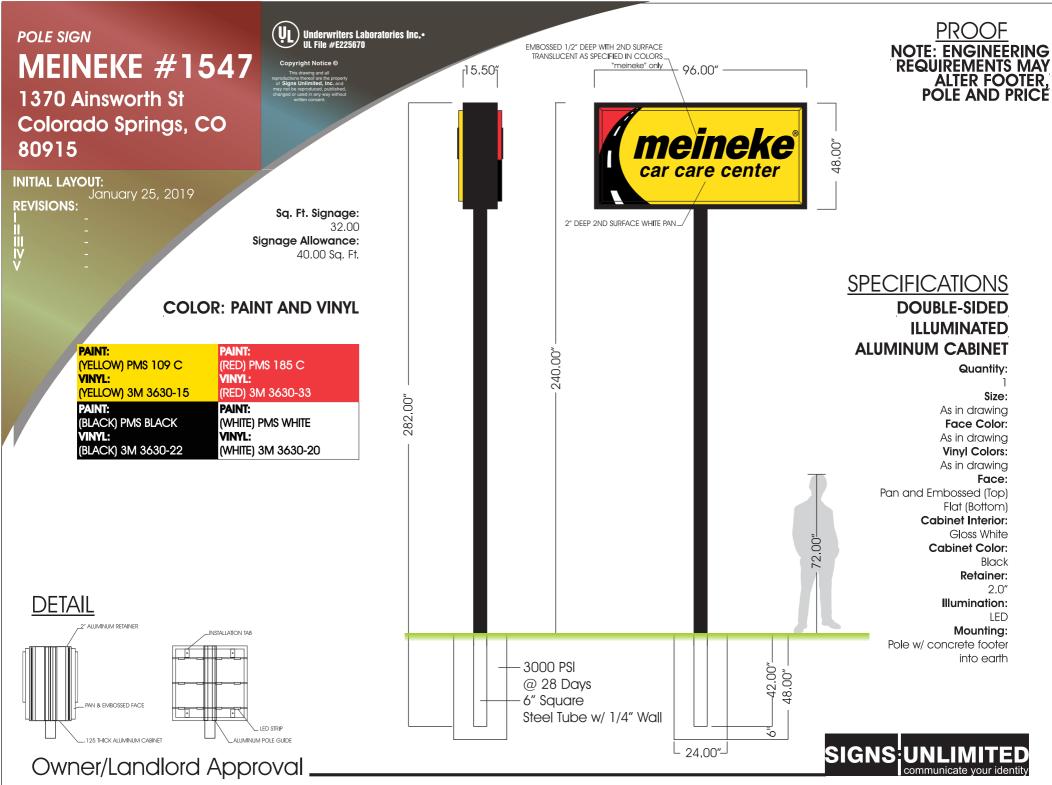
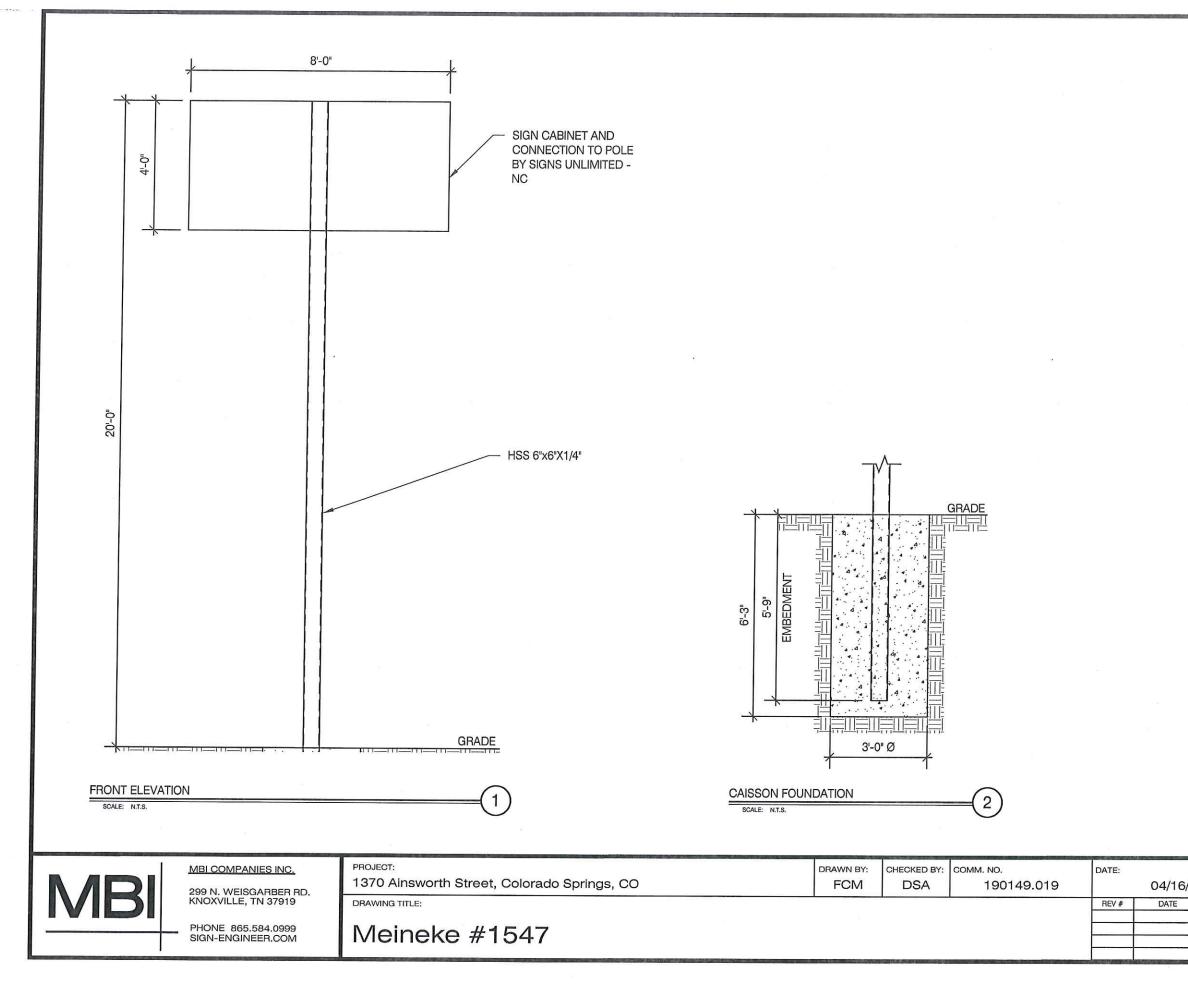


EL PASO COUNTY PLANNING & COMMUNITY DEVELOPMENT SIGN APPLICATION

	Vicinity Map Tax Schedule #	Sign Plan (Please indicate dimensions and sign copy)
DATE:	Please show major intersections.	
<u>Business</u>	NORTH	
Name: <u>Meineke #1547</u> Address: <u>1370 Ainsworth</u> <u>Colorado Springs, C</u> O		
Zone: _CC Legal Description:		
<u>Type of Sign</u>		Please find attached.
Illuminated: <u>Yes</u> Non-Illuminated: <u></u> Valuation: <u>8800</u>		
Storefront Length &/or Width:		
Proposed Sign Sq. Ft. 32		
Existing Sign Sq. Ft.	Elevation Drawing Indicate storefront length & placement of	
Total Sign Sq. Ft	sign.	Approved
<u>Contractor Information</u> Name: <u>A Budget Signs</u>		By:Gabe Sevigny Date:05/29/2019 El Paso County Planning & Community Development
Address: 3224 N Nevada		Er Faso County Planning & Community Development
<u>Ave, Colorado Spri</u> ng	S N/A Sign is Freestanding	Allowed 2 sqft for each
Phone: 719 635 8811		A linear foot of the building
Type of License: <u>Sign</u>		40 sqft, whichever is 01 footsandles at the
Contractor ID#_17040		district.
Fee of \$262	2.00 must accompany this Application. Additional si	gns lake checks payable to Err add downly.



6801 Mount Hermon Church Rd, Ste C, Durham, NC 27705 • (P) 919-552-8689 • (F) 919-557-1322



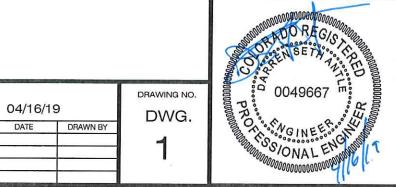
NOTES

1.) SEE MANUFACTURERS DRAWINGS FOR ADDITIONAL DETAILS AND DIMENSIONS.

2.) SIGN CABINET AND CONNECTION BY SIGNS UNLIMITED - NC.

* CLIENT - SIGNS UNLIMITED - NC

- * PIKES PEAK RBC 2017 EDITION (2015 IBC)
- * 130 MPH WIND SPEED, EXP. C
- * (1) POLE, (1) FOOTING



DESIGN SPECIFICATIONS:

REFER TO SIGN COMPANY'S DRAWINGS FOR MORE DETAILS. ALL DESIGNS, DETAILING FABRICATION AND CONSTRUCTION SHALL CONFORM TO:

PIKES PEAK RBC 2017 EDITION (2015 IBC)

ACI

AISC

AMERICAN WELDING SOCIETY

LOCAL BUILDING CODES & ORDINANCES

CONCRETE: 2500 PSI @ 28 DAYS

STD. STEEL PIPE SECTION: ASTM A53 GRADE B (Fy=35 KSI), U.N.O.

STEEL PIPE SECTION (> 20" Ø): ASTM A252 GRADE 3 (Fy=42 KSI MIN.) U.N.O.

HSS ROUND SECTION: ASTM A500 GRADE B (Fy=42 KSI) U.N.O.

HSS SQUARE/RECTANGULAR SECTION: ASTM A500 GRADE B (Fy=46 KSI)

W SHAPES: ASTM A992 (Fy = 50 KSI)

ANCHOR BOLTS: ASTM F1554 GRADE 36 U.N.O. (ALTERNATES GRADE 55 & 105) CONNECTION BOLTS: ASTM A325

THREADED RODS: ASTM A193 GRADE B7

STEEL ANGLES, CHANNELS, STRUCTURAL SHAPES & PLATES ASTM A36 REINFORCING: GRADE 60 ASTM A615

PROVIDE A MINIMUM OF THREE INCHES OF CONCRETE COVER OVER EMBEDDED STEEL.

THE CONTRACTOR (INSTALLER) IS RESPONSIBLE FOR THE MEANS &

METHODS OF CONSTRUCTION IN REGARDS TO JOBSITE SAFETY. NO FIELD HEATING FOR BENDING OR CUTTING OF STEEL SHALL BE

ALLOWED WITHOUT THE ENGINEER'S APPROVAL.

WELDING ELECTRODES: E70XX

ALLOWABLE SOIL BEARING PRESSURE ASSUMED: 2000 PSF

ASSUMED HORIZONTAL (PASSIVE PRESSURE) ASSUMED AT 150 PSF/FT OF DEPTH. ISOLATED LATERAL BEARING FOUNDATIONS FOR SIGNS NOT ADVERSELY AFFECTED

- A 1/2" MOTION AT THE GROUND SURFACE DUE TO SHORT TERM LATERAL LOADS SHALL BE PERMITTED TO BE DESIGNED USING TWO TIMES THE TABULATED CODE VALUES.
- ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED RESIDUAL SOIL AND/OR ENGINEERED EARTH.
- FILL COMPACTED TO 98% OF ITS MAXIMUM DRY DENSITY AS PER ASTM D 698-70 (STANDARD PROCTOR) UNLESS NOTED OTHERWISE. THE SOIL BEARING CAPACITY IS TO BE VERIFIED BY A GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION. IF ALLOWABLE BEARING AND/OR LATERAL PRESSURE IS LESS THAN THE ABOVE ASSUMED AND/OR CALCULATED PRESSURES, THE ENGINEER SHOULD BE CONTACTED FOR RE-EVALUATION.

EXCAVATION SHALL BE FREE OF LOOSE SOIL BEFORE POURING CONCRETE.

WELDERS SHALL BE CERTIFIED FOR THE TYPE OF WELDING.

ADEQUATELY BRACE POLE(S) UNTIL CONCRETE HAS SET UP FOR 14 DAYS. GROUT UNDER BASE PLATES WITH NON-SHRINK GROUT.

THIS ENGINEER DOES NOT WARRANT THE ACCURACY OF DIMENSIONS FURNISHED BY OTHERS.

ALL EXPOSED STEEL SHALL BE PAINTED WITH AN ENAMEL PAINT TO INHIBIT CORROSION.

THIS DESIGN IS FOR THE INDICATED ADDRESS ONLY, AND SHOULD NOT BE USED AT OTHER LOCATIONS WITHOUT WRITTEN PERMISSION OF THE ENGINEER. DESIGN OF DETAILS AND STRUCTURAL MEMBERS NOT SHOWN, BY OTHERS.

WIND D	AIA												DEFLECT	ION ANAL	1515	
Building	Code	Pikes Peak RBC 20	01 Importar	ice Factor	,1	1.0		Damping	Ratio, β		0.005		Deflectio	on Limit	1.1	
Wind Loa	ad Criteria	ASCE 7-10	Direction	ality Facto	or, K ₄ ⁽²⁾	0.85		Natural Fi	equency	, n ₁	1.59 Hz		Deflectio	on at 0.7*V	1	
Wind Speed, V 130 mph					1.0	Gust Effect Factor, G			0.85		Deflection Ratio			1		
Exposure	Category	C	Base Pres	ssure, y(q,	/K.,	22.1 psf		ASD Wind	Load Fac	tor. y (5)	0.6					
Wind Pre	essure Over	ride per				Notes:	(1) Looo	ing values			based upon	n average	K, volue	s for each s	egment.	Actua
Jurisdicti	on Regulre	0 psf					A CONTRACT OF	d on hidde				a na garan ang sa	2010 • 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		Carlo anna a	
								directiona		1999 - 1999 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -						2010/02/2010
		0	122				101100-011100		1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -					2	teuti uj 0.	05. 11
	RY INPUT	and the second	: No					6-21 has b							I and Car	
No. of Po	les	1 No. of Footings	1		1 11			pressures	listea bel	ow nave a		20100200	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	200 - Contra		
11 11 - 12 - 12 - 12	- 1920		Height	Width	Horiz.	Area	Тор	Centrold			Wind		port Pole			ooting
Section	Location	Туре			Offset		Elev.		K,	G	Press.	Trib.	Shear	Moment	Trib.	Shea
			ft	ft	ft	sq ft	ft	ft			psf	Factor	kips	k-ft	Factor	kips
1	Base	Single Pole (Not Round)	16.00	0.50		8.0	16.0	8.0	0.86	1.89	30.6	1.0	0.2	2.0	1.0	0.2
2		Single Pole w/ Cabinet	4.00	8.00	00.21	32.0	20.0	18.0	0.90	1.81	30.6	1.0	1.0	17.6	1.0	1.0
3		None			an in start	0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
4		None		N. 124.7		0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
5		None	and the second second			0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
6		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
7		None	Contraction of the		1.0.0	0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
8		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
9		None			int fint	0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
10	Тор	None			1000 miles	0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0
		Overall Height:	20.00 ft				Sun	mation bo	sed upon	average	s above:		1.2	19.6		1.2
													1.2	19.2		1.2

Base Elev Section			Requ	ired Stren	gth Values	(ASD)	Allow	able Stren	gth Value:	s (ASD)		Unity	Ratios		Interaction Ratios		
	Section	Axis	V,	M,	T,	Ρ,	٧,	M,	T,	Pc	V./V.	M./M.	'T,/Te	P./P.	D M	P-M-V-T	Statu
			kips	kip-ft	kip-ft	kips	kips	kip-ft	kip-ft	kips	v,/ v,	INIT INIC	11/10	FilPe	P-M	P-IVI-V-1	
0.00	HSS6X6X1/4	Strong	1.2	19.2	1.6	0.7	45.5	25.7	21.2	18.7	2.6%	74.7%	7.3%	3.7%	78.4%	0.0%	~
0.00	None	Strong	1.2	19.Z	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	V
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	V
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	V
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	V
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	~
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	~
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	V

FOUNDATION DESIGN SUMMARY

MIND DATA

	Turna	Diameter Width Thickness Length Depth Volume Reinforcing		Palafordar	Status	Allowable Soil				
	Туре	ft	ft	ft	ft	ft	CY	Kennorcing	Status	Pressure
1	Caisson	3.00				6.25	1.64		ОК	300 psf/ft
Γ	Vertical Slab									
	Spread									

	MBI COMPANIES INC. 299 N. WEISGARBER RD.	PROJECT: 1370 Ainsworth Street, Colorado Springs, CO	DRAWN BY: FCM	CHECKED BY: DSA	сомм. NO. 190149.019	DATE:	04/
IVIDI	KNOXVILLE, TN 37919	DRAWING TITLE:				REV #	DAT
	PHONE 865.584.0999 SIGN-ENGINEER.COM	Meineke #1547					

H/60 3.42 in H/70

DECLECTION ANALVER

ual values are urposes only. The C_f value

•	
Lo	ads
ar	Moment
s	k-ft
2	2.0
0	17.6
0	0.0
0	0.0
0	0.0
0	0.0
0	0.0
0	0.0
0	0.0
0 0 2	0.0
2	19.6
2	19.2



NOTES

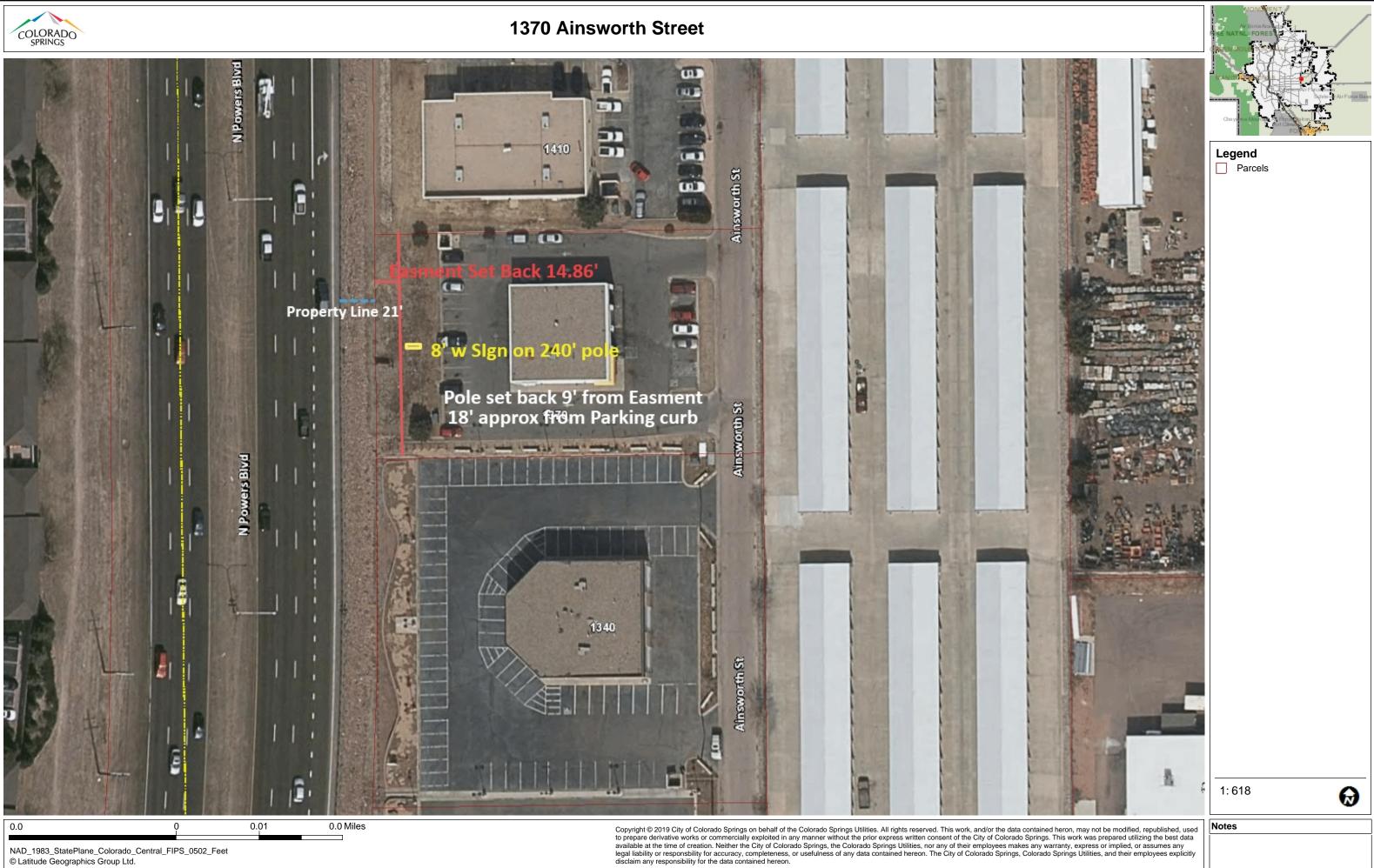
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NAD_1983_StatePlane_Colorado_Central_FIPS_0502_Feet © Latitude Geographics Group Ltd.