



EL PASO COUNTY PLANNING & COMMUNITY DEVELOPMENT
SIGN APPLICATION

Vicinity Map

Tax Schedule # _____
Please show major intersections.



Sign Plan

(Please indicate dimensions and sign copy)

Please find attached.

DATE: _____

Business

Name: Meineke #1547
Address: 1370 Ainsworth
Colorado Springs, CO

Zone: CC
Legal Description: _____

Type of Sign

Illuminated: Yes
Non-Illuminated: _____
Valuation: 8800

Storefront Length &/or
Width: _____

Proposed Sign Sq. Ft. 32

Existing Sign Sq. Ft. _____

Total Sign Sq. Ft. _____

Elevation Drawing

Indicate storefront length & placement of
sign.

N/A Sign is Freestanding

Contractor Information

Name: A Budget Signs

Address: 3224 N Nevada
Ave, Colorado Springs

Phone: 719 635 8811

Type of License: Sign

Contractor ID# 17040

For Planning & Community Development Use Only

Approved Date: _____ Resubmittal? Yes _____ No _____

Denied Date: _____

Comments: _____

Fee of \$262.00 must accompany this Application. Additional signs at time of first Application are \$125.00. Make checks payable to El Paso County.

POLE SIGN

MEINEKE #1547

1370 Ainsworth St
Colorado Springs, CO
80915



Underwriters Laboratories Inc. •
UL File #E225670

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changed or used in any way without
written consent.

EMBOSSED 1/2" DEEP WITH 2ND SURFACE
TRANSLUCENT AS SPECIFIED IN COLORS
"meineke" only

PROOF
**NOTE: ENGINEERING
REQUIREMENTS MAY
ALTER FOOTER,
POLE AND PRICE**

INITIAL LAYOUT:
January 25, 2019

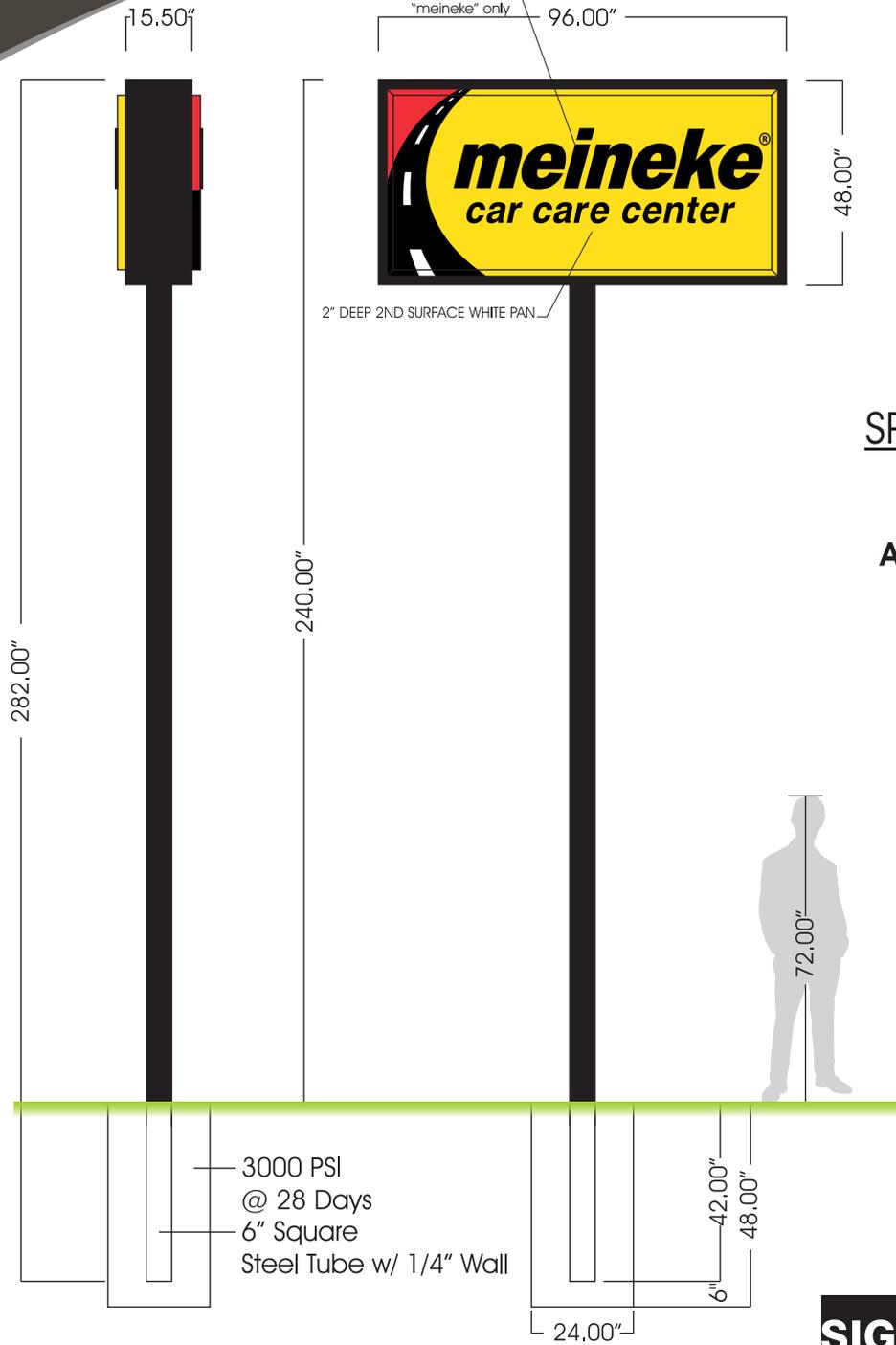
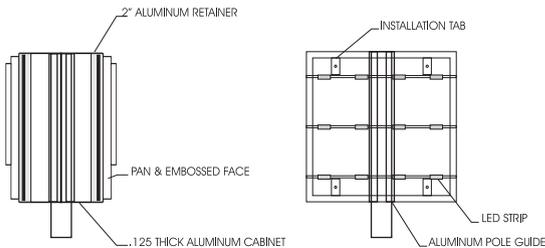
REVISIONS:
I
II
III
IV
V

Sq. Ft. Signage: 32.00
Signage Allowance: 40.00 Sq. Ft.

COLOR: PAINT AND VINYL

PAINT: (YELLOW) PMS 109 C VINYL: (YELLOW) 3M 3630-15	PAINT: (RED) PMS 185 C VINYL: (RED) 3M 3630-33
PAINT: (BLACK) PMS BLACK VINYL: (BLACK) 3M 3630-22	PAINT: (WHITE) PMS WHITE VINYL: (WHITE) 3M 3630-20

DETAIL



SPECIFICATIONS
DOUBLE-SIDED
ILLUMINATED
ALUMINUM CABINET

- Quantity: 1
- Size: As in drawing
- Face Color: As in drawing
- Vinyl Colors: As in drawing
- Face: Pan and Embossed (Top)
Flat (Bottom)
- Cabinet Interior: Gloss White
- Cabinet Color: Black
- Retainer: 2.0"
- Illumination: LED
- Mounting: Pole w/ concrete footer into earth

Owner/Landlord Approval _____



6801 Mount Hermon Church Rd, Ste C, Durham, NC 27705 • (P) 919-552-8689 • (F) 919-557-1322

SITE PLAN EXHIBIT



Underwriters Laboratories Inc. •
UL File #E225670

MEINEKE #1547

1370 Ainsworth St
Colorado Springs, CO
80915

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REVISIONS:
I -
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III -
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V -

PROPOSED MONUMENT LOCATION

NOTE:
10' SETBACK FROM PROPERTY LINE



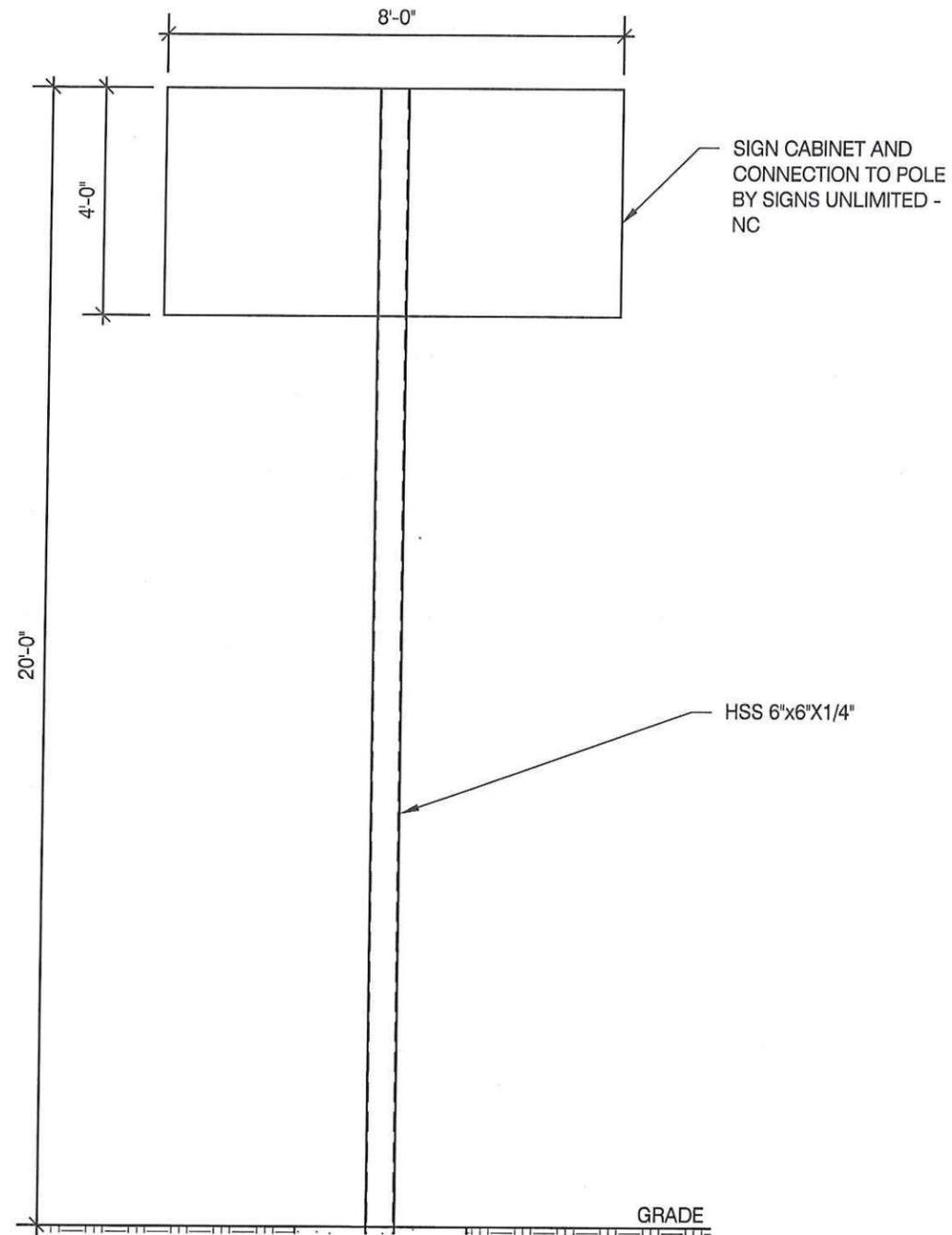
Owner/Landlord Approval _____



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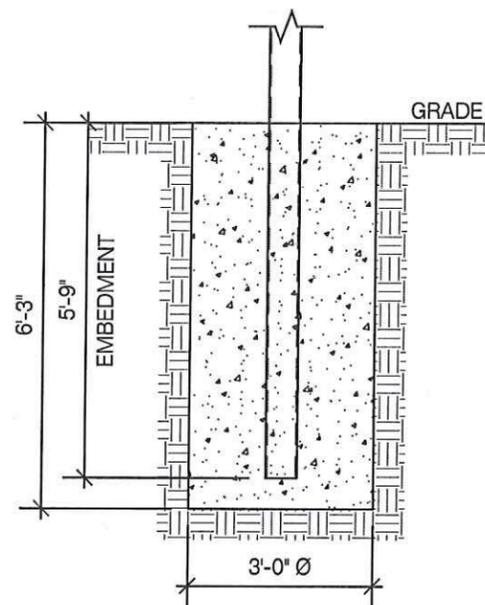
NOTES
 1.) SEE MANUFACTURERS DRAWINGS FOR ADDITIONAL DETAILS AND DIMENSIONS.
 2.) SIGN CABINET AND CONNECTION BY SIGNS UNLIMITED - NC

* CLIENT - SIGNS UNLIMITED - NC
 * PIKES PEAK RBC 2017 EDITION (2015 IBC)
 * 130 MPH WIND SPEED, EXP. C
 * (1) POLE, (1) FOOTING



FRONT ELEVATION
 SCALE: N.T.S.

1



CAISSON FOUNDATION
 SCALE: N.T.S.

2

MBI

MBI COMPANIES INC.
 299 N. WEISGARBER RD.
 KNOXVILLE, TN 37919
 PHONE 865.584.0999
 SIGN-ENGINEER.COM

PROJECT:
 1370 Ainsworth Street, Colorado Springs, CO

DRAWING TITLE:
Meineke #1547

DRAWN BY: FCM
 CHECKED BY: DSA
 COMM. NO. 190149.019

DATE: 04/16/19

REV #	DATE	DRAWN BY

DRAWING NO.
 DWG.
1



DESIGN SPECIFICATIONS:

REFER TO SIGN COMPANY'S DRAWINGS FOR MORE DETAILS.
ALL DESIGNS, DETAILING FABRICATION AND CONSTRUCTION SHALL CONFORM TO:

- PIKES PEAK RBC 2017 EDITION (2015 IBC)
- ACI
- AISC
- AMERICAN WELDING SOCIETY
- LOCAL BUILDING CODES & ORDINANCES

CONCRETE: 2500 PSI @ 28 DAYS
STD. STEEL PIPE SECTION: ASTM A53 GRADE B (Fy=35 KSI), U.N.O.
STEEL PIPE SECTION (> 20" Ø): ASTM A252 GRADE 3 (Fy=42 KSI MIN.) U.N.O.
HSS ROUND SECTION: ASTM A500 GRADE B (Fy=42 KSI) U.N.O.
HSS SQUARE/RECTANGULAR SECTION: ASTM A500 GRADE B (Fy=46 KSI)
W SHAPES: ASTM A992 (Fy = 50 KSI)
ANCHOR BOLTS: ASTM F1554 GRADE 36 U.N.O. (ALTERNATES GRADE 55 & 105)
CONNECTION BOLTS: ASTM A325
THREADED RODS: ASTM A193 GRADE B7
STEEL ANGLES, CHANNELS, STRUCTURAL SHAPES & PLATES ASTM A36
REINFORCING: GRADE 60 ASTM A615
PROVIDE A MINIMUM OF THREE INCHES OF CONCRETE COVER OVER EMBEDDED STEEL.

THE CONTRACTOR (INSTALLER) IS RESPONSIBLE FOR THE MEANS & METHODS OF CONSTRUCTION IN REGARDS TO JOBSITE SAFETY.
NO FIELD HEATING FOR BENDING OR CUTTING OF STEEL SHALL BE ALLOWED WITHOUT THE ENGINEER'S APPROVAL.

WELDING ELECTRODES: E70XX
ALLOWABLE SOIL BEARING PRESSURE ASSUMED: 2000 PSF
ASSUMED HORIZONTAL (PASSIVE PRESSURE) ASSUMED AT 150 PSF/FT OF DEPTH.
ISOLATED LATERAL BEARING FOUNDATIONS FOR SIGNS NOT ADVERSELY AFFECTED A 1/2" MOTION AT THE GROUND SURFACE DUE TO SHORT TERM LATERAL LOADS SHALL BE PERMITTED TO BE DESIGNED USING TWO TIMES THE TABULATED CODE VALUES.

ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED RESIDUAL SOIL AND/OR ENGINEERED EARTH.
FILL COMPACTED TO 98% OF ITS MAXIMUM DRY DENSITY AS PER ASTM D 698-70 (STANDARD PROCTOR) UNLESS NOTED OTHERWISE. THE SOIL BEARING CAPACITY IS TO BE VERIFIED BY A GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION. IF ALLOWABLE BEARING AND/OR LATERAL PRESSURE IS LESS THAN THE ABOVE ASSUMED AND/OR CALCULATED PRESSURES, THE ENGINEER SHOULD BE CONTACTED FOR RE-EVALUATION.

EXCAVATION SHALL BE FREE OF LOOSE SOIL BEFORE POURING CONCRETE.
WELDERS SHALL BE CERTIFIED FOR THE TYPE OF WELDING.
ADEQUATELY BRACE POLE(S) UNTIL CONCRETE HAS SET UP FOR 14 DAYS.
GROUT UNDER BASE PLATES WITH NON-SHRINK GROUT.

THIS ENGINEER DOES NOT WARRANT THE ACCURACY OF DIMENSIONS FURNISHED BY OTHERS.
ALL EXPOSED STEEL SHALL BE PAINTED WITH AN ENAMEL PAINT TO INHIBIT CORROSION.
THIS DESIGN IS FOR THE INDICATED ADDRESS ONLY, AND SHOULD NOT BE USED AT OTHER LOCATIONS WITHOUT WRITTEN PERMISSION OF THE ENGINEER.
DESIGN OF DETAILS AND STRUCTURAL MEMBERS NOT SHOWN, BY OTHERS.

WIND DATA

Building Code	Pikes Peak RBC 201	Importance Factor, I	1.0	Damping Ratio, β	0.005
Wind Load Criteria	ASCE 7-10	Directionality Factor, K_d	0.85	Natural Frequency, n_1	1.59 Hz
Wind Speed, V	130 mph	Topography Factor, K_{zt}	1.0	Gust Effect Factor, G	0.85
Exposure Category	C	Base Pressure, $\gamma(q/K_z)$	22.1 psf	ASD Wind Load Factor, γ	0.6
Wind Pressure Override per Jurisdiction Requirement	0 psf				

DEFLECTION ANALYSIS

Deflection Limit	H/60
Deflection at 0.7*W	3.42 in
Deflection Ratio	H/70

Notes: (1) Loading values in chart below are based upon average K_z values for each segment. Actual values are calculated on hidden sheet using derived V-M equations. Chart is provided for information purposes only.
(2) Wind directionality (K_d) factor is 0.95 for Single Pole (Round) segments instead of 0.85. The C_f value from Fig. 6-21 has been increased by 0.95/0.85 to account for this variation.
(3) Wind pressures listed below have already been multiplied by the ASD Wind Load Factor, γ .

GEOMETRY INPUT

Section	Location	Type	Height		Horiz. Offset	Area	Top Elev.	Centroid	K_z	C_f	Wind Press.	Support Pole Loads			Footing Loads		
			ft	ft								Trib. Factor	Shear kips	Moment k-ft	Trib. Factor	Shear kips	Moment k-ft
1	Base	Single Pole (Not Round)	16.00	0.50		8.0	16.0	8.0	0.86	1.89	30.6	1.0	0.2	2.0	1.0	0.2	2.0
2		Single Pole w/ Cabinet	4.00	8.00		32.0	20.0	18.0	0.90	1.81	30.6	1.0	1.0	17.6	1.0	1.0	17.6
3		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
4		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
5		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
6		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
7		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
8		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
9		None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
10	Top	None				0.0	20.0	20.0	0.90	1.46	24.7	0.0	0.0	0.0	0.0	0.0	0.0
Overall Height: 20.00 ft											Summation based upon averages above:			Summation based upon averages above:			
											1.2			19.6			
											1.2			19.2			

SUPPORT POLE DESIGN SUMMARY

Base Elev	Section	Axis	Required Strength Values (ASD)				Allowable Strength Values (ASD)				Unity Ratios				Interaction Ratios		Status
			V_r	M_r	T_r	P_r	V_c	M_c	T_c	P_c	V_r/V_c	M_r/M_c	T_r/T_c	P_r/P_c	P-M	P-M-V-T	
0.00	HSS6X6X1/4	Strong	1.2	19.2	1.6	0.7	45.5	25.7	21.2	18.7	2.6%	74.7%	7.3%	3.7%	78.4%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓
0.00	None	Strong	1.2	19.2	1.6	0.7	0.0	0.0	0.0	0.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	✓

FOUNDATION DESIGN SUMMARY

Type	Diameter	Width	Thickness	Length	Depth	Volume	Reinforcing	Status	Allowable Soil Pressure
✓ Caisson	3.00	--	--	--	6.25	1.64		OK	300 psf/ft
Vertical Slab									
Spread									

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