# The Glen at Widefield Filing No. 9 <br> Transportation Memorandum PCD File No. SF185 

(LSC \#174850)
July 30, 2018

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.


二小i. $30^{\text {th }} 2018$
Date

CONSULTANTS, INC.
July 30, 2018

Mr. J. Ryan Watson
Widefield Investment Group
3 Widefield Boulevard
Colorado Springs, CO 80911

$$
\begin{array}{ll}
\text { RE: } & \text { The Glen at Widefield Filing No. } 9 \\
& \text { Transportation Memorandum } \\
\text { PCD File No. SF185 } \\
& \text { EI Paso County, Colorado } \\
\text { LSC \#174850 }
\end{array}
$$

Dear Mr. Watson:

In response to your request, LSC Transportation Consultants, Inc. has prepared this updated transportation memorandum for The Glen at Widefield Filing No. 9. As shown in Figure 1, the site is located northwest of the Marksheffel Road/Mesa Ridge Parkway intersection in El Paso County, Colorado. Filing 9 is planned to contain 106 lots for single-family homes. This memorandum is a supplement to the overall Glen at Widefield East Preliminary Plan traffic report dated January 18, 2016. Please contact our office to obtain a copy of this report, if needed.

A copy of the plat for the 106 single-family lots is attached for reference. The lot and street layout for this filing matches the Preliminary Plan.

## REPORT CONTENTS

This report is being prepared as part of a submittal to El Paso County. It identifies the traffic impacts of this development. The report contains the following:

- Updated traffic count data.
- Projections of short-term (2022) baseline/background traffic volumes at the key area intersections.
- The projected average weekday and peak-hour vehicle-trips to be generated by Filing No. 9.
- The assignment of the Filing No. 9 projected trips to the key area intersections for the short term.
- The short-term level of service at these intersections.
- The short-term level of service and queuing analysis at the intersection of Powers Boulevard/ Mesa Ridge Parkway.
- Findings and recommendations.
- Signal escrow analysis tables.


## LAND USE AND ACCESS

Since completion of the 2016 Glen at Widefield East Preliminary Plan Traffic Report, 249 of the 577 proposed lots for single-family homes within the preliminary plan area have been platted as The Glen at Widefield Filings 7 and 8. At the time traffic counts were conducted in March 2018 about 59 homes have been constructed in The Glen at Widefield Filing 7. Mesa Ridge Parkway has been extended east to Marksheffel Road. Primary access for these filings is via the new intersection of Spring Glen Drive and the recently completed section of Mesa Ridge Parkway. A secondary access to Marksheffel Road is currently under construction that will align with Peaceful Valley Road.

The currently proposed Glen at Widefield Filing No. 9 is planned to contain 106 lots for singlefamily homes. Figure 2 shows the location of The Glen at Widefield Filing Nos. 7, 8, and 9. No additional access is proposed with this filing.

## ROADWAY AND TRAFFIC CONDITIONS

## Area Roadways

Figure 1 shows the roadways in the vicinity of the site. The major roadways are identified below, followed by a brief description of each.

Powers Boulevard is a four-lane Expressway extending north from Mesa Ridge Parkway. In the future, Powers Boulevard is planned to be extended south to connect to Interstate 25, potentially at Exit 122. In the vicinity of the site, Powers Boulevard has two through lanes in each direction and a posted speed limit of 55 miles per hour (mph). The Colorado Department of Transportation has been collecting escrow funds from the previous Glen at Widefield filings as participation toward the recently installed traffic signal at the intersection of Mesa Ridge/Powers.

Marksheffel Road extends north from the Link Road/C\&S Road intersection in Fountain, Colorado to north of Woodmen Road. Marksheffel has recently been upgraded to an interim three-lane facility between Mesa Ridge Parkway and Bradley Road as part of a PPRTA project. Marksheffel Road is shown as a future four-lane Expressway on the El Paso County Major Transportation Corridors Plan (MTCP). The posted speed limit on Marksheffel Road is 55 mph north of Mesa Ridge Parkway and 45 mph south of Mesa Ridge Parkway.

Mesa Ridge Parkway is a four-lane median-divided Principal Arterial extending east from I-25 to Powers Boulevard. A half-section of Mesa Ridge Parkway with one through lane in each direction has been constructed east from Powers Boulevard to Marksheffel Road. It is our understanding that the construction of the other half-section is not the applicant's responsibility. LSC estimates that Mesa Ridge Parkway will likely need to be widened to provide two lanes in each direction once the average weekday traffic volumes reach 14,000 to 18,000 vehicles per day. Mesa Ridge

Parkway improvement are listed as an "A-List" PPRTA project. The posted speed limit in the vicinity of the site is 45 mph .

Peaceful Valley Road is a two-lane City of Fountain street that extends east from Marksheffel Road about two-and-a-half miles to the location of a future extension of Meridian Road. The posted speed limit on Peaceful Valley Road is 30 mph . Most of Peaceful Valley Road is located within the City of Fountain.

## Notable Recent Area Roadway System Improvements

The Marksheffel South project has been completed, a traffic signal has been installed at the intersection of Mesa Ridge Parkway and Powers, and it is our understanding that this signal has only been fully operational since early January. The temporary Roanfield Drive street connection to Powers Boulevard has been closed. Also, the southbound left-turn lane at the Mesa Ridge/ Powers intersection has been lengthened as required with The Glen at Widefield Filing No. 7. The Marksheffel painted center median at the intersection of Peaceful Valley Road/ Marksheffel Road has been striped as a channelized T-configuration (with southbound left-turn deceleration and left-turn acceleration lanes). The configuration may need to change through restriping of the center painted median with the addition of the fourth/west leg of this intersection with The Glen at Widefield Filing No. 8.

## EXISTING TRAFFIC VOLUMES

Figure 3 shows the existing peak-hour traffic volumes and existing lane geometries and traffic controls. The traffic volumes are based on traffic counts conducted by LSC in March 2018. These counts were taken after the signal was installed and placed into operation at the intersection of Powers/Mesa Ridge Parkway. Note: The through traffic volumes at the intersection of Peaceful Valley Road/Marksheffel were adjusted based on the 2018 count at Marksheffel/Mesa Ridge Parkway. The left and right turning movements at this intersection were not re-counted as it is unlikely that the installation of the signal at Mesa Ridge/Powers would affect those turning movements. No significant new development/new home construction has recently occurred in the area served by Peaceful Valley Road in the past 12 months. The traffic count reports are attached.

## LEVEL OF SERVICE

Level of service (LOS) is a quantitative measure of the level of delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. Table 1 shows the level of service delay ranges.

| Thtersection Levels of Service Delay Ranges |  |  |  |
| :---: | :---: | :---: | :---: |
| Level of Service | Signalized Intersections <br> Average Control Delay <br> (seconds per vehicle) | V/C ${ }^{(1)}$ | Unsignalized <br> Intersections |
|  | 10.0 sec or less | less than 0.60 | Average Control Delay <br> (seconds per vehicle) |
|  | $10.1-20.0$ sec | $0.60-0.69$ | 10.0 sec or less |
|  | $20.1-35.0$ sec | $0.70-0.79$ | $15.1-25.0$ sec |
| D | $35.1-55.0$ sec | $0.80-0.89$ | $25.1-35.0$ sec |
| E | 55.1-80.0 sec | $0.90-0.99$ | $35.1-50.0$ sec |
| F | 80.1 sec or more | 1.00 and greater | 50.1 sec or more |
| (1) Source: Transportation Research Circular 212 <br> (2) For unsignalized intersections if V/C ratio is greater than 1.0 the level of service is LOS F <br> regardless of the projected average control delay per vehicle. |  |  |  |

The intersections of Powers/Mesa Ridge, Marksheffel/Mesa Ridge and Marksheffel/Peaceful Valley were analyzed to determine the existing levels of service. The intersection of Powers/Mesa Ridge was analyzed using Synchro. The intersections of Marksheffel/Mesa Ridge and Marksheffel/Peaceful Valley were analyzed using the unsignalized method of analysis procedures outlined in the Highway Capacity Manual, 6th Edition by the Transportation Research Board.

The intersection of Powers/Mesa Ridge was recently signalized and as such, the intersection has been analyzed as a signalized intersection. The current signal timing has been estimated by LSC. It is currently operating at an overall LOS B or better during the peak hours. The westbound leftturn movement at this intersection is operating at LOS D during the peak hours.

All movements at the stop-sign-controlled intersections of Marksheffel/Mesa Ridge and Marksheffel/Peaceful Valley are currently operating at LOS C or better during the peak hours.

## SHORT-TERM (YEAR 2022) BACKGROUND TRAFFIC

Figure 4a shows the short-term background traffic volumes at the key area intersections. Background traffic is the traffic estimated to be on the roadways without the Glen at Widefield Filing No. 9 traffic.

Background traffic includes the existing traffic volume (from Figure 3) plus increases in through traffic due to regional growth plus traffic estimated to be generated by buildout of existing and currently proposed subdivisions in the vicinity of the site. These estimates include traffic projected to be generated by the development of the 190 single-family homes within The Glen at Widefield Filing Nos. 7 and 8 that were unoccupied when traffic counts were conducted in March 2018 and traffic projected to be generated by buildout of all the existing and currently
proposed developments within the Lorson Ranch development located east of the intersection of Marksheffel/Fontaine.

Increases in the through traffic volumes on Powers Boulevard were estimated based on the growth rate calculated from the Colorado Department of Transportation 20-year growth factor for this section of Powers Boulevard.

Figure 4b shows the lane geometry, traffic control, and level of service at the key area intersections of based on the short-term background volumes.

## TRIP GENERATION

The Filing No. 9 site-generated vehicle-trips have been estimated using the nationally published trip generation rates from Trip Generation, 10th Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 2 shows the trip generation estimates for this filing.

Filing 9 is expected to generate 1,001 vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 20 vehicles would enter and 59 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between 4:15 and 6:15 p.m., about 66 vehicles would enter and 39 vehicles would exit the site.

## SITE-GENERATED TRAFFIC

Figure 5 shows the projected short-term site-generated traffic volumes for Filing No. 9. These volumes are based on the distribution and short-term roadway system assumptions contained in The Glen at Widefield East Preliminary Plan report (please refer to this report for additional detail) and the access plan for The Glen at Widefield Filing 9 only, as described above.

## SHORT-TERM TOTAL TRAFFIC

Figure 6a shows the projected short-term total traffic volumes at the key areas. The short-term total traffic volumes are the sum of the short-term background traffic volumes (from Figure 4) plus the Filing No. 9 short-term site-generated traffic volumes (from Figure 5).

Figure 6b shows the lane geometry, traffic control, and level of service at the key area intersections of based on the short-term total volumes.

## LEVEL OF SERVICE

The intersections of Marksheffel/Mesa Ridge and Marksheffel/Peaceful Valley were analyzed to determine the projected levels of service based on the short-term background and total traffic volumes using the unsignalized method of analysis procedures outlined in the Highway Capacity

Manual, 6th Edition by the Transportation Research Board. The signalized intersection of Powers/ Mesa Ridge was analyzed using Synchro. The results of the analysis are shown in Figures 4b and 6b.

All movement at the intersection of Powers/Mesa Ridge is projected to continue to operate at a LOS D or better during the peak hours based on the projected short-term background and total peak-hour traffic volumes.

All movements at the stop-sign-controlled intersections of Marksheffel/Mesa Ridge and Marksheffel/Peaceful Valley are projected to operate at LOS C or better during the peak hours.

Please refer to the Glen at Widefield East Preliminary Plan traffic report for the long-term analysis of the key area intersections.

## QUEUING ANALYSIS

A queuing analysis has been performed for the southbound and westbound left turn at Powers/ Mesa Ridge. The analysis has been completed based on dual left-turn lanes with existing length for the westbound Mesa Ridge left-turn lane, the recently extended southbound left-turn lane, and projected short-term total traffic.

The maximum southbound left-turn queue on Powers Boulevard approaching Mesa Ridge Parkway is projected to be about 237 feet long based on the projected short-term total traffic volumes. The southbound left-turn lane has recently been lengthened to 1,108 feet plus a 222foot taper.

The maximum westbound left-turn queue on Mesa Ridge Parkway approaching Powers Boulevard is projected to be about 301 feet long based on the projected short-term total traffic volumes assuming dual westbound left-turn lanes.

## CONCLUSIONS AND RECOMMENDATIONS

## Trip Generation

- Filing 9 is expected to generate 1,001 vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour about 20 vehicles would enter and 59 vehicles would exit the site. During the afternoon peak hour about 66 vehicles would enter and 39 vehicles would exit the site.


## Level of Service

- The signalized intersection of Mesa Ridge Parkway/Powers Boulevard is projected to continue to operate at a satisfactory level of service based on the projected short-term background and total peak-hour traffic volumes.
- The intersections of Marksheffel/Peaceful Valley Road and Mesa Ridge Parkway/Spring Glen Drive (the two access points to Filing No. 9) and the intersection of Marksheffel/Mesa Ridge would operate at satisfactory levels of service as stop-sign-controlled intersections based on the projected short-term background and total peak-hour traffic volumes.


## Intersection Lane Configurations

- A 475-foot left-turn lane approaching Spring Glen Drive has been installed with the construction of Mesa Ridge Parkway.
- Mesa Ridge Parkway has been constructed and striped with 10 -foot paved shoulders in the vicinity of Spring Glen Drive. Once the full four-lane Principal Arterial section is completed, it is anticipated that the acceleration lane will be implemented at that time. The width for a future westbound right-turn acceleration lane on Mesa Ridge Parkway will become available as the half-section to be built with the initial Mesa Ridge construction will be sufficiently wide. This has been shown on the Mesa Ridge Parkway design plans.
- The addition of Filing No. 9 site-generated traffic will not require the addition a westbound right-turn deceleration lane on Mesa Ridge Parkway at Spring Glen Drive.
- The painted center median on Marksheffel Road at the Peaceful Valley Road intersection is currently striped for a dedicated southbound left turn lane and a dedicated southbound leftturn acceleration lane. The west leg of this intersection is planned to be completed and opened in the short term to provide a second access to the Glen at Widefield East. This painted center median may need to be restriped as this intersection will no longer be a Tintersection, rather a four-leg intersection. LSC recommends removing the white channelized T pavement markings from the center of the intersection while maintaining the existing southbound eft-turn acceleration lane south of Peaceful Valley Road. This striping will need to be reevaluated with the next final plat within the Glen at Widefield.
- The southbound left-turn lane on Powers Boulevard approaching Mesa Ridge Parkway has recently been lengthenedas part of the Glen at Widefield Filing 7 access permit. The level of service analysis and queueing analysis for the short-term total traffic volumes indicates acceptable operations with the current single-lane configuration.
- Based on the projected short-term and total traffic volumes, Mesa Ridge Parkway should be widened approaching Powers Boulevardto provide dual westbound left-turn lanes. Based on the queueing analysis, dual 350 -foot left turn lanes (plus transition taper) would be adequate to accommodate the projected queues. Deceleration distance would not be necessarv, as

1. Instead of removing the white channelizedn"T" pavement markings in the intersection concider replacing with a dotted line left turn markings to chapnelize the EBLTeinto the existing accelfane. 2. It seems the TIS is assuming only small amount of the Glen East (Filing 7-9) will be using this for ingress/egress. The concern is that the amount of site generated traffic may be higher than anticipated and without a dedicated auxiliary turn lanes the general through traffic on Marksheffel Road (which has a high posted speed limit) will have to significantly reduce their speed at this intersection because of vehicles turning into the subdivision. Include the trip distribution figure and elaborate the narrative to detail the assumptions made and its impact to the intersection.

## Proposed Subdivision Street Classifications

- Figure 7 shows the recommended street classifications for the entire Preliminary Plan, including Filing 9.


## Mesa Ridge Parkway/Powers Boulevard Intersection

- CDOT has agreed to a signal escrow amount of $\$ 107,018$ for all of Glen at Widefield East. The number of total lots in the Preliminary Plan has been reduced to 578 and therefore the corresponding escrow amount would be $\$ 103,960$ for all of Glen at Widefield East. For purposes of the Filing 9 access permit, the amount would be $\$ 19,065$. Table 3 presents the signal escrow analysis including the previously identified amount for Filings 7 and 8 and the remaining amount for future filings.
- CDOT will require the submittal of an access permit application with this filing for purposes of processing the signal escrow and for work in the CDOT right-of-way to construct the westbound dual left-turn lanes and any associated traffic signal modifications. A new Access Permit and associated Notice-to-Proceed will be required.


## Mesa Ridge Parkway/Spring Glen Drive Signal Escrow

- The Glen East Preliminary Plan traffic report contains an estimated signal escrow amount for the entire Preliminary Plan and states that the developer's percentage contribution toward this signal will be calculated and a proportional contribution made toward the signal construction with each filing. The estimated proportional contribution for Filing 9 is $\$ 6,189$. Table 4 presents the signal escrow analysis for this intersection including the previously identified amount for Filings 7 and 8 and the remaining amount for future filings.


## Marksheffel Road/Peaceful Valley Road

- The Glen at Widefield East Preliminary Plan traffic report contains an estimated escrow amount for the Preliminary Plan and states that the developer's percentage contribution toward this signal will be calculated and a proportional contribution made toward the signal construction with each filing. The estimated proportional contribution for Filing 9 is $\$ 6,648$. Table 5 presents the signal escrow analysis for this intersection including the previously identified amount for Filings 7 and 8 and the remaining amount for future filings.


## ROADWAY IMPROVEMENT FEE PROGRAM

- This project will be required to participate in the El Paso County Road Improvement Fee Program. The Glen at Widefield Filing No. 9 will join the ten-mil PID. The ten-mil PID building permit fee portion associated with this option is $\$ 923$ per single-family dwelling unit. Based on 101 lots, the total building permit fee would be $\$ 97,838$.

Please contact me if you have any questions regarding this report.
Sincerely,
LSC TRANSPORTATION CONSULTANTS, INC.
By: Kirstin D. Ferrin
Senior Transportation Engineer
JCH:KDF:bjwb
Enclosures: Tables 2-5
Figures 1-7
Traffic Count Reports
Level of Service Reports
Queuing Reports

| Table 2 <br> Trip Generation Estimate The Glen at Widefield Filing No. 9 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Trip Generation Units | Trip Generation Rates ${ }^{(1)}$ |  |  |  |  | Total Trips Generated |  |  |  |  |
|  | Land Use | Land Use |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon Peak Hour |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon Peak Hour |  |
| Filing | Code | Description |  |  | In | Out | In | Out |  | In | Out | In | Out |
| Approved Filings |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | 210 | Single-Family Detached Housing | $148 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,397 | 27 | 82 | 92 | 54 |
| 8 | 210 | Single-Family Detached Housing | 101 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 953 | 19 | 56 | 63 | 37 |
|  |  |  | 249 DU |  |  |  |  |  | 2,351 | 46 | 138 | 155 | 91 |
| Currently Proposed Filing |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9 | 210 | Single-Family Detached Housing | $106 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,001 | 20 | 59 | 66 | 39 |
|  |  | Total | 355 DU |  |  |  |  |  | 3,351 | 66 | 197 | 221 | 130 |
| Future Filings |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | 210 | Single-Family Detached Housing | $40 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 378 | 7 | 22 | 25 | 15 |
| 12 | 210 | Single-Family Detached Housing | 103 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 972 | 19 | 57 | 64 | 38 |
| 13 | 210 | Single-Family Detached Housing | 79 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 746 | 15 | 44 | 49 | 29 |
|  |  |  | 222 DU |  |  |  |  |  | 2,096 | 41 | 123 | 138 | 81 |
|  |  | Total | 577 DU |  |  |  |  |  | 1,123 | 22 | 66 | 74 | 44 |
| Notes: <br> (1) Source: "Trip Generation, 10th Edition, 2017" by the Institute of Transportation Engineers (ITE) <br> (2) DU = dwelling unit |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |  |


| Table 3 <br> Glen East Preliminary Plan CDOT Access Permit and Escrow Analysis <br> Mesa Ridge \& Powers (SH 21) <br> Filings 7, 8, 9 and Future Filings |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Shown in TIS | Subdivisions Currently Proposed |  |  | Currently Proposed Separate Access Permits and Escrow Amounts per Access Permit |  |  |  |
| Number of Lots | Subdivision Name | Number of Lots | Status | Portion of total Escrow of \$103,960 | Access Permits | Access <br> Permit Escrow Amt. | Escrow to be deposited in account with CDOT |
| 148 | Filing 7 | 148 | Platted | \$26,648 | Approved | \$26,648 | NTP Issued; Turn Lane Extended |
| 101 | Filing 8 | 101 | Plat Approved - not recorde | \$18,166 | Application pending | \$18,166 | Prior to issuance of NTP |
| 106 | Filing 9 | 106 | Pending | \$19,065 | Application to be submitted soon | \$19,065 | Prior to issuance of NTP |
| 223 | Remaining Filings | 223 | Future | \$40,081 | Application(s) not submitted | TBD |  |
|  |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |


| Table 4 <br> Glen East Preliminary Plan County Intersection Escrow Analysis Mesa Ridge Parkway \& Spring Glen Drive Intersection Filings 7, 8, 9, and Future Filings |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Shown in TIA | Subdivisions Currently Proposed |  |  | Signal Escrow <br> Amounts |
| Number of Lots | Subdivision Name | Number of Lots | Status | Portion of Total Escrow of \$33,750 |
| 148 | Filing 7 | 148 | Platted | \$8,875 |
| 101 | Filing 8 | 101 | Plat Approved - not recorded | \$6,057 |
| 106 | Filing 9 | 106 | Pending | \$6,189 |
| 223 | Remaining Filings | 223 | Future | \$12,629 |
|  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. August 24, 2016 |  |  |  |  |


| Table 5 <br> Glen East Preliminary Plan County Intersection Signal Escrow Analysis Peaceful Valley Road \& Marksheffel Road Intersection Filings 7, 8, 9, and Future Filings |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Shown in TIS | Subdivisions Currently Proposed |  |  | Signal Escrow Amounts |
| Number of Lots | Subdivision Name | Number of Lots | Status | Portion of total Escrow of \$36,250 |
| 148 | Filing 7 | 148 | Platted | Deferred to Fil 8 |
| 101 | Filing 8 | 101 | Plat Approved - not recorded | \$15,615 |
| 106 | Filing 9 | 106 | Pending | \$6,648 |
| 223 | Remaining Filings | 223 | Future | \$13,987 |
| Note: The escrow amount for Filing 8 includes the deferred amount for Filing 7 Source: LSC Transportation Consultants, Inc. August 24, 2016 |  |  |  |  |




Figure 2








Figure 7
LEGEND:
= Urban Local Low Volume

## Recommended Street Classifications

The Glen at Widefield Fil. 9 (LSC \#174850)

## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Marksheffel Rd - Mesa Ridge Pkwy AM
Site Code : 00174850
Start Date : 11/29/2017
Page No : 1
Groups Printed- Unshifted

|  | Marksheffel Rd From North |  |  |  | From East |  |  |  | Marksheffel Rd From South |  |  |  | Mesa Ridge Pkwy From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 13 | 39 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 59 | 3 | 0 | 7 | 0 | 16 | 0 | 137 |
| 06:45 AM | 10 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 61 | 2 | 0 | 11 | 0 | 7 | 0 | 159 |
| Total | 23 | 107 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 120 | 5 | 0 | 18 | 0 | 23 | 0 | 296 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 07:00 AM | 10 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 63 | 4 | 0 | 18 | 0 | 19 | 0 | 174 |
| $07: 15 \mathrm{AM}$ | 11 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 67 | 3 | 0 | 3 | 0 | 14 | 0 | 149 |
| $07: 30 \mathrm{AM}$ | 6 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 80 | 9 | 0 | 6 | 0 | 16 | 0 | 153 |
| $07: 45 \mathrm{AM}$ | 10 | 41 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 69 | 3 | 0 | 8 | 0 | 12 | 0 | 143 |
| Total | 37 | 188 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 279 | 19 | 0 | 35 | 0 | 61 | 0 | 619 |


| 08:00 AM | 17 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 32 | 6 | 0 | 2 | 0 | 16 | 0 | 111 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 22 | 39 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 53 | 5 | 0 | 1 | 0 | 18 | 0 | 138 |
| Grand Total | 99 | 372 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 484 | 35 | 0 | 56 | 0 | 118 | 0 | 1164 |

File Name : Marksheffel Rd - Mesa Ridge Pkwy AM
Site Code : 00174850
Start Date : 11/29/2017
Page No : 2

|  | Marksheffel Rd From North |  |  |  |  | From East |  |  |  |  | Marksheffel Rd From South |  |  |  |  | Mesa Ridge Pkwy From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | $\begin{gathered} \text { Rig } \\ \text { ht } \\ \hline \end{gathered}$ | $\begin{array}{r\|} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | Thr | Lef t | $\begin{aligned} & \hline \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | Thr | Lef | $\begin{aligned} & \hline \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |




## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Marksheffel Rd - Mesa Ridge Pkwy PM
Site Code : 00174850
Start Date : 11/28/2017
Page No : 1
Groups Printed- Unshifted

|  | Marksheffel Rd From North |  |  |  | From East |  |  |  | Marksheffel Rd From South |  |  |  | Mesa Ridge Pkwy From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 15 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 3 | 0 | 7 | 0 | 26 | 0 | 140 |
| 04:15 PM | 13 | 54 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 49 | 3 | 0 | 3 | 0 | 21 | 0 | 143 |
| 04:30 PM | 10 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 39 | 5 | 0 | 1 | 0 | 25 | 0 | 124 |
| 04:45 PM | 16 | 83 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 39 | 3 | 0 | 3 | 0 | 18 | 0 | 162 |
| Total | 54 | 232 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 165 | 14 | 0 | 14 | 0 | 90 | 0 | 569 |
| 05:00 PM | 9 | 46 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 40 | 4 | 0 | 5 | 1 | 28 | 0 | 133 |
| 05:15 PM | 13 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 42 | 4 | 0 | 7 | 0 | 14 | 0 | 148 |
| 05:30 PM | 7 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 5 | 0 | 7 | 0 | 28 | 0 | 147 |
| 05:45 PM | 5 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 30 | 6 | 0 | 17 | 0 | 22 | 0 | 116 |
| Total | 34 | 207 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 155 | 19 | 0 | 36 | 1 | 92 | 0 | 544 |
| Grand Total | 88 | 439 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 320 | 33 | 0 | 50 | 1 | 182 | 0 | 1113 |
| Apprch \% | 16.7 | 83.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 90.7 | 9.3 | 0.0 | 21.5 | 0.4 | 78.1 | 0.0 |  |
| Total \% | 7.9 | 39.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 28.8 | 3.0 | 0.0 | 4.5 | 0.1 | 16.4 | 0.0 |  |

File Name : Marksheffel Rd - Mesa Ridge Pkwy PM
Site Code : 00174850
Start Date : 11/28/2017
Page No : 2



## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Marksheffel Rd - Peaceful Valley Rd AM
Site Code : 00174850
Start Date : 12/07/2017
Page No : 1
Groups Printed- Unshifted

|  | Marksheffel Rd From North |  |  |  | Peaceful Valley Rd From East |  |  |  | Marksheffel Rd From South |  |  |  | From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 0 | 35 | 2 | 0 | 12 | 0 | 8 | 0 | 0 | 78 | 0 | 0 | 0 | 0 | 0 | 0 | 135 |
| 06:45 AM | 0 | 51 | 1 | 0 | 11 | 0 | 11 | 0 | 5 | 71 | 0 | 0 | 0 | 0 | 0 | 0 | 150 |
| Total | 0 | 86 | 3 | 0 | 23 | 0 | 19 | 0 | 5 | 149 | 0 | 0 | 0 | 0 | 0 | 0 | 285 |


| 207 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- | :--- | :--- | :--- | :--- |
| $07: 00 \mathrm{AM}$ | 0 | 64 | 8 | 0 | 21 | 0 | 22 | 0 | 4 | 88 | 0 | 0 | 0 | 0 | 0 | 0 | 207 |
| 07:15 AM | 0 | 51 | 5 | 0 | 8 | 0 | 8 | 0 | 9 | 76 | 0 | 0 | 0 | 0 | 0 | 0 | 157 |
| 07:30 AM | 0 | 51 | 4 | 0 | 16 | 0 | 10 | 0 | 6 | 59 | 0 | 0 | 0 | 0 | 0 | 0 | 146 |
| $07: 45 \mathrm{AM}$ | 0 | 33 | 4 | 0 | 2 | 0 | 5 | 0 | 9 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 104 |
| Total | 0 | 199 | 21 | 0 | 47 | 0 | 45 | 0 | 28 | 274 | 0 | 0 | 0 | 0 | 0 | 0 | 614 |


| 08:00 AM | 0 | 30 | 6 | 0 | 12 | 0 | 14 | 0 | 16 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 129 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 0 | 32 | 20 | 0 | 10 | 0 | 19 | 0 | 13 | 31 | 0 | 0 | 0 | 0 | 0 | 0 | 125 |
| Grand Total | 0 | 347 | 50 | 0 | 92 | 0 | 97 | 0 | 62 | 505 | 0 | 0 | 0 | 0 | 0 | 0 | 1153 |

File Name : Marksheffel Rd - Peaceful Valley Rd AM
Site Code : 00174850
Start Date : 12/07/2017
Page No : 2

|  | Marksheffel Rd From North |  |  |  |  | Peaceful Valley Rd From East |  |  |  |  | Marksheffel Rd From South |  |  |  |  | From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r\|} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \hline \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | Thr u | Lef t | Pe <br> ds | App. <br> Total | Int. Total |




## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Marksheffel Rd - Peaceful Valley Rd PM
Site Code : 00174850
Start Date : 12/07/2017
Page No : 1
Groups Printed- Unshifted

|  | Marksheffel Rd From North |  |  |  | Peaceful Valley Rd From East |  |  |  | Marksheffel Rd From South |  |  |  | From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 0 | 64 | 13 | 0 | 4 | 0 | 4 | 0 | 13 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 142 |
| 04:15 PM | 0 | 74 | 10 | 0 | 6 | 0 | 10 | 0 | 13 | 59 | 0 | 0 | 0 | 0 | 0 | 0 | 172 |
| 04:30 PM | 0 | 67 | 7 | 0 | 7 | 0 | 7 | 0 | 9 | 50 | 0 | 0 | 0 | 0 | 0 | 0 | 147 |
| 04:45 PM | 0 | 77 | 10 | 0 | 9 | 0 | 9 | 0 | 13 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 162 |
| Total | 0 | 282 | 40 | 0 | 26 | 0 | 30 | 0 | 48 | 197 | 0 | 0 | 0 | 0 | 0 | 0 | 623 |


| $05: 00 ~ P M ~$ | 0 | 55 | 8 | 0 | 10 | 0 | 8 | 0 | 13 | 69 | 0 | 0 | 0 | 0 | 0 | 0 | 163 |
| ---: | :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- | :--- | :--- | :--- | :--- |
| $05: 15 \mathrm{PM}$ | 0 | 72 | 12 | 0 | 6 | 0 | 11 | 0 | 13 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 167 |
| $05: 30 \mathrm{PM}$ | 0 | 57 | 5 | 0 | 2 | 0 | 4 | 0 | 12 | 42 | 0 | 0 | 0 | 0 | 0 | 0 | 122 |
| $05: 45 \mathrm{PM}$ | 0 | 42 | 8 | 0 | 4 | 0 | 5 | 0 | 11 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 108 |
| Total | 0 | 226 | 33 | 0 | 22 | 0 | 28 | 0 | 49 | 202 | 0 | 0 | 0 | 0 | 0 | 0 | 560 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Grand Total | 0 | 508 | 73 | 0 | 48 | 0 | 58 | 0 | 97 | 399 | 0 | 0 | 0 | 0 | 0 | 0 | 1183 |
| Apprch \% | 0.0 | 87.4 | 12.6 | 0.0 | 45.3 | 0.0 | 54.7 | 0.0 | 19.6 | 80.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total \% | 0.0 | 42.9 | 6.2 | 0.0 | 4.1 | 0.0 | 4.9 | 0.0 | 8.2 | 33.7 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |

File Name : Marksheffel Rd - Peaceful Valley Rd PM
Site Code : 00174850
Start Date : 12/07/2017
Page No : 2


## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Powers Blvd - Mesa Ridge AM
Site Code : 00174850
Start Date : 11/30/2017
Page No : 1
Groups Printed- Unshifted

|  | Powers Blvd From North |  |  |  | Mesa Ridge Pkwy From East |  |  |  | Powers Blvd From South |  |  |  | From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 0 | 152 | 9 | 0 | 11 | 0 | 65 | 0 | 19 | 100 | 0 | 0 | 0 | 0 | 0 | 0 | 356 |
| 06:45 AM | 0 | 232 | 12 | 0 | 13 | 0 | 81 | 0 | 20 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 469 |
| Total | 0 | 384 | 21 | 0 | 24 | 0 | 146 | 0 | 39 | 211 | 0 | 0 | 0 | 0 | 0 | 0 | 825 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 07:00 AM | 0 | 186 | 9 | 0 | 10 | 0 | 71 | 0 | 24 | 146 | 0 | 0 | 0 | 0 | 0 | 0 | 446 |
| 07:15 AM | 0 | 128 | 11 | 0 | 18 | 0 | 67 | 0 | 36 | 136 | 0 | 0 | 0 | 0 | 0 | 0 | 396 |
| 07:30 AM | 0 | 151 | 13 | 0 | 13 | 0 | 64 | 0 | 26 | 130 | 0 | 0 | 0 | 0 | 0 | 0 | 397 |
| $07: 45 \mathrm{AM}$ | 0 | 140 | 11 | 0 | 4 | 0 | 57 | 0 | 38 | 126 | 0 | 0 | 0 | 0 | 0 | 0 | 376 |
| Total | 0 | 605 | 44 | 0 | 45 | 0 | 259 | 0 | 124 | 538 | 0 | 0 | 0 | 0 | 0 | 0 | 1615 |


| 08:00 AM | 0 | 127 | 8 | 0 | 15 | 0 | 40 | 0 | 45 | 122 | 0 | 0 | 0 | 0 | 0 | 0 | 357 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 0 | 166 | 17 | 0 | 19 | 0 | 72 | 0 | 26 | 105 | 0 | 0 | 0 | 0 | 0 | 0 | 405 |
| Grand Total | 0 | 1282 | 90 | 0 | 103 | 0 | 517 | 0 | 234 | 976 | 0 | 0 | 0 | 0 | 0 | 0 | 3202 |
| Apprch \% | 0.0 | 93.4 | 6.6 | 0.0 | 16.6 | 0.0 | 83.4 | 0.0 | 19.3 | 80.7 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total \% | 0.0 | 40.0 | 2.8 | 0.0 | 3.2 | 0.0 | 16.1 | 0.0 | 7.3 | 30.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |

File Name : Powers Blvd - Mesa Ridge AM
Site Code : 00174850
Start Date : 11/30/2017
Page No : 2


## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Powers Blvd - Mesa Ridge PM
Site Code : 00174850
Start Date : 11/30/2017
Page No : 1
Groups Printed- Unshifted

|  | Powers Blvd From North |  |  |  | Mesa Ridge Pkwy From East |  |  |  | Powers Blvd From South |  |  |  | From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 0 | 136 | 30 | 0 | 7 | 0 | 33 | 0 | 59 | 192 | 0 | 0 | 0 | 0 | 0 | 0 | 457 |
| 04:15 PM | 0 | 151 | 23 | 0 | 10 | 0 | 32 | 0 | 67 | 208 | 0 | 0 | 0 | 0 | 0 | 0 | 491 |
| 04:30 PM | 0 | 139 | 27 | 0 | 13 | 0 | 33 | 0 | 79 | 201 | 0 | 0 | 0 | 0 | 0 | 0 | 492 |
| 04:45 PM | 0 | 153 | 24 | 0 | 16 | 0 | 30 | 0 | 85 | 243 | 0 | 0 | 0 | 0 | 0 | 0 | 551 |
| Total | 0 | 579 | 104 | 0 | 46 | 0 | 128 | 0 | 290 | 844 | 0 | 0 | 0 | 0 | 0 | 0 | 1991 |
| 05:00 PM | 0 | 125 | 22 | 1 | 21 | 0 | 23 | 0 | 77 | 229 | 0 | 0 | 0 | 0 | 0 | 0 | 498 |
| 05:15 PM | 0 | 154 | 31 | 0 | 17 | 0 | 37 | 0 | 81 | 209 | 0 | 0 | 0 | 0 | 0 | 0 | 529 |
| 05:30 PM | 0 | 138 | 24 | 0 | 13 | 0 | 36 | 0 | 69 | 240 | 0 | 0 | 0 | 0 | 0 | 0 | 520 |
| 05:45 PM | 0 | 113 | 30 | 0 | 9 | 0 | 36 | 0 | 96 | 207 | 0 | 0 | 0 | 0 | 0 | 0 | 491 |
| Total | 0 | 530 | 107 | 1 | 60 | 0 | 132 | 0 | 323 | 885 | 0 | 0 | 0 | 0 | 0 | 0 | 2038 |
| Grand Total | 0 | 1109 | 211 | 1 | 106 | 0 | 260 | 0 | 613 | 1729 | 0 | 0 | 0 | 0 | 0 | 0 | 4029 |
| Apprch \% | 0.0 | 84.0 | 16.0 | 0.1 | 29.0 | 0.0 | 71.0 | 0.0 | 26.2 | 73.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total \% | 0.0 | 27.5 | 5.2 | 0.0 | 2.6 | 0.0 | 6.5 | 0.0 | 15.2 | 42.9 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |

File Name : Powers Blvd - Mesa Ridge PM
Site Code : 00174850
Start Date : 11/30/2017
Page No : 2


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{r}$ | $\mathbf{r}$ | $\mathbf{4}$ | $\mathbf{r}$ | $\mathbf{1}$ | $\mathbf{4}$ |
| Traffic Vol, veh/h | 51 | 56 | 329 | 24 | 18 | 227 |
| Future Vol, veh/h | 51 | 56 | 329 | 24 | 18 | 227 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | Stop | - | None | - | None |
| Storage Length | 0 | 0 | - | 290 | 340 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 62 | 62 | 86 | 86 | 82 | 82 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 82 | 90 | 383 | 28 | 22 | 277 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | $\mathbf{r}$ | $\mathbf{r}$ | 1 | 4 | 个 | $\mathbf{r}$ |
| Traffic Vol, veh/h | 92 | 41 | 14 | 261 | 191 | 87 |
| Future Vol, veh/h | 92 | 41 | 14 | 261 | 191 | 87 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 500 | - | - | 290 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 94 | 94 | 80 | 80 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 102 | 46 | 15 | 278 | 239 | 109 |





|  | 7 | $4$ | $\dagger$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | 「 | 个个 | 「 | \％ | 个个 |
| Traffic Volume（vph） | 350 | 154 | 484 | 136 | 64 | 669 |
| Future Volume（vph） | 350 | 154 | 484 | 136 | 64 | 669 |
| Turn Type | Prot | Perm | NA | Perm | pm＋pt | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 40.0 | 40.0 | 40.0 | 40.0 | 10.0 | 50.0 |
| Total Split（\％） | 44．4\％ | 44．4\％ | 44．4\％ | 44．4\％ | 11．1\％ | 55．6\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 24.2 | 24.2 | 46.1 | 46.1 | 55.8 | 55.8 |
| Actuated g／C Ratio | 0.27 | 0.27 | 0.51 | 0.51 | 0.62 | 0.62 |
| v／c Ratio | 0.77 | 0.30 | 0.28 | 0.16 | 0.13 | 0.34 |
| Control Delay | 40.8 | 5.1 | 15.1 | 3.6 | 9.1 | 9.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 40.8 | 5.1 | 15.1 | 3.6 | 9.1 | 9.6 |
| LOS | D | A | B | A | A | A |
| Approach Delay | 30.0 |  | 12.6 |  |  | 9.6 |
| Approach LOS | C |  | B |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： $0(0 \%)$ ，Referenced to phase 2：NBT and $6: S B T L$ ，Start of Green
Natural Cycle： 45
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.77
Intersection Signal Delay： $15.9 \quad$ Intersection LOS：B
Intersection Capacity Utilization 48．8\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Mesa Ridge Pkwy／Powers Blvd \＆Mesa Ridge Pkway


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{r}$ | $\mathbf{r}$ | 个 | $\mathbf{r}$ | $\mathbf{1}$ | $\mathbf{4}$ |
| Traffic Vol, veh/h | 32 | 27 | 315 | 51 | 35 | 307 |
| Future Vol, veh/h | 32 | 27 | 315 | 51 | 35 | 307 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | Stop | - | None | - | None |
| Storage Length | 0 | 0 | - | 290 | 340 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 87 | 87 | 98 | 98 | 88 | 88 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 37 | 31 | 321 | 52 | 40 | 349 |






| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 140 | 0 | 0 | 0 | 408 | 133 |  |
| Stage 1 | - | - | - - | - | 133 | - |  |
| Stage 2 | - | - | - - | - | 275 | - |  |
| Critical Hdwy | 4.12 | - | - - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - - | - | 3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1443 | - | - - | - | 599 | 916 |  |
| Stage 1 | - | - | - - | - | 893 | - |  |
| Stage 2 | - | - | - - | - | 771 | - |  |
| Platoon blocked, \% |  | - | - - | - |  |  |  |
| Mov Cap-1 Maneuver | 1443 | - | - - | - | 579 | 916 |  |
| Mov Cap-2 Maneuver | - | - | - - | - | 579 | - |  |
| Stage 1 | - | - | - - | - | 864 | - |  |
| Stage 2 | - | - | - - | - | 771 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 1.6 |  | 0 |  | 9.5 |  |  |
| HCM LOS |  |  |  |  | A |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1443 | - | - | - | 579 | 916 |
| HCM Lane V/C Ratio |  | 0.033 | , | - | - | 0.017 | 0.05 |
| HCM Control Delay (s) |  | 7.6 | , | - | - | 11.3 | 9.1 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0.1 | , | - | - | 0.1 | 0.2 |


|  | 7 |  | $\dagger$ | $>$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 7 | F | 个4 | 「 | \％ | 个个 |
| Traffic Volume（vph） | 205 | 58 | 685 | 422 | 122 | 558 |
| Future Volume（vph） | 205 | 58 | 685 | 422 | 122 | 558 |
| Turn Type | Prot | Perm | NA | Perm | pm＋pt | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 40.0 | 40.0 | 40.0 | 40.0 | 10.0 | 50.0 |
| Total Split（\％） | 44．4\％ | 44．4\％ | 44．4\％ | 44．4\％ | 11．1\％ | 55．6\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 15.9 | 15.9 | 51.3 | 51.3 | 64.1 | 64.1 |
| Actuated g／C Ratio | 0.18 | 0.18 | 0.57 | 0.57 | 0.71 | 0.71 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.66 | 0.18 | 0.35 | 0.40 | 0.26 | 0.25 |
| Control Delay | 44.0 | 9.3 | 12.0 | 2.5 | 6.1 | 5.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 44.0 | 9.3 | 12.0 | 2.5 | 6.1 | 5.3 |
| LOS | D | A | B | A | A | A |
| Approach Delay | 36.4 |  | 8.4 |  |  | 5.5 |
| Approach LOS | D |  | A |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： $0(0 \%)$ ，Referenced to phase 2：NBT and $6: S B T L$ ，Start of Green
Natural Cycle： 40
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.66
Intersection Signal Delay：10．8 Intersection LOS：B
Intersection Capacity Utilization 49．6\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Mesa Ridge Pkwy／Powers Blvd \＆Mesa Ridge Pkway


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 3.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\leqslant$ |  |  | $\uparrow$ | 「゙ | ${ }^{1 /}$ | 4 | 「 | ${ }^{1 /}$ | $\uparrow$ |  |
| Traffic Vol，veh／h | 12 | 0 | 2 | 51 | 0 | 56 | 1 | 351 | 24 | 18 | 289 | 4 |
| Future Vol，veh／h | 12 | 0 | 2 | 51 | 0 | 56 | 1 | 351 | 24 | 18 | 289 | 4 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Stap | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | － | － | None | － | － | Stop | － | － | None | － | － | None |
| Storage Length | － | － | － | － | － | 0 | 340 | － | 290 | 340 | － | － |
| Veh in Median Storage，\＃ | \＃ | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 92 | 92 | 62 | 62 | 62 | 86 | 86 | 86 | 82 | 82 | 82 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 0 | 2 | 82 | 0 | 90 | 1 | 408 | 28 | 22 | 352 | 5 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |





10：Mesa Ridge Pkwy／Powers Blvd \＆Mesa Ridge Pkway

|  | 7 | $4$ | $\dagger$ | $p$ | $\checkmark$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \％${ }^{1 / 1}$ | F＇ | 个个 | 「 | ${ }^{*}$ | 个个 |
| Traffic Volume（vph） | 432 | 178 | 515 | 164 | 72 | 712 |
| Future Volume（vph） | 432 | 178 | 515 | 164 | 72 | 712 |
| Turn Type | Prot | Perm | NA | Perm | pm＋pt | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 30.0 | 30.0 | 50.0 | 50.0 | 10.0 | 60.0 |
| Total Split（\％） | 33．3\％ | 33．3\％ | 55．6\％ | 55．6\％ | 11．1\％ | 66．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 17.2 | 17.2 | 53.2 | 53.2 | 62.8 | 62.8 |
| Actuated g／C Ratio | 0.19 | 0.19 | 0.59 | 0.59 | 0.70 | 0.70 |
| v／c Ratio | 0.69 | 0.41 | 0.26 | 0.17 | 0.14 | 0.32 |
| Control Delay | 39.3 | 7.4 | 10.6 | 2.4 | 5.6 | 6.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 39.3 | 7.4 | 10.6 | 2.4 | 5.6 | 6.1 |
| LOS | D | A | B | A | A | A |
| Approach Delay | 30.0 |  | 8.6 |  |  | 6.0 |
| Approach LOS | C |  | A |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： $0(0 \%)$ ，Referenced to phase 2：NBT and $6: S B T L$ ，Start of Green
Natural Cycle： 40
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.69
Intersection Signal Delay： $13.7 \quad$ Intersection LOS：B
Intersection Capacity Utilization 43．0\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Mesa Ridge Pkwy／Powers Blvd \＆Mesa Ridge Pkway




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.9 |  |  |  |  |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 159 | 0 | - | 0 | 685 | 151 |  |
| Stage 1 | - | - | - | - | 151 | - |  |
| Stage 2 | - | - | - | - | 534 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1420 | - | - | - | 414 | 895 |  |
| Stage 1 | - | - | - | - | 877 | - |  |
| Stage 2 | - | - | - | - | 588 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1420 | - | - | - | 368 | 895 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 368 | - |  |
| Stage 1 | - | - | - | - | 780 | - |  |
| Stage 2 | - | - | - | - | 588 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 3.3 |  | 0 |  | 10.1 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR | SBLn1 | BLn2 |
| Capacity (veh/h) |  | 1420 | - | - | - | 368 | 895 |
| HCM Lane V/C Ratio |  | 0.111 | - | - | - | 0.032 | 0.121 |
| HCM Control Delay (s) |  | 7.9 | - | - | - | 15.1 | 9.6 |
| HCM Lane LOS |  | A | - | - | - | C | A |
| HCM 95th \%tile Q(veh) |  | 0.4 | - | - | - | 0.1 | 0.4 |

10：Powers Blvd \＆Mesa Ridge Pkway

|  | 7 | $4$ | $\uparrow$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \％${ }^{*}$ | 「 | 个个 | 「 | ${ }^{*}$ | 个 $\uparrow$ |
| Traffic Volume（vph） | 259 | 73 | 730 | 514 | 148 | 594 |
| Future Volume（vph） | 259 | 73 | 730 | 514 | 148 | 594 |
| Turn Type | Prot | Perm | NA | Perm | pm＋pt | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 30.0 | 30.0 | 50.0 | 50.0 | 10.0 | 60.0 |
| Total Split（\％） | 33．3\％ | 33．3\％ | 55．6\％ | 55．6\％ | 11．1\％ | 66．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 12.1 | 12.1 | 55.0 | 55.0 | 67.9 | 67.9 |
| Actuated g／C Ratio | 0.13 | 0.13 | 0.61 | 0.61 | 0.75 | 0.75 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.56 | 0.26 | 0.35 | 0.45 | 0.31 | 0.25 |
| Control Delay | 40.9 | 11.0 | 9.8 | 2.3 | 4.9 | 3.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 40.9 | 11.0 | 9.8 | 2.3 | 4.9 | 3.8 |
| LOS | D | B | A | A | A | A |
| Approach Delay | 34.3 |  | 6.7 |  |  | 4.0 |
| Approach LOS | C |  | A |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： $0(0 \%)$ ，Referenced to phase 2：NBT and $6: S B T L$ ，Start of Green
Natural Cycle： 40
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.56
Intersection Signal Delay： $9.5 \quad$ Intersection LOS：A
Intersection Capacity Utilization 48．4\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Powers Blvd \＆Mesa Ridge Pkway




| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 7 | ${ }^{7}$ | 4 | 4 | 「 |
| Traffic Vol, veh/h | 103 | 47 | 16 | 276 | 236 | 109 |
| Future Vol, veh/h | 103 | 47 | 16 | 276 | 236 | 109 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 500 | - | - | 290 |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 90 | 90 | 94 | 94 | 80 | 80 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 114 | 52 | 17 | 294 | 295 | 136 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.8 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{*}$ | + | F |  | ${ }^{7}$ |  |
| Traffic Vol, veh/h | 92 | 132 | 103 | 21 | 18 | 180 |
| Future Vol, veh/h | 92 | 132 | 103 | 21 | 18 | 180 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 475 | - | - | - | 0 | 0 |
| Veh in Median Storage, \# | \# - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 83 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvut Flow | 100 | 159 | 112 | 23 | 20 | 196 |



|  | $\checkmark$ |  | $\dagger$ | ＞ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \％ | F | 个4 | 「 | \％ | 个4 |
| Trafic Volume（vph） | 467 | 187 | 515 | 176 | 75 | 712 |
| Future Volume（vph） | 467 | 187 | 515 | 176 | 75 | 712 |
| Turn Type | Prot | Perm | NA | Perm | $\mathrm{pm}+\mathrm{pt}$ | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 30.0 | 30.0 | 50.0 | 50.0 | 10.0 | 60.0 |
| Total Split（\％） | 33．3\％ | 33．3\％ | 55．6\％ | 55．6\％ | 11．1\％ | 66．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effict Green（s） | 18.2 | 18.2 | 52.3 | 52.3 | 61.8 | 61.8 |
| Actuated g／C Ratio | 0.20 | 0.20 | 0.58 | 0.58 | 0.69 | 0.69 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.70 | 0.41 | 0.27 | 0.19 | 0.14 | 0.32 |
| Control Delay | 38.6 | 7.0 | 11.0 | 2.4 | 6.0 | 6.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 38.6 | 7.0 | 11.0 | 2.4 | 6.0 | 6.6 |
| LOS | D | A | B | A | A | A |
| Approach Delay | 29.6 |  | 8.8 |  |  | 6.5 |
| Approach LOS | C |  | A |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： $0(0 \%)$ ，Referenced to phase 2：NBT and $6: S B T L$ ，Start of Green
Natural Cycle： 40
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.70
Intersection Signal Delay：14．1 Intersection LOS：B
Intersection Capacity Utilization 44．2\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Mesa Ridge Pkwy／Powers Blvd \＆Mesa Ridge Pkway




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





10：Powers Blvd \＆Mesa Ridge Pkway

|  | 7 | $4$ | $\dagger$ | $p$ | － | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \％${ }^{1 / 1}$ | 「 | 个个 | 「 | ${ }^{7}$ | 个个 |
| Traffic Volume（vph） | 282 | 79 | 730 | 554 | 158 | 594 |
| Future Volume（vph） | 282 | 79 | 730 | 554 | 158 | 594 |
| Turn Type | Prot | Perm | NA | Perm | pm＋pt | NA |
| Protected Phases | 8 |  | 2 |  | 1 | 6 |
| Permitted Phases |  | 8 |  | 2 | 6 |  |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 30.0 | 30.0 | 50.0 | 50.0 | 10.0 | 60.0 |
| Total Split（\％） | 33．3\％ | 33．3\％ | 55．6\％ | 55．6\％ | 11．1\％ | 66．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag |  |  | Lag | Lag | Lead |  |
| Lead－Lag Optimize？ |  |  | Yes | Yes | Yes |  |
| Recall Mode | None | None | C－Max | C－Max | None | C－Max |
| Act Effct Green（s） | 12.7 | 12.7 | 54.1 | 54.1 | 67.3 | 67.3 |
| Actuated g／C Ratio | 0.14 | 0.14 | 0.60 | 0.60 | 0.75 | 0.75 |
| v／c Ratio | 0.58 | 0.27 | 0.35 | 0.48 | 0.33 | 0.25 |
| Control Delay | 40.8 | 10.6 | 10.3 | 2.5 | 5.3 | 4.0 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 40.8 | 10.6 | 10.3 | 2.5 | 5.3 | 4.0 |
| LOS | D | B | B | A | A | A |
| Approach Delay | 34.2 |  | 6.9 |  |  | 4.3 |
| Approach LOS | C |  | A |  |  | A |
| Intersection Summary |  |  |  |  |  |  |

Cycle Length： 90
Actuated Cycle Length： 90
Offset： 0 （0\％），Referenced to phase 2：NBT and 6：SBTL，Start of Green
Natural Cycle： 40
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.58
Intersection Signal Delay： 9.9 Intersection LOS：A
Intersection Capacity Utilization 51．4\％ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：10：Powers Blvd \＆Mesa Ridge Pkway


Intersection: 10: Mesa Ridge Pkwy/Powers Blvd \& Mesa Ridge Pkway

| Movement | WB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | R | T | T | R | L | T | T |
| Maximum Queue (ft) | 200 | 301 | 95 | 198 | 185 | 78 | 74 | 140 | 157 |
| Average Queue (ft) | 115 | 171 | 43 | 93 | 44 | 34 | 34 | 86 | 49 |
| 95th Queue (ft) | 214 | 235 | 81 | 159 | 121 | 65 | 61 | 140 | 110 |
| Link Distance (ft) |  |  | 824 | 517 | 517 |  |  | 1437 | 1437 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 350 | 350 |  |  | 0 |  |  |  |  |

Intersection: 10: Powers Blvd \& Mesa Ridge Pkway

| Movement | WB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | L | R | T | T | R | L | T | T |
| Maximum Queue (ft) | 150 | 197 | 79 | 259 | 262 | 248 | 237 | 118 | 74 |
| Average Queue (ft) | 49 | 124 | 18 | 158 | 89 | 107 | 90 | 61 | 39 |
| 95th Queue (ft) | 130 | 180 | 42 | 232 | 201 | 180 | 161 | 113 | 75 |
| Link Distance (ft) |  |  | 824 | 517 | 517 |  |  | 1624 | 1624 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 350 | 350 |  |  | 1 | 150 | 1000 |  |  |
| Storage Blk Time (\%) |  |  |  |  | 6 | 4 |  |  |  |

## Markup Summary

dsdlaforce (1)


Subject: Callout<br>Page Label: 8<br>Author: dsdlaforce<br>Date: 8/29/2018 3:13:26 PM<br>Color:

1. Instead of removing the white channelized "T" pavement markings in the intersection concider replacing with a dotted line left turn markings to channelize the EBLT into the existing accel lane. 2. It seems the TIS is assuming only small amount of the Glen East (Filing 7-9) will be using this for ingress/egress. The concern is that the amount of site generated traffic may be higher than anticipated and without a dedicated auxiliary turn lanes the general through traffic on Marksheffel Road (which has a high posted speed limit) will have to significantly reduce their speed at this intersection because of vehicles turning into the subdivision. Include the trip distribution figure and elaborate the narrative to detail the assumptions made and its impact to the intersection.
