on the site is in the southeastern portion of the site. No major drainageways flow through the site and no significant drainage improvements or drainage facilities currently exist on the site.

According to the National Resource Conservation Service, there are three (3) soil types in the immediate area of the Ellicott School Addition - 2 Buildings site. Columbine Gravelly Sandy Loam (map unit 19) makes up a portion of the soil in the eastern portion of the site and Truckton Loamy Sand (map unit 95) makes up a portion of the soil in the western portion. Truckton Sandy Loam (map unit 96) makes up a small portion of approximately 0.1 acres in the center portion of the site. A portion of the Soil Map and data tables from the **NRCS/USDA Web Soil Survey** and relevant Soil Descriptions from the **Soil Survey of El Paso County Area, Colorado** are included in the **Appendix**.<sup>1 2</sup>

Columbine Gravelly Sandy Loam is typically deep and well drained excessively drained. Permeability is very rapid, surface runoff is very low, and the hazard of erosion is slight to moderate. Columbine Gravelly Sandy Loam is classified as being part of Hydrologic Soil Group "A".

Truckton Loamy Sand is typically deep and well drained. Permeability is moderately rapid, surface runoff is low, and the hazard of erosion is moderate to high. Truckton Loamy Sand is classified as being part of Hydrologic Soil Group "A".

Truckton Sandy Loam is typically deep and well drained. Permeability is moderately rapid, surface runoff is very low, and the hazard of erosion is moderate. Truckton Sandy Loam is classified as being part of Hydrologic Soil Group "A".

The current Flood Insurance Study of the region includes Flood Insurance Rate Maps (FIRMs), effective December 7, 2018. The project site is included in Community Panel Number 08041C0809 G of the FIRMs for El Paso County, Colorado. No portion of the site lies within FEMA designated Special Flood Hazard Areas (SFHAs). An excerpt of the current **FEMA Flood Insurance Rate Maps** with the site delineated is included in the **Appendix**.<sup>3 4</sup>

#### **1.3 Description of Development**

The development on this site include site grading and construction of two (2) new buildings along with sidewalks, drives, and connected utilities. Construction will be done in two phases, Phase I and Phase II. Phase I will consist of constructing one (1) Votech building having two (2) stories and 11,499 sf building footprint with a gravel access driveway, paved sidewalks, and landscaping. Phase I will serve Ellicott Sr High School at 375 S Ellicott Hwy. Phase II will consist of one (1) classroom building having one (1) story and 19,123 sf building footprint containing 13 classrooms, paved

<sup>1</sup> WSS 2 SS-EPC

<sup>3</sup> FIS

<sup>4</sup> FIRM, Map No. 08041C0809G

sidewalks, and landscaping. Phase II will serve Ellicott Elementary School at 399 S Ellicott Hwy. The project also includes connecting internal potable water and sanitary sewer service lines.

#### 2 Drainage Basins and Sub-Basins

#### 2.1 Major Basin Descriptions

The Ellicott School Addition - 2 Buildings site is located in the Ellicott Consolidated Major Drainage Basin (CHBS1200). This basin drains to Black Squirrel Creek. The Ellicott School Addition - 2 Buildings site is located approximately 1,200 feet east of a tributary to Black Squirrel Creek, which flows into Black Squirrel Creek approximately 4 miles southeast of the site. The scope of this report does not include the entirety of the site but is limited to the regions of the site where grading and construction will be occurring. The site is not included in a Drainage Basin Planning Study.

#### 2.2 Sub-Basin Description

The scope of this report does not include the entirety of the site but is limited to the regions of the site where grading and construction will be occurring. The existing drainage patterns of said region can be described by five (5) on-site drainage basins. All Existing basins drain southerly offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek, which flows into Black Squirrel Creek approximately 4 miles southeast of the site. All of these basins are previously disturbed or developed to a degree as described below. All existing basin delineations and data are depicted on the attached **Existing Drainage Maps**.

#### 2.3 Existing Drainage Patterns

Existing sub-basin EX-A, located on the northern portion of the site, containing a gravel parking and pasture/meadow area, drains easterly towards an existing swale into the existing depression in the southwest edge of the basin, continues easterly onto the existing ball field, and continues southerly into Handle Road.

Existing sub-basin EX-B, located in the northern portion of the site, containing a paved asphalt drive, concrete pavement and meadow/pasture area. This basin drain easterly towards the existing ball field, and continues southerly into Handle Road.

Existing sub-basin EX-C, , located in the northern portion of the site is comprised of meadow/pasture area. The sub-basin drains easterly towards the existing ball field and continues southerly into Handle Road.

Existing sub-basin EX-D, located in the southern portion of the site, contains meadow/pasture area. Flows from this basin drain westerly towards Ellicott Highway and continues southerly and offsite into Handle Road.

Existing sub-basin EX-E, located in the southern portion of the site and containing a small area of concrete pavement and meadow/pasture area, drains easterly towards the ball fields in the southern portion on the site and continues southerly and offsite into Handle Road.

#### 3 Drainage Design Criteria

roadside ditch?

#### 3.1 Development Criteria Reference

This Final Drainage Report for Ellicott School Addition - 2 Buildings has been prepared according to the report guidelines presented in the latest edition of *El Paso County Drainage Criteria Manual*<sup>5</sup>. The County has also adopted portions of the City of Colorado Springs Drainage Criteria Manual Volumes 1 and 2, especially concerning the calculation of rainfall runoff flow rates.<sup>6 7</sup> The hydrologic

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Provide discussion of previous drainage studies (Name and date) for the complete site in that influence or are influenced by the drainage design and how the studies affect drainage design for the site.

<sup>5</sup> DCM Section I, Chapter 4.3 and 4.4

<sup>6</sup> CS DCM Vol 1 7 CS DCM Vol 2

<sup>7</sup> CS DCM Vol

analysis is based on a collection of data from the DCM, the NRCS/USDA Web Soil Survey<sup>8</sup>, Existing topographic data by Clark Land Surveying Inc., and proposed site plan by Architivity, Ltd and MVE, Inc.

#### 3.2 Hydrologic Criteria

For this Final Drainage Report, the Rational Method as described in the *City of Colorado Springs Drainage Criteria Manual (DCM)* has been used for all Storm Runoff calculations, as the development and all sub-basins are less than 130 acres in area. "Colorado Springs Rainfall Intensity Duration Frequency" curves, Figure 6-5 in the DCM, was used to obtain the design rainfall values; a copy is included in the **Appendix**. The "Overland (Initial) Flow Equation" (Eq. 6-8) in the DCM, and Manning's equation with estimated depths were used in time of concentration calculations. "Runoff Coefficients for Rational Method", Table 6-6 in the DCM, was utilized as a guide in estimating runoff coefficient and Percent Impervious values; a copy is included in the **Appendix**. Peak runoff discharges were calculated for each drainage sub-basin for both the 5-year storm event and the 100-year storm event with the Rational Method formula, (Eq. 6-5) in the DCM.<sup>9</sup>

The Water Quality Control Volume reduction procedure detailed in Chapter 3, Section 4.3 and Chapter 4, Fact Sheet T-00 "Quantifying Runoff Reduction" of the *Urban Storm Drainage Criteria Manual, Volume 3* <sup>10</sup> was used for volume reduction calculation with the aid of the "UD-BMP\_v3.07" spreadsheet developed by Mile High Flood District.<sup>11</sup>

#### 4 Drainage Facility Design

#### 4.1 General Concept

The intent of the drainage concept presented in this Final Drainage Report is to maintain the existing drainage patterns on the site while addressing water quality requirements for the site. Major and minor storm flows will continue to be safely conveyed through the site and downstream.

The existing and proposed drainage hydrologic conditions are described in more detail below. Input data and results for all calculations are included in the **Appendix**. Drainage maps for the hydrology are also included in the **Appendix**.

#### 4.2 Specific Details

4.2.1 Existing Hydrologic Conditions

Provide details on offsite flows that enter site from adjacent properties and areas or state no flows enter site

Existing sub-basin EX-A is 2.94 acres in area located on the northern portion of the site and contains a gravel parking and meadow area. Sub-basin EX-A produces peak discharges of  $Q_5 = 2.6$  cfs and  $Q_{100} = 8.2$  cfs (existing flows) which drain easterly towards the existing depression located near Design Point 1 (DP1) in a combination of sheet flow and an area of concentrated flow in the southwest edge of the basin. The concentrated flow occurs in an existing swale flowing to the west, and into the depression. The flows continue easterly onto the existing ball field, southerly into offsite Handle Road and then westerly into a tributary to Black Squirrel Creek.

Existing sub-basin EX-B, located in the northern portion of the site and just south of sub-basin EX-A, is 0.85 acres in area. Sub-basin EX-B contains a paved asphalt drive, concrete pavement and meadow/pasture area. Peak storm runoff rates are  $Q_5 = 1.7$  cfs and  $Q_{100} = 3.6$  cfs (existing flows) which drain easterly towards Design Point 3 (DP3) onto the existing ball field, southerly offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek.

Existing sub-basin EX-C (0.10 acres) is comprised of meadow/pasture area. The sub-basin generates flows of  $Q_5 = 0.0$  cfs and  $Q_{100} = 0.2$  cfs (existing flow), which drain easterly towards Design Point 2 (DP2) onto the existing ball field. These flows, along with the flows from existing DP1 and

Provide discussion and analysis of existing roadside ditches along property and Handle Road and capacity to handle total flows from property Explain how flows are conveyed to creek beyond property and end of road ditch at southwest corner of property at Elliott and Handle Road int. Explain suitable outfall for site

<sup>8</sup> WSS

<sup>9</sup> CS DCM Vol 1 10 USDCM-V.3

<sup>10</sup> USDCM-V.3 11 UD-BMP

DP3, continue southerly and offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek and into Black Squirrel Creek.

Existing sub-basin EX-D, located in the southern portion of the site, is 0.91 acres in area. Sub-basin EX-D contains meadow/pasture area. Peak storm runoff rates are  $Q_5 = 0.3$  cfs and  $Q_{100} = 2.1$  cfs (existing flows) which drain westerly towards Design Point 4 (DP4), southerly offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek.

Existing sub-basin EX-E, located in the southern portion of the site, is 1.29 acres in area. Sub-basin EX-E contains a small area of concrete pavement and meadow/pasture area. Peak storm runoff rates are  $Q_5 = 0.5$  cfs and  $Q_{100} = 3.0$  cfs (existing flows) which drain easterly towards Design Point 5 (DP5), southerly offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek.

The **Existing Drainage Map** depicts the existing topographic mapping, drainage basin delineations, drainage patterns, existing drives, drainage facilities, and runoff quantities with a data table including drainage areas and flow rates.

#### 4.2.2 **Proposed Hydrologic Conditions**

Water quality treatment for the new disturbed and impervious areas on the site will be provided by Runoff Reduction Standard which will treat and release the Water Quality Capture Volume (WQCV). No detention for flood control is being provided because the downstream effects of the minor increases in peak flow rates are negligible. Additionally, the developed flows from the offsite and onsite sub-basins travel through the site and enters a tributary to Black Squirrel Creek located approximately 1,200 feet southwest of the site. The small increase in flows have no effect on peak flows in Black Squirrel Creek and do not present a hazard to the downstream properties, drainage basin, or drainageways and no storm detention is required in addition to the WQCV.

Proposed sub-basin A (2.94 acres) is located in the north of site, where the existing basin EX-A is located. The sub-basin will contain the north portion of the proposed new Votech Building and a proposed gravel drive. A concrete sidewalk will also be installed adjacent to the building on all sides. The developed discharges from sub-basin A are  $Q_5 = 3.1$  cfs and  $Q_{100} = 8.9$  cfs (proposed flows). These flows travel overland easterly towards the proposed depression located near Design Point 1 (DP1) in a combination of sheet flow and an area of concentrated flow in the southwest edge of the basin. These flows will continue to be conveyed in the existing drainage swale which will be reshaped. The flows continue to the west and into the depression, then continue easterly onto the existing ball field, southerly into offsite Handle Road and then westerly into a tributary to Black Squirrel Creek.

Proposed sub-basin B (0.85 acres) will contain the southwestern portion of the proposed new Votech Building and adjacent concrete sidewalk. The developed discharges from sub-basin B are  $Q_5 = 2.0$  cfs and  $Q_{100} = 3.9$  cfs (proposed flows). These flows travel overland easterly towards Design Point 3 (DP3) in the southwest edge of the basin. The flows continue easterly onto the existing ball field, southerly into offsite Handle Road and then westerly into a tributary to Black Squirrel Creek.

Proposed sub-basin C (0.10 acres) will be more developed with the southeastern portion of the proposed new Votech Building and adjacent concrete sidewalk, as well as the southern portion of the proposed gravel drive. The sub-basin generates flows of  $Q_5 = 0.2$  cfs and  $Q_{100} = 0.4$  cfs (proposed flow), which drain easterly towards Design Point 2 (DP2) onto the existing ball field. These flows, along with the flows from existing DP1 and DP3, continue southerly and offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek.

Proposed sub-basin D (0.95 acres) will be more developed with the western portion of the proposed new Classroom Building and adjacent concrete sidewalk. The sub-basin generates flows of  $Q_5 = 1.0$  cfs and  $Q_{100} = 3.0$  cfs (proposed flow), which drain westerly towards Design Point 4 (DP4). These flows continue southerly and offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek.

Proposed sub-basin E (1.21 acres) will be more developed with the eastern portion of the proposed new Classroom Building and adjacent concrete sidewalk. The sub-basin generates flows of  $Q_5 = 1.1$  cfs and  $Q_{100} = 3.6$  cfs (proposed flow), which drain easterly towards Design Point 5 (DP5). These

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flows continue southerly and offsite into Handle Road and then westerly into a tributary to Black Squirrel Creek and into Black Squirrel Creek as described in Section 2.3 above, the same as existing conditions. The existing stable flow path through the adjacent site is adequate to carry the existing and developed flows. Flow velocities in the existing flow path are not erosive and require no special lining. The flow path delivers the flows west to a tributary to Black Squirrel Creek.

#### 4.3 Erosion Control

During future construction, Control Measures (CMs) for erosion control will be employed based on the previously referenced City of Colorado Springs Drainage Criteria Manual Volume 2 and the Erosion Control Plan for the site. During Construction, silt fencing, sediment control logs, vehicle tracking control and concrete washout area will be in place to minimize erosion from the site. Silt Fencing will be placed along the downstream sides of the disturbed areas. This will inhibit suspended sediment form leaving the site during construction. Silt fencing, Inlet Protection, and sediment control logs are to remain in place until the disturbed area is stabilized and until vegeta tion is reestablished in the other disturbed areas which are to be reseeded. Vehicle tracking control will be placed at the access points to the areas of construction/disturbance. Inlet protection will be placed at the outlet location, in the southern portion of the site. CMs will be utilized as deemed necessary by the contractor, engineer, owner, or County inspector and are not limited to the measures described above.

#### 4.4 Water Quality Enhancement Control Measures

There is no storage for the Water Quality Capture Volume (WQCV) for the site. A Grading and Erosion Control Plan for the construction of the site has been prepared in accordance with the provisions of the DCM. Placement of construction stormwater CMs will as required by the plan will limit soil erosion and deposition by stormwater flowing over the site.

The El Paso County Engineering Criteria Manual (Appendix I, Section I.7.2) requires the consideration of a "Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long term source controls". The Four Step Process is incorporated in this project and the elements are discussed below.

1) Runoff Reduction Practices are employed in this project. Impervious surfaces have been reduced as much as practically possible. There is only minimal concrete or other hard surfaces proposed. The proposed drive area will be stabilized with gravel, which remains a partially pervious surface. Minimized Directly Connected Impervious Areas (MDCIA) is employed on the project because runoff passes through the eastern open space meadow area before leaving the site.

2) All drainage paths on the site are stabilized with pavement or appropriate landscape treatment.

3) The project contains no potentially hazardous uses. All developed areas drain into a proposed WQCV BMP.

4) The site contains no storage of potentially harmful substances or use of potentially harmful substances. No Site Specific or Other Source Control CMs are required.

#### 5 Opinion of Probable Cost for Drainage Facilities

There are no drainage improvements associated with this project. This project does not include the installation of permanent water quality control measures.

#### 6 Drainage and Bridge Fees

The site is not being platted. No Drainage or Bridge Fees are due for this project.

#### 7 Conclusion

This Final Drainage Report presents existing and proposed drainage conditions for the proposed Ellicott School Addition - 2 Buildings project. The development will have negligible and inconsequential effects on the existing site drainage and drainage conditions downstream. Water Quality treatment will be provided. The proposed project will not, with respect to stormwater runoff, negatively impact the adjacent properties and downstream properties.



*NRCS/USDA Web Soil Survey*. United States Department of Agriculture, Natural Resources Conservation Service ("https://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx", accessed August, 2022).

*Soil Survey of El Paso County Area, Colorado*. Lynn Seymour Larsen (: National Cooperative Soil Survey, June, 1981).

*Flood Insurance Study for El Paso County, Colorado and incorporated Areas*. Federal Emergency Management Agency (Washington D.C.: FEMA, December 7, 2018).

*Flood Insurance Rate Map.* Federal Emergency Management Agency, National Flood Insurance Program (Washington D.C.: FEMA, December 7, 2018).

*El Paso County Drainage Criteria Manual*. City of Colorado Springs, Department of Public Works; HDR Infrastructure, Inc.; El Paso County, Department of Public Works (: , Revised October 31, 2018).

*City of Colorado Springs Drainage Criteria Manual, Volume 2: Stormwater Quality Policies, Procedures and Best Management Practices (BMPs).* City of Colorado Springs Engineering Division (Colorado Springs: , May 2014).

*City of Colorado Springs Drainage Criteria Manual, Volume 1*. City of Colorado Springs Engineering Division (Colorado Springs: , May 2014).

*Design Procedure Form: Runoff Reduction Spreadsheet*. Mile High Flood District ("https://mhfd.org/wp-content/uploads/2020/03/UD-BMP\_v3.07.xlsm", accessed August, 2022).

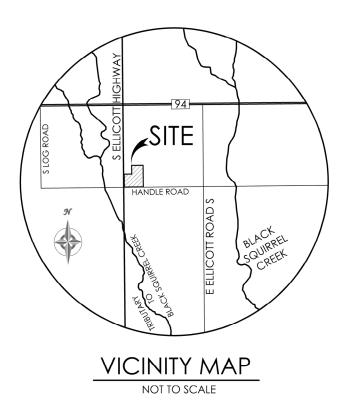
*Urban Storm Drainage Criteria Manual Volume* 3. Mile High Flood District (Denver, Colorado: MHFD, October, 2019).

Add EPC Engineering Criteria Manual

# Appendices

#### 8 General Maps and Supporting Data

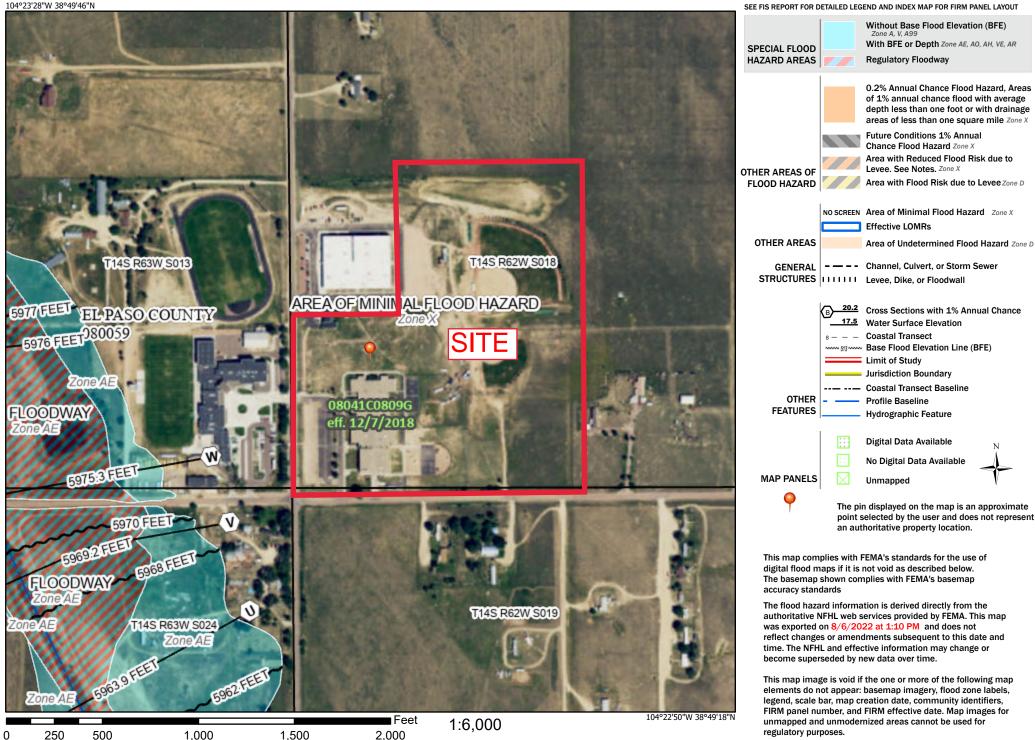
Vicinity Map Portions of Flood Insurance Rate Map Portion of Drainage Area Identification Study Map NRCS Soil Map and Tables Soil Descriptions from Soil Survey of El Paso County Area, Colorado Hydrologic Soil Group Map and Tables



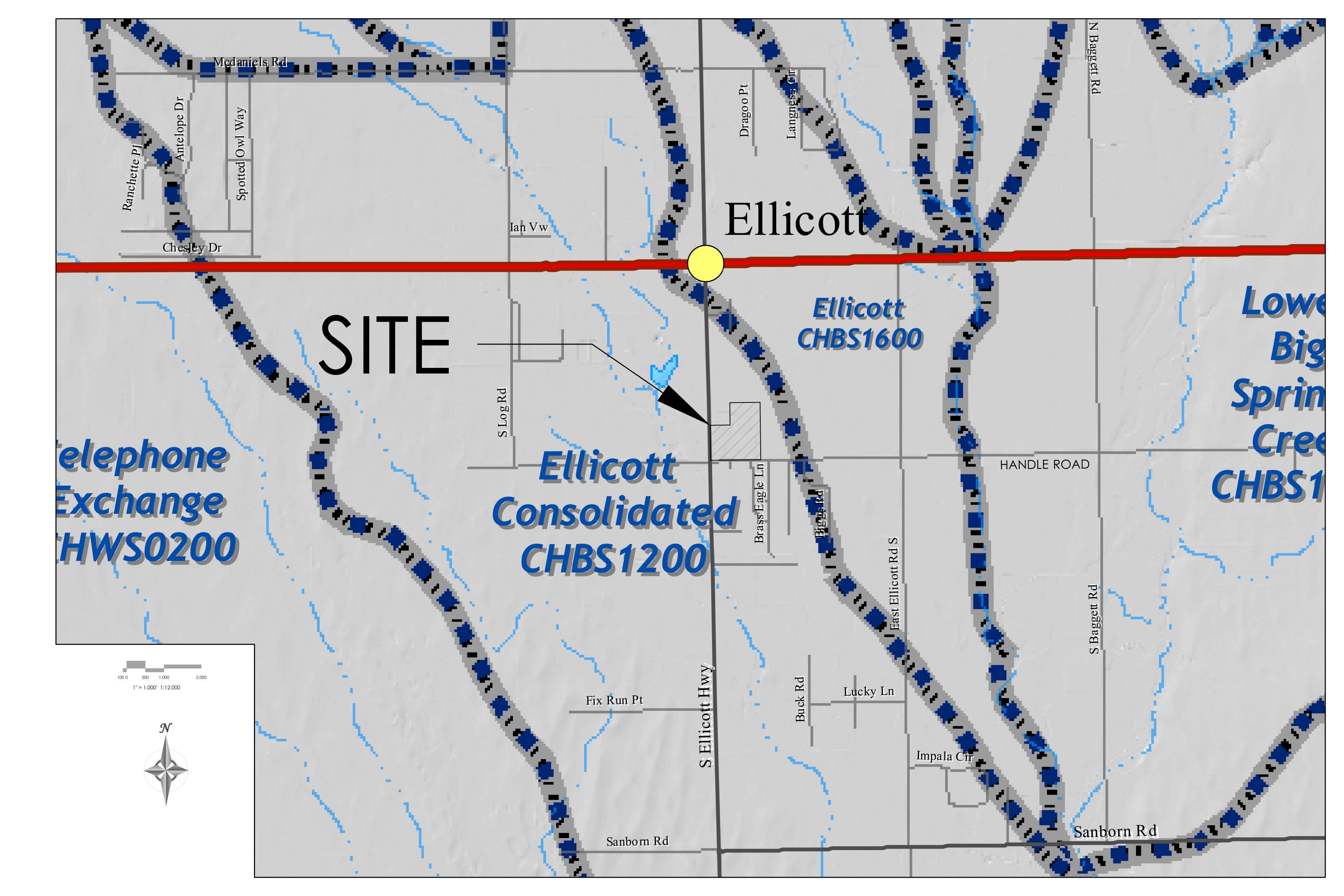
### National Flood Hazard Layer FIRMette



#### Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020





United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for El Paso County Area, Colorado



### Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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### **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

## Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP LEGEND			MAP INFORMATION	
Area of Int	terest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:24,000.	
Soils	Soil Map Unit Polygons	00 V	Very Stony Spot Wet Spot	Warning: Soil Map may not be valid at this scale.	
ĩ	Soil Map Unit Lines Soil Map Unit Points	۵ •	Other Special Line Features	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of	
ల	Point Features Blowout Borrow Pit	Water Fea		contrasting soils that could have been shown at a more detailed scale.	
×	Clay Spot	Transporta	<b>ation</b> Rails	Please rely on the bar scale on each map sheet for map measurements.	
◇ ¥	Closed Depression Gravel Pit Gravelly Spot	~	Interstate Highways US Routes	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)	
 Ø	Landfill Lava Flow	~	Major Roads Local Roads	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts	
2 2 2	Marsh or swamp Mine or Quarry	Background	na Aerial Photography	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.	
0	Miscellaneous Water Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.	
~ +	Rock Outcrop Saline Spot			Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021	
· ·: •	Sandy Spot Severely Eroded Spot			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.	
<b>◇</b> ≫	Sinkhole Slide or Slip			Date(s) aerial images were photographed: Sep 11, 2018—Oct 20, 2018	
ø	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.	

### **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	24.1	78.8%
95	Truckton loamy sand, 1 to 9 percent slopes	6.4	21.0%
96	Truckton sandy loam, 0 to 3 percent slopes	0.1	0.2%
Totals for Area of Interest	1	30.5	100.0%

### Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

#### El Paso County Area, Colorado

#### 19—Columbine gravelly sandy loam, 0 to 3 percent slopes

#### **Map Unit Setting**

National map unit symbol: 367p Elevation: 6,500 to 7,300 feet Mean annual precipitation: 14 to 16 inches Mean annual air temperature: 46 to 50 degrees F Frost-free period: 125 to 145 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

Columbine and similar soils: 97 percent Minor components: 3 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Columbine**

#### Setting

Landform: Flood plains, fan terraces, fans Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium

#### **Typical profile**

*A - 0 to 14 inches:* gravelly sandy loam *C - 14 to 60 inches:* very gravelly loamy sand

#### **Properties and qualities**

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.5 inches)

#### Interpretive groups

Land capability classification (irrigated): 4e Land capability classification (nonirrigated): 6e Hydrologic Soil Group: A Ecological site: R049XY214CO - Gravelly Foothill Hydric soil rating: No

#### **Minor Components**

#### Fluvaquentic haplaquolls

Percent of map unit: 1 percent Landform: Swales Hydric soil rating: Yes

#### Other soils

Percent of map unit: 1 percent Hydric soil rating: No

#### Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

#### 95—Truckton loamy sand, 1 to 9 percent slopes

#### Map Unit Setting

National map unit symbol: 2yvrm Elevation: 5,800 to 7,100 feet Mean annual precipitation: 12 to 19 inches Mean annual air temperature: 46 to 50 degrees F Frost-free period: 90 to 155 days Farmland classification: Not prime farmland

#### Map Unit Composition

*Truckton and similar soils:* 87 percent *Minor components:* 13 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Truckton**

#### Setting

Landform: Fan remnants, interfluves Down-slope shape: Linear Across-slope shape: Linear Parent material: Wind re-worked alluvium derived from arkose

#### **Typical profile**

A - 0 to 4 inches: loamy sand Bt1 - 4 to 12 inches: sandy loam Bt2 - 12 to 19 inches: sandy loam C - 19 to 80 inches: sandy loam

#### **Properties and qualities**

Slope: 1 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 1 percent
Maximum salinity: Nonsaline to very slightly saline (0.1 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Moderate (about 6.5 inches)

#### Interpretive groups

Land capability classification (irrigated): 6e Land capability classification (nonirrigated): 6e Hydrologic Soil Group: A Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### **Minor Components**

#### Blakeland

Percent of map unit: 5 percent Landform: Hills, interfluves Landform position (two-dimensional): Shoulder, backslope, summit Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex, linear Across-slope shape: Convex, linear Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### Bresser

Percent of map unit: 5 percent Landform: Terraces, interfluves Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### **Urban land**

Percent of map unit: 2 percent Hydric soil rating: No

#### Ellicott, occasionally flooded

Percent of map unit: 1 percent Landform: Drainageways, flood plains Down-slope shape: Linear Across-slope shape: Concave, linear Ecological site: R067BY031CO - Sandy Bottomland Hydric soil rating: No

#### 96—Truckton sandy loam, 0 to 3 percent slopes

#### Map Unit Setting

National map unit symbol: 2yvrd Elevation: 5,400 to 7,000 feet Mean annual precipitation: 14 to 23 inches Mean annual air temperature: 45 to 52 degrees F Frost-free period: 90 to 155 days *Farmland classification:* Prime farmland if irrigated and the product of I (soil erodibility) x C (climate factor) does not exceed 60

#### **Map Unit Composition**

*Truckton and similar soils:* 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Truckton**

#### Setting

Landform: Interfluves, fan remnants Down-slope shape: Linear Across-slope shape: Linear Parent material: Wind re-worked alluvium derived from arkose

#### **Typical profile**

A - 0 to 4 inches: sandy loam Bt1 - 4 to 12 inches: sandy loam Bt2 - 12 to 19 inches: sandy loam C - 19 to 80 inches: sandy loam

#### **Properties and qualities**

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 1 percent
Maximum salinity: Nonsaline to very slightly saline (0.1 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 6.6 inches)

#### Interpretive groups

Land capability classification (irrigated): 3e Land capability classification (nonirrigated): 4e Hydrologic Soil Group: A Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### **Minor Components**

#### Blakeland

Percent of map unit: 5 percent Landform: Interfluves, hills Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex, linear Across-slope shape: Convex, linear Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### Bresser

Percent of map unit: 5 percent

Landform: Interfluves, terraces Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

#### Pleasant, frequently ponded

Percent of map unit: 2 percent Landform: Closed depressions Down-slope shape: Concave, linear Across-slope shape: Concave Ecological site: R067BY010CO - Closed Upland Depression Hydric soil rating: Yes

#### **Urban land**

Percent of map unit: 2 percent Hydric soil rating: No

#### Ellicott, occasionally flooded

Percent of map unit: 1 percent Landform: Flood plains, drainageways Down-slope shape: Linear Across-slope shape: Concave, linear Ecological site: R067BY031CO - Sandy Bottomland Hydric soil rating: No

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gravelly subsoil is exposed during site preparation. Access roads must be designed to control surface runoff and help stabilize cut slopes. The Midway soil has poor potential for homesites and roads because of shallow depth to shale, high frost-action potential, and high shrink-swell potential. Special designs are necessary to overcome these limitations. Capability subclass VIIe.

19—Columbine gravelly sandy loam, 0 to 3 percent slopes. This deep, well drained to excessively drained soil formed in coarse textured material on alluvial terraces and fans and on flood plains. Elevation ranges from 6,500 to 7,300 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is grayish brown gravelly sandy loam about 14 inches thick. The underlying material is light yellowish brown very gravelly loamy sand.

Included with this soil in mapping are small areas of Stapleton sandy loam, 3 to 8 percent slopes; Blendon sandy loam, 0 to 3 percent slopes; Louviers silty clay loam, 3 to 18 percent slopes; and Fluvaquentic Haplaquolls, nearly level. In places the parent arkose beds of sandstone or shale are at a depth of 0 to 40 inches.

Permeability of this Columbine soil is very rapid. Effective rooting depth is 60 inches or more. Available water capacity is low to moderate. Surface runoff is slow, and the hazard of erosion is slight to moderate.

This soil is used mainly for grazing livestock and for wildlife habitat. It is also used for homesites.

Native vegetation is mainly western wheatgrass, sideoats grama, needleandthread, and little bluestem. The main shrub is true mountainmahogany.

Proper location of livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings are fairly well suited to this soil. Blowing sand and low available water capacity are the principal limitations to the establishment of trees and shrubs. The soil is so loose that trees need to be planted in the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

Rangeland wildlife, such as pronghorn antelope, cottontail, coyote, and scaled quail, is best adapted to life on this droughty soil. Forage production is typically loam, and proper livestock grazing management is necessary if wildlife and livestock share the range. Livestock watering developments are also important and are used by various wildlife species.

The main limitation of this soil for urban development is a hazard of flooding in some areas. Care must be taken when locating septic tank absorption fields because of possible pollution as a result of the very rapid permeability of this soil. Capability subclass VIe. 20—Connerton-Rock outcrop complex, 8 to 90 percent slopes. This moderately sloping to extremely steep complex is in the Garden of the Gods area, west of Colorado Springs. Elevation ranges from 6,200 to 6,500 feet. The average annual precipitation is about 16 inches, and the average annual air temperature is about 47 degrees F.

The Connerton soil makes up about 45 percent of the complex and has slopes of 8 to 30 percent, Rock outcrop makes up about 40 percent, and other soils about 15 percent.

Included with this complex in mapping are areas of Neville fine sandy loam, 3 to 9 percent slopes; Penrose-Manvel complex, 3 to 45 percent slopes; and Fortwingate-Rock outcrop complex, 15 to 60 percent slopes. Also included are small areas of soils that contain more sand than is typical for the series.

The Connerton soil is deep and well drained. It formed in alluvium derived from reddish sandstone on moderately sloping alluvial fans and valley side slopes. Typically, the surface layer is reddish brown loam about 13 inches thick. The substratum is reddish brown sandy clay loam.

Permeability of the Connerton soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium to rapid, and the hazard of erosion is moderate. A few gullies are in areas of this soil, especially along paths and trails and in drainageways.

Rock outcrop is in long, narrow bands in the form of cliffs or as monoliths and monuments. It consists of red to gray sandstone and limestone.

This complex is used for recreation, wildlife habitat, homesites, and limited livestock grazing.

Native vegetation is mainly western wheatgrass, needlegrasses, big bluestem, side-oats grama, blue grama, and native bluegrasses.

If the range has deteriorated, blue grama, junegrass, and native bluegrasses increase. Sleepygrass and annuals replace these grasses if the range has seriously deteriorated. Seeding is a good practice if the range is in poor condition. Seeding of the native vegetation is desirable, but the range can also be seeded with tame species of grasses such as Nordan crested wheatgrass, Russian wildrye, pubescent wheatgrass, or intermediate wheatgrass.

This complex is suited to the production of juniper and pinyon pine. It is capable of producing 4 cords per acre in a stand of trees that average 5 inches in diameter at a height of 1 foot. The limitations for the production of wood crops are the presence of stones on the surface and a high hazard of erosion. Stones on the surface can influence felling, yarding, and other operations involving the use of equipment. Special care must be taken to minimize erosion when harvesting timber.

This complex is relatively unproductive for vegetation, especially in times of drought, when annual production may be as low as 300 pounds per acre. Rangeland wildlife, such as antelope and scaled quail, can be encouraged by properly managing livestock grazing, installing livestock watering facilities, and reseeding range where needed. Permeability of the Crowfoot soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate. Some gullies are present in some drainageways and along stock trails.

The soils in this complex are used as rangeland, for recreation and wildlife habitat, and as homesites.

Native vegetation is mainly mountain muhly, bluestem, mountain brome, needleandthread, and blue grama. These soils are subject to invasion by Kentucky bluegrass and Gambel oak. Noticeable forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Proper location of livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings are fairly well suited to these soils. Blowing sand and moderate available water capacity are the main limitations for the establishment of trees and shrubs. The soils are so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

These soils are best suited to habitat for openland wildlife species, such as pronghorn antelope and sharp-tailed grouse. Although sharp-tailed grouse are not plentiful, they could be encouraged on these soils, especially where brush species are interspersed with grasses and forbs. If these soils are used as rangeland, wildlife production can be increased by managing livestock grazing to preclude overuse of the more desirable grass species and depletion of the various brush species.

The main limitations for urban uses are frost-action potential and slope on the Crowfoot soil and slope on the Tomah soil. Buildings and roads must be designed to overcome these limitations. Access roads must have adequate cut-slope grade and be provided with drains to control surface runoff. Maintaining the existing vegetation on building sites during construction helps to control erosion. Capability subclass VIe.

94—Travessilla-Rock outcrop complex, 8 to 90 percent slopes. This moderately sloping to extremely steep complex is mostly on rocky uplands (fig. 5). Elevation ranges from 6,200 to 6,700 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frostfree period is about 140 days.

The Travessilla soil makes up about 45 percent of the complex, Rock outcrop about 30 percent, and included areas about 25 percent.

Included with this complex in mapping are areas of Bresser sandy loam, 5 to 9 percent slopes, Elbeth sandy loam, 8 to 15 percent slopes, Kettle gravelly loamy sand, 8 to 40 percent slopes, and Louviers silty clay loam, 3 to 18 percent slopes. The Elbeth and Kettle soils commonly are on the north-facing slopes. The Travessilla soil is shallow and well drained. It formed in residuum derived from sandstone. Typically, the surface layer is light brownish gray sandy loam about 3 inches thick. The underlying material is pale brown sandy loam about 8 inches thick. Hard arkosic sandstone that has some fractures is at a depth of about 11 inches.

Permeability of the Travessilla soil is moderately rapid. Effective rooting depth is 6 to 20 inches. Available water capacity is low. Surface runoff is medium to rapid, and the hazard of erosion is high. Gullies are common along drainageways and trails.

Rock outcrop occurs mostly as ledges on cliffs.

This complex is used for urban development, as homesites, and for recreation and wildlife habitat.

This complex is suited to the production of ponderosa pine. The main limitations are the presence of stones and rock outcrop on the surface and a high hazard of erosion. Stones on the surface can hinder felling, yarding, and other operations involving the use of equipment. Practices must be used to minimize soil erosion when harvesting timber. The low available water capacity can influence seedling survival.

Wildlife on these soils is limited mostly to small animals such as cottontail, squirrel, and birds because of the extent of urban development. Ponderosa pine, mountainmahogany, Gambel oak, and various grasses provide food, cover, and nesting areas.

This complex is extensively used for urban development and as homesites (fig. 6). The main limitations for these uses are depth to bedrock, rock outcrop, and steep slopes. Septic tank absorption fields do not function properly because of the depth to bedrock. Special designs for buildings and roads and streets are needed to overcome the limitations. Plans for homesite development should provide for the preservation of as many trees as possible because of their esthetic value. Capability subclass VIIe.

95—Truckton loamy sand, 1 to 9 percent slopes. This deep, well drained soil formed in alluvium and residuum derived from arkosic sedimentary rock on uplands. Elevation ranges from 6,000 to 7,000 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frostfree period is about 135 days.

Typically, the surface layer is grayish brown loamy sand about 8 inches thick. The subsoil is brown sandy loam about 18 inches thick. The substratum is light yellowish brown coarse sandy loam to a depth of 60 inches or more.

Included with this soil in mapping are small areas of Blakeland loamy sand, 1 to 9 percent slopes; Bresser sandy loam, 3 to 5 percent sloeps; Bresser sandy loam, 5 to 9 percent slopes; Truckton sandy loam, 0 to 3 percent slopes; and Truckton sandy loam, 3 to 9 percent slopes.

Permeability of this Truckton soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is moderate to high. Almost all areas of this soil are used as rangeland. A few areas of crops such as alfalfa and corn are grown under sprinkler irrigation.

This soil is well suited to the production of native vegetation suitable for grazing. It is best suited to deeprooted grasses. The native vegetation is mainly cool- and warm-season grasses such as western wheatgrass, sideoats grama, and needleandthread.

Proper range management is needed to prevent excessive removal of the plant cover. Interseeding is used to improve the existing vegetation. Deferment of grazing in spring increases plant vigor and soil stability. Properly locating livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings are fairly well suited to this soil. Blowing sand is the main limitation for the establishment of trees and shrubs. The soil is so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to wildlife habitat. It is best suited to openland and rangeland wildlife habitat. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for use as homesites. The main limitation of this soil for roads and streets is frost action potential. Special designs for roads are needed to minimize this limitation. Practices are needed to control soil blowing and water erosion on construction sites where the plant cover has been removed. Capability subclass VIe, nonirrigated.

96—Truckton sandy loam, 0 to 3 percent slopes. This deep, well drained soil formed in alluvium and residuum derived from arkosic sedimentary rock on uplands. Elevation ranges from 6,000 to 7,000 feet. The average annual precipitation is about 15 inches, the average annual air temperatue is about 47 degrees F, and the average frostfree period is about 135 days.

Typically, the surface layer is grayish brown sandy loam about 5 inches thick. The next layer is dark grayish brown sandy loam about 3 inches thick. The subsoil is brown sandy loam about 16 inches thick. The substratum is light yellowish brown coarse sandy loam to a depth of 60 inches or more.

Included with this soil in mapping are small areas of Blakeland loamy sand, 1 to 9 percent slopes; Bresser sandy loam, 0 to 3 percent slopes; Ellicott loamy coarse sand, 0 to 5 percent slopes; and Ustic Torrifluvents, loamy.

Permeability of this Truckton soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazards of erosion and soil blowing are moderate. This soil is used mainly for cultivated crops. It is also used for livestock grazing, for wildlife habitat, and as homesites.

Crops are commonly grown in combination with summer fallow because moisture is insufficient for annual cropping. Alfalfa can also be grown on this soil. When this soil is used as cropland, crop residue management and minimum tillage are necessary conservation practices.

This soil is well suited to the production of native vegetation suitable for grazing (fig. 7). It favors deeprooted grasses. The native vegetation is mainly cool- and warm-season grasses such as western wheatgrass, sideoats grama, and needleandthread.

Proper range management is needed to prevent excessive removal of the plant cover. Interseeding is used to improve the existing vegetation. Deferment of grazing in spring increases plant vigor and soil stability. Properly locating livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings generally are suited to this soil. Soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

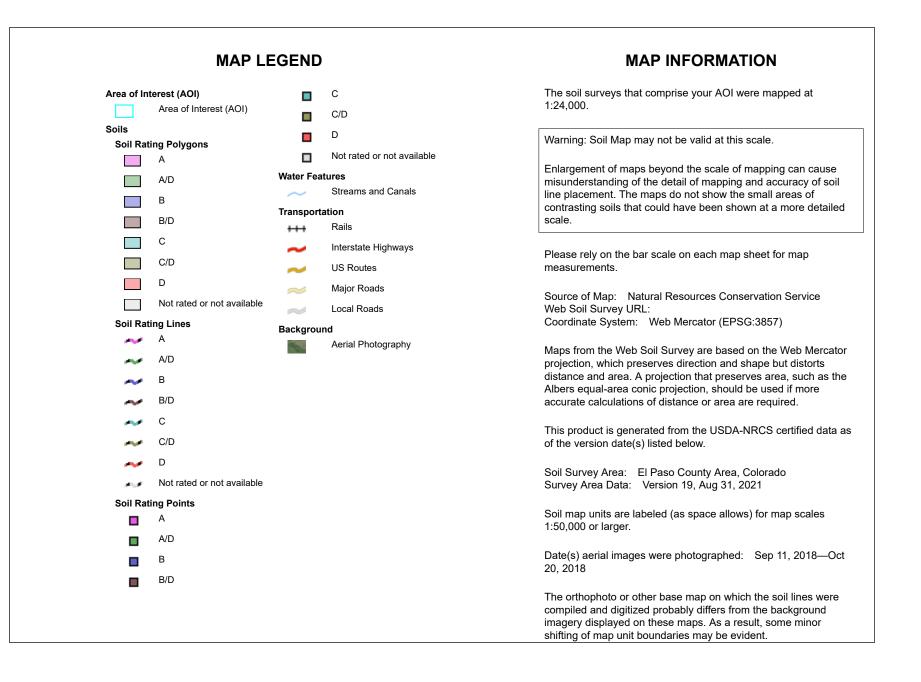
This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing areas for nesting and escape cover. For pheasant, undisturbed nesting cover is vital and should be provided in plans for habitat development. This is especially true in areas of intensive farming. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for use as homesites. The main limitation of this soil for roads and streets is frostaction potential. Special designs for roads are needed to overcome this limitation. Capability subclasses IIIe, nonirrigated, and IIe, irrigated.

97—Truckton sandy loam, 3 to 9 percent slopes. This deep, well drained soil formed in alluvium and residuum derived from arkosic sedimentary rock on uplands. Elevation ranges from 6,000 to 7,000 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is grayish brown sandy loam about 5 inches thick. The next layer is dark grayish brown sandy loam about 3 inches thick. The subsoil is brown sandy loam about 16 inches thick. The substratum is light yellowish brown coarse sandy loam to a depth of 60 inches or more.





# Table—Hydrologic Soil Group (61183)

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	26.8	80.9%
95	Truckton loamy sand, 1 to 9 percent slopes	А	6.3	18.9%
96	Truckton sandy loam, 0 to 3 percent slopes	A	0.1	0.2%
Totals for Area of Intere	st		33.2	100.0%

# Rating Options—Hydrologic Soil Group (61183)

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

# 9 Hydrologic Calculations

Runoff Coefficients and Percent Imperviousness Table 6-6 Colorado Springs Rainfall Intensity Duration Frequency Figure 6-5 Hydrologic Calculations Summary Form SF-1 for Existing & Developed Conditions Hydrologic Calculations Summary 5-yr Form SF-2 for Existing & Developed Conditions Hydrologic Calculations Summary 100-yr Form SF-2 for Existing & Developed Conditions

Land Use or Surface	Percent						Runoff Co	efficients					
Characteristics	Impervious	2-у	ear	5-y	ear	10-1	/ear	ץ-25	/ear	י-50	/ear	100-	year
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30 0.18 0.22 0.25 0.30 0.32 0.38		0.39	0.47	0.43	0.52	0.47	0.57					
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.89	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.33	0.70	0.74
Deiter and Waller	100	0.00	0.00	0.00	0.00	0.02	0.02	0.04	0.04	0.05	0.05	0.00	0.00
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

# Table 6-6. Runoff Coefficients for Rational Method(Source: UDFCD 2001)

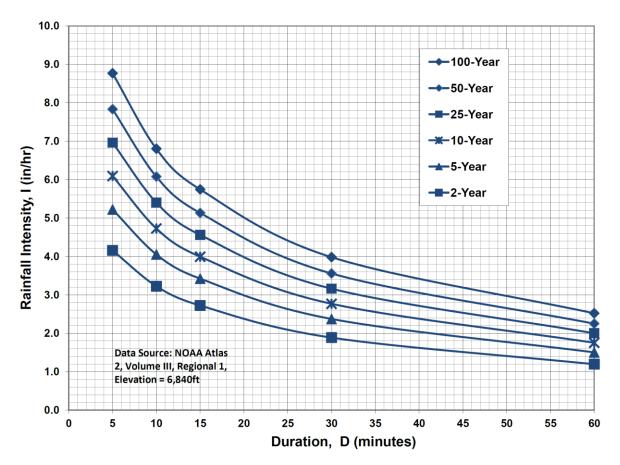


Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency

<b>IDF Equations</b>
$I_{100} = -2.52 \ln(D) + 12.735$
$I_{50} = -2.25 \ln(D) + 11.375$
$I_{25} = -2.00 \ln(D) + 10.111$
$I_{10} = -1.75 \ln(D) + 8.847$
$I_5 = -1.50 \ln(D) + 7.583$
$I_2 = -1.19 \ln(D) + 6.035$
Note: Values calculated by equations may not precisely duplicate values read from figure.

Job No.: Project:	61183 Ellicott D	22 – GS &	HS Additic	on								Date: Calcs By	•	O. Ali		8	/31/202	2 16:18
				Time	of Cor	ncentr	ation (	Modified	from Sta	andard F		Checkeo -1)	d By:					
		Sub-Basi	n Data		(	Overland	b		Shallow	Channe	I		Chanr	nelized		t <sub>c</sub> Ch	neck	
Sub- Basin	Area (Acres)	$C_5$	C <sub>100</sub> /CN	% Imp.	L <sub>0</sub> (ft)	S <sub>0</sub> (%)	t <sub>i</sub> (min)	L <sub>0t</sub> (ft)	S <sub>0t</sub> (ft/ft)	v <sub>0sc</sub> (ft/s)	t <sub>t</sub> (min)	L <sub>0c</sub> (ft)	S <sub>0c</sub> (ft/ft)	v <sub>0c</sub> (ft/s)	t <sub>c</sub> (min)	L (min)	t <sub>c,alt</sub> (min)	t <sub>c</sub> (min)
EX-A	2.94	0.24	0.46	25%	100	3%			0.008	1.8	4.6	144.7	0.014				14.1	14.1
EX-B EX-C	0.85 0.10	0.55 0.08	0.69 0.35	61% 0%	73 68	3% 4%	6.1 9.9	300 0	0.000 0.000	0.0 0.0	600.0 0.0	273 0	0.007 0.000				13.6 10.4	13.6 9.9
EX-D	0.10	0.00	0.33	3%	100	4 % 2%	14.3		0.000	0.0	4.0	65	0.000		1.0		12.0	
EX-E	1.29	0.10		2%	100	3%	13.4		0.016	0.9	3.5	120	0.008		1.8		12.3	
A	2.94	0.30	0.50	33%	100	3%	10.1	364	0.008	1.8	3.3	337	0.006	0.8			14.5	14.5
В	0.85	0.63	0.75	71%	73	3%	5.2	300	0.000	0.0	600.0	273	0.007	0.8	5.4		13.6	
С	0.10	0.36	0.55	41%	83 100	3% 2%	8.5	0	0.000	0.0	0.0	0	0.000		0.0		10.5	
D E	0.95 1.21	0.27 0.23	0.49 0.46	26% 21%	94	2% 3%	11.9 11.0		0.010 0.000	0.7 0.0	1.2 0.0	184 278	0.014 0.011		3.1 5.0		11.9 12.1	11.9 12.1
																		L

Joh	No.:	611	83

Project: Ellicott D22 -	GS & HS Addition	
Design Storm:	5-Year Storm	(20% Probability)
Jurisdiction:	DCM	

O. Ali	
	O. Ali

8/31/2022 16:18

Junsuic	_	DCINI		n		Sub	-Basin a	nd Com	bined Flo	<b>WS</b> (Modi	fied from S	tandard	Form SF	-2)								
					Direct	Runoff			Combine	d Runoff			Streetflov				ipe Flow			т	ravel Tin	ne
	Sub-	Area		t <sub>c</sub>	CA	15	Q5	t <sub>c</sub>	CA	15	Q5	Slope	Length	Q	Q	Slope	Mnngs	Length	D <sub>Pipe</sub>	Length	V <sub>0sc</sub>	t
DP	Basin	(Acres)	C5	(min)	(Acres)	(in/hr)	(cfs)	(min)	(Acres)	(in/hr)	(cfs)	(%)	(ft)	(cfs)	(cfs)	(%)	n	(ft)	(in)	(ft)	(ft/s)	(min)
							0.57															
DP1 DP2	EX-A EX-B	2.94	0.24	14.1	0.71		2.57 1.71															
DP2 DP3	EX-B EX-C	0.85 0.10	0.55 0.08		0.47 0.01																	
DP3 DP4	EX-D	0.10	0.08		0.01																	
DP4 DP5	EX-E	1.29	0.10		0.03																	
DIS	LX-L	1.23	0.10	12.0	0.15	0.02	0.40															
DP1	А	2.94	0.30	14.5	0.88	3.58	3.15															
DP2	в	0.85	0.63		0.54																	
DP3	С	0.10	0.36		0.03																	
DP4	D E	0.95	0.27	11.9	0.26	3.87	1.00															
DP5	E	1.21	0.23	12.1	0.28	3.85	1.09															
								1	1			I	1	1		1	1		1	1	1	

DCM: I = C1 \* In (tc) + C2

C1: 1.5

C1: 7.583

Joh	No.:	611	83

Project: Ellicott D22 – GS & HS Addition Design Storm: 100-Year Storm (1% Probability) Undediction: DCM										
Design Storm:	100-Year Storm	(1% Probability)								
Jurisdiction:	DCM									

Sub-Basin and Combined Flows	(Modified from Standard Form SF-2)
------------------------------	------------------------------------

					Direct F				Combine			1	Streetflov			D	ipe Flow	,		Travel Time		
	Sub-	Area		tc	CA	1100	Q100	tc	COMDINE	1100	Q100		Length		Q			Length	Drine			t,
DP	Basin	(Acres)	C100	(min)	(Acres)	(in/hr)	(cfs)	(min)	(Acres)	(in/hr)	(cfs)	(%)	(ft)	(cfs)	(cfs)	(%)		(ft)	(in)	(ft)	(ft/s)	(min)
	Buom	(710100)	0.00	()	(/ 10/00/	(,)	(0.07	()	(/ 10/ 00/	(	(0.0)	(70)	()	(0.07	(0.0)	(70)		(11)	()	()	(	()
DP1	EX-A	2.94	0.46	14.1	1.36	6.06	8.24															
DP2	EX-B	0.85	0.69			6.16																
DP3	EX-C	0.10	0.35			6.96	0.23															
DP4	EX-D	0.87	0.37	12.0	0.32	6.46	2.06															
DP5	EX-E	1.29	0.36	12.3	0.47	6.42	3.00															
DP1	A	2.94	0.50			6.00	8.88															
DP2	B C	0.85	0.75			6.16	3.93															
DP3	С	0.10	0.55			7.36	0.38															
DP4	D	0.95	0.49			6.50	3.01															
DP5	E	1.21	0.46	12.1	0.56	6.46	3.60															

DCM: I = C1 \* In (tc) + C2 C1: 2.52

C1: 12.735

8/31/2022 16:18 O. Ali

Calcs By: Checked By:

Date:

# Sub-Basin EX-A (DP1) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali	
		Checked by:		
Jurisdiction	DCM	Soil Type	)	Α
Runoff Coefficient	Surface Type	Urbaniza	tion	Urban

#### **Basin Land Use Characteristics**

	Area			Runo	off Coeffici	ent			%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Gravel	36,180	0.83	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	2,520	0.06	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	354	0.01	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	88,987	2.04	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	128,041	2.94	0.19	0.24	0.30	0.38	0.42	0.46	24.8%
	128041								

# **Basin Travel Time**

## Shallow Channel Ground Cover Paved areas/shallow paved swales

	L <sub>max,Overland</sub>	100	ft		Cv	20
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	740	2	-	-	-	-
Initial Time	100	3	0.030	-	10.8	14.1 DCM Eq. 6-8
Shallow Channel	495	4	0.008	1.8	4.6	- DCM Eq. 6-9
Channelized	145	2	0.014	0.8	3.0	- V-Ditch
				t <sub>c</sub>	14.1 r	nin.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality is	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-yea	100-year			Desigi	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.0000	) 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0.000	0.14561	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0 0	6,343	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.89	3.61	4.22	4.82	5.42	6.06
Runoff (cfs)	1.6	2.6	3.7	5.4	6.7	8.2
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.6	2.6	3.7	5.4	6.7	8.2
DCM: I	= C1 * ln (	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin EX-B (DP2) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali	
		Checked by:		
Jurisdiction	DCM	Soil Type		Α
Runoff Coefficient	Surface Type	Urbanizat	tion	Urban

#### **Basin Land Use Characteristics**

	Area			Runo	off Coeffici	ent			%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Paved	16,806	0.39	0.89	0.9	0.92	0.94	0.95	0.96	100%
Gravel	6,992	0.16	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	13,184	0.30	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	26.092	0.95	0.52	0.55	0.50	0.64	0.67	0.60	60.6%
Combined	36,982	0.85	0.52	0.55	0.59	0.64	0.67	0.69	60.6%
	36982								

# **Basin Travel Time**

## Shallow Channel Ground Cover Short Pasture/Lawns

	L <sub>max,Overland</sub>	100	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	646	4	-	-	-	-
Initial Time	73	2	0.027	-	6.1	13.6 DCM Eq. 6-8
Shallow Channel	300		0.000	0.0	600.0	- DCM Eq. 6-9
Channelized	273	2	0.007	0.8	5.4	- V-Ditch
				t <sub>c</sub>	13.6 ı	min.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality i	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-yea	ar 100-year			Desig	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.000	0 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.00 0.00	0 0.10642	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 4,636	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.93	3.67	4.28	4.89	5.50	6.16
Runoff (cfs)	1.3	1.7	2.1	2.7	3.1	3.6
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.3	1.7	2.1	2.7	3.1	3.6
DCM: I	= C1 * ln (†	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin EX-C (DP3) Runoff Calculations

Job No.:	61183			I	Date:			8/31/20	022 16:25
Project:	Ellicott D22 – GS	& HS Add	lition		Calcs by: Checked b	ov:	O. Ali		
Jurisdiction	DCM				enconcu a	Soil Type		4	
Runoff Coefficient	Surface Type					Urbanizati		Jrban	
Basin Land Use Characte	eristics								
	Area				off Coeffic				%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Pasture/Meadow	4,152	0.10	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	4,152	0.10	0.02	0.08	0.15	0.25	0.30	0.35	0.0%
	4152								
Basin Travel Time									
Sna	allow Channel Gro	una Cover 100		ure/Lawns	Cv	7			
	L <sub>max,Overland</sub>			v (ft/a)					
Total	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)			
Initial Time	68 68	3	0.037	-	- 9.9	- 10 4	DCM Eq. 6-8		
Shallow Channel	00	Ű	0.000	0.0	0.0		DCM Eq. 6-9		
Channelized			0.000	0.0	0.0		V-Ditch		
				t <sub>c</sub>	9.9	min.			
Storage Volume									
<b>3</b>			40	-hr release	time		Detention is N	OT required	
EURV	0.00	(in)	a =	1			Water Quality	is NOT requi	red
WQCV	0.00	(in)							
i (return period)	5-year	10-year	100-year				Desig	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000	0.0000	0			% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000	0.000	-0.00095		EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0	-42	N N	WQCV	0%	0	0	0
Rainfall Intensity & Rund	off								
-			2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	
	Intens	sity (in/hr)	3.31	4.15	4.84	5.53	6.22	6.96	
	Rı	unoff (cfs)	0.0	0.0	0.1	0.1	0.2	0.2	
	Release Rate	es (cfs/ac)	-	-	-	-	-	-	
	Allowed Rel		0.0	0.0	0.1	0.1	0.2	0.2	
		DCM: I	= C1 * ln (t	ic) + C2					
		C1	1.19	1.5	1.75	2	2.25	2.52	
		C2	6.035	7.583	8.847	10.111	11.375	12.735	

# Sub-Basin EX-D (DP4) Runoff Calculations

Paved 1,031 0.02 0.89 0.9 0.92 0.94 0.95 0.96	Job No.:	61183	-		C	Date:			8/31/2	022 16:2
Jurisdiction       DCM       Soil Type       A         Runoff Coefficient       Surface Type       Urbanization       Urban         sin Land Use Characteristics       Area       Runoff Coefficient       Image: Coefficient         Surface       (SF)       (Acres)       C2       C5       C10       C25       C50       C100       Image: Coefficient         Paved       1,031       0.02       0.89       0.9       0.92       0.94       0.95       0.96	Project:	Ellicott D22 – G	S & HS Add	ition	C	Calcs by:	C	). Ali		
Runoff Coefficient         Surface Type         Urbanization         Urban           sin Land Use Characteristics         Area         Runoff Coefficient         Image: Coefficien					C	Checked by	<i>.</i>			
Sin Land Use Characteristics           Runoff Coefficient           Surface         C2         C5         C10         C2           Paved         1,031         0.02         0.89         0.92         0.94         0.95         0.96	Jurisdiction	DCM				5	Soil Type	/	A	
Area         Runoff Coefficient         Image: Surface         Surface         C2         C5         C10         C25         C50         C100         Image: Surface         Image: Surface         Image: Surface         Image: Surface         Image: Surface         Surface         C10         C25         C50         C100         Image: Surface         Image: Surface         Image: Surface         Surface         Surface         Image: Surface         Image: Surface         Image: Surface	Runoff Coefficient	Surface Type				ι	Jrbanizatior	n <b>l</b>	Urban	
Paved 1,031 0.02 0.89 0.9 0.92 0.94 0.95 0.96	in Land Use Char	racteristics								
	in Land Use Char	Area			Runo	ff Coeffici	ent			0
		Area		C2				C50	C100	ہ Imperv
Pasture/Meadow 36,928 0.85 0.02 0.08 0.15 0.25 0.3 0.35	Surface	Area (SF)	(Acres)		C5	C10	C25			Imperv
	Surface	Area (SF)	(Acres)		C5	C10	C25			<b>Imper</b> 100

0.04

0.10

0.17

0.27

0.32

0.37

2.7%

#### **Basin Travel Time**

Combined

#### Shallow Channel Ground Cover Short Pasture/Lawns

0.87

37,959

37959

	L <sub>max,Overland</sub>	100 f	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	368	6	-	-	-	-
Initial Time	100	2	0.020	-	14.3	12.0 DCM Eq. 6-8
Shallow Channel	203	3	0.015	0.9	4.0	- DCM Eq. 6-9
Channelized	65	1	0.008	1.1	1.0	- C&G
				t <sub>c</sub>	12.0 ı	min.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality i	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-ye	ar 100-year			Desig	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.000	0 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.00 0.0	-0.0028	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 -122	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.07	3.85	4.49	5.13	5.78	6.46
Runoff (cfs)	0.1	0.3	0.7	1.2	1.6	2.1
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.1	0.3	0.7	1.2	1.6	2.1
DCM: I	= C1 * ln (†	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin EX-E (DP5) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali	
		Checked by:		
Jurisdiction	DCM	Soil Type	e	Α
Runoff Coefficient	Surface Type	Urbaniza	ation	Urban

#### **Basin Land Use Characteristics**

	Area	Area		Runoff Coefficient					%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Paved	1,240	0.03	0.89	0.9	0.92	0.94	0.95	0.96	100%
Pasture/Meadow	54,765	1.26	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	<b>56,005</b>	1.29	0.04	0.10	0.17	0.27	0.31	0.36	2.2%

# **Basin Travel Time**

Shallow Channel Ground Cover Short Pasture/Lawns

	L <sub>max,Overland</sub>	100 f	t		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	407	7	-	-	-	-
Initial Time	100	3	0.025	-	13.4	12.3 DCM Eq. 6-8
Shallow Channel	187	3	0.016	0.9	3.5	- DCM Eq. 6-9
Channelized	120	1	0.008	1.1	1.8	- C&G
				t <sub>c</sub>	12.3	min.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality is	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-yea	r 100-year			Desigi	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.000	) 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0.00	-0.00573	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	-250	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.05	3.82	4.46	5.10	5.74	6.42
Runoff (cfs)	0.2	0.5	1.0	1.7	2.3	3.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.2	0.5	1.0	1.7	2.3	3.0
DCM: I	= C1 * In (t	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin A (DP1) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali	
		Checked by:		
Jurisdiction	DCM	Soil Type	)	Α
Runoff Coefficient	Surface Type	Urbanizat	tion	Urban

#### **Basin Land Use Characteristics**

	Area			Runo	off Coeffici	ent			%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Gravel	37,365	0.86	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	3,896	0.09	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	8,959	0.21	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	77,821	1.79	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	128,041	2.94	0.26	0.30	0.36	0.43	0.47	0.50	32.7%
	128041								

# **Basin Travel Time**

## Shallow Channel Ground Cover Paved areas/shallow paved swales

	L <sub>max,Overland</sub>	100	ft		Cv	20
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	801	2	-	-	-	-
Initial Time	100	3	0.030	-	10.1	14.5 DCM Eq. 6-8
Shallow Channel	364	3	0.008	1.8	3.3	- DCM Eq. 6-9
Channelized	337	2	0.006	0.8	7.2	- V-Ditch
				t <sub>c</sub>	14.5 r	nin.

# Storage Volume

		40	) -hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	= 1		Water Quality is	is NOT required	
WQCV	0.00 (in)						
i (return period)	5-year 10-y	ear 100-yea	r		Desigi	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.00	000 000	)	% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0.0	0.19759	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 8,607	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.86	3.58	4.17	4.77	5.37	6.00
Runoff (cfs)	2.1	3.1	4.4	6.0	7.3	8.9
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	2.1	3.1	4.4	6.0	7.3	8.9
DCM: I	= C1 * ln (†	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin B (DP2) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali	
		Checked by:		
Jurisdiction	DCM	Soil Type		Α
Runoff Coefficient	Surface Type	Urbanizat	ion	Urban

#### **Basin Land Use Characteristics**

	Area		Runoff Coefficient						%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Paved	18,466	0.42	0.89	0.9	0.92	0.94	0.95	0.96	100%
Gravel	6,992	0.16	0.57	0.59	0.63	0.66	0.68	0.7	80%
Roofs	2,535	0.06	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	8,989	0.21	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	36,982	0.85	0.61	0.63	0.67	0.71	0.73	0.75	71.2%
	36982								

# **Basin Travel Time**

# Shallow Channel Ground Cover Short Pasture/Lawns

	L <sub>max,Overland</sub>	100	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	646	4	-	-	-	-
Initial Time	73	2	0.027	-	5.2	13.6 DCM Eq. 6-8
Shallow Channel	300		0.000	0.0	600.0	- DCM Eq. 6-9
Channelized	273	2	0.007	0.8	5.4	- V-Ditch
				t <sub>c</sub>	13.6 ı	min.

# Storage Volume

		40	) -hr release time		Detention is NC	T required	
EURV	0.00 (in)	a =	= 1		Water Quality is	NOT require	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-	-year 100-yea	r		Design	Volume (	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.0	0000 0	)	% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0	0.000 0.12369	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 5,388	3 WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.93	3.67	4.28	4.89	5.50	6.16
Runoff (cfs)	1.5	2.0	2.4	2.9	3.4	3.9
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.5	2.0	2.4	2.9	3.4	3.9
DCM: I	= C1 * ln (	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin C (DP3) Runoff Calculations

Job No.:	61183	Date:		8/31/2022 16:	25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali		
		Checked by:			
Jurisdiction	DCM	Soil T	уре	Α	
Runoff Coefficient	Surface Type	Urbar	nization	Urban	

#### **Basin Land Use Characteristics**

	Area			Runo	off Coeffici	ent			%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Gravel	1,307	0.03	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	319	0.01	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	365	0.01	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	2,161	0.05	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	4,152	0.10	0.32	0.36	0.41	0.48	0.51	0.55	40.8%
<b>e</b>	4152	1 11							

# **Basin Travel Time**

## Shallow Channel Ground Cover Short Pasture/Lawns

	L <sub>max,Overland</sub>	100	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	83	3	-	-	-	-
Initial Time	83	3	0.030	-	8.5	10.5 DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				t <sub>c</sub>	8.5 r	nin.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality i	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-yea	r 100-year			Desig	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.000 0.000	0 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0.00	0.00809	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 352	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.50	4.38	5.11	5.84	6.57	7.36
Runoff (cfs)	0.1	0.2	0.2	0.3	0.3	0.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.1	0.2	0.2	0.3	0.3	0.4
DCM: I	= C1 * In (1	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin D (DP4) Runoff Calculations

Job No.:	61183				Date:			8/31/2	022 16:25
Project:	Ellicott D22 – G	S & HS Add	ition		Calcs by:	C	). Ali		
					Checked b	y:			
Jurisdiction	DCM					Soil Type		A	
Runoff Coefficient	Surface Type					Urbanizatior	ı I	Urban	
sin Land Use Cha	racteristics								
	Area			Rund	off Coeffic	ient			%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv
Paved	2,296	0.05	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	9,361	0.21	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	29,651	0.68	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	41,308	0.95	0.22	0.27	0.33	0.41	0.45	0.49	26.0%
	41308								
asin Travel Time									
	Shallow Channel Gro	ound Cover	Short Pastu	re/Lawns					
		100 f	ft		C	7			

	L <sub>max,Overland</sub>	100	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	335	5	-	-	-	-
Initial Time	100	2	0.020	-	11.9	11.9 DCM Eq. 6-8
Shallow Channel	51	1	0.010	0.7	1.2	- DCM Eq. 6-9
Channelized	184	3	0.014	1.0	3.1	- V-Ditch
				t <sub>c</sub>	11.9	min.

# Storage Volume

			40	-hr release time		Detention is NO	OT required	
EURV	0.00 (ir	า)	a =	1		Water Quality i	s NOT require	d
WQCV	0.00 (ir	า)						
i (return period)	5-year	10-year	100-year			Desig	n Volume (	ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000	0.0000	0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000	0.000	0.04942	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0	2,153	WQCV	0%	0	0	0

# Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.09	3.87	4.52	5.16	5.81	6.50
Runoff (cfs)	0.7	1.0	1.4	2.0	2.5	3.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.7	1.0	1.4	2.0	2.5	3.0
DCM: I	= C1 * In (t	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# Sub-Basin E (DP5) Runoff Calculations

Job No.:	61183	Date:		8/31/202	2 16:25
Project:	Ellicott D22 – GS & HS Addition	Calcs by:	O. Ali		
		Checked by:			
Jurisdiction	DCM	Soil T	уре	Α	
Runoff Coefficient	Surface Type	Urbar	nization	Urban	

#### **Basin Land Use Characteristics**

	Area		Runoff Coefficient						%
Surface	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	Imperv.
Paved	2,205	0.05	0.89	0.9	0.92	0.94	0.95	0.96	100%
Roofs	9,761	0.22	0.71	0.73	0.75	0.78	0.8	0.81	90%
Pasture/Meadow	40,691	0.93	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	52,657	1.21	0.18	0.23	0.29	0.38	0.42	0.46	20.9%
	52657								

# **Basin Travel Time**

Shallow Channel Ground Cover Short Pasture/Lawns

	L <sub>max,Overland</sub>	100	ft		Cv	7
	L (ft)	$\Delta Z_0$ (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	372	6	-	-	-	-
Initial Time	94	3	0.027	-	11.0	12.1 DCM Eq. 6-8
Shallow Channel	0	0	0.000	0.0	0.0	- DCM Eq. 6-9
Channelized	278	3	0.011	0.9	5.0	- V-Ditch
				t <sub>c</sub>	12.1 r	nin.

# Storage Volume

		40	-hr release time		Detention is NO	OT required	
EURV	0.00 (in)	a =	1		Water Quality i	s NOT requir	ed
WQCV	0.00 (in)						
i (return period)	5-year 10-y	ear 100-year			Desig	n Volume	(ft <sup>3</sup> )
K <sub>i</sub> (ft)	0.0000 0.00	000 0		% Storage	100-year	WQCV	Total
V <sub>i</sub> (acre-ft)	0.000 0.0	000 0.04888	EURV	0%	0	0	0
$V_i$ (ft <sup>3</sup> )	0	0 2,129	WQCV	0%	0	0	0

# **Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.07	3.85	4.49	5.13	5.77	6.46
Runoff (cfs)	0.7	1.1	1.6	2.3	2.9	3.6
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.7	1.1	1.6	2.3	2.9	3.6
DCM: I	= C1 * ln (†	tc) + C2				
C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

# 10 Hydraulic Calculations

Design Procedure Form: Runoff Reduction Spreadsheet

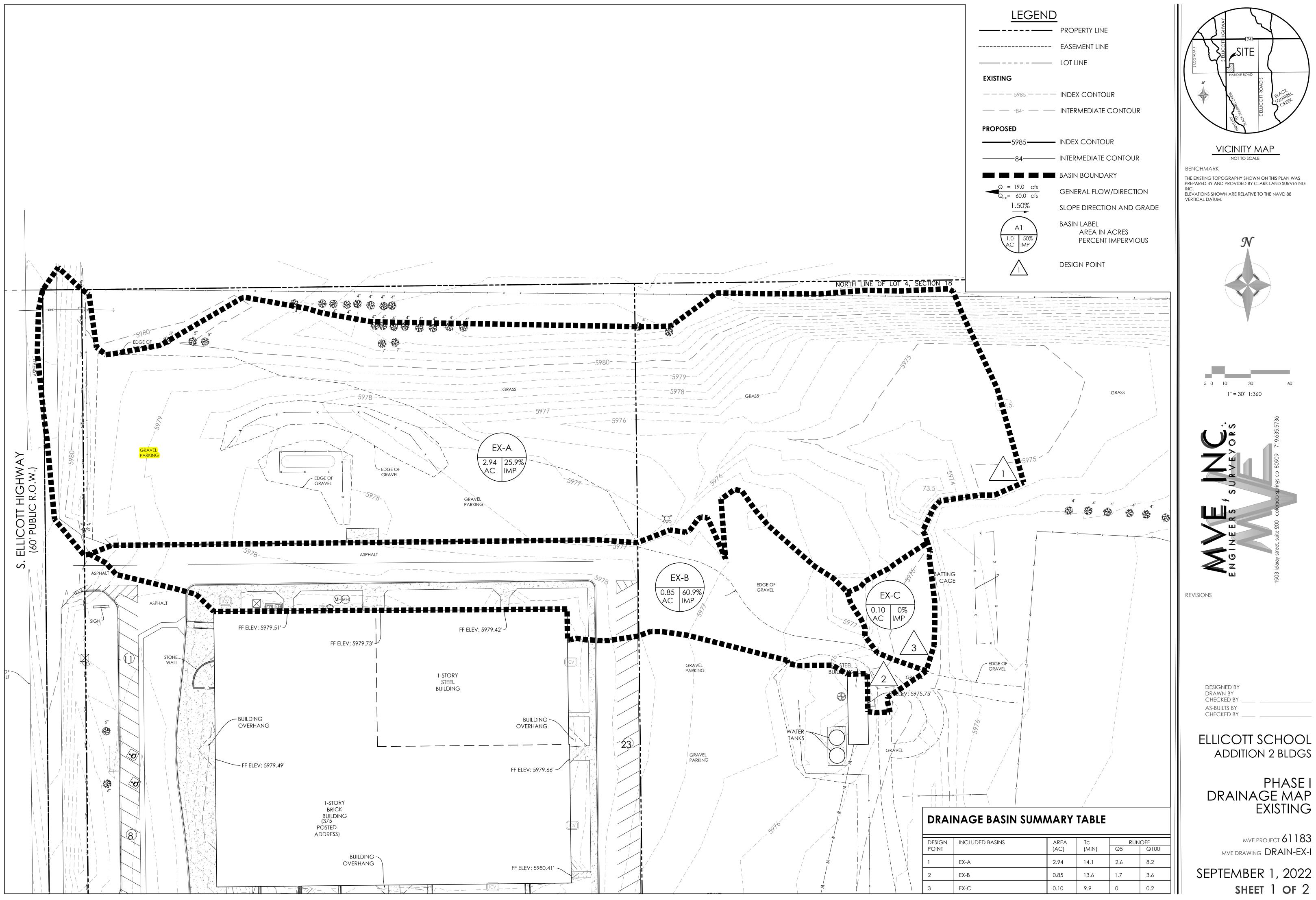
Map: Runoff Reduction

UB-NP (Perceios 127, Name 3216)         Sheft 1 of 1           Company:         M.V.L. floc.				Desig	JII Procedu	ire Form. I	Runoff Red	uction				 
Compary:         V.L. III.           Date:         Auguit 1202           Project:         Effect School Addition 2 Mdgs           State:         Effect School Addition 2 Mdgs					UD-BMP (Ve	ersion 3.07, Ma	rch 2018)					Sheet 1 of 1
Date         Payet:           Electric Electric Addition 2 Mage           STE NFORMATION (User Input In Blue Calls)           MCV Rainfall Deght           Deght of Average Rund Postson Res, 4 = 0.03           And Tage           Deght of Average Rund Postson Res, 4 = 0.03           Downsteam Blue Postson Res, 4 = 0.000           Downsteam Blue Postson Res, 4 = 0.0000           Dow	5											
Public         Emilia Scale           STE INCOMMING User Input in Blac Calls         MCC Panied Daph         0.00         nches           Deard Average Rund Producing Storm, 4, =         0.00         nches         0.00         nches           Downstream Example         0.00         nches         0.00         nches         0.00												
Index colspan="2">Index colspan="2" Index colspan	Project:	Ellicott Schoo	ol Addition 2 b	dgs								
WCC Relatel Depth         0.60         Private           Depth of Weare Routh Throwing Storm, 4;         0.43         Inches (for Waterheds Oxtaide of the Denver Region, Figure 3-1 in USDCM Vol. 3)           Area Type         MARPA         NARPA         NARPA         NARPA           Area D         P4         P41         Image: P4         Image: P4           Area D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4         Image: P4           Downstream Design Forti D         P41         Image: P4         Image: P4         Image: P4         Image: P4           D45 (5)         9%         9%         9%         Image: P4         Image: P4         Image: P4         Image: P4           D41 (Arrange Arrange         P41         Image: P4         P41         Image: P4         Image: P4           D41 (Arrange Arrange         P41         Image: P4         P41         Image: P4         Image: P4           UIARPA Arrange         P41 <td< td=""><td>Location:</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	Location:											
WCC Relatel Depth         0.60         Private           Depth of Weare Routh Throwing Storm, 4;         0.43         Inches (for Waterheds Oxtaide of the Denver Region, Figure 3-1 in USDCM Vol. 3)           Area Type         MARPA         NARPA         NARPA         NARPA           Area D         P4         P41         Image: P4         Image: P4           Area D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4           Downstream Design Forti D         P4         P41         Image: P4         Image: P4         Image: P4           Downstream Design Forti D         P41         Image: P4         Image: P4         Image: P4         Image: P4           D45 (5)         9%         9%         9%         Image: P4         Image: P4         Image: P4         Image: P4           D41 (Arrange Arrange         P41         Image: P4         P41         Image: P4         Image: P4           D41 (Arrange Arrange         P41         Image: P4         P41         Image: P4         Image: P4           UIARPA Arrange         P41 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>												
Area Type         UARPA         UARPA <thuarpa< th="">         UARPA         UARPA</thuarpa<>		WQCV F	Rainfall Depth		-							
Available         P-I         P-II         P-III         P-III         P-III         P-III         P-III         P-III         P-III         P-III         P-III         P-IIII         P-IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	Depth of Average Ru	nom Producin	ig Storm, a <sub>6</sub> =	0.43	inches (for W	/atersheds Ot	itside of the L	enver Regior	n, Figure 3-1 i	n USDCM Vo	1. 3)	
Downstram Design Paint ID         PAIL         Image: Control of the second seco	••											
Downstram BMP Type         None         Image: Second Secon												
DCA (ff)   <	-											
UA (h)         17.345         21.383         Image: Control of the second seco												
RPA (ff)         -<					ļ					ļ		
SPA (#)         -          -         - </td <td></td>												
HSG A (%)         100%         100%         1         1         1         1         1           Average Skipe of RPA (http         0.010         0.090         1					L							
HSG B (%)         0%					ļ		L	L			L	
HSG CD (%)         0%												
Average Slope of RPA (tth)         0.010         0.090         Image: control of the state of the	· · ·											
UAR-PA Interface Width (th)         150.00         141.00         Image: Constraint of the system of the s	. ,											
CALCULATED RUNOFF RESULTS           Marka 10         P-1         P-1         P-1           UK-RA Area (ff)         5000         80.000         1					<u> </u>							
Area (f)         P-I         P-I         P-I           UIA:RPA Area (fi)         80.000         80.000         90.0000         90.0000         90.000	UNA.INFA INTENACE WILLIN (IL)	130.00	141.00		1	1			1	1		
L / W Raito UIA / Area 0.2168 0.2670	Area ID	P-I										
UIA / Area         0.2168         0.2870         Image: Constraint of the second seco	( )											
Runoff (in)         0.00         0.00         0												 
Runoff (R <sup>3</sup> )         0         0         0           Runoff Reduction (R <sup>3</sup> )         723         890         0												 
Runoff Reduction (n <sup>2</sup> )         723         890         Image: constraint of the second seco												
CALCULATED WQCV RESULTS           Area ID         P-I         P-II												
Area ID         P-I         P-II         Image: Constraint of the second secon	Runon Reduction (it )	120	000									
Area ID         P-I         P-II         Image: Constraint of the second secon	CALCULATED WQCV RE	SULTS										
WQCV (th³)         723         890         Image: Constraint of the second sec			P-II									
WQCV Reduction (#i)         723         890         Image: Contract of the second sec												
WQCV Reduction (%)         100%         100%         Image: constraint of the symbol of the												
Untreated WQCV (ft <sup>3</sup> )         0			100%									
Downstream Design Point ID         P-I         P-II         I         P-II         I		0	0									
Downstream Design Point ID         P-I         P-II         I         P-II         I				culto from a		ith the same	Downetroom	Design Roi	at ID)			
DCIA (ft²)         0					in columns w	an the same	DownStredit	Design FOI				
UIA (ft <sup>2</sup> )       17,345       21,363       Image: Constraint of the second s					<u> </u>			-			-	
RPA (ft <sup>2</sup> )       62,655       58,637       Image: constraint of the second s					t							
SPA (ft <sup>2</sup> )       0 <td< td=""><td></td><td></td><td></td><td></td><td>t</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>					t							
Total Area (ft <sup>2</sup> )       80,000       80,000       Image: constraint of the second s												
Total Impervious Area (ft <sup>2</sup> )       17,345       21,363       Image: Constraint of the second seco												
WQCV (ft <sup>3</sup> )         723         890         Image: Constraint of the second s					İ							
WQCV Reduction (ft <sup>3</sup> )       723       890       Image: Constraint of the second sec					1							
WQCV Reduction (%)         100%         100%         Image: Constraint of the second												
CALCULATED SITE RESULTS (sums results from all columns in worksheet)           Total Area (ft <sup>2</sup> )         160.000           Total Impervious Area (ft <sup>2</sup> )         38.708           WQCV (ft <sup>3</sup> )         1.613           WQCV Reduction (ft <sup>2</sup> )         1.613           WQCV Reduction (%)         100%	WQCV Reduction (%)		100%									
Total Area (ft²)       160,000         Total Impervious Area (ft²)       38,708         WQCV (ft³)       1,613         WQCV Reduction (ft³)       1,613         WQCV Reduction (%)       100%	Untreated WQCV (ft <sup>3</sup> )	0	0									
WQCV (ft <sup>3</sup> )         1,613           WQCV Reduction (ft <sup>3</sup> )         1,613           WQCV Reduction (%)         100%			results from	all columns	in workshee	et)						
WQCV (ft <sup>3</sup> )         1,613           WQCV Reduction (ft <sup>3</sup> )         1,613           WQCV Reduction (%)         100%		38,708	]									
WQCV Reduction (ft <sup>3</sup> )         1,613           WQCV Reduction (%)         100%												
Untreated WQCV (ft <sup>3</sup> ) 0		1000/	1									
	WQCV Reduction (%)	100%										

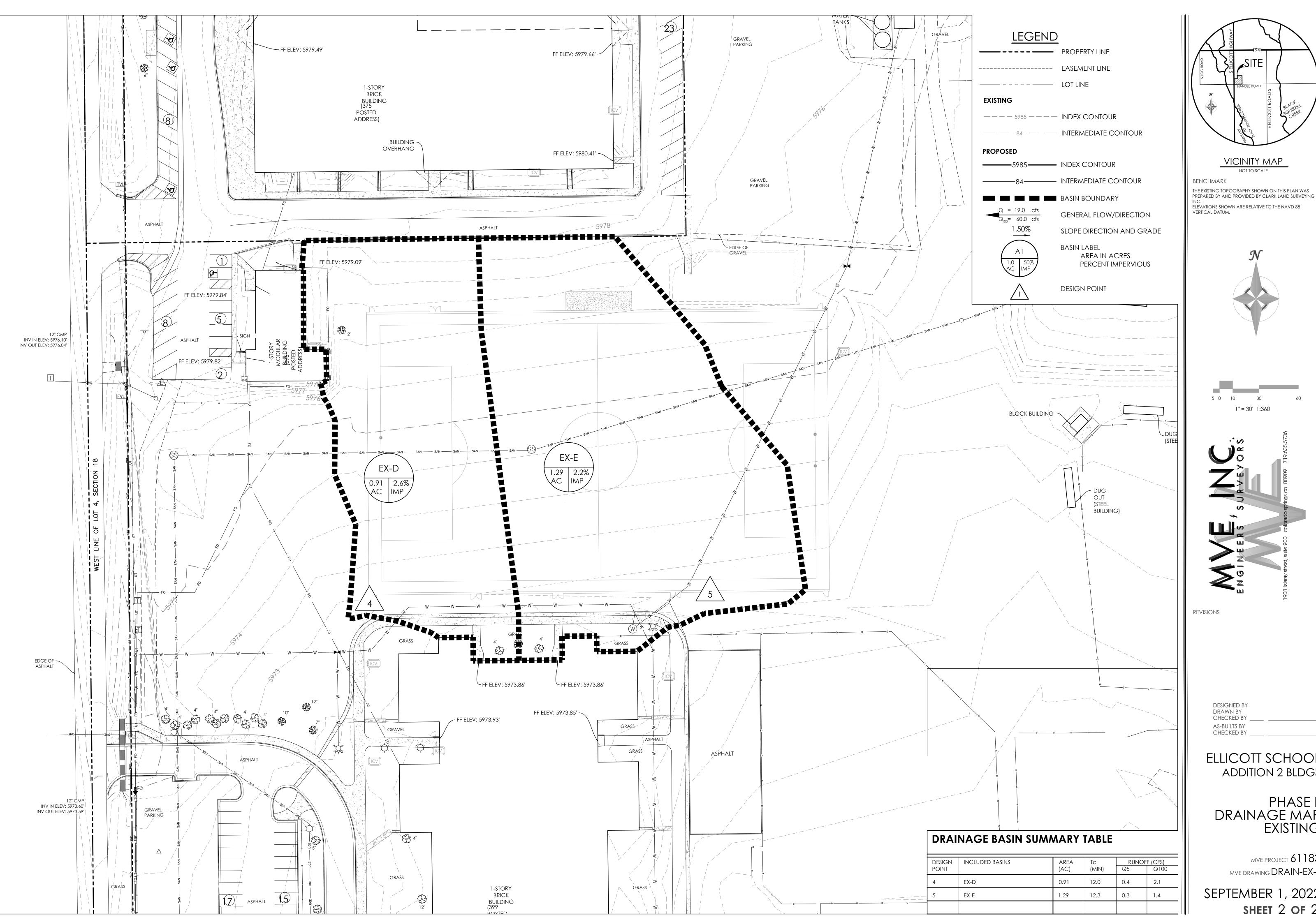


# 11 Report Maps

Phase I Existing Condition Drainage Map Phase II Existing Condition Drainage Map Phase I Proposed Condition Drainage Map Phase II Proposed Condition Drainage Map



DESIGN POINT	INCLUDED BASINS	AREA (AC)	Tc (MIN)	RUNOFF Q5 Q100	
1	EX-A	2.94	14.1	2.6	8.2
2	EX-B	0.85	13.6	1.7	3.6
3	EX-C	0.10	9.9	0	0.2

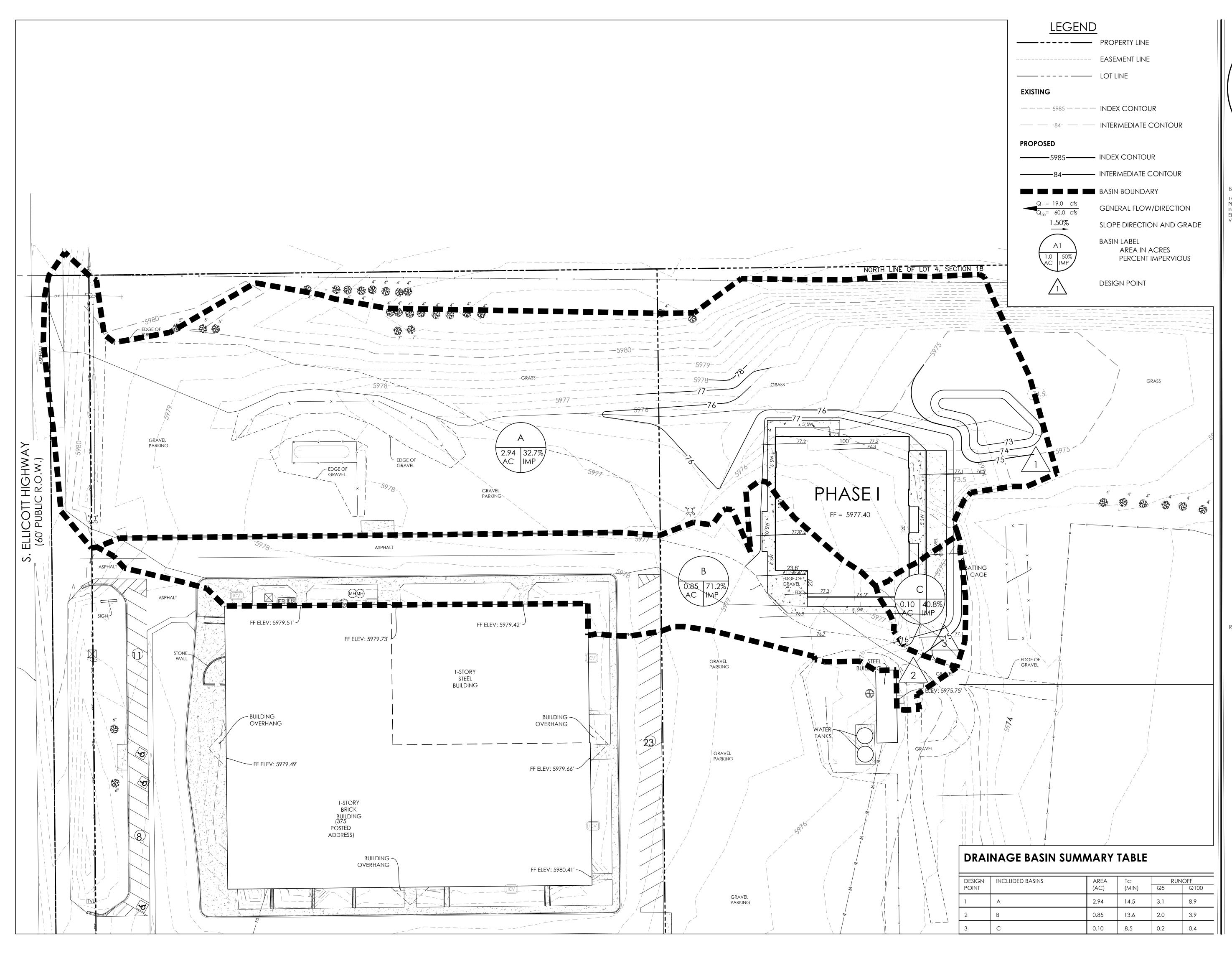


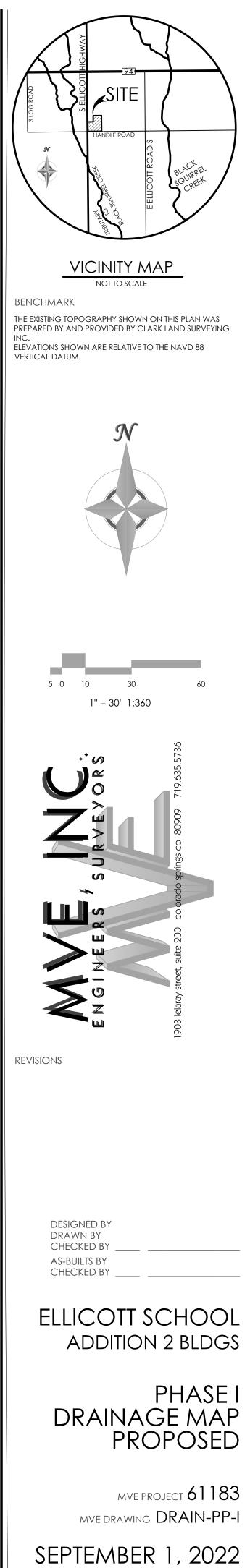
ELLICOTT SCHOOL ADDITION 2 BLDGS

PHASE II DRAINAGE MAP EXISTING

MVE PROJECT 61183 MVE DRAWING DRAIN-EX-II

SEPTEMBER 1, 2022 sheet 2 of 2





SHEET 1 OF 2

