LOTS 3, 4, AND 5 NORTHCREST CENTER FILING NO 2
PHASE 1 SUBDIVISION
REFERRED TO AS: "NORTHCREST CENTER"
FINAL DRAINAGE REPORT

2510 & 2522 CANADA DRIVE
COLORADO SPRINGS, COLORADO
80922

PREPARED FOR: LEISURE CONSTRUCTION
3443 TAMPA ROAD, SUITE B
PALM HARBOR, FL 34684
(727) 242-5121

PCD FILE NO. PPR-21-036

February 8th, 2023

Prepared by
David Walker, P.E.
Rocky Mountain Group
2910 Austin Bluffs Blvd. | Colorado Springs, CO 80918 | 719-434-5638

SIGNATURE PAGE

LOTS 3, 4, AND 5 NORTHCREST CENTER FILING NO 2 PHASE 1 SUBDIVISION

ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by negligent acts, errors or omissions on my part in preparing this report.

SIGNATURE (Affix Seal): ______________________ 12/16/22

David Walker, P.E. No.: 51909 Date:

COLORADO LICENSED PROFESSIONAL ENGINEER 51909 12/16/22

DEVELOPER'S STATEMENT

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

K&S Development, LLC

Name of Developer

SL Edwards, MM 11-14-22

Authorized Signature Date

Sean L. Edwards

Printed Name

Managing Member

Title

3442 Tampa Rd., Suite B, Palm Harbor, FL 34684

Address

EL PASO COUNTY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Approved
By: Elizabeth Nijkamp, PE
Date:02/21/2023
El Paso County Department of Public Works

Joshua Palmer, P.E.
County Engineer / ECM Administrator

Conditions:

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APPENDICES

I. PURPOSE

This report is a Final Drainage Report for Lots 3, 4 & 5 Northcrest Center Subdivision, a currently unaddressed set of lots, for the development for a multi-unit commercial development, Northcrest Center.

The purpose of this report is to identify on-site and off-site drainage patterns, assess stormwater conditions per delineated basin and sub-basins, demonstrate adequate design standards for storm water flow and release into the existing storm water system or right-of-way, and provide a narrative for any other drainage considerations related to the development of this parcel.

II. GENERAL LOCATION AND DESCRIPTION

A. LOCATION

The proposed development of thirteen 2,280 square foot commercial buildings, is located at the address of Northcrest Center (formerly Lots 3, 4, & 5) in the City of Colorado Springs, Colorado in El Paso County within the Northcrest Center Subdivision. The parcel schedule numbers are 5332309004, 5332309006, and 5332309005 and the legal descriptions are Lot 3 Northcrest Center, A Vacation & Replat Of Tract B Northcrest Fil No 2 Phase 1, Lot 5 Northcrest Center, A Vacation & Replat Of Tract B Northcrest Fil No 2 Phase 1, and Lot 4 Northcrest Center, A Vacation & Replat Of Tract B Northcrest Fil No 2 Phase 1. The parcel is located to the north of Constitution Avenue, west of Canada Drive, east of Peterson Road, and south of Bismark Road.

The surrounding parcels are as follows:

North of Lots 3, 4 & 5 Northcrest Center, A Vacation & Replat of Tract B Northcrest Fil No 2 Phase 1 (Northcrest Center (formerly Lots 3, 4, & 5)) from west to east:

2508 Weyburn Way, Schedule No. 5332308031, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 15 Constitution Hills Fil No 8

2507 Weyburn Way, Schedule No. 5332308032, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 14 Constitution Hills Fil No 8

2630 Tibburn Way, Schedule No. 5332308040, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 6 Constitution Hills Fil No 8

2610 Tibburn Way, Schedule No. 5332308041, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 5 Constitution Hills Fil No 8

2605 Tibburn Way, Schedule No. 5332308042, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 4 Constitution Hills Fil No 8

2624 Tibburn Way, Schedule No. 5332308043, Zoning RS-6000 CAD-O, Plat No. 10281, Lot 3 Constitution Hills Fil No 8

East of Lots 3, 4 & 5 Northcrest Center, A Vacation & Replat of Tract B Northcrest Fil No 2 Phase 1 (Northcrest Center (formerly Lots 3, 4, & 5)) from north to south:

2535 Canada Drive, Schedule No. 5332310002, Zoning RS-6000 CAD-O, Plat No. 8956, Lot 1 Living Waters Sub

2525 Canada Drive, Schedule No. 5332310003, Zoning PUD CAD-O, Plat No. 8956, Lot 2 Living Waters Sub

2455 Canada Drive, Schedule No. 5405207050, Zoning RM-30 CAD-O, Plat No. 7588, Lot 2 Northcrest Fil No 4

South of Lots 3, 4 & 5 Northcrest Center, A Vacation & Replat of Tract B Northcrest Fil No 2 Phase 1 (Northcrest Center (formerly Lots 3, 4, & 5)) from west to east:

6855 Constitution Avenue, Schedule No. 5405218002, Zoning CC CAD-O, Plat No. 9808, Lot 1 Eight Line Sub

West of Lots 3, 4 & 5 Northcrest Center, A Vacation & Replat of Tract B Northcrest Fil No 2 Phase 1 (Northcrest Center (formerly Lots 3, 4, & 5)) from north to south:

6805 Bismark Road, Schedule No. 5332309007, Zoning CC CAD-O, Plat No. 7776, Lots 1 & 2 Northcrest Center, A Vacation & Replat of Tract B Northcrest Fil No 2 Phase 1

B. Description of Property – Existing Conditions

Lots 3, 4, and 5 (Northcrest Center (formerly Lots 3, 4, & 5)) is approximately 144,776 square feet combined (3.32 acres) and is located on the north side of Constitution Avenue, east side of Peterson Road, West of Canada Drive, and south of Bismark Road. The parcels fall within the SW 1/4 of Section 32, Township 13 South, Range 65 West of the 6^{th} P.M. of Colorado Springs, El Paso County, Colorado.

The property currently consists of undeveloped natural vegetation. There is existing curb and gutter along Bismark Drive, Canada Drive, and Constitution Avenue.

The existing percent imperviousness is approximately 1.5 percent on Lots 3, 4, and 5.

The existing topography consists of grades between 5.3 and 19.17 percent. Drainage patterns sheet flow across the parcel southeasterly to the corner of Canada Drive and Constitution Avenue.

C. Existing Soils

The soils indicative to the site are classified as Truckton sandy loam by the USDA Soil Conservation Service and are listed as NRCS (National Resources Conservation Service) Hydrologic Soil Group A. A USDA Soil Map is provided in the Appendix.

A subsurface soils investigation was conducted for Commercial Development Site Constitution and Peterson, with a letter entitled *Geotechnical Report* by RMG – Rocky Mountain Group dated

February 23, 2021. The investigation "revealed similar substance subsurface soil conditions across the site, being primarily silty sand extending from the ground surface to the extent of the test borings. Neither expansive clay soil not bedrock were encountered in the borings."

"Test Borings for structures and storage yards were advanced with a power-driven, continuous-flight auger drill rig to depths of 15 and 20-feet below the existing ground surface. Pavement Borings were advanced to 5 and 10-foot depths."

The study found that "groundwater was not encountered in the test borings during field exploration."

D. EXISTING DRAINAGE

The existing topography consists of grades between 5.3 and 19.17 percent within the entire parcel that ultimately flows southeast. The existing imperviousness of the lot is approximately 1.5 percent. The existing vegetation consists of native grasses and has been identified via site visits and aerial photography as well as survey data and pictures.

The existing drainage pattern from storm runoff is generally characterized as overland flow to the southeast of the parcel across pervious landscaped yard. The runoff from this parcel and the surrounding neighborhood flows via curb and gutter in the public right of way of Bismark Road, Constitution Avenue, and Canada Drive. The runoff flows south on Canada Drive into the existing Public 15' CDOT Type R Curb Inlet located at the northwest corner of the intersection of Canada Drive and Constitution Avenue. This Public Storm Inlet is a branch of the Public 24" RCP Storm Main that flows west to east along the north side of Constitution Avenue and ultimately outfalls into Jimmy Camp Creek.

Lots 3, 4, and 5 do not lie within a designated floodplain according to information published in the Federal Emergency Management Agency Floodplain Map No. 08041C0752G, dated December 7, 2018. The FEMA Floodplain map is provided in Appendix C showing it lies within Zone X, a minimal flood hazard area.

There are no known non-stormwater discharges that contribute to the storm water systems on site and downstream, both private and public.

E. DESCRIPTION OF PROPERTY – PROPOSED CONDITIONS

The proposed development consists of thirteen 2,280 square foot commercial units and approximately 69,000 square feet of concrete and asphalt pavement for drive accesses, sidewalks, and curb and gutter. Other on-site features includes approximately 29,000 square feet of landscaping, 760 linear feet of retaining wall, and 2,800 square feet for a full spectrum detention pond.

There is no existing access point to the property. Two new curb cuts are proposed along Canada Drive.

III. DRAINAGE BASINS AND SUB-BASINS

A. EXISTING MAJOR DRAINAGE BASIN AND SUB-BASINS

The parcel is delineated into sub-basins according to the existing and proposed grading for existing and developed conditions.

Basin E is the entirety of the parcel representing existing conditions and consists of one on-site sub-basin. There are no off-site flows that enter the property due to the slope of the existing drive aisle between this lot and the neighboring lot.

Sub-basin E-1 (3.24 ac.; Q_{10} = 2.15 cfs, Q_{100} = 6.86 cfs) is the entirety of Lots 3, 4, and 5 to be replatted via vacation into a single lot which contains natural vegetation that flows to the right of ways of Bismark Road, Canada Drive, and Constitution Ave. Those right of ways have curb and gutter directly adjacent to the lot that flow to a Public 15' CDOT Type R Curb Inlet. This public stormwater system is connected to a Public 24" RCP Storm Main that runs west to east along the south side of the lot within Constitution Avenue. The Public 15' & 5' CDOT Type R Curb Inlets located at the northwest and northeast corners, respectively, of the intersection of Canada Drive and Constitutions Avenue are branches that connect to the public stormwater main within Constitution Avenue. The public stormwater system ultimately flows to the East Fork Sand Creek.

EP1 is the existing conditions design point representing the Public 15' CDOT Type R Curb Inlet at the northwest corner at the intersection of Canada Drive and Constitution Avenue where sub-basin E-1 flows over the pervious lot containing native grasses and vegetation to the public roadways. The emergency flow route of this public storm inlet is due east along the north side of Constitution Avenue.

B. DEVELOPED MAJOR DRAINAGE BASIN AND SUB-BASINS

Basin D is the entirety of the developed parcel representing developed conditions and consists of nine sub-basins. Some off-site flows will enter the proposed site from the western paved drive access currently serving the existing school.

Sub-basin D-1 (0.29 ac. ; Q_{10} = 1.14 cfs, Q_{100} = 1.80 cfs) is the northwestern area of the development consisting of an asphalt drive access off of the common drive access for this property and the neighboring lot from Bismark Road as well as enclosed storage and large vehicle parking area that consists of compacted gravel. The sub-basin is sloped southeast and overland flow is directed to a concrete drainage pan within the drive aisle and channeled to a Private 3'x3' CDOT Type C Depressed Inlet with Grate.

DP1 is the Design Point representing the Private 3'x3' CDOT Type C Depressed Inlet with Grate for Sub-basin D-1. The Private Storm Inlet flows downstream to subsequent Design Point 6 before entering the Full Spectrum Extended Detention Basin via the Private Storm Sewer system. The total peak runoff flowing into DP1 is 1.14 cfs for the 10-year peak flow and 1.80 cfs for the 100-year peak flow. The emergency flow route for DP1 is an overflow condition of the Private Storm Inlet that would cause storm water pooling within the northern drive aisle and would flow due east into Canada Drive and ultimately to the Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-2 (0.45 ac. ; Q_{10} = 1.86 cfs, Q_{100} = 2.90 cfs) is the middle-north area of the development consisting of an asphalt drive access off of the common drive access for this property as well as enclosed storage and large vehicle parking area that consists of compacted gravel. The sub-basin is sloped southeast and overland flow is directed to the concrete drain pan within the drive aisle and channeled to a Private 3’x3’ CDOT Type C Depressed Inlet with Grate, the second in a series of storm inlets in the northern area.

DP2 is the Design Point representing the Private 3’x3’ CDOT Type C Depressed Inlet with Grate for Sub-basin D-2. The Private Storm Inlet flows downstream to subsequent Design Points 1 and 6 before entering the Full Spectrum Extended Detention Basin via the Private Storm Sewer system. The total peak runoff flowing into DP2 is 1.86 cfs for the 10-year peak flow and 2.90 cfs for the 100-year peak flow. The emergency flow route for DP2 is an overflow condition of the Private Storm Inlet that would cause storm water pooling within the northern drive aisle and would flow due east into Canada Drive and ultimately to the Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-3 (0.37 ac. ; Q_{10} = 1.70 cfs, Q_{100} = 2.63 cfs) is the northeast area of the development consisting of an asphalt drive access off of the common drive access for this property as well as enclosed storage and large vehicle parking area that consists of compacted gravel. The sub-basin is sloped southeast and overland flow is directed to the concrete drain pan within the drive aisle and channeled to a Private 3’x3’ CDOT Type C Depressed Inlet with Grate, the third in a series of storm inlets in the northern area.

DP3 is the Design Point representing the Private 3’x3’ CDOT Type C Depressed Inlet with Grate for Sub-basin D-3. The Private Storm Inlet flows downstream to subsequent Design Points 2, 1, and 6 before entering the Full Spectrum Extended Detention Basin via the Private Storm Sewer system. The total peak runoff flowing into DP3 is 1.70 cfs for the 10-year peak flow and 2.63 cfs for the 100-year peak flow. The emergency flow route for DP3 is an overflow condition of the Private Storm Inlet that would cause storm water pooling within the northern drive aisle and would flow due east into Canada Drive and ultimately to the Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-4 (0.73 ac. ; Q_{10} = 3.29 cfs, Q_{100} = 5.12 cfs) is the roof of the proposed building within the center of the lot. This sub-basin captures stormwater runoff from the roof and distributes it to roof drains along the northern edge of the building. Roof drains will be connected to the underground storm drain line under the concrete drainage pan within the north drive aisle. Runoff from this sub-basin is only included in the detention basin design point DP4.

DP4 is the Design Point representing the proposed roof runoff. The total peak runoff flowing into DP4 is 3.29 cfs for the 10-year peak flow and 5.12 cfs for the 100-year peak flow.

Sub-basin D-5 (0.50 ac. ; Q_{10} = 2.69 cfs, Q_{100} = 4.07 cfs) is the southern area of the development consisting of the south frontage parking lot. The sub-basin is generally sloped southeast within the parking lot with storm water conveyances via overland flow and curb and gutter. The sub-basin flows to a proposed Private 10’ CDOT Type R Curb Inlet and conveys it to the Full Spectrum Extended Detention Basin via the Private Storm Sewer system.

DP5 is the Design Point representing the Private 10’ CDOT Type R Curb Inlet for Sub-basin D-5. The Private Storm Inlet flows downstream to the Full Spectrum Extended Detention Basin via the Private Storm Sewer system. The total peak runoff flowing into DP5 is 2.69 cfs for the 10-year peak flow and 4.07 cfs for the 100-year peak flow. The emergency flow route for DP5 is to the east following proposed curb and gutter into Canada Drive and ultimately to the Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-6 (0.31 ac. ; Q_{10} = 1.58 cfs, Q_{100} = 2.42 cfs) is the tributary area within the property boundary that consists of the common drive access off of Bismark Road to be extended to the south parking lot frontage of the development. This sub-basin previously flowed southeast to the public storm system and is proposed to flow southeast to the proposed curb inlet within the south parking lot, DP-6. The existing drainage pattern along the western property line will be preserved to accomplish this.

DP6 is the Design Point representing the Private 10’ CDOT Type R Curb Inlet for Sub-basin D-6. The Private Storm Inlet flows downstream to the Full Spectrum Extended Detention Basin via the Private Storm Sewer system. The total peak runoff flowing into DP6 is 1.58 cfs for the 10-year peak flow and 2.42 cfs for the 100-year peak flow. The emergency flow route for DP6 is to the east following proposed curb and gutter into Canada Drive and ultimately to the Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-7 (0.14 ac. ; Q_{10} = 0.26 cfs, Q_{100} = 0.56 cfs) accounts for the tributary area of the development that consists of the Full Spectrum Extended Detention Basin footprint. Runoff in this area is directly collected by the basin.

DP7 is the Design Point representing the Full Spectrum Extended Detention Basin that is designed to detain a major storm event for the developed lot and provide Water Quality. The EDB is proposed to consist of 5.5-foot high structural walls with a detention volume of 24,000 cubic feet. The EDB consists of two forebays, concrete trickle channel, micropool, and outlet structure with an engineered orifice plate to comply with release rates for Water Quality Capture Volume, Excess Urban Runoff Volume, and the 100-Year Major Storm Event. The foundation drain of the EDB will be directed into the EDB and will have no effect on the design of the EDB. The emergency spillway of the detention basin is via a 6’ wide wall notch in the westerly wall that discharges into a riprap lined slope flowing due south to Constitution Avenue, ultimately flowing into the existing Public 15’ CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-8 (0.21 ac. ; Q_{10} = 0.17 cfs, Q_{100} = 0.66 cfs) is the southwest area of the development consisting of landscaping. The sub-basin flow is directed to the grass swale within the landscape area and channeled to a Private 3’x3’ CDOT Type C Depressed Inlet with Grate.

DP8 is the Design Point representing the Private 3'x3' CDOT Type C Depressed Inlet with Grate for Sub-basin D-8. The Private Storm Inlet flows downstream to subsequent Design Points via the Private Storm Sewer system. The total peak runoff flowing into DP8 is 0.17 cfs for the 10-year peak flow and 0.58 cfs for the 100-year peak flow. The emergency flow route for DP8 is to the east into Canada Drive and ultimately to the Public 15' CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

Sub-basin D-9 (0.63 ac. ; Q_{10} = 1.20 cfs, Q_{100} = 2.54 cfs) is the tributary area within the property boundary that flows toward the adjacent right of ways of Bismark Road, Canada Drive, and Constitution Avenue. The sub-basin consists mostly of proposed landscaped areas, public concrete sidewalk, and ingress/egress curb cuts. The sub-basin flows to Design Point 9 via the adjacent roadways' curb and gutter, ultimately to the Public 15' CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue.

DP9 is the Design Point representing the Public 15' CDOT Type R Curb Inlet at the northwest corner of the intersection of Canada Drive and Constitution Avenue. This Public Storm Inlet is a branch line of the Public Storm Main that runs west to east within Constitution Avenue and is the ultimate recipient of storm water flow from the developed lot as it is proposed to connect to the outlet structure of the EDB (DP7). The emergency flow route of this public storm inlet is due east along the north side of Constitution Avenue.

There is an existing Water Quality Capture Volume BMP/control measure constructed for the neighboring lot (Northcrest Center Fil No 2 Lots 1 & 2). This feature will not see additional runoff due to the proposed development, and was not evaluated for its current conditions.

The difference between Basin E and Basin D results in an overall increase of the 100-year storm Water volume of 14.71 cfs overall due to increased impervious surfaces.

2.65 acres (83.9% imperviousness) of on-site flows, and 0.31 acres of off-site flows drain to the Full Spectrum Detention Basin, with a total runoff of 20.19 cfs (100-yr storm) being captured.

A Full Spectrum Extended Detention Basin is proposed for the site to provide water quality and detention prior to attenuated storm water release to the public storm system. The vertical concrete walls on all four sides of the Extended Detention Basin are due to site constraints including an existing electric vault and existing easement where the pond is being constructed. The Full Spectrum Extended Detention Basin does include a 10' wide concrete maintenance access ramp with a grass bottom that slopes to the trickle channel.

It is anticipated that there will be no negative impact to downstream developments or infrastructure as a result of this development.

IV. DRAINAGE DESIGN CRITERIA

A. REGULATIONS

The hydrological and hydraulic calculations and design of the site conform to the City of Colorado Springs Drainage Criteria Manuals I and II (latest revision, May 2014) as well as the Mile High Flood District Drainage Criteria manuals revised August 2018.

B. DEVELOPMENT CRITERIA REFERENCE AND CONSTRAINTS

The parcel falls within the Sand Creek major drainage basin (East Fork Sand Creek) designated by the City of Colorado Springs Water Resources Engineering Department with the ultimate receiving waters of Arkansas River. The drainage on this parcel will have no effect on downstream infrastructure or facilities, streets, utilities, transit, or further development of adjacent lots. Relevant criteria for the calculations shown further include equations and design criteria for the rational method, volumes and runoff of carious storm events.

C. HYDROLOGICAL CRITERIA

The rational method was used to calculate the peak runoff of the delineated sub-basins using the manuals referenced prior with the C, I, and P1 values from the Design Criteria Manual Volume I, Chapter 6 as well as the Colorado Springs designated IDF curve values. Specific calculations and tables are provided further with inputs including design rainfall, sub-basin acreage and percent imperviousness, runoff coefficients, one-hour rainfall depths, rainfall intensities, time of concentration, and peak discharge of various storm events. The default rainfall intensities and volumes use runoff coefficients based on soil types. Weighted runoff coefficients were calculated for each basin and sub-basin due to the mix of impervious surfaces, shown in the Appendix exhibits.

D. FOUR-STEP PROCESS

The selection of appropriate control measures is based on the characteristics of the site and potential pollutants. The Four-Step Process provides a method of going through the selection process. The following applies the four-step process to the Development Plan for the Northcrest Center.

Step 1: Employ Runoff Reduction Practices

The Development Plan including the Landscape Plan utilizes landscaping areas for plantings and grass or mulch wherever possible without obstructing utilities or drainageways. Given the proposed land use, the majority of the site is to be paved for vehicular use. Within the mostly-impervious site, the storm water runoff is kept to the site limits where possible by use of grading, a private storm system and all other areas are to be landscaped, including the right of way landscape buffer areas.

Step 2: Provide Water Quality Capture Volume

The Development Plan and Final Drainage Report indicate the use of a storm water detention pond as a control measure for capturing storm water runoff and properly treating the storm water prior to release either via percolation into the soil or attenuated to the public storm system. The detention pond is to be installed and the configuration is sized for capture of the WQCV as well as the EURV and full-spectrum detention, and 100-year detention.

Step 3: Stabilize Drainageways

The drainage within the site is stabilized by way of pavement with features such as drainage pans, curb and gutter, and sloped pavement to direct storm water to the private storm system. There are no unpaved or unstabilized drainageways on this site.

Step 4: Implement Site Specific and Other Source Control BMPs

No control measures beyond the Full Spectrum Extended Detention Basin are proposed as there are no unusual land uses that would result in the need for other source control BMPs.

V. DRAINAGE INFRASTRUCTURE COSTS AND FEES

A. DRAINAGE AND BRIDGE FEES

The development falls within the Sand Creek drainage basin (FOFO4000) which has a drainage basin fee of $20,387 per impervious acre and a bridge fee of $8,339 per impervious acre according to the 2021 El Paso County Drainage Basin Fees document. The development has a total impervious acreage of 2.72acres (3.24acres * 83.9% imperviousness).

Drainage Basin Fee: $20,387/impervious acre * 2.72 impervious acres = $55,452.64

Bridge Fee: $8,339/impervious acre * 2.72 impervious acres = $22,682.08

Since the site is already platted, drainage fees are assumed to have already been paid. Since this development is increasing imperviousness, the County shall review their records and make a decision on fee requirements.

Any outstanding fees must be paid prior to plat recordation.

B. STORM DRAIN SYSTEM QUANTITIES AND COSTS ESTIMATE

The following summarizes the Engineer’s Opinion of Probable Cost for the proposed storm facilities for the development (installation plus materials).

Private System

Description	Quantity	Unit	Unit Price	Cost
Earthwork for cut of Pond (Less than 1,000)	612	CY	$ 8	$ 4,896
12” RCP	230	LF	$ 55	$ 12,650
18” RCP	226	LF	$ 65	$ 14,690
24” RCP	74	LF	$ 78	$ 5,772
Curb Inlet (Type R) L=10’ 5’ ≤ Depth < 10’	2	EA	$ 8,136	$ 16,272
Pond Outlet Structure	1	EA	$ 10,000	$ 10,000
Grated Inlet (Type C) Depth < 5’	4	EA	$ 4,802	$ 19,208
Storm Sewer Manhole, Box Base	2	EA	$ 12,034	$ 24,068
Concrete Basin Walls and Safety Railing	1	EA	$20,000	$20,000

Total Cost Engineering	**$127,556.00**
Contingency (10%)	**$ 12,755.60**
Grand Total (w/ Contingency) Non-Reimbursable	**$140,311.60**

VI. CONCLUSIONS

A. COMPLIANCE WITH STANDARDS

The criteria used to design the storm water runoff volumes are formulas and figures within the City of Colorado Springs Drainage Manuals as well as the Mile High Flood District Drainage Criteria manual. Grading practices for optimal drainage shall comply with the geotechnical investigative report and City standards. The development of Lots 3-5 is within compliance and standards and meets the requirements for the Northcrest Center.

The proposed grading and drainage is within substantial conformance for the master drainage plan for the Subdivision and Drainage Basin. There is no impact on major drainageway planning studies within the larger drainage basin. This development will not adversely affect downstream development.

VII. References

Colorado Springs Drainage Manual Volumes I & II (May 2014)

Colorado Urban Drainage and Flood Control District Drainage Criteria Manual, Volume I (August 2018)

Colorado Urban Drainage and Flood Control District Drainage Criteria Manual, Volume III (April 2018)

Urban Storm Drainage Criteria Manual, Volume III (November, 2015)

FEMA Flood Map Service Center

United States Department of Agriculture National Resources Conservation Service

Subsurface Soil Investigation prepared by RMG-Rocky Mountain Group Engineers dated February 23, 2021

VIII. Appendices

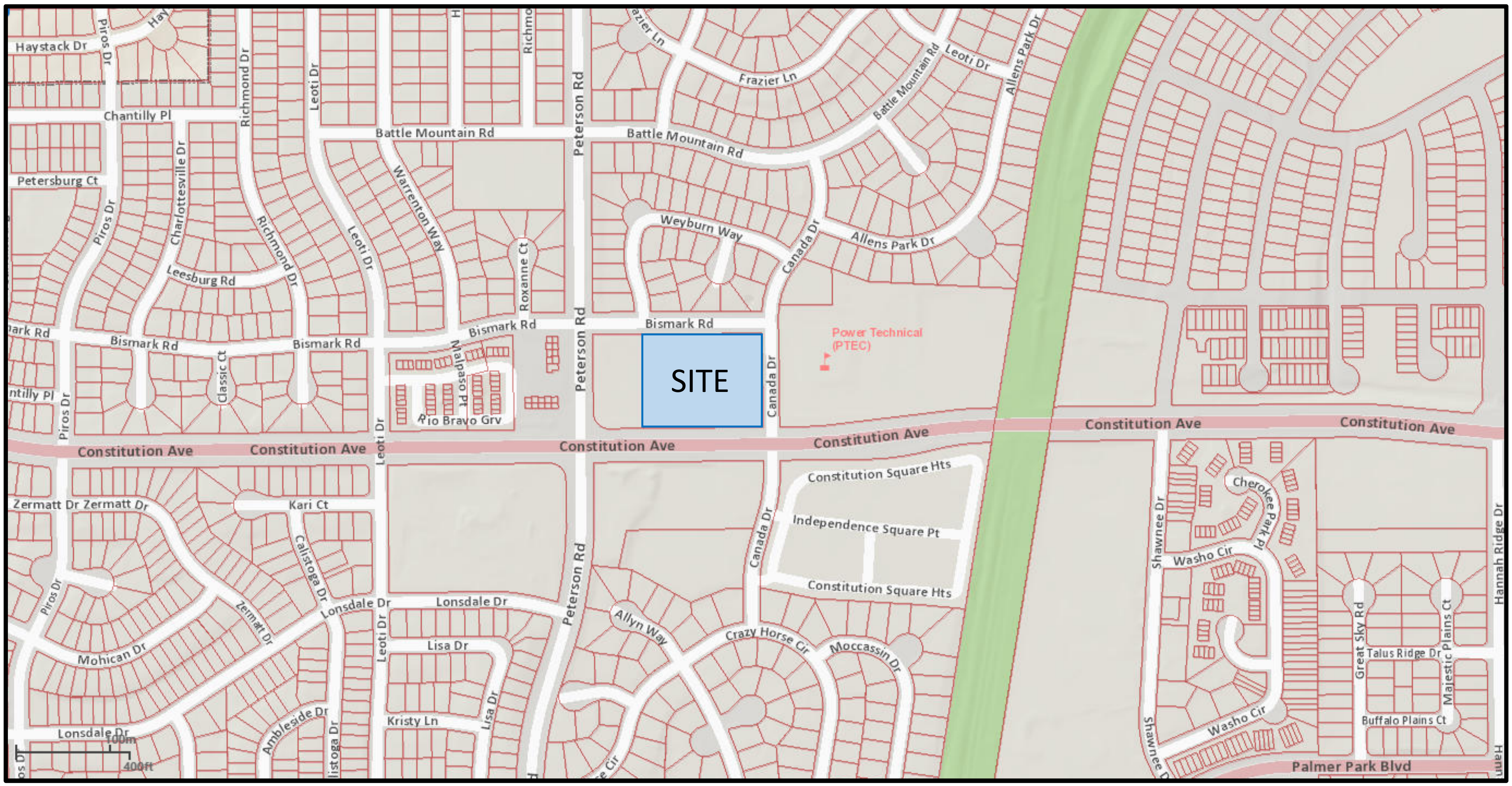
APPENDIX A – VICINITY MAP

VICINITY MAP

NORTHCREST CENTER

TBD BISMARK ROAD
COLORADO SPRINGS, COLORADO 80922
EL PASO COUNTY

N



Appendix B – Hydrologic and Hydraulic Computations

Project: Northcrest Center PEMBs Development - EXISTING CONDITIONS
Engineer: David Walker, PE
Date: 11/11/2022
Address: 2510 & 2522 Canada Dr

Sub-Basin:	E-1	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	15.15				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
2.800816441	3.506298035	4.0908477	4.6753974	5.2599471	5.8861407

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000
Pavement	2155	0.049	0.89	0.90	0.92	0.94	0.95	0.96	0.044	0.045	0.046	0.047	0.047	0.047
Lawn	139193	3.195	0.02	0.08	0.15	0.25	0.30	0.35	0.064	0.256	0.479	0.799	0.959	1.118
A_t :	141348.11	3.245												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.033	**0.093**	**0.162**	**0.261**	**0.310**	**0.359**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
0.30	**1.05**	**2.15**	**3.95**	**5.29**	**6.86**

Design Points

Design Point	Q_{10}	Q_{100}
EX DP1	2.15	6.86
TOTAL SITE	**2.15**	**6.86**

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	E-1	
C_{10}:	0.16	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.045	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + ...C_iA_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	2155	0.05	0.92
Lawn	139193	3.20	0.15
A_t :	141348	3.24	

$C_c = (0.92*0.05 + 0.15*3.20) / 3.24 =$ **0.16**

$t_i = (0.395*(1.1\text{-}C10)*sqrt(L))/(S\text{^}0.33)$

$t_i = (0.395*(1.1\text{-}0.09)*sqrt(550))/($ **0.045** $\text{^}0.33) =$ **10.31** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.006)^{0.5} =$ **1.55** ft/s

Flow Distance: **450.00** ft

$t_t = L/V =$ 290.47 sec.
4.84 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **15.15** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **15.15** min.

Project:	**Northcrest Center PEMBs Commercial Development - DEVELOPED CONDITIONS**
Engineer:	**David Walker, PE**
Date:	**11/8/2022**
Address:	**2510 & 2522 Canada Dr**

Sub-Basin:	**D-1**	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	**5.91**				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
3.919923358	4.916937006	5.7365932	6.5562493	7.3759055	8.2560142

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000
Pavement	4720	0.108	0.89	0.90	0.92	0.94	0.95	0.96	0.096	0.098	0.100	0.102	0.103	0.104
Lawn	1217	0.028	0.02	0.08	0.15	0.25	0.30	0.35	0.001	0.002	0.004	0.007	0.008	0.010
Gravel	6507	0.149	0.57	0.59	0.63	0.66	0.68	0.70	0.085	0.088	0.094	0.099	0.102	0.105
A_t:	12444	0.29												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.638	**0.658**	**0.693**	**0.726**	**0.745**	**0.764**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
0.71	**0.92**	**1.14**	**1.36**	**1.57**	**1.80**

Sub-Basin:	**D-2**	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	**5.89**				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
3.925794957	4.924338181	5.7452279	6.5661176	7.3870073	8.2684481

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000
Pavement	6183	0.142	0.89	0.90	0.92	0.94	0.95	0.96	0.126	0.128	0.131	0.133	0.135	0.136
Lawn	0	0.000	0.02	0.08	0.15	0.25	0.30	0.35	0.000	0.000	0.000	0.000	0.000	0.000
Gravel	13359	0.307	0.57	0.59	0.63	0.66	0.68	0.70	0.175	0.181	0.193	0.202	0.209	0.215
A_t:	19542	0.45												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.671	**0.688**	**0.722**	**0.749**	**0.765**	**0.782**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
1.18	**1.52**	**1.86**	**2.21**	**2.54**	**2.90**

Sub-Basin:	**D-3**	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	**5.00**				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
4.119768884	5.168843131	6.0304837	6.8921242	7.7537647	8.6792165

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000
Pavement	7141	0.164	0.89	0.90	0.92	0.94	0.95	0.96	0.146	0.148	0.151	0.154	0.156	0.157
Lawn	0	0.000	0.02	0.08	0.15	0.25	0.30	0.35	0.000	0.000	0.000	0.000	0.000	0.000
Gravel	9050	0.208	0.57	0.59	0.63	0.66	0.68	0.70	0.118	0.123	0.131	0.137	0.141	0.145
A_t:	16191	0.37												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.711	**0.727**	**0.758**	**0.783**	**0.799**	**0.815**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
1.09	**1.40**	**1.70**	**2.01**	**2.30**	**2.63**

Sub-Basin:	**D-4**	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	**5.00**				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
4.119768884	5.168843131	6.0304837	6.8921242	7.7537647	8.6792165

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	31733	0.728	0.71	0.73	0.75	0.78	0.80	0.81	0.517	0.532	0.546	0.568	0.583	0.590
Pavement	0	0.000	0.89	0.90	0.92	0.94	0.95	0.96	0.000	0.000	0.000	0.000	0.000	0.000
Lawn	0	0.000	0.02	0.08	0.15	0.25	0.30	0.35	0.000	0.000	0.000	0.000	0.000	0.000
A_t:	31733	0.73												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.710	**0.730**	**0.750**	**0.780**	**0.800**	**0.810**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
2.13	**2.75**	**3.29**	**3.92**	**4.52**	**5.12**

Sub-Basin:	**D-5**	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	**5.00**				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
4.119768884	5.168843131	6.0304837	6.8921242	7.7537647	8.6792165

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000
Pavement	20994	0.482	0.89	0.90	0.92	0.94	0.95	0.96	0.429	0.434	0.443	0.453	0.458	0.463
Lawn	784	0.018	0.02	0.08	0.15	0.25	0.30	0.35	0.000	0.001	0.003	0.004	0.005	0.006
A_t:	21778	0.50												

2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
0.859	**0.870**	**0.892**	**0.915**	**0.927**	**0.938**

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
1.77	**2.25**	**2.69**	**3.15**	**3.59**	**4.07**

Sub-Basin:	D-6	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	5.00				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
4.119768884	5.168843131	6.0304837	6.8921242	7.7537647	8.6792165

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$	2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000	**0.818**	**0.832**	**0.856**	**0.883**	**0.896**	**0.910**
Pavement	12249	0.281	0.89	0.90	0.92	0.94	0.95	0.96	0.250	0.253	0.259	0.264	0.267	0.270						
Lawn	1104	0.025	0.02	0.08	0.15	0.25	0.30	0.35	0.001	0.002	0.004	0.006	0.008	0.009						
A_t:	13353	0.31																		

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
1.03	**1.32**	**1.58**	**1.87**	**2.13**	**2.42**

Sub-Basin:	D-7	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	5.49				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
4.008564235	5.028669204	5.8669474	6.7052256	7.5435038	8.4437243

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$	2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000	**0.295**	**0.339**	**0.394**	**0.468**	**0.506**	**0.543**
Pavement	1985	0.046	0.89	0.90	0.92	0.94	0.95	0.96	0.041	0.041	0.042	0.043	0.043	0.044						
Lawn	4290	0.098	0.02	0.08	0.15	0.25	0.30	0.35	0.002	0.008	0.015	0.025	0.030	0.034						
A_t:	6275	0.14																		

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
0.17	**0.25**	**0.33**	**0.45**	**0.55**	**0.66**

Sub-Basin:	D-8	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	6.40				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
3.826289583	4.79891124	5.5988964	6.3988817	7.1988669	8.0577309

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$	2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000	**0.020**	**0.080**	**0.150**	**0.250**	**0.300**	**0.350**
Pavement	0	0.000	0.89	0.90	0.92	0.94	0.95	0.96	0.000	0.000	0.000	0.000	0.000	0.000						
Lawn	8969	0.206	0.02	0.08	0.15	0.25	0.30	0.35	0.004	0.016	0.031	0.051	0.062	0.072						
A_t:	8969	0.21																		

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
0.02	**0.08**	**0.17**	**0.33**	**0.44**	**0.58**

Sub-Basin:	D-9	(IDF Curve Equations from Figure 6-5 of the DCM Volume 1)			
t_t Duration:	11.17				
I_2	I_5	I_{10}	I_{25}	I_{50}	I_{100}
3.163099546	3.962957411	4.623617	5.2842765	5.9449361	6.6533285

Hydrologic Soil Type:	A

Coefficient (Table 6-6)

Land Use or Surface Characteristic	Square Feet	Acreage	Coefficient $_2$	Coefficient $_5$	Coefficient $_{10}$	Coefficient $_{25}$	Coefficient $_{50}$	Coefficient $_{100}$	2 Yr: $C_i * A_i$	5 Yr: $C_i * A_i$	10 Yr: $C_i * A_i$	25 Yr: $C_i * A_i$	50 Yr: $C_i * A_i$	100 Yr: $C_i * A_i$	2 Yr C_c	5 Yr C_c	10 Yr C_c	25 Yr C_c	50 Yr C_c	100 Yr C_c
Roof	0	0.000	0.71	0.73	0.75	0.78	0.80	0.81	0.000	0.000	0.000	0.000	0.000	0.000	**0.232**	**0.280**	**0.338**	**0.418**	**0.459**	**0.499**
Pavement	6715	0.154	0.89	0.90	0.92	0.94	0.95	0.96	0.137	0.139	0.142	0.145	0.146	0.148						
Lawn	20812	0.478	0.02	0.08	0.15	0.25	0.30	0.35	0.010	0.038	0.072	0.119	0.143	0.167						
A_t:	27527	0.63																		

Q Peak Flow (cfs)

2 Year Q	5 Year Q	10 Year Q	25 Year Q	50 Year Q	100 Year Q
0.56	**0.85**	**1.20**	**1.69**	**2.09**	**2.54**

Design Points

Design Points	Q_{10}	Q_{100}
DP1	1.14	1.80
DP2	1.86	2.90
DP3	1.70	2.63
DP4	3.29	5.12
DP5	2.69	4.07
DP6	1.58	2.42
DP7	0.33	0.66
DP8	0.17	0.58
TOTAL ON-SITE	12.77	20.19
DP9	1.20	2.54
TOTAL OFF-SITE	1.20	2.54

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-1	
C_{10}:	0.69	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.025	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + ...C_iA_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	4720	0.11	0.92
Lawn	1217	0.03	0.15
Gravel	6507	0.15	0.63
A_t :	12444	0.29	

C_c = (0.92*0.11 + 0.15*0.03 + 0.63*0.15) / 0.29 = **0.69**

t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)

t_i = (0.395*(1.1-0.48)*sqrt(145))/(**0.055** ^0.33) = **5.47** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \qquad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.025)^{0.5}$ = **3.16** ft/s

Flow Distance: **85.00** ft

t_t = L/V = 26.88 sec.
0.45 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **5.91** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.91** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-2	
C_{10}:	0.72	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.025	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + ...C_iA_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	6183	0.14	0.92
Lawn	0	0.00	0.15
Gravel	13359	0.31	0.63
A_t :	19542	0.45	

C_c = (0.92*0.14 + 0.63*0.31) / 0.45 = **0.72**

t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)
t_i = (0.395*(1.1-0.39)*sqrt(124))/(**0.03** ^0.33) = **5.05** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$
$V = (20)(0.025)^{0.5}$ = **3.16** ft/s

Flow Distance: **159.00** ft

t_t = L/V = 50.28 sec.
0.84 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = 5.89 min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: 5.89 min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-3	
C_{10}:	**0.76**	[Table 6-6. Runoff Coefficients for Rational Method]
L:	**100**	ft
S:	**0.025**	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + ...C_i A_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	7141	0.16	0.92
Lawn	0	0.00	0.15
Gravel	9050	0.21	0.63
A_t :	16191	0.37	

C_c = (0.92*0.16 + 0.63*0.21) / 0.37 = **0.76**

t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)

t_i = (0.395*(1.1-0.90)*sqrt(124))/(**0.03** ^0.33) = **4.56** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.025)^{0.5}$ = **3.16** ft/s

Flow Distance: **82.00** ft

t_t = L/V = 25.93 sec.
0.43 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **5.00** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.00** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-4	
C_{10}:	0.75	[Table 6-6. Runoff Coefficients for Rational Method]
L:	95	ft
S:	0.083	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + ...C_i A_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	31714	0.73	0.75
Pavement	0	0.00	0.92
Lawn	0	0.000	0.15
A_t :	31714	0.73	

C_c = (0.75*0.73) / 0.73 = **0.75**

t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)

t_i = (0.395*(1.1- **0.87**)*sqrt(90))/(**0.035** ^0.33) = **3.06** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.042)^{0.5}$ = **0.00** ft/s

Flow Distance: **0.00** ft

t_t = L/V = 0.00 sec.
0.00 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **3.06** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.00** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-5	
C_{10}:	0.89	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.035	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + ...C_i A_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	21716	0.50	0.92
Lawn	784	0.018	0.15
A_t :	22500	0.517	

*C_c = (0.92*0.50 + 0.15*0.018) / 0.515 =* **0.89**

t_i = (0.395(1.1-C_{10})*sqrt(L))/(S^0.33)*

t_i = (0.395(1.1- **0.88**)*sqrt(125))/(**0.035** ^0.033) =* **2.47** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)

C_v = conveyance coefficient (from Table 6-7)

S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.035)^{0.5}$ = **3.74** ft/s

Flow Distance: **166.00** ft

t_t = L/V = 44.37 sec.

0.74 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **3.21** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.00** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-6	
C_{10}:	0.86	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.07	ft/ft

Composite Runoff Coefficient Calculation:
$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + ...C_iA_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	12249	0.28	0.92
Lawn	1104	0.03	0.15
A_t :	13353	0.31	

$C_c = (0.92*0.28 + 0.15*0.03) / 0.31 =$ **0.86**

$t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)$
$t_i = (0.395*(1.1-$ **0.33** $)*sqrt(7))/($ **0.1** $^0.33) =$ **2.31** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$
$V = (20)(0.053)^{0.5} =$ **4.60** ft/s

Flow Distance: **188.00** ft

$t_t = L/V =$ 40.83 sec.
0.68 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t =$ **3.00** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.00** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-7	
C_{10}:	0.40	[Table 6-6. Runoff Coefficients for Rational Method]
L:	15	ft
S:	0.01	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + ...C_iA_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	1985	0.05	0.92
Lawn	4119	0.09	0.15
A_t :	6104	0.14	

*C_c = (0.75*0.0 + 0.92*0.10 + 0.15*0.07) / 0.08 =* **0.40**

t_i = (0.395(1.1-C_{10})*sqrt(L))/(S^0.33)*
t_i = (0.395(1.1- **0.20**)*sqrt(15))/(**0.01** ^0.33) =* **4.89** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \qquad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$
$V = (20)(0.01)^{0.5}$ = **2.00** ft/s

Flow Distance: **66.00** ft

t_t = L/V = 33.00 sec.
0.55 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **5.44** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **5.44** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-8	
C_{10}:	0.15	[Table 6-6. Runoff Coefficients for Rational Method]
L:	25	ft
S:	0.04	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + ...C_i A_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	0	0.00	0.92
Lawn	8402	0.19	0.15
A_t :	8402	0.19	

$C_c = (0.15*0.25) / 0.25 =$ **0.15**

$t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^{0.33})$

$t_i = (0.395*(1.1-\mathbf{0.35})*sqrt(46))/(\mathbf{0.25}^{0.33}) =$ **5.43** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.04)^{0.5} =$ **4.00** ft/s

Flow Distance: **233.00** ft

$t_t = L/V =$ 58.25 sec.
0.97 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **6.40** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

Final t_c: **6.40** min.

Time of Concentration $t_c = t_i + t_t$

3.2.1 - Overland (Initial) Flow Time

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft <u>maximum</u> for non-urban land uses, 100 ft <u>maximum</u> for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

Sub-Basin or DP:	D-9	
C_{10}:	0.34	[Table 6-6. Runoff Coefficients for Rational Method]
L:	100	ft
S:	0.04	ft/ft

Composite Runoff Coefficient Calculation:

$C_c = (C_1 A_1 + C_2 A_2 + C_3 A_3 + \ldots C_i A_i) / A_t$

Land Use or Surface Characteristic	Square Feet	Acreage	C_{10}
Roof	0	0.00	0.75
Pavement	6715	0.15	0.92
Lawn	20727	0.48	0.15
A_t :	27442	0.63	

C_c = (0.15*0.39 + 0.92*0.10) / 0.50 = **0.34**

t_i = (0.395*(1.1-C_{10})*sqrt(L))/(S^0.33)

t_i = (0.395*(1.1- **0.35**)*sqrt(46))/(**0.25** ^0.33) = **8.70** mins

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \qquad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)
C_v = conveyance coefficient (from Table 6-7)
S_w = watercourse slope (ft/ft)

$V = C_v S_w^{0.5}$

$V = (20)(0.04)^{0.5}$ = **4.00** ft/s

Flow Distance: **591.00** ft

t_t = L/V = 147.75 sec.

2.46 min.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

$t_c = t_i + t_t$ = **11.16** min.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

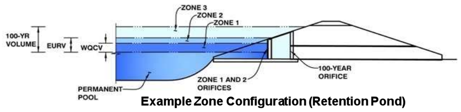
Final t_c: **11.16** min.

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **Northcrest Center**

Basin ID: **EDB, Full Spectrum Extended Detention Basin**



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	3.02	acres
Watershed Length =	400	ft
Watershed Length to Centroid =	175	ft
Watershed Slope =	0.050	ft/ft
Watershed Imperviousness =	86.46%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.094	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.351	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.219	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.283	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	0.334	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	0.391	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	0.447	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	0.511	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.48 in.) =	0.731	acre-feet	3.48	inches
Approximate 2-yr Detention Volume =	0.231	acre-feet		
Approximate 5-yr Detention Volume =	0.300	acre-feet		
Approximate 10-yr Detention Volume =	0.357	acre-feet		
Approximate 25-yr Detention Volume =	0.422	acre-feet		
Approximate 50-yr Detention Volume =	0.460	acre-feet		
Approximate 100-yr Detention Volume =	0.493	acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.094	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.257	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.142	acre-feet
Total Detention Basin Volume =	0.493	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

Depth Increement = 0.20 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	100	0.002		
	--	0.20	--	--	--	907	0.021	101	0.002
	--	0.40	--	--	--	2,357	0.054	427	0.010
	--	0.60	--	--	--	3,257	0.075	988	0.023
	--	0.80	--	--	--	3,873	0.089	1,701	0.039
	--	1.00	--	--	--	4,400	0.101	2,529	0.058
	--	1.20	--	--	--	4,846	0.111	3,453	0.079
	--	1.40	--	--	--	4,876	0.112	4,426	0.102
	--	1.60	--	--	--	4,905	0.113	5,404	0.124
	--	1.80	--	--	--	4,935	0.113	6,388	0.147
	--	2.00	--	--	--	4,964	0.114	7,378	0.169
	--	2.20	--	--	--	4,994	0.115	8,373	0.192
	--	2.40	--	--	--	5,024	0.115	9,375	0.215
	--	2.60	--	--	--	5,053	0.116	10,383	0.238
	--	2.80	--	--	--	5,083	0.117	11,396	0.262
	--	3.00	--	--	--	5,113	0.117	12,416	0.285
	--	3.20	--	--	--	5,142	0.118	13,442	0.309
	--	3.40	--	--	--	5,172	0.119	14,473	0.332
	--	3.60	--	--	--	5,202	0.119	15,510	0.356
	--	3.80	--	--	--	5,232	0.120	16,554	0.380
	--	4.00	--	--	--	5,267	0.121	17,604	0.404
	--	4.20	--	--	--	5,297	0.122	18,660	0.428
	--	4.40	--	--	--	5,327	0.122	19,722	0.453
	--	4.60	--	--	--	5,358	0.123	20,791	0.477
	--	4.80	--	--	--	5,388	0.124	21,866	0.502
	--	5.00	--	--	--	5,419	0.124	22,946	0.527
	--	5.20	--	--	--	5,451	0.125	24,033	0.552
	--	5.40	--	--	--	5,480	0.126	25,126	0.577
	--	5.60	--	--	--	5,513	0.127	26,226	0.602
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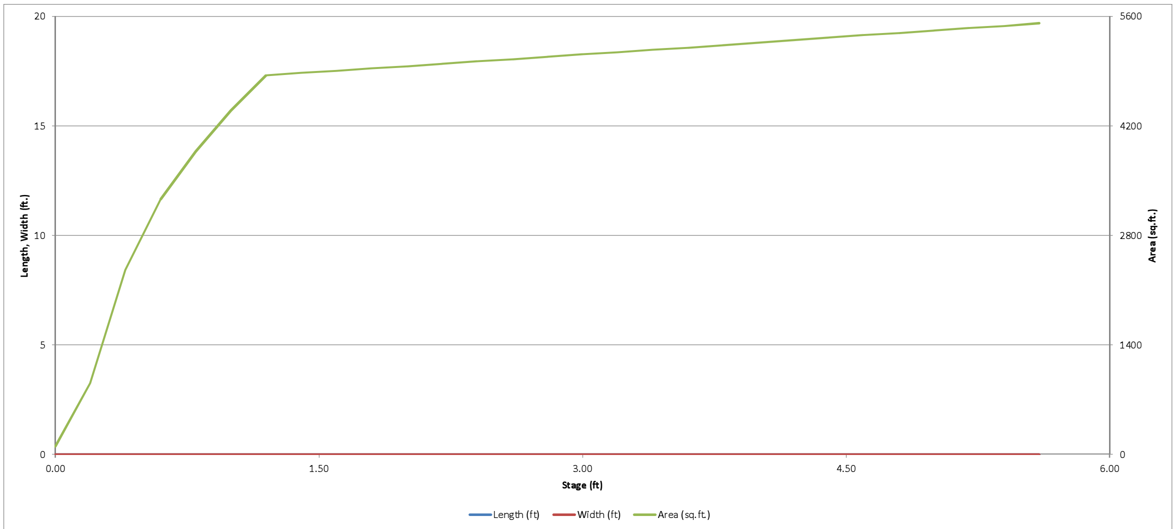
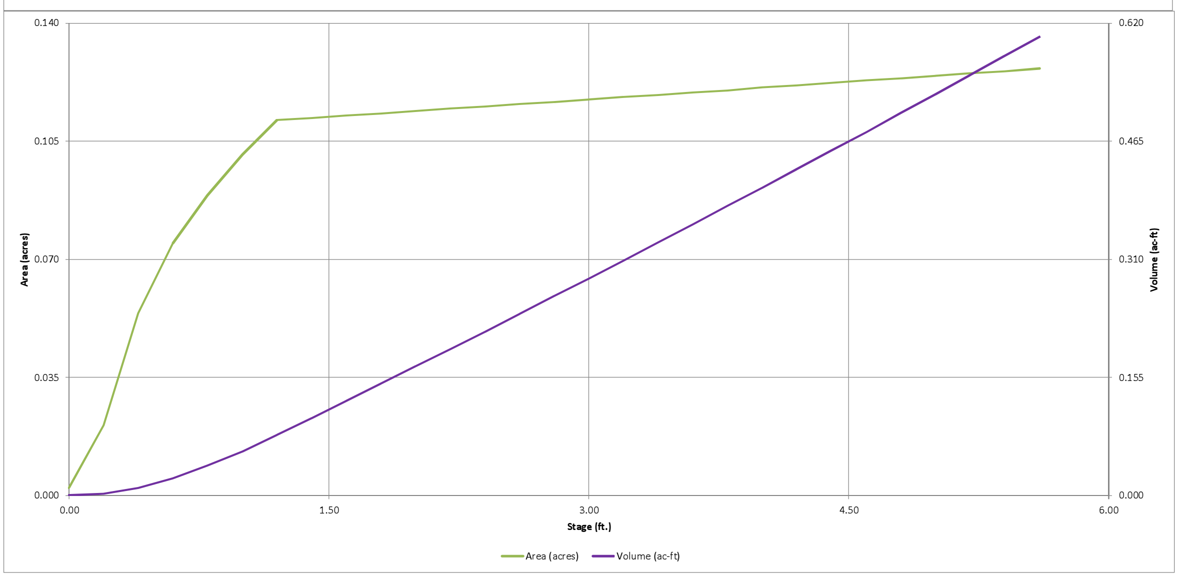
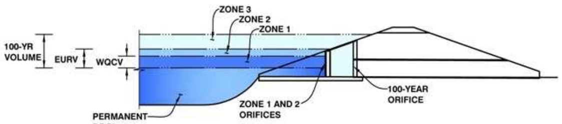

Length, Width (ft.)
Area (sq.ft.)
Stage (ft)
Length (ft)
Width (ft)
Area (sq.ft.)


Area (acres)
Volume (ac-ft)
Stage (ft.)
Area (acres)
Volume (ac-ft)

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Northcrest Center**
Basin ID: **EDB, Full Spectrum Extended Detention Basin**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.34	0.094	Orifice Plate
Zone 2 (EURV)	3.56	0.257	Orifice Plate
Zone 3 (100-year)	4.73	0.142	Weir&Pipe (Restrict)
Total (all zones)		0.493	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	3.56	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.89	1.78	2.67				
Orifice Area (sq. inches)	0.79	0.79	0.44	0.44				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.56	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	1.25	N/A	feet
Overflow Weir Grate Slope =	3.00	N/A	H:V
Horiz. Length of Weir Sides =	2.50	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	0%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	4.39	N/A	feet
Overflow Weir Slope Length =	2.64	N/A	feet
Grate Open Area / 100-yr Orifice Area =	5.72	N/A	
Overflow Grate Open Area w/o Debris =	2.29	N/A	ft^2
Overflow Grate Open Area w/ Debris =	2.29	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.50	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	5.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.40	N/A	ft^2
Outlet Orifice Centroid =	0.25	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.11	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	3.75	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	6.00	feet
Spillway End Slopes =	0.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth=	0.78	feet
Stage at Top of Freeboard =	5.53	feet
Basin Area at Top of Freeboard =	0.13	acres
Basin Volume at Top of Freeboard =	0.59	acre-ft

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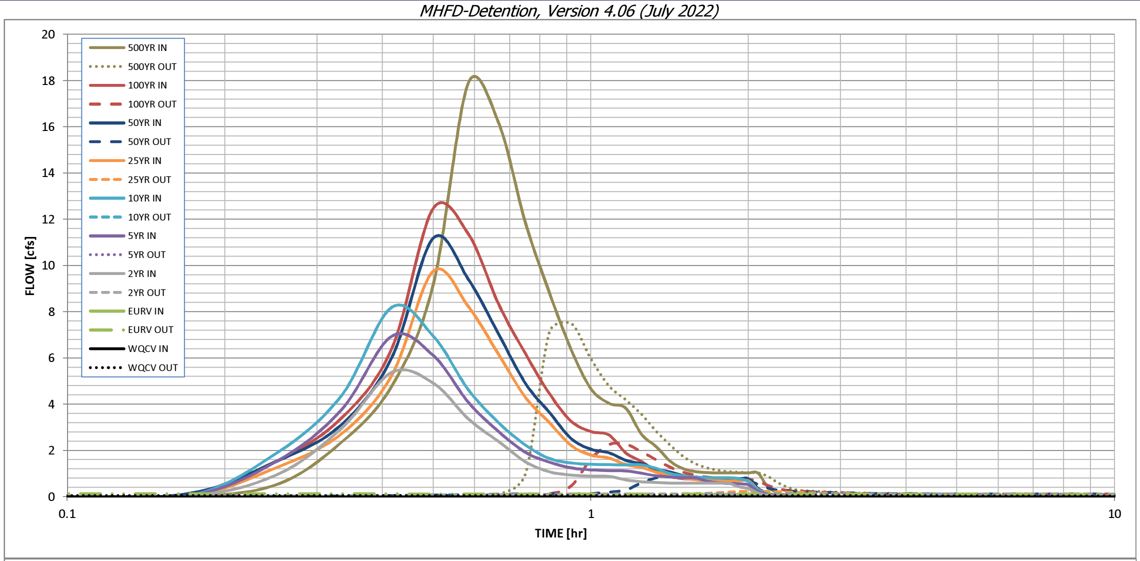
Routed Hydrograph Results

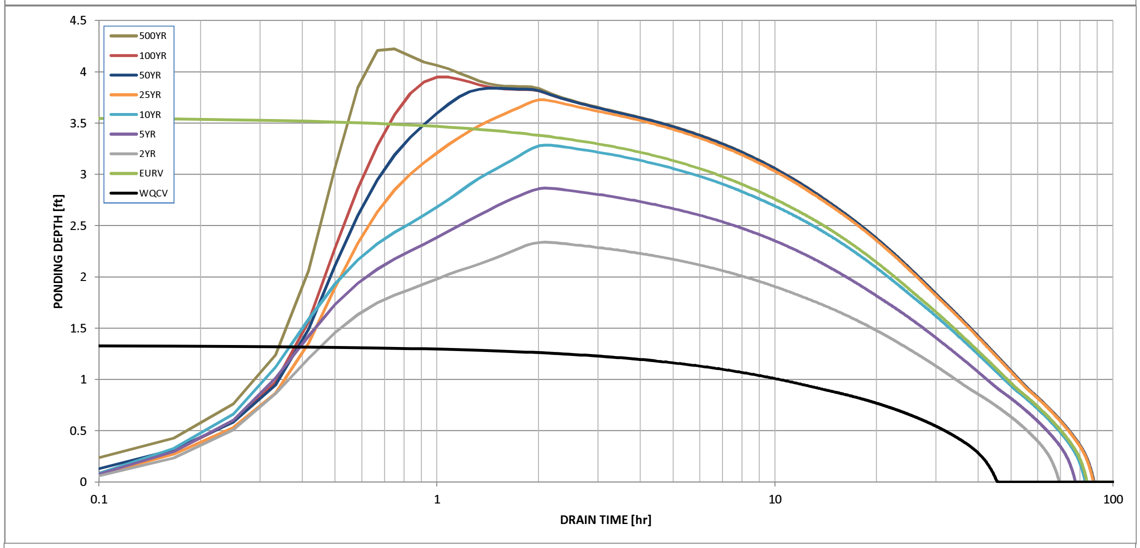
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

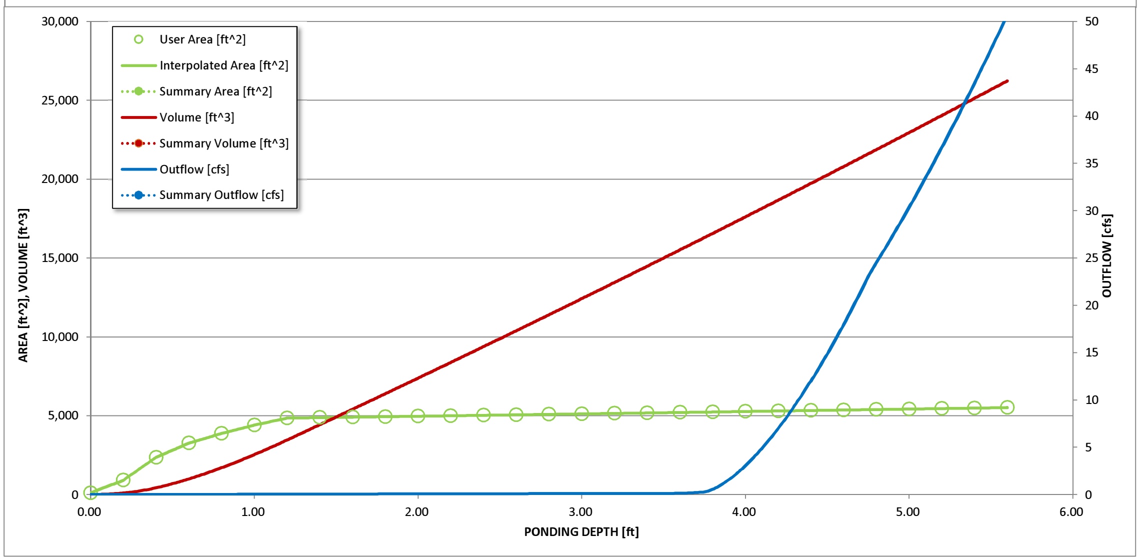
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.48
CUHP Runoff Volume (acre-ft) =	0.094	0.351	0.219	0.283	0.334	0.391	0.447	0.511	0.731
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.219	0.283	0.334	0.391	0.447	0.511	0.731
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.1	0.1	0.9	1.7	2.8	6.2
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A	0.3	1.1	2.2	4.0	5.3	6.9	8.0
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.10	0.35	0.71	1.31	1.75	2.27	2.65
Peak Inflow Q (cfs) =	N/A	N/A	5.4	6.9	8.2	9.8	11.2	12.5	17.9
Peak Outflow Q (cfs) =	0.0	0.1	0.1	0.1	0.1	0.2	0.9	2.3	7.5
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.1	0.1	0.1	0.2	0.3	0.9
Structure Controlling Flow =	Plate	Overflow Weir 1	Plate	Plate	Plate	Overflow Weir 1	Spillway	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.0	0.1	0.2	0.6
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	42	74	62	69	73	77	76	75	71
Time to Drain 99% of Inflow Volume (hours) =	**44**	79	66	74	79	83	83	82	80
Maximum Ponding Depth (ft) =	1.34	3.56	2.34	2.87	3.28	3.73	3.84	3.95	4.22
Area at Maximum Ponding Depth (acres) =	0.11	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
Maximum Volume Stored (acre-ft) =	0.095	0.351	0.207	0.269	0.318	0.370	0.384	0.398	0.431

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.10	0.01	0.51
	0:15:00	0.00	0.00	0.93	1.51	1.86	1.25	1.52	1.51	2.34
	0:20:00	0.00	0.00	2.94	3.74	4.35	2.71	3.10	3.38	4.79
	0:25:00	0.00	0.00	5.38	6.95	8.20	5.25	6.02	6.41	9.17
	0:30:00	0.00	0.00	4.92	6.11	6.95	9.75	11.18	12.47	17.90
	0:35:00	0.00	0.00	3.37	4.09	4.63	8.24	9.41	11.37	16.17
	0:40:00	0.00	0.00	2.39	2.82	3.20	6.15	7.03	8.33	11.86
	0:45:00	0.00	0.00	1.51	1.92	2.24	4.30	4.90	6.19	8.83
	0:50:00	0.00	0.00	1.08	1.49	1.65	3.21	3.65	4.46	6.38
	0:55:00	0.00	0.00	0.94	1.25	1.47	2.22	2.52	3.27	4.68
	1:00:00	0.00	0.00	0.89	1.16	1.41	1.81	2.06	2.82	4.04
	1:05:00	0.00	0.00	0.88	1.13	1.39	1.67	1.90	2.67	3.83
	1:10:00	0.00	0.00	0.73	1.12	1.38	1.39	1.57	1.89	2.70
	1:15:00	0.00	0.00	0.65	1.01	1.38	1.27	1.43	1.52	2.16
	1:20:00	0.00	0.00	0.61	0.91	1.22	1.06	1.20	1.10	1.54
	1:25:00	0.00	0.00	0.59	0.85	1.01	0.96	1.08	0.87	1.21
	1:30:00	0.00	0.00	0.59	0.83	0.90	0.81	0.91	0.79	1.09
	1:35:00	0.00	0.00	0.59	0.82	0.85	0.74	0.83	0.75	1.04
	1:40:00	0.00	0.00	0.59	0.68	0.83	0.71	0.80	0.75	1.03
	1:45:00	0.00	0.00	0.59	0.61	0.82	0.69	0.78	0.75	1.03
	1:50:00	0.00	0.00	0.59	0.58	0.82	0.69	0.78	0.75	1.03
	1:55:00	0.00	0.00	0.43	0.56	0.78	0.69	0.78	0.75	1.03
	2:00:00	0.00	0.00	0.36	0.52	0.66	0.69	0.78	0.75	1.03
	2:05:00	0.00	0.00	0.16	0.24	0.31	0.33	0.37	0.35	0.49
	2:10:00	0.00	0.00	0.07	0.11	0.13	0.14	0.16	0.16	0.21
	2:15:00	0.00	0.00	0.03	0.04	0.05	0.06	0.07	0.06	0.09
	2:20:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.02
	2:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

MHFD-Inlet, Version 5.01 (April 2021)

INLET MANAGEMENT

Worksheet Protected

INLET NAME	DP1	DP2	DP6	DP5	DP3	DP8
Site Type (Urban or Rural)						
Inlet Application (Street or Area)	AREA	AREA	STREET	STREET	AREA	AREA
Hydraulic Condition	Swale	Swale	On Grade	On Grade	Swale	Swale
Inlet Type	CDOT Type C (Depressed)	CDOT Type C (Depressed)	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type C (Depressed)	CDOT Type C (Depressed)

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{Known} (cfs)	0.4	1.0	1.4	2.3	1.7	0.1
Major Q_{Known} (cfs)	1.0	2.4	2.4	4.1	2.6	0.4

Bypass (Carry-Over) Flow from Upstream

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0	0.0

Watershed Characteristics

Subcatchment Area (acres)						
Percent Impervious						
NRCS Soil Type						

Watershed Profile

Overland Slope (ft/ft)						
Overland Length (ft)						
Channel Slope (ft/ft)						
Channel Length (ft)						

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)						
One-Hour Precipitation, P_1 (inches)						

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)						
One-Hour Precipitation, P_1 (inches)						

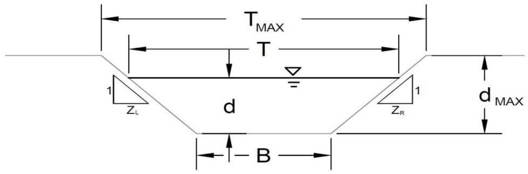
CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**0.4**	**1.0**	**1.4**	**2.3**	**1.7**	**0.1**
Major Total Design Peak Flow, Q (cfs)	**1.0**	**2.4**	**2.4**	**4.1**	**2.6**	**0.4**
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0	0.0
Major Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0	0.0

AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP1



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

	NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D, or E =		
	Manning's n (Leave cell D16 blank to manually enter an n value)	n =	0.012	
	Channel Invert Slope	S_O =	0.0300	ft/ft
	Bottom Width	B =	4.00	ft
Warning 01	Left Side Slope	Z1 =	0.02	ft/ft
Warning 01	Right Side Sloe	Z2 =	0.02	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
- ☐ **Non-Cohesive**
- ☐ **Cohesive**
- ☐ **Paved**

		Minor Storm	Major Storm	
Maximum Allowable Top Width of Channel for Minor & Major Storm	**T_{MAX} =**	**30.00**	**30.00**	**ft**
Maximum Allowable Water Depth in Channel for Minor & Major Storm	**d_{MAX} =**	**0.50**	**0.50**	**ft**

Allowable Channel Capacity Based On Channel Geometry

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**23.4**	**23.4**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	**d_{allow} =**	**0.50**	**0.50**	**ft**

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	**Q_o =**	**0.4**	**1.0**	**cfs**
Water Depth	**d =**	**0.04**	**0.07**	**ft**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

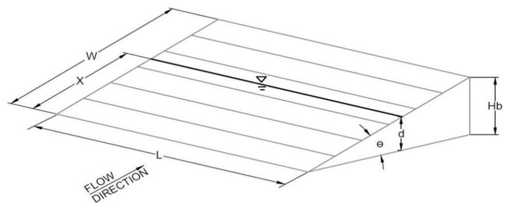
AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP1

Inlet Design Information (Input)

Type of Inlet	CDOT Type C (Depressed)	Inlet Type =	CDOT Type C (Depressed)
Angle of Inclined Grate (must be <= 30 degrees)	θ =	1.19	degrees
Width of Grate	W =	3.00	ft
Length of Grate	L =	3.00	ft
Open Area Ratio	A_{RATIO} =	0.70	
Height of Inclined Grate	H_B =	0.06	ft
Clogging Factor	C_f =	0.50	
Grate Discharge Coefficient	C_d =	0.79	
Orifice Coefficient	C_o =	0.52	
Weir Coefficient	C_w =	1.68	



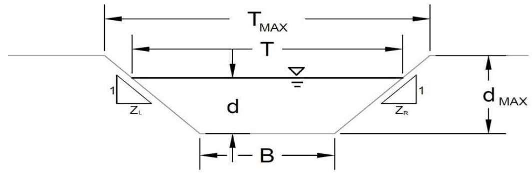
		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	1.04	1.07	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**15.6**	**16.3**	**cfs**
Bypassed Flow	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Qa/Qo	**C% =**	**100**	**100**	**%**

Warning 01: Sideslope steepness exceeds USDCM Volume I recommendation.
Warning 02: Depth (d) exceeds USDCM Volume I recommendation.

AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP2



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

	NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D, or E =		
	Manning's n (Leave cell D16 blank to manually enter an n value)	n =	0.012	
	Channel Invert Slope	S_O =	0.0300	ft/ft
	Bottom Width	B =	4.00	ft
Warning 01	Left Side Slope	Z1 =	0.02	ft/ft
Warning 01	Right Side Sloe	Z2 =	0.02	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
- ☐ **Non-Cohesive**
- ☐ **Cohesive**
- ☐ **Paved**

		Minor Storm	Major Storm	
Maximum Allowable Top Width of Channel for Minor & Major Storm	T_{MAX} =	**30.00**	**30.00**	**ft**
Maximum Allowable Water Depth in Channel for Minor & Major Storm	d_{MAX} =	**0.50**	**0.50**	**ft**

Allowable Channel Capacity Based On Channel Geometry

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**23.4**	**23.4**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	d_{allow} =	**0.50**	**0.50**	**ft**

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	Q_o =	**1.0**	**2.4**	**cfs**
Water Depth	d =	**0.07**	**0.12**	**ft**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

MHFD-Inlet, Version 5.01 (April 2021)

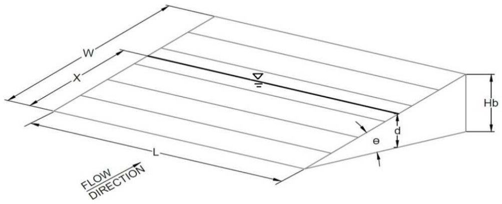
AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP2

Inlet Design Information (Input)

Type of Inlet	CDOT Type C (Depressed)	Inlet Type =	CDOT Type C (Depressed)	
Angle of Inclined Grate (must be <= 30 degrees)		θ =	1.19	degrees
Width of Grate		W =	3.00	ft
Length of Grate		L =	3.00	ft
Open Area Ratio		A_{RATIO} =	0.70	
Height of Inclined Grate		H_B =	0.06	ft
Clogging Factor		C_f =	0.50	
Grate Discharge Coefficient		C_d =	0.79	
Orifice Coefficient		C_o =	0.52	
Weir Coefficient		C_w =	1.68	



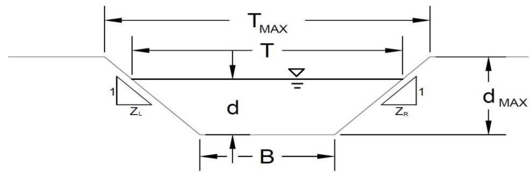
		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	1.07	1.12	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**16.4**	**17.4**	**cfs**
Bypassed Flow	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Qa/Qo	**C% =**	**100**	**100**	**%**

Warning 01: Sideslope steepness exceeds USDCM Volume I recommendation.
Warning 02: Depth (d) exceeds USDCM Volume I recommendation.

AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP3



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

	NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D, or E =		
	Manning's n (Leave cell D16 blank to manually enter an n value)	n =	0.012	
	Channel Invert Slope	S_O =	0.0300	ft/ft
	Bottom Width	B =	4.00	ft
Warning 01	Left Side Slope	Z1 =	0.02	ft/ft
Warning 01	Right Side Sloe	Z2 =	0.02	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
- ☐ **Non-Cohesive**
- ☐ **Cohesive**
- ☐ **Paved**

		Minor Storm	Major Storm	
Maximum Allowable Top Width of Channel for Minor & Major Storm	T_{MAX} =	**30.00**	**30.00**	**ft**
Maximum Allowable Water Depth in Channel for Minor & Major Storm	d_{MAX} =	**0.50**	**0.50**	**ft**

Allowable Channel Capacity Based On Channel Geometry

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**23.4**	**23.4**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	d_{allow} =	**0.50**	**0.50**	**ft**

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	Q_o =	**1.7**	**2.6**	**cfs**
Water Depth	d =	**0.10**	**0.13**	**ft**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

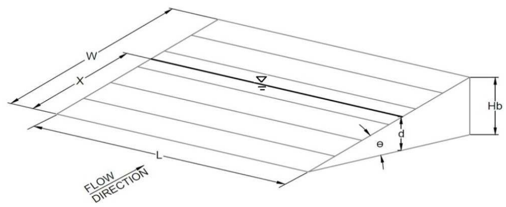
AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP3

Inlet Design Information (Input)

Type of Inlet: CDOT Type C (Depressed) — Inlet Type = CDOT Type C (Depressed)



Angle of Inclined Grate (must be <= 30 degrees)	θ =	1.20	degrees
Width of Grate	W =	3.00	ft
Length of Grate	L =	3.00	ft
Open Area Ratio	A_{RATIO} =	0.70	
Height of Inclined Grate	H_B =	0.06	ft
Clogging Factor	C_f =	0.50	
Grate Discharge Coefficient	C_d =	0.78	
Orifice Coefficient	C_o =	0.52	
Weir Coefficient	C_w =	1.68	

		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	1.10	1.13	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**16.9**	**17.6**	**cfs**
Bypassed Flow	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Qa/Qo	**C% =**	**100**	**100**	**%**

Warning 01: Sideslope steepness exceeds USDCM Volume I recommendation.

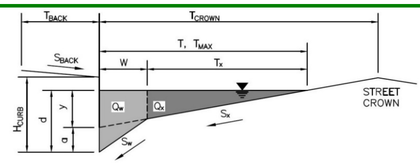
Warning 02: Depth (d) exceeds USDCM Volume I recommendation.

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: NORTHCREST CENTER PEMBs - EL PASO COUNTY

Inlet ID: DP5



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	0.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =		ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =		
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	60.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.040	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.035	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

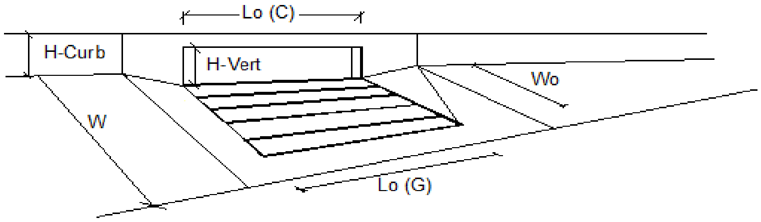
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	5.0	6.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**3.1**	**4.6**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



Design Information (Input)	CDOT Type R Curb Opening		MINOR	MAJOR	
Type of Inlet		Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')		a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)		L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)		W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**2.3**	**4.1**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**100**	**%**

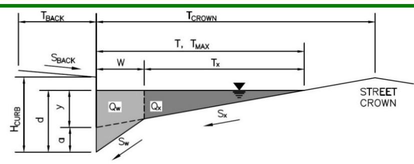
MHFD-Inlet, Version 5.01 (April 2021)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: NORTHCREST CENTER PEMBs - EL PASO COUNTY

Inlet ID: DP6



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	0.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =		ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =		
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	12.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.040	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.042	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

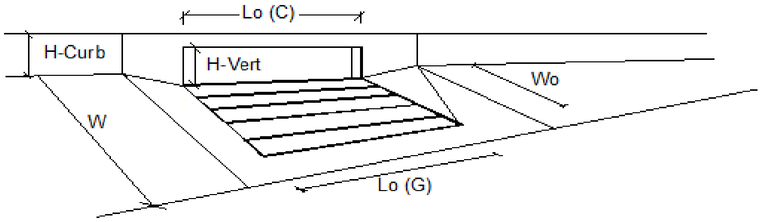
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	4.0	5.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**2.2**	**3.4**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



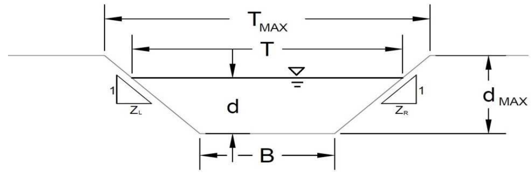
Design Information (Input)	CDOT Type R Curb Opening		MINOR	MAJOR	
Type of Inlet		Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')		a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)		L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)		W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**1.3**	**2.4**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**100**	**%**

AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP8



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

	NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D, or E =		
	Manning's n (Leave cell D16 blank to manually enter an n value)	n =	0.012	
	Channel Invert Slope	S_O =	0.0300	ft/ft
	Bottom Width	B =	4.00	ft
Warning 01	Left Side Slope	Z1 =	0.02	ft/ft
Warning 01	Right Side Sloe	Z2 =	0.02	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
- (•) **Non-Cohesive**
- ( ) **Cohesive**
- ( ) **Paved**

		Minor Storm	Major Storm	
Maximum Allowable Top Width of Channel for Minor & Major Storm	T_{MAX} =	**30.00**	**30.00**	**ft**
Maximum Allowable Water Depth in Channel for Minor & Major Storm	d_{MAX} =	**0.50**	**0.50**	**ft**

Allowable Channel Capacity Based On Channel Geometry

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**23.4**	**23.4**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	d_{allow} =	**0.50**	**0.50**	**ft**

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	Q_o =	**0.1**	**0.4**	**cfs**
Water Depth	d =	**0.02**	**0.04**	**ft**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

MHFD-Inlet, Version 5.01 (April 2021)

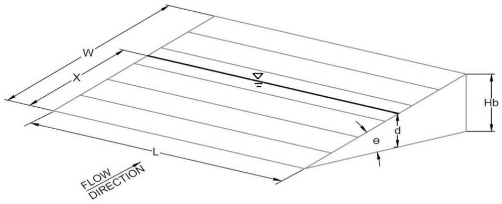
AREA INLET IN A SWALE

NORTHCREST CENTER PEMBs - EL PASO COUNTY

DP8

Inlet Design Information (Input)

Type of Inlet	CDOT Type C (Depressed)	Inlet Type =	CDOT Type C (Depressed)	
Angle of Inclined Grate (must be <= 30 degrees)		θ =	0.50	degrees
Width of Grate		W =	3.00	ft
Length of Grate		L =	3.00	ft
Open Area Ratio		A_{RATIO} =	0.70	
Height of Inclined Grate		H_B =	0.03	ft
Clogging Factor		C_f =	0.50	
Grate Discharge Coefficient		C_d =	0.82	
Orifice Coefficient		C_o =	0.55	
Weir Coefficient		C_w =	1.75	



		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	1.02	1.04	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**16.1**	**16.6**	**cfs**
Bypassed Flow	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Qa/Qo	**C% =**	**100**	**100**	**%**

Warning 01: Sideslope steepness exceeds USDCM Volume I recommendation.
Warning 02: Depth (d) exceeds USDCM Volume I recommendation.

Warning 04: Froude No. exceeds USDCM Volume I recommendation.

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.07, March 2018)

Designer:	
Company:	**Rocky Mountain Group**
Date:	**February 8, 2023**
Project:	**Northcrest**
Location:	

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a — I_a = 86.5 %

B) Tributary Area's Imperviousness Ratio (i = I_a / 100) — i = 0.865

C) Contributing Watershed Area — Area = 3.020 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm — d_6 = ___ in

E) Design Concept (Select EURV when also designing for flood control)

Choose One
- ○ Water Quality Capture Volume (WQCV)
- ◉ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time (V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area) — V_{DESIGN}= ___ ac-ft

G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume ($V_{WQCV\ OTHER}$ = (d_6*(V_{DESIGN}/0.43)) — $V_{DESIGN\ OTHER}$= ___ ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired) — $V_{DESIGN\ USER}$= 0.094 ac-ft

I) NRCS Hydrologic Soil Groups of Tributary Watershed
- i) Percentage of Watershed consisting of Type A Soils — HSG_A = 100 %
- ii) Percentage of Watershed consisting of Type B Soils — HSG_B = 0 %
- iii) Percentage of Watershed consisting of Type C/D Soils — $HSG_{C/D}$ = 0 %

J) Excess Urban Runoff Volume (EURV) Design Volume
- For HSG A: $EURV_A$ = 1.68 * $i^{1.28}$
- For HSG B: $EURV_B$ = 1.36 * $i^{1.08}$
- For HSG C/D: $EURV_{C/D}$ = 1.20 * $i^{1.08}$

$EURV_{DESIGN}$ = ___ ac-f t

K) User Input of Excess Urban Runoff Volume (EURV) Design Volume (Only if a different EURV Design Volume is desired) — $EURV_{DESIGN\ USER}$= 0.257 ac-f t

2. Basin Shape: Length to Width Ratio (A basin length to width ratio of at least 2:1 will improve TSS reduction.) — L : W = 3.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred) — Z = 1.00 ft / ft — TOO STEEP (< 3)

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations: ___

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 2% of the WQCV) — V_{FMIN} = 0.002 ac-ft

B) Actual Forebay Volume — V_F = 0.003 ac-ft

C) Forebay Depth (D_F = 18 inch maximum) — D_F = 8.0 in

D) Forebay Discharge
- i) Undetained 100-year Peak Discharge — Q_{100} = 10.33 cfs
- ii) Forebay Discharge Design Flow (Q_F = 0.02 * Q_{100}) — Q_F = 0.21 cfs

E) Forebay Discharge Design

Choose One
- ○ Berm With Pipe
- ◉ Wall with Rect. Notch
- ○ Wall with V-Notch Weir

Flow too small for berm w/ pipe

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = ___ in

G) Rectangular Notch Width — Calculated W_N = 3.0 in

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.07, March 2018)

Designer:	
Company:	Rocky Mountain Group
Date:	February 8, 2023
Project:	Northcrest
Location:	

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a	I_a =	86.5	%
B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)	i =	0.865	
C) Contributing Watershed Area	Area =	3.020	ac
D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 =		in
E) Design Concept (Select EURV when also designing for flood control)	Choose One: ○ Water Quality Capture Volume (WQCV) ◉ Excess Urban Runoff Volume (EURV)		
F) Design Volume (WQCV) Based on 40-hour Drain Time ($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area)$)	V_{DESIGN}=		ac-ft
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume ($V_{WQCV\ OTHER} = (d_6*(V_{DESIGN}/0.43))$)	$V_{DESIGN\ OTHER}$=		ac-ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{DESIGN\ USER}$=	0.094	ac-ft
I) NRCS Hydrologic Soil Groups of Tributary Watershed			
i) Percentage of Watershed consisting of Type A Soils	HSG_A =	100	%
ii) Percentage of Watershed consisting of Type B Soils	HSG_B =	0	%
iii) Percentage of Watershed consisting of Type C/D Soils	$HSG_{C/D}$ =	0	%
J) Excess Urban Runoff Volume (EURV) Design Volume For HSG A: $EURV_A = 1.68 * i^{1.28}$ For HSG B: $EURV_B = 1.36 * i^{1.08}$ For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$	$EURV_{DESIGN}$ =		ac-f t
K) User Input of Excess Urban Runoff Volume (EURV) Design Volume (Only if a different EURV Design Volume is desired)	$EURV_{DESIGN\ USER}$=	0.257	ac-f t

2. Basin Shape: Length to Width Ratio (A basin length to width ratio of at least 2:1 will improve TSS reduction.) — L : W = 3.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred) — Z = 1.00 ft / ft — **TOO STEEP (< 3)**

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 2% of the WQCV)	V_{FMIN} =	0.002	ac-ft
B) Actual Forebay Volume	V_F =	0.003	ac-ft
C) Forebay Depth (D_F = 18 inch maximum)	D_F =	8.0	in
D) Forebay Discharge			
i) Undetained 100-year Peak Discharge	Q_{100} =	4.07	cfs
ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$)	Q_F =	0.08	cfs
E) Forebay Discharge Design	Choose One: ○ Berm With Pipe ◉ Wall with Rect. Notch ○ Wall with V-Notch Weir	**Flow too small for berm w/ pipe**	
F) Discharge Pipe Size (minimum 8-inches)	Calculated D_P =		in
G) Rectangular Notch Width	Calculated W_N =	2.1	in

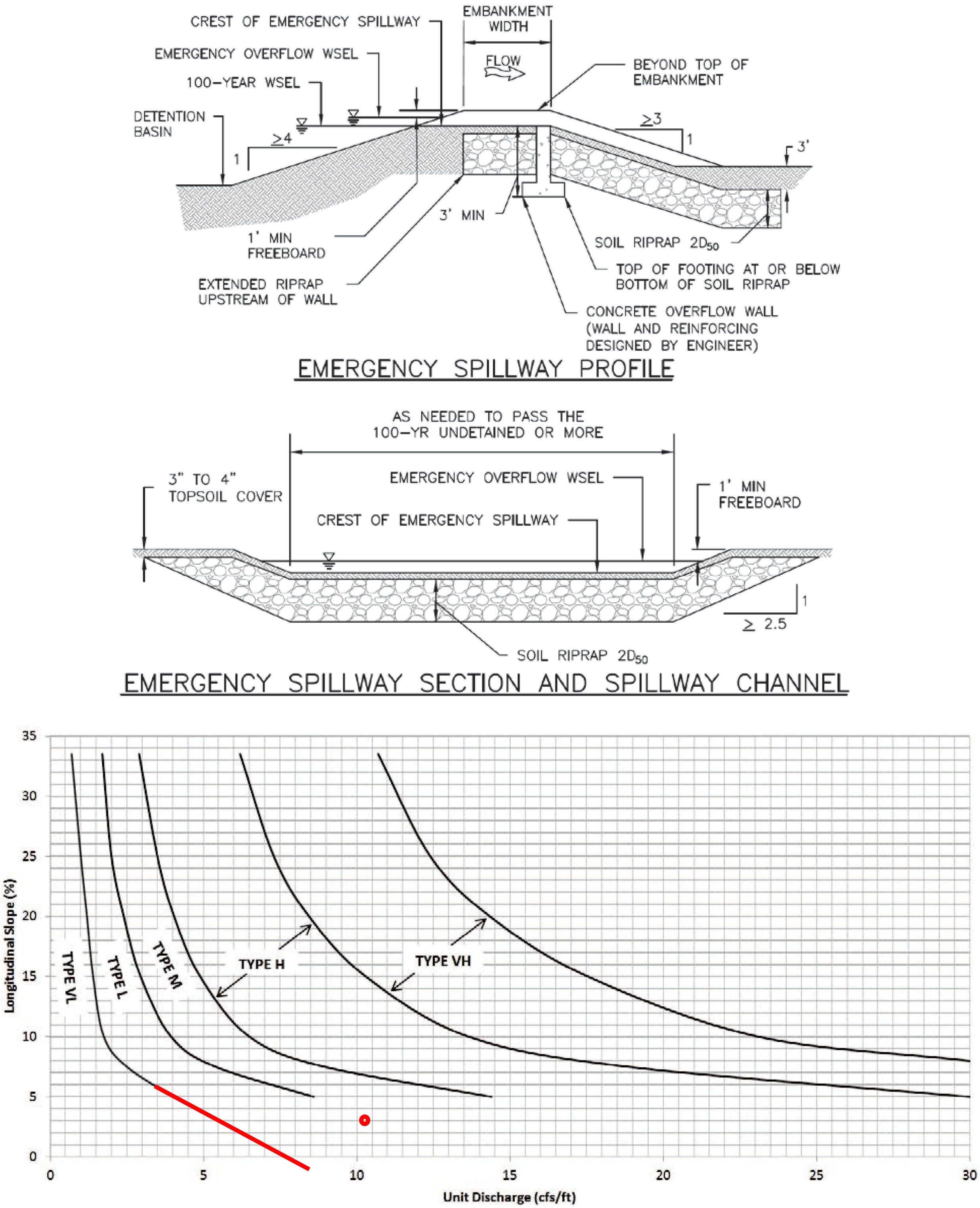
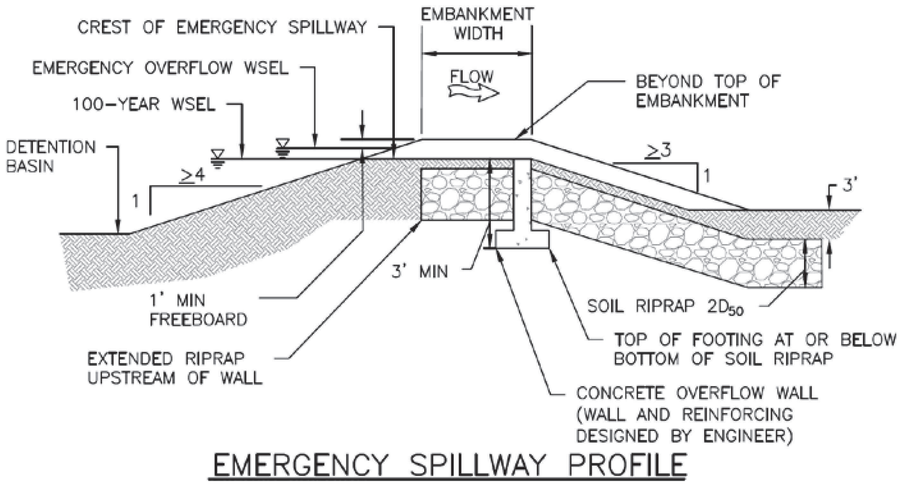
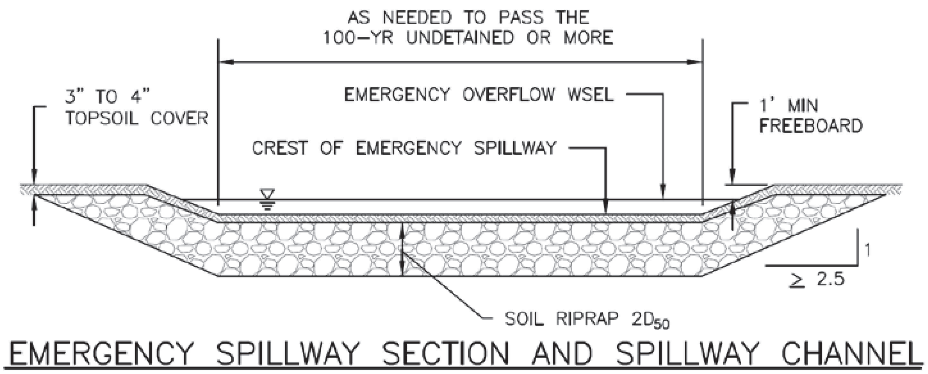


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

Forebay #1 (West) ~~ Slope = 3% and 100-Year Peak Discharge = 10.33 cfs





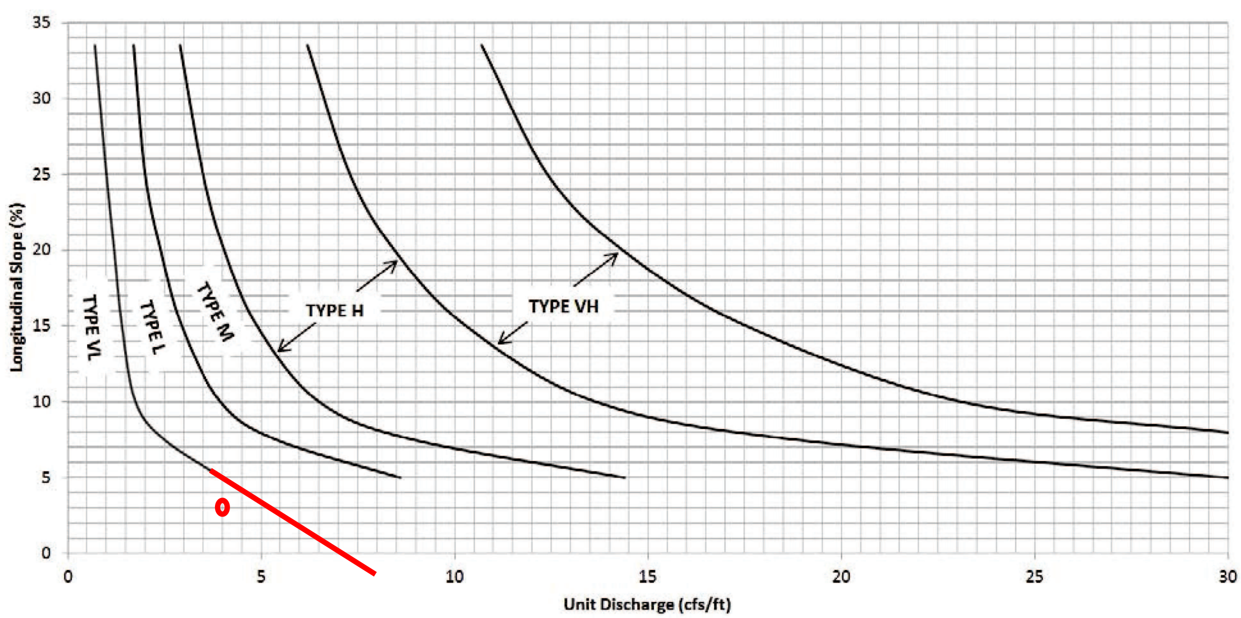


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

Forebay #2 (East)~~ Slope = 3% and 100-Year Peak Discharge = 4.07 cfs

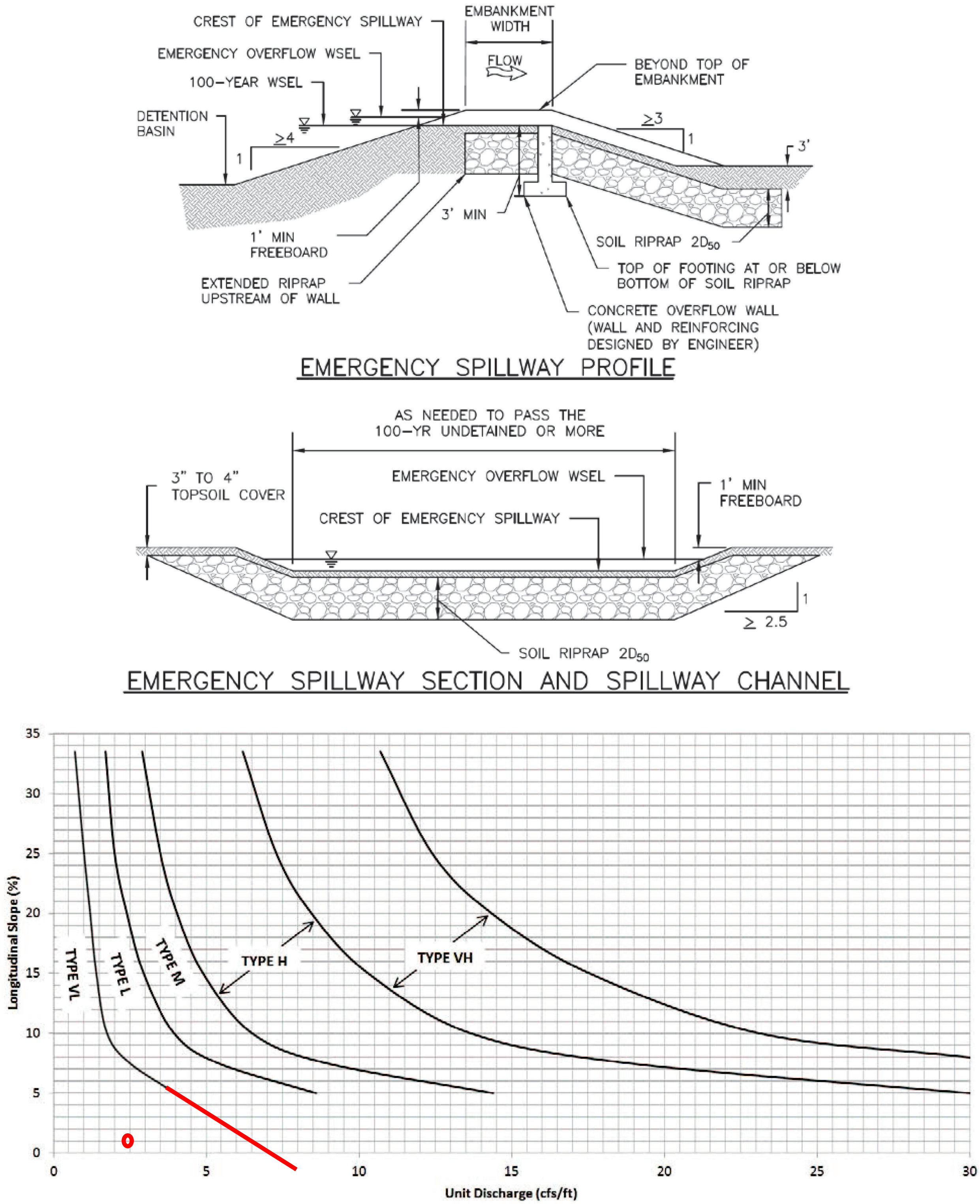


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

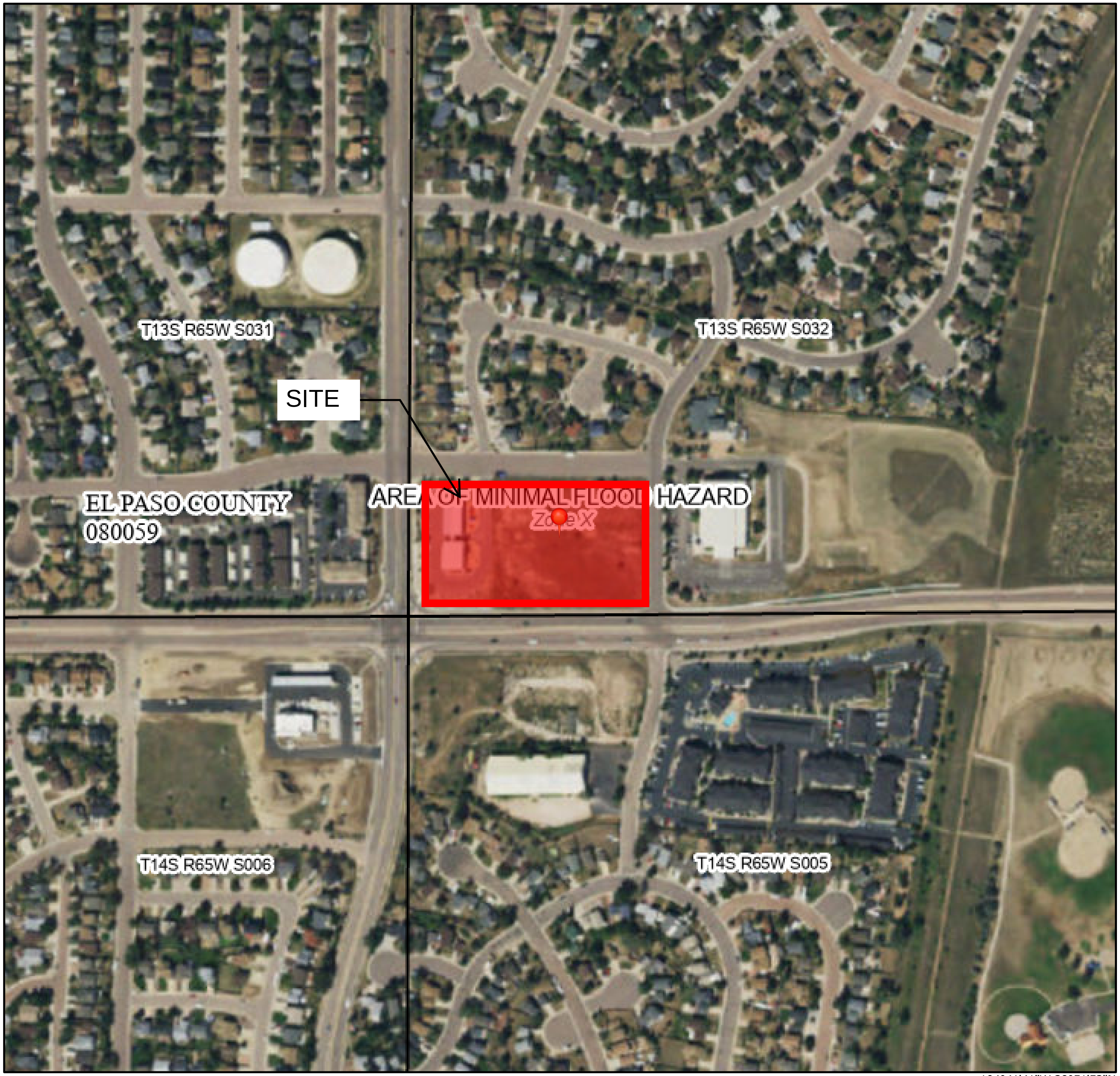
Emergency Spillway ~~ Slope = 1% and 100-Year Peak Discharge = 2.3 cfs

APPENDIX C – FEMA FLOODPLAIN MAP

National Flood Hazard Layer FIRMette

FEMA

104°42'18"W 38°52'21"N



104°41'41"W 38°51'53"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- B 20.2 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
- 8 Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

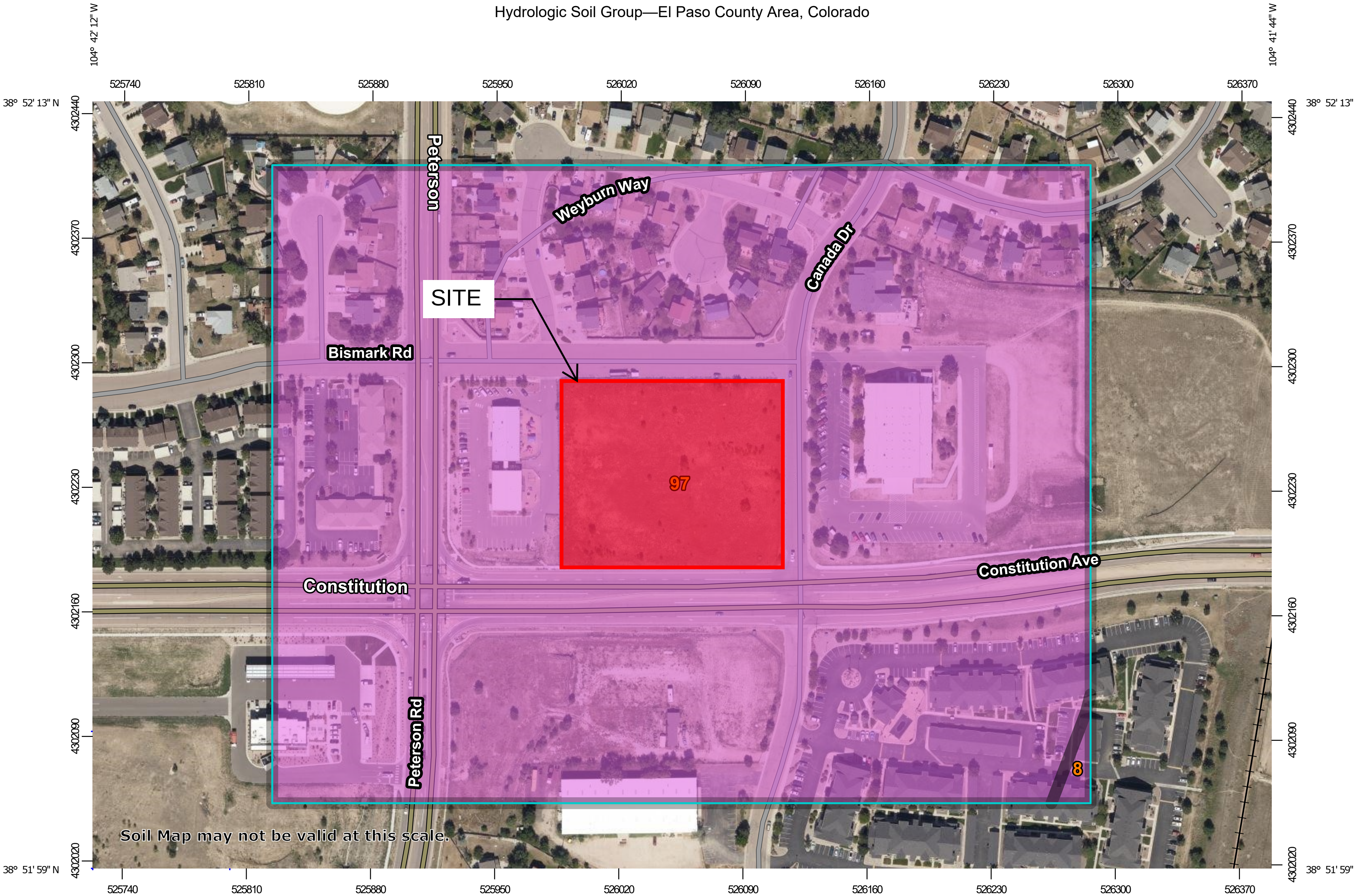
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/17/2021 at 3:21 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Appendix D – USGS Soils Survey Map



Map Scale: 1:3,040 if printed on A landscape (11" x 8.5") sheet.

Meters: 0, 45, 90, 180, 270

Feet: 0, 100, 200, 400, 600

N

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	0.2	0.4%
97	Truckton sandy loam, 3 to 9 percent slopes	A	40.9	99.6%
Totals for Area of Interest			**41.1**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

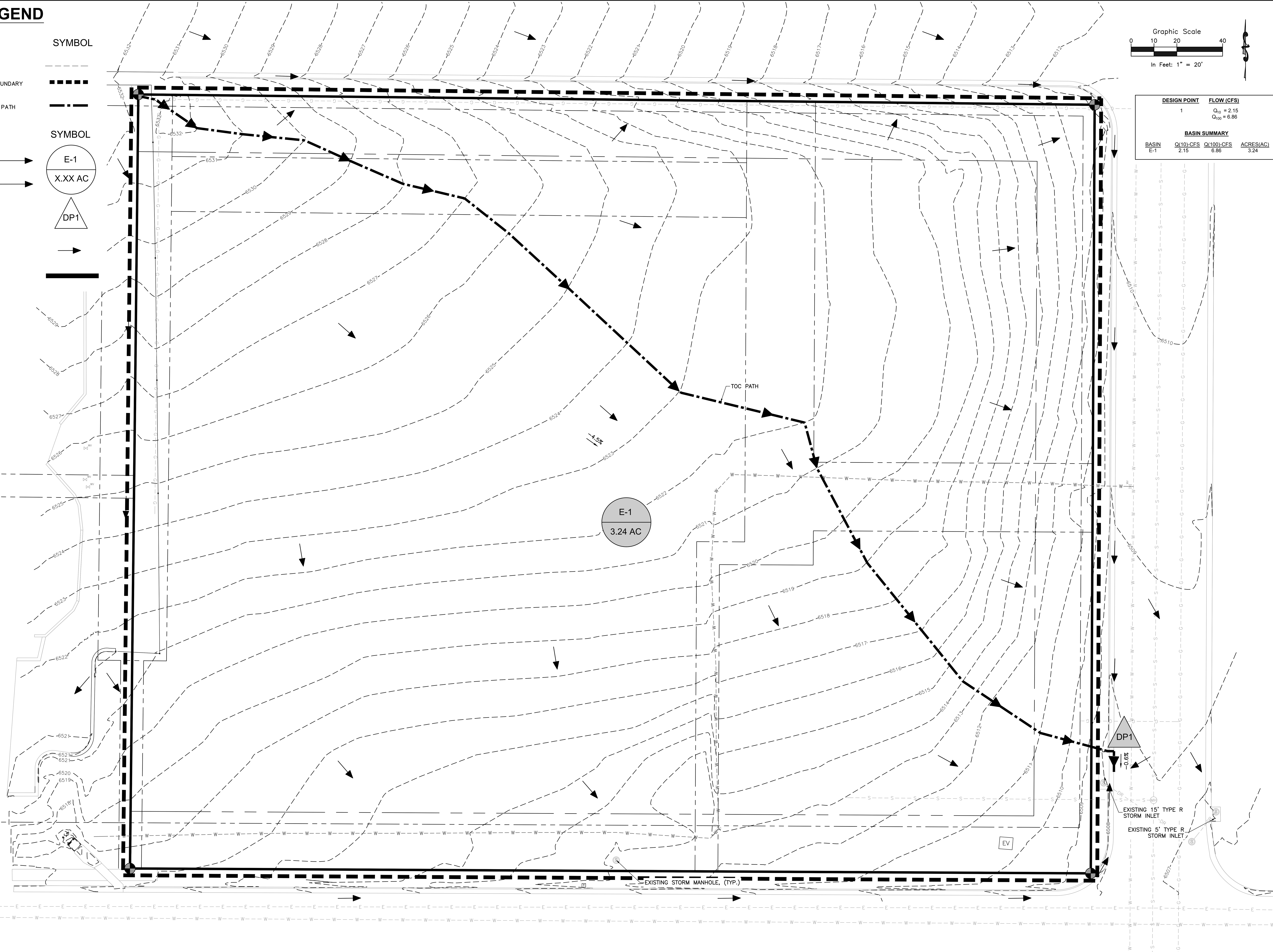
Tie-break Rule: Higher

Appendix E – Drainage Plans

LEGEND

DESCRIPTION	SYMBOL
EX. CONTOUR	
DRAINAGE SUB-BASIN BOUNDARY	
TIME OF CONCENTRATION PATH	

DESCRIPTION	SYMBOL
BASIN IDENTIFIER	E-1
AREA IN ACRES	X.XX AC
DESIGN POINT ID	DP1
FLOW ARROW	
PROPERTY LINE	



DESIGN POINT	FLOW (CFS)
1	Q_{10} = 2.15 Q_{100} = 6.86

BASIN SUMMARY

BASIN	Q(10)-CFS	Q(100)-CFS	ACRES(AC)
E-1	2.15	6.86	3.24

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Southern Colorado, Denver Metro, Northern Colorado

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FOR CIVIL ONLY

NORTHCREST PEMB DEVELOPMENT
2510 & 2522 CANADA DRIVE
COLORADO SPRINGS, COLORADO
LEISURE CONSTRUCTION

SHEET NAME
EXISTING CONDITIONS
SUB-BASIN HYDROLOGY

PROJECT STATUS
DESIGN DEVELOPMENT

ENG: DGW
DRAWN: TPT
CHECKED: DGW
DATE
11/11/2022

#	REVISION	DATE
	—	—
	—	—
	—	—
	—	—
	—	—
	—	—
	—	—

JOB NO.
180649

SHEET NO.
DR-01

LEGEND

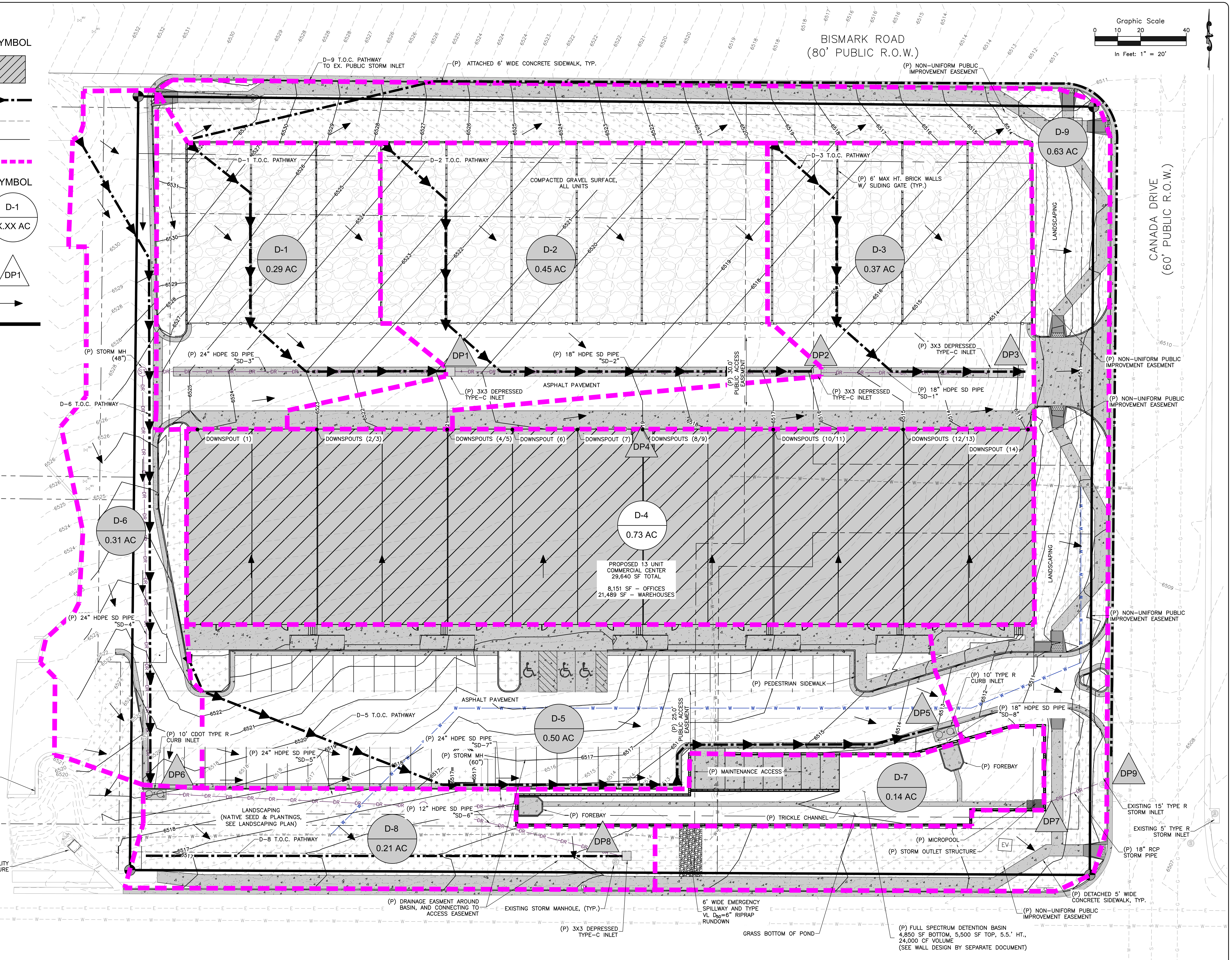
DESCRIPTION	SYMBOL
PROP. STRUCTURE/BUILDING	
TIME OF CONCENTRATION PATH	
EX. CONTOUR	
PROP. CONTOUR	
DRAINAGE SUB-BASIN BOUNDARY	

DESCRIPTION	SYMBOL
BASIN IDENTIFIER	D-1
AREA IN ACRES	X.XX AC
DESIGN POINT ID	DP1
FLOW ARROW	
PROPERTY LINE	

DESIGN POINT	FLOW (CFS)
DP-1	Q_{10} = 1.14 Q_{100} = 1.80
DP-2	Q_{10} = 1.86 Q_{100} = 2.90
DP-3	Q_{10} = 1.70 Q_{100} = 2.63
DP-4	Q_{10} = 3.29 Q_{100} = 5.12
DP-5	Q_{10} = 2.69 Q_{100} = 4.07
DP-6	Q_{10} = 1.58 Q_{100} = 2.42
DP-7	Q_{10} = 0.33 Q_{100} = 0.66
DP-8	Q_{10} = 0.17 Q_{100} = 0.58
DP-9	Q_{10} = 1.20 Q_{100} = 2.54

BASIN SUMMARY

BASIN	Q(10)-CFS	Q(100)-CFS	ACRES(AC)
D-1	1.14	1.80	0.29
D-2	1.86	2.90	0.45
D-3	1.70	2.63	0.37
D-4	3.29	5.12	0.73
D-5	2.69	4.07	0.50
D-6	1.58	2.42	0.31
D-7	0.33	0.66	0.14
D-8	0.17	0.58	0.21
D-9	1.20	2.54	0.63



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ROCKY MOUNTAIN GROUP

RMG ARCHITECTS ENGINEERS

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Geotechnical, Materials Testing, Civil, Planning

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NORTHCREST PEMB DEVELOPMENT

2510 & 2522 CANADA DRIVE
COLORADO SPRINGS, COLORADO

LEISURE CONSTRUCTION

SHEET NAME
DEVELOPED CONDITIONS SUB-BASINS HYDROLOGY

PROJECT STATUS
DESIGN DEVELOPMENT

ENG: DGW
DRAWN: TPT
CHECKED: DGW
DATE: 11/11/2022

#	REVISION	DATE
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JOB NO. 180649

SHEET NO. DR-02