July 13, 2018

Platte Valley, LLC 1375 Promontory Bluff View Colorado Springs, CO 80921

Attn: Ron Waldthausen

Soil, Geology and Geologic Hazard Evaluati showing the test hole locations Re: Lot 2, Appaloosa Hwy 24 Subdivision Filing (Section 8.4.9.C.2.a.iii) including all El Paso County, Colorado

This report does not include a map other applicable requirements in Section 8.4.9.C.

Dear Mr. Waldthausen:

Include this in submittal.

As requested, personnel of Entech Engineering, Inc. have investigated the above referenced site to evaluate the conditions with respect to geology and geologic hazards affecting development of the site. The subsurface soil conditions were previously investigated by Entech Engineering, Inc. in a Subsurface Soil Investigation, May 30, 2017 (Reference 1).

The project consists of commercial development on a 4.67-acre site. The site lies in El Paso County, Colorado, immediately east of Colorado Springs city limits. The approximate location of the site is shown on the Vicinity Location Map, Figure 1. The site as platted and proposed replat is included in Figure 2.

The topography of the site is generally flat to very gradually sloping to the southwest. No major drainages exist on the site. However, a concrete drainage channel is located along the eastern boundary of the site. The site is currently vacant. Vegetation consists of field grasses and weeds. A fill pile is located in the northern portion of the site. The site lies in a portion of Sections 7, Township 14 South, Range 65 West of the 6th Principal Meridian in El Paso County, Colorado. The Site Plan is presented in Figure 2.

Seven test borings were drilled as part of the original Subsurface Soil Investigation (Reference 1). to evaluate the subsurface soil conditions. Three of the test borings (Test Boring Nos. 1, 2 and 3) were drilled on the subject site. The Test Boring logs and Laboratory Tests Results are included in Appendix B. Information from this report was used evaluating the site.

The scope of this report includes a geologic analysis/ evaluation of the site utilizing published geologic data, available subsurface soils information and site-specific mapping of major geologic features, and identification of geologic hazards with respect to the development with recommended mitigation techniques. The Natural Resource Conservation Service (NRCS), previously the Soil Conservation Service (SCS) Survey was also reviewed to evaluate the site.





505 ELKTON DRIVE COLORADO SPRINGS, CO 80907 PHONE (719) 531-5599 (719) 531-5238 FAX

SOIL AND GEOLOGIC CONDITIONS

Soil Survey

The Natural Resource Conservation Service (NRCS) (Reference 2, Figure 3), previously the Soil Conservation Service (Reference 3) has mapped one soil type on the site. Complete description of the soils is presented in Appendix C. In general, the soils consist of sandy loam. The soils are described as follows:

<u>Type</u>	<u>Description</u>
96	Truckton sandy loam, 0-3% slopes

<u>Soils</u>

The soils encountered in the Test Boring Nos. 1, 2 and 3 from Subsurface Soil Investigation (Reference 1) consisted of four to six feet of silty to clayey sand fill/possible fill, native silty to slightly silty and gravelly sand and sandy clay. Bedrock was not encountered in the test borings, which were drilled on the subject site to 20 feet. The upper soils were encountered at loose to medium dense states and moist conditions. The sand soils are considered to have low expansion potential. A FHA Swell pressure of 750 psf was measured on the sandy clay, indicating low expansion potential.

Groundwater

Groundwater was encountered at 20 feet in Test Boring No. 2 (Reference 1). Groundwater was not encountered in Test Boring Nos. 1 and 3, which were drilled to 20 feet. It is anticipated groundwater will not affect shallow foundations on site. Fluctuations in groundwater conditions may occur due to variations in rainfall or other factors not readily apparent at this time. Isolated sand layers within the soil profile can carry water in the subsurface. Contractors should be cognizant of the potential for the occurrence of subsurface water features during construction.

Geology

Approximately 8 miles west of the site is a major structural feature known as the Rampart Range Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within a large structural feature known as the Denver Basin. Bedrock in the area is typically gently dipping in a northerly direction. The bedrock underlying the site consists of the Dawson Formation of Cretaceous Age. The Dawson Formation typically consists of coarse-grained arkosic sandstone with interbedded layers siltstone or claystone. Overlying the Dawson Formation are deposits of man-made fill soils and soils associated with wind blown sands and water-deposited alluvial sands.

The geology of the site was evaluated using the *Geologic Map of the Elsmere Quadrangle*, by Madole and Thorson in 2003, (Reference 4, Figure 4). The Geology for the site is presented in Figure 5. Three mappable units were identified on this site which, are described as follows:

- Qaf Artificial Fill of Quaternary Age: These are man-made fill deposits associated with past grading and fill piles observed on the site. Areas of fill other than those mapped may be encountered.
- Qam Middle Alluvium of Quaternary Age: This deposit typically consists of water deposited silty sands and may have clay layers. This deposit occurs as a stream terrace deposit associated with Sand Creek.
- Qes Eolian Sands of Quaternary Age: These are wind-blown sands deposited by the action of prevailing winds. The materials typically consist of silty sands and may contain sandy silt layers.

ENGINEERING GEOLOGIC HAZARDS

Mapping has been performed on this site to identify areas where various geologic conditions exist of which developers should be cognizant during the planning, design and construction stages should new construction be proposed. The engineering geologic hazards identified on this site include artificial fill, loose soils, potentially expansive soils and floodplain areas. These hazards and recommended mitigation techniques are discussed as follows:

Artificial Fill

Up to six feet of possible fill was encountered in two of the borings test drilled on site. Fill was also observed on the site associated with the existing fill piles.

<u>Mitigation:</u> It is anticipated the fill piles will be removed during site grading. Areas of fill other than those encountered may be encountered. The fill piles are considered uncontrolled. Unless records can be obtained, other fill on the site will also be considered uncontrolled. Any uncontrolled fill encountered beneath foundations should be removed and recompacted at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557. Fill mitigation should occur during individual building construction.

Loose Soils

Loose soils were encountered in borings drilled on site (Reference 1). Loose soils if encountered beneath the foundation or floor slabs will require mitigation.

<u>Mitigation:</u> Should loose soils be encountered beneath the foundations or floor slabs, mitigation will be necessary. Overexcavation and recompaction at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557 is a suitable mitigation, which is common in the area. An overexcavation depth of 2 to 3 feet is anticipated.

Expansive Soils

A layer of expansive soils was encountered in one the test borings. Expansive clays, if encountered beneath foundations, can cause differential movement in the structure foundation. <u>Mitigation</u>: Should expansive soils be encountered beneath the foundation, mitigation will be necessary. Mitigation of expansive soils will require special foundation design. Overexcavation and replacement with non-expansive soils at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557 is a suitable mitigation, which is common in the area. Floor slabs on expansive soils should be expected to experience movement. Overexcavation and replacement has been successful in minimizing slab movements.

Floodplain Areas

Portions of the site lie within a floodplain according to the FIRM Map, No. 08041C0754F (Reference 5, Figure 6). Finished floors must be a minimum of one foot above the floodplain level. Any site grading considered should be modified to direct surface flows around the structures or roads, or carried off-site so as to not produce any areas of ponded water. Additionally, subsurface perimeter drains may be required. Specific drainage studies and exact floodplain locations are beyond the scope of this report.

RELEVANCE OF GEOLOGIC CONDITIONS TO LAND USE PLANNING

As mentioned, the proposed development will be commercial. The existing geologic and engineering geologic conditions will impose minor constraints on development and construction. The geologic conditions on the site include artificial fill, loose soils, potentially expansive soil layers and floodplain areas, which can be satisfactorily mitigated through proper engineering design and construction practices.

The upper granular soils encountered in the borings drilled on the site were encountered at loose to medium dense states. Loose or collapsible soils, if encountered beneath foundation or floor slabs, will require recompaction. Expansive layers may also be encountered in the soil on this site. Expansive soils, if encountered, will require special foundation design. These soils will not prohibit development.

Fill exists on this site that is associated with a fill pile on site and in the upper soils in two test borings. Other areas of fill may be encountered on the site. All fill piles and debris should be completely removed and prior to construction. Any uncontrolled fill encountered beneath new foundations and floor slabs will require removal and recompaction at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557.

Groundwater was encountered at 20 feet in Test Boring No. 2. It is anticipated groundwater will not affect a shallow foundation with slab-on-grade construction and no basement construction. According to the FEMA FIRM Map No. 08041C00754F (Reference 5, Figure 6) a portion of the site is located in a floodplain. Finished floors must be a minimum of one foot above the floodplain level. Specific drainage studies and exact floodplain locations are beyond the scope of this report.

In summary, the recompacted granular soils will likely provide suitable support for shallow foundations. The geologic conditions encountered on site can be mitigated with proper engineering and construction practices. Specific recommendations have been made in the Subsurface Soil Investigation (Reference 1).

CLOSURE

It should be pointed out that because of the nature of data obtained by random sampling of such variable nonhomogeneous materials as soil and rock, it is important that we be informed of any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Any new construction considered on this site will require additional investigation. Construction and design personnel should be made familiar with the contents of this report. Specific construction and foundation recommendations will be provided when investigations are completed for any new construction.

This report has been prepared for Platte Valley, LLC for application to the proposed development in accordance with generally accepted geologic, soil and engineering practices. No other warranty expresses or implied is made.

We trust that this report has provided you with all the information that you required. Should you have any questions or require additional information, please do not hesitate to contact us.

Respectfully Submitted,

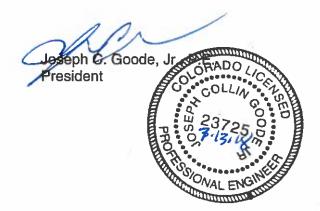
ENTECH ENGINEERING, INC.

Logan L. Langford Geologist

LLL/III

Encl.

Entech Job No. 181169 AAprojects/2018/181169 sg&ghs **Reviewed by:**

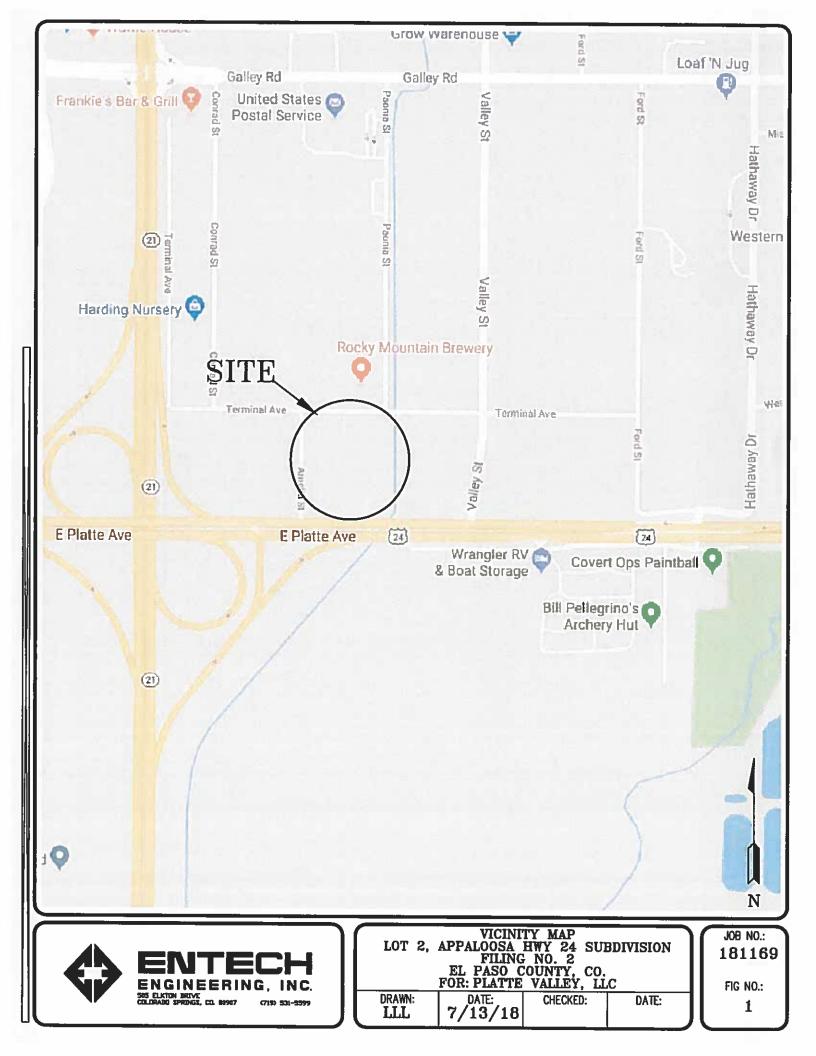


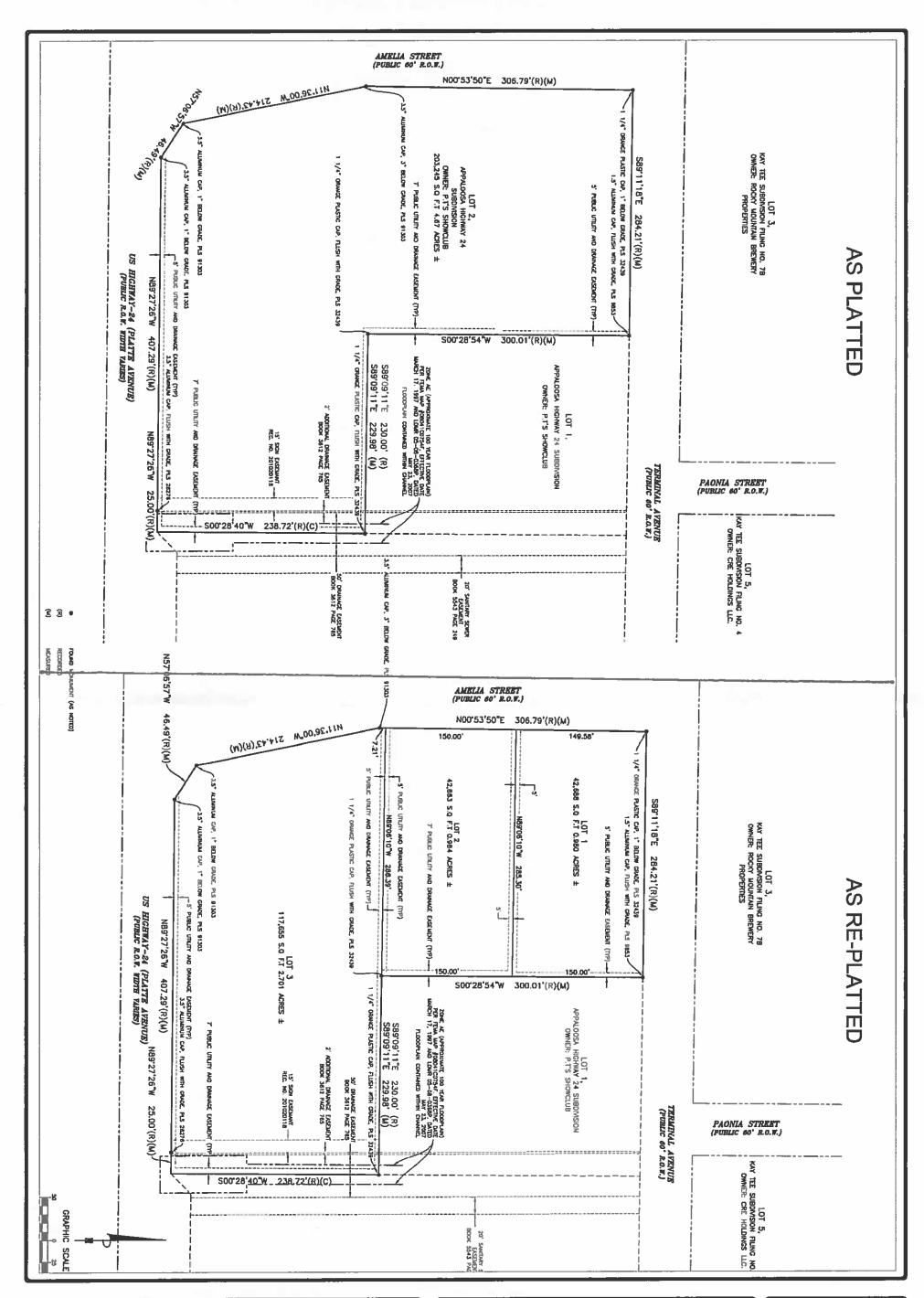
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- 1. Entech Engineering, Inc. May 30, 2007. Preliminary Subsurface Soil Investigation, Lot 2, Appaloosa Hwy 24 Subdivision Filing No. 2, El Paso County, Colorado. Entech Job No. 94727.
- 2. Natural Resources Conservation Service. September 23, 2016. *Web Soil Survey.* United States Department of Agriculture. http://websoilsurvey.sc.egov.usda.gov.
- 3. United States Department of Agriculture Soil Conservation Service. June, 1981. Soil Survey of El Paso County Area, Colorado.
- 4. Madole, R. F, Thorson, J. P. 2003. *Elsmere Quadranagle Geologic Map, El Paso County, Colorado. Colorado Geological Survey.* Open-File Report 02-2.
- 5. Federal Emergency Management Agency, March 17, 1997. Flood Insurance Rate Maps for the City of Colorado Springs, Colorado. Map Number 08041CO754F

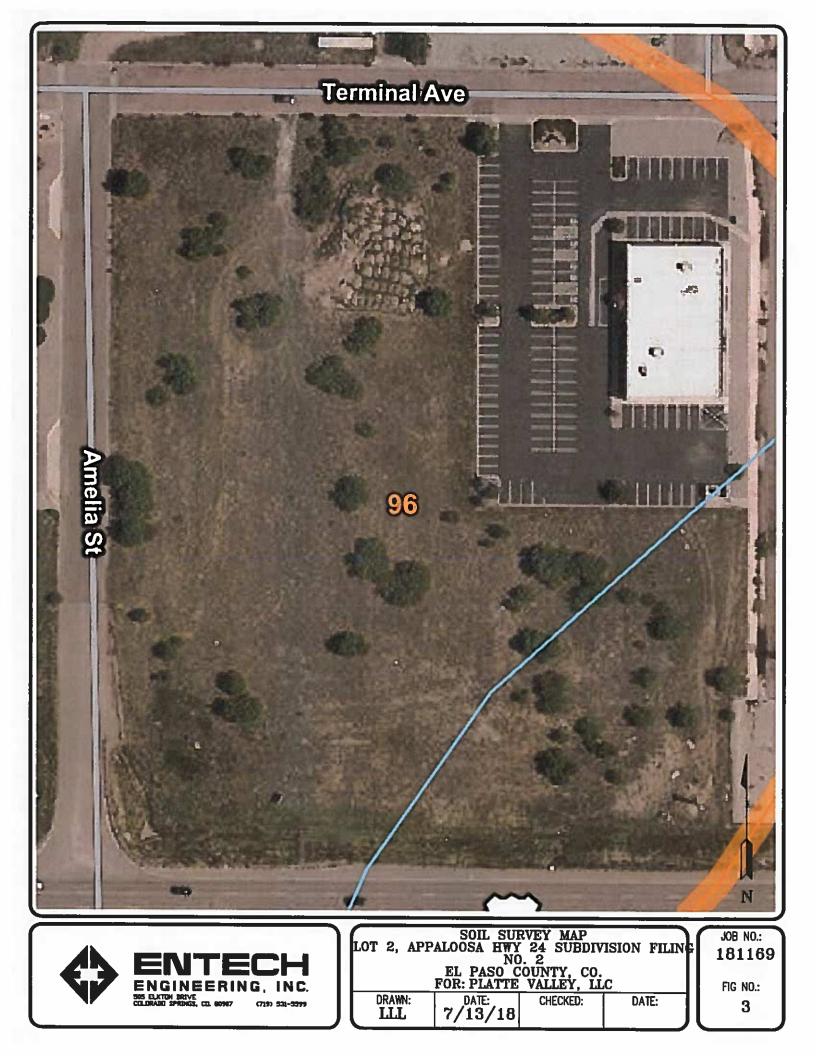
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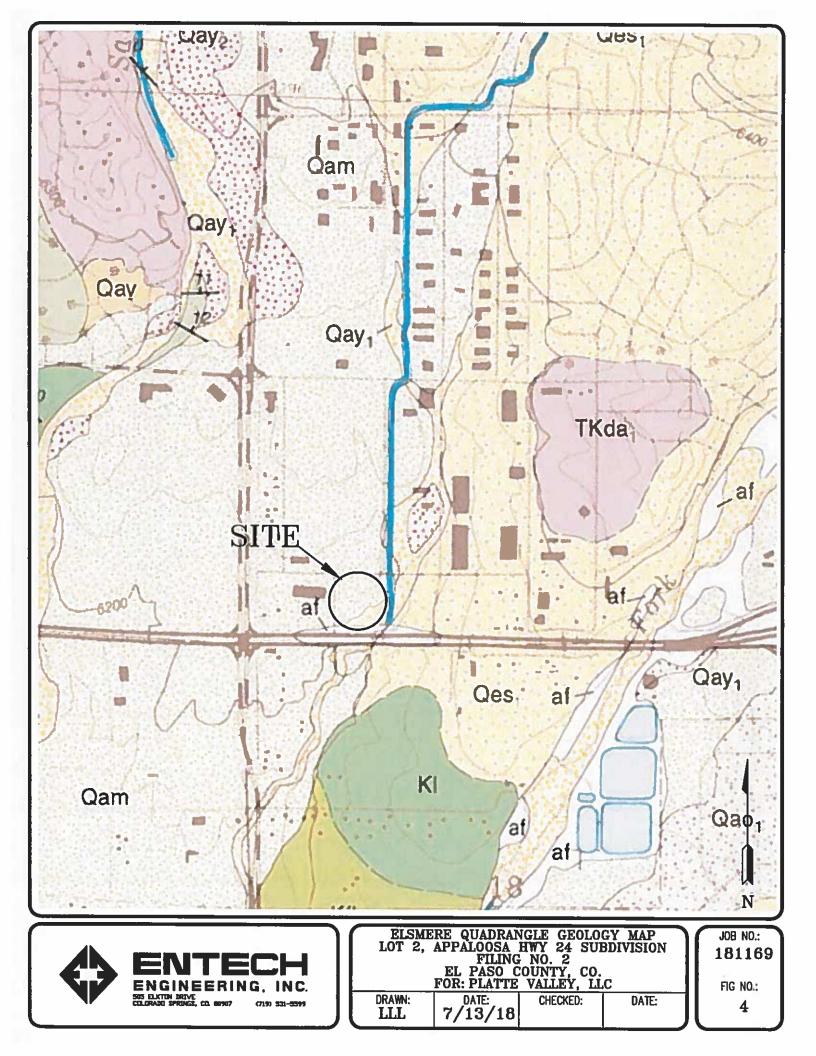
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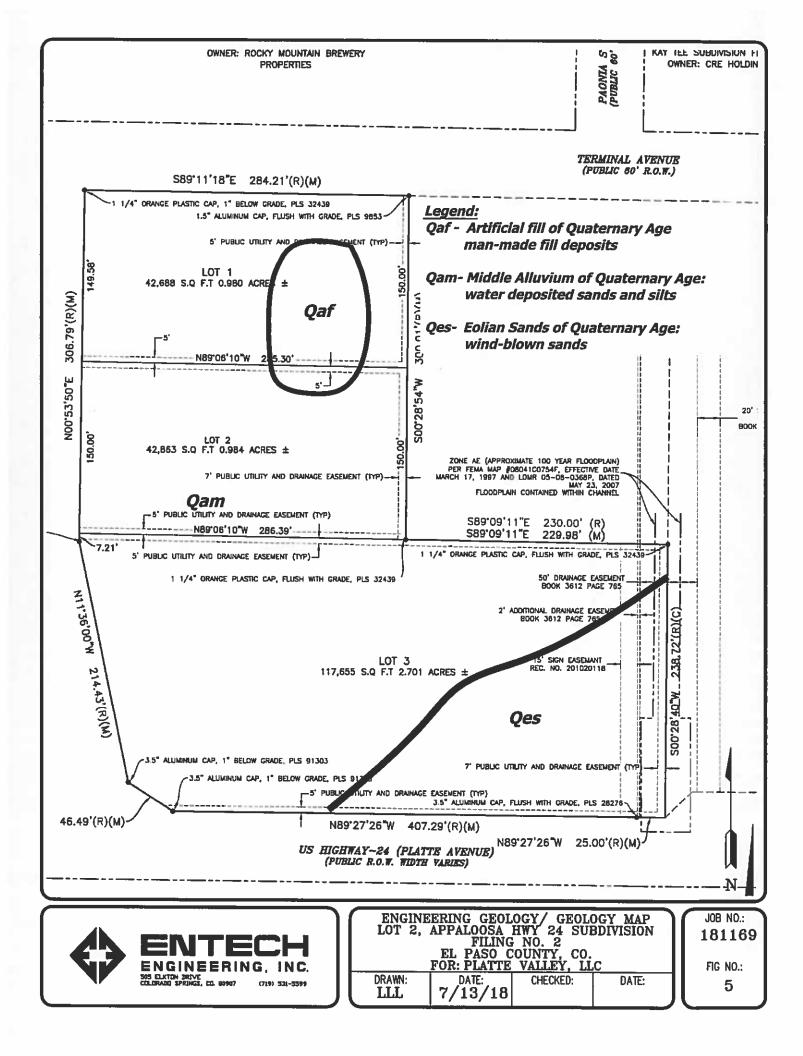


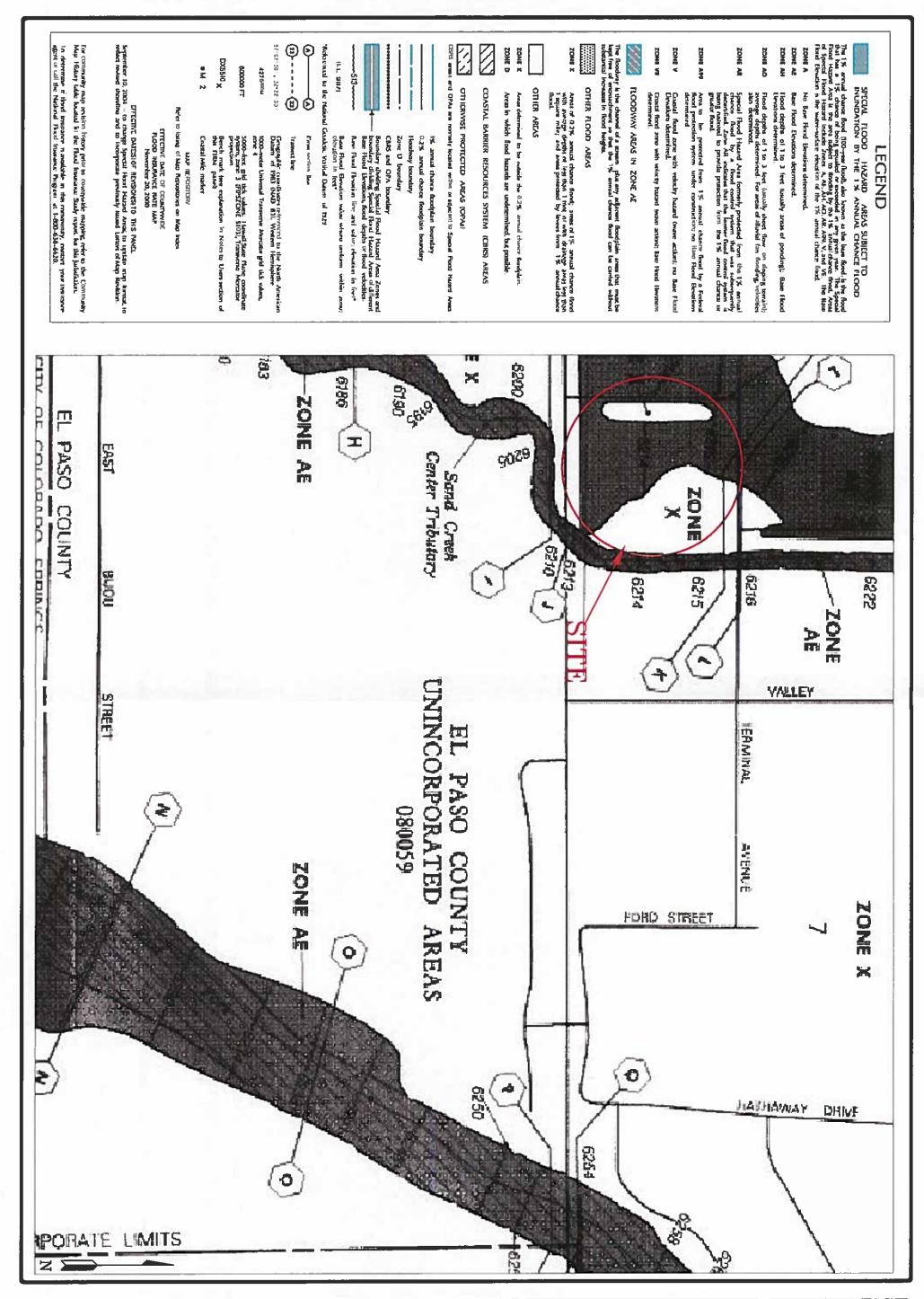






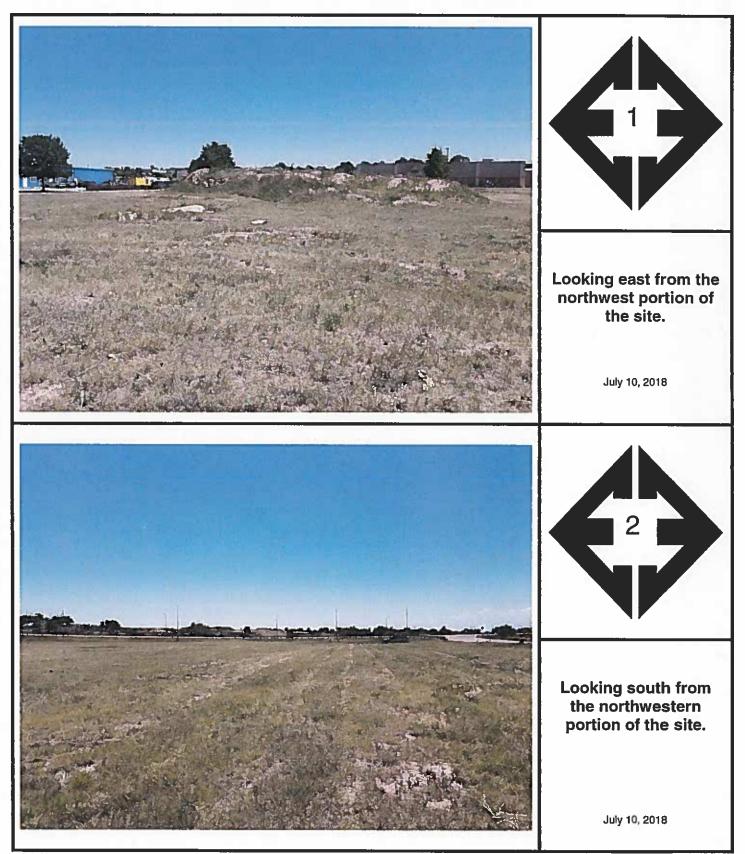








APPENDIX A: Site Photographs



Job No. 181169



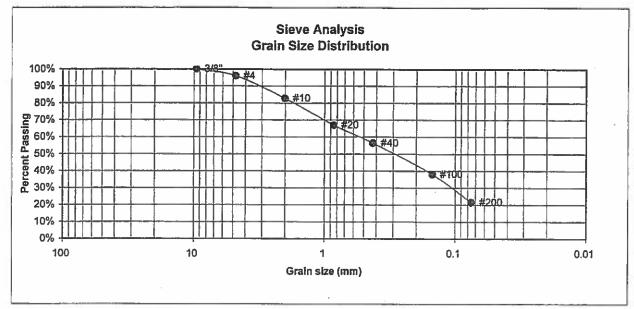
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APPENDIX B: Test Boring Logs and Laboratory Testing Results

TEST BORING NO. 1 DATE DRILLED 5/14/2007 Job # 94727 REMARKS		j×.	 1		I		TEST BORING NO. DATE DRILLED CLIENT LOCATION REMARKS	2 5/14/2007 MILLENN APPALOO	IUM E		& K	EY	TEE	
DRY TO 20', 5/15/07	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	WATER @ 20', 5/15		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
SAND, SILTY, FINE TO COARSE GRAINED, BROWN TO TAN, LOOSE TO MEDIUM DENSE, MOIST	5			10 9	8.9 7.0	2	POSS. FILL 0-4', SANI TO SILTY, BROWN, ME DENSE, MOIST CLAY, SANDY, DARK I FIRM, MOIST	DIUM	5			22 7	7.4 23.4	1 3
	10		1	16	4.1	2	SAND, GRAVELLY, SIL SLIGHTLY SILTY, FIN COARSE GRAINED, TA MEDIUM DENSE, MOIS	E TO N.	- 10	0.000000	潮	12	15.4	2
VERY SILTY, FINE GRAINED, LENSES	15 15 -			19	9.2	2	37		15			23	3.8	2
	20			21	14.7	2		<u> </u>	20			21	6.4	2
2													2	
14. 1	ŝ													
<i>6</i> .							14							
ENTECH ENGINEERING, 505 ELKTON DRIVE COLORADO SPRINGS, CC	INC	; DO 80	907		DRA	.wn:	TEST	BORING L		DATE: 25 U	7			JOB NO.: FIG NO.: B-1

TEST BORING NO. 3							TEST BORING NO. 4
DATE DRILLED 5/14/2007 Job # 94727							DATE DRILLED 5/15/2007 CLIENT MILLENNIUM DEV.
REMARKS		<u> </u>					LOCATION APPALOOSA HWY & KEY TEE REMARKS I
DRY TO 20', 5/14/07 CAVED TO 18', 5/15/07, DRY	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	Depth (ft) Symbol Samples Blows per foot Soil Type
POSS. FILL O-6', SAND, VERY SILTY, CLAYEY, FINE TO MEDIUM GRAINED, TAN, MEDIUM			護衛	12	15.1	1	SAND, SILTY, FINE TO MEDIUM GRAINED, BROWN TO TAN, MEDIUM DENSE, MOIST
DENSE, MOIST	5_			19	5.2	1	5 11 17.9 2
SAND, SILTY, FINE TO MEDIUM GRAINED, BROWN TO TAN, LOOSE, MOIST	-			-			
MOIST	10			7	13.1	2	SAND, SLIGHTLY SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE, MOIST
SAND, SLIGHTLY SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE, MOIST	15 <mark>-</mark>			10	3.4	2	
	20 _						20 20 3.2 2
							e 2,
9			24				
							TEST BORING LOG
ENGINEERING, 505 ELKTON DRIVE COLORADO SPRINGS, COL	LORAD	00 809	07		DRAV	WN:	DATE: CHECKED: STOATE: 07 B-2

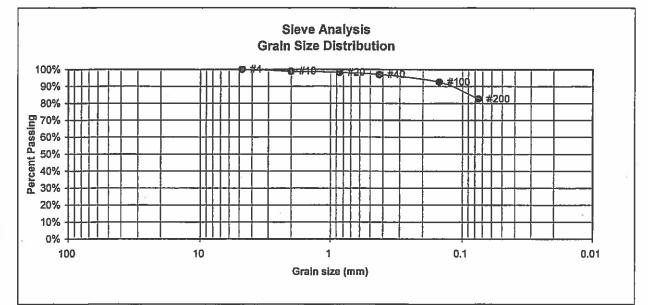
UNIFIED CLASSIFICA	<u>TION</u> SM	CLIENT	MILLENNIUM DEV.
SOIL TYPE #	2	PROJECT	APPALOOSA HWY & KEY TEE
TEST BORING #	1	JOB NO.	94727
DEPTH (FT)	5	TEST BY	DG



U.S. <u>Sieve #</u> 3"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Lim	it	5
1 1/2"		Liquid Limi		
3/4"		Plastic Inde		
1/2"			22	
3/8"	100.0%	6		
4	96.0%	Swell		
10	82.7%	Moisture at	l start	
20	67.0%	Moisture at	t finish	
40	56.3%	Moisture in	crease	
100	38.0%	Initial dry d	ensity (pcf)	
200	21.5%	Swell (psf)	* * *	

3	ENTECH ENGINEERING, INC.		LABORAT RESULTS	ORY TEST		ſ	JOB NO.: FIG NO.:
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE:	CHECKED:	5/25-67	J	B-3

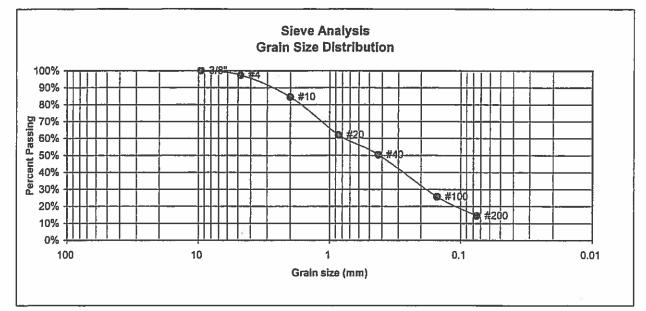
UNIFIED CLASSIFICA	TIONCL	CLIENT	MILLENNIUM DEV.
<u>SOIL TYPE #</u>	3	PROJECT	APPALOOSA HWY & KEY TEE
TEST BORING #	2	<u>JOB NO.</u>	94727
DEPTH (FT)	5	TEST BY	DG



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	2	21	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index	
4 10	100.0% 98.9%		×	<u>Swell</u> Moisture at start	15.9%
20 40	98.2% 97.0%			Moisture at finish Moisture increase	23.4% 7.5%
100 200	92.5% 82.6%			Initial dry density (pcf) Swell (psf)	96 752

$\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{$	ENTECH ENGINEERING, INC.		LABORAT RESULTS	ORY TEST			JOB NO.: FIG NO.:
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE	CHECKED:	5/25/07	J	B-4

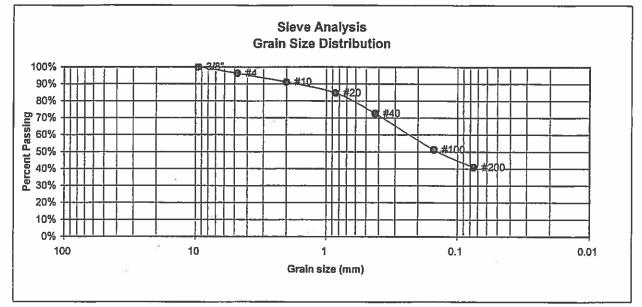
UNIFIED CLASSIFICA	TIONSM	CLIENT	MILLENNIUM DEV.
SOIL TYPE #	2	PROJECT	APPALOOSA HWY & KEY TEE
TEST BORING #	2	JOB NO.	94727
DEPTH (FT)	10	TEST BY	DG



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>		Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%		
4	97.5%		Swell
10 🐘	84.4%		Moisture at start
20	62.1%		Moisture at finish
40	50.3%		Moisture increase
100	25.6%		Initial dry density (pcf)
200	14.4%		Swell (psf)
	•	201	

\leftarrow	ENTECH ENGINEERING, INC.		LABORAT	ORY TEST			JOB NO.: FIG NO.:
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE:	CHECKED;	ST25107]	B-5

UNIFIED CLASSIFICA	TION SC-SM	CLIENT	MILLENNIUM DEV.
SOIL TYPE #	1	PROJECT	APPALOOSA HWY & KEY TEE
<u>TEST BORING #</u>	3	JOB NO.	94727
DEPTH (FT)	2-5	TEST BY	DG



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>			Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index	15 22 7	
1/2" 3/8"	100.0%					
4	96.3%	52		Swell		
10	91.1%			Moisture at start		7.3%
20	84.6%			Moisture at finish		17.2%
40	72.5%			Moisture increase		9.9%
100	51.1%			Initial dry density (pcf)	104
200	40.8%			Swell (psf)		61

	ENTECH ENGINEERING, INC.		LABORATORY TEST RESULTS				
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE:	CHECKED:	DATE DISTOT]	B-6

Markup Summary

dsdkendall (2)

Include this in submittal. restigated the above referenced und geologic hazards affecting reviously investigated by Entech 17 (Reference 1).

cre site. The site lies in El Paso

See ELICTON DRIVE OCCAMONS SPREAS, CO 20007 PAX (119) 511 0289 This report does not include a map

This report does not include a map Evaluatii showing the test hole locations 1 Filling I (Section 8.4.9.C.2.a.iii) including all other applicable requirements in Section 8.4.9.C.

ering, Inc. have investigated the above referenced pect to geology and geologic hazards affecting il conditions were previously investigated by Entech Subject: Callout Page Label: 1 Author: dsdkendall Date: 8/16/2018 3:49:34 PM Color:

Subject: Text Box Page Label: 1 Author: dsdkendall Date: 8/16/2018 9:49:37 AM Color:

Include this in submittal.

This report does not include a map showing the test hole locations (Section 8.4.9.C.2.a.iii) including all other applicable requirements in Section 8.4.9.C.