

Preliminary and Final Drainage Letter

7-Eleven Store #42975

SWC Bradley Road and Legacy Hill Drive
Colorado Springs, CO 80829
Lot 1, Waterview East Commercial Subdivision, Filing No. 1
Submitted: April 24, 2026

Prepared For:

LASCO Development

1207 Antoine Drive
Houston, TX 77055
Contact: Todd Womack
Phone: 713-961-0280

Prepared By:

K2 Civil Consultants Inc.

387 N Corona Street, Suite 605
Denver, CO 80218
Contact: Michael Drago, P.E.
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Email: mdrago@K2civilconsultants.com

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the city/county for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Michael Drago, P.E. 59225

Seal

DEVELOPER'S STATEMENT:

I, _____, the developer have read and will comply with all the requirements specified in this drainage report and plan.

Business Name
By: _____
Title: _____
Address: _____

EL PASO COUNTY ONLY:

Filed in accordance with Section 51.1 of the El Paso Land Development Code as amended.

Director of Public Works

Date

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I. General Location

The proposed 7-Eleven development is in El Paso County, Colorado bound to the north by Bradley Road, Legacy Hill Drive to the east, a private drive to the south, and Lot 2 of Waterview-East Commercial Subdivision Filing No. 1 to the west. The project falls within a part of the west ½ of Section 9, Township 15 South, Range 65 West of the 6th P.M., El Paso County, Colorado.

The proposed development is within Lot 1 Waterview-East Commercial Subdivision Filing No. 1, where an existing stormwater pond has been constructed to receive all flows from the project and surrounding developments. The existing pond is described further in the Drainage Facility Design Portion of this letter. The ultimate outflow for this pond is Jimmy Camp Creek.

The proposed developer falls within the Waterview-East Commercial Subdivision is surrounded by the Trails at Aspen Ridge to west and south, Bradley Road to the north, and S Powers Blvd to the east.

II. Description of Property

The proposed development is located within a 1.99 acre parcel currently comprised of native cover and freshly graded soil. There are little to no trees, shrubs, or vegetation on site due to overlot grading performed by the Master Development as part of the Waterview East Commercial Filing No. 1 infrastructure improvements. The site generally slopes from west to east/southeast. The average slopes on-site are 2-5% and all stormwater drains towards an existing inlet within the private drive to the south of the development.

Soil data for the development was obtained from the United States Department of Agriculture Natural Resources Conservation Service (NRCS) Web Soil Survey. Soil within the site is Nelson-Tassel fine sandy loams, 3 to 18 percent slopes, which falls into hydraulic soil group B.

III. Major Basin Descriptions

The proposed 7-Eleven lot has been analyzed within the “Final Drainage Report – Filing No. 1 Waterview East Commercial”, by Kimley Horn, dated March 2026, herein referred to as “Master Drainage Report”. Within the Master Drainage Report the proposed site is intended to drain into an extended detention pond to be built as part of the Waterview East Commercial Development, south of the 7-Eleven pad.

There are no recently reported cases of flooding within or around the development. The site is located within Zone “X”, areas determined to be outside 0.2% annual chance floodplain, of the Flood Plain, as designated on the Flood Insurance Rate Map (FIRM Map No. 08041C0768G last revised 12/07/2018. Refer to the Appendix for the FIRMette.

The existing Extended Detention Pond provided by the Waterview-East development is to capture 11.85 acres, detain, and treat all stormwater through the use of an extended detention basin. The basin is described within the report entitled, “Waterview East Commercial” by Kimley-Horn and Associates, Inc., project Number 196195001, prepared March 12, 2026.

There are no irrigation facilities or other obstructions that could influence or be influenced by the local drainage on-site.

IV. Sub-Basin Descriptions

The proposed 7-Eleven development falls under sub-basin A1 as described in the Master Drainage Report:

The on-site sub-basin A1 consists of native vegetation in the northeast portion of the site. Sub-Basin A1 encompasses Lot 1 which is to be developed into a future convenience store. Detailed drainage design of the convenience store and drainage basins will be provided with a separate Site Development Plan and Final Drainage Report submittal. The sub-basin has an area of 2.00 acres and a weighted imperviousness of 95%. Runoff in this basin will be collected by future proposed storm sewer infrastructure and routed to the 18” RCP storm sewer stub (Design Point A1) to discharge to the pond. Runoff during the 5-year and 100-year events are 5.92 cfs and 13.93 cfs respectively.

The proposed 7-Eleven development will be captured in 6 sub-basins. The proposed basins are listed below:

Basin A1.1

Basin A1.1 is 0.11 acres and consists of the proposed convenience store for an impervious value of 90.0%. The 10-year and 100-year runoff coefficients are 0.75 and 0.81, respectively. Runoff for the 10-year and 100-year is 0.44 cfs and 0.83 cfs, respectively. Runoff from the building is captured within roof drains and conveyed towards Design Point 1 via proposed private underground 6" PVC storm sewer. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

Basin A1.2

Basin A1.2 is 0.12 acres and consists of drive aisles and landscape area to the west of the proposed convenience store for an impervious value of 75.0%. The 10-year and 100-year runoff coefficients are 0.73 and 0.81, respectively. Runoff for the 10-year and 100-year is 0.46 cfs and 0.90 cfs, respectively. Runoff from the basin is captured within a 5' Type R Inlet on the far west side of the site. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

Basin A1.3

Basin A1.3 is 0.09 acres and consists of the proposed fueling canopy for an impervious value of 90.0%. The 10-year and 100-year runoff coefficients are 0.75 and 0.81, respectively. Runoff for the 10-year and 100-year is 0.36 cfs and 0.68 cfs, respectively. Runoff from the canopy is captured within roof drains and conveyed towards Design Point 2 via proposed private underground 8" PVC storm sewer. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

Basin A1.4

Basin A1.4 is 1.64 acres and consists of the majority of the drive aisles, sidewalks, and landscape area on-site for an impervious value of 57.3%. The 10-year and 100-year runoff coefficients are 0.59 and 0.70, respectively. Runoff for the 10-year and 100-year is 5.13 cfs and 10.70 cfs, respectively. Runoff from the basin will drain through proposed 2' curb cuts along the eastern drive aisles and into a 3' grass swale on the east side of the site. Flows are conveyed towards a proposed 30" Nyloplast Basin at the end of the grass swale. Curb cut and swale calculations are provided within the appendix of this letter. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

Basin A1.5

Basin A1.5 is 0.03 acres and consists of the existing public sidewalk on the SE side of the property for an impervious value of 33.3%. The 10-year and 100-year runoff coefficients are 0.41 and 0.55, respectively. Runoff for the 10-year and 100-year is 0.06 cfs and 0.15 cfs, respectively. Runoff from the basin flows into the private drive south of the site and into an existing 10' Type R inlet (Design Point PR1 per the Master Drainage Report) on the south side of the drive. These flows cannot be captured on-site due to existing public sidewalk draining away from the site. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

Basin A1.6

Basin A1.6 is 0.06 acres and consists of the proposed access points and public sidewalk along the private drive for an impervious value of 83.3%. The 10-year and 100-year runoff coefficients are 0.79 and 0.86, respectively. Runoff for the 10-year and 100-year is 0.25 cfs and 0.48 cfs, respectively. Runoff from the basin will be captured by an existing 10' Type Inlet (Design Point PR1 per the Master Drainage Report) on the south side of the private drive. Captured runoff will continue towards the private extended detention basin south of the site through underground storm sewer.

V. Drainage Design Criteria

The proposed development utilizes the El Paso County Drainage Criteria Manual, current version as of October 31, 2018.

The basin falls within the Waterview East Commercial Master Drainage Report. Per the comparison table provided below, the proposed site will not negatively affect any downstream structures as the impervious percent and flows is less than what is allowed per the approved Master Drainage Report.

	Acres	Impervious	C ₁₀₀	Q ₁₀₀ (cfs)
Per Master Drainage Report	2.00	95.0%	0.80	13.63
Proposed Development	2.05	62.0%	0.72	13.27
Difference	+0.05	-33.0%	-0.08	-0.36

As shown in the table above, there is a minor increase of area due to a portion of the improvements falling outside the A1 basin as described within the Master Drainage Report. Even with the additional area, the total flow generated from the development is less than what is allowed per the Master Drainage Report. Therefore, no negative downstream impacts are anticipated as a part of this development and no adjustments to existing storm sewer is required.

VI. Four Step Process

Step 1. Employ Runoff Reduction Practices

The proposed development will include the design and construction of a 3' grass swale that will capture a majority of the runoff generated on-site and provide runoff reduction. This will reduce the runoff flowing off-site as throughout the grass swale stormwater will slow down and infiltrate into the soil.

Step 2. Provide a Water Quality Capture Volume

Permanent Water Quality Capture Volume will be provided by the existing extended detention pond built as part of the Waterview East Commercial development. Refer to the Master Drainage Report for additional information.

Step 3. Stabilize Drainageways

The proposed site is approximately a mile from Jimmy Camp Creek, there will be no open channels on or adjacent to the site, therefore no stabilization will be necessary as a part of this development.

Step 4. Implement Site Specific and Other Source Control BMPs

Erosion Control construction and design of BMPs are proposed as a part of this development and shown within the GEC drawings. BMPs include vehicle tracking control, stockpile management, silt fence, stabilized staging areas, and inlet protection.

VII. Hydrologic Criteria

The design rainfall source for this project is the NOAA Atlas 14, one hour point rainfall data. The minor storm, 10-year rainfall value is 1.56 inches. The major storm, 100-year rainfall value is 2.75 inches.

The analysis and design of the Stormwater management system for this project was prepared in accordance with the criteria set forth by the El Paso County Drainage Criteria Manual (hereafter referred to as the DCM) and the Mile High Flood District (MHFD). The Rational Method was used to calculate runoff from the 10-year minor, and 100-year major design storm recurrence intervals. Peak runoff values were calculated using the rational method:

$Q = CIA$, where

Q = Storm runoff in cubic feet per second (cfs)

C = Rainfall coefficients – ratio runoff to rainfall

I = Rainfall intensity in inches per hour

A = Drainage area in acres

Table 6-6 of the El Paso County Drainage Criteria Manual was used for runoff coefficients.

The proposed storm sewers were modeled, and hydraulic grade lines generated, using Bentley StormCAD and FlowMaster software's. The user-defined design inputs for the software include peak flow runoff, pipe diameter, pipe slope & length, pipe material coefficient, and tailwater. For the onsite storm sewer system, the tailwater input was based on free outfall conditions. The hydraulic grade and energy lines have been designed to maintain a minimum of one foot below the final grade.

Inlet capacities calculations were based on utilizing the Mile High Flood District spreadsheet "MHFD_v6.0".

VIII. Conclusion

As described above, the proposed development will not negatively impact the existing stormwater pond built as part of the Waterview East Commercial Development. The proposed site reduces the impervious %, C value, and total flows draining into the basin. Refer to the Master Drainage Report for full details and information on the proposed pond.

IX. References

1. County of El Paso Drainage Criteria Manual, Volume 1, October 2018.
2. County of El Paso Drainage Criteria Manual Volume 2, October 2018.
3. NRCS Soil Survey for El Paso County, Colorado, dated February 2026.
4. Waterview East Commercial Final Drainage Report, Filing No. 1, prepared by Kimley-Horn and Associates, Inc., dated March 12, 2026.

APPENDIX A
General Site Information

1. FEMA Firm Map
2. NRCS Soil Report
3. Runoff C – Table 6-6
4. NOAA Rainfall Information

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services
 NOAA, NINGS12
 National Geodetic Survey
 SSMC-3, #9202
 1315 East-West Highway
 Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

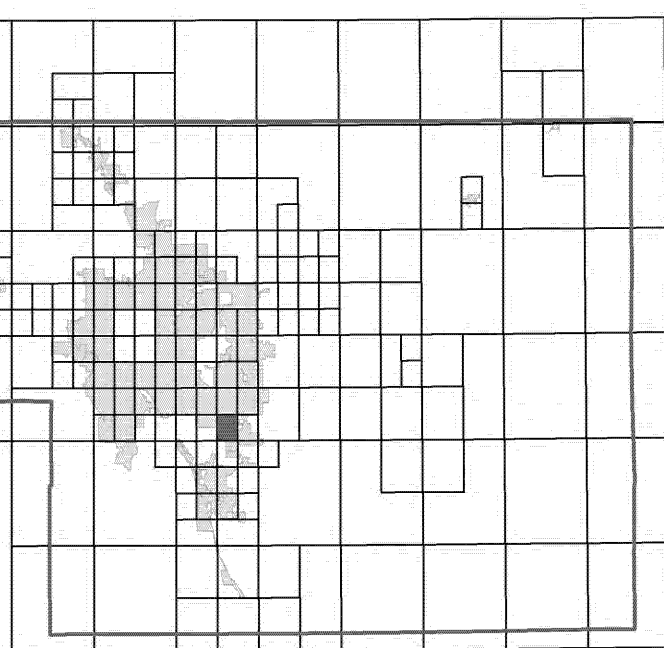
Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FIMX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov/>.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfp>.

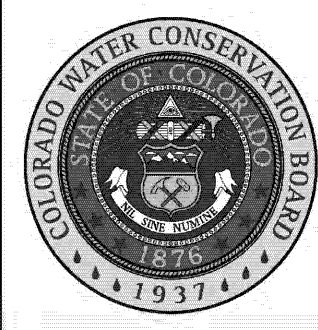
El Paso County Vertical Datum Offset Table

Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	

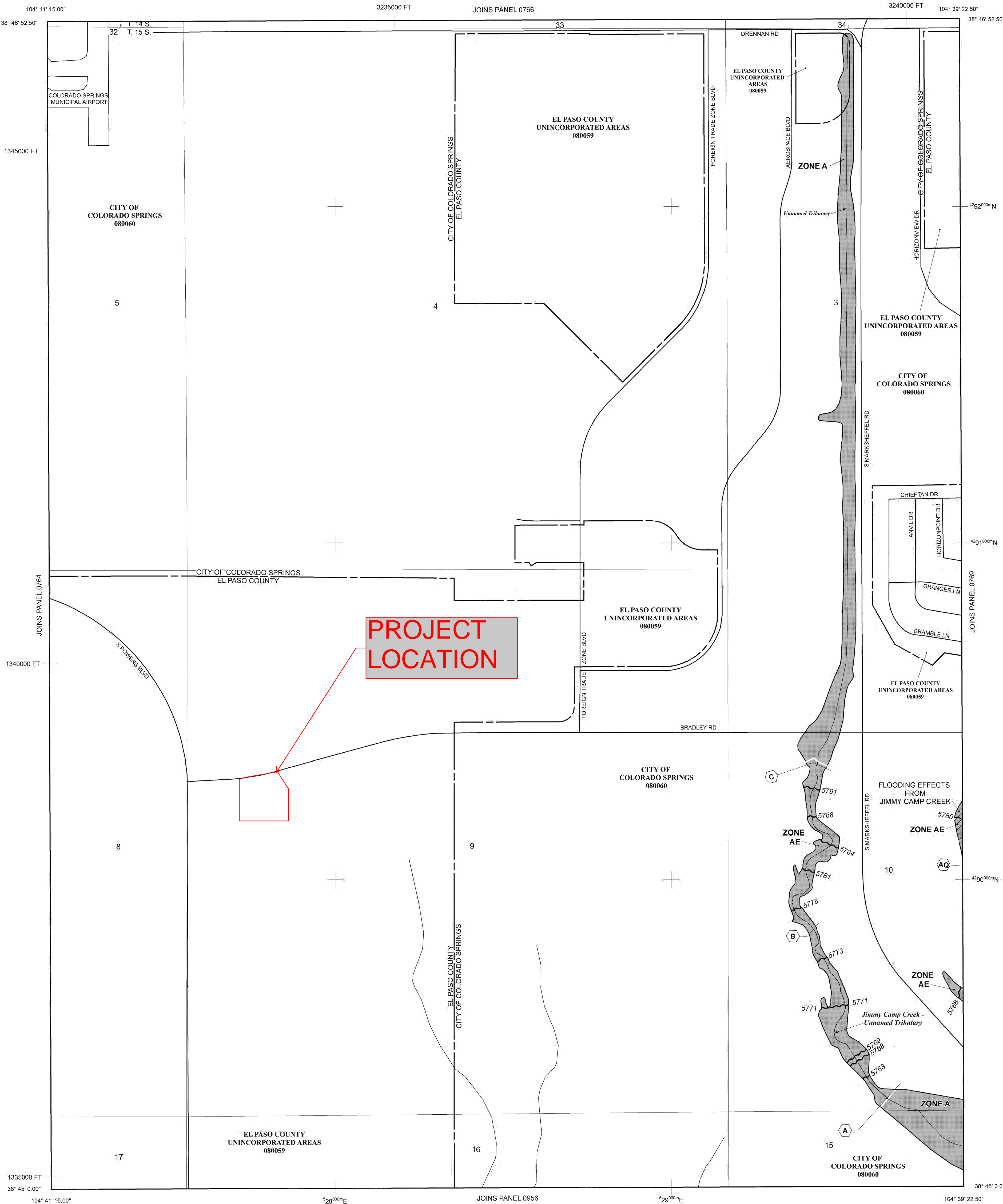
Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 14 SOUTH, RANGE 65 WEST, AND TOWNSHIP 15 SOUTH, RANGE 65 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equalled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decreedified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE
 The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot, or with drainage areas less than 1 square mile, and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D Boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

513 Base Flood Elevation line and value; elevation in feet*
 (EL 987) Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A-A Cross section line
23-23 Transsect line

97° 07' 30.00" 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)

4725000N 1000-meter Universal Transverse Mercator grid ticks, zone 13

6000000 FT 5000-foot grid ticks; Colorado State Plane coordinate system, central zone (FIPSZONE 0902), Lambert Conformal Conic Projection

DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel)

M1.5 River Mile

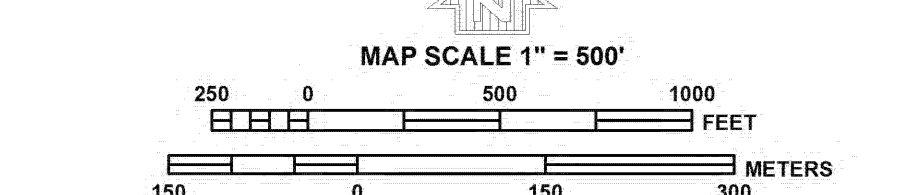
MAP REPOSITORIES
 Refer to Map Repository list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
 MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
 DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.



PANEL 0768G

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 768 OF 1300
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

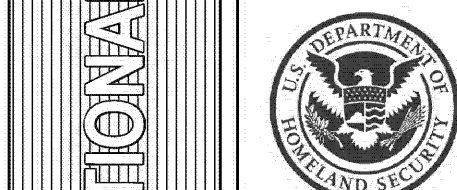
CONTAINS	COMMUNITY	NUMBER	PANEL	SUFFIX
	COLORADO SPRINGS, CITY OF	080060	0768	G
	EL PASO COUNTY	080059	0768	G

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0768G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency





United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for El Paso County Area, Colorado



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

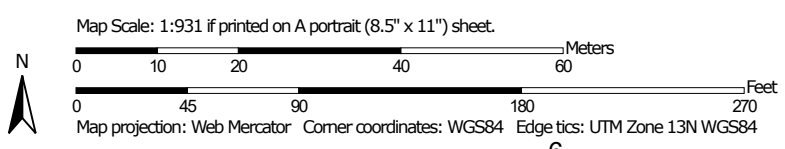
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Soil Map


The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map




MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)


Soils


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit

 Gravelly Spot


 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole


 Slide or Slip


 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 23, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 23, 2024—Aug 4, 2024

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
56	Nelson-Tassel fine sandy loams, 3 to 18 percent slopes	2.5	100.0%
Totals for Area of Interest		2.5	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Custom Soil Resource Report

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

56—Nelson-Tassel fine sandy loams, 3 to 18 percent slopes

Map Unit Setting

National map unit symbol: 3690
Elevation: 5,600 to 6,400 feet
Mean annual precipitation: 12 to 14 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Nelson and similar soils: 55 percent
Tassel and similar soils: 40 percent
Minor components: 5 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Nelson

Setting

Landform: Upland hills
Landform position (three-dimensional): Crest, side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Calcareous residuum weathered from interbedded sedimentary rock

Typical profile

A - 0 to 5 inches: fine sandy loam
Ck - 5 to 23 inches: fine sandy loam
Cr - 23 to 27 inches: weathered bedrock

Properties and qualities

Slope: 3 to 12 percent
Depth to restrictive feature: 20 to 40 inches to paralithic bedrock
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.06 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 10 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Very low (about 2.8 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: R067BY045CO - Shaly Plains
Other vegetative classification: SHALY PLAINS (069AY046CO)
Hydric soil rating: No

Description of Tassel

Setting

Landform: Upland hills

Landform position (three-dimensional): Crest, side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Calcareous slope alluvium over residuum weathered from sandstone

Typical profile

A - 0 to 4 inches: fine sandy loam

C - 4 to 10 inches: fine sandy loam

Cr - 10 to 14 inches: weathered bedrock

Properties and qualities

Slope: 3 to 18 percent

Depth to restrictive feature: 6 to 20 inches to paralithic bedrock

Drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 10 percent

Available water supply, 0 to 60 inches: Very low (about 1.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: R067BY045CO - Shaly Plains

Other vegetative classification: SHALY PLAINS (069AY046CO)

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: 4 percent

Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent

Landform: Depressions

Hydric soil rating: Yes

3.1. - Rational Method Runoff Coefficient (C)

The runoff coefficient represents the integrated effects of infiltration, detention storage, evaporation, retention, flow routing, and interception, all of which affect the time distribution and peak rate of runoff. Runoff coefficients are based on the imperviousness of a particular land use and the hydrologic soil type of the area and are to be selected in accordance with Table 6-6.

The procedure for determining the runoff coefficient includes these steps:

1. Categorize the site area into one or more similar land uses, each with a representative imperviousness, according to the information in Table 6-6.
2. Based on the dominant hydrologic soil type in the area, use Table 6-6 to estimate the runoff coefficient for the particular land use category for the design storms of interest.
3. Calculate an area-weighted average runoff coefficient for the site based on the runoff coefficients from individual land use areas of the site.

When analyzing an area for design purposes, urbanization of the full watershed, including both on-site and off-site areas, shall be assumed.

Gravel parking areas, storage areas, and access drives proposed on Site Improvement Plans shall be analyzed based on an imperviousness of 80%. This is due to the potential for gravel areas being paved over time by property owners and the resulting adverse impacts on the stormwater management facilities and adjacent properties.

There are some circumstances where the selection of impervious percentage values may require additional investigation due to unique land characteristics (e.g., recent burn areas). When these circumstances arise, it is the designer's responsibility to verify that the correct land use assumptions are made.

When multiple sub-basins are delineated, the composite C value calculation is:

$$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + \dots C_i A_i) / A_t \quad (\text{Eq. 6-6})$$

Where:

C_c = composite runoff coefficient for total area

C_i = runoff coefficient for subarea corresponding to surface type or land use

A_i = area of surface type corresponding to C_i (units must be the same as those used for total area)

A_t = total area of all subareas for which composite runoff coefficient applies

i = number of surface types in the drainage area

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients					
		2-year	5-year	10-year	25-year	50-year	100-year

		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													

Historic Flow Analysis— Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50



NOAA Atlas 14, Volume 8, Version 2
Location name: Colorado Springs, Colorado, USA*
Latitude: 38.7606°, Longitude: -104.6804°
Elevation: 5929 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

PF tabular

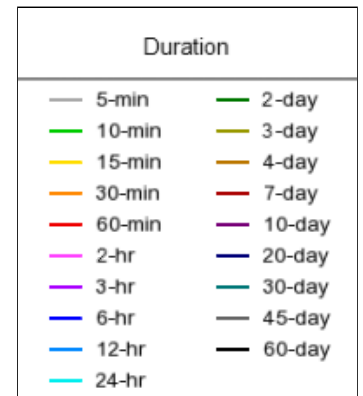
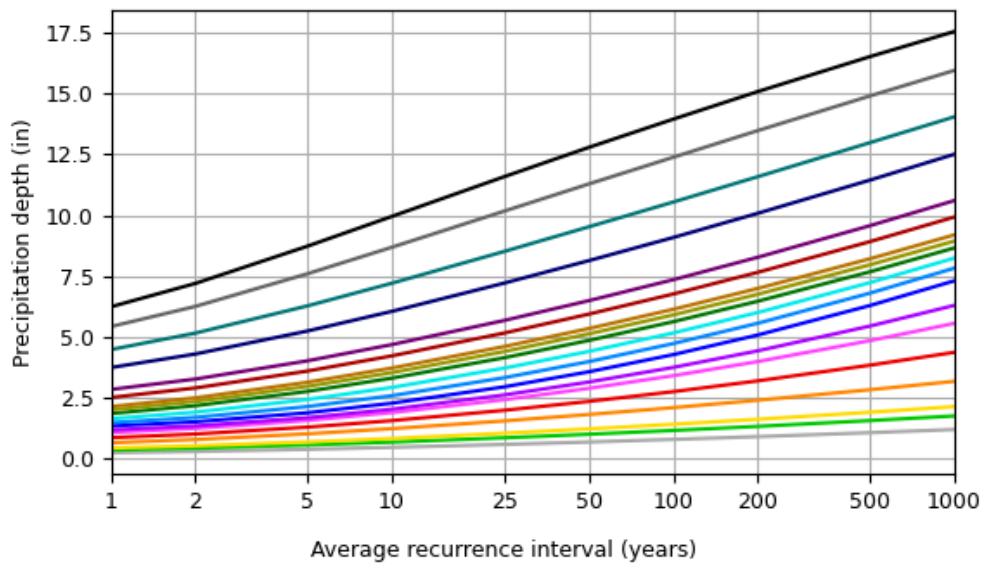
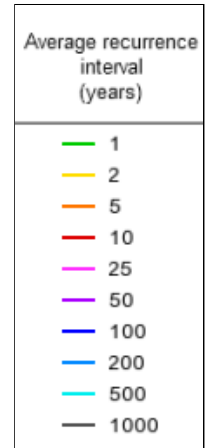
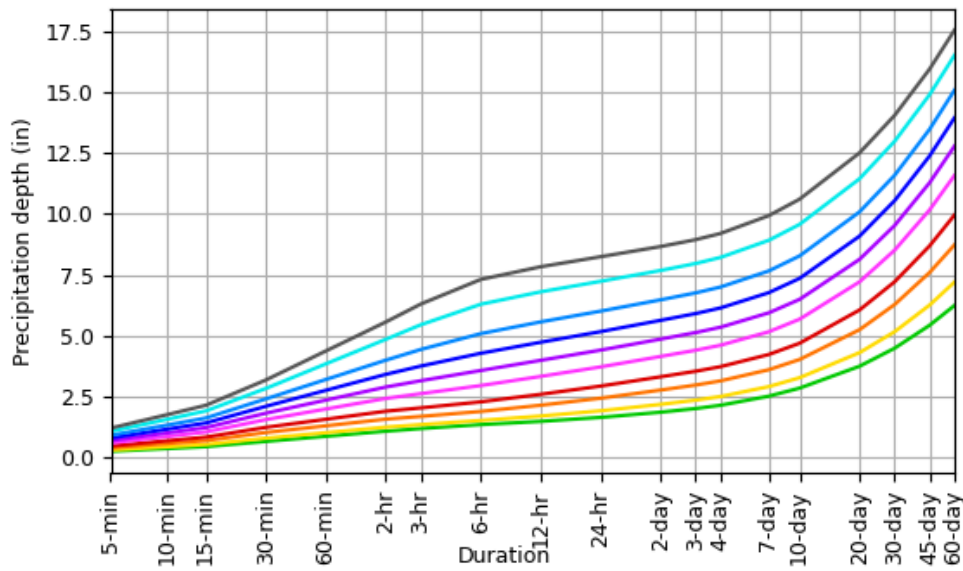
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.245 (0.204-0.298)	0.295 (0.246-0.359)	0.384 (0.319-0.469)	0.464 (0.383-0.570)	0.584 (0.467-0.753)	0.684 (0.531-0.892)	0.791 (0.589-1.06)	0.905 (0.643-1.25)	1.07 (0.726-1.51)	1.20 (0.788-1.71)
10-min	0.359 (0.299-0.437)	0.433 (0.360-0.526)	0.563 (0.467-0.687)	0.680 (0.560-0.834)	0.856 (0.684-1.10)	1.00 (0.777-1.31)	1.16 (0.863-1.55)	1.33 (0.942-1.82)	1.56 (1.06-2.21)	1.75 (1.15-2.51)
15-min	0.438 (0.365-0.532)	0.528 (0.439-0.642)	0.686 (0.569-0.838)	0.829 (0.683-1.02)	1.04 (0.834-1.34)	1.22 (0.948-1.59)	1.41 (1.05-1.89)	1.62 (1.15-2.23)	1.91 (1.30-2.70)	2.14 (1.41-3.06)
30-min	0.654 (0.545-0.795)	0.786 (0.654-0.957)	1.02 (0.846-1.24)	1.23 (1.02-1.51)	1.55 (1.24-2.00)	1.81 (1.41-2.36)	2.10 (1.56-2.80)	2.40 (1.71-3.30)	2.83 (1.92-4.01)	3.18 (2.09-4.54)
60-min	0.863 (0.719-1.05)	1.01 (0.843-1.23)	1.29 (1.07-1.58)	1.56 (1.29-1.92)	1.98 (1.60-2.58)	2.35 (1.83-3.08)	2.75 (2.06-3.70)	3.20 (2.28-4.42)	3.84 (2.62-5.46)	4.37 (2.87-6.24)
2-hr	1.07 (0.900-1.30)	1.24 (1.04-1.50)	1.57 (1.31-1.90)	1.89 (1.57-2.31)	2.42 (1.97-3.15)	2.89 (2.27-3.78)	3.41 (2.57-4.57)	3.99 (2.87-5.50)	4.84 (3.34-6.86)	5.56 (3.68-7.89)
3-hr	1.18 (0.995-1.42)	1.34 (1.13-1.62)	1.68 (1.41-2.03)	2.03 (1.69-2.46)	2.61 (2.15-3.41)	3.14 (2.49-4.12)	3.75 (2.85-5.03)	4.43 (3.21-6.11)	5.44 (3.77-7.70)	6.30 (4.19-8.91)
6-hr	1.34 (1.14-1.61)	1.51 (1.28-1.81)	1.88 (1.59-2.26)	2.28 (1.91-2.75)	2.95 (2.45-3.84)	3.56 (2.85-4.66)	4.27 (3.28-5.72)	5.08 (3.72-6.97)	6.29 (4.39-8.85)	7.30 (4.90-10.3)
12-hr	1.48 (1.26-1.75)	1.70 (1.45-2.02)	2.14 (1.82-2.55)	2.59 (2.19-3.10)	3.32 (2.77-4.26)	3.98 (3.20-5.14)	4.72 (3.65-6.25)	5.56 (4.09-7.56)	6.80 (4.78-9.48)	7.82 (5.30-10.9)
24-hr	1.64 (1.41-1.93)	1.91 (1.64-2.26)	2.43 (2.08-2.88)	2.92 (2.49-3.48)	3.71 (3.10-4.70)	4.40 (3.56-5.62)	5.16 (4.00-6.75)	6.00 (4.44-8.07)	7.23 (5.11-9.98)	8.24 (5.62-11.4)
2-day	1.85 (1.61-2.17)	2.17 (1.89-2.55)	2.76 (2.39-3.25)	3.31 (2.84-3.91)	4.14 (3.46-5.17)	4.85 (3.94-6.12)	5.62 (4.39-7.28)	6.47 (4.81-8.60)	7.67 (5.46-10.5)	8.66 (5.96-11.9)
3-day	2.00 (1.75-2.34)	2.35 (2.04-2.74)	2.97 (2.58-3.48)	3.53 (3.05-4.16)	4.40 (3.69-5.45)	5.12 (4.17-6.42)	5.90 (4.62-7.59)	6.75 (5.05-8.93)	7.96 (5.69-10.8)	8.94 (6.18-12.3)
4-day	2.14 (1.87-2.49)	2.50 (2.18-2.91)	3.14 (2.73-3.67)	3.72 (3.22-4.37)	4.60 (3.87-5.68)	5.34 (4.36-6.67)	6.13 (4.82-7.86)	6.99 (5.24-9.22)	8.21 (5.89-11.1)	9.19 (6.38-12.6)
7-day	2.51 (2.21-2.91)	2.90 (2.55-3.36)	3.60 (3.15-4.18)	4.22 (3.68-4.93)	5.16 (4.36-6.31)	5.93 (4.87-7.35)	6.76 (5.34-8.60)	7.65 (5.77-10.0)	8.91 (6.43-12.0)	9.92 (6.94-13.5)
10-day	2.84 (2.51-3.28)	3.27 (2.89-3.78)	4.02 (3.54-4.65)	4.68 (4.10-5.45)	5.67 (4.81-6.90)	6.49 (5.35-8.00)	7.35 (5.83-9.30)	8.27 (6.26-10.8)	9.56 (6.93-12.8)	10.6 (7.44-14.4)
20-day	3.74 (3.33-4.29)	4.30 (3.83-4.93)	5.24 (4.65-6.03)	6.05 (5.33-7.00)	7.21 (6.13-8.65)	8.13 (6.74-9.91)	9.08 (7.24-11.4)	10.1 (7.68-13.0)	11.4 (8.34-15.2)	12.5 (8.85-16.8)
30-day	4.48 (4.01-5.12)	5.16 (4.61-5.89)	6.27 (5.59-7.19)	7.21 (6.38-8.30)	8.50 (7.25-10.1)	9.52 (7.91-11.5)	10.5 (8.43-13.1)	11.6 (8.85-14.8)	13.0 (9.50-17.1)	14.0 (9.99-18.8)
45-day	5.42 (4.88-6.17)	6.25 (5.62-7.12)	7.59 (6.80-8.66)	8.68 (7.72-9.96)	10.2 (8.68-12.0)	11.3 (9.40-13.5)	12.4 (9.94-15.3)	13.5 (10.3-17.1)	14.9 (10.9-19.5)	15.9 (11.4-21.3)
60-day	6.23 (5.63-7.07)	7.20 (6.49-8.17)	8.72 (7.84-9.93)	9.95 (8.88-11.4)	11.6 (9.90-13.6)	12.8 (10.7-15.3)	13.9 (11.2-17.1)	15.1 (11.6-19.1)	16.5 (12.2-21.5)	17.5 (12.6-23.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves
 Latitude: 38.7606°, Longitude: -104.6804°



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Maps & aerials

Small scale terrain



Large scale terrain



Large scale map



Large scale aerial



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Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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APPENDIX B

Hydrology

1. Master Report Drainage Subbasin Information
2. Developed Drainage Subbasin Information

SUMMARY - PROPOSED RUNOFF TABLE						
DESIGN POINT	BASIN DESIGNATION	BASIN AREA (ACRES)	DIRECT 5-YR RUNOFF (CFS)	DIRECT 100-YR RUNOFF (CFS)	CUMMULATIVE 5-YR RUNOFF (CFS)	CUMMULATIVE 100-YR RUNOFF (CFS)
A1	A1	2.00	5.92	13.63	5.92	13.63
A2	A2	1.50	4.48	10.32	4.48	10.32
FD 1	FD 1	2.91	8.41	19.39	8.41	19.39
FD 2	FD 2	3.19	9.17	21.14	9.17	21.14
PR 1	PR 1	0.26	1.02	2.30	1.02	2.30
PR 2	PR 2	0.61	2.38	5.37	2.38	5.37
P	P	1.38	0.82	5.22	31.48	48.16
FS1	UD 1	0.70	0.77	4.63	0.77	4.63
FS1	UD 2	0.03	0.13	0.17	0.13	0.17
FS 1	FS 1	0.31	1.23	2.76	1.23	2.76
FS 2	FS 2	0.29	1.15	2.58	1.15	2.58

DEVELOPED CONDITIONS



Runoff Coefficients

Project:	7-Eleven - Bradley and Legacy Ridge	City/State:	El Paso County, CO
Designed by:	MRD	Date:	4/14/2026
Reviewed by:	KMH	Date:	4/14/2026

Soil Type: B

Sub-Basin Data						Sub-Area (Roof)				Sub-Area (Drives/Walks)				Sub-Area (Landscape)			
Basin ID	Description	Area (ac)	C ₁₀	C ₁₀₀	(%)	C ₁₀	C ₁₀₀	(%)	(ac)	C ₁₀	C ₁₀₀	(%)	(ac)	C ₁₀	C ₁₀₀	(%)	(ac)
Developed Onsite																	
A1.1	PROPOSED C-STORE	0.11	0.75	0.81	90.0	0.75	0.81	90	0.11	0.92	0.96	100	0.00	0.15	0.35	0	0.00
A1.2	SW CORNER OF SITE	0.12	0.73	0.81	75.0	0.75	0.81	90	0.00	0.92	0.96	100	0.09	0.15	0.35	0	0.03
A1.3	FUELING CANOPY	0.09	0.75	0.81	90.0	0.75	0.81	90	0.09	0.92	0.96	100	0.00	0.15	0.35	0	0.00
A1.4	DRIVES, SIDEWALKS, AND LANDSCAPE AREAS	1.64	0.59	0.70	57.3	0.75	0.81	90	0.00	0.92	0.96	100	0.94	0.15	0.35	0	0.70
A1.5	PUBLIC SIDEWALK AND LANDSCAPE AREAS	0.03	0.41	0.55	33.3	0.75	0.81	90	0.00	0.92	0.96	100	0.01	0.15	0.35	0	0.02
A1.6	ACCESS POINTS AND PUBLIC SIDEWALK	0.06	0.79	0.86	83.3	0.75	0.81	90	0.00	0.92	0.96	100	0.05	0.15	0.35	0	0.01
Composite	Developed Site Composite	2.05	0.62	0.72	62.0	0.75	0.81	90	0.20	0.92	0.96	100	1.09	0.15	0.35	0	0.76

DEVELOPED CONDITIONS



Time of Concentration

Project:	7-Eleven - Bradley and Legacy Ridge	City/State:	El Paso County, CO
Designed by:	MRD	Date:	4/14/2026
Reviewed by:	KMH	Date:	4/14/2026

Urban TOC _{min} =	5 min
Rural TOC _{min} =	10 min

SUB-BASIN DATA				INITIAL/OVERLAND FLOW						TRAVEL TIME					Tc CHECK				FINAL Tc
Basin ID	Description	C ₁₀	Area (ac)	(t _o)			(t _t)					TOTAL	(Urbanized basins)				(min)		
				Length (ft)	Slope (ft/ft)	t _o (min)	Length (ft)	S _w (ft/ft)	Code	Description	Convey Coef (C _v)		Velocity	t _t Travel Time (min)	t _c = t _t + t _o (min)	Urban (Yes)		Length (ft)	T _c max (min)
Developed Onsite																			
A1.1	PROPOSED C-STORE	0.75	0.11	100	0.02	1.80	0	0.00	6	Paved areas and shallow paved swales	20.00	0.00	0.00	1.80	YES	100.00	10.56	Regional Tc	5.00
A1.2	SW CORNER OF SITE	0.73	0.12	70	0.02	1.46	0	0.00	6	Paved areas and shallow paved swales	20.00	0.00	0.00	1.46	YES	70.00	10.39	Regional Tc	5.00
A1.3	FUELING CANOPY	0.75	0.09	35	0.02	1.06	0	0.00	6	Paved areas and shallow paved swales	20.00	0.00	0.00	1.06	YES	35.00	10.19	Regional Tc	5.00
A1.4	DRIVES, SIDEWALKS, AND LANDSCAPE AREAS	0.59	1.64	150	0.02	3.20	175	0.02	5	Grassed waterway	15.00	2.12	1.37	4.58	YES	325.00	11.81	Regional Tc	5.00
A1.5	PUBLIC SIDEWALK AND LANDSCAPE AREAS	0.41	0.03	25	0.02	1.78	0	0.00	5	Grassed waterway	15.00	0.00	0.00	1.78	YES	25.00	10.14	Regional Tc	5.00
A1.6	ACCESS POINTS AND PUBLIC SIDEWALK	0.79	0.06	70	0.01	1.06	0	0.00	6	Paved areas and shallow paved swales	20.00	0.00	0.00	1.06	YES	70.00	10.39	Regional Tc	5.00

Notes:

t_i = (1.87*(1.1-C10)*(L^0.5))/(S^0.33), from El Paso County Drainage Manual

Velocity from V = C_v*S_w^0.5, from UDFCD Eqn 6-4, C_v from Table 6-2(See Sheet Design Info)

t_i = L/60V

t_i max = 10+L/180

Final Tc > 10 min for nonurban watersheds

Code	Type of Land Surface	Conveyance Factor, K
1	Heavy meadow	2.5
2	Tillage/field	5
3	Short pasture and lawns	7
4	Nearly bare ground	10
5	Grassed waterway	15
6	Paved areas and shallow paved swales	20

DEVELOPED CONDITIONS



Runoff (Q)

Project: 7-Eleven - Bradley and Legacy Ridge City/State: El Paso County, CO
 Designed by: MRD Date: 4/14/2026
 Reviewed by: KMH Date: 4/14/2026

10 YEAR MINOR STORM

Design Storm: 10-yr P = 1.56 in

a	b	c
28.5	10	0.786

LOCATION	BASIN DESCRIPTION	DIRECT RUNOFF								TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME		
		BASIN NAME	AREA (AC)	RUNOFF COEFF (10 Yr)	t _c (MIN)	C.A. (AC)	I IN / HR	Q (CFS)	t _c (MIN)	SUM (C*A) (AC)	I (IN / HR)	Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	DESIGN FLOW (CFS)	SLOPE (%)	PIPE SIZE (in)	LENGTH (FT)	VELOCITY (FPS)	t _c (MIN)	
DP	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)
DEVELOPED																					
	A1.1	PROPOSED C-STORE	A1.1	0.11	0.75	5.00	0.08	5.29	0.44												
	A1.2	SW CORNER OF SITE	A1.2	0.12	0.73	5.00	0.09	5.29	0.46												
1	A1.1+A1.2									5.00	0.17	5.29	0.90								
	A1.3	FUELING CANOPY	A1.3	0.09	0.75	5.00	0.07	5.29	0.36												
2	DP1 + A1.3									5.00	0.24	5.29	1.26								
	A1.4	DRIVES, SIDEWALKS, AND LANDSCAPE AREAS	A1.4	1.64	0.59	5.00	0.97	5.29	5.13												
3	A1.4									5.00	0.97	5.29	5.13								
4	D3+D4									5.00	1.21	5.29	6.39								
	A1.5	PUBLIC SIDEWALK AND LANDSCAPE AREAS	A1.5	0.03	0.41	5.00	0.01	5.29	0.06												
	A1.6	ACCESS POINTS AND PUBLIC SIDEWALK	A1.6	0.06	0.79	5.00	0.05	5.29	0.25												
5	ALL IMPROVEMENTS									5.00	1.22	5.29	6.45								

- | | | | |
|---|---|--------------------------------------|----------------------------------|
| (1) Enter the Basin Name from C Value Sheet | (7) =Column 4 x Column 5 | (13) Sum of Qs | (19) Additional Flow Length |
| (2) Basin Description linked to C-Value Sheet | (8) =a*P/(b+Column 6)^c | (14) Additional Street Overland Flow | (20) Overland Velocity |
| (3) Enter the Basin Name from C Value Sheet | (9) =Column 7 x Column 8 | (15) Additional Street Overland Flow | (21) =Column 16 / Column 20 / 60 |
| (4) Basin Area linked to C-Value Sheet | (10) =Column 6 x Column 21 | (16) Additional Pipe Flow | |
| (5) Composite C linked to C-Value Sheet | (11) Add the Basin Areas (7) to get the combined basin AC | (17) Additional Pipe Flow | |
| (6) Time of Concentration linked to C-Value Sheet | (12) =28.5*P/(10+Column 10)^0.786 | (18) Additional Pipe Flow | |

DEVELOPED CONDITIONS



Runoff (Q)

Project: 7-Eleven - Bradley and Legacy Ridge City/State: El Paso County, CO
 Designed by: MRD Date: 4/14/2026
 Reviewed by: KMH Date: 4/14/2026

100 YEAR MAJOR STORM

Design Storm: 100-yr P = 2.75 in

a	b	c
28.5	10	0.786

Rainfall Intensity Equation Coefficients:

LOCATION	BASIN DESCRIPTION	DIRECT RUNOFF								TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME		
		BASIN NAME	AREA (AC)	RUNOFF COEFF (100 Yr)	t _c (MIN)	C.A. (AC)	I (IN / HR)	Q (CFS)	t _c (MIN)	SUM (C*A) (AC)	I (IN / HR)	Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	DESIGN FLOW (CFS)	SLOPE (%)	PIPE SIZE (in)	LENGTH (FT)	VELOCITY (FPS)	t _c (MIN)	
DP	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)
DEVELOPED																					
	A1.1	PROPOSED C-STORE	A1.1	0.11	0.81	5.00	0.09	9.33	0.83												
	A1.2	SW CORNER OF SITE	A1.2	0.12	0.81	5.00	0.10	9.33	0.90												
1	A1.1+A1.2									5.00	0.19	9.33	1.73								
	A1.3	FUELING CANOPY	A1.3	0.09	0.81	5.00	0.07	9.33	0.68												
2	DP1 + A1.3									5.00	0.26	9.33	2.41								
	A1.4	DRIVES, SIDEWALKS, AND LANDSCAPE AREAS	A1.4	1.64	0.70	5.00	1.15	9.33	10.70												
3	A1.4									5.00	1.15	9.33	10.70								
4	D3+D4									5.00	1.41	9.33	13.12								
	A1.5	PUBLIC SIDEWALK AND LANDSCAPE AREAS	A1.5	0.03	0.55	5.00	0.02	9.33	0.15												
	A1.6	ACCESS POINTS AND PUBLIC SIDEWALK	A1.6	0.06	0.86	5.00	0.05	9.33	0.48												
5	ALL IMPROVEMENTS									5.00	1.42	9.33	13.27								

- (1) Enter the Basin Name from C Value Sheet
- (2) Basin Description linked to C-Value Sheet
- (3) Enter the Basin Name from C Value Sheet
- (4) Basin Area linked to C-Value Sheet
- (5) Composite C linked to C-Value Sheet
- (6) Time of Concentration linked to C-Value Sheet
- (7) =Column 4 x Column 5
- (8) =a*P/(b+Column 6)*c
- (9) =Column 7 x Column 8
- (10) =Column 6 x Column 21
- (11) Add the Basin Areas (7) to get the combined basin AC
- (12) =28.5*P/(10+Column 10)*0.786
- (13) Sum of Qs
- (14) Additional Street Overland Flow
- (15) Additional Street Overland Flow
- (16) Additional Pipe Flow
- (17) Additional Pipe Flow
- (18) Additional Pipe Flow
- (19) Additional Flow Length
- (20) Overland Velocity
- (21) =Column 16 / Column 20 / 60

APPENDIX C
Hydraulic Conveyance

1. Curb Cut Sizing Calculations
2. Inlet Capacity Calculations
3. Grass Swale Capacity and Design
4. Storm Drain Summary Table
5. Storm Drain Profiles
6. Riprap Sizing Calculations

Weir Report

2FT CURB CUT

Rectangular Weir

Crest = Sharp
Bottom Length (ft) = 3.00
Total Depth (ft) = 0.50

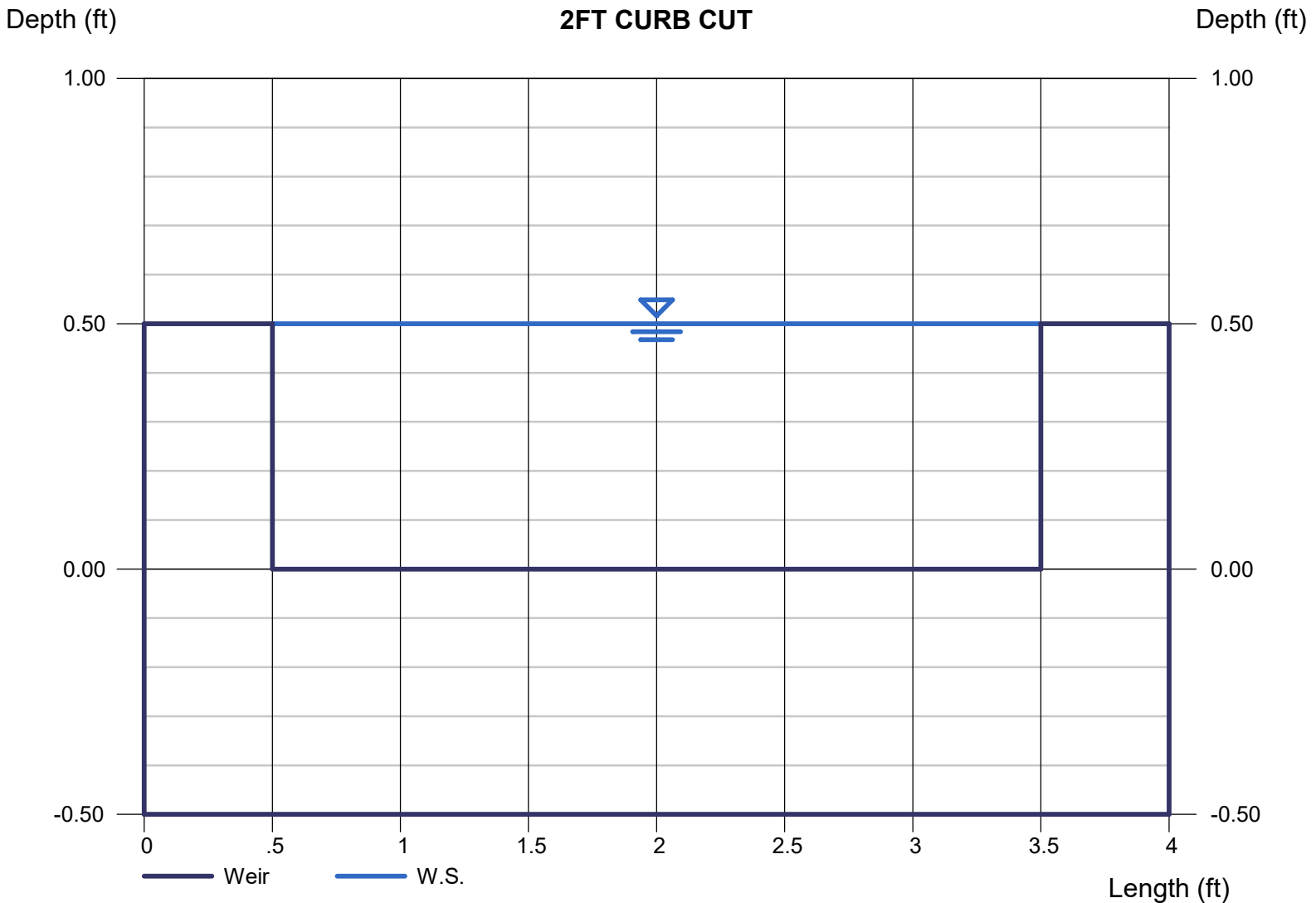
Calculations

Weir Coeff. Cw = 3.33
Compute by: Known Depth
Known Depth (ft) = 0.50

Highlighted

Depth (ft) = 0.50
Q (cfs) = 3.532
Area (sqft) = 1.50
Velocity (ft/s) = 2.35
Top Width (ft) = 3.00

4 CURB CUTS PROVIDED TO
CONVEY 10.61 CFS.
4 * 3.53 = 14.13 CFS CAPACITY



INLET MANAGEMENT

Project: 7-Eleven - El Paso County - Bradley and Legacy Ridge
Minor: 10-year
Major: 100-year

Worksheet Protected

INLET NAME	IN-01
Inlet Application (Street or Area)	STREET
Hydraulic Condition	In Sump
Inlet Type	CDOT Type R Curb Opening
Number of Inlet Units	1

USER-DEFINED INPUT

User-Defined Peak Flows	
Minor Peak Flow, Q (cfs)	0.47
Major Peak Flow, Q (cfs)	0.90

Bypass (Carry-Over) Flow from Upstream		Inlets must be organized from upstream (left) to
Receive Bypass Flow from:	No Bypass Flow Received	
Bypass Flow Description (Optional):		
Minor Bypass Flow Received, Q_b (cfs)	0.00	
Major Bypass Flow Received, Q_b (cfs)	0.00	

CALCULATED OUTPUT

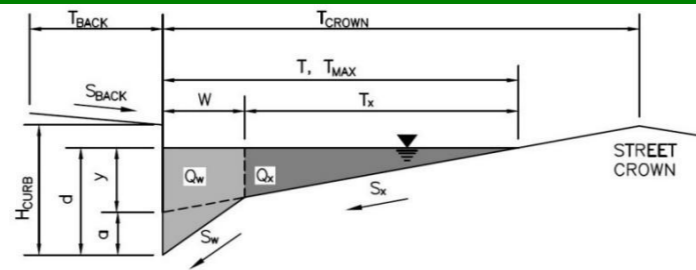
Minor Total Design Peak Flow, Q (cfs)	0.47
Major Total Design Peak Flow, Q (cfs)	0.90
Minor Inlet Interception Capacity, Q_a (cfs)	1.78
Major Inlet Interception Capacity, Q_a (cfs)	5.38
Minor Flow Bypassed Downstream, Q_b (cfs)	N/A
Major Flow Bypassed Downstream, Q_b (cfs)	N/A
Minor Flow Capture Percentage, C%	100%
Major Flow Capture Percentage, C%	100%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: 7-Eleven - El Paso County - Bradley and Legacy Ridge

Inlet ID: IN-01



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 10.0$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width
 Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 35.0$ ft
 $W = 2.00$ ft
 $S_X = 0.020$ ft/ft
 $S_W = 0.083$ ft/ft
 $S_O = 0.000$ ft/ft
 $n_{STREET} = 0.012$

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

	Minor Storm	Major Storm	
$T_{MAX} =$	10.0	20.0	ft
$d_{MAX} =$	6.0	6.0	inches
	<input type="checkbox"/>	<input type="checkbox"/>	

MINOR STORM Allowable Capacity is not applicable to Sump Condition
MAJOR STORM Allowable Capacity is not applicable to Sump Condition

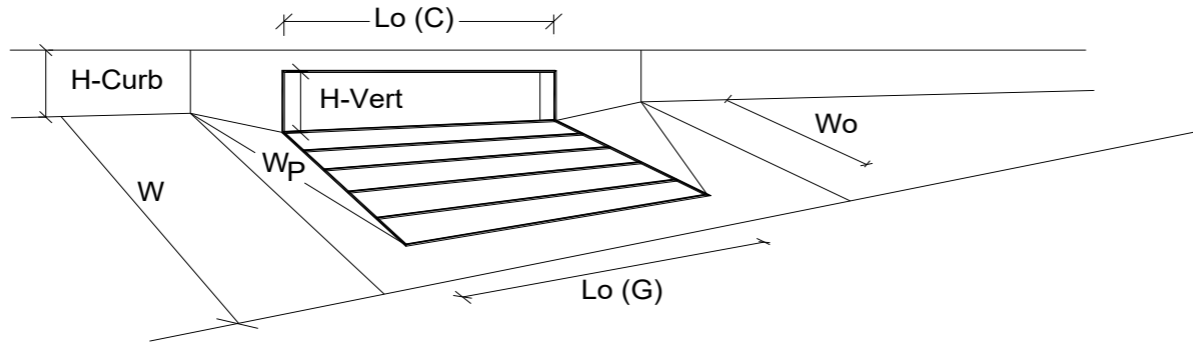
$Q_{allow} =$

Minor Storm	Major Storm
SUMP	SUMP

 cfs

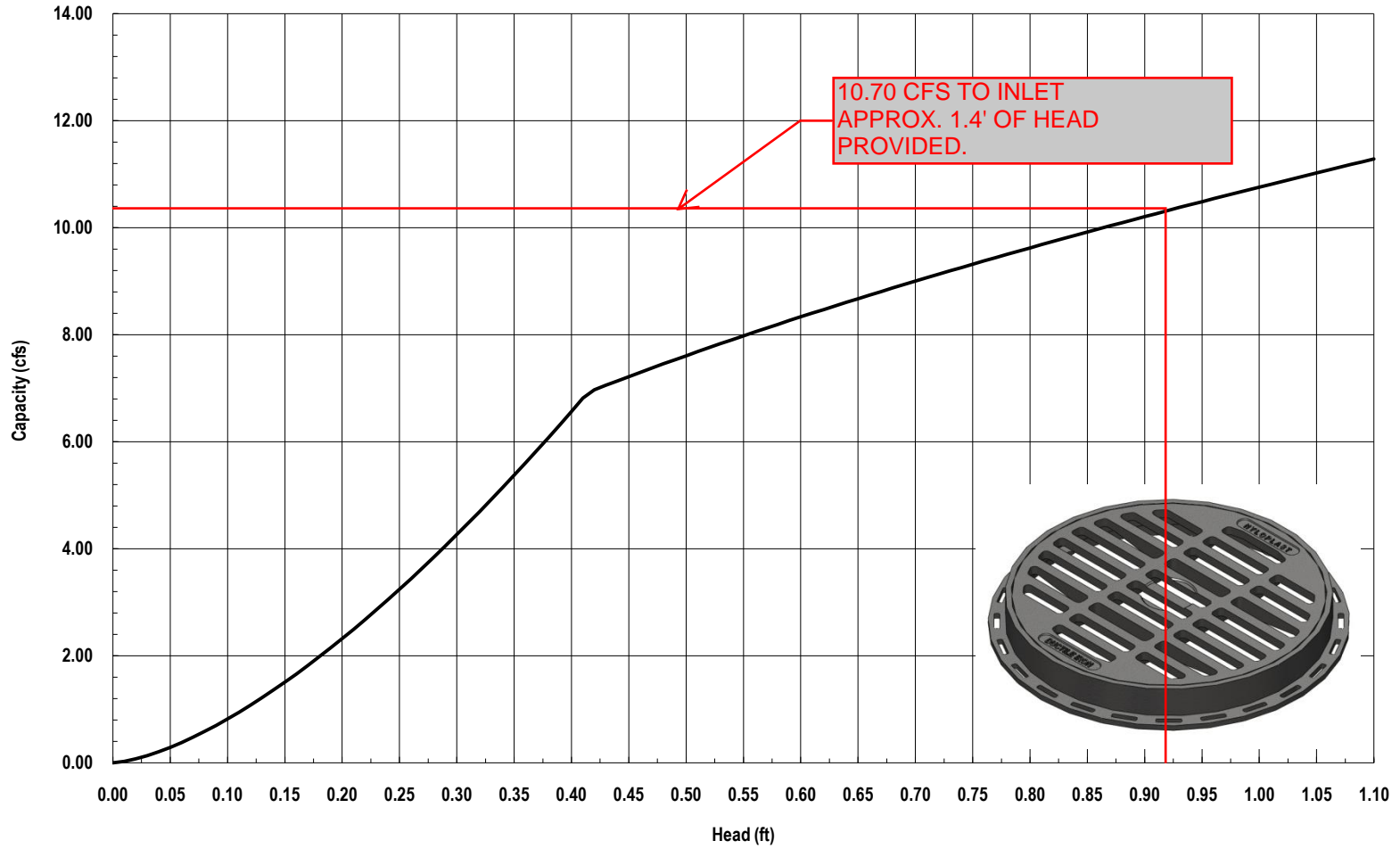
INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 6.00 (August 2025)



Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	1	1	
Water Depth at Flowline (outside of local depression)	3.9	6.0	inches
Grate Information			
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information			
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)			
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.16	0.33	ft
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	1.00	1.00	
Combination Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)			
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	1.8	5.4	cfs
Q_{PEAK REQUIRED}	0.5	0.9	cfs

Nyloplast 30" Standard Grate Inlet Capacity Chart



Nyloplast[®]

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Channel Report

A1.4 GRASS SWALE

Trapezoidal

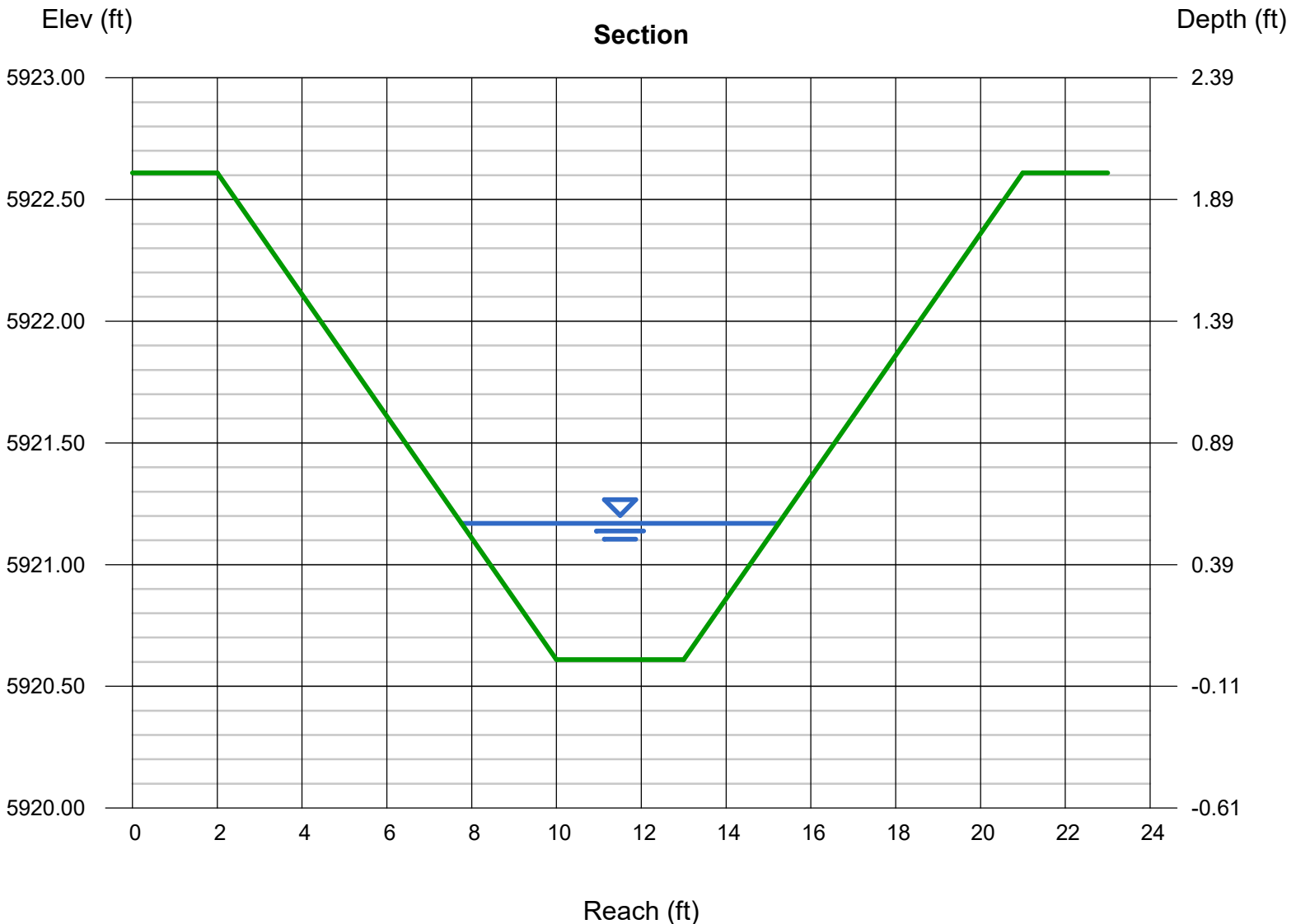
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 2.00
Invert Elev (ft) = 5920.61
Slope (%) = 2.00
N-Value = 0.030

Highlighted

Depth (ft) = 0.56
Q (cfs) = 10.61
Area (sqft) = 2.93
Velocity (ft/s) = 3.62
Wetted Perim (ft) = 7.62
Crit Depth, Yc (ft) = 0.57
Top Width (ft) = 7.48
EGL (ft) = 0.76

Calculations

Compute by: Known Q
Known Q (cfs) = 10.61



Scenario: 5-yr
Current Time Step: 0.000 h
Conduit FlexTable: DOT Report

Label	-Node- Upstream Downstream	-Depth- Upstream Downstream (ft)	-EGL- Upstream Downstream (ft)	-Ground- Upstream Downstream (ft)	-HGL- Upstream Downstream (ft)	-Invert- Upstream Downstream (ft)	Section Discharge Capacity (cfs)	-X- Upstream Downstream (ft)	-Y- Upstream Downstream (ft)
P-37	MH-01 (STORM)	1.01	5,918.86	5,924.78	5,918.41	5,917.40	6.82	12,532.17	8,283.64
-	EX MH	(N/A)	(N/A)	5,923.00	5,918.36	5,915.73	25.71	12,540.74	8,257.19
P-35	IN-01 (STORM)	0.50	5,919.64	5,927.79	5,919.44	5,918.93	1.42	12,327.10	8,280.23
-	MH-01 (STORM)	1.01	5,918.60	5,924.78	5,918.41	5,917.40	3.28	12,532.17	8,283.64
P-36	IN-02 (STORM)	0.99	5,919.36	5,920.62	5,918.99	5,918.00	5.40	12,556.40	8,349.22
-	MH-01 (STORM)	1.01	5,918.60	5,924.78	5,918.56	5,917.40	9.50	12,532.17	8,283.64

P:\LASCO\CO, El Paso County - Bradley Rd at Legacy Hill\07 Design\Drainage\STORM.stsw

Scenario: 100-yr
Current Time Step: 0.000 h
Conduit FlexTable: DOT Report

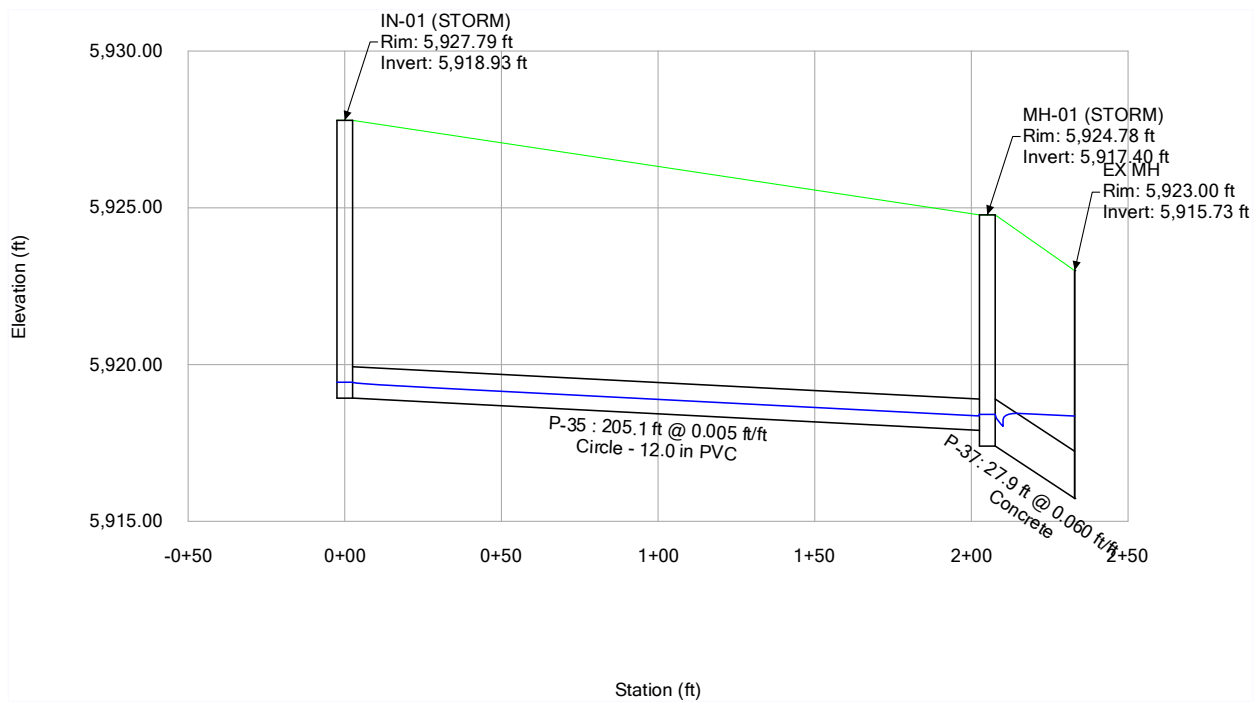
Label	-Node- Upstream Downstream	-Depth- Upstream Downstream (ft)	-EGL- Upstream Downstream (ft)	-Ground- Upstream Downstream (ft)	-HGL- Upstream Downstream (ft)	-Invert- Upstream Downstream (ft)	Section Discharge Capacity (cfs)	-X- Upstream Downstream (ft)	-Y- Upstream Downstream (ft)
P-37	MH-01 (STORM)	1.36	5,919.73	5,924.78	5,918.76	5,917.40	13.28	12,532.17	8,283.64
-	EX MH	(N/A)	(N/A)	5,923.00	5,918.36	5,915.73	25.71	12,540.74	8,257.19
P-35	IN-01 (STORM)	0.70	5,919.95	5,927.79	5,919.63	5,918.93	2.67	12,327.10	8,280.23
-	MH-01 (STORM)	1.36	5,918.98	5,924.78	5,918.76	5,917.40	3.28	12,532.17	8,283.64
P-36	IN-02 (STORM)	1.49	5,920.08	5,920.62	5,919.49	5,918.00	10.61	12,556.40	8,349.22
-	MH-01 (STORM)	1.36	5,918.98	5,924.78	5,919.00	5,917.40	9.50	12,532.17	8,283.64

P:\LASCO\CO, El Paso County - Bradley Rd at Legacy Hill\07 Design\Drainage\STORM.stsw

Profile Report

Engineering Profile - IN-01-EX MH (STORM.stsw)

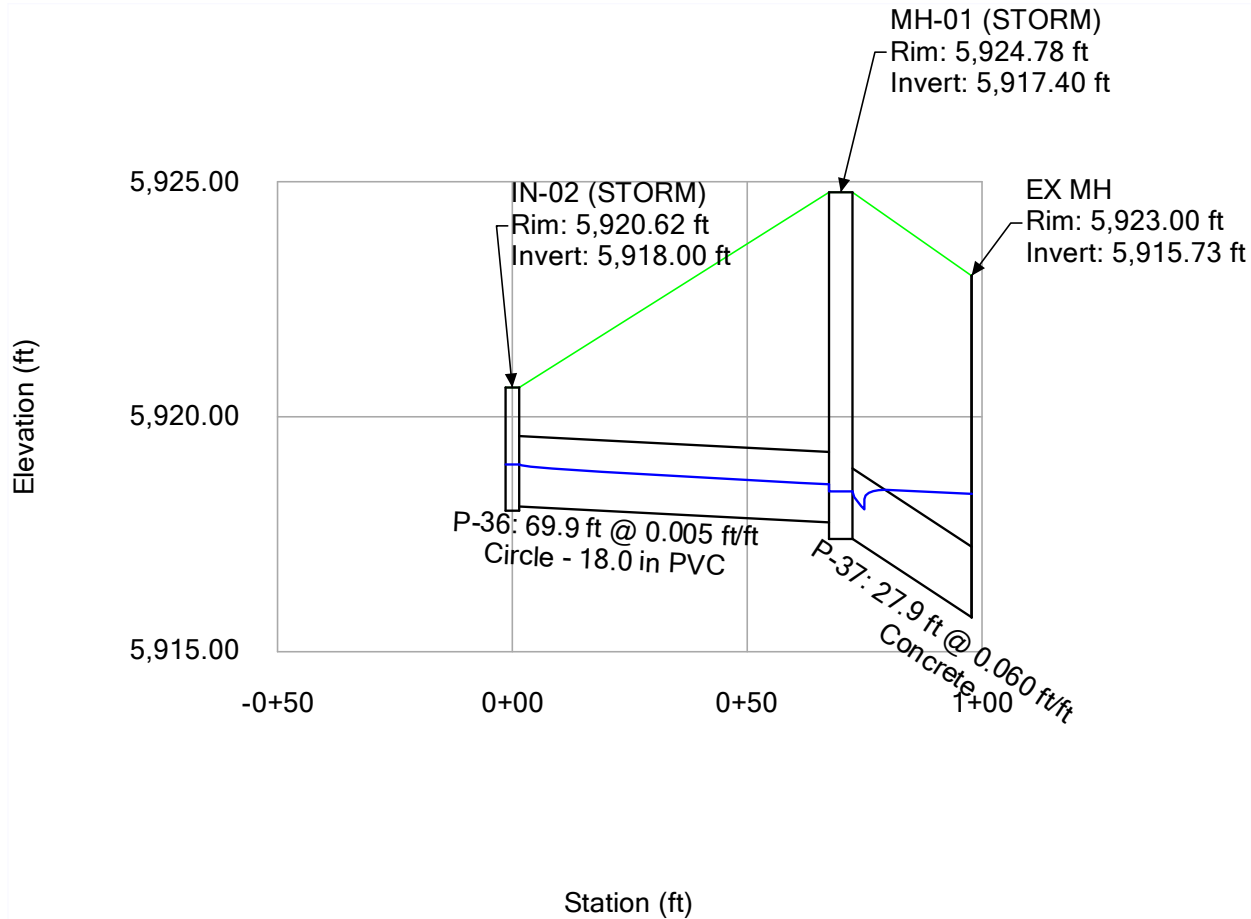
10-YEAR MINOR STORM



Profile Report

Engineering Profile - IN-02-EX MH (STORM.stsw)

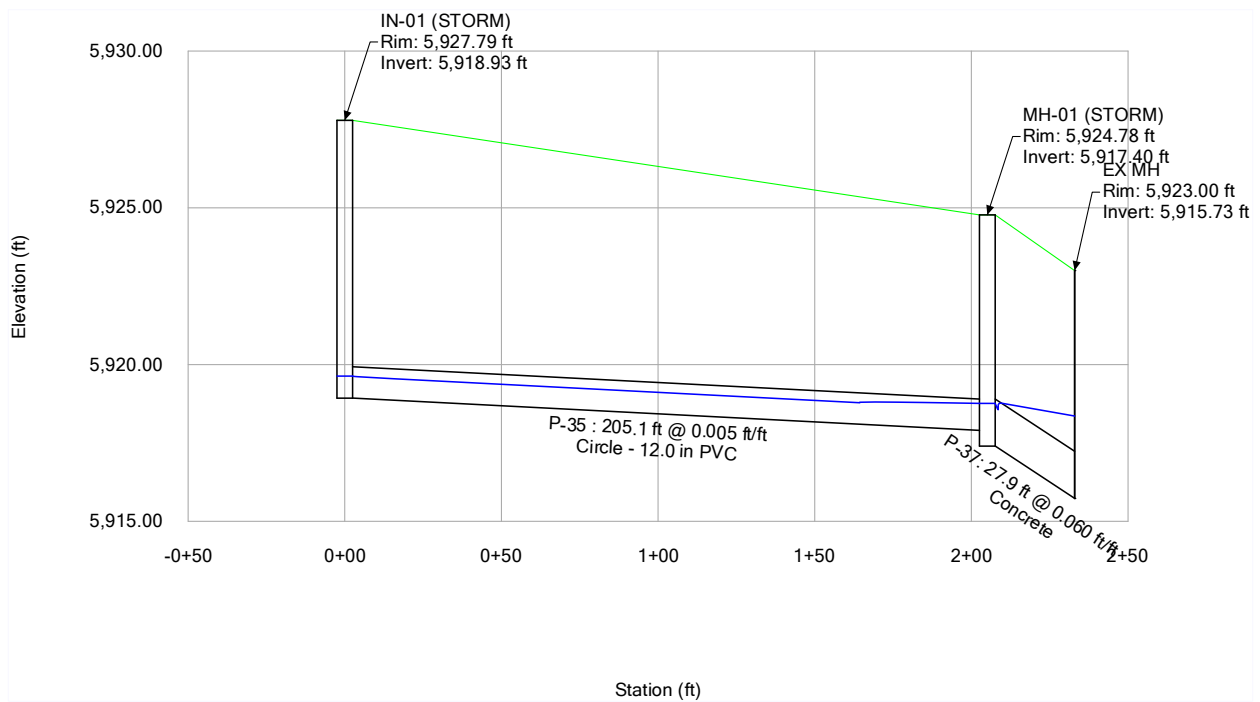
10-YEAR MINOR STORM



Profile Report

Engineering Profile - IN-01-EX MH (STORM.stsw)

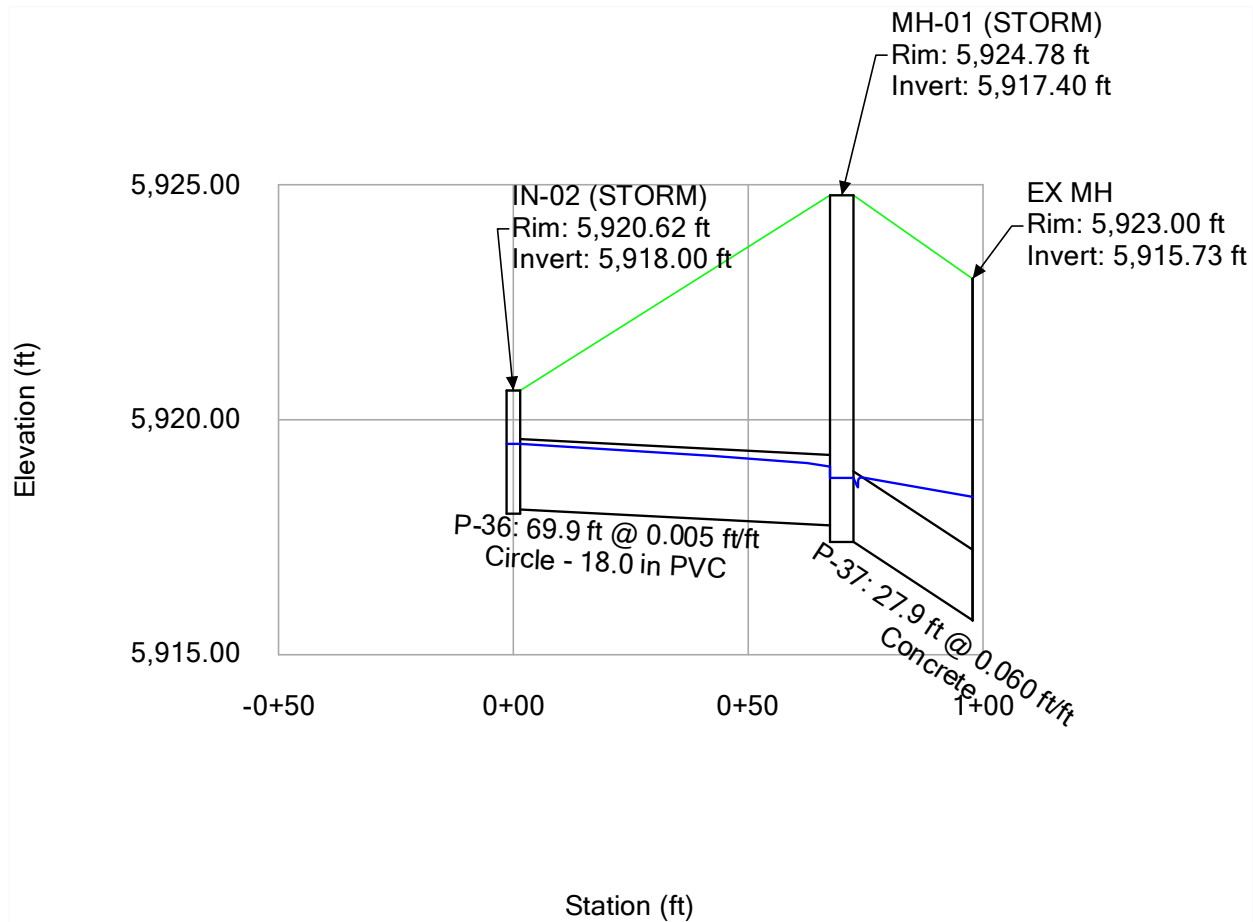
100-YEAR MINOR STORM



Profile Report

Engineering Profile - IN-02-EX MH (STORM.stsw)

100-YEAR MINOR STORM



Low Tail Water Basin and Riprap Design 2' CURB CUT

Project Name 7-11 - Legacy and Bradley
17-Mar-26

Instructions: Refer to Section 3.4.3.2 of Chap 8 Vol 2 in UD Manual. Enter values in blue cells. Green cells are calculated.
100-year design flows

Outlet Pipe Information:

Type of Pipe: Rectangular

Storm Sewer Height, H = 0.5 ft

Riprap Size:

Velocity = 2.35 ft/s ⁽¹⁾ 100 Year

Design Depth, d = 0.5 ft ⁽²⁾

Gravity, g = 32.2 ft/s²

Eqn: HS-16e

P_d = 4.65 ft/s

Use Figure HS-20c to find the size and type of riprap to use in the outlet protection basin.

Riprap Selection: Type L

Riprap Diameter, D₅₀ = 9 inches

Riprap Minimum Thickness:

Eqn: HS-17

Thickness, T = 1.31 ft

Basin Dimensions:

Storm Sewer Height, H = 0.5 ft

Length is defined as being the greater of the following:

L = 4H = 2 ft Eqn: HS-18

L=(H)^{0.5}(V/2) = 0.830850468 ft Eqn: HS-19

L = 2 ft

Width:

w = width of box culvert = 2 ft

W = w+4H = 4 ft Eqn: HS-20 or HS-21

Cutoff Wall:

B = 1.56 ft Eqn: HS-22

(1) Obtain Velocity from Section 3.4.3.1 of Vol 2 in the UD Manual or program such as FlowMaster or StormCad

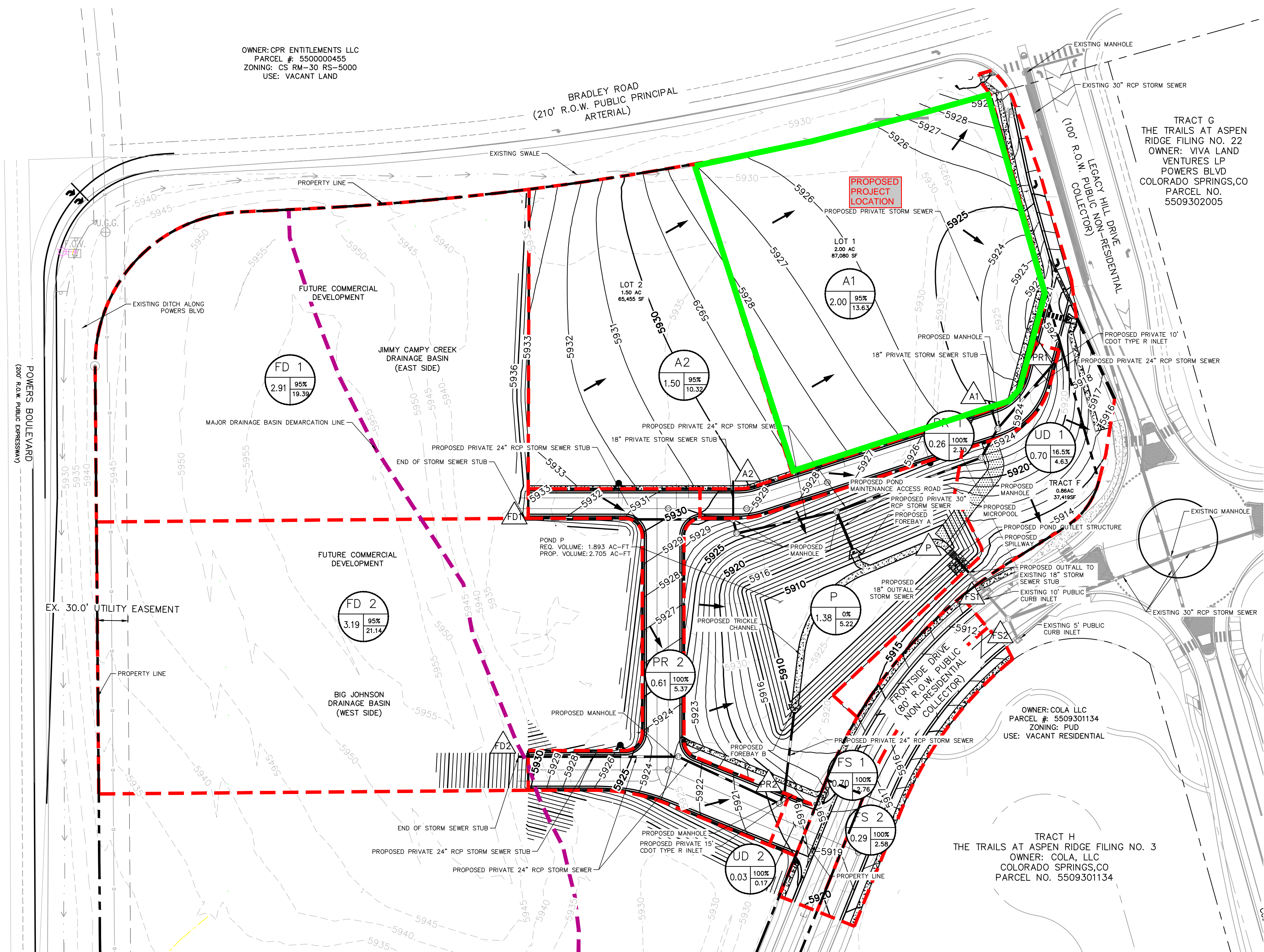
(2) Obtain flow depth from Section 3.4.3.1 of Vol 2 in the UD Manual or program such as FlowMaster or StormCad

APPENDIX D
Drainage Maps

1. Master Drainage Report Drainage Map
2. Developed Drainage Map

K:\COS_LA\196195000 - Waterview East Commercial\CADD\Exhibits\2026-0310_Final Plat Drainage Exhibit\Final Plat Drainage Exhibit.dwg McCollum, Jessica 3/12/2026 6:31 PM

OWNER: CPR ENTITLEMENTS LLC
 PARCEL #: 550000455
 ZONING: CS RM-30 RS-5000
 USE: VACANT LAND



LEGEND

	A = BASIN DESIGNATION B = AREA (ACRES) C = BASIN IMPERVIOUSNESS D = 100-YR DESIGN STORM RUNOFF (CFS)
	DESIGN POINT
	FLOW DIRECTION
	MAJOR DRAINAGE BASIN BOUNDARY
	DRAINAGE BASIN BOUNDARY
	PROPERTY LINE
	PROPOSED MAJOR CONTOUR
	PROPOSED MINOR CONTOUR
	EXISTING MAJOR CONTOUR
	EXISTING MAJOR CONTOUR
	EXISTING DITCH/SWALE
	PROPOSED SWALE
	PROPOSED STORM SEWER
	PROPOSED STORM MANHOLE
	PROPOSED STORM INLET
	EXISTING STORM SEWER
	EXISTING STORM MANHOLE
	EXISTING STORM INLET

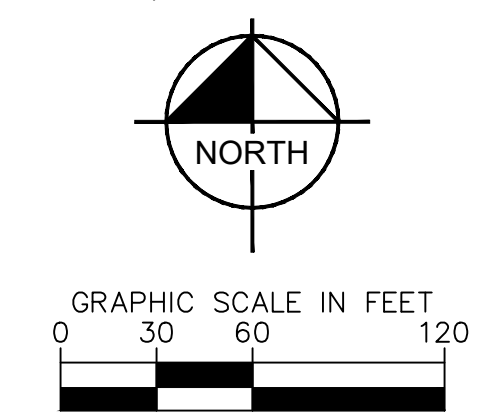
SUMMARY - PROPOSED RUNOFF TABLE

DESIGN POINT	BASIN DESIGNATION	BASIN AREA (ACRES)	DIRECT 5-YR RUNOFF (CFS)	DIRECT 100-YR RUNOFF (CFS)	CUMMULATIVE 5-YR RUNOFF (CFS)	CUMMULATIVE 100-YR RUNOFF (CFS)
A1	A1	2.00	5.92	13.63	5.92	13.63
A2	A2	1.50	4.48	10.32	4.48	10.32
FD 1	FD 1	2.91	8.41	19.39	8.41	19.39
FD 2	FD 2	3.19	9.17	21.14	9.17	21.14
PR 1	PR 1	0.26	1.02	2.30	1.02	2.30
PR 2	PR 2	0.61	2.38	5.37	2.38	5.37
P	P	1.38	0.82	5.22	31.48	48.16
FS1	UD 1	0.70	0.77	4.63	0.77	4.63
FS1	UD 2	0.03	0.13	0.17	0.13	0.17
FS 1	FS 1	0.31	1.23	2.76	1.23	2.76
FS 2	FS 2	0.29	1.15	2.58	1.15	2.58

PBMP SUMMARY TABLE

BASINS	PBMP TRIBUTARY AREA (AC)	PBMP
A1-A2, FD1-FD-2, PR1-PR2, P	11.8500	EDB - P
A26, OS1	2.8800	EAST POND

NOTES:
 1. SUB-BASINS FS1 AND FS2 NOT TREATED BY A PBMP ARE EXCLUDED BASED ON ECM APP 1.7.1.B.2.1 AND 1.7.1.B.7.
 2. SUB-BASINS UD1 AND UD2 NOT TREATED BY A PBMP ARE EXCLUDED BASED ON ECM APP 1.7.1.C.1.A.



NO. _____ BY _____ DATE _____

REVISION _____

Kimley»Horn

2026 KIMLEY-HORN AND ASSOCIATES, INC.
 2 North Nevada Avenue, Suite 900
 Colorado Springs, Colorado 80903 (719) 453-0180

DESIGNED BY: ANF
 DRAWN BY: ANF
 CHECKED BY: JMM
 DATE: 03/13/2026

WATERVIEW EAST COMMERCIAL
 CONSTRUCTION DOCUMENTS
 PROPOSED DRAINAGE MAP

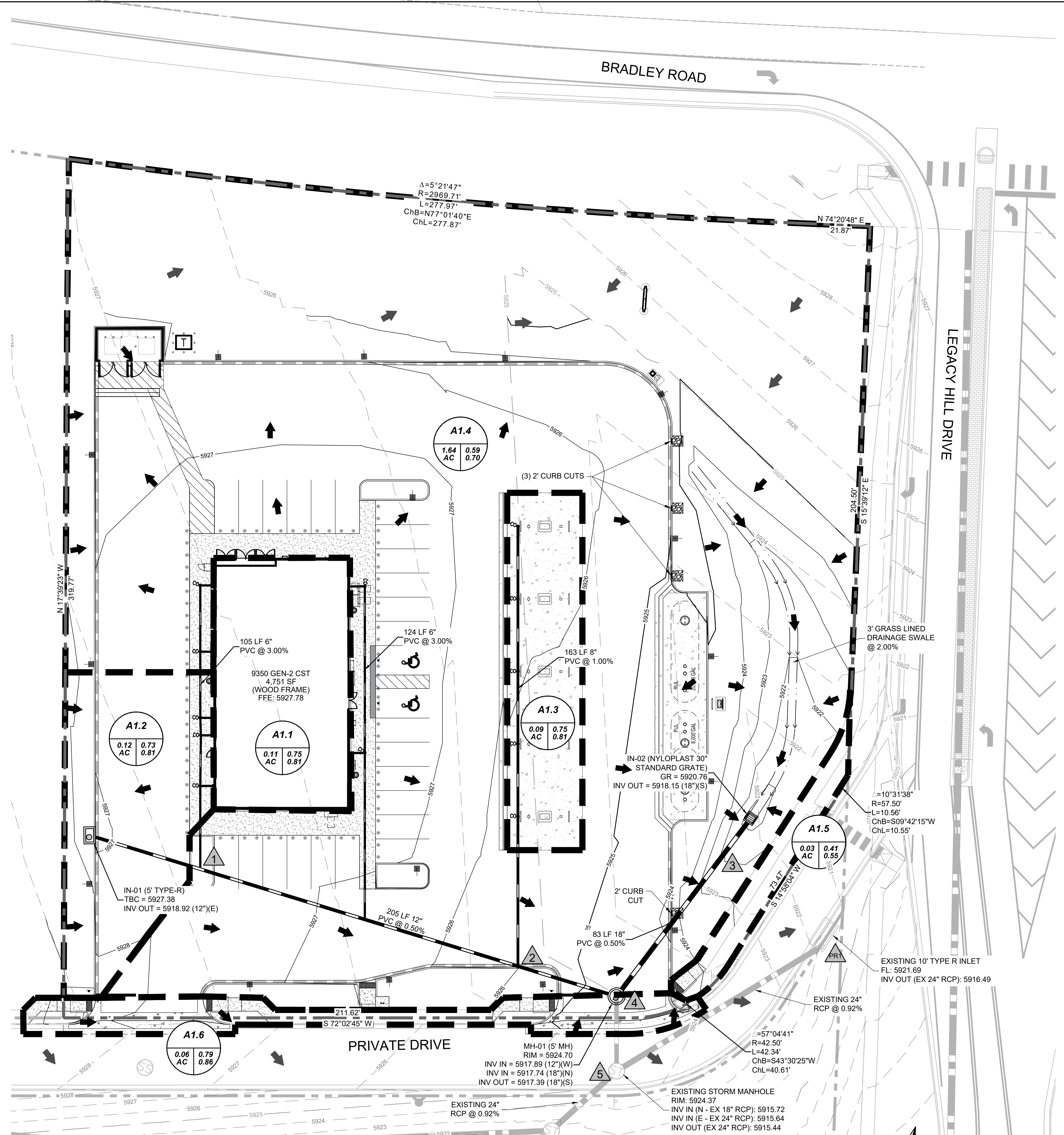
PRELIMINARY
 FOR REVIEW ONLY
 NOT FOR CONSTRUCTION

 Kimley-Horn and Associates, Inc.

PROJECT NO.
196195000

SHEET
DR-1





LEGEND

- BASIN DELINEATION
- EXISTING STORM SEWER LINE
- EXISTING STORM INLET
- FLOW ARROW
- DESIGN POINT
- BASIN DESIGNATION
- 10-YEAR RUNOFF COEFFICIENT
- 100-YEAR RUNOFF COEFFICIENT
- BASIN AREA IN ACRES

DEVELOPED RUNOFF SUMMARY TABLE

DESIGN POINT	TRIBUTARY BASIN	TRIBUTARY AREA (AC)	PERCENT IMPERVIOUSNESS	COEFFICIENT		TOTAL RUNOFF	
				C10	C100	Q10 (CFS)	Q100 (CFS)
1	A1.1	0.11	90.0	0.75	0.81	0.44	0.83
	A1.2	0.12	75.0	0.73	0.81	0.46	0.90
	A1.1+A1.2					0.90	1.73
2	A1.3	0.09	90.0	0.75	0.81	0.36	0.68
	DP1 + A1.3					1.26	2.41
3	A1.4	1.64	57.3	0.59	0.70	5.13	10.70
	A1.4					5.13	10.70
	D3+D4					6.39	13.12
PR1	A1.5	0.03	33.3	0.41	0.55	0.06	0.15
	A1.6	0.06	83.3	0.79	0.86	0.25	0.48
5	Composite	2.05	62.0	0.62	0.72	6.45	13.27

ENGINEER'S STATEMENT:
 THE ATTACHED DRAINAGE PLAN AND REPORT WERE PREPARED UNDER MY DIRECTION AND SUPERVISION AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. SAID DRAINAGE REPORT HAS BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE CITY/COUNTY FOR DRAINAGE REPORTS AND SAID REPORT IS IN CONFORMITY WITH THE MASTER PLAN OF THE DRAINAGE BASIN. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARING THIS REPORT.

 MICHAEL DRAGO, P.E. 59225 SEAL

DEVELOPER'S STATEMENT:
 I, _____, THE DEVELOPER HAVE READ AND WILL COMPLY WITH ALL THE REQUIREMENTS SPECIFIED IN THIS DRAINAGE REPORT AND PLAN.

 BUSINESS NAME

BY: _____
 TITLE: _____
 ADDRESS: _____

EL PASO COUNTY ONLY:
 FILED IN ACCORDANCE WITH SECTION 51.1 OF THE EL PASO LAND DEVELOPMENT CODE AS AMENDED.

 DIRECTOR OF PUBLIC WORKS DATE

CITY OF COLORADO SPRINGS ONLY:
 FILED IN ACCORDANCE WITH SECTION 15-3-906 OF THE CODE OF THE CITY OF COLORADO SPRINGS, 1980, AS AMENDED.

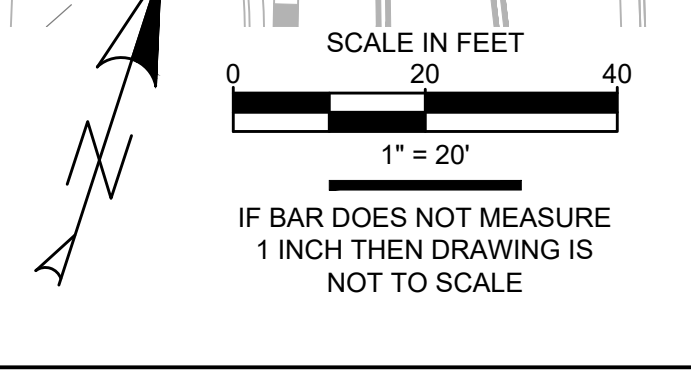
 CITY ENGINEER DATE

CONDITIONS: _____

811
 CALL UTILITY NOTIFICATION CENTER OF COLORADO
 1-800-922-1987 or 811
 CALL 3-BUSINESS DAYS (NOT INCLUDING INITIAL DAY OF CONTACT) IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.
 Know what's below. Call before you dig.

PROJECT BENCHMARK
 COLORADO SPRINGS UTILITY (FIMS) MONUMENT F206
 A BERTNSEN TOP SECURITY MONUMENT SYSTEM WITH A 3.5 INCH DIAMETER ALUMINUM CAP IN A ROAD BOX, LOCATED AT THE NORTHWEST CORNER OF FONTAINE BOULEVARD AND POWERS BOULEVARD (NOW HIGHWAY 21)
 ELEVATION - 5897.89' (NGVD 1929)

DETENTION POND NOTE
 EXISTING STORMWATER POND SOUTH OF PRIVATE DRIVE TO TREAT AND DETAIN STORMWATER RUNOFF - REVIEW TO WATERVIEW-EAST COMMERCIAL PLAN FOR ADDITIONAL INFORMATION



PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR STAMP
 PCD FILE NO. XXX-XXXX



PRELIMINARY NOT FOR CONSTRUCTION

MAJOR SITE DEVELOPMENT PLAN
7-ELEVEN - STORE #42975
SITE #1057022
 BRADLEY ROAD AND LEGACY DRIVE
DEVELOPED DRAINAGE MAP

NO.	REVISION	DATE

PROJECT NO: LAS008.01
 DESIGNED BY: MRD
 REVIEWED BY: MRD
 DATE: 04/24/2026

D1.2