## COLORADO KIDS RANCH

### SPECIAL USE TRAFFIC IMPACT STUDY

El Paso County, Colorado

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Completed By:

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## **CERTIFICATION PAGE**

18065 Saddlewood Road Monument, CO 80132

<u>Engineer's Statement</u>	
The attached traffic report and supporting responsible charge and they comport wit consistent with the standard of care, said conformance with the criteria established	h the standard of care. So far as is I report was prepared in general
Brett Louk, PE #0055474	Date:
Developer's Statement	
I, the Developer, have read and will complete behalf within this report.	oly with all commitments made on my
Colorado Pumpkin Patch LLC	Date:

#### **EXECUTIVE SUMMARY**

SMH Consultants, P.A. completed a traffic impact study, to support the special use application, for the Colorado Kids Ranch located at 18065 Saddlewood Road in El Paso County, Colorado. The Colorado Kids Ranch holds two main events annually: (i) the Fall Festival is held during the last two weeks of September through November 7; and (ii) the Tulip Festival is held during two weekends in May (collectively, the "Events").

Attendance information, from last year's events, was utilized to determine the number of trips generated by the Fall Festival and Tulip Festival. Trip generation for uses that fall within the Agritainment use were not included in the analysis presented in this report. These generated trips, along with the peak hour counts, were then used to perform a level of service (LOS) analysis for the following intersections: Saddlewood Rd./Canterbury Dr.; Canterbury Dr./Hwy. 105; Cherry Springs Ranch/Hwy. 105; and Appaloosa Rd./Hwy. 105. Additional offsite major intersections such as Hwy. 105/Hwy. 83 and Roller Coaster Rd./Hwy. 105 were not included in the analysis, because the site generated trips did not contribute a 10 percent impact during the A.M. or P.M. peak hour to any approach leg of the intersection that is operating at LOS C or better; or a 5 percent impact during the A.M. or P.M. peak hour to any approach leg of the intersection that is operating at LOS D or worse. Additional offsite minor intersections such as Sahara Rd./Roller Coaster Rd. were not included in the analysis, because the site generated trips did not contribute a 30 percent increase in volume during the A.M. or P.M. peak hour to any approach leg of the intersection that is operating at LOS E or worse. The LOS analysis for all intersections indicated that the LOS for all approaches to the intersection would continue to operate at LOS C or better, for the A.M. and P.M. peak hours of the events, with the lone exception being the NB approach at the intersection of Canterbury Dr./Hwy.105 which will operate at LOS D in the weekend P.M. peak hour under the long-range horizon scenario. This remains true for the existing plus development, short-range horizon, and long-range horizon scenarios. It should be noted that the intersection LOS analysis, for each intersection, was completed with the existing lane configuration and no auxiliary lanes were accounted for.

Auxiliary lane analyses showed that a westbound left turn lane and eastbound right turn lane are warranted at the intersection of Canterbury Dr. and Highway 105 during the weekday A.M. and P.M. peak hours. A southbound left turn lane is warranted at the intersection of Canterbury Dr. and Saddlewood Rd. during the weekday and weekend A.M. and P.M. peak hours. No right turn acceleration lanes are warranted at any of the study intersections.

Based on the analysis presented in this report, the Colorado Kids Ranch generated traffic will require some roadway improvements to negate the impacts to the surrounding roadway network.

#### **INTRODUCTION**

The ownership of the Colorado Kids Ranch has requested that SMH Consultants, P.A. conduct a Traffic Impact Study (TIS) to support a special use application for the Colorado Kids Ranch. The tulip festival is planned to take place over two weekends in May and will operate from 9 A.M. to 5 P.M. on Friday, Saturday, and Sunday. The fall festival is planned to take place the last two weeks of September through November 7th and will be open Tuesdays from 9 A.M. to 1 P.M. and Wednesday through Sunday from 9 A.M. to 5 P.M. The purpose of this study is to determine the traffic impacts of the Colorado Kids Ranch on the surrounding transportation network. A vicinity map has been included in the appendix of this report.

This TIS will determine the trips generated by the Colorado Kids Ranch, perform a level of service (LOS) analysis for the following intersections: Saddlewood Rd./Canterbury Dr.; Canterbury Dr./Hwy. 105; Cherry Springs Ranch/Hwy 105; and Appaloosa Rd./Hwy. 105, perform auxiliary lane analyses at each intersection, and identify any improvements that may be required to the surrounding transportation network.

### **METHODOLOGY**

On December 17<sup>th</sup> and 21<sup>st</sup> 2022, SMH Consultants conducted weekday and weekend A.M. and P.M. peak hour turning movement counts at the intersections of Saddlewood Rd./Canterbury Dr. and Canterbury Dr./Hwy. 105. Upon further discussion with El Paso County staff, further intersections were identified that would need analyzed. On March 14<sup>th</sup> and 18<sup>th</sup> 2023, SMH Consultants conducted weekday and weekend A.M. and P.M. peak hour turning movement counts at the intersections of Appaloosa Rd./Hwy. 105 and Cherry Springs Ranch/Hwy. 105. All turning movement count information has been included in the appendix of this report.

McTrans HCS7 Software was used to analyze the existing, existing plus development, short-range horizon, and long-range horizon scenarios for all aforementioned intersections.

Intersection Level of Service (LOS) is a concept defined by the *Highway Capacity Manual* (HCM) to qualitatively describe operating conditions within a traffic stream. LOS is typically stratified into six categories (A through F). These range from LOS A indicating free-flow, low density, or nearly negligible delay conditions to LOS F where demand exceeds capacity and large queues are experienced.

For unsignalized intersections, the HCM uses control delay, measured in average seconds of delay per vehicle, as the basis for determining LOS. Control delay at an intersection is the average stopped time per vehicle traveling through the intersection plus the movements at slower speeds due to the vehicles moving

up in the queue or slowing upstream of the approach. For two-way stop-controlled intersections, individual approach delays as well as an overall average delay are calculated for each intersection. Table 1 below shows the LOS criteria for an unsignalized intersection.

Level of Service	Control Delay Range (s/veh)
	Unsignalized
Α	0-10
В	>10-15
С	>15-25
D	>25-35
E	>35-50
F	>50

Table 1: LOS Criteria

#### **EXISTING CONDITIONS**

The existing site is located at 18065 Saddlewood Road in El Paso County, Colorado. The existing site consists of a single-family home, outbuildings, and agricultural ground. The site is bordered by Saddlewood Rd. to the south, residential properties to the east and west, and Highway 105 to the north. Canterbury Dr. is located west of the site and Appaloosa Rd. is located east of the site.

Saddlewood Rd. is a two-lane gravel road that is approximately 28-feet wide, with roadside ditches on both sides, and is classified as a rural local road. At the intersection with Canterbury Dr., westbound Saddlewood Rd. consists of a shared right/left lane. There is no eastbound approach to the intersection of Saddlewood Rd. and Canterbury Dr. Saddlewood Rd. does not have a posted speed limit, however, based on the posted speed limit for Canterbury Dr. and Appaloosa Rd., the speed limit is assumed to be 25 mph. In the southbound direction along Canterbury Dr., there is a measured intersection sight distance of 378', compared to the ECM minimum intersection sight distance of 165' and minimum stopping sight distance of 280'. In the northbound direction along Canterbury Dr., there is a measured intersection sight distance of 406', compared to the ECM minimum intersection sight distance of 165' and minimum stopping sight distance of 280'. Intersection sight distance at this intersection is adequate in both directions. An intersection sight distance exhibit has been included in the appendix of this report.

Canterbury Dr. is a two-lane paved road that is approximately 28-feet wide, with roadside ditches on both sides, and is classified as a rural local road. At the intersection with Saddlewood Rd., northbound Canterbury Dr. consists of a shared thru/right lane and southbound consists of a shared thru/left lane. At the intersection with Highway 105, northbound Canterbury Dr. consists of a shared

right/left lane. There is no southbound approach to the intersection of Canterbury Dr. and Highway 105. Canterbury Dr. has a posted speed limit of 25 mph. In the eastbound direction along Highway 105, there is a measured intersection sight distance of 1100', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 638'. In the westbound direction along Highway 105, there is a measured intersection sight distance of 980', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 515'. Intersection sight distance at this intersection is adequate in both directions. An intersection sight distance exhibit has been included in the appendix of this report.

Cherry Springs Ranch Dr. is a two-lane paved road that is approximately 28-feet wide, with roadside ditches on both sides, and is classified as a rural local road. At the intersection with Highway 105, southbound Cherry Springs Ranch Dr. consists of a shared left/right lane. There is no northbound approach to the intersection of Cherry Springs Ranch Dr. and Highway 105. Cherry Springs Ranch Dr. has a posted speed limit of 30 mph. In the eastbound direction along Highway 105, there is a measured intersection sight distance of 1026', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 570'. In the westbound direction along Highway 105, there is a measured intersection sight distance of 1140', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 570'. Intersection sight distance at this intersection is adequate in both directions. An intersection sight distance exhibit has been included in the appendix of this report.

Appaloosa Rd. is a two-lane gravel road that is approximately 28-feet wide, with roadside ditches on both sides, and is classified as a rural local road. At the intersection with Highway 105, northbound Appaloosa Rd. consists of a shared left/right lane. There is no southbound approach to the intersection of Appaloosa Rd. and Highway 105. Appaloosa Rd. has a posted speed limit of 25 mph. In the eastbound direction along Highway 105, there is a measured intersection sight distance of 4000', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 515'. In the westbound direction along Highway 105, there is a measured intersection sight distance of 843', compared to the ECM minimum intersection sight distance of 555' and minimum stopping sight distance of 638'. Intersection sight distance at this intersection is adequate in both directions. An intersection sight distance exhibit has been included in the appendix of this report.

Highway 105 is a two-lane paved road that is approximately 24-feet wide, with roadside ditches on both sides, and is classified as a principal arterial. Currently, Highway 105 is undergoing a corridor analysis, from Interstate 25 to Highway 83, to determine improvements that may be necessary along this stretch of Highway 105. At this time, the section of Highway 105, adjacent to the Colorado Kids Ranch, is recommended to be a three-lane section with one thru lane in each

direction and center left turn lanes at major intersections. It should be noted that the intersections studied in this report for the Colorado Kids Ranch were not studied as part of the El Paso County Highway 105 Traffic Study Update, performed by HDR, as they were considered side roads with insignificant traffic volumes. At the intersection with Canterbury Dr., eastbound Highway 105 consists of a shared thru/right lane and westbound consists of a shared thru/left lane. At the intersection with Cherry Springs Ranch Dr., eastbound Highway 105 consists of a shared thru/left lane and westbound consists of a shared thru/right lane. At the intersection with Appaloosa Rd., eastbound Highway 105 consists of a shared thru/right lane and westbound consists of a shared thru/left lane. Highway 105 has a posted speed limit of 50 mph.

Per the MTCP, Highway 105 from Knollwood Boulevard to Highway 83 is identified as a 2040 roadway improvement project. This project would expand Highway 105 from a two-lane principal arterial to a four-lane principal arterial. In addition, there is a current corridor analysis being completed for Highway 105, from Interstate 25 to Highway 83. However, this corridor analysis for the section of Highway 105 adjacent to the Colorado Kids Ranch has not been completed, and thus, has not recommended any roadway improvements for Highway 105 in this area.

Based on information provided in the El Paso County Highway 105 Traffic Study Update, performed by HDR, Highway 105 has peak hours of 7 A.M. to 8 A.M. and 5 P.M. to 6 P.M. However, these peak hours are different than the A.M. and P.M. peak hours for the Colorado Kids Ranch. Based on information provided by the owner, from last year's festivals, the A.M. peak hour is 9 A.M. to 10 A.M. and the P.M. peak hour is 1 P.M. to 2 P.M. Therefore, SMH Consultants performed existing turning movement counts for the timeframes coinciding with the peak hours of the Colorado Kids Ranch festivals. Existing peak hour counts and turning movements can be seen in the appendix of this report.

Table 2 shows the existing weekday level of service for each intersection. Detailed intersection level of service calculations for each intersection are provided in the appendix.

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		Weekda	ay A.M.	Peak Hour	Weekda	ay P.M. Pe	eak Hour
Intersection	Movement			95%			95%
		Delay (s)	LOS	Queue	Delay (s)	LOS	Queue
Canterbury & Saddlewood	WB RT/LT	8.4	Α	0'	8.5	Α	0'
	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
	SB Thru/RT	7.2	Α	0'	7.2	Α	0'
	EB Thru/RT	7.6	Α	0'	7.5	А	0'
Canterbury & Hwy 105	WB Thru/LT	7.5	Α	0'	7.7	Α	0'
11Wy 103	NB RT/LT	9.9	Α	0'	11	В	0'
	EB Thru/RT	7.6	Α	0'	7.5	Α	0'
Appaloosa &	WB Thru/LT	7.5	Α	0'	7.5	Α	0'
Hwy 105	NB RT/LT	9.9	Α	0'	9.8	Α	0'
	EB Thru/LT	7.6	Α	0'	7.5	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.5	Α	0'	7.5	Α	0'
Q IIWy 103	SB RT/LT	9.9	Α	0'	10.3	В	0'

Table 2: Existing Intersection Weekday LOS

Table 3 shows the existing weekend level of service for each intersection.

		Weeker	nd A.M.	Peak Hour	Weeker	nd P.M. Po	eak Hour
Intersection	Movement			95%			95%
		Delay (s)	LOS	Queue	Delay (s)	LOS	Queue
Canterbury & Saddlewood	WB RT/LT	8.3	Α	0'	8.4	Α	0'
	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
	SB Thru/RT	7.2	Α	0'	7.2	Α	0'
	EB Thru/RT	7.5	Α	0'	7.6	Α	0'
Canterbury & Hwy 105	WB Thru/LT	7.5	Α	0'	7.6	Α	0'
1100 103	NB RT/LT	10.4	В	0'	10.6	В	2.5'
	EB Thru/RT	7.6	Α	0'	7.5	Α	0'
Appaloosa & Hwy 105	WB Thru/LT	7.5	Α	0'	7.6	Α	0'
HWY 103	NB RT/LT	10.3	В	0'	9.2	Α	0'
Charma Carriaga	EB Thru/LT	7.6	Α	0'	7.5	Α	0′
Cherry Springs & Hwy 105	WB Thru/RT	7.4	Α	0'	7.6	Α	0′
Q 11Wy 103	SB RT/LT	9.8	Α	0'	9.5	Α	0'

**Table 3: Existing Intersection Weekend LOS** 

### PROJECT DESCRIPTION

The tulip festival event will be held two weekends in May and will operate from 9 A.M. to 5 P.M. Friday, Saturday, and Sunday. The fall festival will be held the last two weeks of September through November 7th and will be open Tuesdays from 9 A.M. to 1 P.M. and Wednesday through Sunday from 9 A.M. to 5 P.M. Weekday and Weekend access to the Colorado Kids Ranch will be via

Saddlewood Rd. Vehicles will access Saddlewood Rd., from Highway 105, via Canterbury Dr. and Appaloosa Rd.

### **TRIP GENERATION**

The Institute of Transportation Engineers (ITE), *Trip Generation Report, 11<sup>th</sup> Edition*, does not provide trip generation data for events due to the intermittent use of event venues and wide variability in event attendance. The owner provided SMH Consultants attendance information from last year's festivals. SMH then used this information to determine the A.M. and P.M. peak hour trips for the Colorado Kids Ranch. A ratio of one vehicle for every two persons was assumed. The A.M. peak hour was broken into 90% entering and 10% exiting, the P.M. peak hour was broken into 70% entering and 30% exiting, and the daily trips were broken into 50% entering and 50% exiting. Table 4 shows the weekday daily, A.M. peak hour, and P.M. peak hour trips for the Colorado Kids Ranch.

	Attendance (ppl)	Daily			A.M. PH			P.M. PH		
		Total	In	Out	Total	In	Out	Total	In	Out
Max. Weekday	908	454	227	227						
A.M. Peak Hr	298				149	134	15			
P.M. Peak Hr	124							62	43	19

**Table 4: Weekday Generated Trips** 

Table 5 shows the weekend daily, A.M. peak hour, and P.M. peak hour trips for the Colorado Kids Ranch. The same entering and exiting percentages, as stated above for the weekday trips, were applied for the weekend trips.

	Attendance (ppl)	Daily		A.M. PH			P.M. PH			
		Total	In	Out	Total	In	Out	Total	In	Out
Max. Weekend	3356	1678	839	839						
A.M. Peak Hr	346				173	156	17			
P.M. Peak Hr	472							236	165	71

**Table 5: Weekend Generated Trips** 

#### TRIP DISTRIBUTION

There are many ways to distribute trips that are entering or exiting a proposed development. For the purposes of this study, SMH assumed that 60% of the site generated traffic would be coming from, or going to, the west and 40% would be coming from, or going to, the east. This directional distribution was largely based on Interstate 25 being located west of the site and Highway 83 being located east of the site.

From this initial directional distribution, the peak hour trips were further distributed amongst the intersections of Canterbury Dr./Hwy. 105 and Appaloosa Rd./Hwy. 105. For the trips coming from, or going to, the west, it was assumed that 95% of those trips would utilize the Canterbury Dr. and Highway 105 intersection and 5% would utilize the Appaloosa Rd. and Highway 105 intersection. For the trips coming from, or going to, the east, it was assumed that 95% of those trips would utilize the Canterbury Dr. and Highway 105 intersection and 5% would utilize the Appaloosa Rd. and Highway 105 intersection. Canterbury Dr. is a paved road, whereas Appaloosa Rd. is not, and the preferred route for mobile map services that visitors might use to get to the festival. Thus, the intersection of Canterbury Dr. and Highway 105 received a larger distribution of the festival generated trips.

An exhibit has been included in the appendix of this report showing the distribution of the site generated trips.

#### **EXISTING PLUS DEVELOPMENT**

Existing plus development conditions combine the existing A.M. and P.M. peak hour turning movements with the A.M. and P.M. peak hour traffic generated by the site. The existing plus development peak hour turning movements can be seen in the appendix of this report. Detailed intersection level of service calculations for each intersection are provided in the appendix. Table 6 shows the existing plus development weekday level of service for all the study intersections.

		Weekda	ay A.M.	Peak Hour	Weekda	ay P.M. Pe	eak Hour
Intersection	Movement			95%			95%
		Delay (s)	LOS	Queue	Delay (s)	LOS	Queue
Cantarhum	WB RT/LT	8.6	Α	2.5'	8.5	Α	2.5′
Canterbury & Saddlewood	NB Thru/RT	7.2	Α	0′	7.2	Α	0'
Saddlewood	SB Thru/RT	7.4	Α	7.5'	7.3	Α	2.5'
Carlada a	EB Thru/RT	7.6	Α	0'	7.5	Α	0'
Canterbury & Hwy 105	WB Thru/LT	7.8	Α	2.5′	7.8	Α	0'
11Wy 103	NB RT/LT	11.4	В	2.5′	11.4	В	5'
	EB Thru/RT	7.8	Α	0'	7.6	Α	0'
Appaloosa & Hwy 105	WB Thru/LT	7.5	Α	0'	7.5	Α	0'
HWY 103	NB RT/LT	10.3	В	0'	9.9	Α	0'
	EB Thru/LT	7.8	Α	0'	7.6	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.5	Α	0'	7.5	Α	0'
Q 11W y 103	SB RT/LT	10.4	В	0'	10.4	В	0′

Table 6: Existing + Development Weekday Intersection LOS

Table 7 shows the existing plus development weekend level of service for all the study intersections.

		Week	end A.N	1. Peak Hour	Weeker	nd P.M. Po	eak Hour
Intersection	Movement	Delay		95%			95%
		(s)	LOS	Queue	Delay (s)	LOS	Queue
Canterbury & Saddlewood	WB RT/LT	8.4	Α	2.5′	8.6	Α	5′
	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
	SB Thru/RT	7.5	Α	10'	7.5	Α	10'
	EB Thru/RT	7.5	Α	0'	7.6	Α	0'
Canterbury & Hwy 105	WB Thru/LT	7.9	Α	5'	8.1	Α	5′
11WY 103	NB RT/LT	11.9	В	2.5'	13.8	В	17.5′
	EB Thru/RT	7.7	Α	0'	7.7	Α	0'
Appaloosa &	WB Thru/LT	7.5	Α	0'	7.7	Α	0'
Hwy 105	NB RT/LT	10.7	В	0'	10.7	В	0'
	EB Thru/LT	7.8	Α	0'	7.6	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.5	Α	0'	7.7	А	0'
& HWY 105	SB RT/LT	10.3	В	0'	10.1	В	0'

Table 7: Existing + Development Weekend Intersection LOS

### **SHORT-RANGE HORIZON ANALYSIS**

El Paso County requires a short-range horizon analysis as part of the traffic impact study for projects. The short-range horizon analysis is intended to analyze the immediate impacts of the proposed project on the existing and committed roadway network. The short-range horizon year is defined as one year after the full occupancy of the project. To determine the projected peak hour trips for the short-range horizon year, a 2% annual growth rate was applied to the existing peak hour turning movement counts. This growth rate is consistent with the growth rate utilized in the Highway 105 study. The short-range peak hour turning movements can be seen in the appendix of this report. Detailed intersection level of service calculations for each intersection are provided in the appendix.

Table 8 shows the short-range horizon weekday level of service for all the study intersections.

		Weekda	ay A.M.	Peak Hour	Weekda	ay P.M. Pe	eak Hour
Intersection	Movement			95%			95%
		Delay (s)	LOS	Queue	Delay (s)	LOS	Queue
Contain 0	WB RT/LT	8.6	Α	2.5'	8.5	Α	2.5'
Canterbury & Saddlewood	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
	SB Thru/RT	7.4	Α	7.5′	7.3	Α	2.5'
	EB Thru/RT	7.6	Α	0'	7.5	Α	0'
Canterbury & Hwy 105	WB Thru/LT	7.8	Α	2.5'	7.8	Α	0'
HWY 103	NB RT/LT	11.4	В	2.5'	11.5	В	5'
	EB Thru/RT	7.8	Α	0'	7.6	Α	0'
Appaloosa &	WB Thru/LT	7.5	Α	0'	7.6	Α	0'
Hwy 105	NB RT/LT	10.3	В	0'	10.0	Α	0'
	EB Thru/LT	7.8	Α	0'	7.6	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.5	Α	0'	7.5	Α	0'
	SB RT/LT	10.4	В	0'	10.5	В	0'

Table 8: Short-Range Horizon Weekday Intersection LOS

Table 9 shows the short-range horizon weekend level of service for all the study intersections.

		Weeke	end A.N	1. Peak Hour	Weeker	nd P.M. Po	eak Hour
Intersection	Movement	Delay		95%			95%
		(s)	LOS	Queue	Delay (s)	LOS	Queue
Comto who was 0	WB RT/LT	8.4	Α	2.5'	8.6	Α	5′
Canterbury & Saddlewood	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
Saddlewood	SB Thru/RT	7.5	Α	10'	7.5	Α	10'
Control o	EB Thru/RT	7.5	Α	0'	7.6	Α	0'
Canterbury & Hwy 105	WB Thru/LT	7.9	Α	5'	8.1	Α	5′
11Wy 103	NB RT/LT	12.0	В	5'	14.0	В	17.5'
	EB Thru/RT	7.8	Α	0'	7.7	Α	0'
Appaloosa &	WB Thru/LT	7.5	Α	0'	7.7	Α	0'
Hwy 105	NB RT/LT	10.8	В	0'	10.7	В	0'
GI G :	EB Thru/LT	7.8	Α	0'	7.7	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.5	Α	0'	7.7	Α	0'
Q IIWy 103	SB RT/LT	10.4	В	0'	10.2	В	0'

Table 9: Short-Range Horizon Weekend Intersection LOS

#### LONG-RANGE HORIZON ANALYSIS

El Paso County requires a long-range horizon analysis as part of the traffic impact study for projects. The long-range horizon analysis is intended to analyze the impacts of the proposed project on the long-range traffic condition and is based on the current Master Transportation Corridor Plan planning horizon and related modeling. The long-range peak hour turning movements can be seen in the appendix of this report. Detailed intersection level of service calculations for each intersection are provided in the appendix.

Per the ECM, long-range volume projection may be found by using a straight-line projection from the build-out year between existing traffic volumes and the MTCP Model forecast. Map 9 of the 2016 MTCP update was used to find the 2016 and 2040 traffic volumes and a yearly constant growth rate of 4.4% was calculated and utilized for the long-range horizon analysis.

Table 10 shows the long-range horizon weekday level of service for all the study intersections.

		Weekda	ay A.M.	Peak Hour	Weekda	ay P.M. Pe	eak Hour
Intersection	Movement			95%			95%
		Delay (s)	LOS	Queue	Delay (s)	LOS	Queue
Carata da 0	WB RT/LT	9.1	Α	2.5′	8.7	Α	2.5'
Canterbury & Saddlewood	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
	SB Thru/RT	7.5	Α	7.5′	7.3	Α	2.5'
	EB Thru/RT	8.0	Α	0'	8.0	Α	0'
Canterbury & Hwy 105	WB Thru/LT	8.3	Α	5'	8.5	Α	2.5'
11Wy 103	NB RT/LT	15.2	С	5'	17.8	С	10'
	EB Thru/RT	8.4	Α	0'	8.0	Α	0'
Appaloosa & Hwy 105	WB Thru/LT	7.8	Α	0'	7.9	Α	0'
HWY 103	NB RT/LT	13.4	В	2.5′	12.6	В	2.5'
	EB Thru/LT	8.4	Α	0'	8.0	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.8	Α	0'	7.9	А	0'
Q 11W y 103	SB RT/LT	13.5	В	2.5′	14.0	В	2.5'

Table 10: Long-Range Horizon Weekday Intersection LOS

Table 11 shows the long-range horizon weekend level of service for all the study intersections.

		Weekend A.M. Peak Hour			Weekend P.M. Peak Hour		
Intersection	Movement	Delay		95%			95%
		(s)	LOS	Queue	Delay (s)	LOS	Queue
Cambaulaum (	WB RT/LT	8.4	Α	2.5′	8.7	Α	7.5'
Canterbury & Saddlewood	NB Thru/RT	7.2	Α	0'	7.2	Α	0'
Saudiewood	SB Thru/RT	7.5	Α	10'	7.6	Α	10'
	EB Thru/RT	7.8	Α	0'	8.0	Α	0'
Canterbury & Hwy 105	WB Thru/LT	8.3	Α	5'	8.7	Α	5′
11Wy 103	NB RT/LT	17.1	С	10'	25.7	D	42.5'
	EB Thru/RT	8.3	Α	0′	8.0	Α	0'
Appaloosa &	WB Thru/LT	7.8	Α	0'	8.2	Α	0'
Hwy 105	NB RT/LT	14.6	В	2.5′	13.4	В	0'
	EB Thru/LT	8.3	Α	0'	8.0	Α	0'
Cherry Springs & Hwy 105	WB Thru/RT	7.7	Α	0'	8.2	Α	0'
α πwy 105	SB RT/LT	13.2	В	5'	12.5	В	0'

Table 11: Long-Range Horizon Weekend Intersection LOS

### **AUXILIARY LANE ANALYSIS**

Per the Engineering Criteria Manual (ECM), an exclusive left turn lane is required on a principal arterial, at an access, when the projected peak hour left ingress turning volume is 10 vph or greater. Table 12 shows the project generated left turn movements at each intersection versus the ECM criteria.

Intersection	Turn Movement	Peak Hour	Projected Left Turns (vph)	ECM Criteria (vph)	Warranted
		Weekday AM	51	10	Yes
Canterbury &	WB LT	Weekday PM	18	10	Yes
Hwy 105		Weekend AM	60	10	Yes
		Weekend PM	63	10	Yes
		Weekday AM	4	10	No
Appaloosa &	WB LT	Weekday PM	6	10	No
Hwy 105		Weekend AM	4	10	No
		Weekend PM	6	10	No
		Weekday AM	129	25	Yes
Canterbury &	SB LT	Weekday PM	43	25	Yes
Saddlewood		Weekend AM	150	25	Yes
		Weekend PM	157	25	Yes
	NB LT	Weekday AM	11	25	No
Canterbury &		Weekday PM	17	25	No
Hwy 105		Weekend AM	16	25	No
		Weekend PM	48	25	Yes
	osa & NB LT	Weekday AM	4	25	No
Appaloosa &		Weekday PM	4	25	No
Hwy 105		Weekend AM	5	25	No
		Weekend PM	2	25	No
		Weekday AM	1	25	No
Canterbury &	WB LT	Weekday PM	3	25	No
Saddlewood		Weekend AM Weekend	0	25	No
			0	25	No

Table 12: Left Turn Lane Analysis

As can be seen from Table 12, an exclusive left turn lane is warranted at the intersection of Canterbury Dr. and Highway 105 on westbound Highway 105 for the weekday and weekend A.M. and P.M. peak hours and at the intersection of Canterbury Dr. and Saddlewood Rd. on southbound Canterbury Dr. for the weekday and weekend A.M. and P.M. peak hours. An additional exclusive left turn lane is warranted at the intersection of Canterbury Dr. and Highway 105 on northbound Canterbury Dr. for the weekend PM peak hour.

Per the ECM, the left turn auxiliary lane on westbound Highway 105 at Highway 105 and Canterbury Dr. is recommended to have a 540' redirect taper, 160' bay taper, 188' deceleration lane (lane length) and 100' of storage. In the current condition, Highway 105 would not be wide enough to meet the auxiliary lane criteria and will need to be widened. Calculations for the auxiliary lane were made using a 6% upgrade and assuming 12' widening on the north side of the highway.

Per the ECM, the left turn auxiliary lane on southbound Canterbury Dr. at Canterbury Dr. and Saddlewood Rd. is recommended to have a 90' redirect taper, 144' bay taper, 138' deceleration lane (lane length), and 150' of storage. In the current condition, Canterbury Dr. would not be wide enough to meet the auxiliary lane criteria and will need to be widened. Calculations for the auxiliary lane were made using a 4% downgrade and assuming 6' widening on each side of the road.

Per the ECM, the left turn auxiliary lane on northbound Canterbury Dr. at Canterbury Dr. and Highway 105 is recommended to have a 90' redirect taper, 108' bay taper, 104' deceleration lane (lane length), and 50' of storage. In the current condition, Canterbury Dr. would not be wide enough to meet the auxiliary lane criteria and will need to be widened. Calculations for the auxiliary lane were made using a 4% upgrade and assuming 6' widening on each side of the road.

Per the ECM, an exclusive right turn lane is required on a principal arterial, at an access, when the projected peak hour right ingress turning volume is 25 vph or greater. Table 13 shows the project generated right turn movements at each intersection versus the ECM criteria.

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	Turn		Projected	ECM	
Intersection	Movement	Peak Hour	Right Turns (vph)	Criteria (vph)	Warranted
		Weekday AM	78	25	Yes
Canterbury &	EB RT	Weekday PM	32	25	Yes
Hwy 105		Weekend AM	90	25	Yes
		Weekend PM	97	25	Yes
		Weekday AM	6	25	No
Appaloosa &	EB RT	Weekday PM	3	25	No
Hwy 105		Weekend AM	5	25	No
		Weekend PM	9	25	No
		Weekday AM	0	50	No
Canterbury &	NB RT	Weekday PM	3	50	No
Saddlewood		Weekend AM	1	50	No
		Weekend PM	1	50	No
	NB RT	Weekday AM	8	50	No
Canterbury &		Weekday PM	9	50	No
Hwy 105		Weekend AM	6	50	No
		Weekend PM	30	50	No
		Weekday AM	4	50	No
Appaloosa &	NB RT	Weekday PM	5	50	No
Hwy 105		Weekend AM	2	50	No
		Weekend PM	2	50	No
		Weekday AM	14	50	No
Canterbury &	WB RT	Weekday PM	22	50	No
Saddlewood		Weekend AM	18	50	No
		Weekend PM	69	50	Yes

Table 13: Right Turn Lane Analysis

As can be seen from Table 13, an exclusive right turn lane is warranted at the intersection of Canterbury Dr. and Highway 105 on eastbound Highway 105 for the weekday and weekend A.M. and P.M. peak hours. An exclusive right turn lane is also warranted at the intersection of Canterbury Dr. and Saddlewood Rd. on westbound Saddlewood Rd. for the weekend PM peak hour.

Per the ECM, the right turn auxiliary lane on eastbound Highway 105 at Highway 105 and Canterbury Dr. is recommended to have a 270' approach taper, 318' deceleration lane (lane length), and 100' of storage. In the current condition, Highway 105 would not be wide enough to meet the auxiliary lane criteria and will need to be widened. Calculations for the auxiliary lane were made using a 6% downgrade and assuming 12' widening on the south side of the highway.

Per the ECM, the right turn auxiliary lane on westbound Saddlewood at Canterbury Dr. and Saddlewood Rd. is recommended to have a 108' approach taper, 104' deceleration lane (lane length), and 100' of storage. In the current condition, Saddlewood Rd. would not be wide enough to meet the auxiliary lane criteria and will need to be widened. Calculations for the auxiliary lane were made using a 3% upgrade and assuming 12' widening on the north side of the road.

Per the ECM, a right turn acceleration lane is required on a principal arterial when the projected right turning movement volume is 50 vph or greater and the posted speed limit is greater than 40 mph. Table 14 shows the project generated right turn movements at each intersection versus the ECM criteria.

Intersection	Turn Movement	Peak Hour	Projected Right Turns (vph)	ECM Criteria (vph)	Warranted
	NB RT	Weekday AM	8	50	No
Canterbury &		Weekday PM	9	50	No
Hwy 105		Weekend AM	6	50	No
		Weekend PM	30	50	No
Appaloosa & Hwy 105	NB RT	Weekday AM	4	50	No
		Weekday PM	5	50	No
		Weekend AM	2	50	No
		Weekend PM	2	50	No

**Table 14: Right Turn Acceleration Lane Analysis** 

As can be seen from Table 14, a right turn acceleration lane is not warranted at any of the study intersections.

#### ROAD IMPACT FEE

The following is the method for computing the fees which factors in the length of time the event is open per year and will exclude the first 50 parking spaces which are already allowed with the standard agritainment use.

- 1. Event average daily trips for the period it's open = ADT
- Convert to Annual ADT = (ADT) x (# of weeks open) / (52 weeks/year) = AADT
- 3. Reduction Factor = 1 (50 initial spaces)/(Total Parking spaces proposed)
- 4. Road Impact Fee = AADT x (\$398.55 per AADT) x (Reduction Factor)

Following this methodology, the combined road impact fee for both the fall festival and tulip festival is \$28,097. Calculations and assumptions for the road impact fee can be seen in the appendix.

The Farm camp was not considered in road impact fees as the event will not exceed 50 cars, and is covered under the current zoning of the Colorado Kids ranch.

#### **CONCLUSIONS**

This traffic impact study analyzed the traffic impacts of the proposed Colorado Kids Ranch on the adjacent roadway network.

The LOS analysis for all intersections indicated that the LOS for all approaches to the intersection would continue to operate at LOS D or better, for the A.M. and P.M. peak hours of the events. This remains true for the existing plus development, short-range horizon, and long-range horizon scenarios.

An auxiliary left turn lane is warranted in the westbound direction at the intersection of Canterbury Dr. and Highway 105 for weekday and weekend A.M. and P.M. peak hours. This left turn lane would require a 540' redirect taper (assuming 12' widening on the north side of Highway 105), a 160' bay taper, lane length of 188', and 100' of storage.

An auxiliary left turn lane is warranted in the southbound direction at the intersection of Canterbury Dr. and Saddlewood Rd. for the weekday and weekend A.M. and P.M. peak hours. This left turn lane would require 90' of redirect taper (assuming 6' of widening on each side of the road), a bay taper of 144', a 138' lane length, and 200' of storage.

An auxiliary left turn lane is warranted in the northbound direction at the intersection of Canterbury Dr. and Highway 105 for the weekend P.M. peak hour. This left turn lane would require a redirect taper of 90' (assuming 6' lane

widening on each side of the road), a bay taper of 72', a lane length of 104', and a storage length of 50'.

An auxiliary right turn lane is warranted in the eastbound direction at the intersection of Highway 105 and Canterbury Dr. for the weekday and weekend A.M. and P.M. peak hours. This right turn lane would require an approach taper of 200', a lane length of 318', and 150' of storage.

An auxiliary right turn lane is warranted in the westbound direction at the intersection of Canterbury Dr. and Saddlewood Rd. for the weekend P.M. peak hour. This right turn lane would require an approach taper of 108', a lane length of 104', and a storage length of 100'.

Based on the analysis presented in this report, the Colorado Kids Ranch generated traffic will require some roadway improvements to negate the impacts to the surrounding roadway network.

Per the MTCP, Highway 105 from Knollwood Boulevard to Highway 83 is identified as a 2040 roadway improvement project. This project would expand Highway 105 from a two-lane principal arterial to a four-lane principal arterial. In addition, there is a current corridor analysis being completed for Highway 105, from Interstate 25 to Highway 83. However, this corridor analysis for the section of Highway 105 adjacent to the Colorado Kids Ranch has not been completed, and thus, has not recommended any roadway improvements for Highway 105 in this area.

# **APPENDIX**



# VICINITY MAP





# EXISTING TURNING MOVEMENT COUNTS



Four Approach Field Sheet

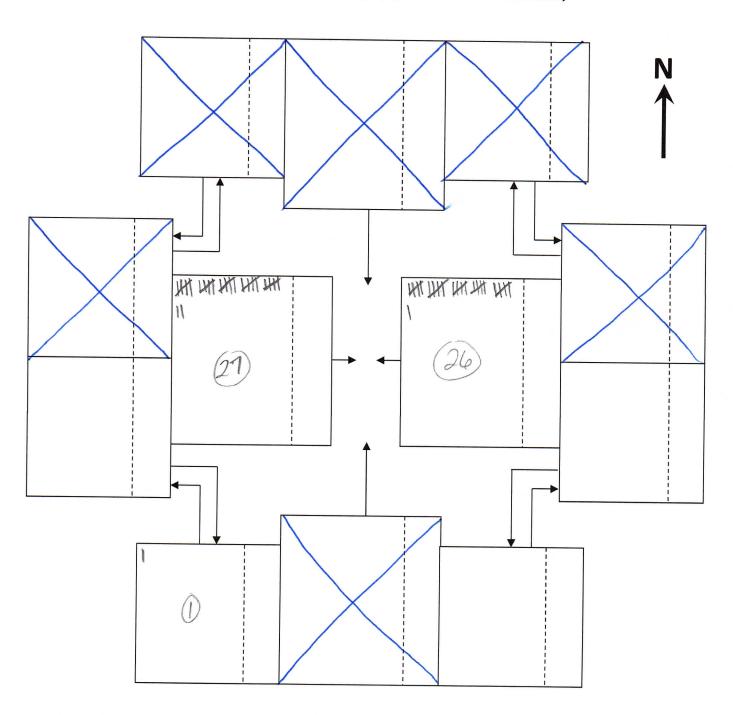
N/S Street: Cantubury Dr E/W Street: 105 Time: 9:00 to 9:15

Date: 12/17/22

Weather:

Observer: Brett

## Counts are Conducted From the Direction of Travel



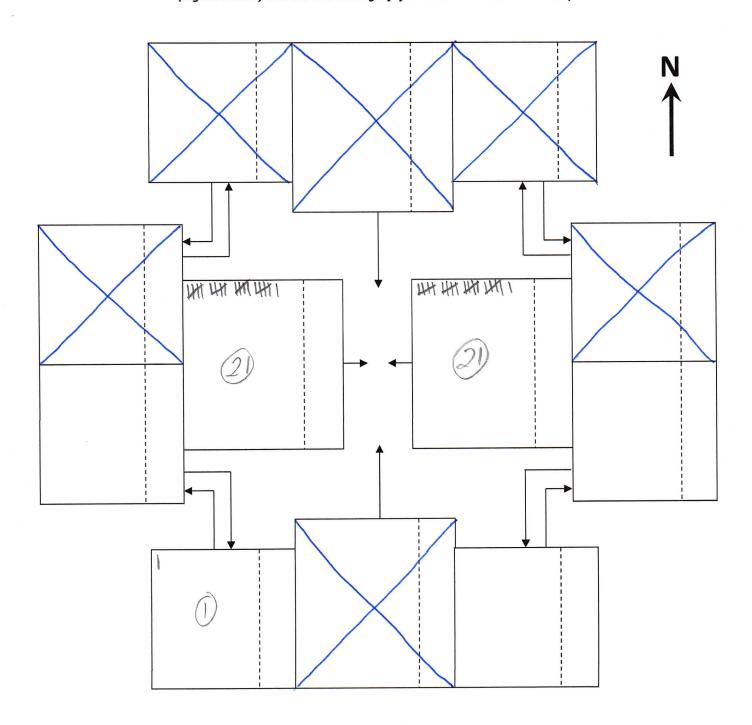
Four Approach Field Sheet

Time: 9:15 to 9:30

Date: 12/17/22

Weather: Brett N/S Street: Carterbury Dr E/W Street: 105

### Counts are Conducted From the Direction of Travel



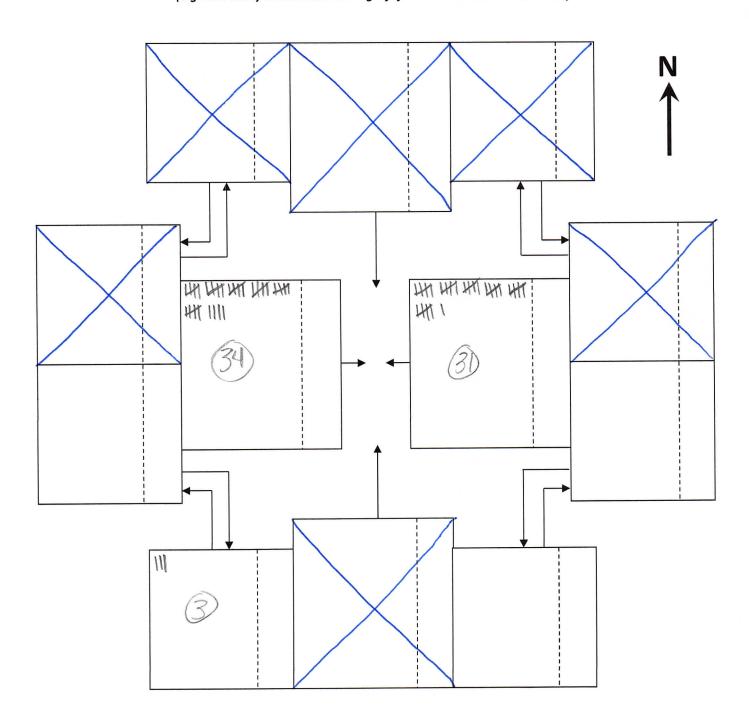
Four Approach Field Sheet

Time: 9:30 to 9:45

Date: 12/17/22

Weather:
Observer: Brett

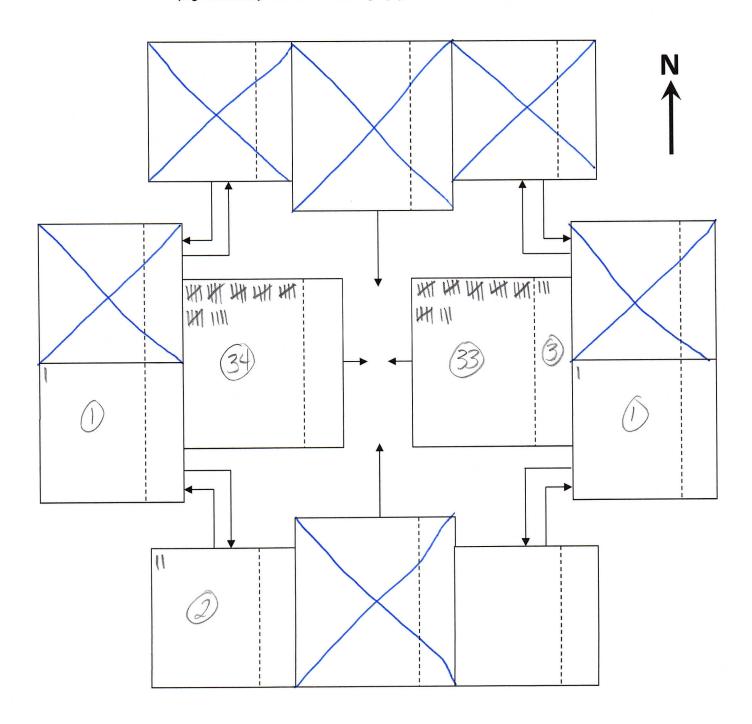
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:45 to 10:00	
N/S Street:	Canterbury Dr	Date:	12/17/22	
E/W Street:	105	Weather:	, ,	
		Observer:	But	

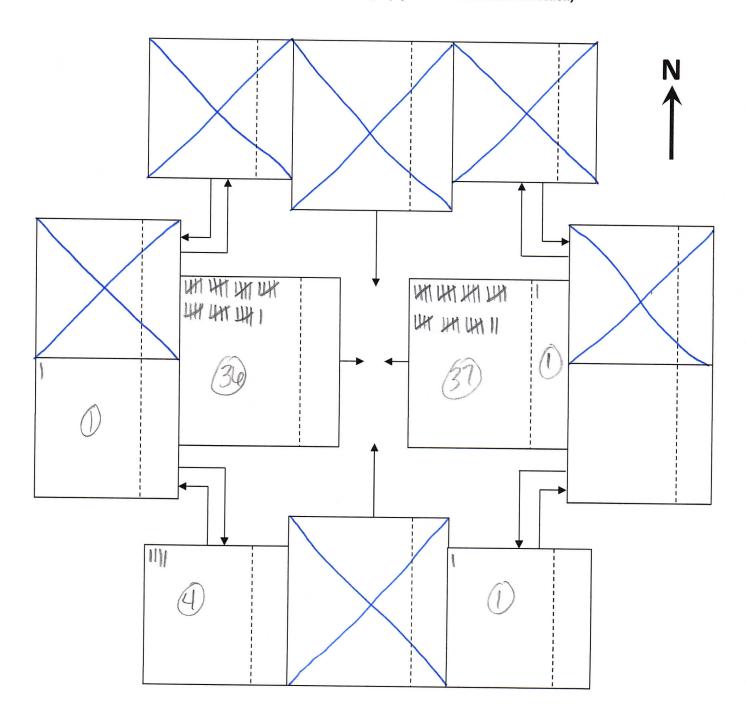
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1:00 to 1:15	
N/S Street:	Canterbury Dr	Date:	12/17/22	
E/W Street:	105	Weather:	· · · · · · · · · · · · · · · · · · ·	_
		Observer:	Brett	_

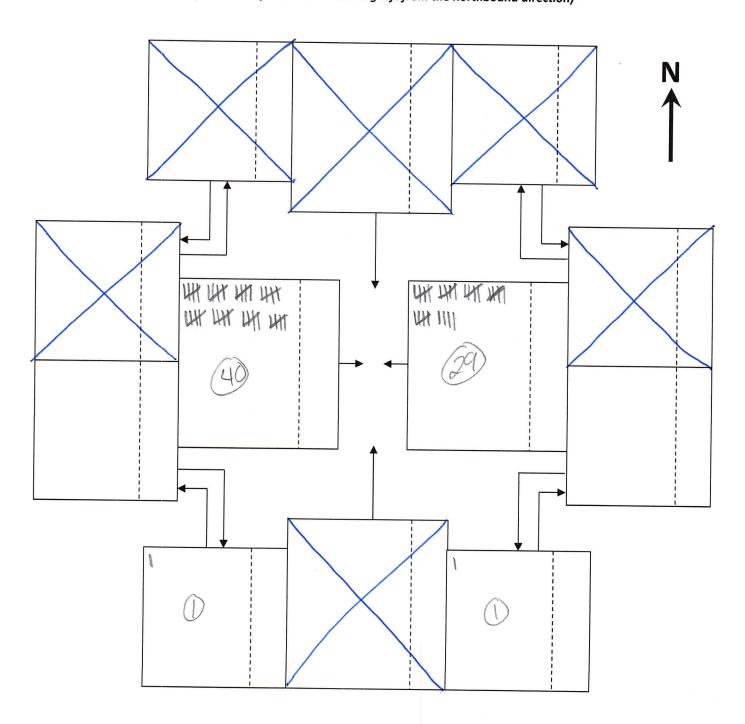
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street:	Canterbury Dr	Date:	12/17/22	
E/W Street:	105	Weather:	12/1//22	
		Observer:	Brett	

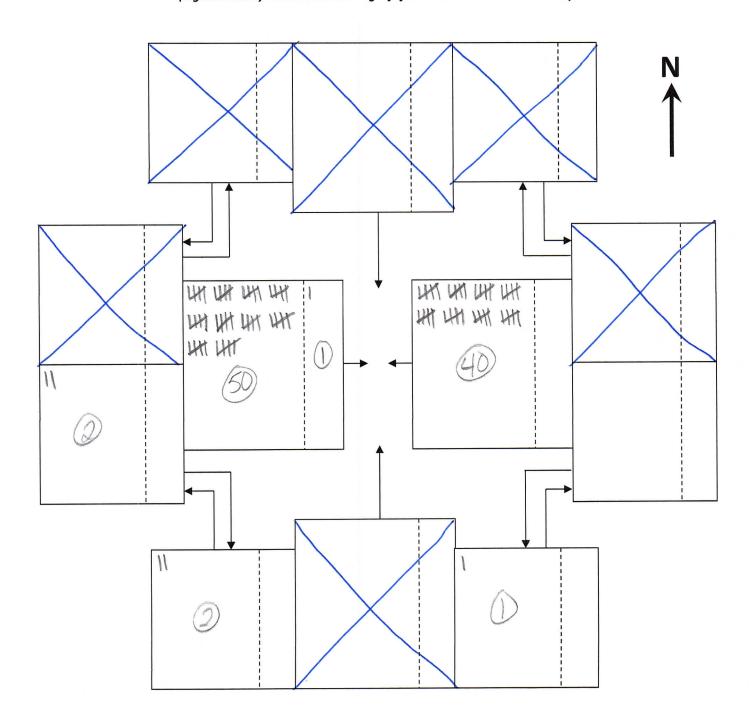
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1.30 to 1:43	
N/S Street:	Canterbury Dr	Date:	12/17/22	
E/W Street:	105	Weather:		
		Observer:	Brett	

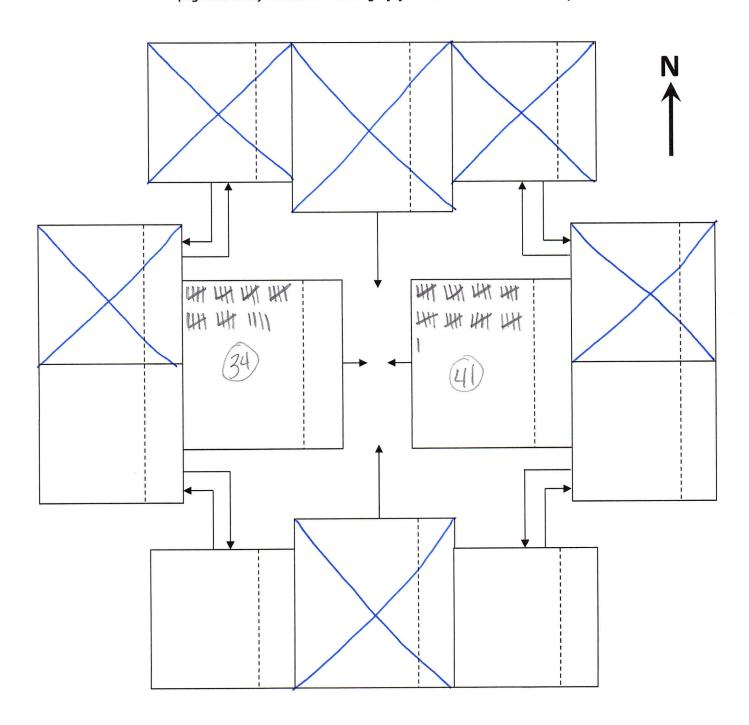
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1:45 to 2:00
N/S Street:	<b>Canterbury Dr</b>	Date:	12/17/22
E/W Street:	105	Weather:	
		Observer:	Brett

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

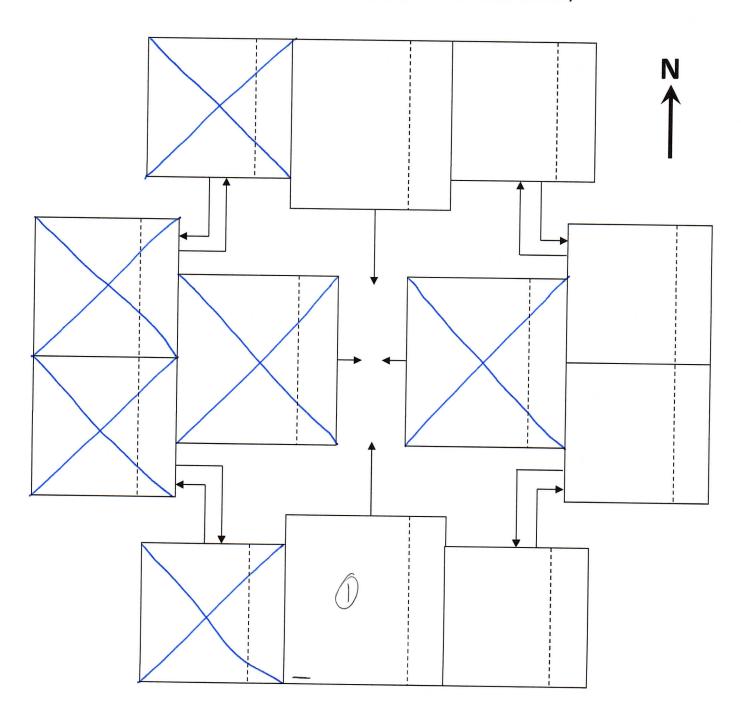
N/S Street: Canterbury Dr E/W Street: Saddlewood Rd Time: 9:00 to 9:15

Date: 12/17/22

Weather: 7º Sunny

Observer: 1000 to 9:15

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

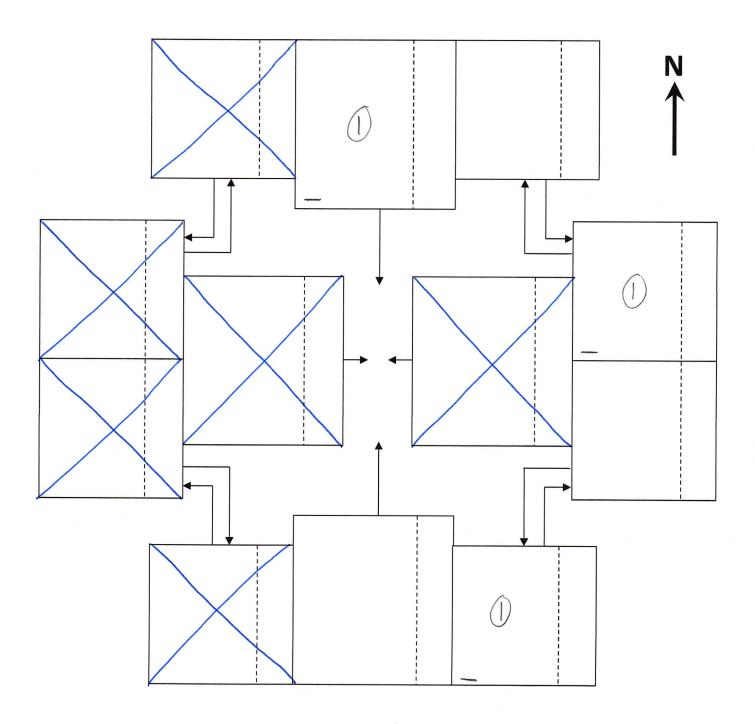
N/S Street: <u>Canterbury Or</u> E/W Street: <u>Saddlewood Rd</u> Time: 9:15 to 9:30

Date: 12/17/22

Weather: 7º Sunny

Observer: Jerrife

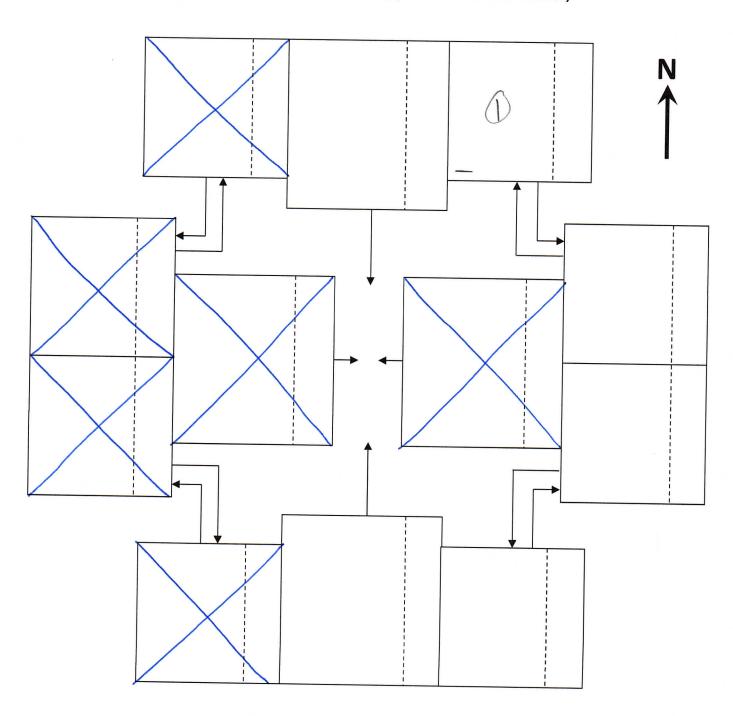
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Canterbury Dr E/W Street: Saddlewood Rd

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

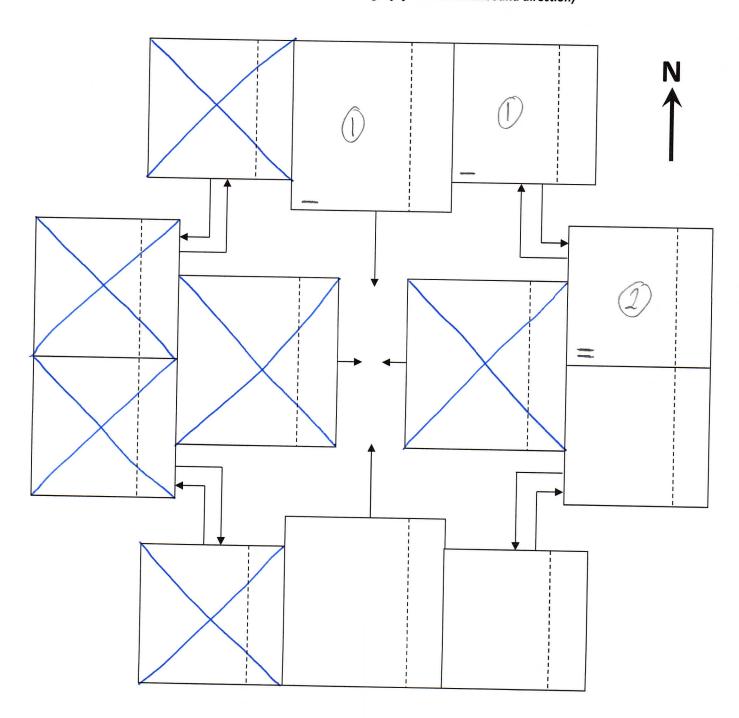
N/S Street: <u>Lantubury</u> Dr E/W Street: <u>Saddlewood</u> Rd Time: 9:45 to 10:00

Date: 12/17/22

Weather: 16 Sunny

Observer: 1000 jets

### **Counts are Conducted From the Direction of Travel**

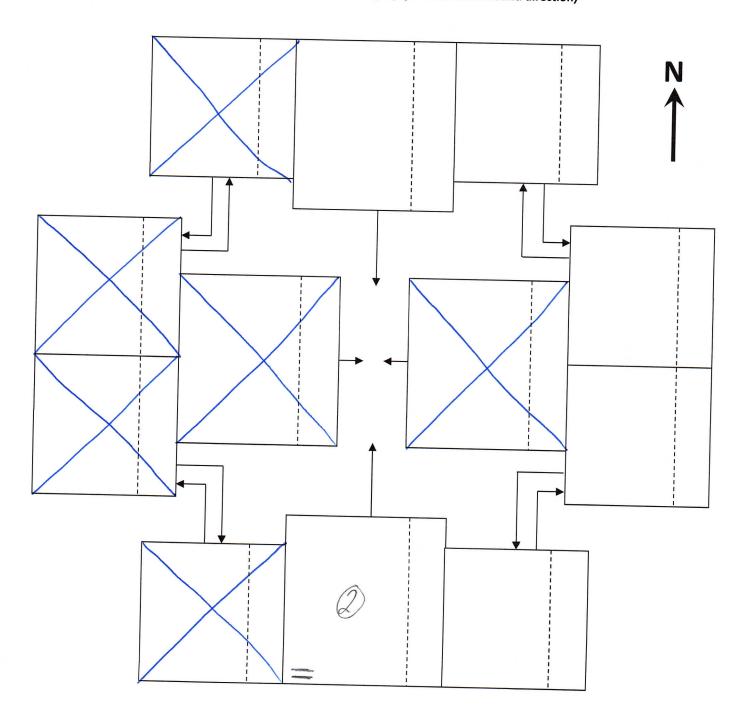


Four Approach Field Sheet

N/S Street: Canterbury Dr
E/W Street: Saddlewod Rd

Time: /:00 to /:15
Date: 12/17/22
Weather:
Observer: Jennifer

## Counts are Conducted From the Direction of Travel



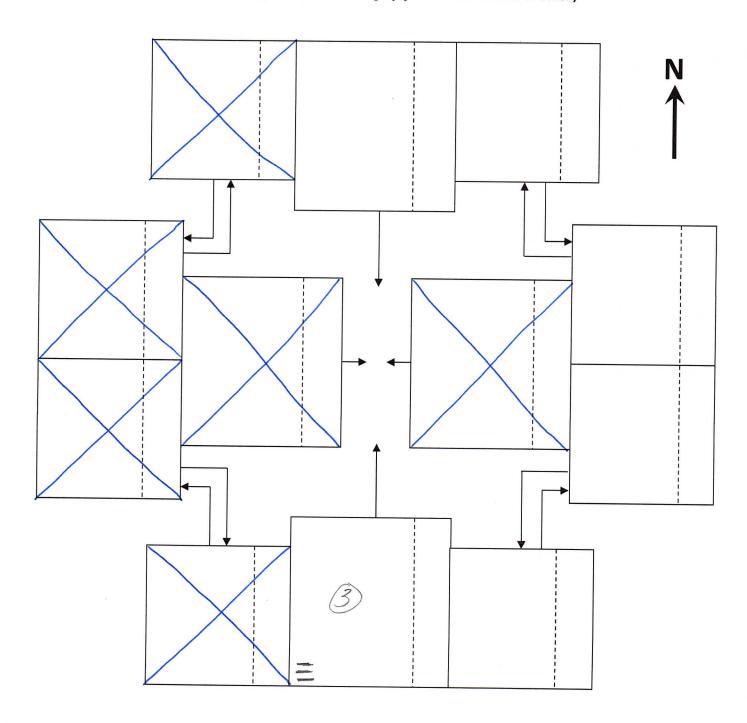
Four Approach Field Sheet

N/S Street: Canterbury Dr E/W Street: Saddlewod Rd Time: /:/5 to /:30

Date: 12/17/22

Weather: 28° Juny
Observer: Jennifer

#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

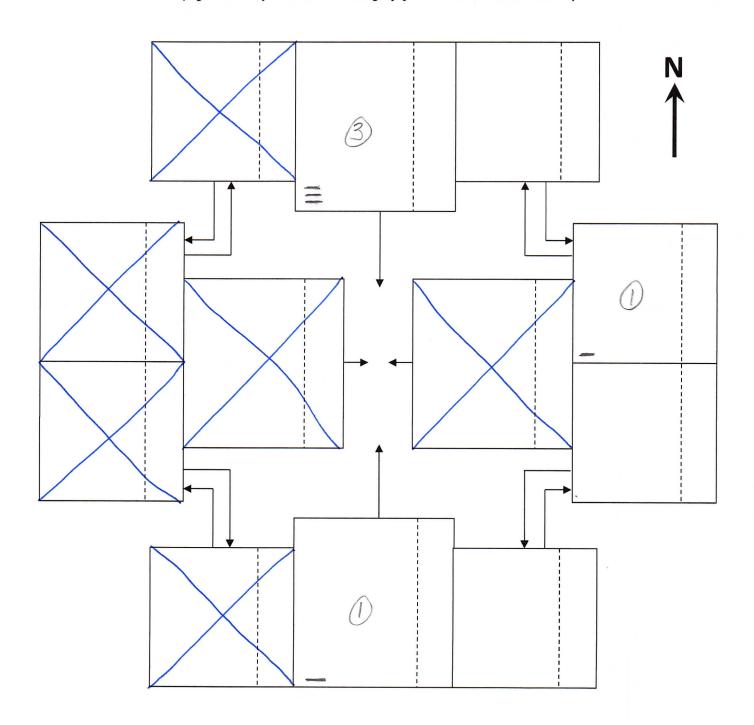
N/S Street: Canterbury Dr
E/W Street: Saddlewod Rd We

 Time:
 /:30
 to /:45

 Date:
 12/17/22

 Weather:
 Jennifer

#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Canterbury Dr
E/W Street: Saddlewod Rd

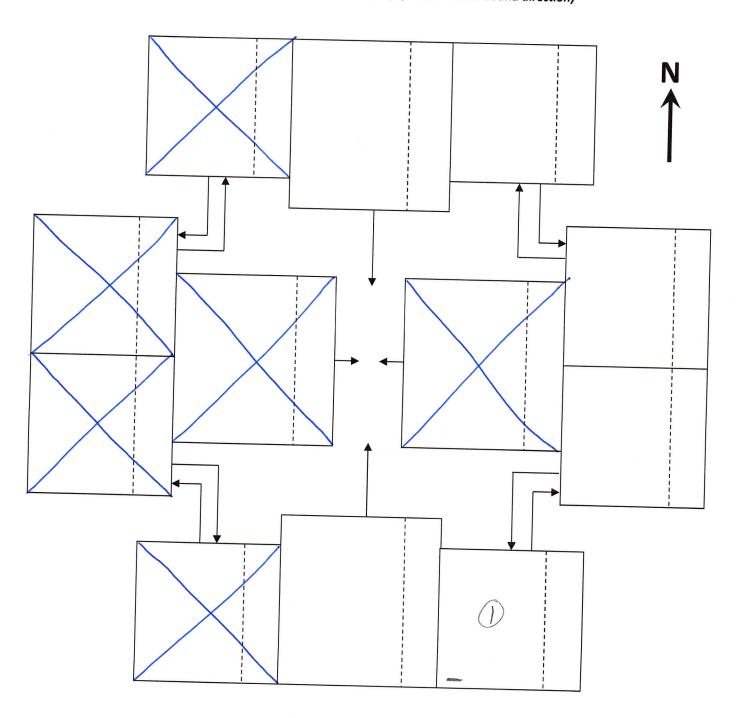
 Time:
 /: 45
 to 2:00

 Date:
 12/17/22

 Weather:
 28° Sunny

 Observer:
 Jennifer

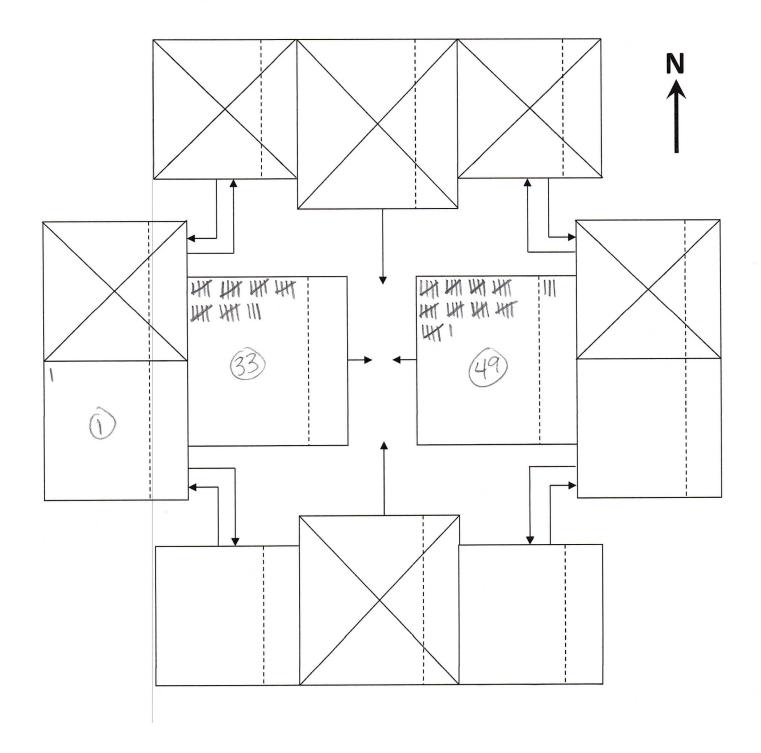
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:00 to 9:15
N/S Street:	Canterbury Dr	Date:	12/21/22
E/W Street:	105	Weather:	
_		Observer:	Brett

#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

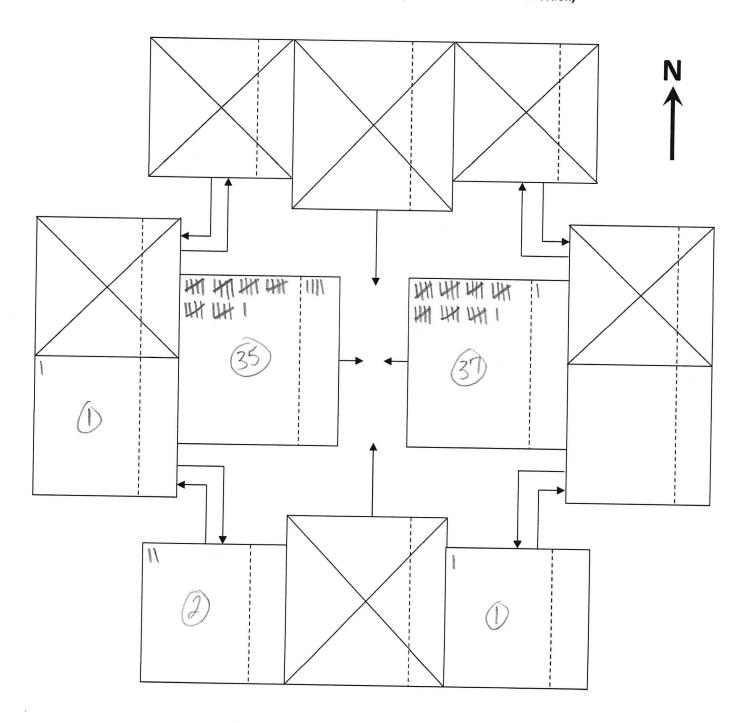
N/S Street:	Canterbury Dr
E/W Street:	105

 Time:
 9:15 to 9:30

 Date:
 12/21/22

 Weather:
 Brett

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

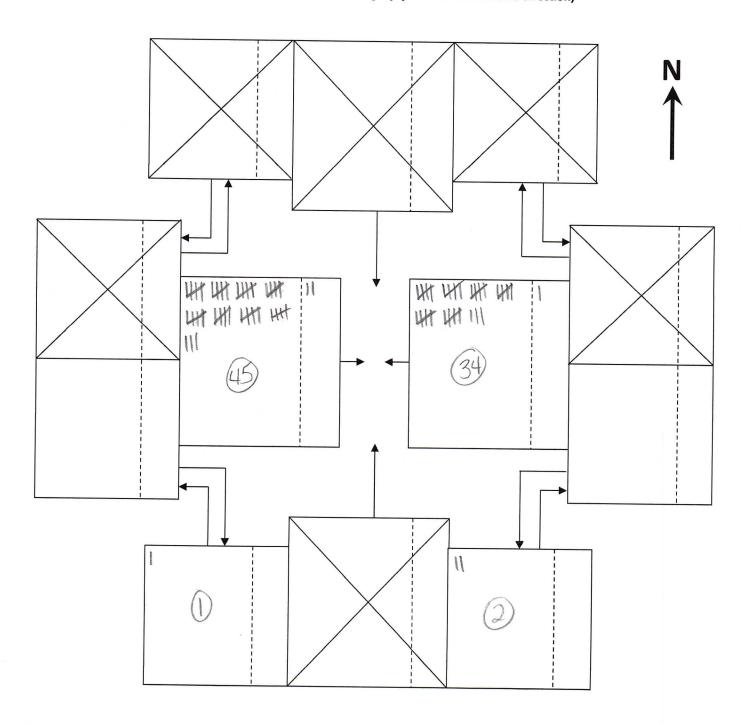
N/S Street: Canterbury Dr Date: E/W Street: 105 Weather:

Time: 9:30 to 9:45

Date: 12/21/22

Weather: Brett

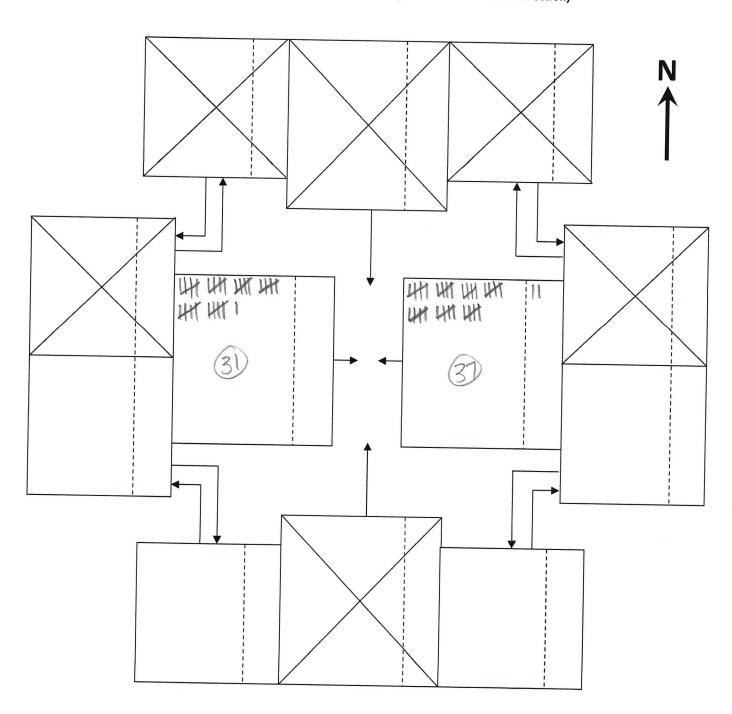
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:45 to 10:00	
N/S Street: _ E/W Street: _	Canterbury Dr 105	Date:	12/21/22	
		Weather:		
		Observer:	Brett	

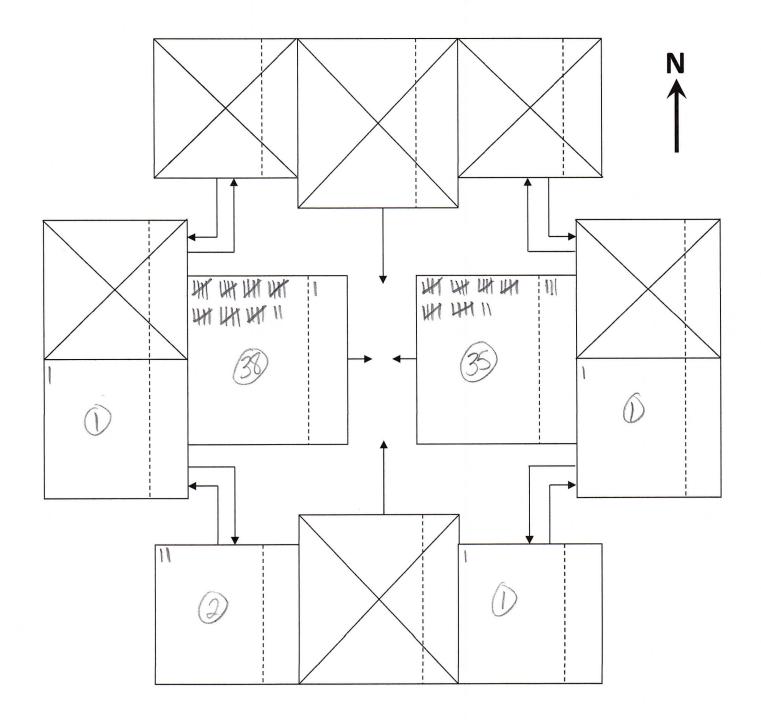
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1.00 to 1.13	
N/S Street:	Canterbury Dr	Date:	12/21/22	
E/W Street:	105	Weather:		
		Observer:	Brett	_

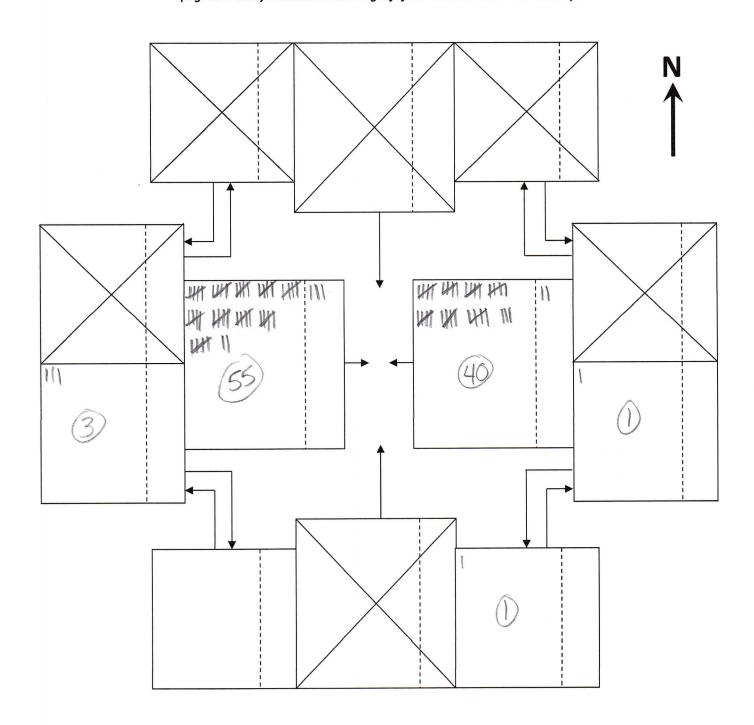
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

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E/W Street:	105	Weather:		
_		Observer:	Brett	

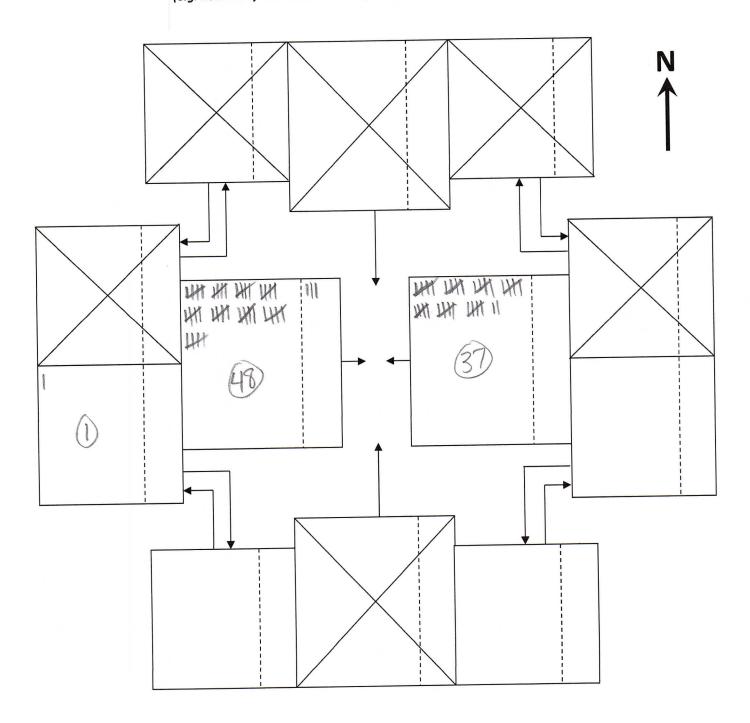
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		· Time:	1:30 to 1:45
N/S Street:	Canterbury Dr	Date:	12/21/22
E/W Street:	105	Weather:	
		Observer:	Brett

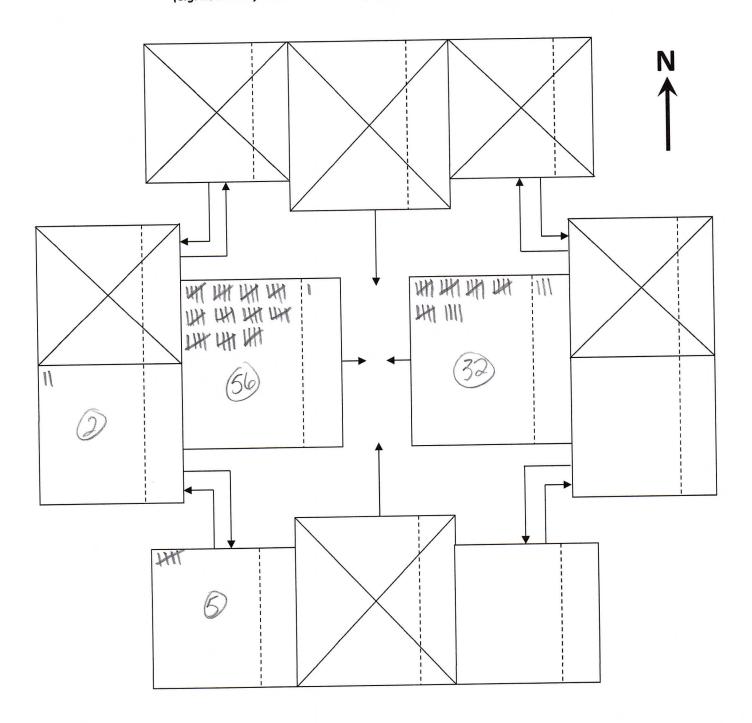
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1:45 to 2:00
N/S Street:	Canterbury Dr	Date:	12/21/22
E/W Street:	105	Weather:	
		Ohserver:	Brett

### **Counts are Conducted From the Direction of Travel**

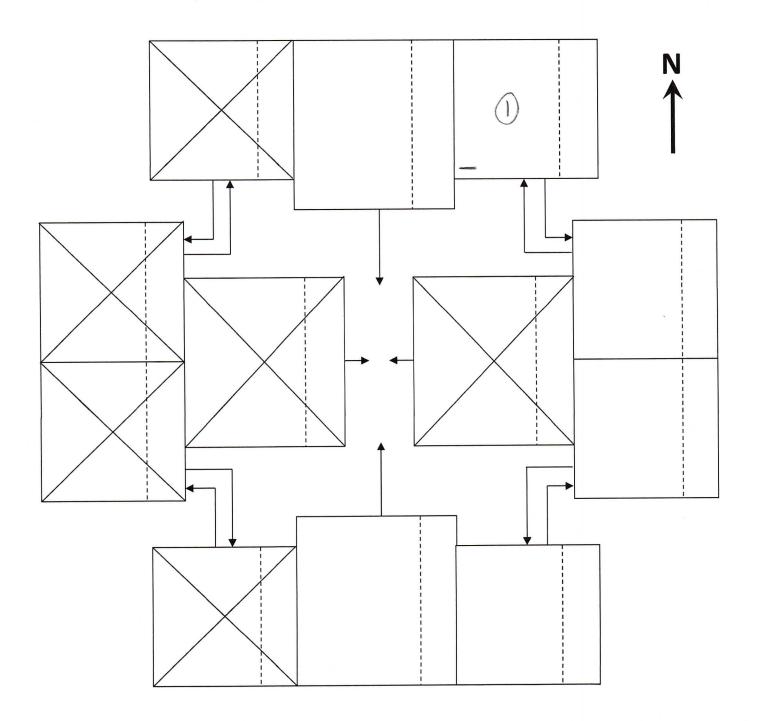


Four Approach Field Sheet

N/S Street: Canterbury Dr Date: 12/21/22

E/W Street: Saddlewood Rd Weather: Observer: Jennifer

#### Counts are Conducted From the Direction of Travel

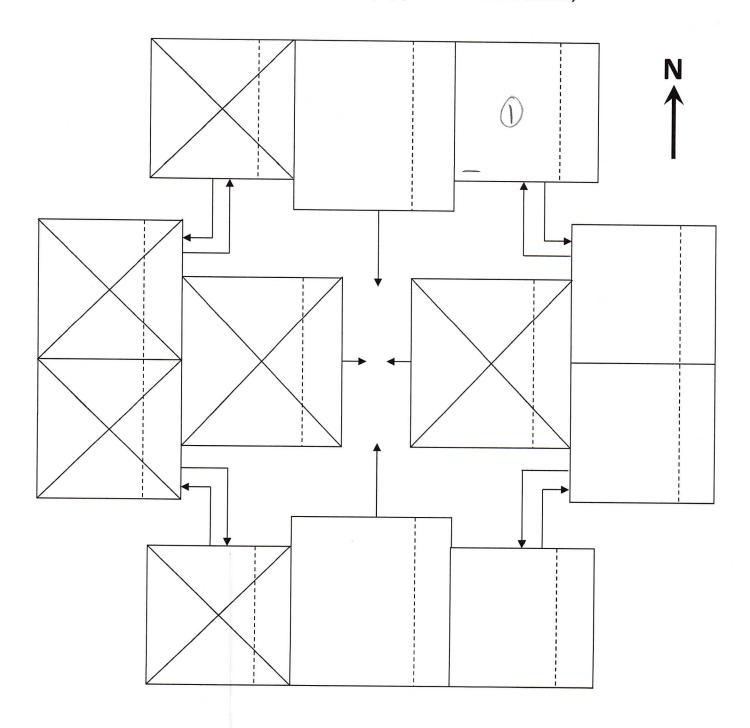


Four Approach Field Sheet

N/S Street:	Canterbury Dr	
E/W Street:	Saddlewood Rd	

Time:	9:15 to 9:30
Date:	12/21/22
Weather:	
Observer:	Jennifer

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Canterbury Dr Date:

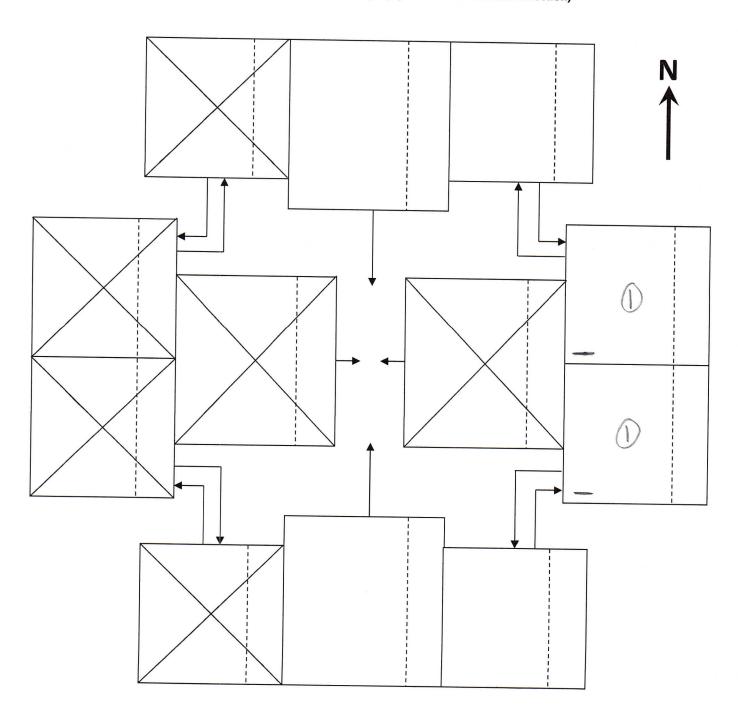
E/W Street: Saddlewood Rd Weather:

Time: 9:30 to 9:45

Date: 12/21/22

Weather: Jennifer

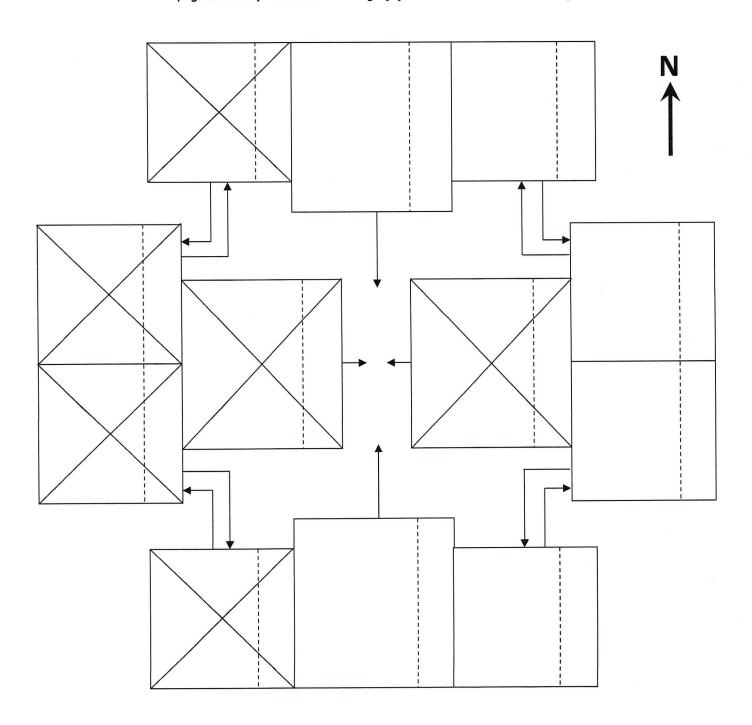
### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:43 to 10:00		
N/S Street:	<b>Canterbury Dr</b>	Date:	12/21/22		
E/W Street:	Saddlewood Rd	Weather:	,		
		Observer:	lennifer		

#### Counts are Conducted From the Direction of Travel

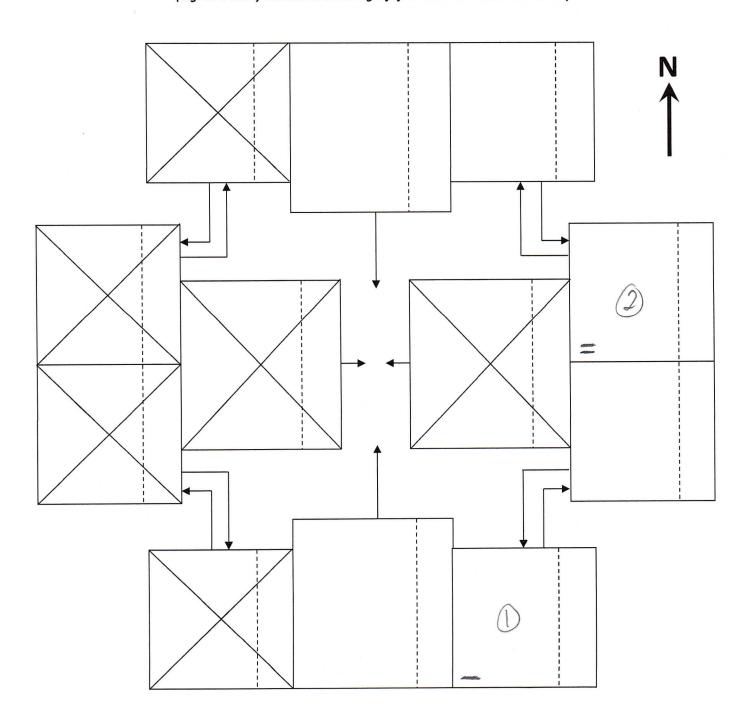


Four Approach Field Sheet

N/S Street: Canterbury Dr Date: 12/21/22

E/W Street: Saddlewood Rd Weather: Observer: Jennifer

#### Counts are Conducted From the Direction of Travel

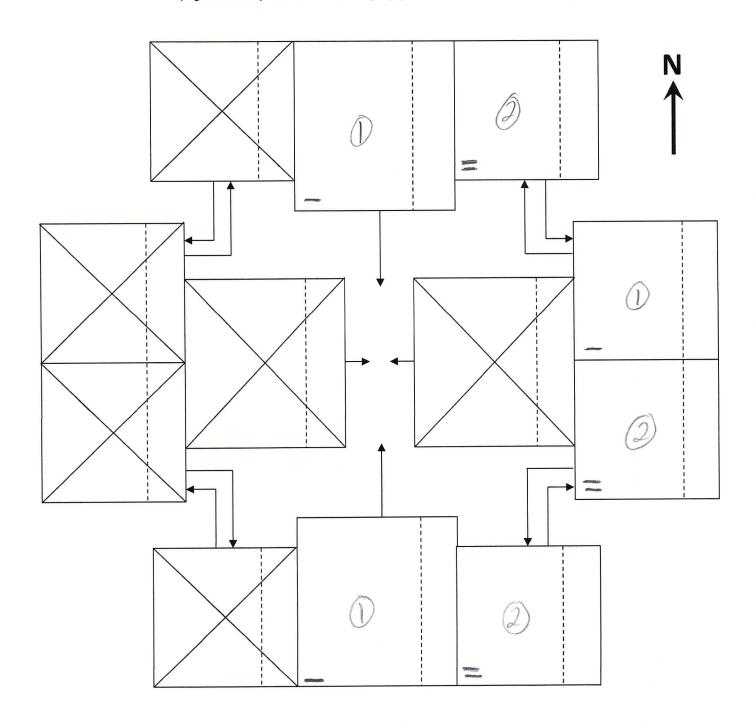


Four Approach Field Sheet

N/S Street: Canterbury Dr Date: 12/21/22

E/W Street: Saddlewood Rd Weather: Observer: Jennifer

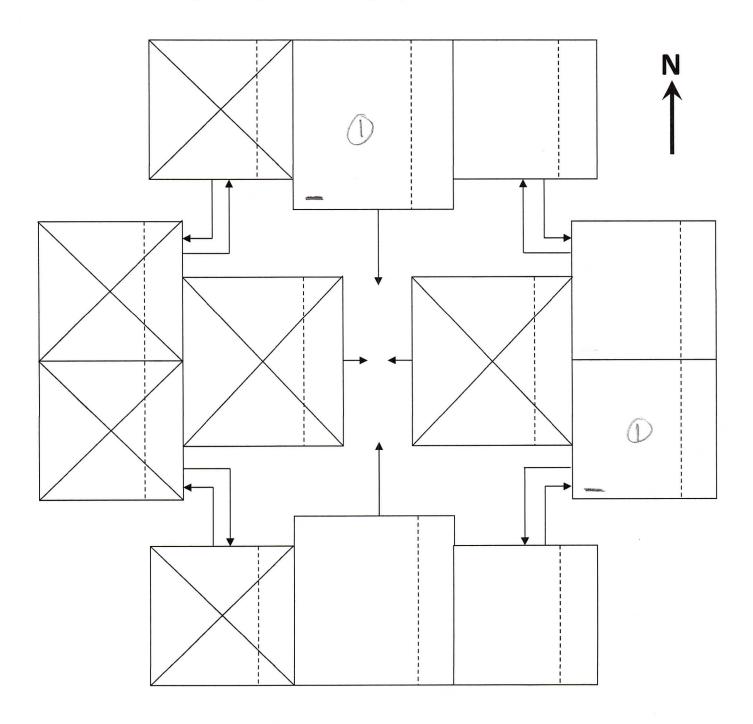
Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1:30 to	1:45	
N/S Street:	<b>Canterbury Dr</b>	Date:	12/21/22		
E/W Street:	Saddlewood Rd	Weather:			
_		Ohserver:	leni	nifer	

#### Counts are Conducted From the Direction of Travel

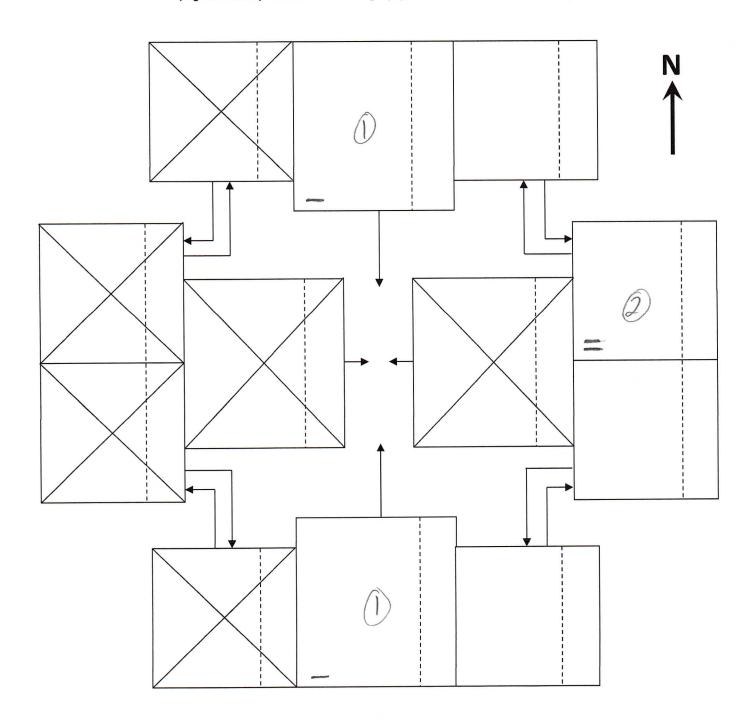


Four Approach Field Sheet

N/S Street: Canterbury Dr Date: 12/21/22

E/W Street: Saddlewood Rd Weather: Observer: Jennifer

#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

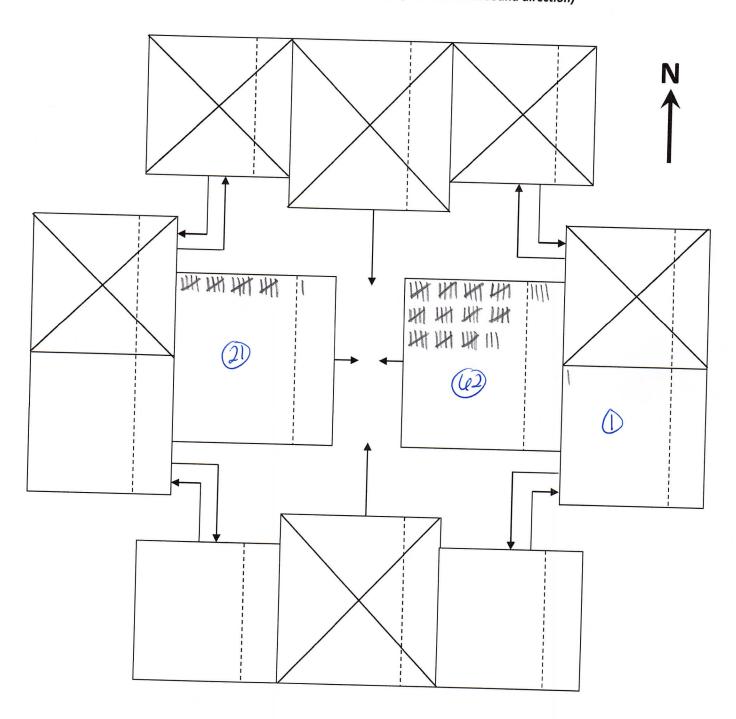
Time: 9:00 to 9:15

Date: 3/14/23

Weather: Partly cloudy

Observer: Stell loads

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

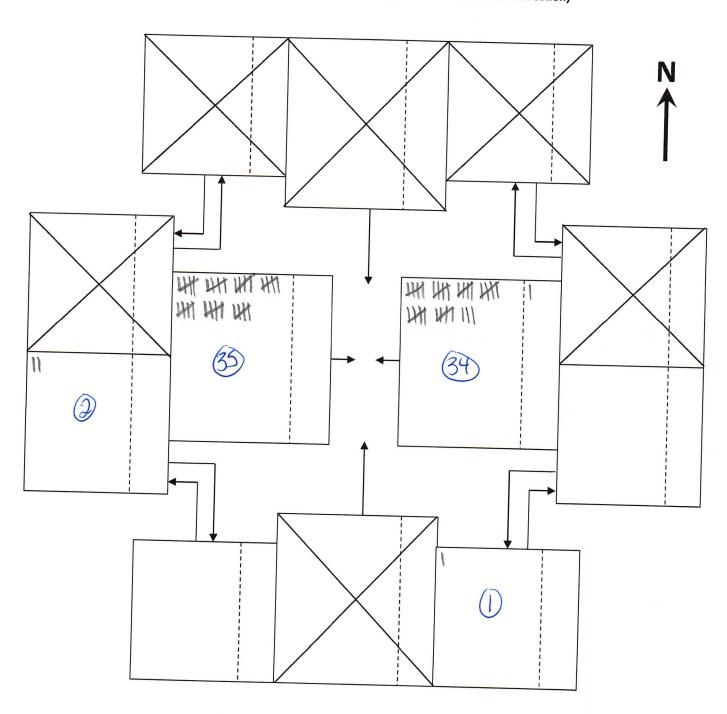
Time: 9:15 to 9:30

Date: 3/14/23

Weather: Partly Cloudy

Observer: brett Louk

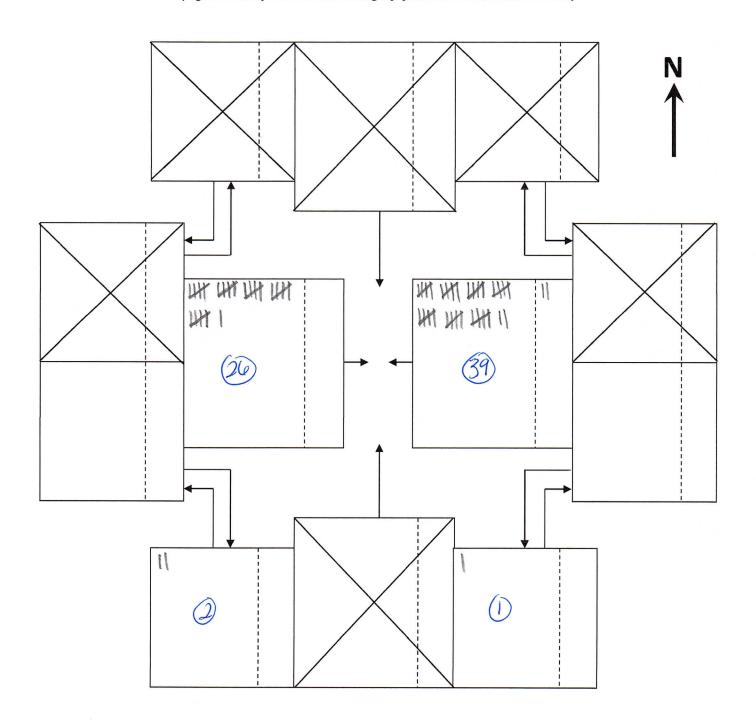
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:30	to	9:45	
N/S Street:	Appaloosa Rd	Date:	3/14/23			
E/W Street:	Highway 105	Weather:				
		Observer:				

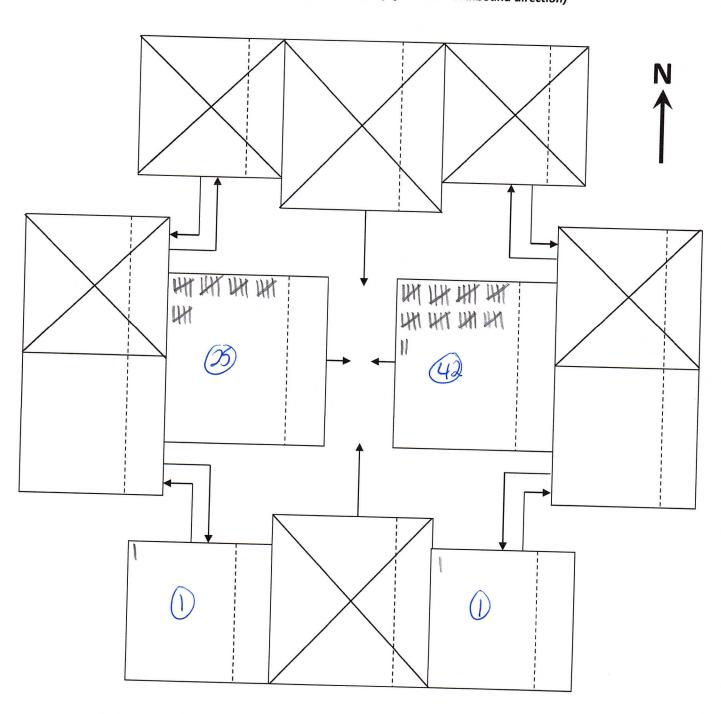
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

NI/C Charles		Time:	9:45	to 10:00	
	Appaloosa Rd	Date:	3/14/23		
E/W Street:	Highway 105	Weather:			
		Observer:			

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

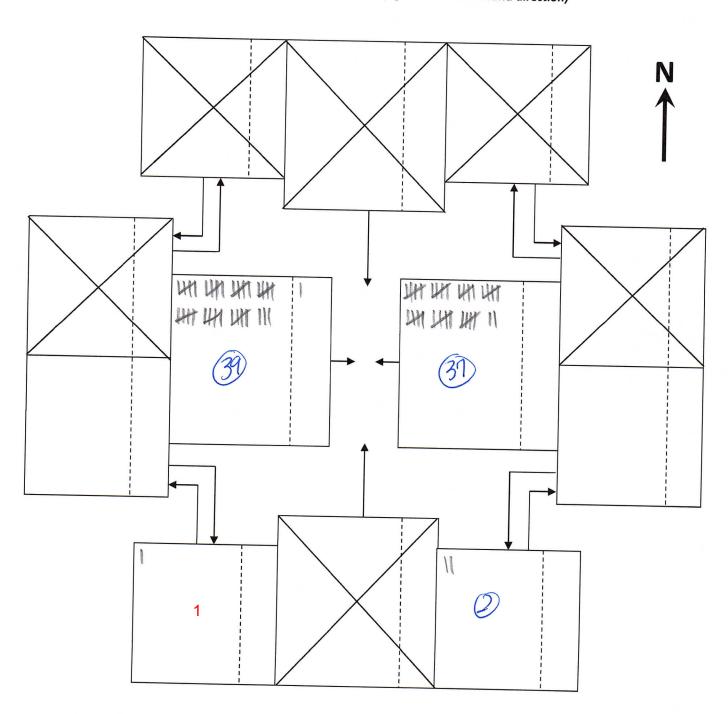
Time: /:00 to /:15

Date: 3/14/23

Weather: | Claudy | Cl

Observer: South Louk

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

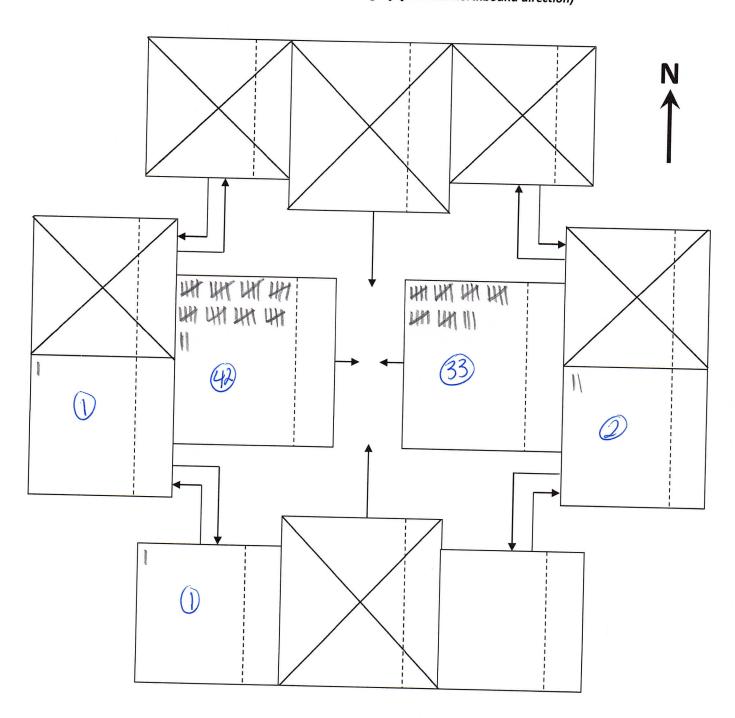
Time: 1:15 to 1:30

Date: 3/14/23

Weather: Raitly Cloudy

Observer: Brett Louk

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

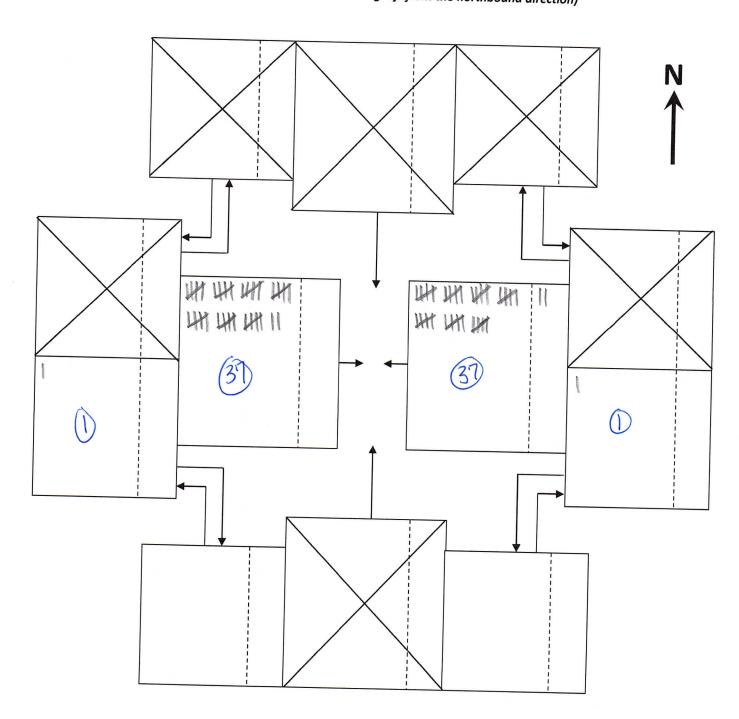
Time: 1:30 to 1:45

Date: 3/14/23

Weather: Claudy

Observer: Brett Louk

## Counts are Conducted From the Direction of Travel



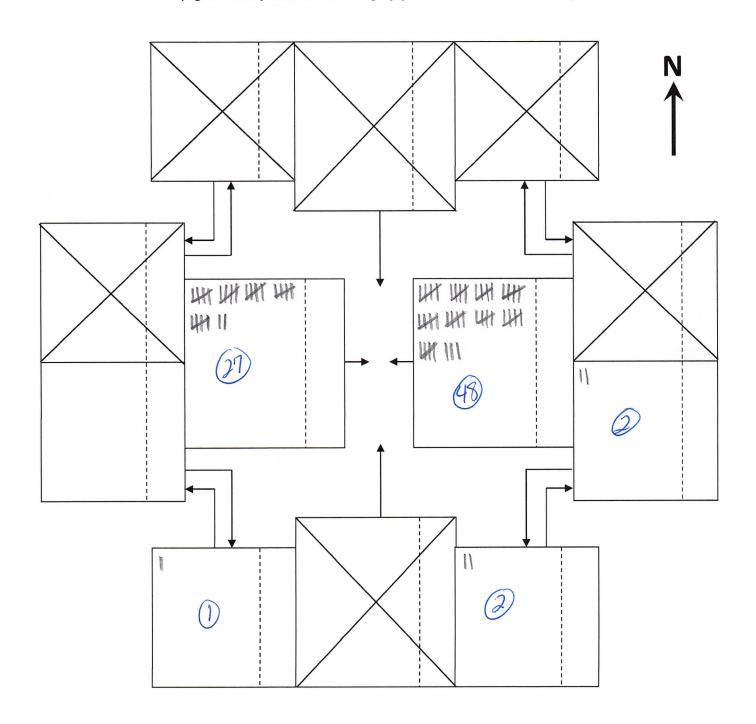
Four Approach Field Sheet

*Time:* 1:45 to 2:00 *N/S Street:* Appaloosa Rd *Date:* 3/14/23

E/W Street: Highway 105

Weather: Partly cloudy
Observer: Rett Louk

#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

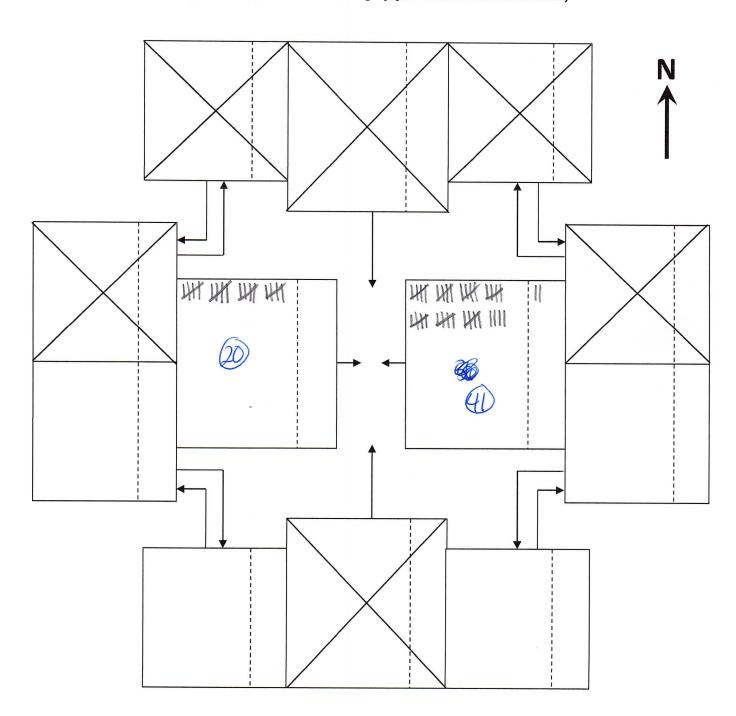
Time: 9:00 to 9:15

Date: 3/18/23

Weather: Sung cold

Observer: Brett Look

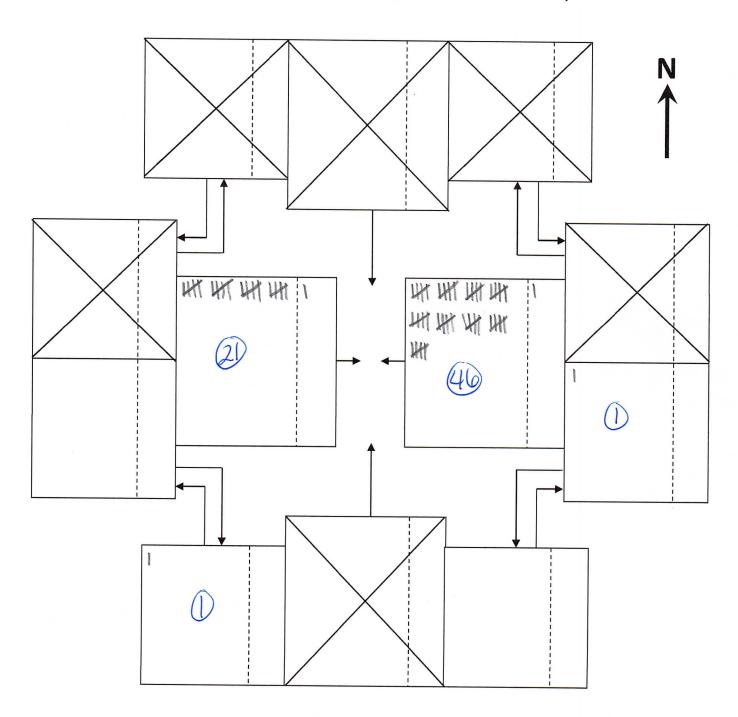
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:15	to	4:30	
N/S Street:	Appaloosa Rd	Date:	3/18/23			
E/W Street:	Highway 105	Weather:			*	
		Observer:				

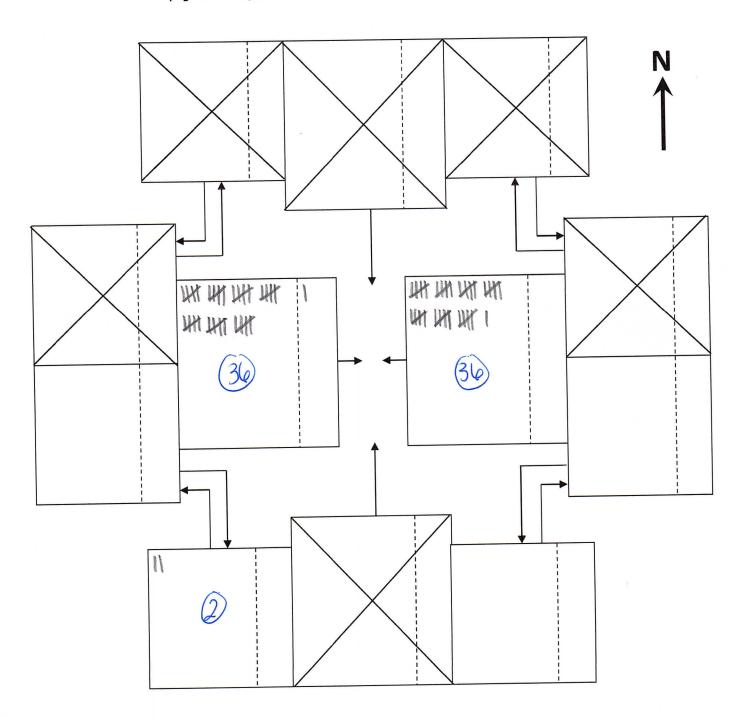
## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	9:30	to	9:45	
N/S Street:	Appaloosa Rd	Date:	3/18/23			
•	Highway 105	Weather:				
		Observer:				

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Appaloosa Rd
E/W Street: Highway 105

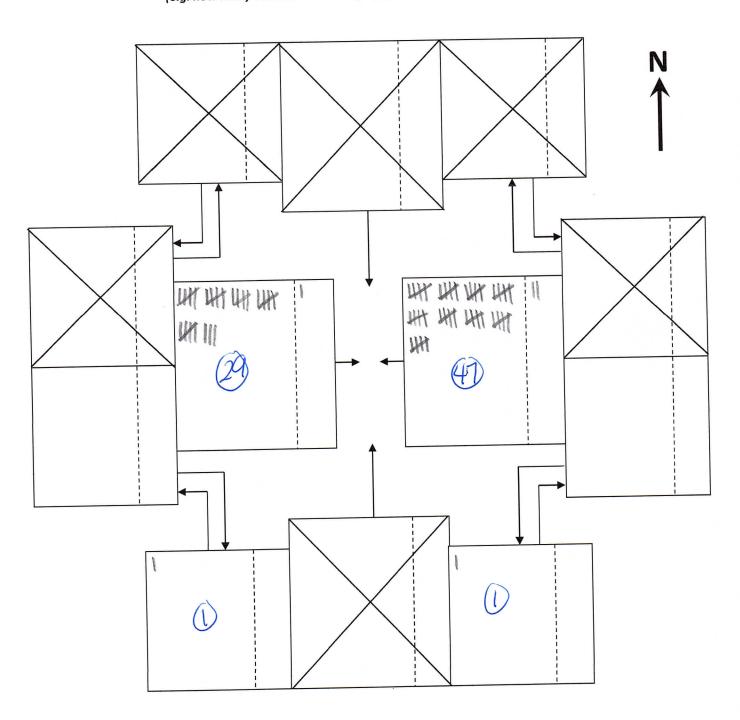
Time: 9:45 to 10:00

Date: 3/18/23

Weather: Surff Look

Observer: Breft Look

### **Counts are Conducted From the Direction of Travel**



Four Approach Field Sheet

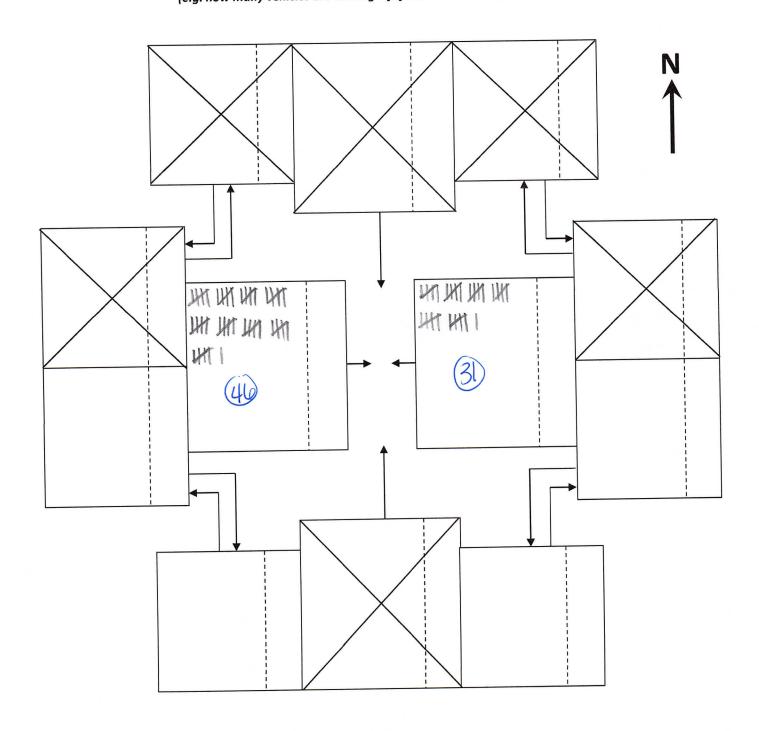
N/S Street:	Appaloosa Rd	
E/W Street:	Highway 105	

Time: 1:00 to 1:15

Date: 3/18/23

Weather:
Observer: Sreft Louk

### **Counts are Conducted From the Direction of Travel**



Four Approach Field Sheet

N/S Street:	Appaloosa Rd	
E/W Street:	Highway 105	

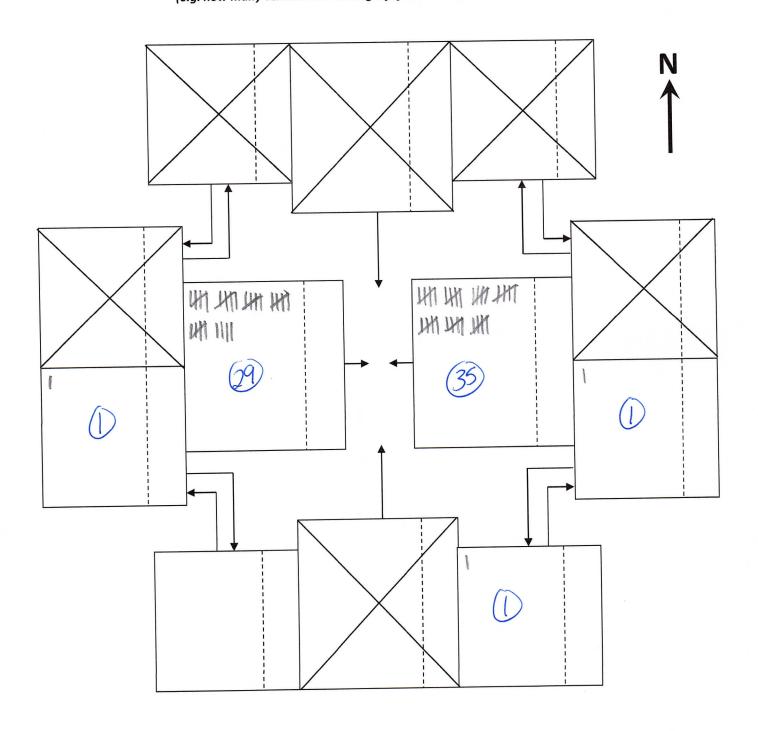
Time: /:/5 to /:30

Date: 3/18/23

Weather:

Observer: Bred Lowe

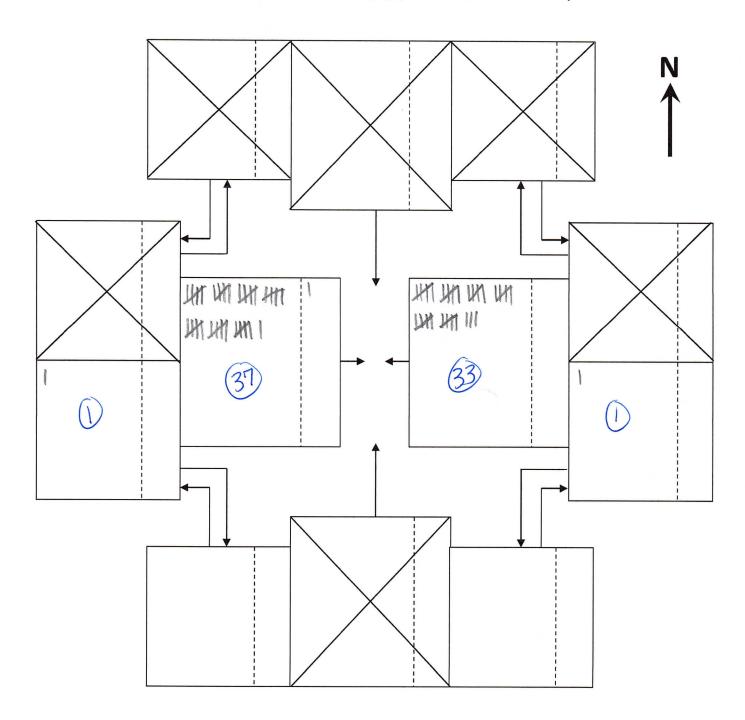
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1.30	to /:45	
N/S Street:	Appaloosa Rd	Date:	3/18/23		
E/W Street:	Highway 105	Weather:			
		Observer:	Ruff	2.18	

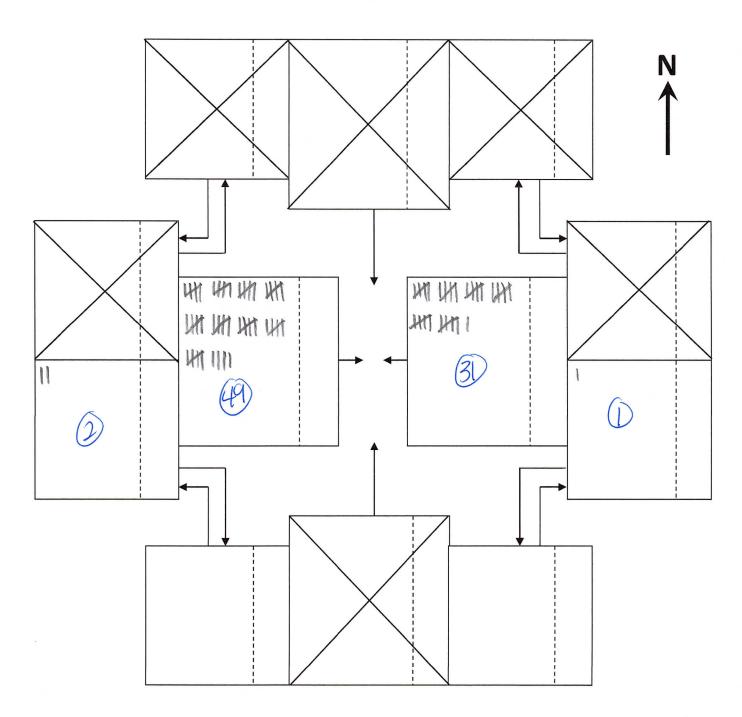
#### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

		Time:	1.45	to 1:00	
N/S Street:	Appaloosa Rd	Date:	3/18/23		
E/W Street:	Highway 105	Weather:			
		Observer:			

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

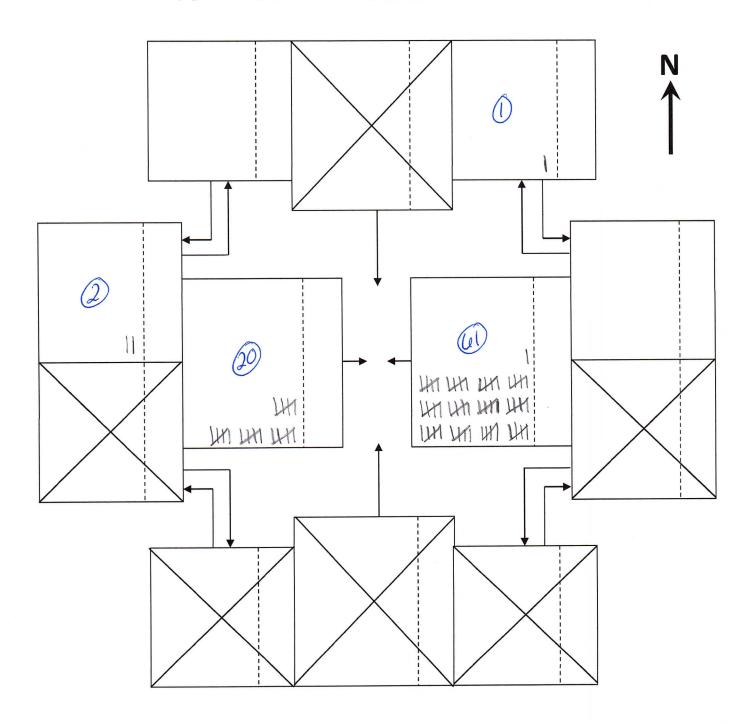
**N/S Street:** Cherry Springs Ranch Dr

E/W Street: Highway 105

Time: 9:00 to 9:15

**Date:** 3/14/23

# Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

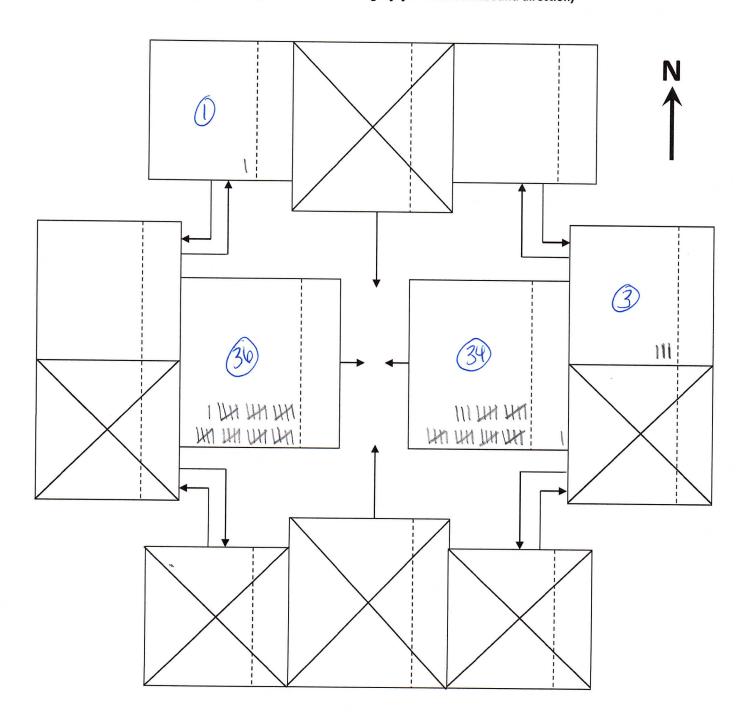
**N/S Street:** Cherry Springs Ranch Dr

E/W Street: Highway 105

Time: 9:15 to 9:30 **Date:** 3/14/23

Weather: Observer: Jennifer

# **Counts are Conducted From the Direction of Travel**



Four Approach Field Sheet

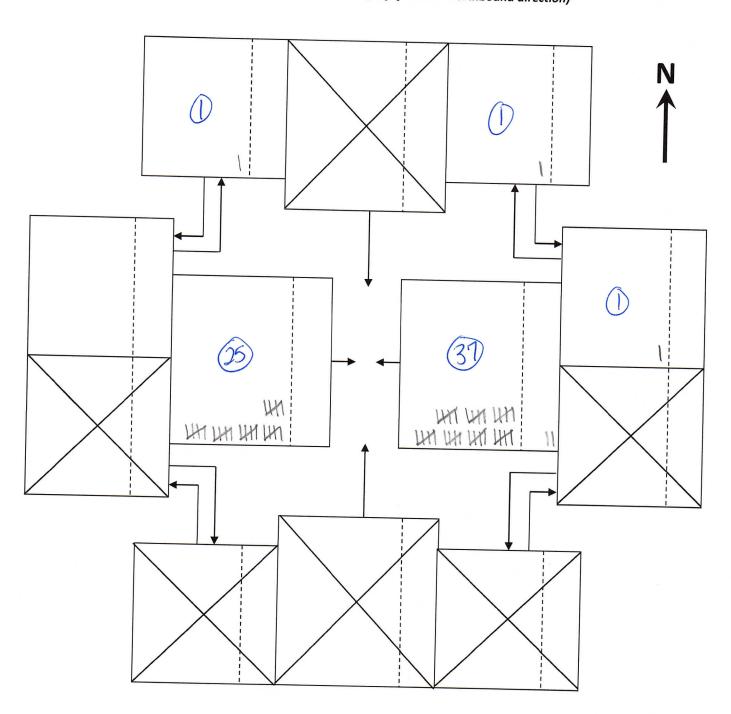
N/S Street: Cherry Springs Ranch Dr

E/W Street: Highway 105

Time: 9:30 to 9:45 **Date:** 3/14/23

Weather: Observer: Jernifes

# Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Cherry Springs Ranch Dr

E/W Street: Highway 105

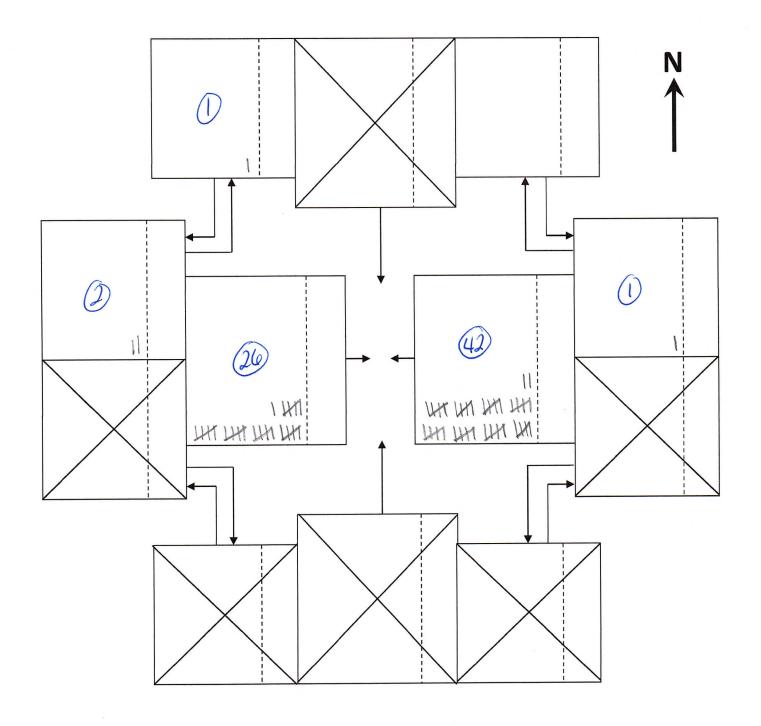
Time: 9:45 to 10:00

**Date:** 3/14/23

Weather:

Observer: Jennifer

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Cherry Springs Ranch Dr

E/W Street: Highway 105

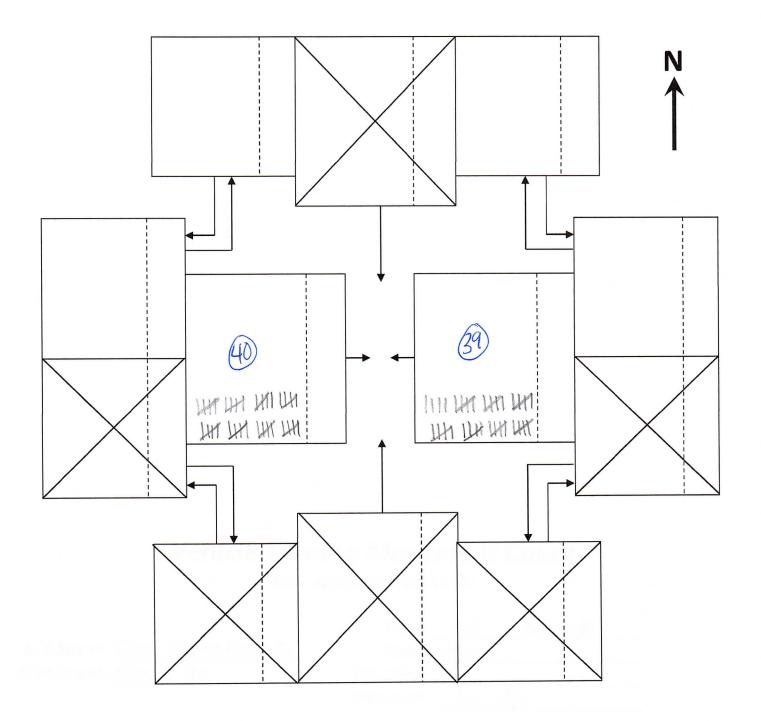
Time: 1:00 to 1:15

**Date:** 3/14/23

Weather:

Observer: Jennifer

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Cherry Springs Ranch Dr

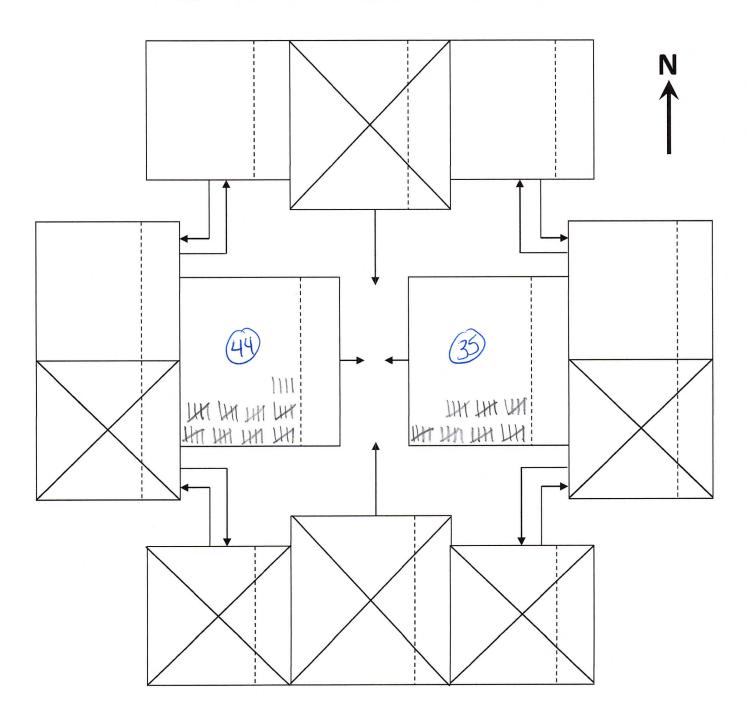
E/W Street: Highway 105

Time: 1:15 to 1:30

**Date:** 3/14/23

Weather:
Observer: Leniter

## Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Cherry Springs Ranch Dr

E/W Street: Highway 105

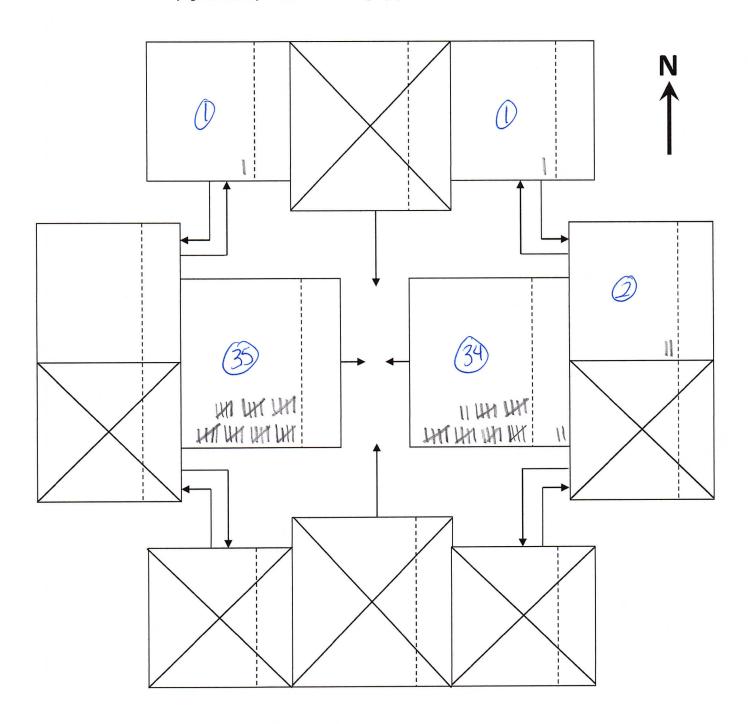
Time: 1:30 to 1:45

**Date:** 3/14/23

Weather:

Observer: Jerriter

### Counts are Conducted From the Direction of Travel



Four Approach Field Sheet

N/S Street: Cherry Springs Ranch Dr

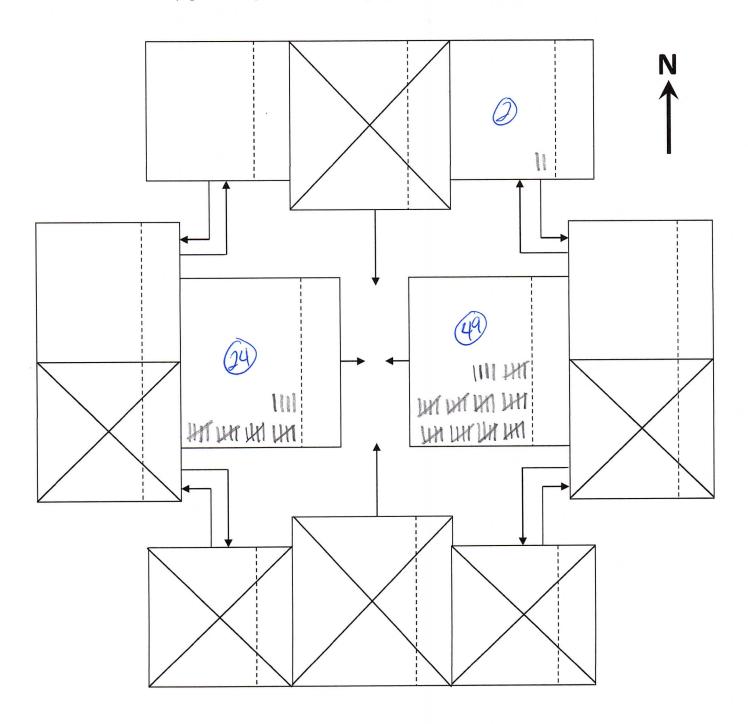
E/W Street: Highway 105

Time: <u>/:45</u> to <u>/:00</u>

Date: 3/14/23

Weather:
Observer: Jenifel

## Counts are Conducted From the Direction of Travel

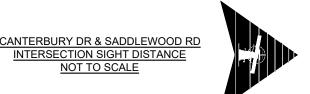


# INTERSECTION SIGHT DISTANCE EXHIBIT









\*EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM) REQUIRES THAT THE INTERSECTION SIGHT DISTANCE BE BASED ON THE ROADWAY DESIGN SPEED. HOWEVER, TABLE 2-21, IN THE ECM, DOES NOT PROVIDE AN INTERSECTION SIGHT DISTANCE FOR A DESIGN SPEED OVER 50 MPH. THE DESIGN SPEED FOR HIGHWAY 105 IS 60 MPH. THEREFORE, THE DESIGN INTERSECTION SIGHT DISTANCE SHOWN ON THIS EXHIBIT IS BASED ON THE HIGHEST ROADWAY DESIGN SPEED SHOWN IN THE TABLE. TABLE 2-1 ALSO STATES THAT THE VALUES ARE ONLY APPLICABLE TO TWO-LANE ROADS WITH STOP CONTROL, WHICH IS THE SCENARIO AT ALL INTERSECTIONS ANALYZED.

# RADO PUMPKIN PATCH

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Kansas City (913) 444-9615 Colorado Springs, CO (719) 465-2145

COLORADO KID'S RANCH SPECIAL USE A

PROJECT #: 2211-0442
CHECKED BY: BML
DRAWN BY: EDM

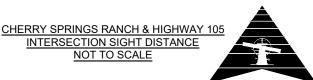
DATE: 7/17/2023

SHEET #

1

TOTAL SHEETS









\*EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM) REQUIRES THAT THE INTERSECTION SIGHT DISTANCE BE BASED ON THE ROADWAY DESIGN SPEED. HOWEVER, TABLE 2-21, IN THE ECM, DOES NOT PROVIDE AN INTERSECTION SIGHT DISTANCE FOR A DESIGN SPEED OVER 50 MPH. THE DESIGN SPEED FOR HIGHWAY 105 IS 60 MPH. THEREFORE, THE DESIGN INTERSECTION SIGHT DISTANCE SHOWN ON THIS EXHIBIT IS BASED ON THE HIGHEST ROADWAY DESIGN SPEED SHOWN IN THE TABLE. TABLE 2-1 ALSO STATES THAT THE VALUES ARE ONLY APPLICABLE TO TWO-LANE ROADS WITH STOP CONTROL, WHICH IS THE SCENARIO AT ALL INTERSECTIONS ANALYZED.

COLORADO KID'S RANCH SPECIAL USE A

DATE COMPANDES

SHEET # **2** 

7/17/2023

PROJECT #: 2211-0442 CHECKED BY: BML DRAWN BY: EDM

TOTAL SHEETS

DATE:

# EXISTING PEAK HOUR CALCULATIONS



9:30 to 9:45 am 9:45 to 10:00 am	68 74			1:30 to 1:45 pm 96 1:45 to 2:00 pm 75		
5.45 to 10.00 alli	/4			1.43 to 2.00 pm 73		
Canterbury Dr. &		Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &
Saddlewood Rd.		Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.
12-17-22		12-17-22	12-17-22	12-17-22	12-17-22	12-17-22
Time Period Volu	me	Time Period Volume	Peak Hour Factor	Time Period Volume	Time Period Volume	Peak Hour Factor
9:00 to 9:15 am	1	9:00 to 10:00 am 9	= 0.563	1:00 to 1:15 pm 2	1:00 to 2:00 pm 11	= 0.550

Canterbury Dr. & 105

12-17-22

Volume

80

71

3 5

1

Time Period

1:00 to 1:15 pm

1:15 to 1:30 pm

1:15 to 1:30 pm

1:30 to 1:45 pm

1:45 to 2:00 pm

Canterbury Dr. & 105

12-17-22

Volume

Time Period

1:00 to 2:00 pm

Canterbury Dr. &

105

Peak Hour Factor

= 0.839

Canterbury Dr. & 105

12-17-22

Peak Hour Factor

= 0.807

Canterbury Dr. & 105

12-17-22

Volume

54

43

3

1

4

Time Period

9:00 to 9:15 am

9:15 to 9:30 am

9:15 to 9:30 am

9:30 to 9:45 am

9:45 to 10:00 am

Canterbury Dr. & 105

12-17-22

Volume

239

Time Period

9:00 to 10:00 am

9:30 to 9:45 am	82			1:30 to 1:45 pm 86		
9:45 to 10:00 am	68			1:45 to 2:00 pm 95		
Cantarhum Dr 0		Contachum, Dr. 9	Contorbury Dr. 9	Contorbury Dr. 9	Contorbury Dr. 9	Contorbury Dr. 9
Canterbury Dr. &		Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &	Canterbury Dr. &
Saddlewood Rd.		Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.	Saddlewood Rd.
12-21-22		12-21-22	12-21-22	12-21-22	12-21-22	12-17-22
Time Period Vol	lume	Time Period Volume	Peak Hour Factor	Time Period Volume	Time Period Volume	Peak Hour Factor
9:00 to 9:15 am	1	9:00 to 10:00 am 4	= 0.500	1:00 to 1:15 pm 3	1:00 to 2:00 pm 18	= 0.500
9:15 to 9:30 am	1			1:15 to 1:30 pm 9		

Canterbury Dr. & 105

12-21-22

Volume

78

100

2

Time Period

1:00 to 1:15 pm

1:15 to 1:30 pm

1:30 to 1:45 pm

1:45 to 2:00 pm

Canterbury Dr. & 105

12-21-22

Volume

Time Period

1:00 to 2:00 pm

Canterbury Dr. &

105

Peak Hour Factor

= 0.898

Canterbury Dr. & 105

12-21-22

Peak Hour Factor

= 0.931

Canterbury Dr. & 105

12-21-22

Volume

83

76

2

0

Time Period

9:00 to 9:15 am

9:15 to 9:30 am

9:30 to 9:45 am

9:45 to 10:00 am

Canterbury Dr. & 105

12-21-22

Volume

Time Period

9:00 to 10:00 am

03-18-	-23	03-18-23	03-18-23	03-18-23	03-18-23	03-18-23
Time Period	Volume	Time Period Volume	Peak Hour Factor	Time Period Volume	Time Period Volume	Peak Hour Factor
9:00 to 9:15 am	62	9:00 to 10:00 am 295	= 0.934	1:00 to 1:15 pm 78	1:00 to 2:00 pm 297	= 0.917
9:15 to 9:30 am	78			1:15 to 1:30 pm 68		
9:30 to 9:45 am	76			1:30 to 1:45 pm 70		
9:45 to 10:00 am	79			1:45 to 2:00 pm 81		
105 & Appa		105 & Appaloosa Rd	105 & Appaloosa Rd	105 & Appaloosa Rd	105 & Appaloosa Rd	105 & Appaloosa Rd
03-18-	-23	03-18-23	03-18-23	03-18-23	03-18-23	03-18-23
Time Period 9:00 to 9:15 am 9:15 to 9:30 am 9:30 to 9:45 am 9:45 to 10:00 am	Volume 61 69 74	Time Period Volume 9:00 to 10:00 am 282	Peak Hour Factor = 0.904	Time Period Volume 1:00 to 1:15 pm 77 1:15 to 1:30 pm 67 1:30 to 1:45 pm 72	Time Period Volume 1:00 to 2:00 pm 299	Peak Hour Factor = 0.901

105 & Cherry Springs Ranch

9:15 to 9:30 am 9:30 to 9:45 am	74 65	3.00 to 10.00 dill 233	- 0.070	1:15 to 1:30 pm 1:30 to 1:45 pm	79 73	1.00 to 2.00 pm 300	- 0.500
9:45 to 10:00 am	72			1:45 to 2:00 pm	75		
105 & Appaloosa	Rd	105 & Appaloosa Rd	105 & Appaloosa Rd	105 & Appaloosa	Rd	105 & Appaloosa Rd	105 & Appaloosa Rd
03-14-23		03-14-23	03-14-23	03-14-23		03-14-23	03-14-23
Time Period Vol	ume	Time Period Volume	Peak Hour Factor	Time Period Vol	lume	Time Period Volume	Peak Hour Factor
9:00 to 9:15 am	84	9:00 to 10:00 am 293	= 0.872	1:00 to 1:15 pm	79	1:00 to 2:00 pm 314	= 0.981
9:15 to 9:30 am	72			1:15 to 1:30 pm	79		
9:30 to 9:45 am	68			1:30 to 1:45 pm	76		
	69						

105 & Cherry Springs Ranch

03-14-23

Volume

79

Time Period

1:00 to 1:15 pm

105 & Cherry Springs Ranch

03-14-23

Time Period Volume

1:00 to 2:00 pm

105 & Cherry Springs Ranch

03-14-23

Peak Hour Factor

0.968

105 & Cherry Springs Ranch

03-14-23

Peak Hour Factor

= 0.878

105 & Cherry Springs Ranch

03-14-23

Time Period

9:00 to 9:15 am

Volume

84

105 & Cherry Springs Ranch

03-14-23

Time Period Volume

9:00 to 10:00 am

# EXISTING PEAK HOUR TURNING MOVEMENTS



	Canterbury/Hwy 105 Peak Hour: 9:00 am to 10:00 am Existing Weekend								
			105						
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	116	1	1	114	0				
		Cante	rbury						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	0	0	0	0	7				

Daak H	our Volu	ma:	220

	Canterbury/Saddlewood								
	Peak	Hour: 9:00	am to 10:0	0 am					
		Existing '	Weekend						
		Saddle	ewood						
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	0	0	0	0	3				
		Cante	erbury						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	2	2	1	1	0				

Deel Herr	- M-L	-

Appaloosa/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Existing \	Weekday				
		Hwy	105				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	107	2	1	177	0		
		Appa	loosa				
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0 0 0 3 0 3							
Peak Hou	r Volume:	293					

	Cherry Springs Ranch/Hwy 105							
	Peak	Hour: 9:00	am to 10:00	0 am				
		Existing 1	Weekday					
		Hwy	105					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT			
4	107	0	0	174	5			
		Cherry Spr	ings Ranch					
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT			
3	0	2	0	0	0			
	17.1	205						

Canterbury/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Existing \	Weekday				
		Hwy	105				
EB LT EB Thru EB RT WB LT WB Thru WB RT					WB RT		
0	144	2	0	157	0		
	Canterbury						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0	0 0 0 3 0 3						
		200					

Peak Hour Volume:

	Canterbury/Saddlewood										
	Peak	Hour: 9:00	am to 10:0	0 am							
		Existing 1	Weekday								
		Saddle	ewood								
EB LT	EB Thru	EB RT	WBLT	WB Thru	WB RT						
0	0	0	1	0	1						
		Cante	erbury								
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT										
0 0 2 0 0 0											
Dook Hou	r Volumo:	4			Book Hour Volume: 4						

Appaloosa/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Existing 1	Weekend				
		Hwy	105				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	106	0	1	170	0		
		Appa	loosa				
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0 0 0 1 0 4							
Peak Hour Volume: 282							

Cherry Springs Ranch/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Existing \	Weekend				
		Hwy	105				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
7	103	0	0	162	13		
		Cherry Spr	ings Ranch				
SB RT SB Thru SB LT NB RT NB Thru NB LT							
6 0 4 0 0 0							
Peak Hour Volume: 295							

Canterbury/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm Existing Weekend							
Hwy 105							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	161	3	0	148	0		
Canterbury							
SB RT SB Thru SB LT NB RT NB Thru NB LT							
0	0	0	3	0	7		

Peak Hour Volume:

	Canterbury/Saddlewood								
	Peal	k Hour: 1:00	pm to 2:00	) pm					
		Existing '	Weekend						
		Saddle	ewood						
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	0	0	0	0	1				
		Cante	rbury						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT								
0 3 0 1 6 0									

Peak Hour Volume:

		Appaloosa	Hwy 105				
	Peal	k Hour: 1:00	pm to 2:00	) pm			
		Existing '	Weekday				
			105				
EB LT	EBLT EBThru EBRT WBLT WBThru WBRT						
0	145	2	5	155	0		
	143	^		133			
		Appa	10029				
SB RT SB Thru SB LT NB RT NB Thru NB LT							
0 0 0 4 0 3							
Bardi Harra Valuraria 214							

	Cherry Springs Ranch/Hwy 105								
	Peal	k Hour: 1:00	pm to 2:00	pm					
		Existing	Weekday						
		Hwy							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	143	0	0	157	2				
Cherry Springs Ranch									
SB RT SB Thru SB LT NB RT NB Thru NB LT									
1	0	1 0 3 0 0 0							

Peak Hour Volume:

Canterbury/Hwy 105								
Peak Hour: 1:00 pm to 2:00 pm								
		Existing \	Weekday					
		Hwy	105					
EB LT EB Thru EB RT WB LT WB Thru WB RT								
0	197	7	2	144	0			
Canterbury								
SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	0	0	2	0	7			

Peak Hour Volume: 359

	(	Canterbury/	Saddlewoo	d			
	Peal	k Hour: 1:00	) pm to 2:00	) pm			
		Existing \	Weekday				
		Saddle	ewood				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	0	0	3	0	5		
	Canterbury						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT						
0	3	2	3	2	0		

Peak Hour Volume:

Appaloosa/Hwy 105								
	Pea	k Hour: 1:00		) pm				
		Existing \	Weekend					
		Hwy	105					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT			
0	161	4	3	130	0			
		Appa	loosa					
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT							
0 0 0 1 0 0								
Deak Hou	r Volumo:	200						

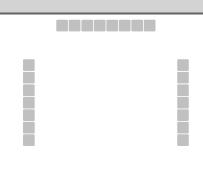
Cherry Springs Ranch/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm							
	Pear	K HOUT: 1:00	pm to 2:00	pm			
		Existing \	Weekend				
		Hwy	105				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	166	0	0	126	2		
	Cherry Springs Ranch						
SB RT SB Thru SB LT NB RT NB Thru NB LT							
2 0 1 0 0 0							
0 1 11	B 111 111 121						

Peak Hour Volume:

# EXISTING LEVEL OF SERVICE (LOS)



HCS7 Two-Way Stop-Control Report							
General Information		Site Information					
Analyst	Brett Louk	Intersection	Canterbury & Saddlewood				
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County				
Date Performed	4/4/2023	East/West Street	Saddlewood				
Analysis Year	2023	North/South Street	Canterbury				
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.85				
Intersection Orientation	Analysis Time Period (hrs)	0.25					
Project Description	Colorado Pumpkin Patch Temporary Use TIS						



Major Street: North-South

Vehicle Volumes and Adj	ustme	nts														
Approach		Eastk	oound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						1		1		0	0	0		2	0	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice	•												
Flow Rate, v (veh/h)	T						2			0				2		
Capacity, c (veh/h)							1049			1623				1623		
v/c Ratio							0.00			0.00				0.00		
95% Queue Length, Q <sub>95</sub> (veh)							0.0			0.0				0.0		
Control Delay (s/veh)							8.4			7.2				7.2		
Level of Service (LOS)							А			А				А		
	_															

Approach Delay (s/veh)

Approach LOS

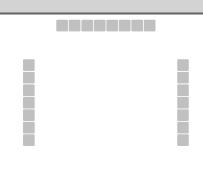
8.4

Α

7.2

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HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	Brett Louk	Intersection	Canterbury & Saddlewood									
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County									
Date Performed	4/4/2023	East/West Street	Saddlewood									
Analysis Year	2023	North/South Street	Canterbury									
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.85									
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25									
Project Description	Colorado Pumpkin Patch Temporary Use TIS											



Major Street: North-South

Vehicle Volumes and Adju	stments
Approach	Ea

Approach	Eastbound Westbound					Northbound				Southbound						
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						3		5		0	2	3		2	3	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)							0									
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							9			0				2		
Capacity, c (veh/h)							1050			1618				1615		
v/c Ratio							0.01			0.00				0.00		
95% Queue Length, Q <sub>95</sub> (veh)							0.0			0.0				0.0		
Control Delay (s/veh)							8.5			7.2				7.2		
Level of Service (LOS)							Α			Α				Α		

Approach Delay (s/veh)

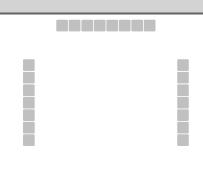
Approach LOS

8.5

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2.9

HCS7 Two-Way Stop-Control Report												
General Information												
Analyst	Brett Louk	Intersection	Canterbury & Saddlewood									
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County									
Date Performed	4/4/2023	East/West Street	Saddlewood									
Analysis Year	2023	North/South Street	Canterbury									
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85									
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25									
Project Description	Colorado Pumpkin Patch Temporary Use TIS											



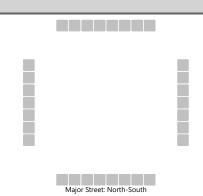
Major Street: North-South

### **Vehicle Volumes and Adjustments**

Approach		Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						0		3		0	1	1		2	2	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Headways																
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)							4			0				2		
Capacity, c (veh/h)							1083			1620				1620		
v/c Ratio							0.00			0.00				0.00		
95% Queue Length, Q <sub>95</sub> (veh)							0.0			0.0				0.0		
Control Delay (s/veh)							8.3			7.2				7.2		
Level of Service (LOS)							А			А				А		
Approach Delay (s/veh)					8.3		0.0			3.6						
Approach LOS					A											

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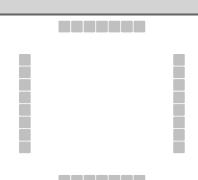
HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	Brett Louk	Intersection	Canterbury & Saddlewood									
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County									
Date Performed	4/4/2023	East/West Street	Saddlewood									
Analysis Year	2023	North/South Street	Canterbury									
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85									
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25									
Project Description	Colorado Pumpkin Patch Temporary Use TIS											



Approach		Eastb	ound			West	bound		Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						0		1		0	6	1		0	3	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)							0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	Т					7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T						1			0				0		
Capacity, c (veh/h)							1075			1618				1612		
v/c Ratio							0.00			0.00				0.00		
95% Queue Length, Q <sub>95</sub> (veh)							0.0			0.0				0.0		
Control Delay (s/veh)							8.4			7.2				7.2		
Level of Service (LOS)							А			А				А		
Approach Delay (s/veh)	8.4						0.0				0.0					
Approach LOS	1	A														

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HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	Brett Louk	Intersection	Canterbury & Hwy 105 AM									
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County									
Date Performed	4/4/2023	East/West Street	Hwy 105									
Analysis Year	2023	North/South Street	Canterbury									
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.93									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	Colorado Pumpkin Patch Temporary Use TIS											



Major Street: East-West

venicie vo	oiumes	and	Aajustm	ents
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Approach		Eastb	ound		Westbound				Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	144	2		0	157	0		3		3				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				

Base Critical Headway (sec)		4.1				4.1				7.1		6.2		
Critical Headway (sec)		4.12				4.12				7.12		6.22		
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3		
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32		
Delay, Queue Length, and Level of Service														
Flow Rate, v (veh/h)		0				0					6			
Capacity, c (veh/h)		1409				1423					737			
v/c Ratio		0.00				0.00					0.01			
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0			
Control Delay (s/veh)		7.6				7.5					9.9			

Α

0.0

Level of Service (LOS)

Approach LOS

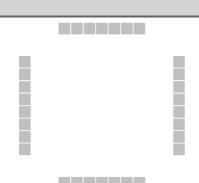
Approach Delay (s/veh)

0.0

9.9

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HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Canterbury & Hwy 105 PM						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.90						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Pumpkn Patch Temporary Use TIS								



Major Street: East-West

venicie volumes	and Adjustments
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Approach		Eastbound				Westl	oound		Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	197	7		2	144	0		7		2					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	0						
Right Turn Channelized																	
Median Type   Storage		Undivided															
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1				4.1				7.1		6.2					

•		<u> </u>									
Base Critical Headway (sec)		4.1			4.1		7.1		6.2		
Critical Headway (sec)		4.12			4.12		7.12		6.22		
Base Follow-Up Headway (sec)		2.2			2.2		3.5		3.3		
Follow-Up Headway (sec)		2.22			2.22		3.52		3.32		
Delay, Queue Length, and	l Leve	l of Se	ervice								
Flow Rate, v (veh/h)		0			2			10			
Capacity, c (veh/h)		1419			1342			612			
v/c Ratio		0.00			0.00			0.02			
95% Queue Length, Q <sub>95</sub> (veh)		0.0			0.0			0.0			
Control Delay (s/veh)		7.5			7.7			11.0			

0.0

Level of Service (LOS)

Approach LOS

Approach Delay (s/veh)

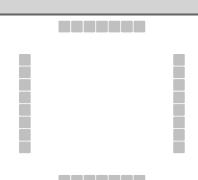
0.1

В

11.0

В

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Canterbury & Hwy 105 AM						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	9:00-10:00 AM Weekend	Peak Hour Factor	0.85						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Pumpkin Patch Temporary Use TIS								



Major Street: East-West

Approach		Eastbound				Westk	oound		Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	116	1		1	114	0		7		0					
Percent Heavy Vehicles (%)		2				2			2	2		2					
Proportion Time Blocked																	
Percent Grade (%)										0							
Right Turn Channelized																	
Median Type   Storage		Undivided															
Critical and Follow-up He	adwa	ys															

· · · · · · · · · · · · · · · · · · ·														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2		
Critical Headway (sec)		4.12				4.12				7.12		6.22		
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3		
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32		
Delay, Queue Length, and	l Level	of Se	ervice											
Flow Rate, v (veh/h)		0				1					8			
Capacity, c (veh/h)		1450				1446					678			

0.00

0.0

7.5

0.1

Approach LOS	
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0.0

7.5

Α

0.0

v/c Ratio

95% Queue Length,  $Q_{95}$  (veh)

Control Delay (s/veh)

Level of Service (LOS)

Approach Delay (s/veh)

0.01

0.0

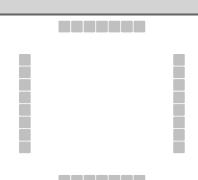
10.4

В

10.4

В

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Canterbury & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Pumpkin Patch Temporary Use TIS								



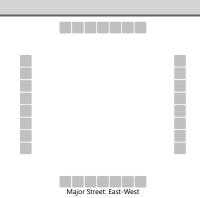
Major Street: East-West

venicie volumes and Adjustment	τs	
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Approach		Eastb	ound			Westk	oound		Northbound					South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	161	3		0	148	0		7		3				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				0					12					
Capacity, c (veh/h)		1402 1380									650					
	1		-		+	-									-	

Delay, Quede Length, and	Level Oi	Jei vice	•									
Flow Rate, v (veh/h)	С				0				12			
Capacity, c (veh/h)	14	)2			1380				650			
v/c Ratio	0.0	0			0.00				0.02			
95% Queue Length, Q <sub>95</sub> (veh)	0.	)			0.0				0.1			
Control Delay (s/veh)	7.	5			7.6				10.6			
Level of Service (LOS)	A				Α				В			
Approach Delay (s/veh)		0.0		0.0				10	).6			
Approach LOS								[	3			

HCS7 Two-Way Stop-Control Report													
General Information		Site Information											
Analyst	Brett Louk	Intersection	Appaloosa & Hwy 105										
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County										
Date Performed	4/4/2023	East/West Street	Hwy 105										
Analysis Year	2023	North/South Street	Appaloosa										
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.87										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	Colorado Pumpkin Patch Temporary Use TIS												



### **Vehicle Volumes and Adjustments**

Approach	Eastbound Westbound								North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	107	2		1	177	0		3		3				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up He	eadwa	dways														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	T	0				1					7					
Capacity, c (veh/h)		1368				1461					745					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.6				7.5					9.9					
Level of Service (LOS)		A									А					

0.0

Approach Delay (s/veh)

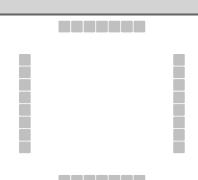
Approach LOS

0.0

9.9

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	HCS7 Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	Brett Louk	Intersection	Appaloosa & Hwy 105										
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County										
Date Performed	4/4/2023	East/West Street	Hwy 105										
Analysis Year	2023	North/South Street	Appaloosa										
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.98										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	Colorado Pumpkin Patch Temporary Use TIS												



Major Street: East-West

Vehicle Vo	olumes	and A	Adjus	tments	;
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Approach		Eastb	Eastbound				oound			North	bound			Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	145	2		5	155	0		3		4					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adways																

		, -														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				5					7					
Capacity, c (veh/h)		1421				1431					762					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.5				7.5					9.8					
	7	1	1	9	1	1	9	1	1	9	1	T .	1	1	7	1

Α

0.0

Level of Service (LOS)

Approach LOS

Approach Delay (s/veh)

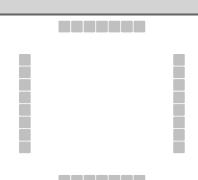
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Α

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Brett Louk	Intersection	Appaloosa & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	4/4/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Appaloosa
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.90
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Pumpkin Patch Temporary Use TIS		



Major Street: East-West

venicie volumes and Adjustment	τs	
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Approach		Eastb	ound		Westbound					Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	106	0		1	170	0		4		1					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1		6.2					

	4.1				4.1				7.1		6.2				
	4.12				4.12				7.12		6.22				
	2.2				2.2				3.5		3.3				
	2.22				2.22				3.52		3.32				
Leve	l of Se	ervice													
	0				1					6					
	1385				1470					686					
	0.00				0.00					0.01					
	0.0				0.0					0.0					
	7.6				7.5					10.3					
	Leve	4.12 2.2 2.22 <b>Level of Se</b> 0 1385 0.00 0.0	4.12 2.2 2.22 Level of Service 0 1385 0.00 0.0	4.12   2.2   2.22	4.12   2.2   2.22	4.12   4.12   2.2   2.2   2.22	4.12   4.12   2.2   2.2   2.22	4.12   4.12	4.12   4.12	4.12	4.12	4.12	4.12	4.12	4.12

0.0

Level of Service (LOS)

Approach LOS

Approach Delay (s/veh)

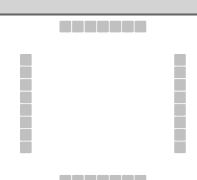
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В

10.3

В

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Appaloosa & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Appaloosa						
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.90						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Pumpkin Patch Temporary Use TIS									



Major Street: East-West

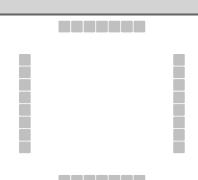
Vehicle Vo	lumes a	ind Adj	ustments
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Approach		Eastb	ound			Westl	oound		Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	161	4		3	130	0		0		1					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1				4.1				7.1		6.2					
Critical Headway (sec)		4.12				4.12				7.12		6.22					
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3					
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32					
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)		0				3					1						
Capacity, c (veh/h)		1438				1392					862						
v/c Ratio		0.00				0.00					0.00						
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0						
Control Delay (s/veh)		7.5				7.6					9.2						
Level of Service (LOS)		А				А					Α						
Approach Delay (s/veh)		0.0 0.2				.2		9.2									

Approach LOS

Α

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Cherry Spr R & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Cherry Springs Ranch						
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.88						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Pumpkin Patch Temporary Use TIS									



Major Street: East-West

Vehicle Volume	es and	Adjust	tments
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Approach		Eastb	ound		Westbound			Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LR	
Volume (veh/h)		4	107	0		0	174	5						2		3
Percent Heavy Vehicles (%)		2				2								2		2
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized																
Median Type   Storage		Undivided														

### **Critical and Follow-up Headways**

95% Queue Length,  $Q_{95}$  (veh)

Control Delay (s/veh)

Level of Service (LOS)

Approach Delay (s/veh)

Approach LOS

Base Critical Headway (sec)		4.1				4.1							7.1		6.2
Critical Headway (sec)		4.12				4.12							7.12		6.22
Base Follow-Up Headway (sec)		2.2				2.2							3.5		3.3
Follow-Up Headway (sec)		2.22				2.22							3.52		3.32
Delay, Queue Length, and	Delay, Queue Length, and Level of Service														
Flow Rate, v (veh/h)		5				0								6	
Capacity, c (veh/h)		1368				1466								736	
v/c Ratio		0.00				0.00								0.01	

0.0

7.5

0.0

0.0

7.6

Α

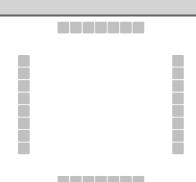
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HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Cherry Spr R & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Cherry Springs Ranch						
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.97						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	ect Description Colorado Pumpkin Patch Temporary Use TIS								



Major Street: East-West

venicie	volumes	and	Adjustments
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Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LR	
Volume (veh/h)		0	143	0		0	157	2						3		1
Percent Heavy Vehicles (%)		2				2								2		2
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up Headways																

Base Critical Headway (sec)		4.1				4.1						7.1		6.2
Critical Headway (sec)		4.12				4.12						7.12		6.22
Base Follow-Up Headway (sec)		2.2				2.2						3.5		3.3
Follow-Up Headway (sec)		2.22				2.22						3.52		3.32
Delay, Queue Length, and Level of Service														
Flow Rate, v (veh/h)		0				0							4	
Capacity, c (veh/h)		1415				1434							689	
v/c Ratio		0.00				0.00							0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0							0.0	

7.5

Α

0.0

Control Delay (s/veh)

Level of Service (LOS)

Approach LOS

Approach Delay (s/veh)

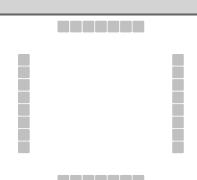
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В

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HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Brett Louk	Intersection	Cherry Spr R & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	4/4/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Cherry Springs Ranch						
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.93						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	t Description Colorado Pumpkin Patch Temporary Use TIS								



Major Street: East-West

venicie	voiumes	and A	ajustments
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Approach		Eastb	ound			Westl	oound			North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0	
Configuration			LTR				LTR								LR		
Volume (veh/h)		7	103	0		0	162	13						4		6	
Percent Heavy Vehicles (%)		2				2								2		2	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	vs															

95% Queue Length,  $Q_{95}$  (veh)

Control Delay (s/veh)

Level of Service (LOS)

Approach Delay (s/veh)

Approach LOS

, , ,																
Base Critical Headway (sec)		4.1				4.1								7.1		6.2
Critical Headway (sec)		4.12				4.12								7.12		6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5		3.3
Follow-Up Headway (sec)		2.22				2.22								3.52		3.32
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)		8				0									11	
Capacity, c (veh/h)		1386				1479									758	
v/c Ratio		0.01				0.00									0.01	

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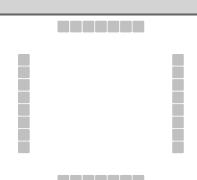
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HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	Brett Louk	Intersection	Cherry Spr R & Hwy 105									
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County									
Date Performed	4/4/2023	East/West Street	Hwy 105									
Analysis Year	2023	North/South Street	Cherry Springs Ranch									
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	Colorado Pumpkin Patch Temporary Use TIS											



Major Street: East-West

Vehicle '	Volumes	and A	Adjust	tments
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Approach		ound		Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LR	
Volume (veh/h)		0	166	0		0	126	2						1		2
Percent Heavy Vehicles (%)		2				2								2		2
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up Headways																

<u> </u>															
Base Critical Headway (sec)		4.1				4.1							7.1		6.2
Critical Headway (sec)		4.12				4.12							7.12		6.22
Base Follow-Up Headway (sec)		2.2				2.2							3.5		3.3
Follow-Up Headway (sec)		2.22				2.22							3.52		3.32
Delay, Queue Length, and Level of Service															
Flow Rate, v (veh/h)		0				0								3	
Capacity, c (veh/h)		1444				1395								795	

0.00

0.0

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0.0

v/c Ratio

95% Queue Length,  $Q_{95}$  (veh)

Control Delay (s/veh)

Level of Service (LOS)

Approach Delay (s/veh)

Approach LOS

HCS<sup>™</sup> TWSC Version 7.6

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## TRIP DISTRIBUTION EXHIBIT



### Project Generated Weekday and Weekend Trip Distribution Percentages Exhibit



## EXISTING + DEVELOPMENT PEAK HOUR TURNING MOVEMENTS



Canterbury/Hwy 105						
	Peak	Hour: 9:00	am to 10:0	0 am		
	Existi	ng + Develo	pment We	ekend		
Hwy 105						
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT					
0	121	90	60	115	0	
Canterbury						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
0	0	0	6	0	16	

30 1/1	36 IIIIu	JD LI	NDINI	ND IIIIU	NDLI
0	0	0	6	0	16
Peak Hou	r Volume:	408	•	•	

Canterbury/Saddlewood										
	Peak	Hour: 9:00	am to 10:0	00 am						
	Existi	ng + Develo	pment We	ekend						
	Saddlewood									
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT									
0	0	0	0	0	18					
	Canterbury									
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT					
0	2	150	1	1	0					
Peak Hou	r Volume:	172			Peak Hour Volume: 172					

	Appaloosa/Hwy 105					
Peak Hour: 9:00 am to 10:00 am						
	Existi	ng + Develo	pment We	ekday		
		Hwy	105			
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB R	
0	112	6	4	228	0	
		Appa	loosa			
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
0	0	0	4	0	4	
Peak Hou	r Volume	358				

Cherry Springs Ranch/Hwy 105						
	Peak Hour: 9:00 am to 10:00 am					
	Existi	ng + Develo	pment We	ekday		
		Hwy	105			
EB LT	BLT EB Thru EB RT WB LT WB Thru WB R1					
4	116	0	0	226	5	
	Cherry Springs Ranch					
SB RT SB Thru SB LT NB RT NB Thru NB LT						
3 0 2 0 0 0						
Peak Hou	r Volume:	356				

		Canterbur	y/Hwy 105			
	Peak	Hour: 9:00	am to 10:0	0 am		
	Existi	ng + Develo	pment We	ekday		
	Hwy 105					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT	
0	148	78	51	158	0	
	Canterbury					
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
0	0	0	8	0	11	

olume: 4
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454

Canterbury/Saddlewood							
	Peak Hour: 9:00 am to 10:00 am						
	Existi	ng + Develo	pment We	ekday			
	Saddlewood						
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT						
0	0	0	1	0	14		
Canterbury							
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0	0	129	0	0	0		

Peak Hour Volume: 144

Appaloosa/Hwy 105						
	Peak	Hour: 9:00	am to 10:0	0 am		
	Existi	ng + Develo	pment We	ekend		
	Hwy 105					
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT					
0	112	5	4	229	0	
Appaloosa						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
0	0	0	2	0	5	

Peak Hour Volume:

	Cherry Springs Ranch/Hwy 105					
	Peak	Hour: 9:00	am to 10:0	0 am		
	Existing + Development Weekend					
	Hwy 105					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT	
7	114	0	0	222	13	
		Cherry Spr	ings Ranch			
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
6	0	4	0	0	0	
B 1 11	2.11. 11.1					

Canterbury/Hwy 105
Peak Hour: 1:00 pm to 2:00 pm
Existing + Development Weekend
Hwy 105

HWY 105									
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	166	97	63	150	0				
		Cante	rbury						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT				
0	0	0	30	0	48				

Peak Hour Volume:

Canterbury/Saddlewood									
Peak Hour: 1:00 pm to 2:00 pm									
	Existi	ng + Develo	pment We	ekend					
		Saddle	ewood						
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	0	0	0	0	69				
		Cante	rbury						
SB RT SB Thru SB LT NB RT NB Thru NB LT									
0	3	157	1	6	0				

Canterbury/Saddlewood Peak Hour: 1:00 pm to 2:00 pm Existing + Development Weekday Saddlewood

Peak Hour Volume: 236

Appaloosa/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm Existing + Development Weekday									
		Hwy	105						
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
0	152	3	6	171	0				
		Appa	loosa						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	0	0	5	0	4				

Peak Hour Volume:

	Cherry Springs Ranch/Hwy 105									
	Peak Hour: 1:00 pm to 2:00 pm									
	Existi	ng + Develo	pment We	ekday						
		Hwy	105							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	151	0	0	174	2					
		Cherry Spr	ings Ranch							
SB RT SB Thru SB LT NB RT NB Thru NB LT										
1	0	3	0	0	0					

Peak Hour Volume:

	Canterbury/Hwy 105										
Peak Hour: 1:00 pm to 2:00 pm											
	Existi	ng + Develo	pment We	ekday							
		Hwy	105								
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT						
0	198	32	18	145	0						
Canterbury											
SB RT SB Thru SB LT NB RT NB Thru NB LT											

Peak Hour Volume:

419

 
 EB LT
 EB Thru
 EB RT
 WB LT
 WB Thru
 WB RT

 0
 0
 0
 3
 0
 22
 Canterbury SB RT SB Thru SB LT NB RT NB Thru NB LT 0 3 43 3 2 0

Peak Hour Volume:

Appaloosa/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm Existing + Development Weekend									
EB LT	Hwy 105  EB LT   EB Thru   EB RT   WB LT   WB Thru   WB RT								
0	188	9	6	193	0				
		Appa	loosa						
SB RT	SB Thru SB LT NB RT NB Thru NB LT								
0	0	0	2	0	2				

Peak Hour Volume: 400

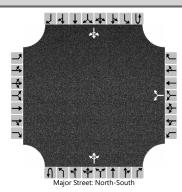
21 2 1 2 1 1 2 2										
	Cherry Springs Ranch/Hwy 105									
	Peal	Hour: 1:00	pm to 2:00	) pm						
	Existi	ng + Develo	pment We	ekend						
		Hwy	105							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	198	0	0	191	2					
		Cherry Spr	ings Ranch							
SB RT	RT SB Thru SB LT NB RT NB Thru NB LT									
2	0	1	0	0	0					

Peak Hour Volume: 394

# EXISTING + DEVELOPMENT LEVEL OF SERVICE (LOS)

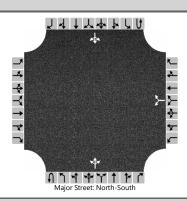


HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Saddlewood						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.85						
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Kids Ranch								



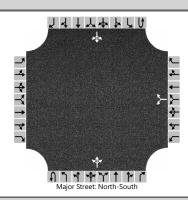
Approach	T	Eastbound			Westbound			Northbound			Southbound					
	_															
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						1		14		0	0	0		129	0	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	)									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							18			0				152		
Capacity, c (veh/h)							1030			1623				1623		
v/c Ratio							0.02			0.00				0.09		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.3		
Control Delay (s/veh)							8.6			7.2				7.4		
Level of Service (LOS)							А			Α				А		
Approach Delay (s/veh)						8	.6						7.4			
Approach LOS	_						4									

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Saddlewood						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.85						
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Kids Ranch								



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound		Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						3		22		0	2	3		43	3	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							29			0				51		
Capacity, c (veh/h)							1045			1618				1615		
v/c Ratio							0.03			0.00				0.03		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.1		
Control Delay (s/veh)							8.5			7.2				7.3		
Level of Service (LOS)							А			А				Α		
Approach Delay (s/veh)						8.5			0.0				6.8			
Approach LOS						A										

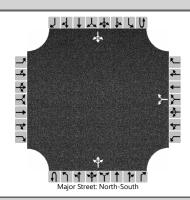
HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Saddlewood							
Analysis Year	2023	North/South Street	Canterbury							
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	oound			Westl	oound		Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						0		18		0	1	1		150	2	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							21			0				176		
Capacity, c (veh/h)							1083			1620				1620		
v/c Ratio							0.02			0.00				0.11		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.4		
Control Delay (s/veh)							8.4			7.2				7.5		
Level of Service (LOS)							А		A					А		
Approach Delay (s/veh)						8.4			0.0				7.4			
Approach LOS					А											

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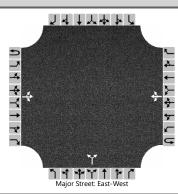
HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Saddlewood							
Analysis Year	2023	North/South Street	Canterbury							
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						0		69		0	6	1		157	3	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							81			0				185		
Capacity, c (veh/h)							1075			1618				1612		
v/c Ratio							0.08			0.00				0.11		
95% Queue Length, Q <sub>95</sub> (veh)							0.2			0.0				0.4		
Control Delay (s/veh)							8.6			7.2				7.5		
Level of Service (LOS)							А			Α				А		
Approach Delay (s/veh)		8.6						0.0				7.4				
Approach LOS		А				4										

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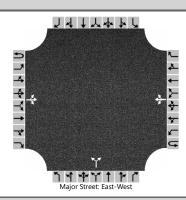
HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/13/2023	East/West Street	Hwy 105								
Analysis Year	2023	North/South Street	Canterbury								
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.93								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description Colorado Kids Ranch											



Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound		Westbound					North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	148	78		51	158	0		11		8				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										. (	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				55					20					
Capacity, c (veh/h)		1407				1323					584					
v/c Ratio		0.00				0.04					0.03					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1					0.1					
Control Delay (s/veh)		7.6				7.8					11.4					
Level of Service (LOS)		А			A			В					Ì	Ì		
Approach Delay (s/veh)		0.0 2.2							11.4							
Approach LOS						,	4				В					

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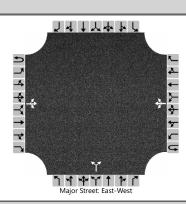
HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Canterbury							
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.90							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



Vehicle Volumes and Ad	justme	nts														
Approach	T	Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	198	32		18	145	0		17		9				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				20					29					
Capacity, c (veh/h)		1418				1309					593					
v/c Ratio		0.00				0.02					0.05					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.2					
Control Delay (s/veh)		7.5				7.8					11.4					
Level of Service (LOS)		А				Α					В					
Approach Delay (s/veh)		0.0 1.0				11.4							•			
Approach LOS					А			В								

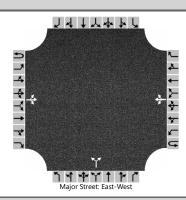
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HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/13/2023	East/West Street	Hwy 105								
Analysis Year	2023	North/South Street	Canterbury								
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description Colorado Kids Ranch											



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westk	oound			North	bound			Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	121	90		60	115	0		16		6					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up Ho	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1		6.2					
Critical Headway (sec)		4.12				4.12				7.12		6.22					
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3					
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32					
Delay, Queue Length, and	d Leve	l of S	ervice														
Flow Rate, v (veh/h)		0				71					26						
Capacity, c (veh/h)		1449				1317					544						
v/c Ratio		0.00				0.05					0.05						
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.1						
Control Delay (s/veh)		7.5				7.9					11.9						
Level of Service (LOS)		А				Α					В						
Approach Delay (s/veh)		0.0 3.0						11.9									
Approach LOS						А			В								

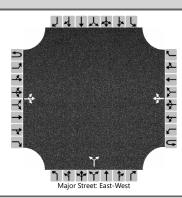
HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Canterbury							
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	166	97		63	150	0		48		30				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				74					92					
Capacity, c (veh/h)		1400				1251					499					
v/c Ratio		0.00				0.06					0.18					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.7					
Control Delay (s/veh)		7.6				8.1					13.8					
Level of Service (LOS)		А				Α					В					
Approach Delay (s/veh)		0.0			2.8			13.8						•		
Approach LOS								В								

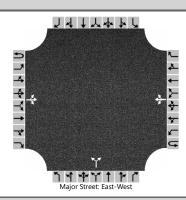
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HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.87							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



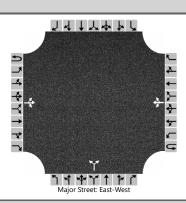
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	112	6		4	228	0		4		4				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)											)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	Τ	0				5					9					
Capacity, c (veh/h)		1302				1449					692					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.8				7.5					10.3					
Level of Service (LOS)		A				A			В							
Approach Delay (s/veh)		0.0 0.2					•	10.3								
Approach LOS						A										

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.98							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



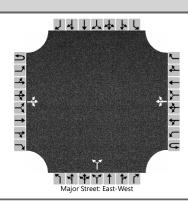
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	152	3		6	171	0		4		5				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				6					9					
Capacity, c (veh/h)		1402				1421					738					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.6				7.5					9.9					
Level of Service (LOS)		А				Α					Α					
Approach Delay (s/veh)		0.0			0.3			9.9					•			
Approach LOS					A			A								

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.90							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



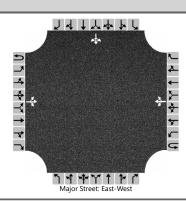
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	112	5		4	229	0		5		2				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				4					8					
Capacity, c (veh/h)		1311				1455					637					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.7				7.5					10.7					
Level of Service (LOS)		А				Α					В					
Approach Delay (s/veh)		0.0 0.2						10.7								
Approach LOS					A			В								

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.90							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



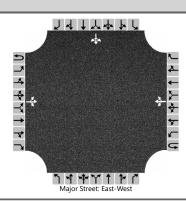
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	188	9		6	193	0		2		2				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				7					4					
Capacity, c (veh/h)		1356				1351					641					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.7				7.7					10.7					
Level of Service (LOS)		А				А					В					
Approach Delay (s/veh)	0.0 0.3					10.7										
Approach LOS									В							

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.88							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



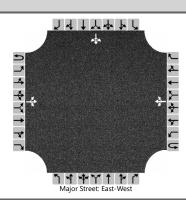
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		4	116	0		0	226	5						2	0	3
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)														(	)	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		5				0									6	
Capacity, c (veh/h)		1302				1453									673	
v/c Ratio		0.00				0.00									0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.8				7.5									10.4	
Level of Service (LOS)		A			A									В		
Approach Delay (s/veh)	0.3			0.0							10.4					
Approach LOS											В					

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.97							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



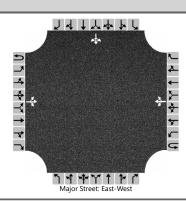
Vehicle Volumes and Adjust	stme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		0	151	0		0	174	2						3	0	1
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Hea	adwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				0									4	
Capacity, c (veh/h)		1394				1424									665	
v/c Ratio		0.00				0.00									0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.6				7.5									10.4	
Level of Service (LOS)		А				А									В	
Approach Delay (s/veh)	0.0					0	.0							10	).4	
Approach LOS									В							

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.93							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		7	114	0		0	222	13						4	0	6
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)											0					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		8				0									11	
Capacity, c (veh/h)		1313				1465									687	
v/c Ratio		0.01				0.00									0.02	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.8				7.5									10.3	
Level of Service (LOS)		А				Α									В	
Approach Delay (s/veh)		0	.5			0	.0	-						10	0.3	
Approach LOS										В						

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		0	198	0		0	191	2						1	0	2
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)											0					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				0									3	
Capacity, c (veh/h)		1361				1355									705	
v/c Ratio		0.00				0.00									0.00	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.6				7.7									10.1	
Level of Service (LOS)		А				А									В	
Approach Delay (s/veh)		0	.0			0	.0	•			•			10	0.1	
Approach LOS										В						

## FUTURE TRAFFIC GROWTH



### Short Range Growth (2023-2024)

Canterbury & Saddlewood Projected A.M. Weekday Peak Hour	Canterbury & Saddlewood Projected P.M. Weekday Peak Hour	Canterbury & Highway 105 Projected A.M. Weekday Peak Hour	Canterbury & Highway 105 Projected P.M. Weekday Peak Hour	Appaloosa & Highway 105 Projected A.M. Weekday Peak Hour	Appaloosa & Highway 105 Projected P.M. Weekday Peak Hour	Cherry Springs Ranch & Highway 105 Projected A.M. Weekday Peak Hour	Cherry Springs Ranch & Highway 105 Projected P.M. Weekday Peak Hour
Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV	Year Growth Expected PHV
2023 4	2023 18	2023 309	2023 359	2023 293	2023 314	2023 295	2023 306
2024 0.02 5	2024 0.02 19	2024 0.02 316	2024 0.02 367	2024 0.02 299	2024 0.02 321	2024 0.02 301	2024 0.02 313
2025 0.02 6	2025 0.02 20	2025 0.02 323	2025 0.02 375	2025 0.02 305	2025 0.02 328	2025 0.02 308	2025 0.02 320
2026 0.02 7	2026 0.02 21	2026 0.02 330	2026 0.02 383	2026 0.02 312	2026 0.02 335	2026 0.02 315	2026 0.02 327
2027 0.02 8	2027 0.02 22	2027 0.02 337	2027 0.02 391	2027 0.02 319	2027 0.02 342	2027 0.02 322	2027 0.02 334
2028 0.02 9	2028 0.02 23	2028 0.02 344	2028 0.02 399	2028 0.02 326	2028 0.02 349	2028 0.02 329	2028 0.02 341
2029 0.02 10	2029 0.02 24	2029 0.02 351	2029 0.02 407	2029 0.02 333	2029 0.02 356	2029 0.02 336	2029 0.02 348
2030 0.02 11	2030 0.02 25	2030 0.02 359	2030 0.02 416	2030 0.02 340	2030 0.02 364	2030 0.02 343	2030 0.02 355
2031 0.02 12	2031 0.02 26	2031 0.02 367	2031 0.02 425	2031 0.02 347	2031 0.02 372	2031 0.02 350	2031 0.02 363
2032 0.02 13	2032 0.02 27	2032 0.02 375	2032 0.02 434	2032 0.02 354	2032 0.02 380	2032 0.02 357	2032 0.02 371
2033 0.02 14	2033 0.02 28	2033 0.02 383	2033 0.02 443	2033 0.02 362	2033 0.02 388	2033 0.02 365	2033 0.02 379
2034 0.02 15	2034 0.02 29	2034 0.02 391	2034 0.02 452	2034 0.02 370	2034 0.02 396	2034 0.02 373	2034 0.02 387
2035 0.02 16	2035 0.02 30	2035 0.02 399	2035 0.02 462	2035 0.02 378	2035 0.02 404	2035 0.02 381	2035 0.02 395
2036 0.02 17	2036 0.02 31	2036 0.02 407	2036 0.02 472	2036 0.02 386	2036 0.02 413	2036 0.02 389	2036 0.02 403
2037 0.02 18	2037 0.02 32	2037 0.02 416	2037 0.02 482	2037 0.02 394	2037 0.02 422	2037 0.02 397	2037 0.02 412
2038 0.02 19	2038 0.02 33	2038 0.02 425	2038 0.02 492	2038 0.02 402	2038 0.02 431	2038 0.02 405	2038 0.02 421
2039 0.02 20	2039 0.02 34	2039 0.02 434	2039 0.02 502	2039 0.02 411	2039 0.02 440	2039 0.02 414	2039 0.02 430
2040 0.02 21	2040 0.02 35	2040 0.02 443	2040 0.02 513	2040 0.02 420	2040 0.02 449	2040 0.02 423	2040 0.02 439
Canterbury & Saddlewood Projected A.M. Weekend Peak Hour	Canterbury & Saddlewood Projected P.M. Weekend Peak Hour	Canterbury & Highway 105 Projected A.M. Weekend Peak Hour	Canterbury & Highway 105 Projected P.M. Weekend Peak Hour	Appaloosa & Highway 105 Projected A.M. Weekend Peak Hour	Appaloosa & Highway 105 Projected P.M. Weekend Peak Hour	Cherry Springs Ranch & Highway 105 Projected A.M. Weekend Peak Hour	Cherry Springs Ranch & Highway 105 Projected P.M. Weekend Peak Hour
A.M. Weekend Peak Hour	P.M. Weekend Peak Hour	A.M. Weekend Peak Hour	Weekend Peak Hour	A.M. Weekend Peak Hour	P.M. Weekend Peak Hour	Projected A.M. Weekend Peak Hour	Projected P.M. Weekend Peak Hour
A.M. Weekend Peak Hour  Year Growth Expected PHV	P.M. Weekend Peak Hour  Year Growth Expected PHV	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244	Weekend Peak Hour  Year Growth Expected PHV  2023 322 2024 0.02 329	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288	P.M. Weekend Peak Hour  Year Growth Expected PHV	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 303
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249	Weekend Peak Hour  Year Growth Expected PHV  2023 322  2024 0.02 329  2025 0.02 336	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 2024 0.02 10	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14	A.M. Weekend Peak Hour  Year Growth Expected PHV 2023 239 2024 0.02 244 2025 0.02 249 2026 0.02 254	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312  2026 0.02 319	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297 2024 0.02 303 2025 0.02 310 2026 0.02 317
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249	Weekend Peak Hour  Year Growth Expected PHV  2023 322  2024 0.02 329  2025 0.02 336	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 303  2025 0.02 310
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14	A.M. Weekend Peak Hour  Year Growth Expected PHV 2023 239 2024 0.02 244 2025 0.02 249 2026 0.02 254	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312  2026 0.02 319	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297 2024 0.02 303 2025 0.02 310 2026 0.02 317
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11 2024 0.02 12 2025 0.02 13 2026 0.02 14 2027 0.02 15	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239 2024 0.02 244 2025 0.02 249 2026 0.02 254 2027 0.02 260	Weekend Peak Hour   Year   Growth   Expected PHV	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282 2024 0.02 288 2025 0.02 294 2026 0.02 300 2027 0.02 306	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299 2024 0.02 305 2025 0.02 312 2026 0.02 319 2027 0.02 326	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295 2024 0.02 301 2025 0.02 308 2026 0.02 315 2027 0.02 322	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297 2024 0.02 303 2025 0.02 310 2026 0.02 317 2027 0.02 324
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2028 0.02 14  2029 0.02 15  2030 0.02 16	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.02 15  2028 0.02 16  2029 0.02 17  2030 0.02 18	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 254  2027 0.02 260  2028 0.02 266  2029 0.02 272  2030 0.02 272  2030 0.02 278	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2029 0.02 320  2030 0.02 320	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 229  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 326  2028 0.02 333  2029 0.02 340  2038 0.02 340	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 306  2026 0.02 315  2027 0.02 322  2028 0.02 329  2029 0.02 336  2030 0.02 336	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 305  2025 0.02 310  2026 0.02 317  2027 0.02 324  2028 0.02 331  2029 0.02 388  2030 0.02 345
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 2024 0.02 10 2025 0.02 11 2026 0.02 12 2027 0.02 13 2028 0.02 14 2029 0.02 15	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11 2024 0.02 12 2025 0.02 13 2026 0.02 14 2027 0.02 15 2028 0.02 16 2029 0.02 17	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239 2024 0.02 244 2025 0.02 249 2026 0.02 254 2027 0.02 260 2028 0.02 266 2029 0.02 272	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282 2024 0.02 288 2025 0.02 294 2026 0.02 300 2027 0.02 306 2028 0.02 313 2029 0.02 320	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299 2024 0.02 305 2025 0.02 312 2026 0.02 319 2027 0.02 326 2028 0.02 333 2029 0.02 333	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295 2024 0.02 301 2025 0.02 308 2026 0.02 315 2027 0.02 322 2028 0.02 329 2029 0.02 336	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297 2024 0.02 305 2025 0.02 310 2026 0.02 317 2027 0.02 324 2028 0.02 331 2029 0.02 338
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2028 0.02 14  2029 0.02 15  2030 0.02 16  2031 0.02 17  2032 0.02 18	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.02 15  2028 0.02 16  2029 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 29  2024 0.02 244  2025 0.02 249  2026 0.02 254  2027 0.02 260  2028 0.02 266  2029 0.02 272  2030 0.02 272  2030 0.02 278  2031 0.02 284  2032 0.02 284  2032 0.02 290	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2029 0.02 320  2030 0.02 327  2031 0.02 334  2032 0.02 341	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 229  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 336  2028 0.02 333  2029 0.02 340  2030 0.02 347  2031 0.02 354  2031 0.02 354	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2027 0.02 322  2028 0.02 329  2029 0.02 336  2030 0.02 336  2030 0.02 343  2031 0.02 350  2032 0.02 357	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 305  2025 0.02 310  2026 0.02 317  2027 0.02 324  2028 0.02 331  2029 0.02 388  2030 0.02 345  2031 0.02 352  2032 0.02 366
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 2024 0.02 10 2025 0.02 11 2026 0.02 12 2027 0.02 13 2028 0.02 14 2029 0.02 15 2030 0.02 16 2031 0.02 17	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11 2024 0.02 12 2025 0.02 13 2026 0.02 14 2027 0.02 15 2028 0.02 16 2029 0.02 17 2030 0.02 18 2031 0.02 19	A.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 239 2024 0.02 244 2025 0.02 249 2026 0.02 254 2027 0.02 260 2028 0.02 266 2029 0.02 272 2030 0.02 278 2031 0.02 284	Year   Growth   Expected PHV	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282 2024 0.02 288 2025 0.02 294 2026 0.02 300 2027 0.02 313 2029 0.02 313 2029 0.02 320 2030 0.02 327 2031 0.02 334	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299 2024 0.02 305 2025 0.02 312 2026 0.02 319 2027 0.02 326 2028 0.02 333 2029 0.02 340 2030 0.02 347 2031 0.02 354	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2022 295 2024 0.02 301 2025 0.02 306 2026 0.02 315 2027 0.02 322 2028 0.02 329 2029 0.02 336 2030 0.02 343 2031 0.02 343	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297 2024 0.02 303 2025 0.02 310 2026 0.02 317 2027 0.02 324 2028 0.02 331 2029 0.02 338 2030 0.02 345 2031 0.02 352
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2028 0.02 14  2029 0.02 15  2030 0.02 16  2031 0.02 17  2032 0.02 18	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.02 15  2028 0.02 16  2029 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 29  2024 0.02 244  2025 0.02 249  2026 0.02 254  2027 0.02 260  2028 0.02 266  2029 0.02 272  2030 0.02 272  2030 0.02 278  2031 0.02 284  2032 0.02 284  2032 0.02 290	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2029 0.02 320  2030 0.02 327  2031 0.02 334  2032 0.02 341	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 229  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 336  2028 0.02 333  2029 0.02 340  2030 0.02 347  2031 0.02 354  2031 0.02 354	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2027 0.02 322  2028 0.02 329  2029 0.02 336  2030 0.02 336  2030 0.02 343  2031 0.02 350  2032 0.02 357	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 305  2025 0.02 310  2026 0.02 317  2027 0.02 324  2028 0.02 331  2029 0.02 388  2030 0.02 345  2031 0.02 352  2032 0.02 366
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 10  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2038 0.02 15  2039 0.02 15  2030 0.02 16  2031 0.02 17  2032 0.02 18  2031 0.02 18  2032 0.02 18  2033 0.02 19  2034 0.02 20  2035 0.02 21	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.03 16  2030 0.02 17  2030 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20  2033 0.02 21  2034 0.02 22  2035 0.02 23	A.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 259  2029 0.02 278  2029 0.02 278  2029 0.02 278  2030 0.02 278  2031 0.02 284  2032 0.02 284  2033 0.02 284  2033 0.02 284  2033 0.02 286  2034 0.02 302  2035 0.02 302	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2030 0.02 327  2031 0.02 337  2031 0.02 334  2032 0.02 341  2033 0.02 341  2033 0.02 348  2034 0.02 355  2035 0.02 355	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312  2026 0.02 319  2026 0.02 339  2028 0.02 349  2039 0.02 347  2031 0.02 354  2030 0.02 354  2031 0.02 354  2032 0.02 356  2033 0.02 370  2034 0.02 378	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2037 0.02 322  2038 0.02 343  2030 0.02 343  2031 0.02 343  2031 0.02 357  2032 0.02 357  2033 0.02 365  2034 0.02 373  2035 0.02 373	Projected P.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 297  2024 0.02 303  2025 0.02 310  2026 0.02 317  2027 0.02 324  2029 0.02 318  2020 0.02 318  2020 0.02 352  2020 0.02 360  2031 0.02 360  2032 0.02 366  2034 0.02 376  2035 0.02 384
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2028 0.02 14  2029 0.02 15  2030 0.02 16  2031 0.02 17  2032 0.02 18  2033 0.02 19  2034 0.02 19	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.02 15  2028 0.02 16  2029 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20  2033 0.02 21  2044 0.02 22	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 254  2027 0.02 260  2028 0.02 266  2029 0.02 272  2030 0.02 278  2031 0.02 284  2032 0.02 284  2032 0.02 284  2032 0.02 250  2034 0.02 256	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2029 0.02 320  2030 0.02 327  2031 0.02 334  2032 0.02 341  2033 0.02 348  2034 0.02 348	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 229  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 336  2028 0.02 333  2029 0.02 340  2030 0.02 347  2031 0.02 354  2032 0.02 362  2033 0.02 370  2034 0.02 370	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2027 0.02 322  2028 0.02 329  2039 0.02 336  2030 0.02 343  2031 0.02 350  2032 0.02 357  2033 0.02 356  2034 0.02 365	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 305  2025 0.02 310  2026 0.02 317  2027 0.02 324  2028 0.02 331  2029 0.02 388  2030 0.02 345  2031 0.02 352  2032 0.02 366  2033 0.02 368  2034 0.02 368
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 10  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2038 0.02 15  2039 0.02 15  2030 0.02 16  2031 0.02 17  2032 0.02 18  2031 0.02 18  2032 0.02 18  2033 0.02 19  2034 0.02 20  2035 0.02 21	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.03 16  2030 0.02 17  2030 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20  2033 0.02 21  2034 0.02 22  2035 0.02 23	A.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 259  2029 0.02 278  2029 0.02 278  2020 0.02 278  2021 0.02 278  2021 0.02 278  2031 0.02 284  2032 0.02 284  2032 0.02 284  2033 0.02 284  2033 0.02 286  2034 0.02 302	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2030 0.02 327  2031 0.02 337  2031 0.02 334  2032 0.02 341  2033 0.02 341  2033 0.02 348  2034 0.02 355  2035 0.02 355	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312  2026 0.02 319  2026 0.02 339  2028 0.02 349  2039 0.02 347  2031 0.02 354  2030 0.02 354  2031 0.02 354  2032 0.02 356  2033 0.02 370  2034 0.02 378	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2037 0.02 322  2038 0.02 343  2030 0.02 343  2031 0.02 343  2031 0.02 357  2032 0.02 357  2033 0.02 365  2034 0.02 373  2035 0.02 373	Projected P.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 297  2024 0.02 303  2025 0.02 310  2026 0.02 317  2027 0.02 324  2029 0.02 318  2020 0.02 318  2020 0.02 352  2020 0.02 360  2031 0.02 360  2032 0.02 366  2034 0.02 376  2035 0.02 384
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 2024 0.02 10 2025 0.02 11 2026 0.02 12 2027 0.02 13 2028 0.02 14 2029 0.02 15 2030 0.02 16 2031 0.02 17 2032 0.02 18 2033 0.02 19 2034 0.02 19 2035 0.02 19 2035 0.02 21 2035 0.02 21 2035 0.02 21 2035 0.02 21 2035 0.02 21 2035 0.02 21 2035 0.02 21	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.02 15  2028 0.02 16  2029 0.02 17  2030 0.02 18  2031 0.02 19  2032 0.02 20  2033 0.02 21  2044 0.02 22  2055 0.02 23	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 254  2027 0.02 260  2028 0.02 272  2030 0.02 272  2030 0.02 278  2031 0.02 284  2032 0.02 284  2032 0.02 290  2033 0.02 290  2034 0.02 296  2034 0.02 300  2035 0.02 309  2035 0.02 309	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2029 0.02 320  2038 0.02 327  2031 0.02 334  2032 0.02 341  2033 0.02 348  2034 0.02 348  2034 0.02 355  2035 0.02 363  2036 0.02 363	P.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 229  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 336  2028 0.02 333  2029 0.02 340  2030 0.02 347  2031 0.02 354  2032 0.02 362  2033 0.02 370  2034 0.02 370  2034 0.02 362  2035 0.02 378  2036 0.02 378  2036 0.02 386  2036 0.02 386	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2027 0.02 322  2028 0.02 336  2039 0.02 336  2039 0.02 343  2031 0.02 350  2032 0.02 357  2033 0.02 365  2034 0.02 373  2035 0.02 381  2036 0.02 381  2036 0.02 381	Projected P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 297  2024 0.02 305  2025 0.02 310  2026 0.02 317  2027 0.02 324  2028 0.02 331  2029 0.02 338  2030 0.02 345  2031 0.02 352  2032 0.02 360  2033 0.02 366  2034 0.02 376  2035 0.02 384  2036 0.02 384  2036 0.02 384  2036 0.02 384
A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 9 9  2024 0.02 10  2025 0.02 11  2026 0.02 12  2027 0.02 13  2039 0.02 15  2039 0.02 16  2030 0.02 16  2031 0.02 17  2032 0.02 18  2031 0.02 18  2033 0.02 19  2034 0.02 19  2036 0.02 20  2036 0.02 21  2036 0.02 21  2036 0.02 22  2036 0.02 22  2036 0.02 22	P. M. Weekend Peak Hour  Year Growth Expected PHV  2023 11  2024 0.02 12  2025 0.02 13  2026 0.02 14  2027 0.03 15  2038 0.02 17  2030 0.02 17  2030 0.02 17  2030 0.02 17  2030 0.02 20  2031 0.02 20  2033 0.02 21  2034 0.02 22  2035 0.02 23  2036 0.02 24  2037 0.02 25	A.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 239  2024 0.02 244  2025 0.02 249  2026 0.02 259  2028 0.02 266  2028 0.02 278  2020 2028 0.02 278  2020 2028 0.02 278  2020 0.02 278  2021 0.02 278  2021 0.02 260  2023 0.02 260  2031 0.02 266  2031 0.02 266  2031 0.02 302  2035 0.02 302  2035 0.02 309  2036 0.02 316	Weekend Peak Hour	A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 282  2024 0.02 288  2025 0.02 294  2026 0.02 300  2027 0.02 306  2028 0.02 313  2039 0.02 334  2031 0.02 344  2032 0.02 345  2033 0.02 348  2034 0.02 355  2035 0.02 363  2036 0.02 371  2037 0.02 363	P.M. Weekend Peak Hour  Year Growth Expected PHV  2023 299  2024 0.02 305  2025 0.02 312  2026 0.02 319  2027 0.02 328  2029 0.02 340  2029 0.02 340  2039 0.02 340  2031 0.02 354  2031 0.02 354  2032 0.02 362  2033 0.02 370  2044 0.02 378  2055 0.02 378  2056 0.02 386  2056 0.02 384	Projected A.M. Weekend Peak Hour  Year Growth Expected PHV  2023 295  2024 0.02 301  2025 0.02 308  2026 0.02 315  2027 0.02 315  2030 0.02 322  2030 0.02 343  2031 0.02 343  2031 0.02 357  2032 0.02 357  2033 0.02 357  2033 0.02 365  2034 0.02 373  2035 0.02 381  2036 0.02 381  2036 0.02 389  2037 0.02 389	Projected P.M. Weekend Peak Hour  Vear Growth Expected PHV  2023 297  2024 0.02 303  2055 0.02 310  2066 0.02 317  207 0.02 324  2088 0.02 331  2030 0.02 351  2030 0.02 352  2031 0.02 360  2031 0.02 360  2031 0.02 368  2034 0.02 376  2035 0.02 384  2036 0.02 376  2037 0.02 384  2036 0.02 382  2037 0.02 384

## Long Range Growth (2023-2040)

Canterbury & Saddlewood A.M. Weekday Peak			ury & Saddle 1. Weekday F	wood Projected Peak Hour		ıry & Highw 1. Weekday	ay 105 Projected Peak Hour		& Highway Weekday Pe	105 Projected P.M. eak Hour		sa & Highwa 1. Weekday	ay 105 Projected Peak Hour		sa & Highw 1. Weekday	ay 105 Projected Peak Hour			& Highway 105 kday Peak Hour			s Ranch & High 1. Weekday Pe	
Year Growth Exp	pected PHV	Year	Growth	Expected PHV	Year	Growth	Expected PHV	Year	Growth	Expected PHV	Year	Growth	Expected PHV	Year	Growth	Expected PHV	Year	Growth	Expected PHV	Y	ear Gr	owth Expe	ected PHV
2023	4	2023		18	2023		309	2023		359	2023		293	2023		314	2023		295	2	023		306
2024 0.044	5	2024	0.044	19	2024	0.044	323	2024	0.044	375	2024	0.044	306	2024	0.044	328	2024	0.044	308			.044	320
2025 0.044	6	2025	0.044	20	2025	0.044	338	2025	0.044	392	2025	0.044	320	2025	0.044	343	2025	0.044	322				335
2026 0.044	7	2026	0.044	21	2026	0.044	353	2026	0.044	410	2026	0.044	335	2026	0.044	359	2026	0.044	337				350
2027 0.044	8	2027	0.044	22	2027	0.044	369	2027	0.044	429	2027	0.044	350	2027	0.044	375	2027	0.044	352			.044	366
2028 0.044	9	2028	0.044	23	2028	0.044	386	2028	0.044	448	2028	0.044	366	2028	0.044	392	2028	0.044	368			.044	383
2029 0.044	10	2029	0.044	24	2029	0.044	403	2029	0.044	468	2029	0.044	383	2029	0.044	410	2029	0.044	385			.044	400
2030 0.044	11	2030	0.044	25	2030	0.044	421	2030	0.044	489	2030	0.044	400	2030	0.044	429	2030	0.044	402			.044	418
2031 0.044	12	2031	0.044	26	2031	0.044	440	2031	0.044	511	2031	0.044	418	2031	0.044	448	2031	0.044	420				437
2032 0.044	13	2032	0.044	27	2032	0.044	460	2032	0.044	534	2032	0.044	437	2032	0.044	468	2032	0.044	439				457
2033 0.044	14	2033	0.044	28	2033	0.044	481	2033	0.044	558	2033	0.044	457	2033	0.044	489	2033	0.044	459				478
2034 0.044	15	2034	0.044	29	2034	0.044	503	2034	0.044	583	2034	0.044	478	2034	0.044	511	2034	0.044	480				500
2035 0.044	16	2035	0.044	30	2035	0.044	526	2035	0.044	609	2035	0.044	500	2035	0.044	534	2035	0.044	502				522
2036 0.044	17	2036	0.044	31	2036	0.044	550	2036	0.044	636	2036	0.044	522	2036	0.044	558	2036	0.044	525			.044	545
2037 0.044	18	2037	0.044	32	2037	0.044	575	2037	0.044	664	2037	0.044	545	2037	0.044	583	2037	0.044	549			.044	569
2038 0.044	19	2038	0.044	33	2038	0.044	601	2038	0.044	694	2038	0.044	569	2038	0.044	609	2038	0.044	574			.044	595
2039 0.044	20	2039	0.044	24	2039	0.044	628	2039	0.044	725	2039	0.044	595	2039	0.044	636	2039	0.044	600				622
2040 0.044	21	2040	0.044	25	2040	0.044	656	2040	0.044	757	2040	0.044	622	2040	0.044	664	2040	0.044	627				650
Canterbury & Saddlewood A.M. Weekend Peak			ury & Saddle 1. Weekend I	wood Projected Peak Hour		ıry & Highw 1. Weekend	ay 105 Projected Peak Hour		& Highway Weekend Pe	105 Projected P.M. eak Hour		sa & Highwa M. Weekend	ay 105 Projected Peak Hour		sa & Highw 1. Weekend	ay 105 Projected I Peak Hour			& Highway 105 kend Peak Hour			s Ranch & High J. Weekend Po	
A.M. Weekend Peak																				Pr	ojected P.I	Л. Weekend Pr	
A.M. Weekend Peak	Hour	P.N		Peak Hour	A.N	1. Weekend	Peak Hour	,	Weekend Pe	eak Hour	A.N	1. Weekend	Peak Hour	P.1	1. Weekend	Peak Hour	Projecte	d A.M. Week	kend Peak Hour	Pr	ojected P.I	M. Weekend Property owth Expe	Peak Hour
A.M. Weekend Peak Year Growth Exp	Hour	P.N Year		Peak Hour	A.N Year	1. Weekend	Peak Hour  Expected PHV	Year	Weekend Pe	Expected PHV	A.N Year	1. Weekend	Peak Hour  Expected PHV	P.f Year	1. Weekend	Peak Hour  Expected PHV	Projecte	d A.M. Week	kend Peak Hour Expected PHV	Pr Y 2	ojected P.P ear Gi 023	A. Weekend Pe	Peak Hour ected PHV
A.M. Weekend Peak Year Growth Exp	pected PHV	P.N Year 2023	1. Weekend I	Peak Hour  Expected PHV  11	A.N Year 2023	f. Weekend Growth	Peak Hour  Expected PHV  239	Year 2023	Weekend Pe	Expected PHV 322	A.N Year 2023	M. Weekend Growth	Peak Hour  Expected PHV  282	P.I Year 2023	1. Weekend	Peak Hour  Expected PHV  299	Projecte Year 2023	d A.M. Week	Expected PHV	Pr Y 2 2	ojected P.P ear Gi 023 024 0	A. Weekend Proof	Peak Hour ected PHV 297
A.M. Weekend Peak Year Growth Exp 2023 2024 0.044	pected PHV 9 10	P.N Year 2023 2024	f. Weekend I Growth 0.044	Peak Hour  Expected PHV  11 12	A.N Year 2023 2024	f. Weekend Growth 0.044	Peak Hour  Expected PHV  239 250	Year 2023 2024	Growth	Expected PHV 322 337	A.N Year 2023 2024	M. Weekend Growth 0.044	Peak Hour Expected PHV 282 295	P.f Year 2023 2024	f. Weekend Growth 0.044	Peak Hour  Expected PHV  299 313	Projecte Year 2023 2024	Growth	Expected PHV 295 308	Pr Y 2: 2: 2:	ojected P.P ear Gi 023 024 0 025 0	A. Weekend Proof	Peak Hour ected PHV 297 311
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A.M. Weekend Peak Year Growth Exp 2023 2024 0.044 2025 0.044 2026 0.044	pected PHV 9 10 11	P.N Year 2023 2024 2025 2026	0.044 0.044 0.044	Expected PHV  11 12 13 14	A.N Year 2023 2024 2025 2026	0.044 0.044 0.044	Peak Hour  Expected PHV  239 250 261 273	Year 2023 2024 2025 2026	Growth  0.044  0.044  0.044	Expected PHV 322 337 352 368	A.N Year 2023 2024 2025 2026	0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322	Year 2023 2024 2025 2026	0.044 0.044 0.044	Peak Hour  Expected PHV  299 313 327 342	Year 2023 2024 2025 2026	Growth  0.044 0.044 0.044	Expected PHV  295 308 322 337	Pr 2 2 2 2 2 2 2 2	ojected P.P. ear Gi 023 024 0 025 0 026 0 027 0	M. Weekend Prowth Expe .044 .044 .044 .044	297 311 325 340
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A.M. Weekend Peak Year Growth Ext 2023 2024 0.044 2025 0.044 2026 0.044 2027 0.044 2028 0.044 2029 0.044	pected PHV  9 10 11 12 13 14	P.N Year 2023 2024 2025 2026 2027 2028	0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11 12 13 14 15 16 17	A.N Year 2023 2024 2025 2026 2027 2028	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239 250 261 273 286 299 313	Year 2023 2024 2025 2026 2027 2028	0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV 322 337 352 368 385 402 420	A.N Year 2023 2024 2025 2026 2027 2028	0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368	P.I Year 2023 2024 2025 2026 2027 2028	0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV 299 313 327 342 358 374 391	Projecte  Year  2023 2024 2025 2026 2027 2028	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  295 308 322 337 352 368 385	Pr 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ear Gr 023 024 0 025 0 026 0 027 0 028 0 029 0 030 0	n. Weekend Provided P	297 311 325 340 355 371 388
A.M. Weekend Peak Year Growth Exp 2023 2024 0.044 2025 0.044 2026 0.044 2027 0.044 2028 0.044 2029 0.044 2030 0.044	9 10 11 12 13 14 15 16	P.N Year 2023 2024 2025 2026 2027 2028 2029 2030	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18	A.N Year 2023 2024 2025 2026 2027 2028 2029 2030	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239  250  261  273  286  299  313  327	Year 2023 2024 2025 2026 2027 2028 2029 2030	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  322 337 352 368 385 402 420 439	A.M. Year 2023 2024 2025 2026 2027 2028 2029 2030	M. Weekend  Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368 385	P.I. Year 2023 2024 2025 2026 2027 2028 2029 2030	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV 299 313 327 342 358 374 391 409	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  295 308 322 337 352 368 385 402	Pr Y 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ear Gi 023 024 0 025 0 026 0 027 0 028 0 029 0 030 0 031 0	N. Weekend Provided P	297 311 325 340 355 371 388 406
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A.M. Weekend Peak Year Growth Ext 2023 2024 0.044 2025 0.044 2027 0.044 2027 0.044 2029 0.044 2030 0.044 2031 0.044 2031 0.044 2031 0.044 2033 0.044 2033 0.044 2033 0.044	pected PHV  9 10 11 12 13 14 15 16 17 18	P.N Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2033 2034	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21	A.N Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2033 2033 2034	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239  250  261  273  286  299  313  327  342  358  374  391	Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  322 337 352 368 385 402 420 439 459 480 502 525	AAN Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2033 2034	6. Weekend  Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368 385 402 420 439 459	P.I Year 2023 2024 2025 2026 2027 2028 2030 2031 2032 2033 2033 2033 2034	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299 313 327 342 358 374 391 409 427 446 466 487	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459	Pr 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	pojected P.M.  poierred P.M.  poierr	7. Weekend Property of the Control o	297 311 325 340 355 371 388 406 424 443 463 484
A.M. Weekend Peak Year Growth Ext 2023 2024 0.044 2025 0.044 2025 0.044 2025 0.044 2020 0.044 2030 0.044 2030 0.044 2031 0.044 2031 0.044 2032 0.044 2034 0.044 2034 0.044 2034 0.044	9 10 11 12 13 14 15 16 17 18 19 20 21 21 1	P.N Year 2023 2024 2025 2026 2027 2028 2030 2031 2032 2033 2034 2034 2035	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21	A.M Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239  250  261  273  286  299  313  327  342  358  374  391  409	Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  322  337  352  368  385  402  420  439  459  480  502  525  549	AAN Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035	6. Weekend  Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368 385 402 420 439 459 480	P.1 Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299 313 327 342 358 374 391 409 427 446 466 487 509	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459 480 502	PI Y 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ojected P.N  ear Gi  023  024 0  025 0  026 0  027 0  028 0  029 0  031 0  031 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0	7. Weekend Provided P	297 311 325 340 355 371 388 406 424 443 463 484 506
A.M. Weekend Peak Year Growth Ext 2023 2024 0,044 2025 0,044 2027 0,044 2027 0,044 2029 0,044 2030 0,044 2030 0,044 2031 0,044 2031 0,044 2033 0,044 2033 0,044 2035 0,044 2035 0,044 2035 0,044 2035 0,044	9 10 11 12 13 14 15 16 17 18 19 19 10 10 11 12 13 14 15 16 17 18 19 20 20 21 22	P.N Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2034 2035 2036	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21  22  23	A.N. Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2034 2035 2036	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239 250 261 273 286 299 313 327 342 358 374 391 409 427	Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2034 2035 2036	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	ask Hour  Expected PHV  322 337 352 368 385 402 420 439 459 480 502 525 549 574	AAN Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2036	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368 385 402 420 439 459 480 502	P.I. Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2035	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299 313 327 342 358 374 391 409 427 446 466 487 509 532	Projecte  Year  2023 2024 2025 2026 2027 2028 2030 2030 2031 2032 2033 2034 2035 2036	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459 480 502 525	Pi Y 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ear Gi 023 024 0 025 0 026 0 027 028 0 029 0 0331 0 0332 0 0333 0 0335 0 0335 0	0.044 .044 .044 .044 .044 .044 .044 .04	297 311 325 340 355 371 388 406 424 443 463 484 506 529
A.M. Weekend Peak Year Growth Ext 2023 2024 0.044 2025 0.044 2025 0.044 2027 0.044 2027 0.044 2027 0.044 2028 0.044 2029 0.044 2030 0.044 2031 0.044 2031 0.044 2032 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044	pected PHV  9  10  11  12  13  14  15  16  17  18  19  20  21  22  23	P.N Year 2023 2024 2025 2026 2027 2028 2030 2031 2031 2032 2033 2034 2035 2036 2035 2036 2036 2037 2037	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21  22  23	A.N. Year 2023 2024 2025 2026 2027 2028 2030 2031 2031 2032 2033 2034 2035 2036 2037 2037	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239 250 261 273 321 286 299 313 327 342 358 374 391 409 427 446	Vear 2023 2024 2025 2026 2027 2028 2029 2031 2032 2033 2034 2035 2036 2037	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	eak Hour  Expected PHV  322  337  352  368  385  402  420  420  429  459  480  502  525  549  574  600	AAN Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2036	6. Weekend  Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 332 352 368 362 402 420 439 459 480 502 525	P.I Year 2023 2024 2025 2026 2027 2030 2031 2032 2032 2033 2034 2035 2036 2036 2036	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299  313  327  342  358  371  400  427  446  466  487  509  532  556	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037	Growth  0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459 480 502 525 549	Pi V 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ear Gi 023 024 0 025 0 026 0 027 0 027 0 028 0 029 0 030 0 031 0 032 0 033 0 033 0 033 0 033 0 033 0 033 0	0.044	297 311 325 340 355 371 388 406 424 443 463 484 506 529 553
A.M. Weekend Peak Year Growth Ext 2023 2024 0,044 2025 0,044 2027 0,044 2027 0,044 2029 0,044 2030 0,044 2030 0,044 2031 0,044 2031 0,044 2033 0,044 2035 0,044 2035 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044 2037 0,044	9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 25 5	P.M. Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2055 2036 2037 2038	Growth  0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21  22  23	A.M. Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037 2038	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  239 250 261 273 286 299 313 327 342 358 374 391 409 427 446 466	Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037	Growth  0.044	Expected PHV  322 337 352 358 385 402 420 439 459 450 502 525 549 574 600 627	An Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037 2038	6. Weekend  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 337 352 368 368 368 400 420 499 489 480 502 525 549	P.I.  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299 313 327 342 358 374 391 409 427 446 466 487 509 532 556 581	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037 2038	Growth  0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459 480 502 525 549 574	Pr 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ear G1  023  024 0  025 0  026 0  027 0  028 0  029 0  030 0  031 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  033 0  034 0  035 0  035 0  036 0  037 0  038 0	0.044 0.044	297 311 325 340 355 371 388 406 424 443 463 484 506 529 553 578
A.M. Weekend Peak Year Growth Ext 2023 2024 0.044 2025 0.044 2025 0.044 2027 0.044 2027 0.044 2027 0.044 2028 0.044 2029 0.044 2030 0.044 2031 0.044 2031 0.044 2032 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044 2036 0.044	pected PHV  9  10  11  12  13  14  15  16  17  18  19  20  21  22  23	P.N Year 2023 2024 2025 2026 2027 2028 2030 2031 2031 2032 2033 2034 2035 2036 2035 2036 2036 2037 2037	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  11  12  13  14  15  16  17  18  19  20  21  22  23	A.N. Year 2023 2024 2025 2026 2027 2028 2030 2031 2031 2032 2033 2034 2035 2036 2037 2037	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  239 250 261 273 321 286 299 313 327 342 358 374 391 409 427 446	Vear 2023 2024 2025 2026 2027 2028 2029 2031 2032 2033 2034 2035 2036 2037	Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	eak Hour  Expected PHV  322  337  352  368  385  402  420  420  429  459  480  502  525  549  574  600	AAN Year 2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2036	6. Weekend  Growth  0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  282 295 308 322 332 352 368 362 402 420 439 459 480 502 525	P.I Year 2023 2024 2025 2026 2027 2030 2031 2032 2032 2033 2034 2035 2036 2036 2036	0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044	Peak Hour  Expected PHV  299  313  327  342  358  371  400  427  446  466  487  509  532  556	Projecte  Year  2023 2024 2025 2026 2027 2028 2029 2030 2031 2032 2033 2034 2035 2036 2037	Growth  0.044	Expected PHV  295 308 322 337 352 368 385 402 420 439 459 480 502 525 549	Pi V 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ojected P.P.  Dear G.  D23  D24	0.044	297 311 325 340 355 371 388 406 424 443 463 484 506 529 553

## SHORT RANGE HORIZON PEAK HOUR TURNING MOVEMENTS



			y/Hwy 105	_							
			am to 10:0								
		Short Rang	e Weekend								
		Hwy	105								
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT						
0	124	90	60	117	0						
		Cante	rbury								
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT										
0	0	0	6	0	16						

	U	U	
Dook Hou	r Volumo:	/112	

Canterbury/Saddlewood											
	Peak Hour: 9:00 am to 10:00 am										
		Short Rang	e Weekend								
		Saddle	ewood								
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT						
0	0	0	0	0	19						
		Cante	rbury								
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT						
0	0 2 150 1 1 0										
Peak Hou	r Volume	173									

Daali Hair	- \ / - I	172			
0	2	150	1	1	0
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB

		Appaloosa	a/Hwy 105							
Peak Hour: 9:00 am to 10:00 am										
		Short Rang	e Weekday							
		Hwy	105							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	114	6	4	232	0					
		Appa	loosa							
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT					
0	0	0	4	0	4					
Poak Hou	r Volumo:	26/								

Appaloosa/Hwy 105

Peak Hour: 9:00 am to 10:00 am Short Range Weekend Hwy 105 EB LT EB Thru EB RT WB LT WB Thru WB RT

> Appaloosa SB LT NB RT

Appaloosa/Hwy 105

Peak Hour: 1:00 pm to 2:00 pm

Short Range Weekday

Hwy 105

EB LT EB Thru EB RT WB LT WB Thru WB RT

Appaloosa

5

0

4

233

NB Thru

0

175

0

NB Thru NB LT

4

NB LT

5

0

0 114

0 0

0 155

0 0

SB RT SB Thru SB LT NB RT

Peak Hour Volume:

Cherry Springs Ranch/Hwy 105								
	Peak	Hour: 9:00	am to 10:0	0 am				
		Short Rang	e Weekday					
		Hwy	105					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT			
4	118	0	0	230	5			
		Cherry Spr	ings Ranch					
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT			
3	3 0 2 0 0 0							
Book Hou	r Volumo:	262						

Canterbury/Hwy 105						
	Peak	Hour: 9:00	am to 10:0	0 am		
		Short Rang	e Weekday			
		Hwy	105			
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT	
0	151	78	51	162	0	
Canterbury						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT	
0	0	0	8	0	11	

170

0

Peak Hour Volume:

0 0

Peak Hour Volume:

ak	Hour	Volume:	461

EDLI	EBIIIIU	EDILI	WDLI	WDIIIIU	WDKI		
0	151	78	51	162	0		
	Canterbury						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0	0	0	8	0	11		

Canterbury/Hwy 105

Peak Hour: 1:00 pm to 2:00 pm

Short Range Weekend

Hwy 105

EB LT EB Thru EB RT WB LT WB Thru WB RT

Canterbury

Canterbury/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm Short Range Weekday Hwy 105

EB LT EB Thru EB RT WB LT WB Thru WB RT 0 203 32 18 148 0 Canterbury SB RT SB Thru SB LT NB RT NB Thru NB LT

427

9

97 63

153

SB LT NB RT NB Thru NB LT

0 30 0 48

Short Range Weekday					
		Saddle	ewood		
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT
0	0	0	1	0	14
Canterbury					
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT
0	Λ	130	Λ	Λ	Λ

Canterbury/Saddlewood

Peak Hour: 9:00 am to 10:00 am

B RT	SB Thru	SB LT	NB RT	NB
Λ	Λ	130	Λ	

eak Hour Volume:	

Peak	Hour	Volume:	1.	4

Peak Hour \	/olume:	14
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eal	k F	lour	٧	o'	ı	ume:	1
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eak	noui	voiuille.	1

	(	Canterbury/	'Saddlewoo	d						
	Peal	k Hour: 1:00	0 pm to 2:0	0 pm						
	Short Range Weekend									
	Saddlewood									
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	0 0 0 0 0 69									
		Cante	erbury							

Short Range Weekend Saddlewood										
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	0	0	0	0	69					
		Cante	rbury							
SB RT	SB Thru	SB Thru SB LT NB RT		NB Thru	NB LT					
0	3	157		7	0					
Peak Hour Volume: 237										

Canterbury/Saddlewood										
	Peak	Hour: 1:00	pm to 2:0	0 pm						
Short Range Weekday										
Saddlewood  EB LT										
EB LT	EB Thru	EB RT	WB LT	WB LT WB Thru						
0	0	0 3 0 23								
		Cante	rbury							
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT					

3 2 0

3 43 Peak Hour Volume:

		Appaloosa	Hwy 105							
Peak Hour: 1:00 pm to 2:00 pm										
Short Range Weekend										
Hwy 105										
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	191	9	6	196	0					
		Appa	loosa							
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT					
0	0	0	2	Λ	2					

Peak Hour Volume: 406

	Che	rry Springs	Ranch/Hwy	105					
	Peak	Hour: 9:00	am to 10:0	0 am					
		Short Rang	e Weekend						
	7 116 0 0 226 13								
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT				
7	116	0	0	226	13				
		Cherry Spr	ings Ranch						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT				
6	0	4	0	0	0				
Peak Hou	r Volume:	372							

Peak Hour Volume:

	Che	rry Springs	Ranch/Hwy	105								
Peak Hour: 1:00 pm to 2:00 pm												
		Short Rang	e Weekday									
	Hwy 105  EB LT EB Thru EB RT WB LT WB Thru WB RT											
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT							
0	154	0	0	178	2							
		Cherry Spr	ings Ranch									
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT							
4	^	1	^	^	0							

Peak Hour Volume:

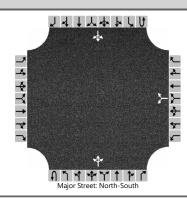
	Che	rry Springs	Ranch/Hwy	105	
	Peal	k Hour: 1:00	pm to 2:00	) pm	
		Short Rang	e Weekend		
		Hwy	105		
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT
0	201	0	0	194	2
		Cherry Spr	ings Ranch		
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT
	_		_	_	-

Peak Hour Volume: 400

# SHORT RANGE HORIZON LEVEL OF SERVICE (LOS)

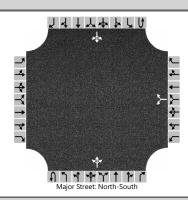


	HCS7 Two-Way Stop-Control Report										
General Information		Site Information									
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/13/2023	East/West Street	Saddlewood								
Analysis Year	2023	North/South Street	Canterbury								
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.85								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description	Colorado Kids Ranch										



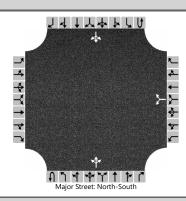
Vehicle Volumes and Ad	justme	nts														
Approach	Т	Eastb	oound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						1		14		0	0	0		130	0	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T					7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	Т						18			0				153		
Capacity, c (veh/h)							1029			1623				1623		
v/c Ratio							0.02			0.00				0.09		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.3		
Control Delay (s/veh)							8.6			7.2				7.4		
Level of Service (LOS)							А			А				А		
Approach Delay (s/veh)						8	.6							7	.4	
Approach LOS	1					,	Ą									

	HCS7 Two-Way Stop-Control Report										
General Information		Site Information									
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/13/2023	East/West Street	Saddlewood								
Analysis Year	2023	North/South Street	Canterbury								
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.85								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description	Colorado Kids Ranch										



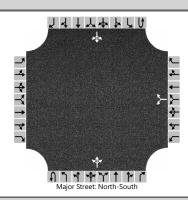
Vehicle Volumes and Adj	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						3		23		0	2	3		43	3	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							31			0				51		
Capacity, c (veh/h)							1047			1618				1615		
v/c Ratio							0.03			0.00				0.03		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.1		
Control Delay (s/veh)							8.5			7.2				7.3		
Level of Service (LOS)							Α			Α				А		
Approach Delay (s/veh)						8	.5		0.0			6.8				
Approach LOS						,	4									

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Saddlewood
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



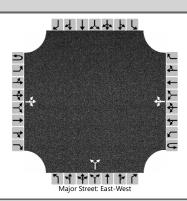
Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration							LR				LTR				LTR		
Volume (veh/h)						0		19		0	1	1		150	2	0	
Percent Heavy Vehicles (%)						2		2		2				2			
Proportion Time Blocked																	
Percent Grade (%)						(	0										
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)						7.1		6.2		4.1				4.1			
Critical Headway (sec)						7.12		6.22		4.12				4.12			
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2			
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22			
Delay, Queue Length, and	l Leve	l of S	ervice														
Flow Rate, v (veh/h)							22			0				176			
Capacity, c (veh/h)							1083			1620				1620			
v/c Ratio							0.02			0.00				0.11			
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.4			
Control Delay (s/veh)							8.4			7.2				7.5			
Level of Service (LOS)							А			А				Α			
Approach Delay (s/veh)						8.4			0.0				7.4				
Approach LOS						А											

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Saddlewood
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration							LR				LTR				LTR		
Volume (veh/h)						0		69		0	7	1		157	3	0	
Percent Heavy Vehicles (%)						2		2		2				2			
Proportion Time Blocked																	
Percent Grade (%)						(	0										
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)						7.1		6.2		4.1				4.1			
Critical Headway (sec)						7.12		6.22		4.12				4.12			
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2			
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22			
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	T						81			0				185			
Capacity, c (veh/h)							1073			1618				1610			
v/c Ratio							0.08			0.00				0.11			
95% Queue Length, Q <sub>95</sub> (veh)							0.2			0.0				0.4			
Control Delay (s/veh)							8.6			7.2				7.5			
Level of Service (LOS)							А			А				А			
Approach Delay (s/veh)		3					.6		0.0				7.4				
Approach LOS						,	4										

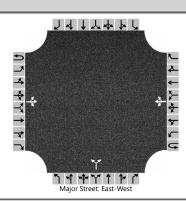
	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.93
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	151	78		51	162	0		11		8				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				55					20					
Capacity, c (veh/h)		1402				1320					579					
v/c Ratio		0.00				0.04					0.04					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1					0.1					
Control Delay (s/veh)		7.6				7.8					11.4					
Level of Service (LOS)		А				А					В					
Approach Delay (s/veh)		0	.0			2	.2			11	.4					
Approach LOS										į.	3					

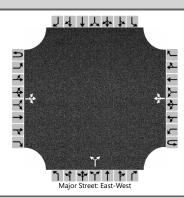
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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.90
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



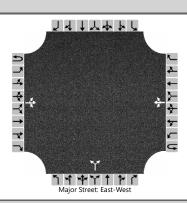
Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	203	32		18	148	0		17		9				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	0					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				20					29					
Capacity, c (veh/h)		1414				1303					586					
v/c Ratio		0.00				0.02					0.05					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.2					
Control Delay (s/veh)		7.5				7.8					11.5					
Level of Service (LOS)		Α	Ì			Α					В					
Approach Delay (s/veh)		C	.0			1	.0			1	1.5					
Approach LOS						,	Α				В					

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



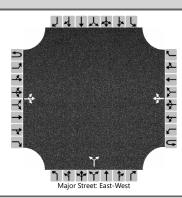
Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	124	90		60	117	0		16		6				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)											)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	Т	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	Т	0				71					26					
Capacity, c (veh/h)		1446				1314					540					
v/c Ratio		0.00				0.05					0.05					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.2					
Control Delay (s/veh)		7.5				7.9					12.0					
Level of Service (LOS)		А				А					В					
Approach Delay (s/veh)		0	.0			3	.0			12	2.0					
Approach LOS							A				 B					

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Canterbury
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			LTR				LTR				LR						
Volume (veh/h)		0	170	97		63	153	0		48		30					
Percent Heavy Vehicles (%)		2				2				2		2					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up Ho	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1		6.2					
Critical Headway (sec)		4.12				4.12				7.12		6.22					
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3					
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32					
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	Τ	0				74					92						
Capacity, c (veh/h)		1396				1246					493						
v/c Ratio		0.00				0.06					0.19						
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.7						
Control Delay (s/veh)		7.6				8.1					14.0						
Level of Service (LOS)		А				А					В						
Approach Delay (s/veh)		0.0 2.7							14.0								
Approach LOS											3						

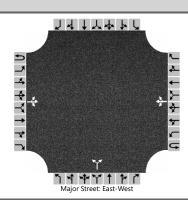
HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Appaloosa						
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.87						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Kids Ranch								



Vehicle Volumes and Ad	justme	nts														
Approach		Eastbound			Westbound			Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	114	6		4	232	0		4		4				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)									0							
Right Turn Channelized																
Median Type   Storage		Undivided							<u>'</u>							
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	Т	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	Т	0				5					9					
Capacity, c (veh/h)		1297				1446					687					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.8				7.5					10.3					
Level of Service (LOS)		А				А					В					
Approach Delay (s/veh)		0.0			0.2			10.3								
Approach LOS					A			В								

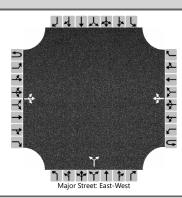
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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Appaloosa
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.98
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	155	3		6	175	0		4		5				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				6					9					
Capacity, c (veh/h)		1397				1418					732					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.6				7.6					10.0					
Level of Service (LOS)		А				Α					А					
Approach Delay (s/veh)		0	.0			0	.3			10	).0				-	
Approach LOS						,	4			,	A					

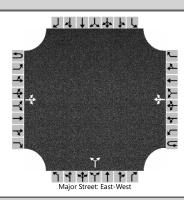
	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Appaloosa
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.90
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	114	5		4	233	0		5		2				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	T	0				4					8					
Capacity, c (veh/h)		1306				1453					632					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.8				7.5					10.8					
Level of Service (LOS)		Α				А					В					
Approach Delay (s/veh)		0	.0			0	.2			10	).8					
Approach LOS						,	Ą			ı	3					

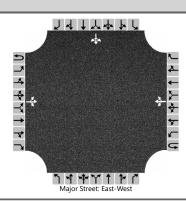
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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/13/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Appaloosa
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.90
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



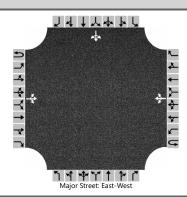
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	191	9		6	196	0		2		2				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				7					4					
Capacity, c (veh/h)		1352				1347					636					
v/c Ratio		0.00				0.00					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0			Ì		0.0			Ì		
Control Delay (s/veh)		7.7				7.7					10.7					
Level of Service (LOS)		А				Α					В					
Approach Delay (s/veh)		0	.0	-		0	.3	-		1(	).7				•	_
Approach LOS											В					

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/20/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Cherry Springs
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.88
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



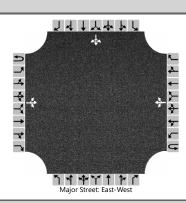
Approach		Eastb	ound			Westl	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		4	118	0		0	230	5						2	0	3
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		5				0									6	
Capacity, c (veh/h)		1297				1450									667	
v/c Ratio		0.00				0.00									0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.8				7.5									10.4	
Level of Service (LOS)		А				А									В	
Approach Delay (s/veh)	0.3				0.0			).4								
Approach LOS	0.3												В			

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/20/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Cherry Springs
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.97
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



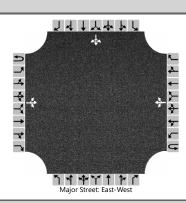
Vehicle Volumes and Ad	justme	nts														
Approach	T	Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		0	154	0		0	178	2						3	0	1
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized																
Median Type   Storage				Undi	ivided											
Critical and Follow-up H	eadwa	ys							•							
Base Critical Headway (sec)	T	4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	Т	0				0									4	
Capacity, c (veh/h)		1389				1421									658	
v/c Ratio		0.00				0.00									0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.6				7.5									10.5	
Level of Service (LOS)		Α				Α									В	
Approach Delay (s/veh)		C	.0			0	.0							1(	).5	
Approach LOS	1														 В	

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County
Date Performed	7/20/2023	East/West Street	Hwy 105
Analysis Year	2023	North/South Street	Cherry Springs
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.93
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Colorado Kids Ranch		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		7	116	0		0	226	13						4	0	6
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		8				0									11	
Capacity, c (veh/h)		1308				1462									682	
v/c Ratio		0.01				0.00									0.02	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.8				7.5									10.4	
Level of Service (LOS)		А				Α									В	
Approach Delay (s/veh)	0.5				0.0							10.4				
Approach LOS															В	

HCS7 Two-Way Stop-Control Report								
General Information Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105					
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County					
Date Performed	7/20/2023	East/West Street	Hwy 105					
Analysis Year	2023	North/South Street	Cherry Springs					
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.92					
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25					
Project Description	Colorado Kids Ranch							



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westk	oound		Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		0	201	0		0	194	2						1	0	2
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized																
Median Type   Storage	Undivided															
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				0									3	
Capacity, c (veh/h)		1357				1351									700	
v/c Ratio		0.00				0.00									0.00	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0	
Control Delay (s/veh)		7.7				7.7									10.2	
Level of Service (LOS)		А				А									В	
Approach Delay (s/veh)		0	.0			0	.0	-			•			10	0.2	
Approach LOS															В	

# LONG RANGE HORIZON PEAK HOUR TURNING MOVEMENTS



Canterbury/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Long Range	e Weekend				
Hwy 105							
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT						
0	252	91	61	244	0		
Canterbury							
SB RT SB Thru SB LT NB RT NB Thru NB LT							
0	0	0	6	0	24		

Peak Hour Volume:

Canterbury/Saddlewood Peak Hour: 9:00 am to 10:00 am								
	i can							
		Long Range	e Weekend					
		Saddle	ewood					
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT			
0	0	0	0	0	26			
	Canterbury							
SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	6	154	3	3	0			

192 Peak Hour Volume:

Appaloosa/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Long Range	e Weekday				
		Hwy	105				
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT		
0	233	8	5	427	0		
		Appa	loosa				
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0	0	0	7	0	7		
Peak Hour Volume: 687							

	Cherry Springs Ranch/Hwy 105						
	Peak	Hour: 9:00	am to 10:0	0 am			
		Long Rang	e Weekday				
ł		Hwy	105				
EB LT	EB Thru EB RT WB LT WB Thru WB RT						
9	236	0	0	422	11		
		Cherry Spr	ings Ranch				
SB RT SB Thru SB LT NB RT NB Thru NB LT							
6 0 4 0 0 0							
Poak Hou	Poak Hour Volumo: 600						

Canterbury/Hwy 105								
	Peak	Hour: 9:00	am to 10:0	0 am				
		Long Range	e Weekday					
		Hwy	105					
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT							
0	310	80	51	335	0			
	Canterbury							
SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	0	0	11	0	14			

Peak Hour Volume:

8		

Canterbury/Saddlewood								
	Peak	Hour: 9:00	am to 10:0	00 am				
		Long Rang	e Weekday					
		Saddle	ewood					
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT							
0	0	0	5	0	18			
	Canterbury							
SB RT SB Thru SB LT NB RT NB Thru NB LT								
0	0	138	0	0	0			

Peak Hour Volume: 161

Appaloosa/Hwy 105							
	Peak	Hour: 9:00	am to 10:0	0 am			
		Long Range	e Weekend				
		Hwy	105				
EB LT EB Thru EB RT WB LT WB Thru WB RT							
0	232	5	5	421	0		
		Appa	loosa				
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT		
0 0 0 3 0 9							
Peak Hou	r Volume:	675					

 	C71

Cherry Springs Ranch/Hwy 105								
	Peak	Hour: 9:00	am to 10:0	00 am				
		Long Range	e Weekend					
		Hwy	105					
EB LT	EB LT EB Thru EB RT WB LT WB Thru WB RT							
15	230	0	0	404	28			
	Cherry Springs Ranch							
SB RT SB Thru SB LT NB RT NB Thru NB LT								
13 0 8 0 0 0								
0 1 11	14.1	500						

Peak Hour Volume:

Canterbury/Hwy 105										
	Peal	k Hour: 1:00	pm to 2:00	) pm						
		Long Range	e Weekend							
		Hwy	105							
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT					
0	347	100	63	317	0					
	Canterbury									
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT									
0	0 0 33 0 56									

Peak Hour Vo

0	lume:	9

Canterbury/Saddlewood										
	Peak Hour: 1:00 pm to 2:00 pm									
		Long Range	e Weekend							
		Saddle	ewood							
EB LT	EB Thru EB RT WB LT WB Thru WB RT									
0	0	0	0	0	71					
		Cante	rbury							
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT									
0	0 9 157 3 18 0									

Peak Hour Volume:

Appaloosa/Hwy 105 Peak Hour: 1:00 pm to 2:00 pm Long Range Weekday Hwy 105									
EB LT									
0	314	5	12	344	0				
		Appa	loosa						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT				
0	0 0 0 9 0 7								
Peak Hou	r Volume:	691							

Cherry Springs Ranch/Hwy 105										
	Peal	Hour: 1:00	pm to 2:00	) pm						
		Long Range	e Weekday							
		Hwy	105							
EB LT	EB Thru	EB Thru EB RT WB LT WB Thru WB RT								
0	312	0	0	351	4					
		Cherry Spr	ings Ranch							
SB RT SB Thru SB LT NB RT NB Thru NB LT										
2 0 6 0 0 0										
Peak Hou	r Volume:	675								

Canterbury/Hwy 105									
	Peal	Hour: 1:00	pm to 2:00	) pm					
		Long Range	e Weekday						
		Hwy	105						
EB LT	EBLT EBThru EBRT WBLT WBThru WBRT								
0	416	40	20	305	0				
	Canterbury								
SB RT SB Thru SB LT NB RT NB Thru NB LT									
0	0	0	11	0	25				

Peak Hour Volume:

817

Canterbury/Saddlewood									
	Peal	Hour: 1:00	pm to 2:0	0 pm					
		Long Rang	e Weekday						
		Saddle	ewood						
EB LT	EB Thru EB RT WB LT WB Thru WB RT								
0	0	0	6	0	26				
		Cante	rbury						
SB RT	SB RT SB Thru SB LT NB RT NB Thru NB LT								
0 6 45 6 4 0									
Dook Hou	- M-1	0.2							

Peak Hour Volume:

		Appaloosa	a/Hwy 105						
	Peal	k Hour: 1:00	) pm to 2:0	) pm					
		Long Range	e Weekend						
		Hwy	105						
EB LT	EB Thru	EB Thru EB RT WB LT WB Thru WB RT							
0	368	14	9	339	0				
		Appa	loosa						
SB RT	SB Thru	SB LT	NB RT	NB Thru	NB LT				
0 0 0 3 0 2									
Peak Hou	r Volume:	735							

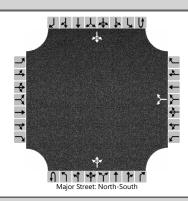
	Cherry Springs Ranch/Hwy 105										
	Peal	Hour: 1:00	pm to 2:00	) pm							
		Long Range	e Weekend								
		Hwy	105								
EB LT	EB Thru	EB RT	WB LT	WB Thru	WB RT						
0	385	0	0	333	4						
		Cherry Spr	ings Ranch								
SB RT	SB Thru SB LT NB RT NB Thru NB LT										
4	0	0 2 0 0 0									

Peak Hour Volume: 728

# LONG RANGE HORIZON LEVEL OF SERVICE (LOS)

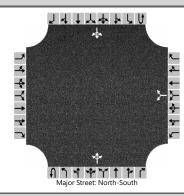


HCS7 Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood					
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County					
Date Performed	7/13/2023	East/West Street	Saddlewood					
Analysis Year	2023	North/South Street	Canterbury					
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.85					
Intersection Orientation	North-South Analysis Time Period (hrs) 0.25							
Project Description	Colorado Kids Ranch							



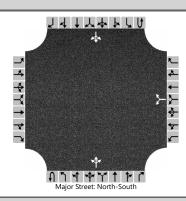
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	oound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						5		18		0	0	0		138	0	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							27			0				162		
Capacity, c (veh/h)							912			1623				1623		
v/c Ratio							0.03			0.00				0.10		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.3		
Control Delay (s/veh)							9.1			7.2				7.5		
Level of Service (LOS)							А			А				А		
Approach Delay (s/veh)						9	.1				7.5					
Approach LOS						,	Α									

HCS7 Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood					
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County					
Date Performed	7/13/2023	East/West Street	Saddlewood					
Analysis Year	2023	North/South Street	Canterbury					
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.85					
Intersection Orientation	North-South Analysis Time Period (hrs) 0.25							
Project Description	Colorado Kids Ranch							



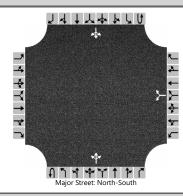
Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westk	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						6		26		0	4	6		45	6	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	)									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T					7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							38			0				53		
Capacity, c (veh/h)							1018			1614				1607		
v/c Ratio							0.04			0.00				0.03		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.1		
Control Delay (s/veh)							8.7			7.2				7.3		
Level of Service (LOS)					A		A					А				
Approach Delay (s/veh)		8.7					0.0				6.5					
Approach LOS					A											

HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Saddlewood							
Analysis Year	2023	North/South Street	Canterbury							
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



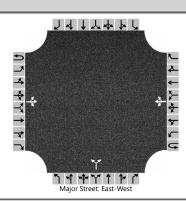
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	oound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR				LTR				LTR	
Volume (veh/h)						0		26		0	3	3		154	6	0
Percent Heavy Vehicles (%)						2		2		2				2		
Proportion Time Blocked																
Percent Grade (%)						(	0									
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2		4.1				4.1		
Critical Headway (sec)						7.12		6.22		4.12				4.12		
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2		
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)							31			0				181		
Capacity, c (veh/h)							1078			1614				1614		
v/c Ratio							0.03			0.00				0.11		
95% Queue Length, Q <sub>95</sub> (veh)							0.1			0.0				0.4		
Control Delay (s/veh)							8.4			7.2				7.5		
Level of Service (LOS)					A		A					А				
Approach Delay (s/veh)					8.4			0.0			7.3					
Approach LOS					A											

HCS7 Two-Way Stop-Control Report									
General Information		Site Information	te Information						
Analyst	Eric Maxwell	Intersection	Canterbury & Saddlewood						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Saddlewood						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85						
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25						
Project Description Colorado Kids Ranch									



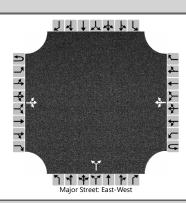
Vehicle Volumes and Ad	justme	nts															
Approach		Eastk	ound			Westl	oound			North	bound			Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration							LR				LTR				LTR		
Volume (veh/h)						0		71		0	18	3		157	9	0	
Percent Heavy Vehicles (%)						2		2		2				2			
Proportion Time Blocked																	
Percent Grade (%)						(	0										
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)						7.1		6.2		4.1				4.1			
Critical Headway (sec)						7.12		6.22		4.12				4.12			
Base Follow-Up Headway (sec)						3.5		3.3		2.2				2.2			
Follow-Up Headway (sec)						3.52		3.32		2.22				2.22			
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)							84			0				185			
Capacity, c (veh/h)							1054			1609				1590			
v/c Ratio							0.08			0.00				0.12			
95% Queue Length, Q <sub>95</sub> (veh)							0.3			0.0				0.4			
Control Delay (s/veh)							8.7			7.2				7.6			
Level of Service (LOS)							А		A					Α			
Approach Delay (s/veh)					8.7			0.0				7.2					
Approach LOS					A												

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.93						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Kids Ranch									



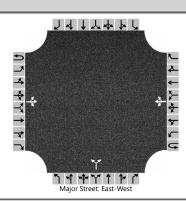
Vehicle Volumes and Adju	stme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	310	80		51	335	0		14		11				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				55					27					
Capacity, c (veh/h)		1198				1140					365					
v/c Ratio		0.00				0.05					0.07					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.2					
Control Delay (s/veh)		8.0				8.3					15.6					
Level of Service (LOS)		A				A			С							
Approach Delay (s/veh)	0.0			1.5			15.6									
Approach LOS						С										

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.90						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Colorado Kids Ranch								



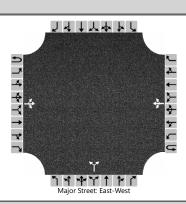
<b>Vehicle Volumes and Adju</b>	stme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	416	40		20	305	0		25		11				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Hea	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				22					40					
Capacity, c (veh/h)		1220				1058					320					
v/c Ratio		0.00				0.02					0.12					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1					0.4					
Control Delay (s/veh)		8.0				8.5					17.8					
Level of Service (LOS)		A				A			С							
Approach Delay (s/veh)	0.0			0.7			17.8									
Approach LOS							С									

HCS7 Two-Way Stop-Control Report									
General Information		Site Information	Site Information						
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.85						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Kids Ranch									



Vehicle Volumes and Adju	stme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	252	91		61	244	0		24		6				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Hea	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				72					35					
Capacity, c (veh/h)		1275				1155					333					
v/c Ratio		0.00				0.06					0.11					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					0.4					
Control Delay (s/veh)		7.8				8.3					17.1					
Level of Service (LOS)		A				A			С							
Approach Delay (s/veh)	0.0 2.2					17.1										
Approach LOS				,	4		С									

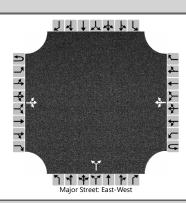
HCS7 Two-Way Stop-Control Report									
General Information		Site Information	nation						
Analyst	Eric Maxwell	Intersection	Canterbury & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Canterbury						
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.85						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Kids Ranch									



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	347	100		63	317	0		56		33				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	0					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				74					105					
Capacity, c (veh/h)		1186				1041					277					
v/c Ratio		0.00				0.07					0.38					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					1.7					
Control Delay (s/veh)		8.0				8.7					25.7					
Level of Service (LOS)		А				А					D					
Approach Delay (s/veh)		0.0 2.1				-	25.7					-	•	_		
Approach LOS							D									

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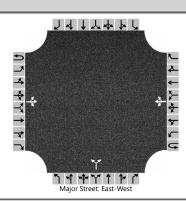
HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Appaloosa						
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.87						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	Project Description Colorado Kids Ranch								



Vehicle Volumes and Adju	stme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	233	8		5	427	0		7		7				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up Hea	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				6					16					
Capacity, c (veh/h)		1073				1286					445					
v/c Ratio		0.00				0.00					0.04					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1					
Control Delay (s/veh)		8.4				7.8					13.4					
Level of Service (LOS)		Α				Α					В					
Approach Delay (s/veh)	0.0 0.1					13.4										
Approach LOS	A					В										

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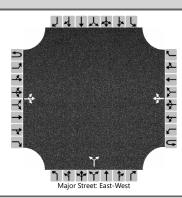
HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.98							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Colorado Kids Ranch									



<b>Vehicle Volumes and Adj</b>	ustme	nts														
Approach	T	Eastk	oound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	314	5		12	344	0		7		9				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				12					16					
Capacity, c (veh/h)		1208				1234					493					
v/c Ratio		0.00				0.01					0.03					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1					
Control Delay (s/veh)		8.0				7.9					12.6					
Level of Service (LOS)		А				А					В					
Approach Delay (s/veh)		0.0 0.4						12.6								
Approach LOS		A						В								

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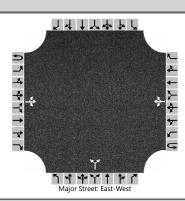
HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105						
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County						
Date Performed	7/13/2023	East/West Street	Hwy 105						
Analysis Year	2023	North/South Street	Appaloosa						
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.90						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description Colorado Kids Ranch									



Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	232	5		5	421	0		9		3				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)											)					
Right Turn Channelized																
Median Type   Storage				Undi	ivided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T	4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T	0				6					13					
Capacity, c (veh/h)		1094				1301					387					
v/c Ratio		0.00				0.00					0.03					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1					
Control Delay (s/veh)		8.3				7.8					14.6					
Level of Service (LOS)		A				А					В					
Approach Delay (s/veh)		0.0 0.1					•	14.6					•			
Approach LOS		A						В								

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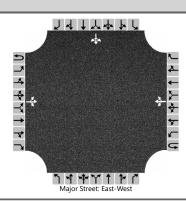
HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Appaloosa & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/13/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Appaloosa							
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.90							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



Approach		Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	T	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			LTR				LTR				LR					
Volume (veh/h)		0	368	14		9	339	0		2		3				
Percent Heavy Vehicles (%)		2				2				2		2				
Proportion Time Blocked																
Percent Grade (%)										(	)					
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1		6.2				
Critical Headway (sec)		4.12				4.12				7.12		6.22				
Base Follow-Up Headway (sec)		2.2				2.2				3.5		3.3				
Follow-Up Headway (sec)		2.22				2.22				3.52		3.32				
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				10					6					
Capacity, c (veh/h)		1182				1135					434					
v/c Ratio		0.00				0.01					0.01					
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		8.0				8.2					13.4					
Level of Service (LOS)		А				Α					В					
Approach Delay (s/veh)		0.0 0.3				.3	13.4									
Approach LOS								В								

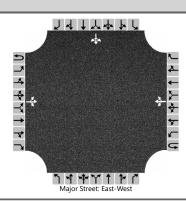
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HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	9:00-10:00 am Weekday	Peak Hour Factor	0.88							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



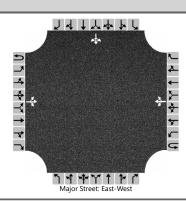
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration			LTR				LTR								LTR	
Volume (veh/h)		9	236	0		0	422	11						4	0	6
Percent Heavy Vehicles (%)		2				2								2	2	2
Proportion Time Blocked																
Percent Grade (%)														(	)	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		10				0									11	
Capacity, c (veh/h)		1071				1295									433	
v/c Ratio		0.01				0.00									0.03	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.1	
Control Delay (s/veh)		8.4				7.8									13.5	
Level of Service (LOS)		A				A									В	
Approach Delay (s/veh)		0.4			0.0							13.5				
Approach LOS											В					

HCS7 Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105							
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County							
Date Performed	7/20/2023	East/West Street	Hwy 105							
Analysis Year	2023	North/South Street	Cherry Springs							
Time Analyzed	1:00-2:00 pm Weekday	Peak Hour Factor	0.97							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description Colorado Kids Ranch										



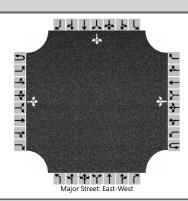
Vehicle Volumes and Adju	ıstme	nts															
Approach	Eastbound				Westbound					North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0	
Configuration			LTR				LTR								LTR		
Volume (veh/h)		0	312	0		0	351	4						6	0	2	
Percent Heavy Vehicles (%)		2				2								2	2	2	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2	
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22	
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3	
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32	
Delay, Queue Length, and	l Leve	l of S	ervice														
Flow Rate, v (veh/h)		0				0									8		
Capacity, c (veh/h)		1193				1238									410		
v/c Ratio		0.00				0.00									0.02		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.1		
Control Delay (s/veh)		8.0				7.9									14.0		
Level of Service (LOS)		А				А									В		
Approach Delay (s/veh)		0	.0			0.0							14.0				
Approach LOS													В				

HCS7 Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/20/2023	East/West Street	Hwy 105								
Analysis Year	2023	North/South Street	Cherry Springs								
Time Analyzed	9:00-10:00 am Weekend	Peak Hour Factor	0.93								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	Colorado Kids Ranch										



Vehicle Volumes and Adju	ıstme	nts															
Approach	Eastbound					Westbound				North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0	
Configuration			LTR				LTR								LTR		
Volume (veh/h)		15	230	0		0	404	28						8	0	13	
Percent Heavy Vehicles (%)		2				2								2	2	2	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2	
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22	
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3	
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32	
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)		16				0									23		
Capacity, c (veh/h)		1097				1319									464		
v/c Ratio		0.01				0.00									0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.2		
Control Delay (s/veh)		8.3				7.7									13.2		
Level of Service (LOS)		Α				Α									В		
Approach Delay (s/veh)		0	.6			0	.0						13.2				
Approach LOS											В						

HCS7 Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	Eric Maxwell	Intersection	Cherry Springs & Hwy 105								
Agency/Co.	SMH Consultants	Jurisdiction	El Paso County								
Date Performed	7/20/2023	East/West Street	Hwy 105								
Analysis Year	2023	North/South Street	Cherry Springs								
Time Analyzed	1:00-2:00 pm Weekend	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	Colorado Kids Ranch										



Vehicle Volumes and Adj	justme	nts															
Approach		Eastb	ound		Westbound					North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0	
Configuration			LTR				LTR								LTR		
Volume (veh/h)		0	385	0		0	333	4						2	0	4	
Percent Heavy Vehicles (%)		2				2								2	2	2	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1								7.1	6.5	6.2	
Critical Headway (sec)		4.12				4.12								7.12	6.52	6.22	
Base Follow-Up Headway (sec)		2.2				2.2								3.5	4.0	3.3	
Follow-Up Headway (sec)		2.22				2.22								3.52	4.02	3.32	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)		0				0									7		
Capacity, c (veh/h)		1192				1141									488		
v/c Ratio		0.00				0.00									0.01		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0									0.0		
Control Delay (s/veh)		8.0				8.2									12.5		
Level of Service (LOS)		А				А									В		
Approach Delay (s/veh)		0	.0		0.0							12.5					
Approach LOS										В							

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# ROAD IMPACT TRAFFIC FEE



## **Road Impact Fees**

### Pumpkin Patch

250 weekday trips

1250 weekend trips

$$ADT = \frac{(250*5) + (1250*2)}{7} = 536$$

$$AADT = \frac{536*7}{52} = 73$$

$$RF = 1 - \frac{50}{325} = 0.85$$

$$FEE = 73 * 398.55 * 0.85 = $24,370$$

ADT = event average daily

trips for period open

$$AADT = \frac{(ADT)*(\#of \ weeks \ open)}{52 \ weeks}$$

$$RF = 1 - \frac{50 \text{ initial spaces}}{\text{total parking spaces}}$$

$$FEE = AADT * $398.55 * RF$$

## **Tulip Festival**

204 weekday trips

428 weekend trips

$$ADT = \frac{(204*5) + (428*2)}{7} = 268$$

$$AADT = \frac{536*2}{52} = 11$$

$$RF = 1 - \frac{50}{325} = 0.85$$

$$FEE = 11 * 398.55 * 0.85 = $3,727$$

ADT = event average daily

trips for period open

$$AADT = \frac{(ADT)*(\#of weeks open)}{52 weeks}$$

$$RF = 1 - \frac{50 \text{ initial spaces}}{\text{total parking spaces}}$$

$$FEE = AADT * $398.55 * RF$$