Schmidt PUD

Master Development Drainage Plan/Preliminary Drainage Report

PCD File No: _____

All Terrain Engineering Project No: 24013

June 2026

Prepared for:

Turkey Canon Quarry Inc

20 Boulder Crescent Street, Suite 250

Colorado Springs, CO 80903

Attn: James Morley

Prepared by:

All Terrain Engineering LLC

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ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

_____

Nicholas Q. Jokerst, PE Date

State of Colorado No. 59273

For and on behalf of All Terrain Engineering LLC

DEVELOPER'S STATEMENT

I, the owner/developer, have read and will comply with all of the requirements specified in this drainage report and plan.

_____

James Morley Date

EL PASO COUNTY ONLY

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

_____

Joshua J. Palmer, P.E. Date
County Engineer/ECM Administrator

Conditions:

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I. General Purpose, Location & Description

a. Purpose & Project Description

The purpose of the Schmidt PUD - Master Development Drainage Plan/Preliminary Drainage Report (MDDP/PDR) is to:

- Identify major drainageways, detention/water quality areas, and locations of culverts, bridges, open channels and drainage areas that are tributary to and downstream of the Schmidt PUD site.
- Describe the site's onsite and offsite drainage patterns, existing and proposed storm infrastructure and demonstrate how the development will capture & treat developed runoff.
- Evaluate the ability of downstream drainage facilities to handle the site's developed runoff.

The Schmidt PUD is a 273 lot single family attached and detached planned unit development (PUD) located in El Paso County, Colorado. The Schmidt PUD is split into the North and South developments. The proposed Road A, connecting Black Forest Road and Brush Top Road, serves as the phase split.

For the purposes of this MDDP/PDR, the following definitions apply:

- *Predevelopment Condition(s)* refers to the native site prior to earth disturbing activities.
- *Existing Condition(s)* refers to the site after the completion of CDR2413 construction activities and final stabilization. This includes the completion of Marksheffel Road Segment M3, Brush Top Road, interim Pond A and associated earthwork/utility improvements.
- *Proposed Condition(s)* refers to the site after the completion of the Schmidt PUD development. This includes the completion of the Schmidt North & Schmidt South developments.
- *Ultimate Condition(s)* refers to the site following the completion of Marksheffel Road Segment M5, Segment M6 and Pond B. Ultimate condition timing will be dictated by Marksheffel Road Segment M5 bridge crossing at Cottonwood Creek.

b. Location

Schmidt PUD, referred to as 'the site' herein, is an unplatted parcel in El Paso County, Colorado (*TSN# 5200000577*). The site is bound by Holiday Hills Filing 1 and Silver Ponds Filing 1 to the north, Black Forest Road to the west, Vollmer Road to the east and Trails at Forest Meadows Filing 3 & 4 to the south. A vicinity map is presented in Appendix A.

c. Description of Property

The site is approximately 97.54 acres. Historically, a portion of the site functioned as a quarry while the remainder was undeveloped. In general, the predevelopment site slopes south and west towards Cottonwood Creek. A portion of the predevelopment site drains east towards Sand Creek. Onsite elevations range from 6965' - 7040' with slopes ranging from 1 to greater than 50%. Per an NRCS soil survey, the site is made up of Type A Soils Blakeland loamy sand/Columbine gravelly sandy loam and Type B soils Pring coarse sandy loam/Stapleton-Bernal sandy loams. On-site vegetation includes prairies grasses and dispersed upland trees such as Cottonwoods. Cottonwood Creek bisects the western portion of the site. On-site, existing utilities include water and sanitary stubs, storm sewer, and communications. There are no known permanent irrigation facilities on site.

The ultimate receiving waters for the site are Cottonwood Creek & Sand Creek.

d. Floodplain Statement

Based on FEMA Firm map 08041C0529G dated December 7, 2018, the site is Zone A, AE & X. Zone X are areas determined to be outside the 0.2% annual chance flood. Zone A are areas within the 1% annual chance of flooding. Zone AE are areas within the 1% annual chance of flooding and have Base Flood Elevations determined. Developed portions of the Schimdt PUD will be limited to Zone X areas of the site.

II. Drainage Basins

a. Major Basin Description

The site is located within the Sand Creek & Cottonwood Creek Drainage Basins. Drainage studies relevant to the site's stormwater analysis include:

- Cottonwood Creek Drainage Basin Planning Study, URS Consultants, June 9, 1994.
- Sand Creek Drainage Basin Planning Study Preliminary Design Report, Kiowa Engineering Corporation, January 1993. Revised October 1995.
- Silver Ponds Subdivision Filing No. 1 Final Drainage Report, M.V.E., Inc., February 1995. Revised May 1996.
- Trails at Forest Meadows Filing 3 Final Drainage Report, M&S Civil Consultants, Inc., August 2015.
- Trails at Forest Meadows Filing 4 Final Drainage Report – Amendment To: MDDP Plan Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No. 21 and No. 4, M&S Civil Consultants, Inc., April 2016.
- Marksheffel Road – Segment M3 CDR Final Drainage Report, All Terrain Engineering LLC. Currently under El Paso County review.

Per the Cottonwood Creek DBPS, the site is within Basin B8 & B9. The Cottonwood Creek onsite reach is *Design Point 6 to Design Point 7 (Reach DP67)* and has an existing 100-year flow of 854 cfs. The bridge at the crossing of Reach DP67 and Black Forest Road was identified for replacement by the DBPS. City of Colorado Springs recently completed these improvements. Per the DBPS, Reach DP67 should be improved with a natural bottom with grass/shrub banks and grade control structures (9 drops).

Per the Sand Creek DBPS, a portion of the site is located within the Upper Sand Creek subbasin. This basin drains to Reach SC2-SC8. Regional detention was recommended for this portion of Sand Creek. However, downstream infrastructure conveyance capacity may be limited and thus require onsite detention alternatives.

The Final Drainage Report for El Paso County Project No. CDR2413 identified a discrepancy between the Sand Creek & Cottonwood Creek drainage basin limits. A deviation is under review with El Paso County to address the basin limit discrepancy and allow for a Sand Creek to Cottonwood Creek basin swap. The Schmidt PUD MDDP/PDR assumes the deviation will be granted. However, the MDDP/PDR will be updated accordingly as deviation approval progresses.

b. Predevelopment Basin Descriptions

The site's predevelopment drainage condition is per the *Existing Subbasin Description* section of Marksheffel Road – Segment M3 CDRs Final Drainage Report (M3 FDR) and is provided for context only. The site will enter the *Existing Condition* prior to the Schmidt PUD. Refer to Appendix E for complete predevelopment sub-basin breakdown and descriptions from the M3 FDR. Based upon existing topography, the site's drainage is split with approximately 79.5 acres draining towards Cottonwood Creek and approximately 18.04 acres draining towards Sand Creek.

Basins EXA, EXD, EXG, EXH, and EXI drain to Cottonwood Creek. The site's predevelopment condition discharge to Cottonwood Creek Drainage Basin is Q_5 = 9.2 cfs Q_{100} = 60.8 cfs at DP7.1 Q_5 = 1.6 cfs Q_{100} = 10.7 cfs at DP8, and Q_5 = 2.4 cfs Q_{100} = 16.3 cfs at DP9. Total predevelopment site discharge to Cottonwood Creek is **Q_5 = 13.3 cfs Q_{100} = 88.3 cfs**.

Basins EXB, EXC, EXE, EXF and EXJ drain to Sand Creek. The site's predevelopment condition discharge to the Sand Creek Drainage Basin is Q_5 = 0.7 cfs Q_{100} = 2.8 cfs at DP5, Q_5 = 0.6 cfs Q_{100} = 4.1 cfs at DP6 and Q_5 = 0.1 cfs Q_{100} = 0.5 cfs at DP10. DP5 is conveyed south along Vollmer Road. DP6 and DP10 are conveyed through Trails at Forest Meadows Filing 3 & 4 to Sand Creek.

c. Existing Basin Descriptions

The site's existing drainage condition reflects the *Proposed Subbasin Description* section of Marksheffel Road – Segment M3 CDRs Final Drainage Report (M3 FDR). However, Existing Basin K, Existing Basin C and Existing Basin H have been updated to reflect their assumed developed condition to ensure downstream facilities are adequately sized. The site's existing condition includes 17 onsite basins and 1 offsite basin.

Existing Basin OSI4 is 27.16 acres of Silver Ponds Subdivision Filing 1. Values for this basin reflect the "Silver Ponds Subdivision Filing No.1 Final Drainage Report". Runoff from this basin (Q_5=19.0 cfs, Q_{100}=44.2 cfs) flows south and is intercepted by the existing swale/berm that directs runoff around the site. Flows are diverted to the west towards DP14 and enter Cottonwood Creek. Refer to the Silver Ponds FDR excerpts fin Appendix E for reference of Existing Basin OSI4 drainage patterns. Basin OSI4 will not be captured nor detained in Pond A.

Existing Basin A is 0.25 acres of an existing berm with stabilized vegetation. Runoff from this basin (Q_5=0.0 cfs, Q_{100}=0.2 cfs) flows along the berm west and enters an existing swale at Existing DP1 and flows offsite to Cottonwood Creek at Existing DP14 (Q_5 = 19.0 cfs, Q_{100} = 44.2 cfs). Water quality will not be provided for Existing Basin A as it will remain undeveloped.

Existing Basin B is 0.09 acres of an existing berm with stabilized vegetation. Runoff from this basin (Q_5=0.0 cfs, Q_{100}=0.1 cfs) follows historic drainage patterns along the berm east to Pond W-4 along Vollmer Road. Water quality will not be provided for Existing Basin B on site as it will undeveloped and maintain historic drainage patterns to Pond W-4.

Existing Basin C is 1.12 acres of future residential development (by others) and a portion of Marksheffel Road. Runoff from this basin (Q_5=3.7 cfs, Q_{100}=7.5 cfs) flows overland south to an existing 5' CS Type 2 Inlet

(sump) at Existing DP3. Existing DP3 flows are piped to Existing DP4.1 (Q_5 = 6.7 cfs, Q_{100} = 13.7 cfs). In the event of inlet failure at Existing DP3, the flow will overtop the Marksheffel Road crown and flow to the inlet at DP4 and Brush Top Road. Water quality and detention is provided for Existing Basin C in Pond A.

Existing Basin D is 1.18 acres of Marksheffel Road. Runoff from this basin (Q_5=3.2 cfs, Q_{100}=6.1 cfs) flows to an existing 5' CS Type 2 Inlet (sump) at Existing DP4. Existing DP4 flows are piped to Existing DP4.1 (Q_5 = 6.7 cfs, Q_{100} = 13.7 cfs). In the event of inlet failure at Existing DP4, the flow would continue south in Brush Top Road. Existing DP4.1 flow continues to Existing DP5 (Q_5 = 5.6 cfs, Q_{100} = 17.2 cfs) in Existing Swale E1. Water quality and detention is provided for Existing Basin D in Pond A.

Existing Basin E is 6.66 acres of undeveloped land. Runoff from this basin (Q_5 = 1.2 cfs & Q_{100} = 8.5 cfs) flows overland south and is captured by Existing Swale E1 at Existing DP5 (Q_5 = 5.6 cfs, Q_{100} = 17.2 cfs). Flows at Existing DP5 continue overland to Existing Swale L1 at Existing DP5.1. Water quality and detention will be provided for the developed condition of Existing Basin E in Pond A.

Existing Basin F is 2.56 acres of Brush Top Road and undeveloped land. Runoff from this basin (Q_5 = 1.8 cfs & Q_{100} = 5.4 cfs) flows to a 15' Type R on-grade inlet at Existing DP6. Captured flows at Existing DP6 are piped to Existing DP7.1. Water quality and detention is provided for Existing Basin F in Pond A.

Existing Basin G is 0.78 acres of Brush Top Road. Runoff (Q_5 = 2.0 cfs & Q_{100} = 3.8 cfs) flows to a 15' Type R on-grade inlet at Existing DP7. Captured flows are piped to Existing DP7.1 (Q_5 = 3.4 cfs & Q_{100} = 8.3 cfs). Flows at Existing DP7.1 flows are piped to Existing DP11.2 (Q_5 = 44.2 cfs, Q_{100} = 112.2 cfs) and continue to DP5.1 in Existing Swale L1. Water quality and detention is provided for Existing Basin G in Pond A.

Existing Basin H is 3.59 acres of future residential development (by others) and undeveloped land. Runoff from this basin (Q_5 = 6.3 cfs & Q_{100} = 13.4 cfs) sheet flows south to Marksheffel Road. Flows combine with Existing Basin I at Existing DP9 (Q_5 = 2.9 cfs, Q_{100} = 18.5 cfs) where an existing 10' CS Type 2 inlet (sump) captures the flow and conveys it to Existing DP10.1. Water quality and detention is provided for Existing Basin H in Pond A.

Existing Basin I is 1.22 acres of Marksheffel Roadway. Runoff from this basin (Q_5=2.9 cfs, Q_{100}=5.6 cfs) flows to a 10' CS Type 2 Inlet (sump) at Existing DP9 (Q_5 = 2.9 cfs, Q_{100} = 18.5 cfs). Existing DP9 flows are piped to Existing DP10.1. In the event of inlet failure at Existing DP9, the flow will overtop the Marksheffel Road crown and flow to the inlet at DP10 and to Vollmer Road. Water quality and detention is provided for Existing Basin I in Pond A.

Existing Basin J is 1.28 acres of Marksheffel Road. Runoff from this basin (Q_5=3.5 cfs, Q_{100} = 6.5 cfs) flows to an existing 10' CS Type 2 Inlet (sump) at Existing DP10 (Q_5=3.5 cfs, Q_{100} = 6.5 cfs). Captured flows are piped to Existing DP10.1 (Q_5 = 5.3 & Q_{100} = 13.9 cfs). Flows from Existing DP10.1 continue to Existing DP11.1 (Q_5 = 40.9 cfs & Q_{100} = 84.8 cfs). In the event of inlet failure at Existing DP10, flows will flow east to Vollmer Road. Water quality and detention is provided for Existing Basin J in Pond A.

Existing Basin K is 24.33 acres of future residential development (by others). Runoff from this basin (Q_5 = 31.8 cfs, Q_{100} = 66.5 cfs) sheet flows south to Existing Swale K1 at Existing DP11 (Q_5 = 31.8 cfs, Q_{100} = 66.5 cfs).

Flows at Existing DP11 enter a 36" FES and are piped to Existing DP11.1 (Q_5 = 40.9 cfs & Q_{100} =84.8 cfs). Flows at DP11.1 continue to Existing DP11.2. Water quality and detention is provided for the developed condition of Basin K in Pond A.

Existing Basin L is 36.07 acres of undeveloped land. Runoff from this basin (Q_5 = 4.6 cfs & Q_{100} = 34.1 cfs) flows to Existing Swale L1 at Existing DP13 (Q_5 = 4.6 & Q_{100} = 34.1). Flows enter Existing Swale L1 and continue to Existing DP13.1 (Q_5 = 37.8 cfs, Q_{100} = 107.6 cfs) 1. Existing DP13.1 flows enter existing, Interim Pond A at Existing DP14. Water quality and detention will be provided for the developed condition of Existing Basin L in Pond A.

Existing Basin M is 4.86 acres of undeveloped land and existing Interim Pond A. Runoff from this basin (Q_5 = 0.7 cfs & Q_{100} = 5.4 cfs) sheet flows directly to existing, Interim Pond A at Existing DP14. Water quality and detention will be provided for the developed condition of Existing Basin M in Pond A.

Existing Basin N is 0.34 acres of undeveloped land. Runoff (Q_5 = 0.1 cfs, Q_{100} = 0.5 cfs) sheet flows east, offsite towards Vollmer Road. Flows are distributed along the length of the basin and are not concentrated. The basin is undeveloped land that will remain undeveloped and follow historic drainage patterns towards Vollmer Road.

Existing Basin O is 0.56 acres of undeveloped land and a portion of Brush Top Road. Basin O runoff (Q_5 = 0.1 cfs and Q_{100} = 0.9 cfs) overland flows south to Trails at Forest Meadows Filing No. 3 at DP16.

Existing Basin P is 2.68 acres of undeveloped land. Runoff (Q_5 = 0.6 cfs and Q_{100} = 4.2 cfs) follows historic drainage patterns south and west to Existing DP17.

Existing Basin Q 10.29 acres of undeveloped land. Runoff from this basin (Q_5=1.5 cfs, Q_{100}=10.8 cfs) flows overland south and to Cottonwood Creek at DP18.

Existing Basins A, C-M, P, & Q drain to Cottonwood Creek. The site's existing condition discharge to Cottonwood Creek Drainage Basin is Q_5 = 0.1 cfs Q_{100} = 40.9 cfs at DP14.1 (Interim Pond A Outfall) Q_5 = 0.6 cfs Q_{100} = 4.2 cfs at DP17 and Q_5 = 1.5 cfs Q_{100} = 10.8 cfs at DP18.

Basins O & N drain to Sand Creek. The site's predevelopment discharge to the Sand Creek Drainage Basin is Q_5 = 0.1 cfs Q_{100} = 0.9 cfs at DP16 and Q_5 = 0.1 cfs Q_{100} = 0.5 cfs along Basin N.

Existing Basins EXB3, EXB4, EXE2 & EXF1 are offsite basins within Silver Ponds Filing 1. Silver Ponds Filing 1 drainage basin patterns shall be maintained.

d. Proposed Subbasin Description

Schmidt North and South are divided into18 proposed basins. For basin descriptions and maps associated with CDR2413 improvements, refer to Appendix E. Marksheffel Road Segments M4-M6 are analyzed for their ultimate condition. Coordination with City of Colorado Springs is ongoing to determine the design and timing of M4-M6 improvements. Preliminary stormwater considerations for M4-M6 are made within the presented

subbasin analysis. A final drainage report(s) will be provided during the construction document phase of M4-M6 to detail final stormwater infrastructure & water quality design. The Schmidt PUD development will proceed prior to the construction of M4-M6 due to the delayed timing of the Cottonwood Creek bridge within M5. As coordination progresses, necessary updates and amendments will be made to the MDDP/PDR. Schmidt North and South will provide subsequent Final Drainage Reports, during the final plat phase, to finalize the preliminary subbasin and stormwater infrastructure design herein.

Basin MS1 is 0.28 acres of Marksheffel Road. Basin MS1 stormwater (Q_5 = 1.0 cfs Q_{100} = 2.0 cfs) will be captured in curb and gutter and conveyed to DPEX3. An existing 5' CS Type 2 sump inlet at DPEX3 captures Basin MS1 and conveys it onsite at DPEX4.1 (Q_5 = 6.7 cfs Q_{100} = 13.1 cfs). Basin MS1 will be detained in Pond A for water quality and detention.

Basin MS2 is 1.76 acres of Marksheffel Road. Basin MS2 stormwater (Q_5 = 4.3 cfs Q_{100} = 8.9 cfs) will be captured in curb and gutter and conveyed to DP1. A future 15' CS Type 2 on-grade inlet at DP1 captures Basin MS2 and conveys it onsite at DP2 (Q_5 = 8.5 cfs Q_{100} = 17.0 cfs). Basin MS2 will be detained in Pond A for water quality and detention.

Basin MS3 is 0.34 acres of Marksheffel Road. Basin MS3 stormwater (Q_5 = 1.3 cfs Q_{100} = 2.5 cfs) will be captured in curb and gutter and conveyed to DPEX4. An existing 5' CS Type 2 sump inlet at DPEX4 captures Basin MS3 and conveys it onsite at DPEX4.1 (Q_5 = 6.7 cfs Q_{100} = 13.1 cfs). Basin MS3 will be detained in Pond A for water quality and detention.

Basin MS4 is 1.41 acres of Marksheffel Road. Basin MS4 stormwater (Q_5 = 4.4 cfs Q_{100} = 8.5 cfs) will be captured in curb and gutter and conveyed to DP2. A future 15' CS Type 2 on-grade inlet at DP2 captures Basin MS2 and conveys it onsite at DP2 (Q_5 = 8.5 cfs Q_{100} = 17.0 cfs). Basin MS4 will be detained in Pond A for water quality and detention.

Basin MS5 is 0.78 acres of Marksheffel Road. Basin MS5 stormwater (Q_5 = 2.4 cfs Q_{100} = 5.0 cfs) will be captured in curb and gutter and conveyed to DP3. Future 15' CS Type 2 on-grade inlet at DP3 captures Basin MS5 and conveys it to Pond B. Basin MS5 will be detained in Pond B for water quality and detention. Final design of Pond B and associated storm sewer will be deferred until the Marksheffel Road bridge at Cottonwood Creek is designed by the City of Colorado Springs.

Basin MS6 is 2.57 acres of Marksheffel Road. Basin MS6 stormwater (Q_5 = 7.1 cfs Q_{100} = 13.8 cfs) will be captured in curb and gutter and conveyed to DP3. Future 15' CS Type 2 on-grade inlet at DP3 captures Basin MS6 and conveys it to Pond B. Basin MS6 will be detained in Pond B for water quality and detention. Final design of Pond B and associated storm sewer will be deferred until the Marksheffel Road bridge at Cottonwood Creek is designed by the City of Colorado Springs.

Basin A is 8.97 acres of Schmidt PUD North and South. Basin A stormwater (Q_5 = 10.3 cfs Q_{100} = 24.8 cfs) will be conveyed in public and private roadways to public and private inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (4) 15' Type R on-grade inlets will capture Basin A flow and convey to DP4 (Q_5 = 10.3 cfs Q_{100} = 24.8 cfs). Offsite flows at DPEX4.1 will be captured by storm sewer within Basin A and piped through

Basin B to DP5 (Q_5 = 26.3 cfs Q_{100} = 57.8 cfs). DP4 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin B is 9.44 acres of Schmidt PUD North. Basin B stormwater (Q_5 = 12.7 cfs Q_{100} = 30.6 cfs) will be conveyed in private roadways to private Type R inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (6) 15' Type R on-grade inlets will capture Basin B flow and convey to DP5 (Q_5 = 26.3 cfs Q_{100} = 57.8 cfs). DP5 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin C is 9.76 acres of Schmidt PUD South. Basin C stormwater (Q_5 = 11.27 cfs Q_{100} = 27.0 cfs) will be conveyed in public roadways to public Type R inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (5) 15' Type R on-grade inlets will capture Basin C flow and convey to DP6 (Q_5 = 54.0 cfs Q_{100} = 116.5 cfs). Offsite flows at DPEX11.2 will be captured by the Basin C storm sewer and piped to DP6. DP6 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin D is 6.00 acres of Schmidt PUD South. Basin D stormwater (Q_5 = 7.9 cfs Q_{100} = 19.0 cfs) will be conveyed in public roadways to public Type R inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (4) 15' Type R on-grade inlets will capture Basin D flow and convey to DP7 (Q_5 = 17.2 cfs Q_{100} = 41.5 cfs). DP7 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin E is 5.37 acres of Schmidt PUD North. Basin E stormwater (Q_5 = 7.8 cfs Q_{100} = 18.9 cfs) will be conveyed in private roadways to private Type R inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (4) 15' Type R inlets will capture Basin E flow and convey to DP8 (Q_5 = 94.2 cfs Q_{100} = 208.9 cfs). DP8 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin F is 3.63 acres of Schmidt PUD South. Basin F stormwater (Q_5 = 7.2 cfs Q_{100} = 15.6 cfs) will be conveyed in public roadways to public Type R inlets. Stormwater will be captured by a series of Type R inlets along the proposed roadways. Based upon preliminary street and inlet capacity analysis, a total of (2) 15' Type R inlets will capture Basin F flow and convey to DP9 (Q_5 = 7.2 cfs Q_{100} = 15.6 cfs). DP9 continues through the proposed storm sewer to Pond A for water quality and detention. Subsequent FDRs will detail final inlet locations and storm sewer hydraulics.

Basin G is 4.18 acres of Schmidt PUD South and Pond A. Basin G stormwater (Q_5 = 2.1 cfs Q_{100} = 10.5 cfs) will flow directly to Pond A.

Basin H is 0.29 acres of Schmidt PUD South roadway and intersection at Black Forest Road. Basin H stormwater (Q_5 = 0.7 cfs Q_{100} = 1.6 cfs) will be conveyed in public curb & gutter to Black Forest Road at DP11.

Additional survey will be required during the Schmidt South FDR to verify & update the Basin H drainage patterns. Basin H is located downstream of Pond A. Basin H will meet an EPC Appendix I water quality exclusion detailed with subsequent FDRs.

Basin I is 0.08 acres of Salt Brush Road and Schmidt South undeveloped area. Basin I stormwater (Q_5 = 0.2 cfs Q_{100} = 0.5 cfs) will be conveyed south in Salt Brush Road curb & gutter at DP12. Existing elevations at the proposed roadway connection at Salt Brush make Basin I impractical to detain in Pond A. Basin I will meet an EPC Appendix I water quality exclusion detailed with subsequent FDRs.

Basin J is 0.47 acres of Schmidt South undeveloped area. Basin J stormwater (Q_5 = 0.2 cfs Q_{100} = 0.9 cfs) will follow historic drainage patterns south to an existing 48" RCP FES at DP13.

Basin K is 8.99 acres of Schmidt North undeveloped area and Cottonwood Creek. Basin K stormwater (Q_5 = 0.7 cfs Q_{100} = 4.8 cfs) will follow historic drainage patterns south in Cottonwood Creek to DP14 (Q_5 = 3.5 cfs Q_{100} = 58.1 cfs). Basin K will remain undeveloped in the Schmidt PUD.

Silver Ponds Filing 1 Existing Basins will be bypassed to their historic outfall locations. Bypass design will be provided with Marksheffel Road M5 & M6 FDR.

PREDEVELOPMENT V. PROPOSED FLOW COMPARISON IN COTTONWOOD CREEK

Prevel. DP	Proposed DP	Predev $Q_{5\text{-YR}}$	Predev $Q_{100\text{-YR}}$	Pr $Q_{5\text{-YR}}$	Pr $Q_{100\text{-YR}}$
Predevel. DP7.1, 8 & 9	DP14	13.3	88.3	3.5	58.1

The Schmidt PUD will not result in an increase in peak flows to Cottonwood Creek.

Drainage Design Criteria

a. Development Criteria Reference

The drainage analysis follows the criteria from the "Drainage Criteria Manual County of El Paso, Colorado" Volumes 1 and 2," as amended.

b. Hydrologic Criteria

Onsite drainage analysis included the 5-year storm (minor event) and 100-year storm (major event). Runoff is calculated per EPCDCM Chapter 5 – Storm Runoff Method of Analysis. 1-hr duration rainfalls depths from NOAA Atlas 14 are utilized for water quality and detention volume calculation. Private, full spectrum pond design was completed using the latest version of Mile High Flood District's (MHFD) UD-Detention. Proposed detention facilities will be limited to historic release rates.

c.

d. Hydraulic Criteria

Hydraulic criteria for channel analysis are obtained from EPCDCM Chapter 10 - Open Channels and Structures. Storm inlet sizing was performed with MHFD-Inlet v5.03. Culvert and swale calculations were performed using the latest version of Hydraflow Expression AutoCAD Extension.

III. Drainage Facility Design

a. General Concept

The Schmidt PUD will develop 273 single family attached & detached lots. Public and private storm sewer will capture and convey stormwater to Pond A. An interim condition of Pond A will be constructed with CDR2413 prior to the Schmidt PUD project. The interim condition of Pond A will provide water quality & detention for Marksheffel Road Segment M3 and Brush Top Road. With the Schmidt PUD, Pond A will be expanded to provide water quality and detention for the entire PUD area, Marksheffel Road Segment M4 and the assumed developed condition of the Stonebridge parcel. The expansion of Pond A will occur with the initial final plat of the Schmidt PUD area. Marksheffel Road Segment M4 & M6 improvements will occur in the future along with City of Colorado Springs coordination on the Segment M5 bridge at the Cottonwood Creek crossing. However, Pond A will be sized for the assumed, ultimate build out of Segment M4. A preliminary design of Pond B provides water quality and detention to Segment M5 & M6. Additional coordination with the City during the construction drawing phase of M6 will dictate Pond B's final location and design.

In general, Pond A & Pond B will provide water quality & detention for the Schmidt PUD area and supporting infrastructure. Disturbed areas that are not detained in Pond A & B must meet an EPC Appendix I water quality exclusion. Exclusions will be detailed during the Final Drainage Report/GEC phases of the Schmidt PUD.

b. Water Quality & Detention

Pond A will provide water quality and detention for existing Basin C, D, F-M (36.79 acres @ 73%) & proposed Basins MS1-MS4 & A-G (51.14 acres @ 60.2%). Pond A is an onsite, private, full spectrum detention pond northeast of the Road A and Black Forest Road intersection. A total of 87.93 acres at 65.6% composite imperviousness will be detained. The WQCV is 1.879 ac-ft, the EURV is 5.253 ac-ft, and the 100-year volume is 3.721 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 84 and 89 hours, respectively. Concrete forebays will be constructed at the outfall locations. 2.0' & 6.0' concrete trickle channels convey flow to the private outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate future maintenance. A 50' emergency overflow spillway conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Cottonwood Creek. The spillway and pipe outfall will be riprap lined. The outlet structure discharges to Cottonwood Creek via a private 30" RCP. Final design calculations for Pond A will be provided with the initial final plat/Final Drainage Repot for the Schmidt PUD.

Pond B will provide water quality and detention for Basins M5 & M6. It is anticipated that offsite basins north of Marksheffel Road Segment M5 & M6 will be bypassed to their historic outfalls. Pond B is an onsite, private, full spectrum detention pond southeast of the future Marksheffel Road and Black Forest Road intersection. A total of 3.82 acres at 65.1% composite imperviousness will be detained. The WQCV is 0.081 ac-ft, the EURV is 0.191 ac-ft, and the 100-year volume is 0.173 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 72 and 71 hours, respectively. A concrete forebay will be constructed at the outfall location. A 2.0'

concrete trickle channel conveys flow to the private outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate future maintenance. An 8' emergency overflow spillway conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Black Forest Road. The spillway and pipe outfall will be riprap lined. The outlet structure discharges via an 18" RCP. *Pond B design is provided for reference only. Coordination is ongoing with City of Colorado Springs. Final Pond B design to be provided with FDR for Marksheffel Road Segment M5*.

Pond design calculations are presented in Appendix D.

c. Major Drainageway

Cottonwood Creek Reach DP67 bisects the western portion of the site. The Cottonwood Creek DBPS recommends an improved natural bottom channel section with grass/shrub banks and grade control structures. The DBPS identifies 9 grade control structures along Reach DP67. A Channel Design Report will be provided with the initial final plat/FDR for the Schmidt PUD. The Channel Design Report will detail proposed stabilization measures, grade control structures, if warranted, potential wetland impacts and jurisdictional determinations and limits of FEMA CLOMR, if applicable. The northern limit of the onsite Cottonwood Creek portion is bound by future Marksheffel Road. Coordination with City of Colorado Springs is ongoing regarding the future Marksheffel Road bridge crossing at Cottonwood Creek. It will be necessary to coordinate timing of the two projects to accommodate adjacent improvements and avoid overlap.

d. Inspections & Maintenance

Inspection and Maintenance Manuals will be provided with the relevant Final Drainage Report and PCM Construction Plans for Pond A and Pond B.

e. Grading & Erosion Control Plan

Grading & Erosion Control for the Schmidt PUD development will be required. GEC Plans and permits will be provided with the relevant Final Drainage Reports.

f. Four Step Method

Step 1 – Employ Runoff Reduction Practices: Roof drains will discharge to landscaped areas where feasible. Implementation of landscaping and open space throughout the site will intercept runoff and increase infiltration.

Step 2 – Stabilize Drainageways: All new and re-development projects are required to construct or participate in the funding of channel stabilization measures. The Schmidt PUD will construct the requisite improvements to the onsite portion of Cottonwood Creek Reach DP67 to stabilize the drainageway. Final design details of the channel improvements will be provided in a Channel Design Report during the initial Schmidt PUD Final Drainage Report.

Step 3 – Provide WQCV: Water quality treatment is provided in Pond A and Pond B for the Schmidt PUD, Marksheffel Road Segments M3 & M4, Brush Top Road and the Stonebridge parcel. The water quality volume will be captured and released over a period of 40 hours.

Step 4 – Consider the need for source controls: No industrial or commercial uses are proposed within this development. Therefore, no source controls are proposed.

g. Drainage Basin & Bridge Fees

The site is within the Sand Creek and Cottonwood Creek Drainage Basins. Drainage basin and bridge will be paid at time of platting for the corresponding phases of the Schmidt PUD. Therefore, drainage basin and bridge fees will be deferred to the relevant Final Drainage Report. 2026 fees associated with Sand Creek and Cottonwood Creek Drainage Basins are presented below. Drainage basin fees and acreages will be finalized pending the CDR2413 basin swap variance.

Sand Creek 2026
Basin Fee: $28,160/impervious acre
Bridge Fee: $11,518/impervious acre

Cottonwood Creek 2026
Basin Fee: $27,282/impervious acre
Bridge Fee: $1,492/impervious acre

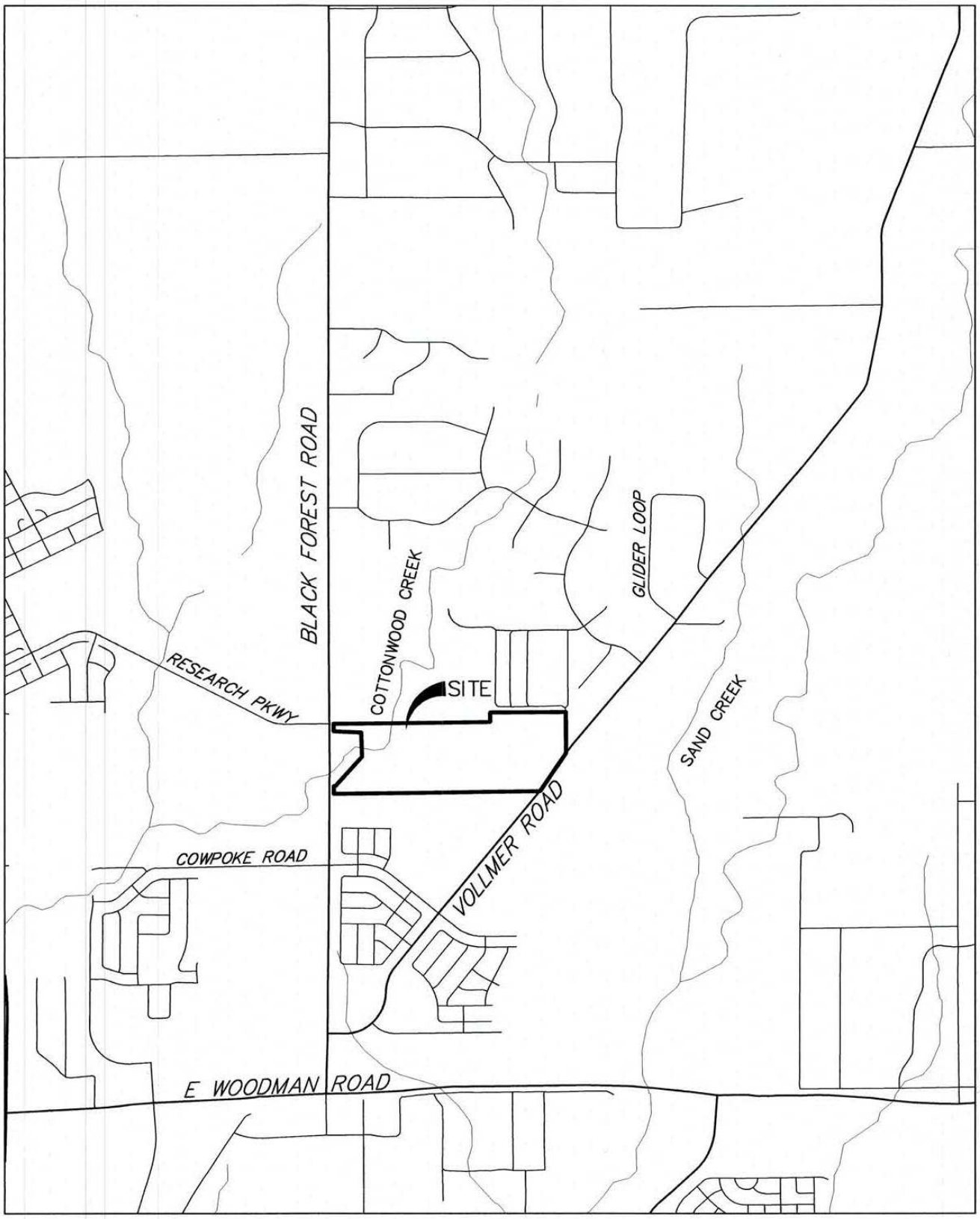
IV. Summary

Schmidt PUD remains consistent with pre-development drainage conditions. The proposed development will not adversely affect downstream stormwater infrastructure or surrounding developments. This report is in accordance with the latest El Paso County Drainage criteria.

V. References

1. El Paso County – Drainage Criteria Manual, 2018 as amended.
2. Urban Storm Drainage Criteria Manual, Mile High Flood District, March 2024.
3. Federal Emergency Management Agency, Flood Map Service Center - https://msc.fema.gov/portal/home, September 2024.
4. Web Soil Survey, Natural Resources Conservation Service - https://websoilsurvey.nrcs.usda.gov/app/, September 2024.
5. Cottonwood Creek Drainage Basin Planning Study, URS Consultants, June 9, 1994.
6. Sand Creek Drainage Basin Planning Study Preliminary Design Report, Kiowa Engineering Corporation, January 1993. Revised October 1995.
7. Silver Ponds Subdivision Filing No. 1 Final Drainage Report, M.V.E., Inc., February 1995. Revised May 1996.
8. Trails at Forest Meadows Filing 3 Final Drainage Report, M&S Civil Consultants, Inc., August 2015.
9. Trails at Forest Meadows Filing 4 Final Drainage Report – Amendment To: MDDP Plan Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No. 21 and No. 4, M&S Civil Consultants, Inc., April 2016.
10. Marksheffel Road – Segment M3 CDR Final Drainage Report, All Terrain Engineering LLC. Currently under El Paso County review.

APPENDIX A – VICINITY MAP, FEMA MAP, NRCS WEB SOIL SURVEY



VICINITY MAP

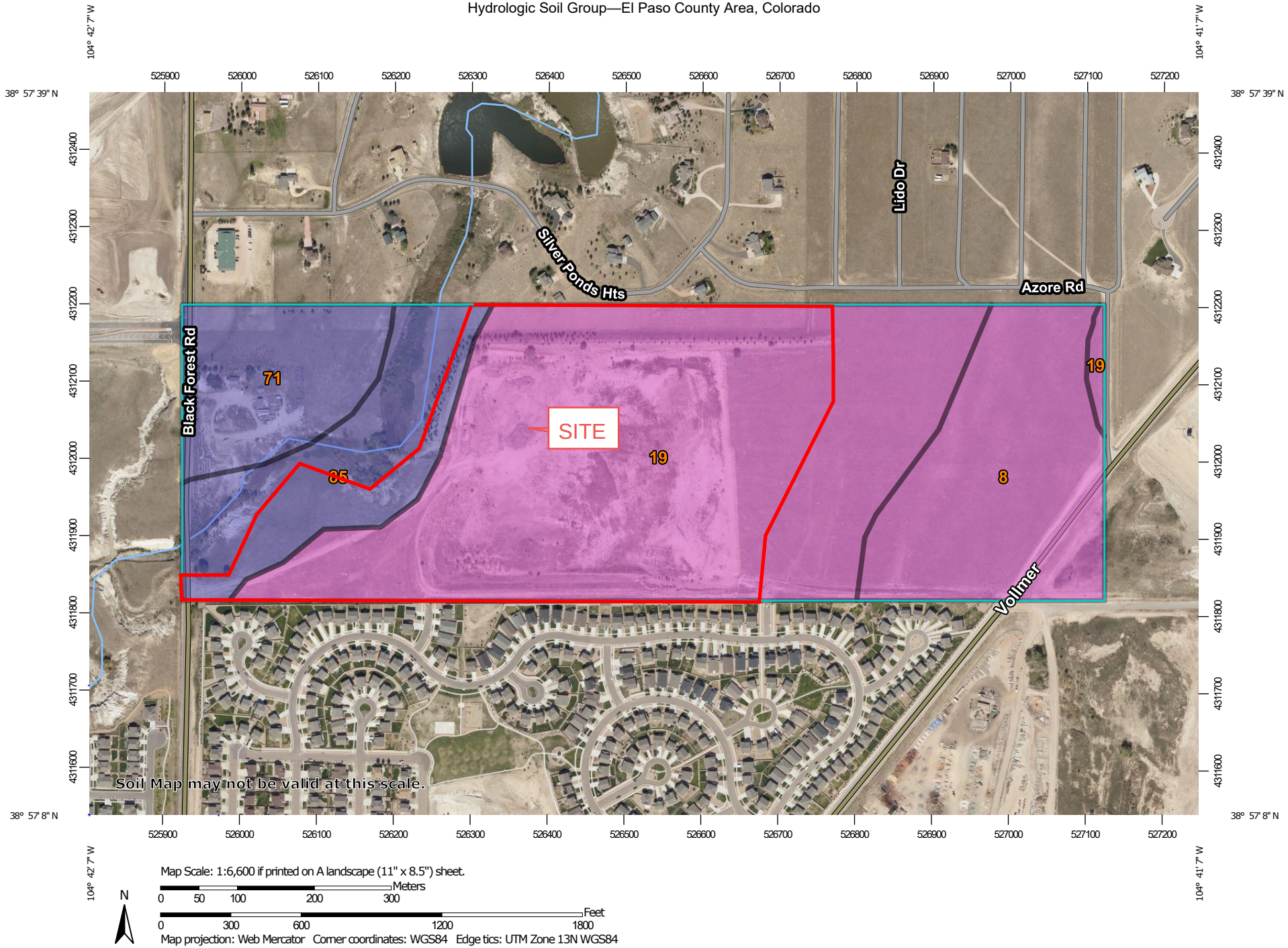
SCALE: 1"=2000'

2000 1000 0 2000

ORIGINAL SCALE: 1" = 2000'

J·R ENGINEERING
A Westrian Company
Centennial 303-740-9393 • Colorado Springs 719-593-2593
Fort Collins 970-491-9888 • www.jrengineering.com

X:\2510000.all\2518813\Drawings\Blocks\2518813VicMap_District_Infrastructure.dwg, 8.5x11 Portrait, 8/22/2023 7:05:44 AM, CS



Map Scale: 1:6,600 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 50 100 200 300

Feet
0 300 600 1200 1800

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	22.3	19.5%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	64.2	56.2%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	12.1	10.6%
85	Stapleton-Bernal sandy loams, 3 to 20 percent slopes	B	15.6	13.6%
Totals for Area of Interest			**114.1**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

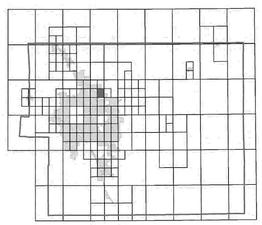
If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

Flooding Source	Vertical Datum Offset (ft)

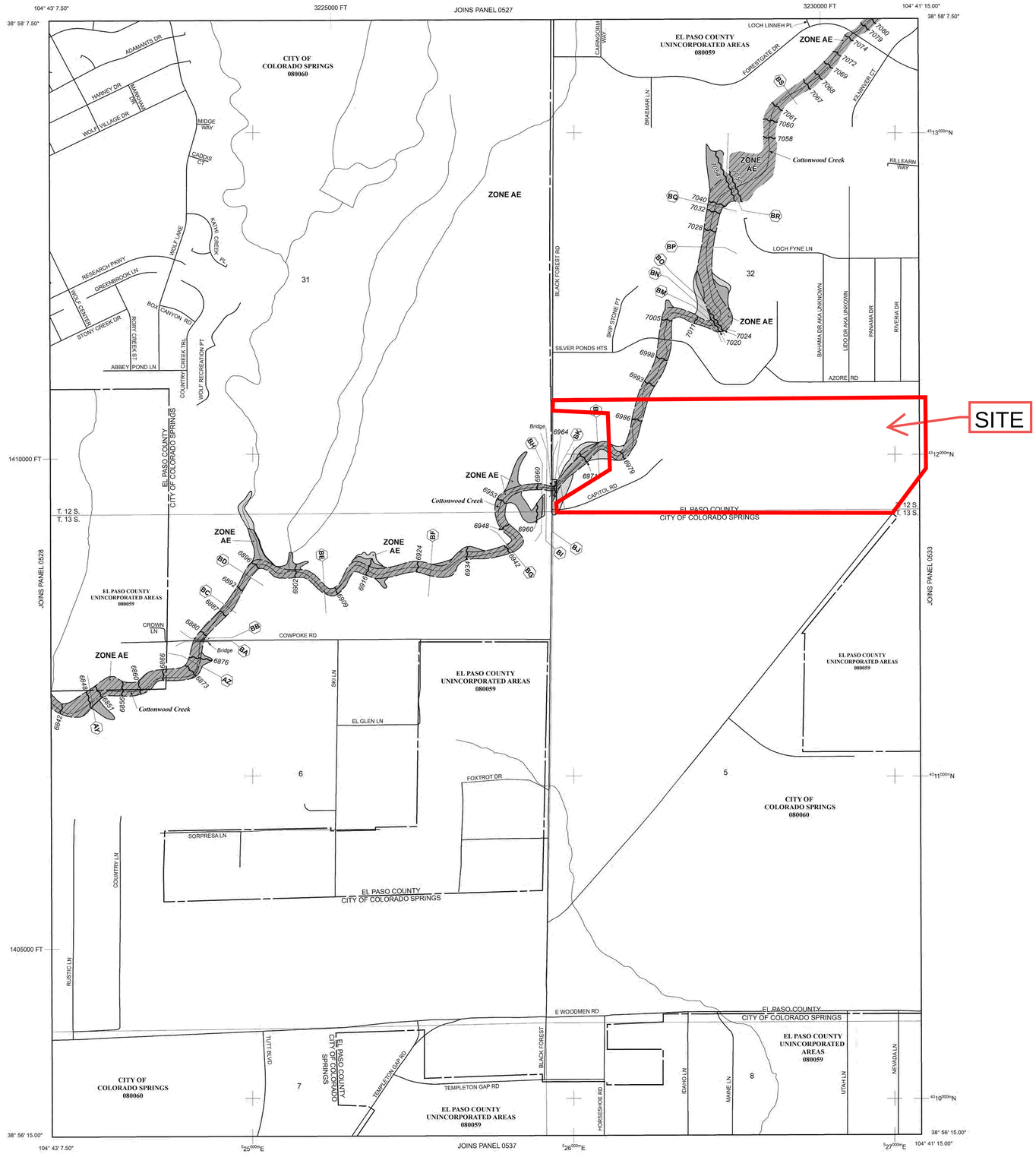
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 13 SOUTH, RANGE 65 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Elevations determined.
ZONE AE Base Flood Elevations determined.
ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99 Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D Boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
- (EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

- A — A — Cross section line
- 23 — 23 — Transect line
- 97° 07' 30.00" / 32° 22' 30.00" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 4275000mN — 1000-meter Universal Transverse Mercator grid ticks, zone 13
- 6000000 FT — 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
- DX5510 × — Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 — River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

PANEL 0529G

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 529 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0529	G
EL PASO COUNTY	080059	0529	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER 08041C0529G

MAP REVISED DECEMBER 7, 2018

Federal Emergency Management Agency

NOAA Atlas 14, Volume 8, Version 2
Location name: Colorado Springs, Colorado, USA*
Latitude: 38.9556°, Longitude: -104.6899°
Elevation: 7015 ft**
* source: ESRI Maps
** source: USGS

RAINFALL DEPTHS USED FOR UD-DETENTION

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.237** (0.195-0.291)	**0.290** (0.238-0.356)	**0.381** (0.311-0.469)	**0.461** (0.374-0.570)	**0.579** (0.456-0.748)	**0.675** (0.518-0.882)	**0.776** (0.574-1.04)	**0.884** (0.625-1.22)	**1.04** (0.702-1.46)	**1.16** (0.759-1.65)
10-min	**0.348** (0.285-0.426)	**0.424** (0.348-0.521)	**0.557** (0.456-0.686)	**0.675** (0.548-0.835)	**0.847** (0.668-1.10)	**0.988** (0.758-1.29)	**1.14** (0.840-1.52)	**1.30** (0.915-1.78)	**1.52** (1.03-2.14)	**1.69** (1.11-2.42)
15-min	**0.424** (0.348-0.520)	**0.517** (0.424-0.635)	**0.680** (0.556-0.837)	**0.823** (0.669-1.02)	**1.03** (0.814-1.34)	**1.20** (0.924-1.58)	**1.39** (1.02-1.86)	**1.58** (1.12-2.17)	**1.85** (1.25-2.61)	**2.06** (1.36-2.94)
30-min	**0.613** (0.504-0.752)	**0.748** (0.613-0.918)	**0.981** (0.802-1.21)	**1.19** (0.964-1.47)	**1.49** (1.17-1.92)	**1.74** (1.33-2.27)	**2.00** (1.48-2.67)	**2.27** (1.61-3.13)	**2.66** (1.80-3.76)	**2.97** (1.95-4.24)
60-min	**0.794** (0.652-0.974)	**0.948** (0.778-1.16)	**1.23** (1.00-1.51)	**1.48** (1.21-1.84)	**1.88** (1.49-2.44)	**2.21** (1.70-2.90)	**2.57** (1.90-3.46)	**2.96** (2.10-4.09)	**3.52** (2.39-4.99)	**3.97** (2.61-5.67)
2-hr	**0.974** (0.806-1.19)	**1.15** (0.949-1.40)	**1.47** (1.21-1.80)	**1.78** (1.46-2.19)	**2.26** (1.82-2.94)	**2.68** (2.08-3.51)	**3.14** (2.35-4.21)	**3.64** (2.60-5.01)	**4.37** (3.00-6.17)	**4.97** (3.29-7.05)
3-hr	**1.08** (0.893-1.30)	**1.25** (1.04-1.51)	**1.58** (1.31-1.93)	**1.92** (1.57-2.34)	**2.45** (1.98-3.19)	**2.92** (2.29-3.83)	**3.45** (2.60-4.62)	**4.03** (2.90-5.55)	**4.90** (3.38-6.90)	**5.61** (3.73-7.93)
6-hr	**1.26** (1.05-1.51)	**1.44** (1.20-1.73)	**1.81** (1.51-2.19)	**2.19** (1.81-2.65)	**2.81** (2.29-3.64)	**3.37** (2.66-4.39)	**4.00** (3.04-5.34)	**4.71** (3.42-6.45)	**5.76** (4.01-8.08)	**6.64** (4.45-9.32)
12-hr	**1.46** (1.23-1.74)	**1.68** (1.41-2.01)	**2.12** (1.78-2.54)	**2.56** (2.13-3.08)	**3.26** (2.68-4.19)	**3.89** (3.09-5.03)	**4.59** (3.51-6.08)	**5.38** (3.94-7.31)	**6.54** (4.58-9.11)	**7.50** (5.07-10.5)
24-hr	**1.68** (1.43-1.99)	**1.97** (1.67-2.34)	**2.51** (2.12-2.99)	**3.02** (2.54-3.61)	**3.81** (3.13-4.82)	**4.49** (3.58-5.73)	**5.23** (4.02-6.84)	**6.05** (4.45-8.13)	**7.23** (5.10-9.97)	**8.20** (5.59-11.4)
2-day	**1.95** (1.67-2.30)	**2.32** (1.98-2.73)	**2.96** (2.52-3.50)	**3.54** (3.00-4.21)	**4.41** (3.63-5.50)	**5.14** (4.11-6.47)	**5.91** (4.56-7.63)	**6.74** (4.97-8.94)	**7.91** (5.60-10.8)	**8.85** (6.08-12.2)
3-day	**2.15** (1.85-2.52)	**2.55** (2.18-2.99)	**3.24** (2.76-3.81)	**3.85** (3.27-4.55)	**4.77** (3.94-5.90)	**5.52** (4.44-6.92)	**6.33** (4.90-8.13)	**7.19** (5.33-9.50)	**8.40** (5.98-11.4)	**9.37** (6.46-12.9)
4-day	**2.32** (2.00-2.72)	**2.73** (2.35-3.19)	**3.44** (2.95-4.03)	**4.08** (3.48-4.80)	**5.02** (4.16-6.20)	**5.81** (4.68-7.25)	**6.64** (5.16-8.50)	**7.53** (5.60-9.92)	**8.79** (6.27-11.9)	**9.80** (6.78-13.4)
7-day	**2.75** (2.38-3.19)	**3.18** (2.75-3.70)	**3.94** (3.40-4.59)	**4.61** (3.96-5.40)	**5.62** (4.69-6.89)	**6.46** (5.24-8.01)	**7.35** (5.75-9.35)	**8.30** (6.21-10.9)	**9.64** (6.93-13.0)	**10.7** (7.47-14.6)
10-day	**3.13** (2.72-3.61)	**3.59** (3.12-4.16)	**4.40** (3.81-5.11)	**5.13** (4.41-5.98)	**6.20** (5.18-7.55)	**7.08** (5.76-8.73)	**8.01** (6.29-10.1)	**9.01** (6.77-11.7)	**10.4** (7.50-13.9)	**11.5** (8.06-15.6)
20-day	**4.20** (3.68-4.82)	**4.82** (4.22-5.53)	**5.86** (5.11-6.74)	**6.75** (5.85-7.81)	**8.03** (6.74-9.64)	**9.04** (7.40-11.0)	**10.1** (7.97-12.6)	**11.2** (8.46-14.4)	**12.7** (9.20-16.8)	**13.9** (9.77-18.7)
30-day	**5.08** (4.47-5.80)	**5.83** (5.13-6.67)	**7.08** (6.20-8.11)	**8.12** (7.07-9.35)	**9.56** (8.03-11.4)	**10.7** (8.76-12.9)	**11.8** (9.35-14.7)	**13.0** (9.83-16.6)	**14.5** (10.6-19.1)	**15.7** (11.1-21.0)
45-day	**6.17** (5.45-7.01)	**7.09** (6.26-8.07)	**8.58** (7.55-9.80)	**9.80** (8.57-11.2)	**11.4** (9.63-13.5)	**12.7** (10.4-15.2)	**13.9** (11.0-17.1)	**15.1** (11.5-19.2)	**16.7** (12.2-21.8)	**17.8** (12.7-23.8)
60-day	**7.08** (6.28-8.03)	**8.15** (7.22-9.24)	**9.84** (8.68-11.2)	**11.2** (9.82-12.8)	**13.0** (10.9-15.2)	**14.3** (11.8-17.1)	**15.6** (12.4-19.1)	**16.8** (12.8-21.2)	**18.4** (13.4-23.9)	**19.5** (13.9-26.0)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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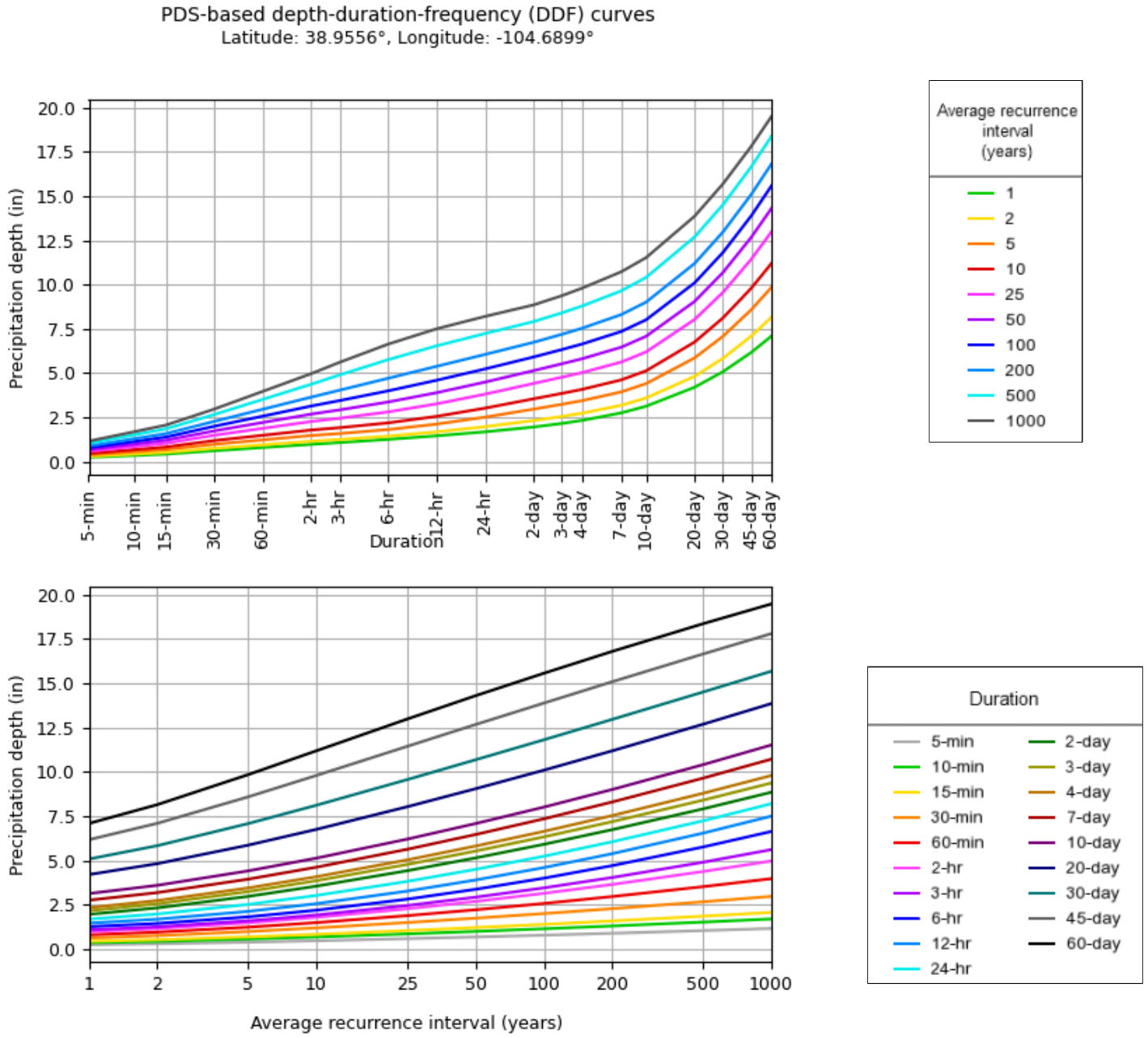
PF graphical



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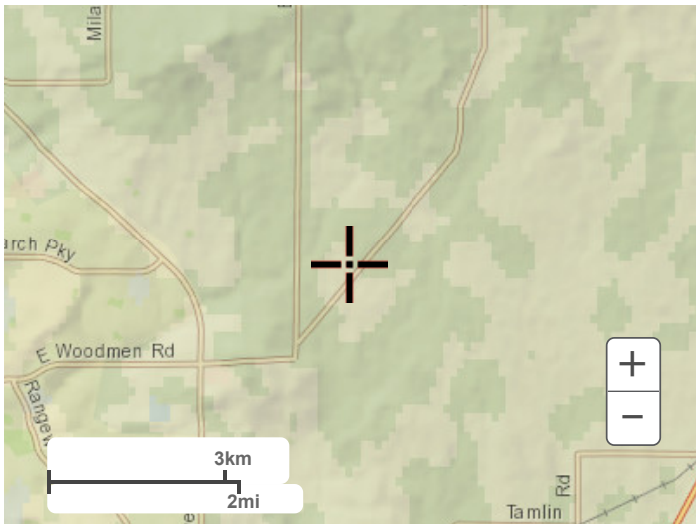
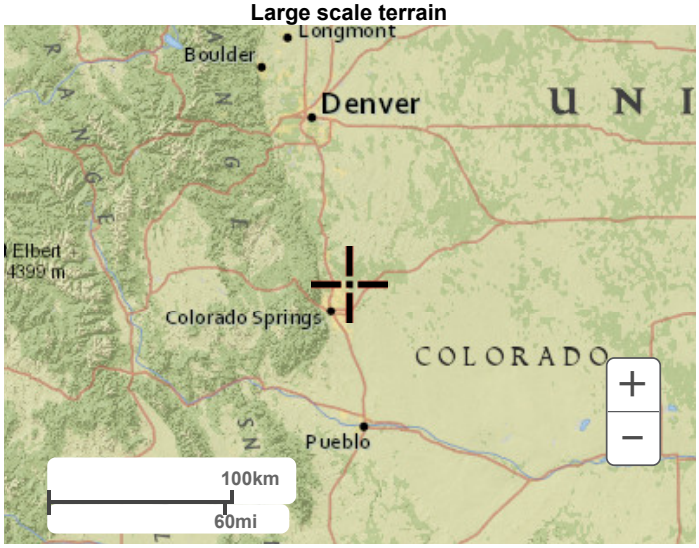
Maps & aerials

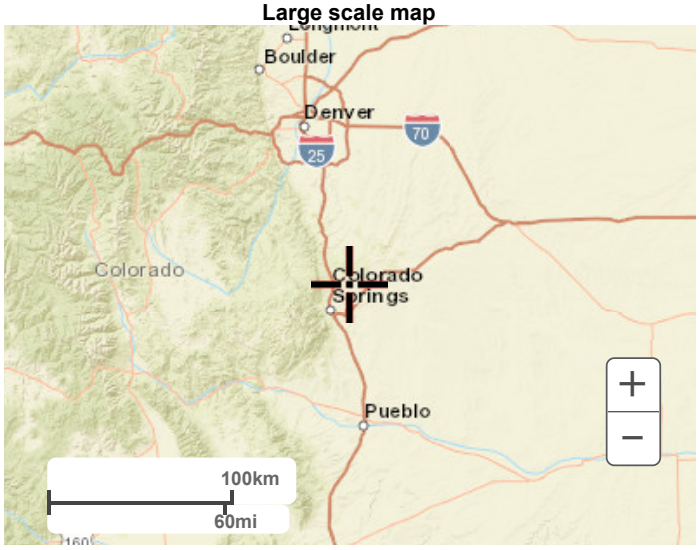
Small scale terrain



Large scale terrain



Large scale map



Large scale aerial

APPENDIX B – HYDROLOGIC CALCULATIONS

ALL TERRAIN ENGINEERING

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A	0.25	0%	0.08	0.35	53.0	0.0	0.2
B	0.09	0%	0.08	0.35	36.1	0.0	0.1
C	1.85	77%	0.61	0.73	17.2	3.7	7.5
D	1.18	86%	0.78	0.87	15.1	3.2	6.1
E	6.66	0%	0.08	0.35	36.8	1.2	8.5
F	2.56	25%	0.29	0.50	29.5	1.8	5.4
G	0.78	85%	0.78	0.87	16.9	2.0	3.8
H	3.59	72%	0.51	0.65	15.9	6.3	13.4
I	1.22	80%	0.73	0.84	17.5	2.9	5.6
J	1.28	87%	0.79	0.88	16.0	3.5	6.5
K	24.33	75%	0.53	0.66	30.3	31.8	66.5
L	36.07	0%	0.08	0.35	53.6	4.6	34.1
M	4.86	0%	0.08	0.35	44.3	0.7	5.4
N	0.34	0%	0.08	0.35	26.1	0.1	0.5
O	0.56	0%	0.08	0.35	26.7	0.1	0.9
P	2.68	0%	0.08	0.35	26.1	0.6	4.2
Q	10.29	0%	0.08	0.35	47.7	1.5	10.8

DESIGN POINT SUMMARY TABLE

DP#	$Q_{5\text{-YR}}$	$Q_{100\text{-YR}}$
1	0.0	0.2
2	0.0	0.1
3	3.7	7.5
4	3.2	6.1
4.1	6.7	13.7
5	5.6	17.2
5.1	44.8	99.4
6	1.8	5.4
7	2.0	3.8
7.1	3.4	8.3
9	2.9	18.5
10	3.5	6.5
10.1	12.3	24.6
11	31.8	66.5
11.1	40.9	84.8
11.2	44.2	92.8
13	4.6	34.1
13.1	37.8	107.6
14	38.4	112.2
14.1	0.1	40.9
15	0.1	0.5
16	0.1	0.9
17	0.6	4.2
18	1.5	10.8
19	6.6	14.4
20	5.5	12.1
21	35.7	83.1
22	39.2	89.8
I4	19.0	44.2

ALL TERRAIN ENGINEERING

COMPOSITE % IMPERVIOUS CALCULATIONS -PROPOSED CONDITIONS

Subdivision: Schmidt PUD
Location: El Paso County

Project Name: Schmidt PUD - Ex Conditions
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/12/26

Basin ID	Total Area (ac)	Paved Streets and Walks (100% Imp.)				Apartments/Mixed Use (75% Imp.)				Future Development (65%)				Lawns/Pasture (0% Imp.)				Basins Total Weighted C		Basins Total Weighted
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
A	0.25	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.25	0.0%	0.08	0.35	0.0%
B	0.09	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.09	0.0%	0.08	0.35	0.0%
C	1.85	0.90	0.96	0.58	31.2%	0.53	0.66	1.12	45.5%	0.45	0.59	0.00	0.0%	0.08	0.35	0.15	0.0%	0.61	0.73	76.7%
D	1.18	0.90	0.96	1.01	85.8%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.17	0.0%	0.78	0.87	85.8%
E	6.66	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	6.66	0.0%	0.08	0.35	0.0%
F	2.56	0.90	0.96	0.65	25.3%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	1.91	0.0%	0.29	0.50	25.3%
G	0.78	0.90	0.96	0.67	85.1%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.12	0.0%	0.78	0.87	85.1%
H	3.59	0.90	0.96	0.00	0.0%	0.53	0.66	3.46	72.2%	0.45	0.59	0.00	0.0%	0.08	0.35	0.13	0.0%	0.51	0.65	72.2%
I	1.22	0.90	0.96	0.97	79.6%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.25	0.0%	0.73	0.84	79.6%
J	1.28	0.90	0.96	1.12	87.2%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.16	0.0%	0.79	0.88	87.2%
K	24.33	0.90	0.96	0.00	0.0%	0.53	0.66	24.33	75.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.00	0.0%	0.53	0.66	75.0%
L	36.07	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	36.07	0.0%	0.08	0.35	0.0%
M	4.86	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	4.86	0.0%	0.08	0.35	0.0%
N	0.34	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.34	0.0%	0.08	0.35	0.0%
O	0.56	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.56	0.0%	0.08	0.35	0.0%
P	2.68	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	2.68	0.0%	0.08	0.35	0.0%
Q	10.29	0.90	0.96	0.00	0.0%	0.53	0.66	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	10.29	0.0%	0.08	0.35	0.0%
TOTAL	**98.58**																			**27.1%**

ALL TERRAIN ENGINEERING

STANDARD FORM SF-2 - PROPOSED CONDITIONS
TIME OF CONCENTRATION

Subdivision: Schmidt PUD
Location: El Paso County

Project Name: Schmidt PUD - Ex Conditions
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/12/26

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A	0.25	A	0.0%	0.08	0.35	10.0	25.0%	2.0	1030	0.5%	15.0	1.1	16.2	18.2	1040.0	53.0	53.0
B	0.09	A	0.0%	0.08	0.35	10.0	25.0%	2.0	385	0.5%	15.0	1.1	6.0	8.1	395.0	36.1	36.1
C	1.85	A	76.7%	0.61	0.73	100.0	1.8%	7.3	525	1.1%	20.0	2.1	4.2	11.5	625.0	17.2	17.2
D	1.18	A	85.8%	0.78	0.87	85.0	2.0%	4.2	490	1.1%	20.0	2.1	3.9	8.1	575.0	15.1	15.1
E	6.66	A	0.0%	0.08	0.35	100.0	6.0%	10.2	900	2.4%	15.0	2.3	6.5	16.7	1000.0	36.8	36.8
F	2.56	A	25.3%	0.29	0.50	45.0	2.0%	7.8	1050	3.2%	20.0	3.6	4.9	12.7	1095.0	29.5	29.5
G	0.78	A	85.1%	0.78	0.87	50.0	2.0%	3.3	1050	2.4%	20.0	3.1	5.6	8.9	1100.0	16.9	16.9
H	3.59	A	72.2%	0.51	0.65	100.0	4.5%	6.5	435	3.0%	15.0	2.6	2.8	9.2	535.0	15.9	15.9
I	1.22	A	79.6%	0.73	0.84	85.0	2.0%	4.9	820	1.8%	20.0	2.7	5.1	10.0	905.0	17.5	17.5
J	1.28	A	87.2%	0.79	0.88	85.0	2.0%	4.0	820	1.8%	20.0	2.7	5.1	9.1	905.0	16.0	16.0
K	24.33	A	75.0%	0.53	0.66	100.0	2.9%	7.2	1530	2.5%	7.0	1.1	23.0	30.3	1630.0	21.5	30.3
L	36.07	A	0.0%	0.08	0.35	100.0	2.0%	14.7	2000	1.8%	7.0	0.9	35.5	50.1	2100.0	53.6	53.6
M	4.86	A	0.0%	0.08	0.35	100.0	2.0%	14.7	700	0.5%	15.0	1.1	11.0	25.7	800.0	44.3	44.3
N	0.34	A	0.0%	0.08	0.35	10	25.0%	2.0	5	2.0%	10.0	1.4	0.1	2.1	15.0	26.1	26.1
O	0.56	A	0.0%	0.08	0.35	25	2.0%	7.3	60	2.5%	10.0	1.6	0.6	8.0	85.0	26.7	26.7
P	2.68	A	0.0%	0.08	0.35	10	25.0%	2.0	5	0.5%	10.0	0.7	0.1	2.1	15.0	26.1	26.1
Q	10.29	A	0.0%	0.08	0.35	100	30.0%	6.0	1170	1.0%	10.0	1.0	19.5	25.5	1270.0	47.7	47.7

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_o = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_o}$
K = NRCS conveyance factor (see Table 6-2).

Equation 6-2

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

Equation 6-4

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Equation 6-5

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

ALL TERRAIN
ENGINEERING

STANDARD FORM SF-3 - PROPOSED CONDITIONS

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 5-Year

Project Name: Schmidt PUD - Ex Conditions
Project No.: 24013.00
Calculated By: NOJ
Checked By: REB
Date: 6/12/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Σc (min)	Σ C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	I4	OSI4	27.16	0.30	29.2	8.15	2.33	19.0															Off-site Basin OSI4 Values from Sliver Pond FDR (Bains I4 & DP21)
	1	A	0.25	0.08	53.0	0.02	1.63	0.03															Existing berm sends off-site flow from north to the west to the existing swale at DP1.
	2	B	0.09	0.08	36.1	0.01	2.20	0.02															Existing berm sends off-site flow from north to the east to the exisitng swale and culvert at DP2.
	3	C	1.85	0.61	17.2	1.12	3.32	3.7								3.7	1.12	1.0	18	85	5.4	0.3	RUNOFF SHEET FLOWS TO C&G, CARRIED TO 5' TYPE R SUMP INLET @ DP3, CAPTURED FLOWS CONTINUE IN PIPE TO DP4.1
	3																						TOTAL FLOW @ DP3 (5' TYPE R SUMP INLET), CAPTURED FLOWS ARE PIPED TO DP4.1
	4	D	1.18	0.78	15.1	0.92	3.51	3.2															RUNOFF SHEET FLOWS TO C&G, CARRIED TO DP4 (5' TYPE R SUMP INLET). CAPTURED FLOWS PIPED TO DP4.1 WHERE THEY COMBINE WITH DP3 FLOWS
	4.1								17.5	2.05	3.29	6.7	6.7	2.047	2.8	6.7	2.05	2.0	24	1000	2.5	6.6	COMBINED FLOW IN PIPE FROM DP3 & 4, FLOWS DISCHARGE TO SWALE E1 AND ARE CARRIED TO DP5 WHERE THEY COMBINE WITH BASIN E FLOWS
	5	E	6.66	0.08	36.8	0.53	2.18	1.2															RUNOFF SHEET FLOWS TOWARDS SWALE E1, FLOWS CONTINUE IN SWALE TO DP5, WHERE THEYC COMBINE WITH FLOWS FROM DP4.1
	5								36.8	2.58	2.18	5.6											COMBINED FLOW IN SWALE E1 @ DP5, FLOWS CONTINUE TO DP5.1 IN SWALE L1
	6	F	2.56	0.29	29.5	0.74	2.51	1.8								1.8	0.74	1.0	18	40	4.4	0.2	RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO 15' TYPE R ON-GRADE INLET @ DP 6, CAPTURED FLOWS ARE PIPED TO DP7.1
	7	G	0.78	0.78	16.9	0.61	3.34	2.0															RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO 15' TYPE R ON-GRADE INLET @ DP 7, CAPTURED FLOWS ARE PIPED TO DP7.1
	7.1								29.6	1.35	2.50	3.4				3.4	1.35	1.0	24	45	5.1	0.1	COMBINED FLOW FROM DP6 & 7 IN 24" RCP, FLOWS ARE PIPED TO DP11.2
	9	H	3.59	0.51	15.9	1.84	3.43	6.3															BASIN H RUNOFF SHEET FLOWS TO C&G IN BASIN I, FLOWS CONTINUE TO DP9 (10' TYPE R SUMP) WHERE THEY COMBINE WITH BASIN I FLOWS
	9	I	1.22	0.73	17.5	0.89	3.29	2.9	17.5	2.73	3.29	9.0				9.0	2.73	1.0	24	90	6.8	0.2	BASIN I RUNOFF CARREID BY C&G TO 10' TYPE R SUMP INLET @ DP9, WHERE FLOWS COMBINE WITH BASIN H FLOWS, CAPTURED FLOWS PIPED TO DP10.1
	10	J	1.28	0.79	16.0	1.02	3.43	3.5															BASIN J RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO DP10 (5' TYPE R SUMP INLET), CAPTURED FLOWS ARE PIPED TO DP10.1
	10.1								17.8	3.75	3.27	12.3				12.3	3.75	1.0	36	1880	7.2	4.3	COMBINED FLOW IN PIPE @ DP10.1, FLOWS CONTINUE IN PIPE TO DP11.1
	11	K	24.33	0.53	30.3	12.89	2.47	31.8								31.8	12.89	2.3	36	136	12.6	0.2	RUNOFF SHEET FLOWS TOWARDS SWALE K1, FLOWS CONTINUE IN SWALE TO DP11, A TYPE C SUMP INLET/ CAPTURED FLOWS ARE PIPED TO DP11.1
	11.1								30.5	16.65	2.46	40.9				40.9	16.65	2.8	42	46	14.4	0.1	COMBINED FLOW IN PIPE @ DP 11.1 FROM DP11 RUNOFF & DP10.1 PIPED FLOW, FLOW CONTINUES IN 42" RCP TO DP11.2
	11.2								30.5	17.99	2.46	44.2	44.2	17.99	0.65					120	1.2	1.7	COMBINED FLOW IN PIPE @ DP 11.2, OUTFALLS TO SWALE L1, CONTINUE TO DP5.1, WHERE FLOWS COMBINE WITH DP5 FLOWS IN SWALE L1
	5.1								36.8	20.57	2.18	44.8	44.8	20.57	1					1600	1.5	17.8	COMBINED FLOW IN SWALE L1 @ DP5.1. FLOWS CONTINUE IN SWALE TO DP13.1, WHERE THEY COMBINE WITH BASIN L RUNOFF
	13	L	36.07	0.08	53.6	2.89	1.61	4.6															RUNOFF FROM BASIN L, SHEET FLOWS TO SWALE @ DP13, FLOWS CONTINUE IN SWALE TO DP13.1 WHERE THEY COMBINE WITH DP5.1 FLOWS
	13.1								53.6	23.46	1.61	37.8											TOTAL FLOWS IN SWALE L1 @ DP13.1, FLOWS CONTINUE TO POND A @ DP14
	14	M	4.86	0.08	44.3	0.39	1.90	0.7	53.6	23.85	1.61	38.4											RUNOFF SHEET FLOWS INTO POND @ DP14 AND COMBINES WITH FLOWS FROM DP13.1
	15	N	0.34	0.08	26.1	0.03	2.69	0.1															RUNOFF SHEET FLOWS OFF-SITE @ DP 15
	16	O	0.56	0.08	26.7	0.04	2.66	0.1															RUNOFF SHEET FLOWS OFF-SITE @ DP 16
	17	P	2.68	0.08	26.1	0.21	2.69	0.6															RUNOFF SHEET FLOWS OFF-SITE @ DP 17
	18	Q	10.29	0.08	47.7	0.82	1.79	1.5															RUNOFF SHEET FLOWS OFF-SITE @ DP 18
	19	EXF1	4.25	-	-	-	-	6.6															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP19 @ COTTONWOOD CREEK
	20	EXE1	4.24	-	-	-	-	5.5															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP20 @ COTTONWOOD CREEK
	21	EXB3	50.03	-	-	-	-	35.7															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP21
	22	EXB4	52.02	-	-	-	-	39.2															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP22

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

IDF Equations

$I_{100} = -2.52 \ln(D) + 12.735$

$I_{50} = -2.25 \ln(D) + 11.375$

$I_{25} = -2.00 \ln(D) + 10.111$

$I_{10} = -1.75 \ln(D) + 8.847$

$I_5 = -1.50 \ln(D) + 7.583$

$I_2 = -1.19 \ln(D) + 6.035$

Note: Values calculated by equations may not precisely duplicate values read from figure.

ALL TERRAIN ENGINEERING

STANDARD FORM SF-3 - PROPOSED CONDITIONS

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 100-Year

Project Name: Schmidt PUD - Ex Conditions
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/12/26

		DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{str\,net}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	I4	OSI4	27.16	0.40	29.2	10.86	4.07	44.2															
	1	A	0.25	0.35	53.0	0.09	2.73	0.2															Existing berm sends off-site flow from north to the west to the existing swale at DP1.
	2	B	0.09	0.35	36.1	0.03	3.70	0.1															Existing berm sends off-site flow from north to the east to the exisitng swale and culvert at DP2.
	3	C	1.85	0.73	17.2	1.35	5.57	7.5								7.5	1.35	1.0	18	85	6.4	0.2	RUNOFF SHEET FLOWS TO C&G, CARRIED TO 5' TYPE R SUMP INLET @ DP3, CAPTURED FLOWS CONTINUE IN PIPE TO DP4.1
	4	D	1.18	0.87	15.1	1.03	5.89	6.1															RUNOFF SHEET FLOWS TO C&G, CARRIED TO DP4 (5' TYPE R SUMP INLET). CAPTURED FLOWS PIPED TO DP4.1 WHERE THEY COMBINE WITH DP3 FLOWS
	4.1								17.4	2.37	5.53	13.1	13.1	2.37	2.8	13.1	2.37	2.0	24	1000	2.5	6.6	COMBINED FLOW IN PIPE FROM DP3 & 4, FLOWS DISCHARGE TO SWALE E1 AND ARE CARRIED TO DP5 WHERE THEY COMBINE WITH BASIN E FLOWS
	5	E	6.66	0.35	36.8	2.33	3.65	8.5															RUNOFF SHEET FLOWS TOWARDS SWALE E1, FLOWS CONTINUE IN SWALE TO DP5, WHERE THEYC COMBINE WITH FLOWS FROM DP4.1
	5								36.8	4.70	3.65	17.2											COMBINED FLOW IN SWALE E1 @ DP5, FLOWS CONTINUE TO DP5.1 IN SWALE L1
	6	F	2.56	0.50	29.5	1.29	4.21	5.4								5.4	1.29	1.0	18	40	6.0	0.1	RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO 15' TYPE R ON-GRADE INLET @ DP 6, CAPTURED FLOWS ARE PIPED TO DP7.1
	7	G	0.78	0.87	16.9	0.68	5.60	3.8															RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO 15' TYPE R ON-GRADE INLET @ DP 7, CAPTURED FLOWS ARE PIPED TO DP7.1
	7.1								29.6	1.97	4.20	8.3				8.3	1.97	1.0	24	45	6.6	0.1	COMBINED FLOW FROM DP6 & 7 IN 24" RCP, FLOWS ARE PIPED TO DP11.2
	9	H	3.59	0.65	15.9	2.33	5.76	13.4															BASIN H RUNOFF SHEET FLOWS TO C&G IN BASIN I, FLOWS CONTINUE TO DP9 (10' TYPE R SUMP) WHERE THEY COMBINE WITH BASIN I FLOWS
	9	I	1.22	0.84	17.5	1.02	5.52	5.6	17.5	3.35	5.52	18.5				18.5	3.35	1.0	24	90	8.0	0.2	BASIN I RUNOFF CARREID BY C&G TO 10' TYPE R SUMP INLET @ DP9, WHERE FLOWS COMBINE WITH BASIN H FLOWS, CAPTURED FLOWS PIPED TO DP10.1
	10	J	1.28	0.88	16.0	1.13	5.75	6.5															BASIN J RUNOFF SHEET FLOWS TO C&G, IS CARRIED TO DP10 (5' TYPE R SUMP INLET), CAPTURED FLOWS ARE PIPED TO DP10.1
	10.1								17.7	4.47	5.49	24.6				24.6	4.47	1.0	36	1880	8.7	3.6	COMBINED FLOW IN PIPE @ DP10.1, FLOWS CONTINUE IN PIPE TO DP11.1
	11	K	24.33	0.66	30.3	16.06	4.14	66.5								66.5	16.06	2.3	36	136	15.2	0.1	RUNOFF SHEET FLOWS TOWARDS SWALE K1, FLOWS CONTINUE IN SWALE TO DP11, A TYPE C SUMP INLET/ CAPTURED FLOWS ARE PIPED TO DP11.1
	11.1								30.4	20.53	4.13	84.8				84.8	20.53	2.8	42	46	17.5	0.0	COMBINED FLOW IN PIPE @ DP 11.1 FROM DP11 RUNOFF & DP10.1 PIPED FLOW, FLOW CONTINUES IN 42" RCP TO DP11.2
	11.2								30.5	22.50	4.12	92.8	92.8	22.5	0.65					120	1.2	1.7	COMBINED FLOW IN PIPE @ DP 11.2, OUTFALLS TO SWALE L1, CONTINUE TO DP5.1, WHERE FLOWS COMBINE WITH DP5 FLOWS IN SWALE L1
	5.1								36.8	27.21	3.65	99.4	99.4	27.2	1					1600	1.5	17.8	COMBINED FLOW IN SWALE L1 @ DP5.1. FLOWS CONTINUE IN SWALE TO DP13.1, WHERE THEY COMBINE WITH BASIN L RUNOFF
	13	L	36.07	0.35	53.6	12.63	2.70	34.1															RUNOFF FROM BASIN L, SHEET FLOWS TO SWALE @ DP13, FLOWS CONTINUE IN SWALE TO DP13.1 WHERE THEY COMBINE WITH DP5.1 FLOWS
	13.1								53.6	39.83	2.70	107.6											TOTAL FLOWS IN SWALE L1 @ DP13.1, FLOWS CONTINUE TO POND A @ DP14
	14	M	4.86	0.35	44.3	1.70	3.18	5.4	53.6	41.53	2.70	112.2											RUNOFF SHEET FLOWS INTO POND @ DP14 AND COMBINES WITH FLOWS FROM DP13.1
	15	N	0.34	0.35	26.1	0.12	4.52	0.5															RUNOFF SHEET FLOWS OFF-SITE @ DP 15
	16	O	0.56	0.35	26.7	0.20	4.46	0.9															RUNOFF SHEET FLOWS OFF-SITE @ DP 16
	17	P	2.68	0.35	26.1	0.94	4.51	4.2															RUNOFF SHEET FLOWS OFF-SITE @ DP 17
	18	Q	10.29	0.35	47.7	3.60	3.00	10.8															RUNOFF SHEET FLOWS OFF-SITE @ DP 18
	19	EXF1	4.25	-	-	-	-	14.4															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP19 @ COTTONWOOD CREEK
	20	EXE1	4.24	-	-	-	-	12.1															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP20 @ COTTONWOOD CREEK
	21	EXB3	50.03	-	-	-	-	83.1															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP21
	22	EXB4	52.02	-	-	-	-	89.8															OFFSITE BASIN, FOLLOWS HISTORIC DRAINAGE PATTERNS TO DP22

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

IDF Equations

$I_{100} = -2.52 \ln(D) + 12.735$

$I_{50} = -2.25 \ln(D) + 11.375$

$I_{25} = -2.00 \ln(D) + 10.111$

$I_{10} = -1.75 \ln(D) + 8.847$

$I_5 = -1.50 \ln(D) + 7.583$

$I_2 = -1.19 \ln(D) + 6.035$

Note: Values calculated by equations may not precisely duplicate values read from figure.

Subdivision:	Schmidt PUD
Location:	El Paso County
Project Name:	Schmidt PUD
Project Number:	24013
Calculated By:	NQJ
Checked By:	REB
Date:	6/9/2026

PROPOSED CONDITIONS - BASIN SUMMARY TABLE

Sub-basin	Area (ac)	Impervious	C_5	C_{100}	t_c (min)	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
MS1	0.28	79%	0.73	0.83	5.4	1.0	2.0
MS2	1.76	64%	0.60	0.74	10.4	4.3	8.9
MS3	0.34	83%	0.76	0.85	5.2	1.3	2.5
MS4	1.41	81%	0.74	0.84	9.2	4.4	8.5
MS5	0.82	64%	0.60	0.74	6.0	2.4	5.0
MS6	2.57	76%	0.70	0.81	11.4	7.1	13.8
A	8.97	65%	0.41	0.59	24.3	10.3	24.8
B	9.44	65%	0.41	0.59	17.6	12.7	30.6
C	9.76	65%	0.41	0.59	24.4	11.2	27.0
D	6.00	65%	0.41	0.59	18.7	7.9	19.0
E	5.37	65%	0.41	0.59	14.7	7.8	18.9
F	3.63	56%	0.54	0.69	13.3	7.2	15.6
G	4.18	7%	0.13	0.39	12.0	2.1	10.5
H	0.29	53%	0.51	0.67	5.6	0.7	1.6
I	0.08	63%	0.60	0.74	5.0	0.2	0.5
J	0.47	12%	0.18	0.42	26.0	0.2	0.9
K	8.99	2%	0.09	0.36	86.4	0.7	4.8
L	0.43	2%	0.09	0.36	7.0	0.2	1.2

DESIGN POINT SUMMARY TABLE

Design Point	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
I4	19.0	44.2
EX3	1.0	2.0
EX4	1.3	2.5
EX4.1	6.7	13.1
EX11.2	44.2	92.8
1	4.3	8.9
2	8.5	17.0
3	9.2	18.9
4	10.3	24.8
5	26.3	57.4
6	54.0	116.5
7	17.2	41.5
8	94.2	208.9
9	7.2	15.6
10	2.1	10.5
11	0.7	1.6
12	0.2	0.5
13	0.2	0.9
14	3.5	58.1

COMPOSITE % IMPERVIOUS CALCULATIONS - PROPOSED CONDITIONS

Subdivision: Schmidt PUD
Location: El Paso County

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

		Lots: 1/8 Acre or Less				Paved				Gravel				Undeveloped				Weighted C_5 & C_{100}		Basins Total Weighted % Imp.
Basin ID	Total Area (ac)	C_5	C_{100}	Area (ac)	% Imp.	C_5	C_{100}	Area (ac)	% Imp.	C_5	C_{100}	Area (ac)	% Imp.	C_5	C_{100}	Area (ac)	% Imp.	C_5	C_{100}	
MS1	0.28	0.41	0.59	0.00	65.0%	0.90	0.96	0.22	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.06	2.0%	0.73	0.83	79.0%
MS2	1.76	0.41	0.59	0.00	65.0%	0.90	0.96	1.11	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.65	2.0%	0.60	0.74	63.8%
MS3	0.34	0.41	0.59	0.00	65.0%	0.90	0.96	0.28	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.06	2.0%	0.76	0.85	82.7%
MS4	1.41	0.41	0.59	0.00	65.0%	0.90	0.96	1.13	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.28	2.0%	0.74	0.84	80.5%
MS5	0.82	0.41	0.59	0.00	65.0%	0.90	0.96	0.52	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.30	2.0%	0.60	0.74	64.1%
MS6	2.57	0.41	0.59	0.00	65.0%	0.90	0.96	1.94	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.63	2.0%	0.70	0.81	76.0%
A	8.97	0.41	0.59	8.97	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.00	2.0%	0.41	0.59	65.0%
B	9.44	0.41	0.59	9.44	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.00	2.0%	0.41	0.59	65.0%
C	9.76	0.41	0.59	9.76	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.00	2.0%	0.41	0.59	65.0%
D	6.00	0.41	0.59	6.00	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.00	2.0%	0.41	0.59	65.0%
E	5.37	0.41	0.59	5.37	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.00	2.0%	0.41	0.59	65.0%
F	3.63	0.41	0.59	0.00	65.0%	0.90	0.96	2.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	1.63	2.0%	0.54	0.69	56.0%
G	4.18	0.41	0.59	0.00	65.0%	0.90	0.96	0.10	100.0%	0.59	0.7	0.15	80.0%	0.09	0.36	3.93	2.0%	0.13	0.39	7.1%
H	0.29	0.41	0.59	0.00	65.0%	0.90	0.96	0.15	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.14	2.0%	0.51	0.67	52.7%
I	0.08	0.41	0.59	0.00	65.0%	0.90	0.96	0.05	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.03	2.0%	0.60	0.74	63.3%
J	0.47	0.41	0.59	0.00	65.0%	0.90	0.96	0.05	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.42	2.0%	0.18	0.42	12.4%
K	8.99	0.41	0.59	0.00	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	8.99	2.0%	0.09	0.36	2.0%
L	0.43	0.41	0.59	0.00	65.0%	0.90	0.96	0.00	100.0%	0.59	0.7	0.00	80.0%	0.09	0.36	0.43	2.0%	0.09	0.36	2.0%
Pond A*	**51.14**																			**60.2%**
Pond B**	**3.82**																			**65.1%**
Total	**64.79**																			**52.0%**

* Pond A Basins: MS1-MS4, A-G

** Pond B Basins: MS5, MS6 & L

STANDARD FORM SF-2 - PROPOSED CONDITIONS
TIME OF CONCENTRATION

Subdivision: Schmidt PUD
Location: El Paso County

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					tc CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Weighted C_5	Weighted C_{100}	Impervious (%)	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
MS1	0.28	B	0.73	0.83	79.0%	36	2.0%	3.2	230	0.8%	20.0	1.8	2.1	5.4	266.0	14.7	5.4
MS2	1.76	B	0.60	0.74	63.8%	36	2.0%	4.3	1140	2.4%	20.0	3.1	6.1	10.4	1176.0	22.0	10.4
MS3	0.34	B	0.76	0.85	82.7%	36	2.0%	3.0	240	0.8%	20.0	1.8	2.2	5.2	276.0	14.1	5.2
MS4	1.41	B	0.74	0.84	80.5%	36	2.0%	3.1	1140	2.4%	20.0	3.1	6.1	9.2	1176.0	18.4	9.2
MS5	0.82	A/B	0.60	0.74	64.1%	36	2.0%	4.3	260	1.5%	20.0	2.4	1.8	6.0	296.0	17.1	6.0
MS6	2.57	A/B	0.70	0.81	76.0%	36	2.0%	3.4	950	1.0%	20.0	2.0	7.9	11.4	986.0	21.1	11.4
A	8.97	A/B	0.41	0.59	65.0%	135	2.0%	11.5	2000	1.7%	20.0	2.6	12.8	24.3	2135.0	29.1	24.3
B	9.44	B	0.41	0.59	65.0%	90	2.0%	9.4	1250	1.6%	20.0	2.5	8.2	17.6	1340.0	24.0	17.6
C	9.76	B	0.41	0.59	65.0%	100	2.0%	9.9	1900	1.2%	20.0	2.2	14.5	24.4	2000.0	30.9	24.4
D	6.00	B	0.41	0.59	65.0%	100	2.0%	9.9	1200	1.3%	20.0	2.3	8.8	18.7	1300.0	24.6	18.7
E	5.37	A/B	0.41	0.59	65.0%	100	2.0%	9.9	750	1.7%	20.0	2.6	4.8	14.7	850.0	20.2	14.7
F	3.63	A/B	0.54	0.69	56.0%	18	2.0%	3.4	1400	1.4%	20.0	2.4	9.9	13.3	1418.0	28.2	13.3
G	4.18	A/B	0.13	0.39	7.1%	50	25.0%	4.3	650	0.5%	20.0	1.4	7.7	12.0	700.0	40.1	12.0
H	0.29	A	0.51	0.67	52.7%	32	2.0%	4.8	130	2.0%	20.0	2.8	0.8	5.6	162.0	18.0	5.6
I	0.08	B	0.60	0.74	63.3%	15	2.0%	2.8	50	2.0%	20.0	2.8	0.3	3.1	65.0	15.6	5.0
J	0.47	B	0.18	0.42	12.4%	10	10.0%	2.5	1000	0.5%	10.0	0.7	23.6	26.0	1010.0	45.8	26.0
K	8.99	A/B	0.09	0.36	2.0%	100	10.0%	8.5	3305	0.5%	10.0	0.7	77.9	86.4	3405.0	109.6	86.4
L	0.43	A	0.09	0.36	2.0%	30	25.0%	3.5	150	0.5%	10.0	0.7	3.5	7.0	180.0	29.5	7.0

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

Eq $$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Equation 6-4 $$t_c = \frac{(26 - 17i) + L_t}{60(14i + 9)\sqrt{S_t}}$$

Equation 6-5

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_o = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_o}$
K = NRCS conveyance factor (see Table 6-2).

:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

STANDARD FORM SF-3 - PROPOSED CONDITIONS

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 5-Year

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

		DIRECT RUNOFF							TOTAL RUNOFF				OVERLAND			PIPE				TRAVEL TIME			
DESCRIPTION	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C* A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C* A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C* A (ac)	Slope (%)	Q_{pipe} (cfs)	C* A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	I4							19.0															Existing Basin OSI4 flow @ DPI4 per Marksheffel Road Segment M3 FDR, follows historic drainage patterns to Cottonwood Creek
	EX4.1				17.5	2.05	3.29	6.7															Existing flow @ DP4.1 per Marksheffel Road Segment M3 FDR Ultimate Conditions, pipe to DP5
	EX11.2				30.5	17.99	2.46	44.2															Existing flow @ DP11.2 per Marksheffel Road Segment M3 FDR Ultimate Conditions, pipe to DP6
	EX3	MS1	0.28	0.73	5.4	0.20	5.06	1.0															Basin MS1 follows historic drainage patterns per Marksheffel Road Segment M3 FDR, captured in 5' CS Type 2 inlet @ DP EX3, pipe to DP4.1
	1	MS2	1.76	0.60	10.4	1.06	4.07	4.3															Basin MS2 captured @ DP1, pipe to DP2
	EX4	MS3	0.34	0.76	5.2	0.26	5.11	1.3															Basin MS3 follows historic drainage patterns per Marksheffel Road Segment M3 FDR, captured in 5' CS Type 2 inlet @ DP EX4, pipe to DP4.1
	2	MS4	1.41	0.74	9.2	1.04	4.25	4.4	10.4	2.10	4.07	8.5											Basin MS4 captured @ DP2, pipe to DP5
		MS5	0.82	0.60	6.0	0.50	4.88	2.4															Basin MS5 captured @ DP3/Pond B
		MS6	2.57	0.70	11.4	1.80	3.94	7.1															Basin MS6 captured @ DP3, pipe to Pond B
	4	A	8.97	0.41	24.3	3.68	2.80	10.3															Basin A captured in series of Type R inlets @ DP4, pipe to DP7
	5	B	9.44	0.41	17.6	3.87	3.28	12.7	17.6	8.02	3.28	26.3											Basin B captured in series of Type R inlets @ DP5, combine with DP EX4.1 & DP2, pipe to DP8
	6	C	9.76	0.41	24.4	4.00	2.79	11.2	30.5	21.99	2.46	54.0											Basin C captured in series of Type R inlets @ DP6, combine with DP EX11.2, pipe to DP8
	7	D	6.00	0.41	18.7	2.46	3.19	7.9	24.3	6.14	2.80	17.2											Basin D captured in series of Type R inlets @ DP7, combine with DP4, pipe to DP8
	8	E	5.37	0.41	14.7	2.20	3.55	7.8	30.5	38.35	2.46	94.2											Basin E captured in series of Type R inlets @ DP8, combine with DP5-DP7, pipe to Pond A
	9	F	3.63	0.54	13.3	1.95	3.70	7.2															Basin F captured in series of Type R inlets @ DP9, pipe to Pond A
	10	G	4.18	0.13	12.0	0.53	3.86	2.1															Basin G captured at DP10 (Pond A), combine with DP8 & DP9
	11	H	0.29	0.51	5.6	0.15	5.01	0.7															Basin H drains offsite to Black Forest Road @ DP11, drains to DP14
	12	I	0.08	0.60	5.0	0.05	5.17	0.2															Basin I drains offsite to Salt Brush Road @ DP12
	13	J	0.47	0.18	26.0	0.08	2.69	0.2															Basin J drains to DP13 (Ex 48" RCP - Trails at Forest Meadows F4)
		K	8.99	0.09	86.4	0.81	0.89	0.7															Undeveloped basin, follows historic drainage patterns to Cottonwood Creek @ DP14
	3	L	0.43	0.09	7.0	0.04	4.67	0.2	11.4	2.34	3.94	9.2											Basin L flows directly to Pond B @ DP3

STANDARD FORM SF-3 - PROPOSED CONDITIONS

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 5-Year

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

		DIRECT RUNOFF							TOTAL RUNOFF				OVERLAND			PIPE				TRAVEL TIME			
DESCRIPTION	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C* A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C* A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C* A (ac)	Slope (%)	Q_{pipe} (cfs)	C* A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	14											3.5											Total Site Discharge to Cottonwood Creek (Pond A & B outflow, Basin H & Basin K)

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3 - PROPOSED CONDITIONS
STORM DRAINAGE SYSTEM DESIGN

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 100-Year

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

		DIRECT RUNOFF							TOTAL RUNOFF				OVERLAND			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	I4							44.2															Existing Basin OSI4 flow @ DPI4 per Marksheffel Road Segment M3 FDR, follows historic drainage patterns to Cottonwood Creek
	EX4.1				17.4	2.37	5.53	13.1															Existing flow @ DP4.1 per Marksheffel Road Segment M3 FDR Ultimate Conditions, pipe to DP5
	EX11.2				30.5	22.50	4.12	92.8															Existing flow @ DP11.2 per Marksheffel Road Segment M3 FDR Ultimate Conditions, pipe to DP6
	EX3	MS1	0.28	0.83	5.4	0.23	8.50	2.0															Basin MS1 follows historic drainage patterns per Marksheffel Road Segment M3 FDR, captured in 5' CS Type 2 inlet @ DP EX3, pipe to DP4.1
	1	MS2	1.76	0.74	10.4	1.30	6.83	8.9															Basin MS3 follows historic drainage patterns per Marksheffel Road Segment M3 FDR, captured in 5' CS Type 2 inlet @ DP EX4, pipe to DP4.1
	EX4	MS3	0.34	0.85	5.2	0.29	8.58	2.5															Basin MS3 follows historic drainage patterns per Marksheffel Road Segment M3 FDR, captured in 5' CS Type 2 inlet @ DP EX4
	2	MS4	1.41	0.84	9.2	1.19	7.13	8.5	10.4	2.49	6.83	17.0											Basin MS4 captured @ DP2, pipe to DP5
		MS5	0.82	0.74	6.0	0.61	8.20	5.0															Basin MS5 captured @ DP3/Pond B
		MS6	2.57	0.81	11.4	2.09	6.61	13.8															Basin MS6 captured @ DP3, pipe to Pond B
	4	A	8.97	0.59	24.3	5.29	4.70	24.8															Basin A captured in series of Type R inlets @ DP4, pipe to DP7
	5	B	9.44	0.59	17.6	5.57	5.50	30.6	17.6	10.42	5.50	57.4											Basin B captured in series of Type R inlets @ DP5, combine with DP EX4.1 & DP2, pipe to DP8
	6	C	9.76	0.59	24.4	5.76	4.69	27.0	30.5	28.26	4.12	116.5											Basin C captured in series of Type R inlets @ DP6, combine with DP EX11.2, pipe to DP8
	7	D	6.00	0.59	18.7	3.54	5.36	19.0	24.3	8.83	4.70	41.5											Basin D captured in series of Type R inlets @ DP7, combine with DP4, pipe to DP8
	8	E	5.37	0.59	14.7	3.17	5.96	18.9	30.5	50.68	4.12	208.9											Basin E captured in series of Type R inlets @ DP8, combine with DP5-DP7, pipe to Pond A
	9	F	3.63	0.69	13.3	2.51	6.21	15.6															Basin F captured in series of Type R inlets @ DP9, pipe to Pond A
	10	G	4.18	0.39	12.0	1.62	6.48	10.5															Basin G captured at DP10 (Pond A), combine with DP8 & DP9
	11	H	0.29	0.67	5.6	0.19	8.41	1.6															Basin H drains offsite to Black Forest Road @ DP11, drains to DP14
	12	I	0.08	0.74	5.0	0.06	8.68	0.5															Basin I drains offsite to Salt Brush Road @ DP12
	13	J	0.47	0.42	26.0	0.20	4.52	0.9															Basin J drains to DP13 (Ex 48" RCP - Trails at Forest Meadows F4)
		K	8.99	0.36	86.4	3.24	1.50	4.8															Undeveloped basin, follows historic drainage patterns to Cottonwood Creek @ DP14

STANDARD FORM SF-3 - PROPOSED CONDITIONS
STORM DRAINAGE SYSTEM DESIGN

Subdivision: Schmidt PUD
Location: El Paso County
Design Storm: 100-Year

Project Name: Schmidt PUD
Project No.: 24013.00
Calculated By: NQJ
Checked By: REB
Date: 6/9/26

		DIRECT RUNOFF							TOTAL RUNOFF				OVERLAND			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	3	L	0.43	0.36	7.0	0.15	7.84	1.2	11.4	2.85	6.61	18.9											Basin L flows directly to Pond B @ DP3
	14											58.1											Total Site Discharge to Cottonwood Creek (Pond A & B outflow, Basin H & Basin K)

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

APPENDIX C – HYDRAULIC CALCULATIONS

MHFD-Inlet, Version 5.03 (August 2023)

INLET MANAGEMENT

Worksheet Protected

INLET NAME	Basin A	Basin B	Basin C
Site Type (Urban or Rural)	URBAN	URBAN	URBAN
Inlet Application (Street or Area)	STREET	STREET	STREET
Hydraulic Condition	On Grade	On Grade	On Grade
Inlet Type	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{Known} (cfs)	10.3	12.7	11.2
Major Q_{Known} (cfs)	24.8	30.6	27.0

Bypass (Carry-Over) Flow from Upstream Inlets must be organized from upstream (left) to downstream (right) in order for bypass flows to be linked.

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0

Watershed Characteristics

Subcatchment Area (acres)			
Percent Impervious			
NRCS Soil Type			

Watershed Profile

Overland Slope (ft/ft)			
Overland Length (ft)			
Channel Slope (ft/ft)			
Channel Length (ft)			

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)			
One-Hour Precipitation, P_1 (inches)			

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)			
One-Hour Precipitation, P_1 (inches)			

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**10.3**	**12.7**	**11.2**
Major Total Design Peak Flow, Q (cfs)	**24.8**	**30.6**	**27.0**
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0
Major Flow Bypassed Downstream, Q_b (cfs)	0.0	0.3	0.5

MHFD-Inlet, Version 5.03 (August 2023)

INLET MANAGEMENT

Worksheet Protected

INLET NAME	Basin D	Basin E	Basin F
Site Type (Urban or Rural)	URBAN	URBAN	URBAN
Inlet Application (Street or Area)	STREET	STREET	STREET
Hydraulic Condition	On Grade	On Grade	On Grade
Inlet Type	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{Known} (cfs)	7.9	7.8	7.2
Major Q_{Known} (cfs)	19.0	18.9	15.6

Bypass (Carry-Over) Flow from Upstream

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0

Watershed Characteristics

Subcatchment Area (acres)			
Percent Impervious			
NRCS Soil Type			

Watershed Profile

Overland Slope (ft/ft)			
Overland Length (ft)			
Channel Slope (ft/ft)			
Channel Length (ft)			

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)			
One-Hour Precipitation, P_1 (inches)			

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)			
One-Hour Precipitation, P_1 (inches)			

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**7.9**	**7.8**	**7.2**
Major Total Design Peak Flow, Q (cfs)	**19.0**	**18.9**	**15.6**
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0
Major Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0

MHFD-Inlet, Version 5.03 (August 2023)

INLET MANAGEMENT

Worksheet Protected

INLET NAME	Basin MS1 & MS2
Site Type (Urban or Rural)	URBAN
Inlet Application (Street or Area)	STREET
Hydraulic Condition	On Grade
Inlet Type	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{Known} (cfs)	8.7
Major Q_{Known} (cfs)	17.4

Bypass (Carry-Over) Flow from Upstream

Receive Bypass Flow from:	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0

Watershed Characteristics

Subcatchment Area (acres)	
Percent Impervious	
NRCS Soil Type	

Watershed Profile

Overland Slope (ft/ft)	
Overland Length (ft)	
Channel Slope (ft/ft)	
Channel Length (ft)	

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)	
One-Hour Precipitation, P_1 (inches)	

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)	
One-Hour Precipitation, P_1 (inches)	

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**8.7**
Major Total Design Peak Flow, Q (cfs)	**17.4**
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0
Major Flow Bypassed Downstream, Q_b (cfs)	0.0

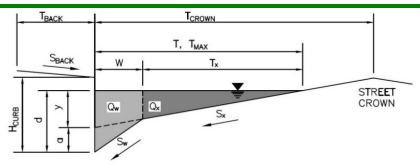
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin A**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	12.5	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	12.5	12.5	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

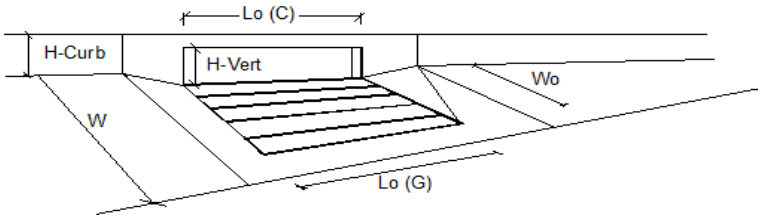
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**8.5**	**20.7**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than the design peak flow of 10.30 cfs on sheet 'Inlet Management'

WARNING: MAJOR STORM max. allowable capacity is less than the design peak flow of 24.80 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	4	4	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR & MAJOR STORM		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**10.3**	**24.8**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**100**	**%**

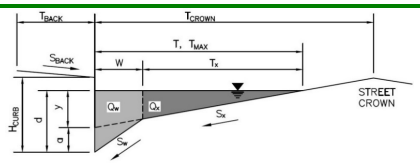
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin B**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	12.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.016	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	12.0	12.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

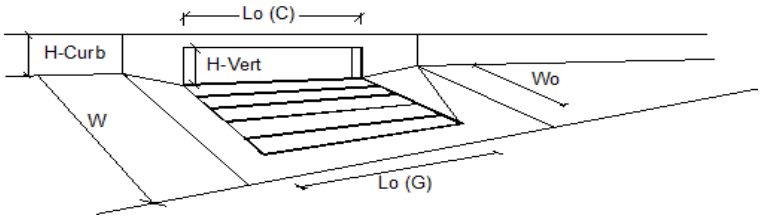
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**5.6**	**20.0**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than the design peak flow of 12.70 cfs on sheet 'Inlet Management'

WARNING: MAJOR STORM max. allowable capacity is less than the design peak flow of 30.60 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	6	6	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR & MAJOR STORM		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**12.7**	**30.3**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.3**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**99**	**%**

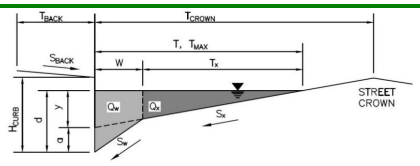
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin C**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	15.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.007	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	15.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

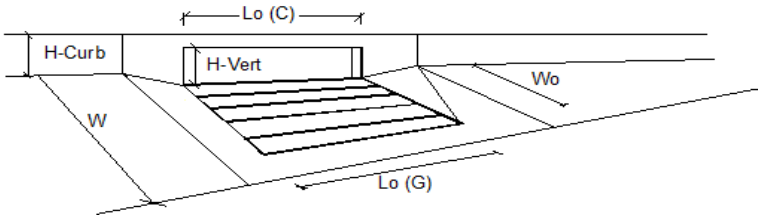
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**6.4**	**14.2**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than the design peak flow of 11.20 cfs on sheet 'Inlet Management'

WARNING: MAJOR STORM max. allowable capacity is less than the design peak flow of 27.00 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	4	4	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR & MAJOR STORM		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**11.2**	**26.5**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.5**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**98**	**%**

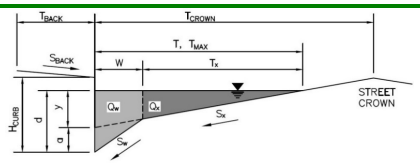
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin D**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	15.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	15.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

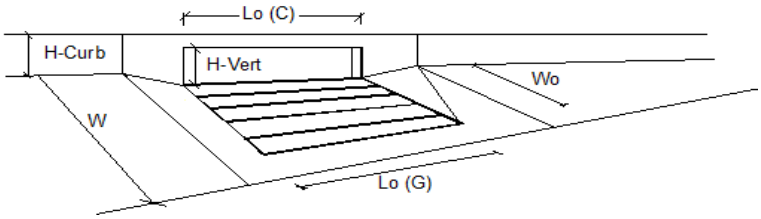
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**9.4**	**20.8**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 7.90 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 19.00 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	4	4	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.9**	**19.0**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**100**	**%**

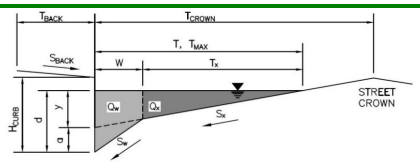
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin E**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	5.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	12.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.010	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	12.0	12.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

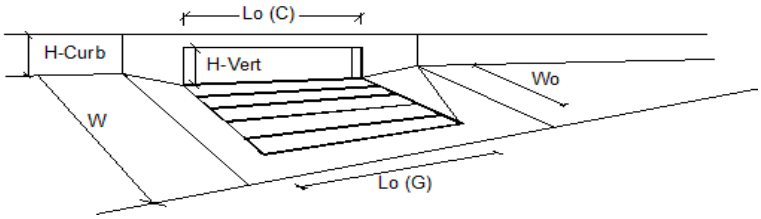
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.4**	**15.8**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than the design peak flow of 7.80 cfs on sheet 'Inlet Management'

WARNING: MAJOR STORM max. allowable capacity is less than the design peak flow of 18.90 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR & MAJOR STORM		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.8**	**18.9**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**100**	**%**

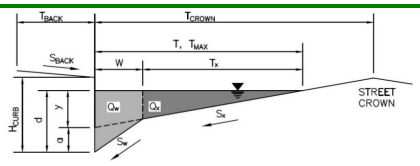
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin F**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	9.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	18.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	18.0	18.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

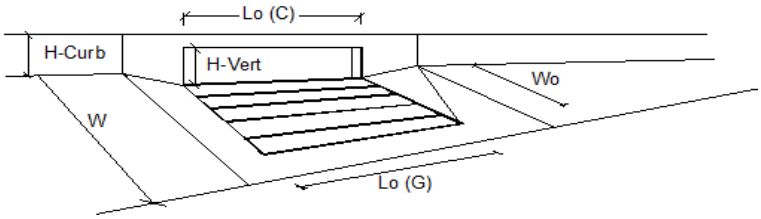
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Spread Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**14.9**	**21.4**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 7.20 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 15.60 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.2**	**15.6**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**100**	**%**

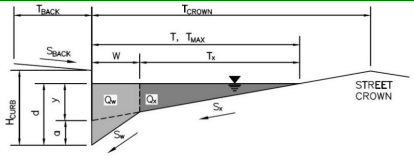
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Schmidt PUD - Preliminary Inlet Calcs**

Inlet ID: **Basin MS1 & MS2**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	34.0	ft
Gutter Width	W =	0.02	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.063	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.040	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	28.0	34.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

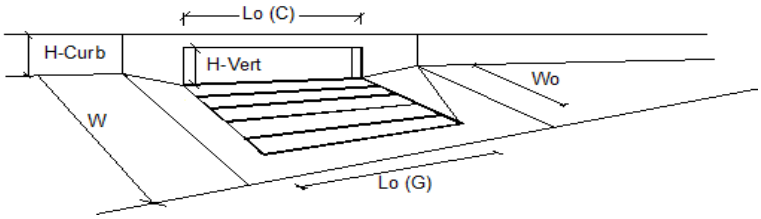
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**32.4**	**32.4**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 8.70 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 17.40 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**8.7**	**17.4**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**100**	**%**

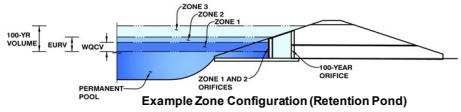
APPENDIX D – WATER QUALITY & DETENTION

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Schmidt PUD

Basin ID: Ultimate Pond A: CDR2413 Basins C, D, F-M+ Schmidt PUD Basins MS1-MS4, A-G = 36.79 ac @ 73% + 51.14 ac @ 60.2% = 87.93 ac @ 65.6%



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	87.93	acres
Watershed Length =	3,930	ft
Watershed Length to Centroid =	1,625	ft
Watershed Slope =	0.016	ft/ft
Watershed Imperviousness =	65.60%	percent
Percentage Hydrologic Soil Group A =	95.0%	percent
Percentage Hydrologic Soil Group B =	5.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

		Optional User Overrides	
Water Quality Capture Volume (WQCV) =	1.879 acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	7.133 acre-feet		acre-feet
2-yr Runoff Volume (P1 = 0.95 in.) =	4.146 acre-feet	0.95	inches
5-yr Runoff Volume (P1 = 1.23 in.) =	5.533 acre-feet	1.23	inches
10-yr Runoff Volume (P1 = 1.48 in.) =	6.862 acre-feet	1.48	inches
25-yr Runoff Volume (P1 = 1.88 in.) =	9.334 acre-feet	1.88	inches
50-yr Runoff Volume (P1 = 2.21 in.) =	11.572 acre-feet	2.21	inches
100-yr Runoff Volume (P1 = 2.57 in.) =	14.287 acre-feet	2.57	inches
500-yr Runoff Volume (P1 = 3.52 in.) =	21.349 acre-feet	3.52	inches
Approximate 2-yr Detention Volume =	3.737 acre-feet		
Approximate 5-yr Detention Volume =	5.024 acre-feet		
Approximate 10-yr Detention Volume =	6.262 acre-feet		
Approximate 25-yr Detention Volume =	8.321 acre-feet		
Approximate 50-yr Detention Volume =	9.550 acre-feet		
Approximate 100-yr Detention Volume =	10.854 acre-feet		

Define Zones and Basin Geometry

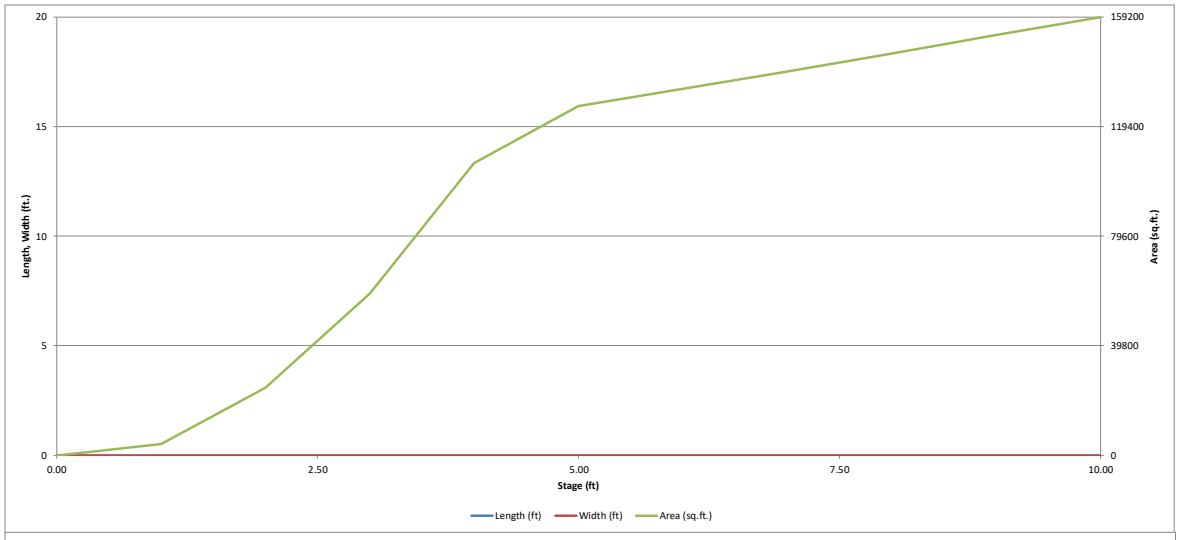
Zone 1 Volume (WQCV) =	1.879	acre-feet
Zone 2 Volume (EURV - Zone 1) =	5.253	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	3.721	acre-feet
Total Detention Basin Volume =	10.854	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

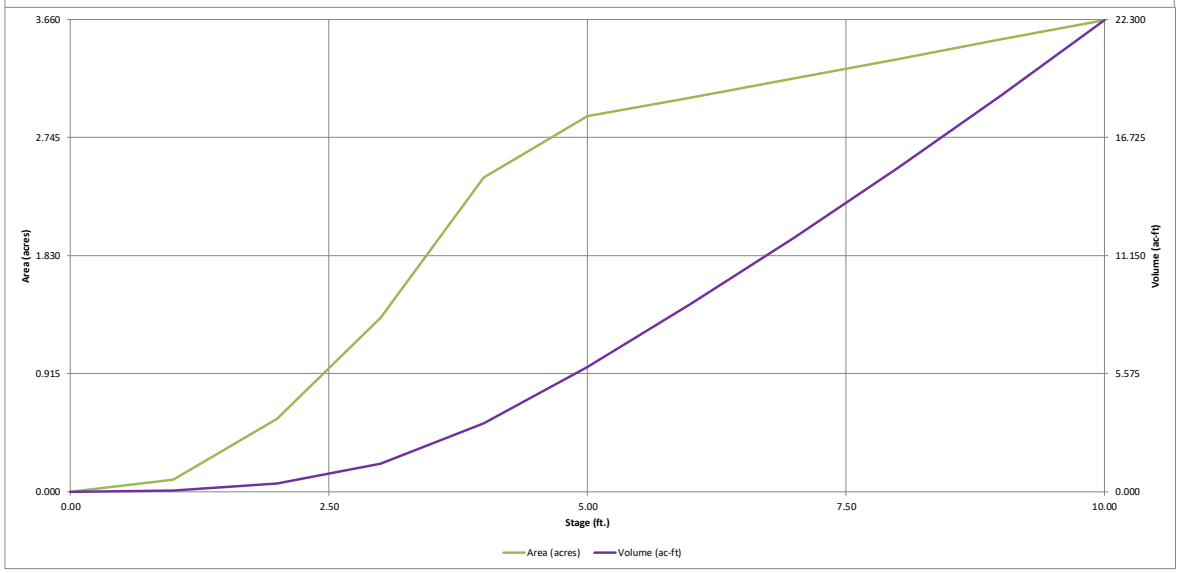
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
6969 **Top of Micropool**	--	0.00	--	--	--	50	0.001		
6970	--	1.00	--	--	--	3,998	0.092	2,024	0.046
6971	--	2.00	--	--	--	24,593	0.565	16,319	0.375
6972	--	3.00	--	--	--	58,705	1.348	57,968	1.331
6973	--	4.00	--	--	--	106,104	2.436	140,373	3.223
6974	--	5.00	--	--	--	126,732	2.909	256,791	5.895
6975	--	6.00	--	--	--	133,018	3.054	386,666	8.877
6976	--	7.00	--	--	--	139,432	3.201	522,891	12.004
6977	--	8.00	--	--	--	145,974	3.351	665,594	15.280
6978	--	9.00	--	--	--	152,646	3.504	814,904	18.708
6779	--	10.00	--	--	--	159,200	3.655	970,827	22.287

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



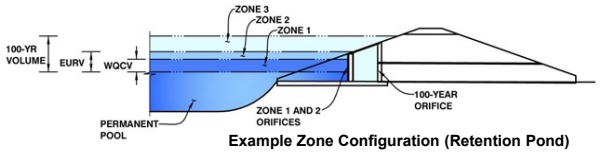


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: Schmidt PUD

Basin ID: Ultimate Pond A: CDR2413 Basins C, D, F-M+ Schmidt PUD Basins MS1-MS4, A-G = 36.79 ac @ 73% + 51.14 ac @ 60.2% = 87.93 ac @ 65.6%



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.36	1.879	Orifice Plate
Zone 2 (EURV)	5.43	5.253	Orifice Plate
Zone 3 (100-year)	6.64	3.721	Weir&Pipe (Restrict)
Total (all zones)		10.854	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft^2
Depth at top of Zone using Orifice Plate =	5.39	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	sq. inches	Elliptical Slot Area =	N/A	ft^2

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.80	3.59	4.25				
Orifice Area (sq. inches)	7.70	7.70	7.70	25.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.58	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	5.58	N/A	feet
Overflow Weir Front Edge Length =	6.00	N/A	feet	Overflow Weir Slope Length =	6.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	8.37	N/A	
Horiz. Length of Weir Sides =	6.00	N/A	feet	Overflow Grate Open Area w/o Debris =	25.06	N/A	ft^2
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	12.53	N/A	ft^2
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.50	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	2.99	N/A	ft^2
Outlet Pipe Diameter =	30.00	N/A	inches	Outlet Orifice Centroid =	0.83	N/A	feet
Restrictor Plate Height Above Pipe Invert =	17.60		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.75	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	7.50	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	1.17	feet
Spillway Crest Length =	50.00	feet	Stage at Top of Freeboard =	9.67	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	3.61	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	21.09	acre-ft

Calculated Parameters for Spillway

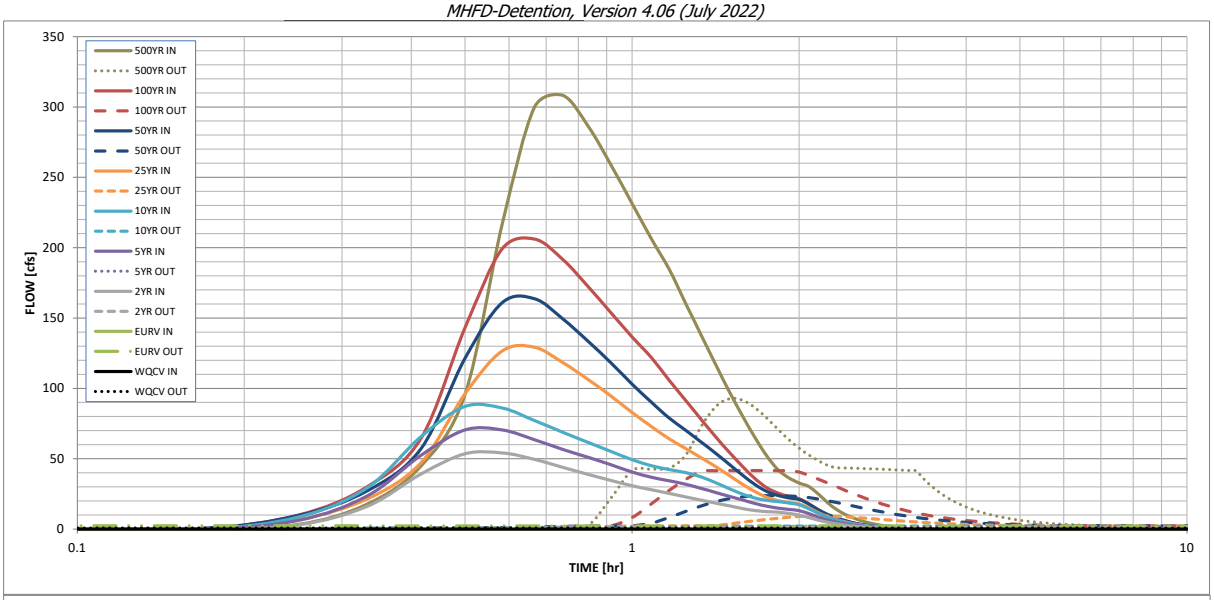
Routed Hydrograph Results

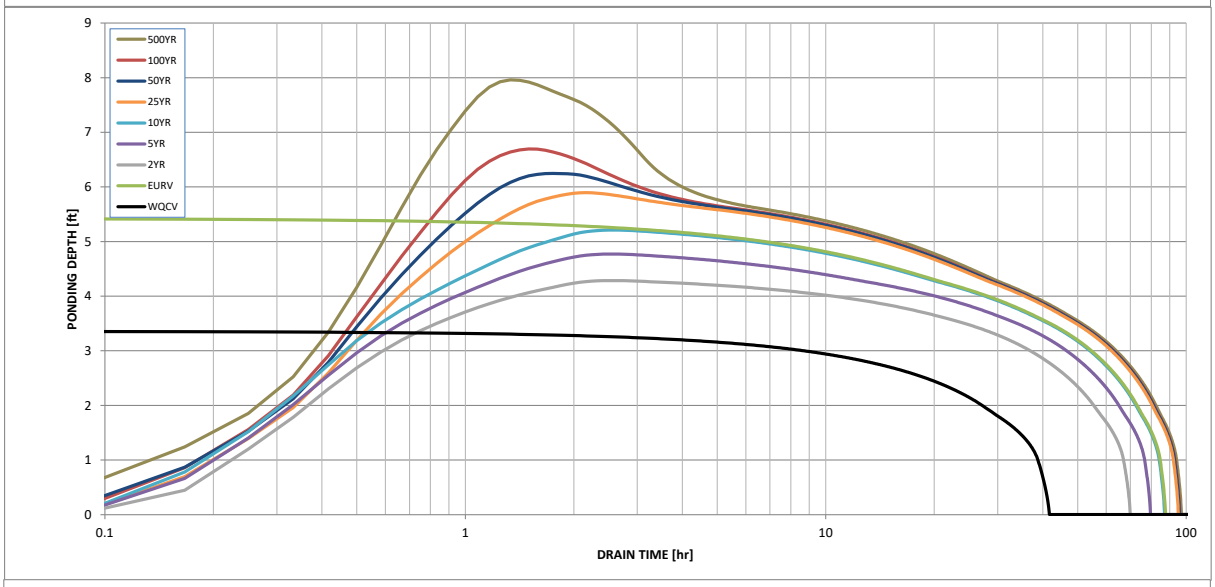
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

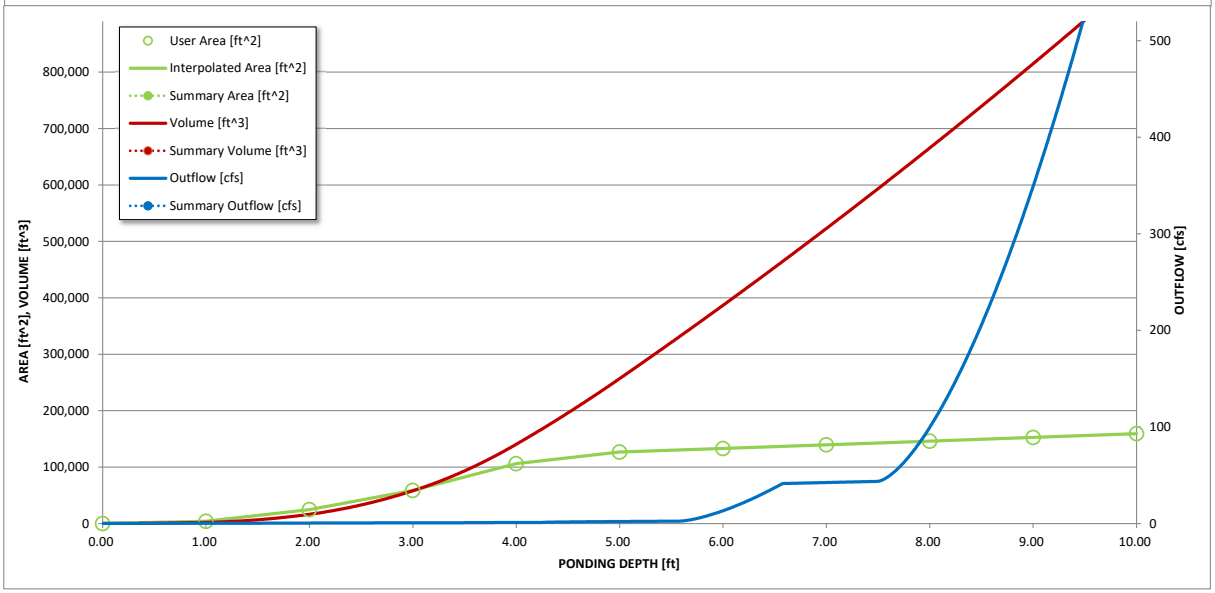
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.95	1.23	1.48	1.88	2.21	2.57	3.52
CUHP Runoff Volume (acre-ft) =	1.879	7.133	4.146	5.533	6.862	9.334	11.572	14.287	21.349
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	4.146	5.533	6.862	9.334	11.572	14.287	21.349
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.1	0.5	0.9	9.3	24.9	45.9	100.0
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.11	0.28	0.52	1.14
Peak Inflow Q (cfs) =	N/A	N/A	54.2	70.5	87.2	129.2	163.8	206.2	308.2
Peak Outflow Q (cfs) =	0.8	2.3	1.3	1.9	2.2	9.4	23.8	41.7	92.7
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	3.6	2.5	1.0	1.0	0.9	0.9
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.3	0.8	1.5	1.6
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	78	64	72	78	83	82	80	77
Time to Drain 99% of Inflow Volume (hours) =	**40**	84	67	76	83	90	90	89	88
Maximum Ponding Depth (ft) =	3.36	5.43	4.28	4.77	5.21	5.89	6.25	6.69	7.96
Area at Maximum Ponding Depth (acres) =	1.74	2.97	2.57	2.80	2.94	3.04	3.09	3.16	3.34
Maximum Volume Stored (acre-ft) =	1.886	7.159	3.923	5.210	6.480	8.542	9.614	11.019	15.113

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.49	0.23	3.33
	0:15:00	0.00	0.00	2.95	6.18	8.47	6.80	9.64	9.93	17.82
	0:20:00	0.00	0.00	16.26	23.62	29.50	21.54	27.24	30.22	44.97
	0:25:00	0.00	0.00	39.25	52.58	65.89	47.29	57.35	64.74	95.75
	0:30:00	0.00	0.00	53.56	70.48	87.18	96.20	121.47	143.12	216.63
	0:35:00	0.00	0.00	54.18	70.44	86.02	126.59	160.88	199.16	299.59
	0:40:00	0.00	0.00	49.57	63.60	77.22	129.23	163.78	206.18	308.20
	0:45:00	0.00	0.00	43.89	56.58	68.77	118.31	149.39	191.60	286.42
	0:50:00	0.00	0.00	38.89	51.04	61.67	106.27	133.46	172.33	258.54
	0:55:00	0.00	0.00	34.55	45.66	55.10	94.45	118.07	153.64	231.38
	1:00:00	0.00	0.00	30.77	40.63	49.34	82.88	103.06	136.61	206.28
	1:05:00	0.00	0.00	27.99	36.84	45.07	72.90	90.17	121.88	184.65
	1:10:00	0.00	0.00	25.38	34.33	42.34	64.10	78.90	105.76	160.12
	1:15:00	0.00	0.00	22.84	31.83	40.08	57.21	70.11	91.30	137.55
	1:20:00	0.00	0.00	20.56	28.91	37.02	50.59	61.72	77.75	116.38
	1:25:00	0.00	0.00	18.46	25.95	32.86	44.23	53.70	65.22	96.94
	1:30:00	0.00	0.00	16.43	23.15	28.55	37.81	45.68	54.19	80.00
	1:35:00	0.00	0.00	14.55	20.70	24.82	31.77	38.17	44.36	64.88
	1:40:00	0.00	0.00	13.14	18.16	22.03	26.55	31.64	35.84	51.82
	1:45:00	0.00	0.00	12.44	16.24	20.47	22.56	26.69	29.35	42.08
	1:50:00	0.00	0.00	12.09	14.95	19.50	20.28	23.92	25.57	36.40
	1:55:00	0.00	0.00	11.11	14.01	18.58	18.85	22.19	23.22	32.76
	2:00:00	0.00	0.00	9.91	13.11	17.29	17.90	21.05	21.61	30.21
	2:05:00	0.00	0.00	8.17	10.98	14.45	15.10	17.74	17.95	24.93
	2:10:00	0.00	0.00	6.36	8.54	11.25	11.71	13.74	13.67	18.87
	2:15:00	0.00	0.00	4.90	6.56	8.64	8.93	10.47	10.26	14.07
	2:20:00	0.00	0.00	3.77	5.04	6.61	6.82	7.98	7.75	10.59
	2:25:00	0.00	0.00	2.89	3.85	5.04	5.21	6.10	5.94	8.09
	2:30:00	0.00	0.00	2.20	2.89	3.77	3.91	4.58	4.47	6.09
	2:35:00	0.00	0.00	1.65	2.13	2.81	2.91	3.39	3.35	4.55
	2:40:00	0.00	0.00	1.21	1.56	2.10	2.17	2.53	2.51	3.41
	2:45:00	0.00	0.00	0.87	1.13	1.55	1.61	1.88	1.86	2.52
	2:50:00	0.00	0.00	0.59	0.78	1.08	1.14	1.33	1.31	1.77
	2:55:00	0.00	0.00	0.36	0.51	0.69	0.75	0.87	0.86	1.15
	3:00:00	0.00	0.00	0.19	0.30	0.40	0.44	0.51	0.50	0.67
	3:05:00	0.00	0.00	0.08	0.14	0.18	0.21	0.24	0.24	0.31
	3:10:00	0.00	0.00	0.03	0.04	0.05	0.07	0.07	0.07	0.09
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

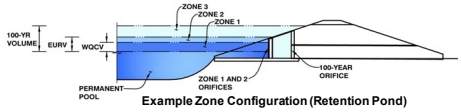
Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Schmidt PUD

Basin ID: Pond B: Basin MS5, MS6 & L



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	3.82	acres
Watershed Length =	1,205	ft
Watershed Length to Centroid =	54	ft
Watershed Slope =	0.007	ft/ft
Watershed Imperviousness =	65.10%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

		Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.081 acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.272 acre-feet		acre-feet
2-yr Runoff Volume (P1 = 0.95 in.) =	0.175 acre-feet	0.95	inches
5-yr Runoff Volume (P1 = 1.23 in.) =	0.244 acre-feet	1.23	inches
10-yr Runoff Volume (P1 = 1.48 in.) =	0.313 acre-feet	1.48	inches
25-yr Runoff Volume (P1 = 1.88 in.) =	0.446 acre-feet	1.88	inches
50-yr Runoff Volume (P1 = 2.21 in.) =	0.547 acre-feet	2.21	inches
100-yr Runoff Volume (P1 = 2.57 in.) =	0.668 acre-feet	2.57	inches
500-yr Runoff Volume (P1 = 3.52 in.) =	0.970 acre-feet	3.52	inches
Approximate 2-yr Detention Volume =	0.168 acre-feet		
Approximate 5-yr Detention Volume =	0.231 acre-feet		
Approximate 10-yr Detention Volume =	0.304 acre-feet		
Approximate 25-yr Detention Volume =	0.364 acre-feet		
Approximate 50-yr Detention Volume =	0.396 acre-feet		
Approximate 100-yr Detention Volume =	0.445 acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.081	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.191	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.173	acre-feet
Total Detention Basin Volume =	0.445	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	50	0.001		
6989	--	1.00	--	--	--	4,770	0.110	2,410	0.055
6990	--	2.00	--	--	--	6,086	0.140	7,838	0.180
6991	--	3.00	--	--	--	7,483	0.172	14,622	0.336
6992	--	4.00	--	--	--	8,961	0.206	22,844	0.524
6993	--	5.00	--	--	--	10,521	0.242	32,585	0.748

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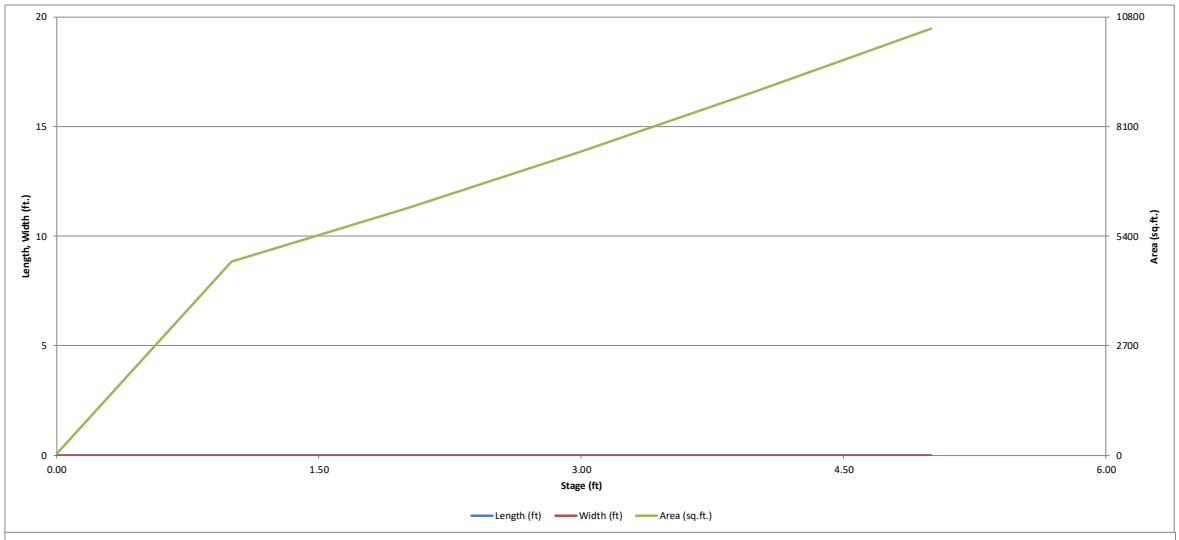
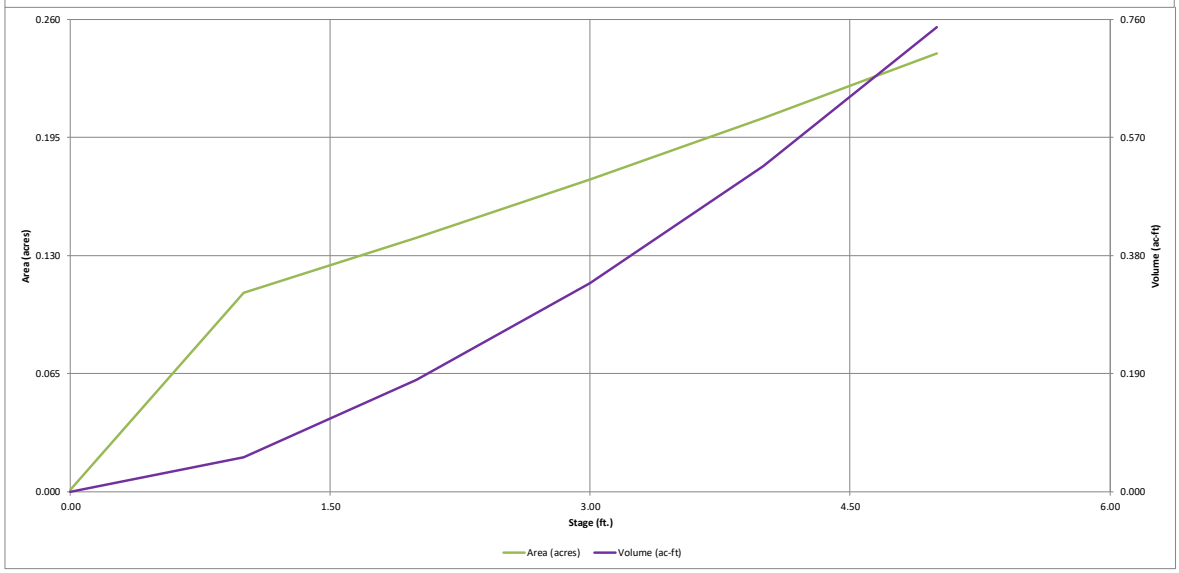
DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



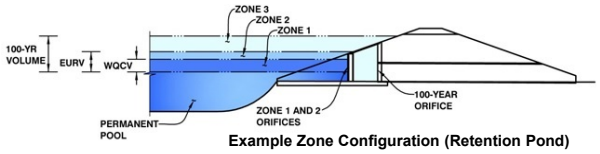


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: Schmidt PUD

Basin ID: Pond B: Basin MS5, MS6 & L



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.23	0.081	Orifice Plate
Zone 2 (EURV)	2.62	0.191	Orifice Plate
Zone 3 (100-year)	3.60	0.173	Weir&Pipe (Restrict)
Total (all zones)		0.445	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	5.347E-03	ft²
Depth at top of Zone using Orifice Plate =	2.61	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	10.40	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	0.77	sq. inches (diameter = 1 inch)	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.87	1.74					
Orifice Area (sq. inches)	0.77	0.77	0.77					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A	ft²
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.61	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	2.61	N/A	feet
Overflow Weir Front Edge Length =	2.00	N/A	feet	Overflow Weir Slope Length =	2.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	5.40	N/A	
Horiz. Length of Weir Sides =	2.00	N/A	feet	Overflow Grate Open Area w/o Debris =	2.78	N/A	ft²
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	1.39	N/A	ft²
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.50	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	0.52	N/A	ft²
Outlet Pipe Diameter =	18.00	N/A	inches	Outlet Orifice Centroid =	0.29	N/A	feet
Restrictor Plate Height Above Pipe Invert =	6.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.23	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	3.30	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.58	feet
Spillway Crest Length =	8.00	feet	Stage at Top of Freeboard =	4.88	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	0.24	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	0.72	acre-ft

Calculated Parameters for Spillway

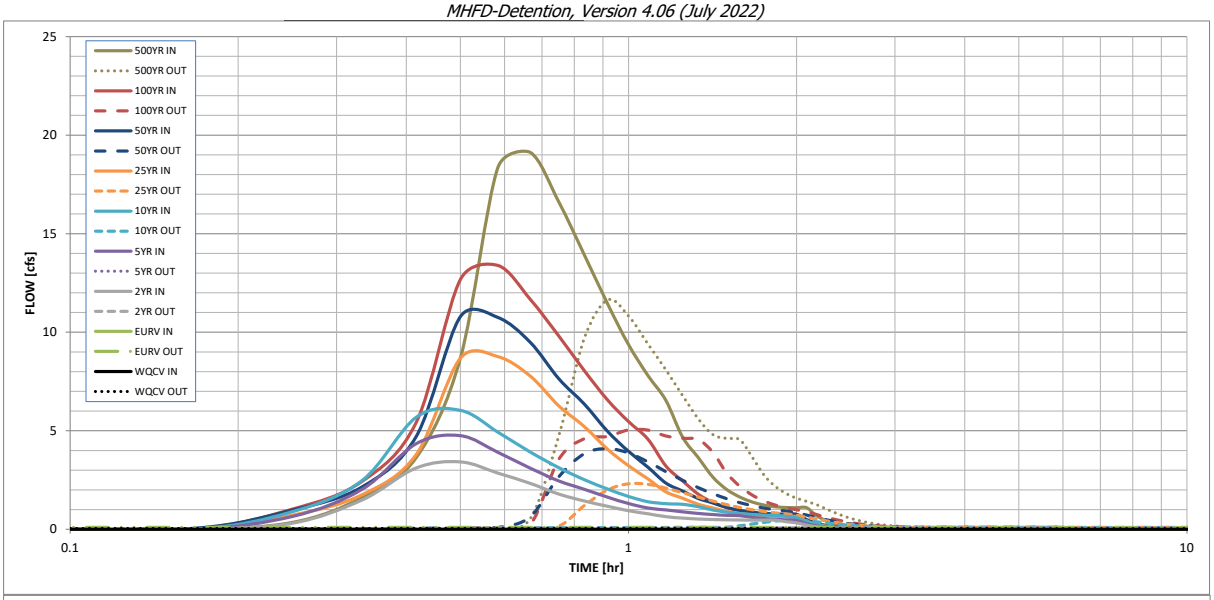
Routed Hydrograph Results

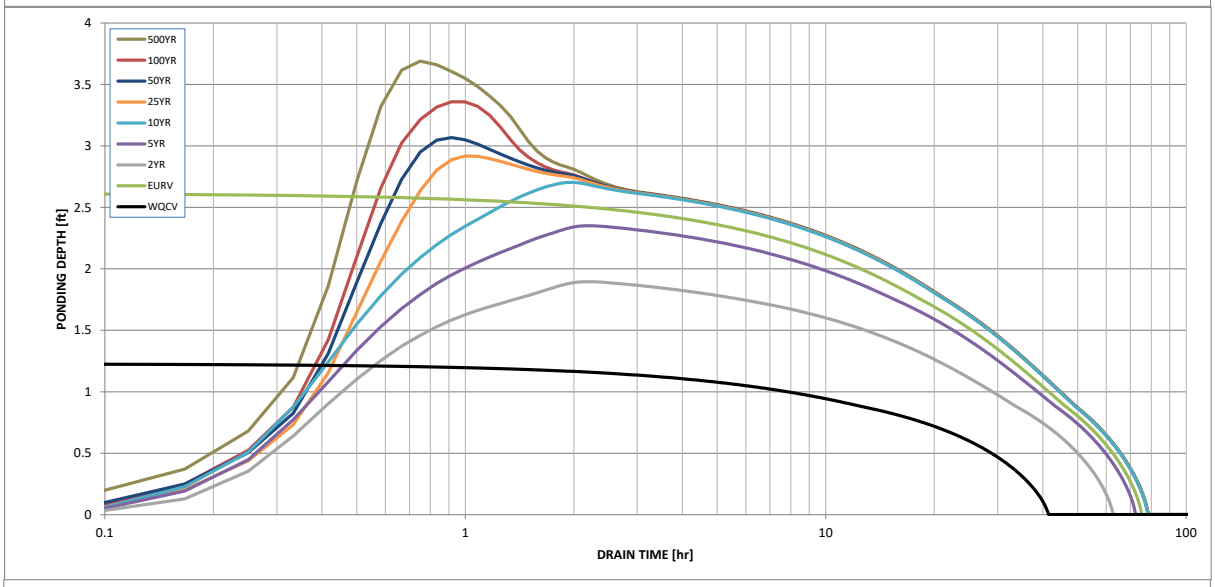
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

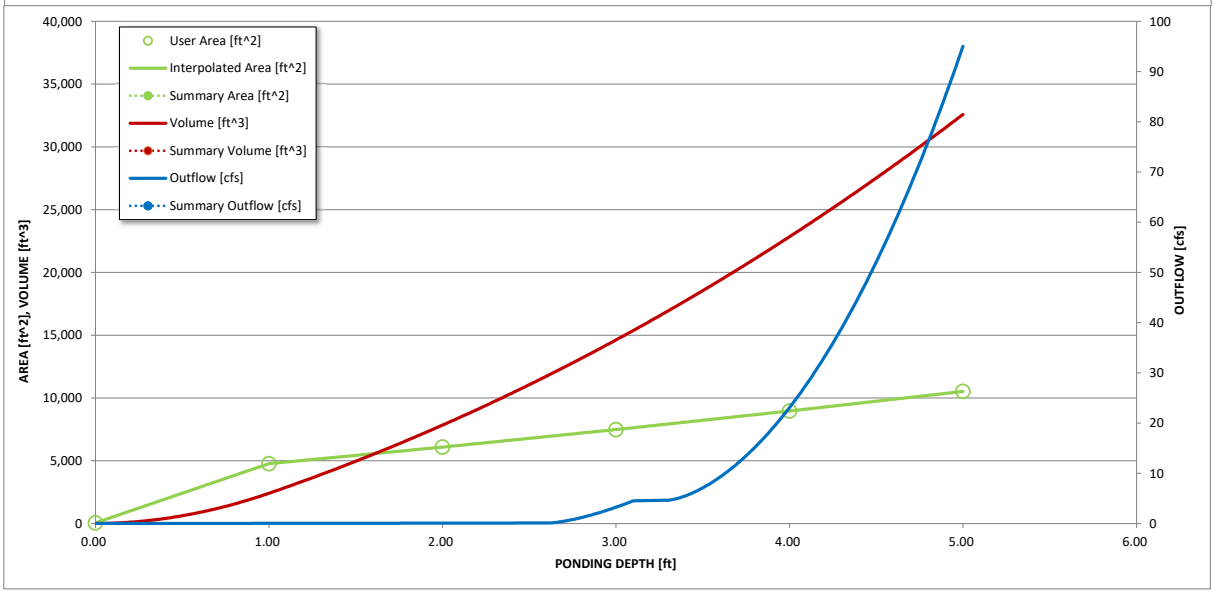
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.95	1.23	1.48	1.88	2.21	2.57	3.52
CUHP Runoff Volume (acre-ft) =	0.081	0.272	0.175	0.244	0.313	0.446	0.547	0.668	0.970
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.175	0.244	0.313	0.446	0.547	0.668	0.970
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.1	0.5	1.2	3.3	4.5	6.0	9.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.13	0.32	0.86	1.18	1.57	2.51
Peak Inflow Q (cfs) =	N/A	N/A	3.4	4.8	6.0	8.8	10.8	13.4	19.1
Peak Outflow Q (cfs) =	0.0	0.1	0.1	0.1	0.5	2.3	4.1	5.0	11.6
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.2	0.4	0.7	0.9	0.8	1.2
Structure Controlling Flow =	Plate	Overflow Weir 1	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	0.00	N/A	N/A	0.1	0.8	1.4	1.6	1.7
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	67	57	65	69	66	64	62	57
Time to Drain 99% of Inflow Volume (hours) =	**40**	72	60	69	74	73	72	71	69
Maximum Ponding Depth (ft) =	1.23	2.62	1.89	2.35	2.70	2.92	3.07	3.36	3.69
Area at Maximum Ponding Depth (acres) =	0.12	0.16	0.14	0.15	0.16	0.17	0.17	0.18	0.19
Maximum Volume Stored (acre-ft) =	0.081	0.273	0.165	0.229	0.286	0.320	0.346	0.398	0.460

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.05	0.02	0.35
	0:15:00	0.00	0.00	0.31	0.65	0.90	0.72	0.98	1.02	1.61
	0:20:00	0.00	0.00	1.49	2.03	2.48	1.76	2.15	2.43	3.64
	0:25:00	0.00	0.00	3.11	4.34	5.68	3.79	4.71	5.42	8.69
	0:30:00	0.00	0.00	3.42	4.75	6.04	8.68	10.79	12.68	18.35
	0:35:00	0.00	0.00	2.85	3.91	4.93	8.76	10.75	13.39	19.13
	0:40:00	0.00	0.00	2.33	3.11	3.92	7.78	9.49	11.68	16.61
	0:45:00	0.00	0.00	1.79	2.45	3.13	6.26	7.64	9.81	13.93
	0:50:00	0.00	0.00	1.44	2.03	2.51	5.22	6.36	8.05	11.46
	0:55:00	0.00	0.00	1.17	1.63	2.05	4.07	4.98	6.57	9.37
	1:00:00	0.00	0.00	0.94	1.31	1.67	3.22	3.96	5.47	7.80
	1:05:00	0.00	0.00	0.79	1.08	1.40	2.56	3.15	4.58	6.54
	1:10:00	0.00	0.00	0.64	0.98	1.30	1.90	2.35	3.21	4.68
	1:15:00	0.00	0.00	0.57	0.89	1.27	1.57	1.94	2.45	3.64
	1:20:00	0.00	0.00	0.52	0.80	1.14	1.27	1.56	1.78	2.64
	1:25:00	0.00	0.00	0.50	0.75	0.98	1.09	1.34	1.36	2.02
	1:30:00	0.00	0.00	0.48	0.71	0.87	0.91	1.11	1.11	1.64
	1:35:00	0.00	0.00	0.47	0.69	0.80	0.80	0.96	0.94	1.38
	1:40:00	0.00	0.00	0.47	0.60	0.75	0.73	0.87	0.83	1.23
	1:45:00	0.00	0.00	0.46	0.54	0.72	0.68	0.81	0.78	1.14
	1:50:00	0.00	0.00	0.46	0.50	0.70	0.66	0.78	0.76	1.11
	1:55:00	0.00	0.00	0.38	0.48	0.66	0.65	0.76	0.75	1.10
	2:00:00	0.00	0.00	0.33	0.45	0.59	0.64	0.76	0.75	1.10
	2:05:00	0.00	0.00	0.21	0.29	0.38	0.42	0.49	0.49	0.71
	2:10:00	0.00	0.00	0.13	0.18	0.24	0.27	0.31	0.31	0.46
	2:15:00	0.00	0.00	0.08	0.11	0.15	0.17	0.20	0.19	0.28
	2:20:00	0.00	0.00	0.05	0.07	0.09	0.10	0.12	0.12	0.17
	2:25:00	0.00	0.00	0.02	0.04	0.05	0.06	0.07	0.07	0.10
	2:30:00	0.00	0.00	0.01	0.02	0.02	0.03	0.03	0.03	0.05
	2:35:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.
The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

APPENDIX E – REFENCE MATERIAL

URS
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TECHNOLOGY
WORK™

Cottonwood Creek Drainage Basin Planning Study

City of Colorado Springs and El Paso County

APPENDIX B

SUPPORTING DOCUMENTATION

JUNE 9, 1994

COTTONWOOD CREEK DBPS - TABLE 3

CHANNEL & CULVERT RECOMMENDATIONS

REACH	HEC-1 FLOW w/det. (CFS)	CHANNEL OR CULVERT LENGTH(FT)	EXISTING IMPROVEMENT	RECOMMENDED IMPROVEMENT	TOTAL BOTTOM WIDTH (FT)	TOTAL DEPTH (FT)
21 TO DESIGN POINT 20	11,173	1,800	NATURAL	NATURAL EXCEPT PROVIDE BURIED RIPRAP & GRADE CONTROL AT I-25 BRIDGES ONLY (500'& 2 DROPS)	50	14.0
20 TO DESIGN POINT 19	11,112	2,400	NATURAL	NATURAL EXCEPT PROVIDE GABION WALLS & GRADE CONTROL AT BRIDGES (600') & PORTIONS OF SOUTH BANK (1,550')	40	15.0
19 TO DESIGN POINT 19.2	11,127	1,500	NATURAL	NATURAL EXCEPT PROVIDE GABION WALLS & GRADE CONTROL FOR PORTIONS OF SOUTH BANK (300') & NORTH BANK (500')	40	15.0
19.2 TO DESIGN POINT 19.1	11,127	700	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 3 DROPS)	40	16.0
19.1 TO DESIGN POINT 18	11,127	1,200	F.LINED	NO IMPROVEMENT REQUIRED	80	12.0
18 TO DESIGN POINT 18.1	10,000	400	P.LINED	NO IMPROVEMENT REQUIRED	115	12.0
18.1 TO DESIGN POINT 17	10,000	2,000	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 12 DROPS)	115	12.0
17 TO DESIGN POINT 17.2	9,837	1,000	P.LINED	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 5 DROPS)	115	12.0
17.2 TO DESIGN POINT 17.1	9,837	1,100	P.LINED	NO IMPROVEMENT REQUIRED	115	9.0
17.1 TO DESIGN POINT 16	9,837	1,000	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 2 DROPS)	100	11.0
16 TO DESIGN POINT 15	9,416	2,500	P.LINED	LOWER CHANNEL BOTTOM BY NATURAL EROSION WITH PHASED BURIED RIPRAP BANKS & GRADE CONTROL (WITH 3 DROPS)	150	6.0
15 TO DESIGN POINT 14	8,790	1,400	P.LINED	INCREASE DEPTH OF BURIED RIPRAP LINING ON NORTH SIDE & GRADE CONTROL (WITH 4 DROPS)	150	7.5
14 TO DESIGN POINT 14.1	8,598	1,300	P.LINED	NO IMPROVEMENT REQUIRED	200	8.0
14.1 TO DESIGN POINT 13	8,598	1,000	P.LINED	INCREASE DEPTH OF BURIED RIPRAP LINING ON NORTH SIDE	200	6.5
13 TO DESIGN POINT 13.2	4,702	2,000	P.LINED	NEED BURIED RIPRAP LINING ON SOUTH SIDE & GRADE CONTROL (WITH 3 DROPS)	200	5.0
13.2 TO DESIGN POINT 13.1	4,702	1,100	P.LINED	NEED BURIED RIPRAP LINING ON SOUTH SIDE & GRADE CONTROL (WITH 3 DROPS)	105	7.0
13.1 TO DESIGN POINT 12	4,702	2,300	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 5 DROPS)	105	7.0
12 TO DESIGN POINT 11	4,026	5,500	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 16 DROPS)	80	8.0
11 TO DESIGN POINT 10	3,008	1,100	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 3 DROPS)	80	6.5
10 TO DESIGN POINT 9	2,531	1,800	NATURAL	NATURAL BOTTOM WITH BURIED RIPRAP BANKS & GRADE CONTROL (WITH 4 DROPS)	80	6.5
9 TO DESIGN POINT 8	2,332	3,000	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 10 DROPS)	80	6.5
8 TO DESIGN POINT 7	1,715	3,500	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 12 DROPS)	80	5.0
7 TO DESIGN POINT 6	854	3,500	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 9 DROPS)	50	4.5
6 TO DESIGN POINT 5	673	7,000	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 12 DROPS)	50	4.0
5 TO DESIGN POINT 4	870	3,000	VEG. LINING	GRADE CONTROL (7 DROPS)	50	4.0
4 TO DESIGN POINT 3	467	3,500	VEG. LINING	GRADE CONTROL (12 DROPS)	20	4.5
3 TO DESIGN POINT 2	335	3,500	VEG. LINING	GRADE CONTROL (11 DROPS)	20	4.0
2 TO DESIGN POINT 1	114	2,300	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 5 DROPS)	20	3.0
T4B TO DESIGN POINT 19G	528	1,700	STORM/NAT.	STORM SEWER FROM EXISTING 48" CMP OUTLET DOWNSTREAM TO MAIN CHANNEL	72" RCP	N/A
19E TO DESIGN POINT 19F	1,560	2,400	CONCRETE	NO IMPROVEMENT REQUIRED	12	6.0
19D TO DESIGN POINT 19E	1,285	3,400	CONCRETE	NO IMPROVEMENT REQUIRED	13	6.0
19B TO DESIGN POINT 19C	810	2,800	CONCRETE	NO IMPROVEMENT REQUIRED	5	5.0
Q1 TO DESIGN POINT 19A	390	2,200	72" PIPE	PARALLEL PIPE (1,650')	36" RCP	N/A
T4A TO DESIGN POINT 18A	260	1,200	48" PIPE	NO IMPROVEMENT REQUIRED	N/A	N/A
17A TO DESIGN POINT 17	312	1,000	66" PIPE	NO IMPROVEMENT REQUIRED	N/A	N/A
T2 TO DESIGN POINT 17A	205	1,200	NATURAL	STORM SEWER	48" RCP	N/A
P2 TO DESIGN POINT 16C	368	2,300	48" PIPE	NO IMPROVEMENT REQUIRED	N/A	N/A
M4 TO DESIGN POINT 16B	492	2,400	P.LINED	NO IMPROVEMENT REQUIRED	N/A	N/A
M3 TO DESIGN POINT 16	291	1,400	36" PIPE	PARALLEL PIPE (1,250')	30" RCP	N/A
M1 TO DESIGN POINT 14C	513	3,000	48" PIPE	PARALLEL PIPE (2,000')	48" RCP	N/A
O1 TO DESIGN POINT 14B	426	3,700	48" PIPE	PARALLEL PIPE (2,600')	48" RCP	N/A
L1 TO DESIGN POINT 14A	371	1,700	42" PIPE	PARALLEL PIPE (1,700')	36"/42" RCP	N/A
J1 TO DESIGN POINT 13S	425	3,100	42" PIPE	PARALLEL PIPE	48" RCP	N/A
13Q TO DESIGN POINT 13R	1,058	750	NATURAL	NATURAL BOTTOM WITH GRASS/SHRUB BANKS & GRADE CONTROL (WITH 1 DROP)	75	4.0

COTTONWOOD CREEK DBPS - TABLE 4
BRIDGE RECOMMENDATIONS

LOCATION	HEC-1 FLOW w/det. (CFS)	EXISTING IMPROVEMENT	PROPOSED IMPROVEMENT
CDOT BRIDGES			
I-25 (DP 21)	11,173	BRIDGE, T=137,B=113,D=18	REPL. BRIDGES (8 LANES), 2 @ 70' WIDE X 140' LONG
I-25 (DP 31)	5,613	BRIDGE, T=189,B=39,D=43	REPL. BRIDGES (8 LANES), 2 @ 70' WIDE X 190' LONG
CITY BRIDGES			
CORPORATE DRIVE (DP 21)	11,173	BRIDGE, T=123,B=61,D=15.8	NO IMPROVEMENT REQUIRED
VINCENT DRIVE (DP 20)	11,112	BRIDGE, T=209,B=53,D=33.5	REPL. BRIDGE (6 LANE ART.), 107' WIDE BY 210' LONG
CURRENT ACCESS RD (DP 20)	11,112	HORSESHOE - 2 @ 22' x 22'	NO IMPROVEMENT REQUIRED
ACADEMY BLVD (DP 18)	10,000	CBC - 5 @ 20' x 9'	NO IMPROVEMENT REQUIRED
UNION BLVD (DP 16)	9,416	CBC - 7 @ 12' x 6'	REPL. BRIDGE (6 LANE ART.), 107' WIDE BY 150' LONG
RANGEWOOD DRIVE (DP 13)	7,844	CM ARCH - 4 @ 25.5' x 13'	NO IMPROVEMENT REQUIRED
WOODMEN ROAD (DP 12)	4,026	BRIDGE - 118' x 22'	REPL. BRIDGE (8 LANE ART.), 143' WIDE BY 200' LONG
AUSTIN BLUFFS (DP 12)	4,026	N/A	REPL. BRIDGE (8 LANE ART.), 143' WIDE BY 200' LONG
POWERS BLVD (DP 9)	2,332	N/A	TRIPLE 10' X 9' CBC (360')
DUBLIN BLVD (DP 13O)	2,414	CMP - 2 @ 132"	ADD PARALLEL 108" CMP
AUSTIN BLUFFS (DP 13J)	3,686	CMP - 2@120"	QUADRUPLE 12' X 9' CBC
RESEARCH PKWY (DP 8G)	1,632	N/A	TRIPLE 14' X 6' CBC
PINE CREEK RD (DP 31)	5,613	BRIDGE, T=185,B=45,D=41	NO IMPROVEMENT REQUIRED
OLD RAILOAD GRADE (DP 31)	5,613	HORSESHOE - 2 @ 25' x 21'	NO IMPROVEMENT REQUIRED
ACADEMY BLVD N (DP SUM13)	2,586	CBC - 10' x 6'	REPLACE WITH TRIPLE 12' X 9' CBC
COUNTY BRIDGES			
BLACK FOREST ROAD (DP 7)	854	BRIDGE, T=40,B=26,D=17	REPL. BRIDGE (5 LANE ART.), 70' WIDE BY 110' LONG

COTTONWOOD CREEK DBPS
RESULTS OF BASIN INVENTORY

TABLE 8

REACH	EXISTING PROTECTION	AVAILABLE/ ADDITIONAL CAPACITY	EROSION CONSIDERATIONS	WETLAND CONSIDERATIONS	WILDLIFE CONSIDERATIONS	COMPATIBILITY W/ UTILITIES	LAND USE COMPATIBILITY	LAND AVAILABILITY	MULTI-USE OPPORTUNITIES	CAPITAL COST PER ALTERNATIVE	MAINTENANCE CONSIDERATIONS PER ALTERNATIVE	SAFETY OR FLOOD PROTECTION CONSIDERATIONS	COMMENTS	CONCLUSIONS
DES. PT. 9 TO DES. PT. 10	NATURAL OR VEGETATED CHANNEL 80 B + 15 D, 1:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 10 TO 20 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MOST LIKELY JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT ARE PRESENT	PROPOSED MAJOR UTILITY MAIN ON NORTH SIDE OF CHANNEL, NEEDS TO BE CONSIDERED IN FINAL DESIGN AND ROW REQUIREMENTS	PARK OR OPEN SPACE LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY	PLANNED TRAIL CORRIDOR PLANNED LINEAR PARK PLANNED UTILITY CORRIDOR	FULL LINING = $ 447,000 PARTIAL LINING = $ 313,000 NATURAL = $ 1,480,000	MAINTENANCE OF ALTERNATIVE A IS NOT PRACTICAL DUE TO HIGH VELOCITIES AND EROSION - OTHER ALTERNATIVES ARE ACCEPTABLE	HIGH VELOCITIES ANTICIPATED - NATURAL ALTERNATIVE MAY NOT BE PRACTICAL SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS		USE ALTERNATIVE B
DES. PT. 8 TO DES. PT. 9	NATURAL OR VEGETATED CHANNEL 80 B + 10 D, 3:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 10 TO 20 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	PROPOSED MAJOR UTILITY MAIN ON NORTH SIDE OF CHANNEL, NEEDS TO BE CONSIDERED IN FINAL DESIGN AND ROW REQUIREMENTS	PARK OR OPEN SPACE LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY	PLANNED TRAIL CORRIDOR PLANNED LINEAR PARK PLANNED UTILITY CORRIDOR	FULL LINING = $ 734,000 PARTIAL LINING = $ 468,000 NATURAL = $ 1,892,000	MAINTENANCE OF ALTERNATIVE A IS NOT PRACTICAL DUE TO HIGH VELOCITIES AND EROSION - OTHER ALTERNATIVES ARE ACCEPTABLE	HIGH VELOCITIES ANTICIPATED - NATURAL ALTERNATIVE MAY NOT BE PRACTICAL SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS		USE ALTERNATIVE B
DES. PT. 7 TO DES. PT. 8	NATURAL OR VEGETATED CHANNEL 80 B + 10 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE BOTTOM SIDE-SLOPES EXPOSED IN ROCK	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	PARK OR OPEN SPACE LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY	PLANNED TRAIL CORRIDOR PLANNED LINEAR PARK	FULL LINING = $ 740,000 PARTIAL LINING = $ 500,000 NATURAL = $ 886,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	HIGH VELOCITIES ANTICIPATED - NATURAL ALTERNATIVE MAY NOT BE PRACTICAL SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS		USE ALTERNATIVE B
DES. PT. 6 TO DES. PT. 7	NATURAL OR VEGETATED CHANNEL 80 B + 10 D, 2 :1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE BOTTOM SIDE-SLOPES EXPOSED IN ROCK	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MOST LIKELY JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT ARE PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 1,102,000 PARTIAL LINING = $ 1,837,000 NATURAL = $ 1,746,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES VELOCITY RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS		USE ALTERNATIVE B
DES. PT. 5 TO DES. PT. 6	NATURAL OR VEGETATED CHANNEL 80 B + 10 D, 2 :1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE BOTTOM SIDE-SLOPES EXPOSED IN ROCK	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MOST LIKELY JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT ARE PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 1,148,000 PARTIAL LINING = $ 840,000 NATURAL = $ 1,750,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES VELOCITY RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS	RURAL TYPE SETTING	USE ALTERNATIVE B USE ALTERNATIVE A
DES. PT. 4 TO DES. PT. 5	NATURAL OR VEGETATED CHANNEL 50 B + 8 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MOST LIKELY JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT ARE PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 347,000 PARTIAL LINING = $ 484,000 NATURAL = $ 787,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	SLOPE RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES VELOCITY RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS	RURAL TYPE SETTING	USE ALTERNATIVE A
DES. PT. 3 TO DES. PT. 4	NATURAL OR VEGETATED CHANNEL 20 B + 8 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 728,000 PARTIAL LINING = $ 626,000 NATURAL = $ 1,316,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES	RURAL TYPE SETTING	USE ALTERNATIVE A
DES. PT. 2 TO DES. PT. 3	NATURAL OR VEGETATED CHANNEL 20 B + 5 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 510,000 PARTIAL LINING = $ 456,000 NATURAL = $ 451,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES	RURAL TYPE SETTING	USE ALTERNATIVE A
DES. PT. 1 TO DES. PT. 2	NATURAL OR VEGETATED CHANNEL 20 B + 4 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	MOST LIKELY NOT JURISDICTIONAL WETLANDS PRESENT CONTACT WITH CORPS OF ENGR. REPRESENTATIVE MAY SURVEY	MOST LIKELY NOT JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 180,000 PARTIAL LINING = $ 242,000 NATURAL = $ 290,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES	RURAL TYPE SETTING	USE ALTERNATIVE A
DES. PT. 5A TO DES. PT. 5	NATURAL OR VEGETATED CHANNEL 20 B + 6 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MOST LIKELY JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT ARE PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 323,000 PARTIAL LINING = $ 418,000 NATURAL = $ 368,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES	RURAL TYPE SETTING	USE ALTERNATIVE A
DES. PT. B2 TO DES. PT. 6A	NATURAL OR VEGETATED CHANNEL 20 B + 6 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	MOST LIKELY JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN STAGE	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 438,000 PARTIAL LINING = $ 414,000 NATURAL = $ 876,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES		USE ALTERNATIVE B
DES. PT. C2 TO DES. PT. 8A	NATURAL OR VEGETATED CHANNEL 20 B + 4 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 223,000 PARTIAL LINING = $ 478,000 NATURAL = $ 799,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES		USE A COMBINATION OF STREETS/STORM SEWER OR STREETS/ROADSIDE DITCH
DES. PT. 8A TO DES. PT. 8B	NATURAL OR VEGETATED CHANNEL 40 B + 4 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	PARK OR OPEN SPACE LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY	PLANNED OPEN SPACE	FULL LINING = $ 817,000 PARTIAL LINING = $ 809,000 NATURAL = $ 1,320,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	STEEP EXISTING SLOPES - DROPS LESS THAN 300 FT APART VELOCITY RANGE IS ACCEPTABLE FOR ALL ALTERNATIVES 100YR. RUNOFF > 500 CFS		USE ALTERNATIVE B
DES. PT. 8C TO DES. PT. 8D	NATURAL OR VEGETATED CHANNEL 40 B + 6 D, 2:1 SIDE-SLOPE	CAPACITY AVAILABLE FOR FULL FLOW THROUGH HYDROLOGY VELOCITY= 5 TO 10 FPS	MODERATELY ERODABLE SOIL BOTTOM AND SIDE-SLOPES	LIKELY SOME JURISDICTIONAL WETLANDS ARE PRESENT DETAILED STUDY REQUIRED AT FINAL DESIGN TO DETERMINE EXTENT OF WETLANDS	MAY BE JURISDICTIONAL WILDLIFE CORRIDORS OR HABITAT PRESENT	NO EXISTING OR PROPOSED MAJOR UTILITY MAINS ANTICIPATED	RESIDENTIAL LAND USES NEXT TO FACILITY	ADJACENT PROPERTY UNDEVELOPED NEXT TO FACILITY		FULL LINING = $ 616,000 PARTIAL LINING = $ 778,000 NATURAL = $ 1,136,000	SIMILAR MAINTENANCE CONSIDERATIONS FOR ALL THREE ALTERNATIVES	100YR. RUNOFF 500 CFS - PRACTICAL ALTERNATIVES ARE STORM SEWER SYSTEM OR ROADSIDE DITCHES		USE A COMBINATION OF STREETS/STORM SEWER OR STREETS/ROADSIDE DITCH

NOTE: FINAL RECOMMENDATIONS ARE INCLUDED IN THE DBPS AND REFLECT MINOR CHANGES DUE TO COMMENTS AND/OR MORE DETAILED EVALUATION.

URS CONSULTANTS
MAKING TECHNOLOGY WORK

COTTONWOOD CREEK DBPS - TABLE 12

SUBBASIN TIME OF CONCENTRATION DATA

DATE 18-May-92

		OVERLAND			STREET			PIPE			CHANNEL			ADDED TIMES	USING OVERALL Tc		ACTUAL Tc PICKED	
BASIN	"C"	S(%)	L(FT)	t(MIN)	L(FT)	V(FPS)	t(MIN)	L(FT)	V(FPS)	t(MIN)	L(FT)	V(FPS)	t(MIN)	TOTAL T(MIN)	TOTAL L(MI)	TOTAL T(MIN)	USE T(MIN)	LAG (HR)
A1	0.25	5.0	750	25.0	600	3.0	3.3		7.0	0.0	3800	5.0	12.7	41.0	0.98	17.8	41.0	0.41
A2	0.25	5.0	500	22.0	600	3.0	3.3		7.0	0.0	3100	5.0	10.3	35.7	0.80	15.3	35.7	0.36
A3	0.25	5.0	500	22.0	600	3.0	3.3		7.0	0.0	3500	5.0	11.7	37.0	0.87	16.4	37.0	0.37
A4	0.25	8.0	500	18.0	600	3.0	3.3		7.0	0.0	3100	5.0	10.3	31.7	0.80	12.7	31.7	0.32
A5	0.25	6.0	500	20.0	600	3.0	3.3		7.0	0.0	3600	5.0	12.0	35.3	0.89	15.5	35.3	0.35
A6	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	3500	5.0	11.7	34.0	0.87	14.4	34.0	0.34
A7	0.25	5.0	500	22.0	600	3.0	3.3		7.0	0.0	3100	5.0	10.3	35.7	0.80	15.3	35.7	0.36
A8	0.25	4.0	500	24.0	600	3.0	3.3		7.0	0.0	3500	5.0	11.7	39.0	0.87	17.8	39.0	0.39
A9	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	3200	5.0	10.7	33.0	0.81	13.6	33.0	0.33
A10	0.25	6.0	500	20.0	600	3.0	3.3		7.0	0.0	4000	5.0	13.3	36.7	0.97	16.5	36.7	0.37
A11	0.25	8.0	500	18.0	600	3.0	3.3		7.0	0.0	2300	5.0	7.7	29.0	0.64	10.8	29.0	0.29
A12	0.25	8.0	500	18.0	600	3.0	3.3		7.0	0.0	2300	5.0	7.7	29.0	0.64	10.8	29.0	0.29
A13	0.25	4.0	500	24.0	600	3.0	3.3		7.0	0.0	2600	5.0	8.7	36.0	0.70	15.1	36.0	0.36
B1	0.25	8.0	500	18.0	600	3.0	3.3		7.0	0.0	2800	5.0	9.3	30.7	0.74	12.0	30.7	0.31
B2	0.25	8.0	500	18.0	600	3.0	3.3		7.0	0.0	2700	7.5	6.0	27.3	0.72	11.8	27.3	0.27
B3	0.25	5.0	300	16.0	600	3.0	3.3		7.0	0.0	2800	7.5	6.2	25.6	0.70	13.8	25.6	0.26
B4	0.25	5.0	300	16.0	600	3.0	3.3		7.0	0.0	5000	10.0	8.3	27.7	1.12	19.8	27.7	0.28
B5	0.90	5.0	100	10.0	600	3.0	3.3	750	7.0	1.8	2300	10.0	3.8	19.0	0.71	14.0	19.0	0.19
B6	0.55	4.0	100	10.0	600	3.0	3.3	1650	7.0	3.9	2000	10.0	3.3	20.6	0.82	17.1	20.6	0.21
B7	0.55	3.0	100	10.0	600	3.0	3.3	8550	7.0	20.4		10.0	0.0	33.7	1.75	34.1	34.1	0.34
B8	0.55	3.0	100	10.0	600	3.0	3.3	6000	7.0	14.3		10.0	0.0	27.6	1.27	26.6	27.6	0.28
B9	0.60	3.0	100	10.0	600	3.0	3.3	3000	7.0	7.1	7000	10.0	11.7	32.1	2.03	38.2	38.2	0.38
C1	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	3500	10.0	5.8	28.2	0.87	14.4	28.2	0.28
C2	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	2400	10.0	4.0	26.3	0.66	11.6	26.3	0.26
C3	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	4300	10.0	7.2	29.5	1.02	16.3	29.5	0.30
C4	0.25	7.0	500	19.0	600	3.0	3.3		7.0	0.0	5000	10.0	8.3	30.7	1.16	17.9	30.7	0.31
C5	0.25	4.0	500	24.0	600	3.0	3.3	6000	7.0	14.3	700	10.0	1.2	42.8	1.48	26.8	42.8	0.43
C6	0.50	3.0	100	10.0	600	3.0	3.3	3600	7.0	8.6	2000	10.0	3.3	25.2	1.19	25.4	25.4	0.25
C7	0.50	5.0	100	10.0	600	3.0	3.3	4500	7.0	10.7	3000	10.0	5.0	29.0	1.55	25.5	29.0	0.29

COTTONWOOD CREEK DBPS - TABLE 13
URS PROJECT NUMBER 49209
SUMMARY OF ALTERNATIVE CONSTRAINTS

AREA 5

COTTONWOOD CREEK MAIN CHANNEL
BLACK FOREST ROAD TO EAST SIDE OF THE BASIN

100-YEAR FLOW RANGES FROM LOW TO 1,500 CFS
100-YEAR VELOCITY RANGES FROM 5 TO 8 FPS

DESCRIPTION

The existing channel in this area transitions from the deeply incised channel characteristic of area 4 to numerous small tributaries in the Black Forest area. The channel varies in width from 20 to 80 feet. The capacity is available to handle the 100-year storm. This area predominantly has herbaceous wetlands where they are present with some shrub wetlands. The potential for wetlands is high with a significant amount of open water but it has been grazed and disturbed. The wildlife habitat has realtively high beta diversity along the channel but is not very significant away from the channel. The land uses planned for this area transition from urban residential densities by Black Forest Road to rural five acre lot densities in the Colorado Black Forest. The soils in this area include some sandstone and claystone outcrops with the majority being glacial deposits. The area in the forest will be difficult to maintain with heavy construction equipment since there is limited access available.

ALTERNATIVE	ADVANTAGES OF ALTERNATIVE	DISADVANTAGES OF ALTERNATIVE
ALTERNATIVE A	The right-of-way is available for this type of channel since the planned densities are low Limited permitting involved - only for bridges, grade control where needed, isolated bank protection and maintenance considerations Limited mitigation required only for any new bridges planned	Downcutting of the streambed and bank erosion may be a problem High maintenance costs may be involved due to the erodable nature of the soils and the access difficulties
ALTERNATIVE B	Minimal right-of-way requirements beyond the existing channel width Use of drop structures could actually enhance the diversity of the wetland types and wildlife habitat Able to control erosion problems that are occurring May be able to improve aesthetics with this treatment	May require some removal and replacement of existing vegetation Some onsite mitigation may be required Some disturbance of wetlands will occur during construction May have some problems with actually constructing this type of facility in the forest area
ALTERNATIVE C/D		This alternative was eliminated due the the difficulties with construction and mitigation

RECOMMENDATIONS

For this area the recommended solution is a combination of alternatives A and B. Alternative B would be used mostly in the westerly portion of the basin in order to transition from the reach downstream and alternative A would be used for the remainder of the area.

COTTONWOOD CREEK DBPS - TABLE 16 SHEET 3 OF 19

RATIONAL METHOD - SUBBASIN HYDROLOGIC DATA - PLATTED AREAS

DATE: 18-May-92

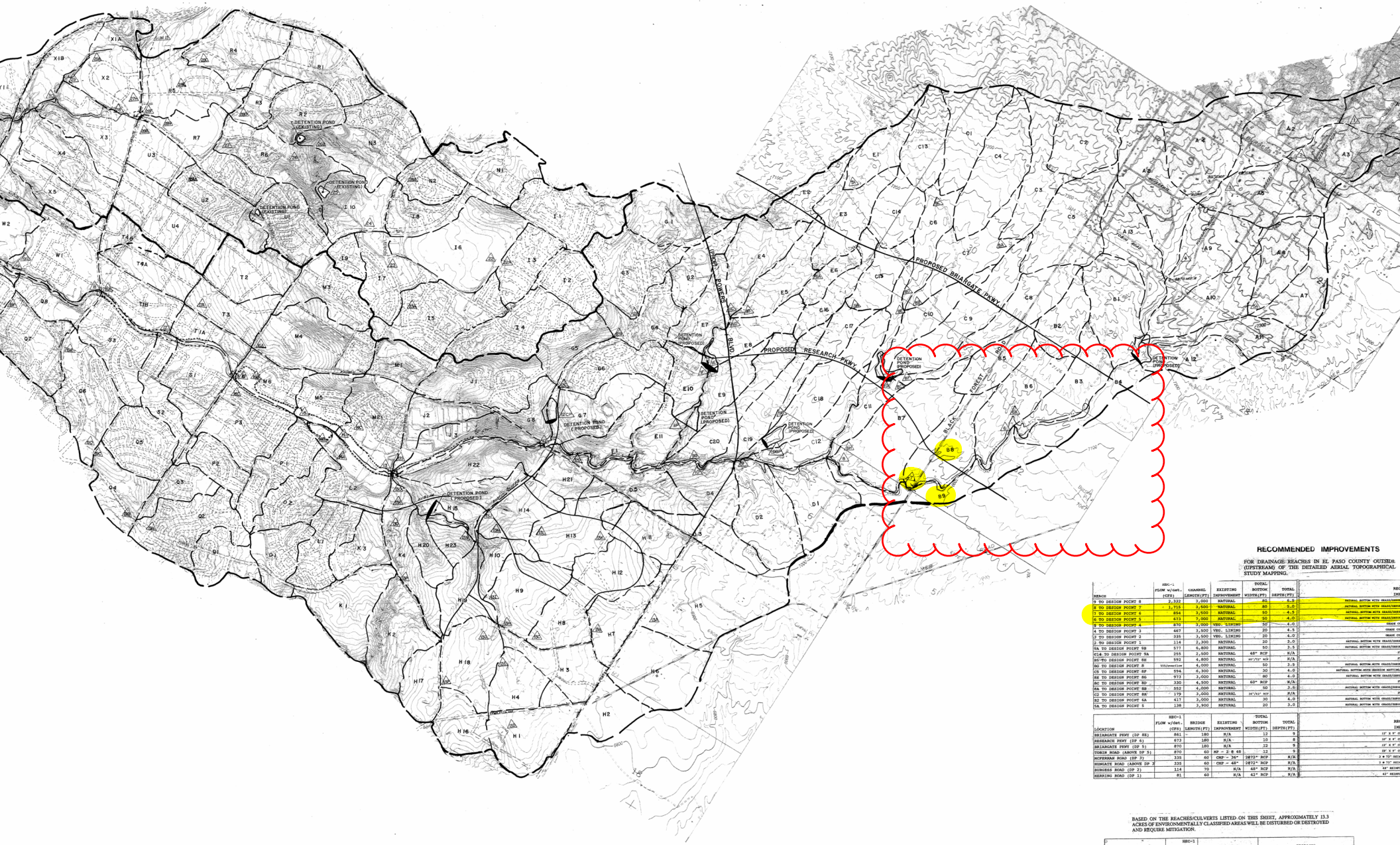
BASIN	HYDROLOGIC SOIL TYPE	PARKS	AVERAGE LAND USE TYPE AND AREA IN ACRES										OVERALL VALUES	
			COMMER OR BUSIN	INDUST	RESID 1/8 AC	RESID 1/6 AC	RESID 1/4 AC	RESID 1/3 AC	RESID 1/2 AC	RESID 1 AC	RESID 5 AC	SCHOOL /CHUR	AREA AC/SM	C
B6	B		0.0		0.0	0.0	9.9						9.9	0.58
	C												0.015	
	D													
B7	B				0.0	0.0	0.0				0.0	0.0	0.0	N/A
	C				0.0	0.0	0.0						0.000	
	D													
B8	B		0.0			0.0							0.0	N/A
	C												0.000	
	D													
B9	B												21.6	0.65
	C					0.0	21.6						0.034	
	D													
C1	B						0.0			38.1	0		38.1	0.40
	C												0.060	
	D													
C2	B									0.0	0.0		0.0	N/A
	C												0.000	
	D													
C3	B						0.0			0.0	2.0		2.0	0.20
	C												0.003	
	D													
C4	B						0.0			0.0	2.0	0.0	2.0	0.20
	C												0.003	
	D													
C5	B						0.0			0.0	37.2		37.2	0.20
	C												0.058	
	D													

COTTONWOOD CREEK DBPS - TABLE 17 SHEET 3 OF 19

RATIONAL METHOD - SUBBASIN HYDROLOGIC DATA - UNPLATTED AREAS

DATE: 18-May-92

BASIN	HYDROLOGIC SOIL TYPE	PARKS	AVERAGE LAND USE TYPE AND AREA IN ACRES										OVERALL VALUES	
			COMMER OR BUSIN	INDUST	RESID 1/8 AC	RESID 1/6 AC	RESID 1/4 AC	RESID 1/3 AC	RESID 1/2 AC	RESID 1 AC	RESID 5 AC	SCHOOL /CHUR	AREA AC/SM	C
B6	B		11.5		24.5	46.3	25.0						107.3	0.65
	C												0.168	
	D													
B7	B				13.5	104.3	21.6				6.4	6.4	190.1	0.63
	C				5.6	28.6	3.7						0.297	
	D													
B8	B		39.4			62.4							101.8	0.73
	C												0.159	
	D													
B9	B												86.4	0.71
	C					64.8	21.6						0.135	
	D													
C1	B						15.6			33.7	17.9		67.2	0.39
	C												0.105	
	D													
C2	B									12.1	86.2		98.3	0.22
	C												0.154	
	D													
C3	B						27.1			49.0	25.5		101.6	0.40
	C												0.159	
	D													
C4	B						53.2			68.8	13.2	5.9	141.1	0.46
	C												0.220	
	D													
C5	B						54.9			45.7	7.8		108.4	0.47
	C												0.169	
	D													



RECOMMENDED IMPROVEMENTS

FOR DRAINAGE REACHES IN EL PASO COUNTY OUTSIDE (UPSTREAM) OF THE DETAILED AERIAL TOPOGRAPHICAL STUDY MAPPING.

REACH	HEC-1 FLOW w/det. (CFS)	CHANNEL LENGTH(FT)	EXISTING IMPROVEMENT	TOTAL BOTTOM WIDTH(FT)	TOTAL DEPTH(FT)	RE IM
9 TO DESIGN POINT 8	2,332	3,000	NATURAL	80	6.5	NATURAL BOTTOM WITH GRASS/SHRUB
8 TO DESIGN POINT 7	1,715	3,500	NATURAL	80	5.0	NATURAL BOTTOM WITH GRASS/SHRUB
7 TO DESIGN POINT 6	854	3,500	NATURAL	50	4.5	NATURAL BOTTOM WITH GRASS/SHRUB
6 TO DESIGN POINT 5	673	7,000	NATURAL	50	4.0	NATURAL BOTTOM WITH GRASS/SHRUB
5 TO DESIGN POINT 4	870	3,000	VEG. LINING	50	4.0	GRASS CH
4 TO DESIGN POINT 3	467	3,500	VEG. LINING	20	4.5	GRASS CH
3 TO DESIGN POINT 2	335	3,500	VEG. LINING	20	4.0	GRASS CH
2 TO DESIGN POINT 1	114	2,300	NATURAL	20	3.0	NATURAL BOTTOM WITH GRASS/SHRUB
9A TO DESIGN POINT 9B	577	6,800	NATURAL	50	3.5	NATURAL BOTTOM WITH GRASS/SHRUB
C14 TO DESIGN POINT 9A	255	2,500	NATURAL	48" RCP	N/A	P
EC-70 TO DESIGN POINT 8E	593	4,800	NATURAL	30"/32" RCP	N/A	P
8G TO DESIGN POINT 8	535/overflow	4,000	NATURAL	50	3.5	NATURAL BOTTOM WITH GRASS/SHRUB
C5 TO DESIGN POINT 8F	594	6,300	NATURAL	30	4.0	NATURAL BOTTOM WITH GRASS/SHRUB
8E TO DESIGN POINT 8G	973	3,000	NATURAL	80	4.0	NATURAL BOTTOM WITH GRASS/SHRUB
8C TO DESIGN POINT 8D	330	4,500	NATURAL	60" RCP	N/A	P
8A TO DESIGN POINT 8B	552	4,000	NATURAL	50	3.5	NATURAL BOTTOM WITH GRASS/SHRUB
C2 TO DESIGN POINT 8A	179	3,000	NATURAL	24"/42" RCP	N/A	P
B2 TO DESIGN POINT 6A	417	3,000	NATURAL	30	4.0	NATURAL BOTTOM WITH GRASS/SHRUB
5A TO DESIGN POINT 5	138	3,900	NATURAL	20	3.0	NATURAL BOTTOM WITH GRASS/SHRUB

LOCATION	HEC-1 FLOW w/det. (CFS)	BRIDGE LENGTH(FT)	EXISTING IMPROVEMENT	TOTAL BOTTOM WIDTH(FT)	TOTAL DEPTH(FT)	RE IM
BRIARGATE PKWY (DP 8E)	861	180	N/A	12	5	12' X 5' C
RESEARCH PKWY (DP 6)	673	180	N/A	10	6	10' X 6' C
BRIARGATE PKWY (DP 5)	870	180	N/A	12	9	12' X 9' C
TORIN ROAD (ABOVE DP 5)	870	60	RP - 2 @ 48	12	9	12' X 9' C
MCFERRAN ROAD (DP 3)	335	60	CMP - 36"	2@72" RCP	N/A	2 @ 72" RCP
HUNGATE ROAD (ABOVE DP 2	335	60	CMP - 48"	2@72" RCP	N/A	2 @ 72" RCP
BURGESS ROAD (DP 2)	114	70	N/A	48" RCP	N/A	48" RCP
HERRING ROAD (DP 1)	81	60	N/A	42" RCP	N/A	42" RCP

BASED ON THE REACHES/CULVERTS LISTED ON THIS SHEET, APPROXIMATELY 13.3 ACRES OF ENVIRONMENTALLY CLASSIFIED AREAS WILL BE DISTURBED OR DESTROYED AND REQUIRE MITIGATION.

LOCATION	HEC-1 FLOW w/det. (CFS)	EXISTING IMPROVEMENT	PROPOSED IMPROVEMENT
CITY BRIDGES			
RESEARCH PKWY.(DP 8G)	1,032	N/A	TRIPLE 14' X 4' CMC
COUNTY BRIDGES			
BLACK FOREST ROAD (DP 7)	854	BRIDGE, S=40,B=26,R=17	REPLACE FOR 3 LANE ARTERIAL, 70' R 110'

LEGEND:

- A1 SUB-BASIN DESIGNATION
- △ DESIGN POINT
- — — MAJOR BASIN BOUNDARY
- — — SUB-BASIN BOUNDARY

SHEET INDEX:

MAPPING:

LANDMARK MAPPING INC.

ENGINEER:

URS CONSULTANTS
MAKING TECHNOLOGY WORK
1042 SOUTH EIGHTH STREET
COLORADO SPRINGS, CO. 80906
(719)634-6699

PROJECT:

COTTONWOOD CREEK
OVERALL BASIN MAP
SCALE 1" = 1000'

NOTE: TRAIL CORRIDORS ARE ALSO S
ON THIS FIGURE.

LEGEND:

- RESIDENTIAL < 1/8AC
- RESIDENTIAL = 1/8AC
- RESIDENTIAL = 1/4AC
- RESIDENTIAL = 1/3 AC
- RESIDENTIAL = 1/2 AC
- RESIDENTIAL = 1 AC
- RESIDENTIAL = 2 1/2 AC
- RESIDENTIAL = 5 AC
- OFFICE, COMMERCIAL
- INDUSTRIAL, GOVERNMENT
- SCHOOL & CHURCH
- PARK
- AIR FORCE ACADEMY
- PUBLIC OPEN SPACE

SHEET INDEX:

MAPPING:

LANDMARK MAPPING

ENGINEER:

URS CONSULTANTS
MAKING TECHNOLOGY WORK
1040 SOUTH EIGHTH STREET
COLORADO SPRINGS, CO. 80906
(719)634-6699

PROJECT:

COTTONWOOD CREEK
LAND USE PLAN
SCALE 1" = 1000'

SAND CREEK
DRAINAGE BASIN PLANNING STUDY

PRELIMINARY DESIGN REPORT

Prepared for:

City of Colorado Springs
Department of Comprehensive Planning, Development And Finance
Engineering Division - MAIL CODE 435
P.O. Box 1575
Colorado Springs, CO 80901-1575

Prepared by:

Kiowa Engineering Corporation
1001 North Weber #200
Colorado Springs, CO 80903

KIOWA Project No. 90.04.09
DXX/R185

JANUARY 1993
Revised APRIL 1993
Revised FEBRUARY 1995
Revised APRIL 1995
Revised OCTOBER 1995

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE BASIN PLANNING STUDY

REGIONAL SUB-BASINS

Project No.	90-04-09
Date:	11/90
Design:	
Drawn:	EAK
Check:	
Revisions:	

FIG. III-1

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE BASIN PLANNING STUDY

REACH DELINEATIONS

Project No.	90-04-09
Date:	1-91
Design:	RNW
Drawn:	EAK
Check:	
Revisions:	

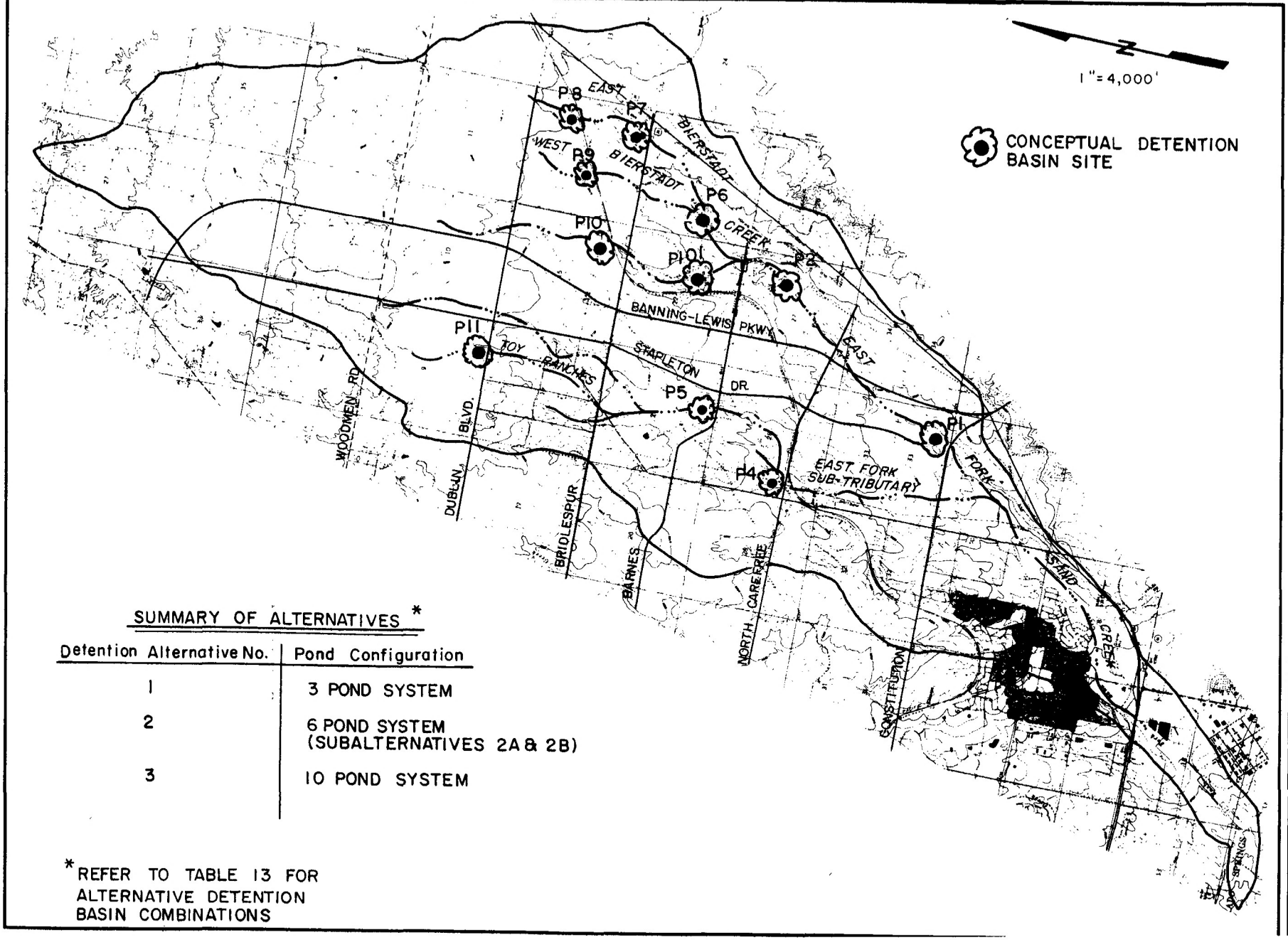
FIG. IV-1

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE
BASIN PLANNING STUDY
DETENTION BASIN ALTERNATIVES

Project No	90/04/07
Date:	9/91
Design:	RNW
Drawn:	EAK
Check:	
Revisions:	

FIG V-1



SUMMARY OF ALTERNATIVES *

Detention Alternative No.	Pond Configuration
1	3 POND SYSTEM
2	6 POND SYSTEM (SUBALTERNATIVES 2A & 2B)
3	10 POND SYSTEM

*REFER TO TABLE 13 FOR ALTERNATIVE DETENTION BASIN COMBINATIONS

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK
DRAINAGE BASIN PLANNING STUDY

DETENTION BASIN ALTERNATIVES

Project No.	88.11.23
Date:	4/89
Design:	RNW
Drawn:	EAK
Check:	
Revisions:	

FIG V-2

VI. DEVELOPMENT OF ALTERNATIVES AND RECOMMENDED PLAN

The concepts which are available for handling stormwater runoff within the Sand Creek basin have been presented and discussed in detail in the Sand Creek Drainage Basin Planning Study Development of Alternatives Report and the draft East Fork Sand Creek Drainage Basin Planning Study. The process of combining the various channel treatment options, detention schemes and roadway crossing structures into a contiguous plan for all of the reaches is presented in this chapter of the report. As a result of the evaluation of the flood control, environmental, open space, operations and maintenance, and implementation concerns within the Sand Creek basin, the following concepts were identified as having sufficient feasibility to warrant further evaluation and review:

Channel Concepts:	Floodplain Preservation Channelization, 10- or 100-year Selective Improvements
Detention:	Regional detention systems

Channel Concepts: The channel concepts listed above have been evaluated with respect to the parameters listed in the previous chapter. A concept's feasibility depends upon its impact, positive or negative, upon the evaluation parameters. ***The floodplain preservation*** concept has been considered to be the same as the ***"do-nothing"*** alternative. The floodplain preservation concept would involve the regulation of the floodplain limits, generally as depicted on the effective City of Colorado Springs and El Paso County Flood Insurance Rate Maps. Regulation of the floodplain so that future encroachments are minimized and the floodproofing of structures which are currently within the 100-year floodplain would presumably be the methods used to address the flood hazard concerns along Sand Creek. In the upper reaches of Sand Creek, the ownership or easements associated with the 100-year floodplain (or greater limits to allow for an erosion buffer zone) would be a primary issue in regards to implementation of such a concept. Detention in the upper reaches of the basin Sand Creek basin and in the East Fork Sand Creek basin will maintain the 100-year floodplain at existing limits within the lower reaches of Sand Creek. The "do-nothing" concept is feasible wherever

the existing drainageway improvements are of adequate capacity to convey flood flows. ***Channelization*** would involve the lining of the Creek into a more confined flow area, and could be done for either the ***100-year or 10-year*** flood discharges. Several typical channel concepts have been presented. The primary bank lining material would probably be riprap. Grade control and/or drop structures would be required in a channelization concept so that the flood velocities could be controlled to a level requiring medium to heavy riprap. Soil cement offers an alternative to riprap and concrete for the construction of drops or grade control structures. Revegetation would occur wherever the native vegetation was disturbed by the channel construction. Willows at the toe of the riprap banks would be a minimum replacement. ***Selective linings*** would involve the construction of grade controls, drop structures, bank linings, storm sewer outlet control structures selectively sited to resist stream erosion or to reduce potential flooding damages. Areas of future concern such as at the outside bends of the creek, or at the outlets of bridges or culverts which will cross the drainageway would be subject to selective improvements.

Detention Concepts: The two general detention concepts evaluated were onsite versus regional detention. During the evaluation process, it was determined that the onsite detention concept has a low feasibility relative to a regional concept. This is because, (1) onsite detention has a unpredictable impact upon lowering peak discharges from urbanized areas to historic conditions (reference, Urbonas and Glidden, "Effect of Detention on Flows in Major Drainageways" ASCE Water Forum '81, 1981), (2) an onsite concept has little impact upon maintaining or enhancing water quality, (3) the number of onsite detention basins, their locations and size cannot be accurately determined in the undeveloped portions of the basin at this time, and (4) onsite detention would present a substantial maintenance responsibility to the jurisdictions involved. For these reasons the onsite detention concept was eliminated and regional detention basin concepts were developed. In the analysis of the channel concepts, regional detention facilities were assumed to be in place.

Channel Alternatives

Presented on Table VI-1 is a matrix of channel alternatives which were evaluated. All reaches of Sand Creek and the East Fork of Sand Creek had at least three alternatives analyzed. Presented on Tables VI-2 through VI-6 are comparative evaluations of the floodplain preservation (do-nothing), channelization and selective lining concepts, for the mainstem Sand Creek basin, by reach. The purpose of the evaluation process was to identify the relative advantages and disadvantages of each concept within each reach.

RECEIVED
MAY 10 1996
Planning Dept.

SILVER PONDS SUBDIVISION FILING NO. 1

FINAL DRAINAGE REPORT

February 2, 1995
Revised May 5, 1996
Project No. 60572

PREPARED FOR:

The Campbell Corporation
4975 Austin Bluffs Parkway
Colorado Springs, CO 80918

PREPARED BY:

M.V.E., Inc.
1911 Lelaray St.
Colorado Springs, CO 80909

Table 3.1 - Developed Condition Hydrologic Data

5-year and 100-Year

Design Point	Included Basins	Cumulative Drainage Area (Ac)	5-yr Discharge (cfs)	100-yr Discharge (cfs)
1	OSA1	18.14	13.1	30.4
2	OSA2	8.72	7.0	16.3
3	OSA1 thru A3	29.05	20.5	47.7
4	OSA1 thru A4	31.04	24.3	53.6
5	OSB1	39.26	29.8	69.3
6	OSB1 thru B2	44.66	25.9	60.3
7	OSB1 thru B3	50.03	35.7	83.1
8	OSB1 thru B4	52.02	39.2	89.8
9	OSD1	8.26	7.9	18.4
10	OSD1 thru D2	19.95	24.1	52.9
11	D3	3.41	4.5	9.9
12	E1	4.24	5.5	12.1
13	F1	4.26	6.6	14.4
14	OSG1	6.66	7.0	16.4
15	OSG1 thru G2	9.22	10.5	24.0
16	OSH1	17.22	17.5	38.4
17	OSH1 thru H2	28.28	27.9	61.3
18	OSI1	3.67	3.3	7.8
19	OSI1 thru I2	11.05	7.9	18.4
20	I3	8.01	6.3	14.6
21	OSI1 thru I4	27.16	19.0	44.2
22	J1	4.19	3.0	6.9

M.V.E., Inc.
Colorado Springs, Colorado

Proj. No.: 60572 Project: SILVER PONDS Date: 1-31-96

DEVELOPED DISCHARGES
RAINFALL/RUNOFF ANALYSIS - RATIONAL METHOD

Design Point	Area (Ac)	C5	C100	Tc (min)	i5 (in/hr)	i100 (in/hr)	Q5 (cfs)	Q100 (cfs)
1	18.14	0.30	0.40	27.7	2.40	4.20	13.1	30.4
2	8.72	0.30	0.40	22.9	2.68	4.68	7.0	16.3
5	39.26	0.30	0.40	25.4	2.53	4.41	29.8	69.3
9	8.26	0.30	0.40	16.5	3.19	5.57	7.9	18.4
14	6.66	0.30	0.40	13.5	3.52	6.15	7.0	16.4
16	17.22	0.39	0.49	24.1	2.60	4.55	17.5	38.4
18	3.67	0.30	0.40	18.2	3.03	5.30	3.3	7.8
3	29.05	0.30	0.40	28.7	2.35	4.11	20.5	47.7
4	31.04	0.34	0.43	29.8	2.30	4.02	24.3	53.6
6	44.66	0.30	0.40	39.5	1.93	3.38	25.9	60.3
B3	5.37	0.30	0.40	26.7	2.45	4.29	4.0	9.2
7	50.03	0.30	0.40	28.2	2.38	4.15	35.7	83.1
8	52.02	0.32	0.42	28.7	2.35	4.11	39.2	89.8
D2	11.69	0.39	0.49	17.6	3.09	5.39	14.1	30.9
10	19.95	0.39	0.49	17.5	3.10	5.41	24.1	52.9
11	3.41	0.39	0.49	14.5	3.40	5.94	4.5	9.9
12	4.24	0.39	0.49	15.0	3.35	5.85	5.5	12.1
13	4.26	0.39	0.49	10.5	3.94	6.89	6.6	14.4
G2	2.56	0.39	0.49	13.1	3.57	6.24	3.6	7.8
15	9.22	0.33	0.43	14.0	3.46	6.04	10.5	24.0
H2	11.06	0.39	0.49	17.5	3.10	5.41	13.4	29.3
17	28.28	0.39	0.49	25.3	2.53	4.42	27.9	61.3
19	11.05	0.30	0.40	28.0	2.39	4.17	7.9	18.4
20	8.01	0.30	0.40	23.9	2.62	4.57	6.3	14.6
I4	8.10	0.30	0.40	23.1	2.67	4.66	6.5	15.1
21	27.16	0.30	0.40	29.2	2.33	4.07	19.0	44.2
22	4.19	0.30	0.40	28.8	2.35	4.10	3.0	6.9

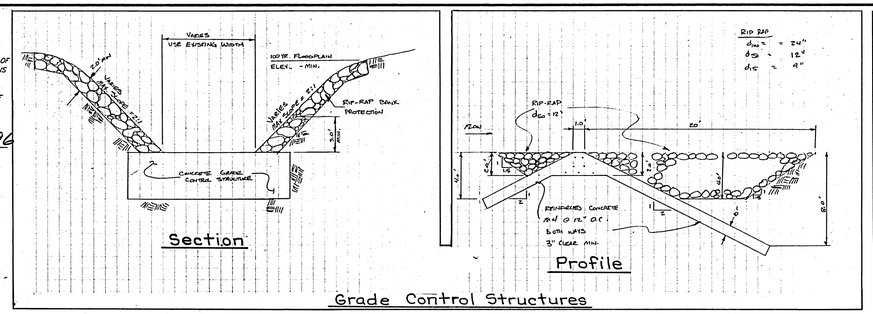
LEGEND

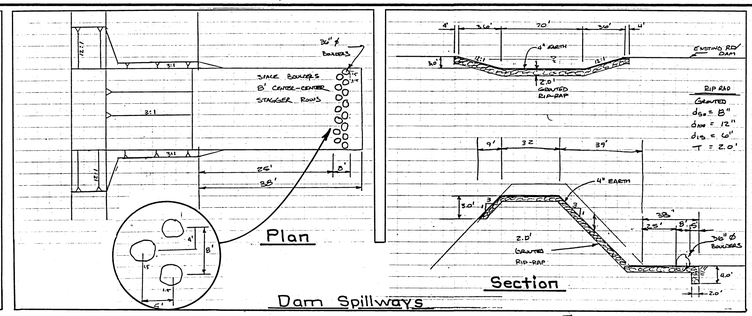
- EXISTING INDEX CONTOUR (7430)
- EXISTING INTERMEDIATE CONTOUR
- DRAINAGE BASIN BOUNDARY LINE
- SUBDIVISION BOUNDARY LINE
- SILT FENCE
- DRAINAGE DIRECTION ARROW
- (5) DESIGN POINT DESIGNATION
- PROPOSED STORM DRAIN CULVERT
- (E 0.40) DRAINAGE BASIN DESIGNATION / DRAINAGE BASIN AREA

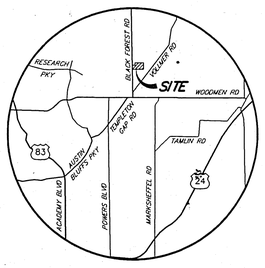
THESE DETAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECTION AND SUPERVISION. SAID DETAILED PLANS AND SPECIFICATIONS HAVE BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR DETAILED PLANS AND SPECIFICATIONS, AND SAID DETAILED PLANS AND SPECIFICATIONS ARE IN CONFORMITY WITH THE MASTER PLAN OF THE DRAINAGE BASIN. SAID DETAILED DRAINAGE PLANS AND SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR DRAINAGE FACILITY IS DESIGNED. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARATION OF THE DETAILED DRAINAGE PLANS AND SPECIFICATIONS.

REGISTERED 13348 PROFESSIONAL ENGINEER STATE OF COLORADO

CHARLES C. CROW, P.E. COLORADO NO. 13348 — DATE 5-6-96







VICINITY MAP

SCALE

100' 50' 0 50' 100' 200'

75' 25'

1"=100' 1:1200

SITE HYDROLOGY DATA

DESIGN POINT	INCL. BASINS	AREA (Ac)	Q_5 (cfs)	Q_{100} (cfs)
1	OSA1	18.14	13.1	30.4
2	OSA2	8.72	7.0	16.3
3	OSA1–A3	29.05	20.5	47.7
4	OSA1–A4	31.04	24.3	53.6
5	OSB1	39.26	29.8	69.3
6	OSB1–B2	44.66	25.9	60.3
7	OSB1–B3	50.03	35.7	83.1
8	OSB1–B4	52.02	39.2	89.8
9	OSD1	8.26	7.9	18.4
10	OSD1–D2	19.95	24.1	52.9
11	D3	3.41	4.5	9.9
12	E1	4.24	5.5	12.1
13	F1	4.26	6.6	14.4
14	OSG1	6.66	7.0	16.4
15	OSG1–G2	9.22	10.5	24.0
16	OSH1	17.22	17.5	38.4
17	OSH1–H2	28.28	27.9	61.3
18	OSI1	3.67	3.3	7.8
19	OSI1–I2	11.05	7.9	18.4
20	I3	8.01	6.3	14.6
21	OSI1–I4	27.16	19.0	44.2
22	J1	4.19	3.0	6.9

NOTE: SEE OFFSITE DRAINAGE BASIN MAP FOR OFFSITE BASINS (OS__)

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

Murray D. Stewart Jr. 5/9/96

MURRAY D. STEWART, JR., P.E. COUNTY ENGINEER

PROJECT: SILVER PONDS SUBDIVISION FILING NO. 1

TITLE: DRAINAGE IMPROVEMENT DETAILS

MONUMENT VALLEY ENGINEERS INC.
ENGINEERS • SURVEYORS
1911 LELARAY STREET
COLORADO SPRINGS, COLORADO 80909
PHONE (719) 635-5736

PROJ. NO.	60572
DRAWN:	DRG
ENGINEER:	DRG
CHECKED:	CCC
SCALE:	1" = 100'
DATE:	3/18/96

REVISIONS:

NO.	DATE	ITEM
1	5/7/96	ADD ESMTS, SIL FENCE & DETAIL

SHEET 1 OF 1

DRAWING NO. 60572008

DISK NO. 1516

PLANNING
KEVIN J. WALKER & ASSOCIATES
105 EAST VERMIJO AVE., SUITE 600 COLORADO SPRINGS, CO 80903 (719) 475-8345

SILVER PONDS FIL. 1

TRAILS AT FOREST MEADOWS
FILING NO. 3
FINAL DRAINAGE REPORT

AMENDMENT TO:
MASTER DEVELOPMENT DRAINAGE PLAN UPDATE FOR WOODMEN HEIGHTS AND FINAL DRAINAGE REPORT FOR FOREST MEADOWS FILING NO.1AND NO.4

August 2015

Prepared for:

Rivers Development, Inc.
13530 Northgate Estates Drive, Suite 200
Colorado Springs, CO 80921

Prepared by:

CIVIL CONSULTANTS, INC.
20 Boulder Crescent, Suite 110
Colorado Springs, CO 80903
(719) 955-5485

Project #08-029

OFFICE COPY

ranges from 12 to 14 inches per year, with the majority of this moisture occurring in the spring and summer in the form of rainfall. Thunderstorms are co=on during the summer months.

FLOODPLAIN STATEMENT

According to the Federal Emergency Management Agency (FEMA) Flood lnsurance Rate Map (FIRM) Panel No. 08041C0529, effective date March 17, 1997, no portion of the site lies within a designated floodplain.

DRAINAGE CRITERIA

The drainage analysis has been prepared in accordance with the current City of Colorado Springs/E) Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities during the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres.

EXISTING DRAINAGE CONDITIONS

The overall site consists of 18.172 acres and is currently undeveloped, with the initial grading project proposed to disturb 18.172 acres for Filing No. 3 residential subdivision. Filing No.3 proposes 87 single family residential lots. The number of lots to be platted in the future filings to the west is unknown at this time. All of the streets, curb, gutter, sidewalk and utilities for the development of Vanderwood Road, Cedar Brush Court, Brush Top Road, and Creek Brush Drive, will be constructed in Filing No. 3 Vanderwood Road will be extended in the future filings of Trails at Forest Meadows.

As shown by the historic drainage map for "Trails at Forest Meadows Filing 2", included in the appendix, offsite historic flows, tributary to Vollmer (EX1 ~ Q5=66 cfs and Q100=300.7 cfs, EX2 ~ Q5=23 cfs and Q100=106 cfs, and EX3 ~ Q5=1.8 cfs and Q100=8.0 cfs, see Historic drainage map DP-1), have been estimated to reach proposed Vollmer Road infrastructure at the Dry Needle Place/Vollmer Road intersection and northerly boundary of Filing No. 2. It has been determined that due to limited roadside capacity along the West side of Vollmer Roadway much of this historic runoff may cross Vollmer prior to reaching DP3 (see proposed Drainage Conditions for addition discussion). No new developed flow will be introduced onto Vollmer Road, north of the intersection. Any new developed flows, south of the intersection, will be intercepted by 2-20' D-10-R inlets within the proposed Vollmer Road Section. It should be noted that upon the construction of the Sterling Ranch subdivision a large percentage of the historic flows, once reaching the subject site (north and west of Vollmer Road/ future Marksheffel Road) will be collected and conveyed in a proposed 54" RCP to Sand Creek.

Per review of the existing topographic drainage patterns, the off-site historic flows, north of the property (OSl, see Historic drainage map DP-1), are conveyed west by an existing berm and swale to an existing gravel borrow pit, as evident by the eroded embankment at the southeast comer of the gravel borrow pit and evaluation of grade during onsite visits. Per the "Master Development Drainage Plan for Woodmen Heights Master ·Plan" prepared by Classic Consulting Engineers and Surveyors, dated June 2004, it was inferred that the entire watershed (historic flows of Q5=65 cfs and Q100=162 cfs) would be transported to the future detention facility through our site. This report was amended by the "Master Development Drainage Plan, Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No.1 and No. 4" (MDDP) by Engineering and Surveying, Inc., dated February 2006, in which it states that 50% of the flow from OS1 (Q5=34 cfs, Q100==84 cfs) will be collected by a proposed concrete pipe and will be conveyed through and combined with the flows of Trails at Forest Meadows Filings to Sand Creek Regional Detention Facility No. 6. Per historic drainage patterns and per the "Preliminary/Final Drainage Report for Highland Park Filing No.2", prepared by Law and Mariotti Consultants, dated June 2002, the remainder of the flows will be transported via road side swale on the west side of Vollmer Road to the proposed drainage infrastructure in Vollmer Road. Any increase in flows due to future development of

Basin OS1 will require the construction of a proposed detention facility, as per the Sand Creek DBPS. Historic flows (EX4- Q5=5.1 cfs and Q100=22.9 cfs, see Historic drainage map DP-1) south of OS1, sheet flow to an existing abandoned road swale and outlet to a roadside swale on Black Forest Road. There are road side swales on either side of the abandoned road. Since topographic drainage patterns have been altered since the approval of the MDDP, existing historic flows have been diverted to the gravel borrow pit, Vollmer Road and Black Forest Road.

The site is tributary to Sand Creek Detention Basin No. 6. The stormwater infrastructure downstream of the subject site has already been constructed to the aforementioned detention pond. Specifically, flows initially intercepted at Design Point E3 are conveyed within the Vollmer Road Storm Sewer System which parallels the west side of Vollmer prior to out-falling (at the East Side of Vollmer Road) into a partially riprap lined channel. The existing channel, which parallels both the southwest and western edge of Forest Meadows Filing No. 1, directs runoff under Forest Meadows Avenue and into the existing Sand Creek Detention Pond No. 6. A rip rap plunge pool and energy dissipater aid to slow the incoming flows while the receiving facility provides both WQCV and detention for this development.

The site was originally studied in the "Sand Creek Drainage Basin Planning Study Preliminary Design Report" (DBPS) prepared by Kiowa Engineering. This study was then updated in the "Sand Creek Drainage Basin Planning Study Preliminary Design Report Technical Addendum" by Kiowa, revised October 1995.

It should be noted that the subject site was most currently studied in the "Master Development Drainage Plan Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No. 1 and No. 4, where the proposed land use had been revised to single-family residential development from previously planned commercial usage.

PROPOSED DRAINAGE CONDITIONS

The following is a description of the onsite basins, offsite bypass flows and the overall proposed drainage characteristics for the development of Trails at Forest Meadows Filing No.3. The following Design Points and Basins were determined using the Rational Method since each individual basin is less than 100 acres and the combined acreage at any Design Point is also less than 100 acres. This method offers a more conservative approach to calculating swale cross sections and storm drain. The proposed drainage design for this subdivision is typical for single family residential, consisting of; homes, landscaping, rear and side lot drainage swales, curb & gutter, streets, curb inlets, and pipes to convey developed flows downstream. Rear and side lot swales will be constructed to get developed flows to street curb and gutter.

Filing No.2 - Basins F2-F, F2-I and F2-J are located to the south of the subject site, in the central and east portions of the Filing 2 site. The flows from these basins were previously analyzed as part of "The Trails at Forest Meadows Filing No. 2 Preliminary/Final Drainage Report" (FDR2). Runoff produced within Basins F2-F, F2-I and F2-J flow to existing inlets in the Trails at Forest Meadows Filing No. 2 subdivision and have been accounted for.

Basin K is located in the northerly portion of the site and contains 0.92 acres of portions of single family residential lots and streets. Basin K has proposed design flows of 1.8 cfs for the minor storm event (5-Year) and 3.9 cfs for the major storm event (100-Year). Runoff from Basin K will flow, overland via side lot swales, to the curb and gutter of existing streets within Basin F2-F (Filing No. 2) to Design Point 1 (6.1 cfs/12.8 cfs), an existing 12' D-10-R inlet in a sump condition. See FDR2 Basin F for pipe conveyance information and over flow conditions. Pipe 2 and 3 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2.

Basin L is located in the northerly portion of the site and contains 0.55 acres of rear yards of single family residential lots. Basin L has proposed design flows of 1.2 cfs for the minor storm event (5-Year) and 2.5 cfs for the major storm event (100-Year). Runoff from Basin L will flow, overland, to lots and tracts within Basin F2-J (Filing No. 2) and be conveyed, via side lot swales, to the curb and gutter of the adjacent streets to Design Point 5 (4.9 cfs/7.4 cfs). Runoff will be intercept at DP-5 via an existing at-grade 12' D-10-R inlet. See FDR2 Basin J for pipe conveyance information and over flow conditions. Pipes 5, 7, 8, 9 and 11- have been sized to accept these developed flows and do not exceed the pipe design flows determined within the FDR for Filing 2.

Basin M is located in the northeast portion of the site and contains 2.62 acres of single family residential lots, landscaping, and an asphalt roadway. Basin M has proposed design flows of 5.4 cfs for the minor storm event (5-Year) and 11.3 cfs for the major storm event (100-Year). Runoff from Basin M will flow, via side lot swales, to the curb and gutter and will be conveyed south to Design Point 10, a 10' D-10-R inlet in an at-grade condition. The inlet at Design Point 10 has been sized to accept flows from Basin M in the developed condition. Collected flows from Design Point 10 will be conveyed in an 18" RCP (Pipe 20) to Design Point II, a 12' D-10-R at-grade inlet.

Basin M-1 is located in the northeast portion of the site and contains 0.65 acres of single family residential lots and asphalt roadway. Basin M-1 has proposed design flows of 1.3 cfs for the minor storm event (5- Year) and 2.7 cfs for the major storm event (100-Year). Basin M-1 was part of a Basin Min FDR2 and is identified only to account for design flows to Design Point 6, an existing 8' D-10-R inlet in a sump condition. See FDR2 Basin M for pipe conveyance information and over flow conditions. Pipes 8, 9 and 11 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2.

Basin O is located in the northeast portion of the site and contains 0.70 acres of rear yards of single family residential lots and Tract G. Basin 0 has proposed design flows of 1.6 cfs for the minor storm event (5-Year) and 3.3 cfs for the major storm event (100-Year). The runoff from Basin 0 will flow overland thru Tract G via the side lot swales, to the curb and gutter of Aspen Brush Court, w/o crossing any lots within Filing 2. Runoff collected by the roadway continues southerly (down-gradient) to Design Point 5, an existing 12' D-10-R inlet in an at-grade condition. See FDR2 Basin J for pipe conveyance information and over flow conditions. Pipe 5, 7, 8, 9 and 11 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2.

Basin P is located in the northerly portion of the site and contains 2.14 acres of single family residential lots, landscaping, and an asphalt roadway. Basin P has proposed design flows of 4.2 cfs for the minor storm event (5-Year) and 8.8 cfs for the major storm event (100-Year). Runoff from Basin P will flow, via side lot swales, to the curb and gutter and then westward to Design Point 9 (accumulated flows 10.2 cfs-5 year, 22.4 cfs-100 year). A temporary riprap pad will be constructed at the end of the proposed roadway to dissipate energy and prevent local scour. The existing diversion swale established with FDR2 (which previously directed runoff reaching this location to the south) will be abandoned by the construction of a new swale which will be to redirect runoff westward to the existing 48" RCP (Pipe 14). It should be noted that the majority of the existing swale along the western boundary of Filing No.2 remain in place to protect existing lots and serve as a secondary level of protection should the swale leading to pipe 14 fail. Pipes 14 and 3 have been sized to accept the contributing developed flows and do not exceed the pipe design flows in FDR2.

Basin Q is located in the northerly portion of the site and contains 3.66 acres of single family residential lots, landscaping, and proposed roadways. Basin Q has proposed design flows of 6.1 cfs for the minor storm event (5-Year) and 12.8 cfs for the major storm event (100-Year). Runoff from Basin Q will flow, via side lot swales, to the curb and gutter to Design Point 9 (accumulated flows 10.2 cfs-5 year, 22.4 cfs-100 year), before continuing within the aforementioned swale to Pipe 14. A temporary sediment basin is recommended to be constructed upstream of Pipe 14, to aid in deterring sediment transport, while riprap protection will be placed immediately adjacent to the pipe entrance to protect the transition slope to the mouth of the pipe . A perimeter berm will be constructed (within a permanent drainage easement) along the north of the filing within Basin Q to ensure that on site property owners are protected from offsite flows (OS1) should lack of maintenance or

removal of the existing offsite berm occur. The combination of the existing grade north of the proposed berm and the berm itself aids to direct any potential runoff to the west. A proposed 3' wide sidewalk chase will be constructed to convey the minor runoff from Basin OS-3 and east third of Basin OS-2 into Brush Top Road. Limited flow from the north is anticipated reach Brush Top, with future development as a high point will be planned to be placed near the north boundary line.

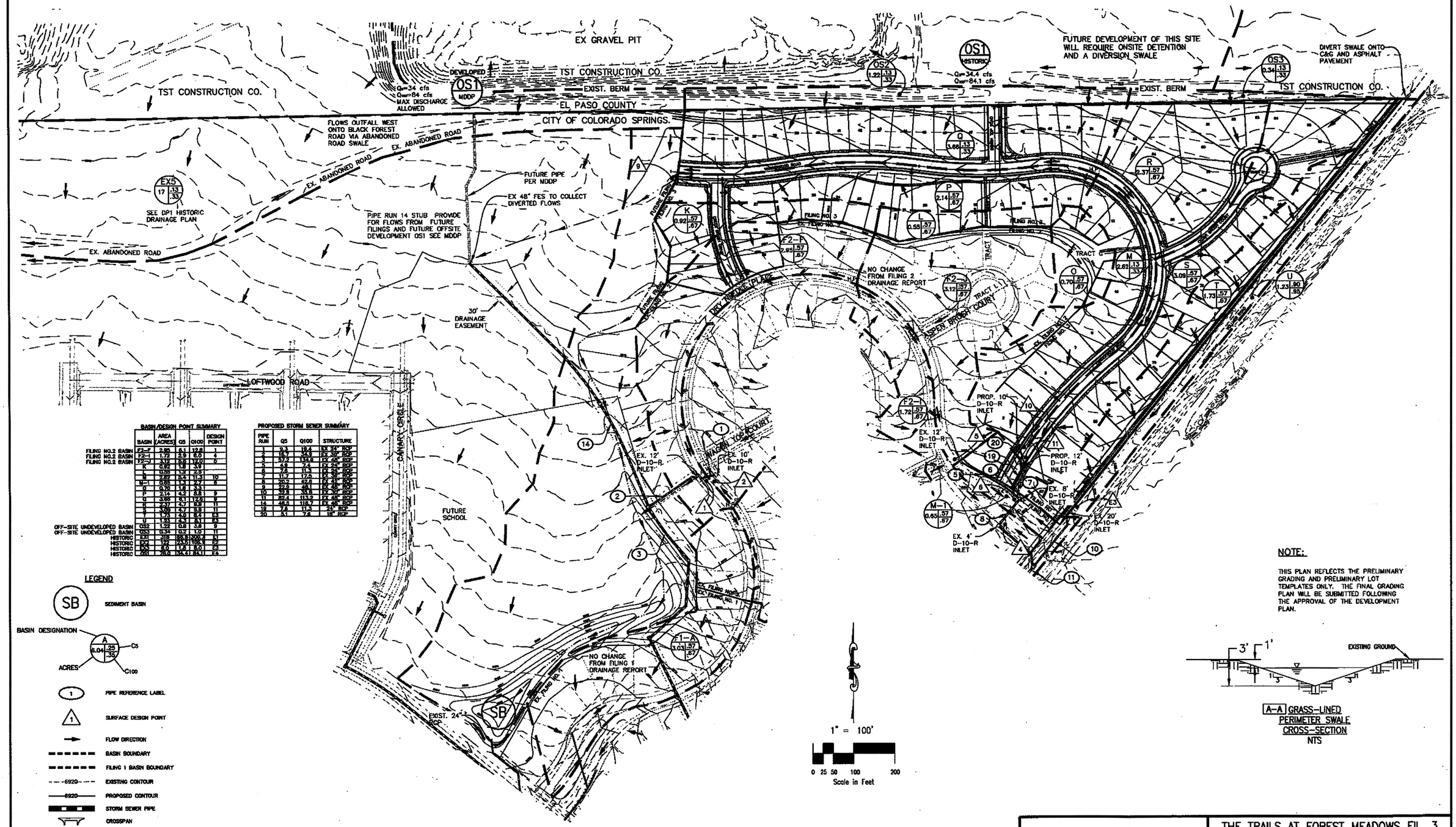
It should be noted that the contours on the provided drainage map illustrate the potential for a low point or break in the existing offsite berm immediately to the north of Brush Top Road which would allow for flows from Basin OS-1 to reach Basin OS-2. M&S Civil Consultants traveled to the sight and evaluated the onsite conditions at this location and determined that the existing contours shown on the provided map are not entirely representative of the current conditions. M&S Civil further ascertained the existing swale/berm at this location maintains a positive grade to the west and is somewhere between 1.7 to 2.0 feet tall which should provide adequate temporary capacity to redirect the offsite flows to the west.

Refer to the existing condition section of this report, the "Master Development Drainage Plan, Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No.1 and No. 4", and Preliminary/Final Drainage Report for Highland Park Filing No.2" for additional information regarding offsite development in this area.

Basin R is located in the northerly portion of the site and contains 2.37 acres of single family residential lots, landscaping, and an asphalt roadway. Basin R has proposed design flows of 4.7 cfs for the minor storm event (5-Year) and 9.9 cfs for the major storm event (100-Year). Runoff from Basin R will flow, via side lot swales, to the curb and gutter to Design Point 11, a 12' D-10-R inlet in an at-grade condition. The inlet at Design Point 11 has been sized to accept flows from Basin R, S and OS3 in the developed condition. Collected flows from Design Point 11 will be conveyed in a 24" RCP (pipe 19) to pipe 6, an existing 24" RCP stub. Pipe 6, 7, 8, 9 and 11 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2. A perimeter berm will be constructed (within a permanent drainage easement) along the north of the filing within Basin R to ensure that on site property owners are protected from offsite flows (OS1) should lack of maintenance or removal of the existing offsite berm occur. The combination of the existing grade north of the proposed berm and the berm itself aids in directing any potential runoff to west.

Basin S is located in the easterly portion of the site and contains 3.09 acres of single family residential lots, landscaping, and an asphalt roadway. Basin S has proposed design flows of 4.7 cfs for the minor storm event (5-Year) and 9.9 cfs for the major storm event (100-Year). Runoff from Basins will flow, via side lot swales, to the curb and gutter to Design Point 11, a 12' D-10-R inlet in an at-grade condition. The inlet at Design Point 11 has been sized to accept flows from Basin R, S and OS3 in the developed condition. Collected flows from Design Point 11 will be conveyed in a 24" RCP (pipe 19) to pipe 6, an existing 24" RCP stub. Pipe 6, 7, 8, 9 and 11 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2. A perimeter berm will be constructed (within a permanent drainage easement) along the north and northeast edges of the proposed filing within Basin S to ensure that on site property owners are protected from offsite flows (OS3). The existing offsite berm located on the adjacent property to the currently aids in directing flows around the majority of the northeast edge of the filing , but terminates prior to eastern boundary line and cannot be relied on in perpetuity for flood control. Flows reaching the onsite berm will be directed to a pair of proposed 4' sidewalk chases which will aid to discharge runoff to the curb and gutter of Vollmer Road.

Basin T is located in the easterly portion of the site and contains 1.73 acres of rear yards of single family residential lots. Basin T has proposed design flows of 4.0 cfs for the minor storm event (5-Year) and 8.4 cfs for the major storm event (100-Year). Runoff from Basin Twill flow, via side lot swales, to the curb and gutter in Vollmer Road to Design Point E3, an existing 12' D-10-R inlet, in an at-grade condition. The inlet at Design Point E3 has been sized to accept flows from Basin T, U and historic flows EX1 and EX2. Collected flows from Design Point E3 will be conveyed in an existing 30" RCP (pipe 10) to pipe 11, an existing 48" RCP. Combined flows in 9, 10 and 11 have been sized to accept these developed flows and do not exceed the pipe design flows in FDR2.



BASIN/DESIGN POINT SUMMARY

BASIN	AREA (ACRES)	C5	C100	DESIGN POINT
FILING NO.2 BASIN F2-P	2.95	.57	.67	1
FILING NO.2 BASIN F2-I	1.72	.57	.67	4
FILING NO.2 BASIN F2-O	3.12	.57	.67	5
K	0.92	.57	.67	
L	0.55	.57	.67	
P	2.89	.57	.67	10
M-1	0.65	.13	.33	8
Q	3.66	.13	.33	
O	0.70	.57	.67	9
T	2.14	.57	.67	9
U	3.95	.57	.67	9
R	2.37	.57	.67	11
S	3.09	.57	.67	11
M	2.62	.13	.33	11
F1-A	3.03	.57	.67	E3
F1-B	1.93	.57	.67	E3
OFF-SITE UNDEVELOPED BASIN OS2	1.22	.13	.33	9
OFF-SITE UNDEVELOPED BASIN OS3	0.34	.13	.33	11
HISTORIC EX1	31.8	SEE BASIN MAP		E1
HISTORIC EX2	1.72	EX.5	11.06.1	E2
HISTORIC OS2	1.22	8.0	17.8	E3
HISTORIC OS1	28.0	34.4	84.1	E4

PROPOSED STORM SEWER SUMMARY

PIPE RUN	Q5	Q100	STRUCTURE
1	9.3	19.4	EX 24" RCP
2	18.7	34.9	EX 30" RCP
3	37.2	134.8	EX 48" RCP
5	5.2	7.4	EX 24" RCP
6	7.8	11.3	EX 30" RCP
7	11.7	17.0	EX 36" RCP
8	20.2	42.8	EX 42" RCP
9	22.9	68.1	EX 48" RCP
10	43.1	98.8	EX 42" RCP
11	82.4	113.2	EX 48" RCP
14	48.8	116.7	EX 48" RCP
18	7.6	11.3	24" RCP
20	5.1	7.8	18" RCP

LEGEND

- SB — SEDIMENT BASIN
- BASIN DESIGNATION — A — 6.04 ACRES — .25 C5 — .35 C100
- 1 — PIPE REFERENCE LABEL
- 1 — SURFACE DESIGN POINT
- FLOW DIRECTION
- BASIN BOUNDARY
- FILING 1 BASIN BOUNDARY
- 5920 — EXISTING CONTOUR
- 5920 — PROPOSED CONTOUR
- STORM SEWER PIPE
- CROSSPAN
- PERIMETER SWALE
- FILING LINE
- H.P. — HIGH POINT
- L.P. — LOW POINT
- INLET

NOTE:

THIS PLAN REFLECTS THE PRELIMINARY GRADING AND PRELIMINARY LOT TEMPLATES ONLY. THE FINAL GRADING PLAN WILL BE SUBMITTED FOLLOWING THE APPROVAL OF THE DEVELOPMENT PLAN.

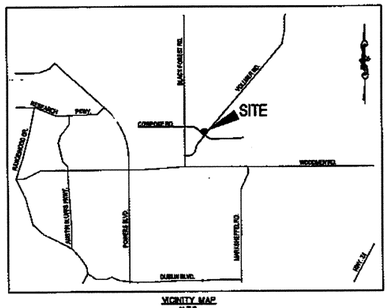
A-A GRASS-LINED PERIMETER SWALE CROSS-SECTION
NTS

CIVIL CONSULTANTS, INC.

THE TRAILS AT FOREST MEADOWS FIL. 3

FILING 3 DRAINAGE PLAN

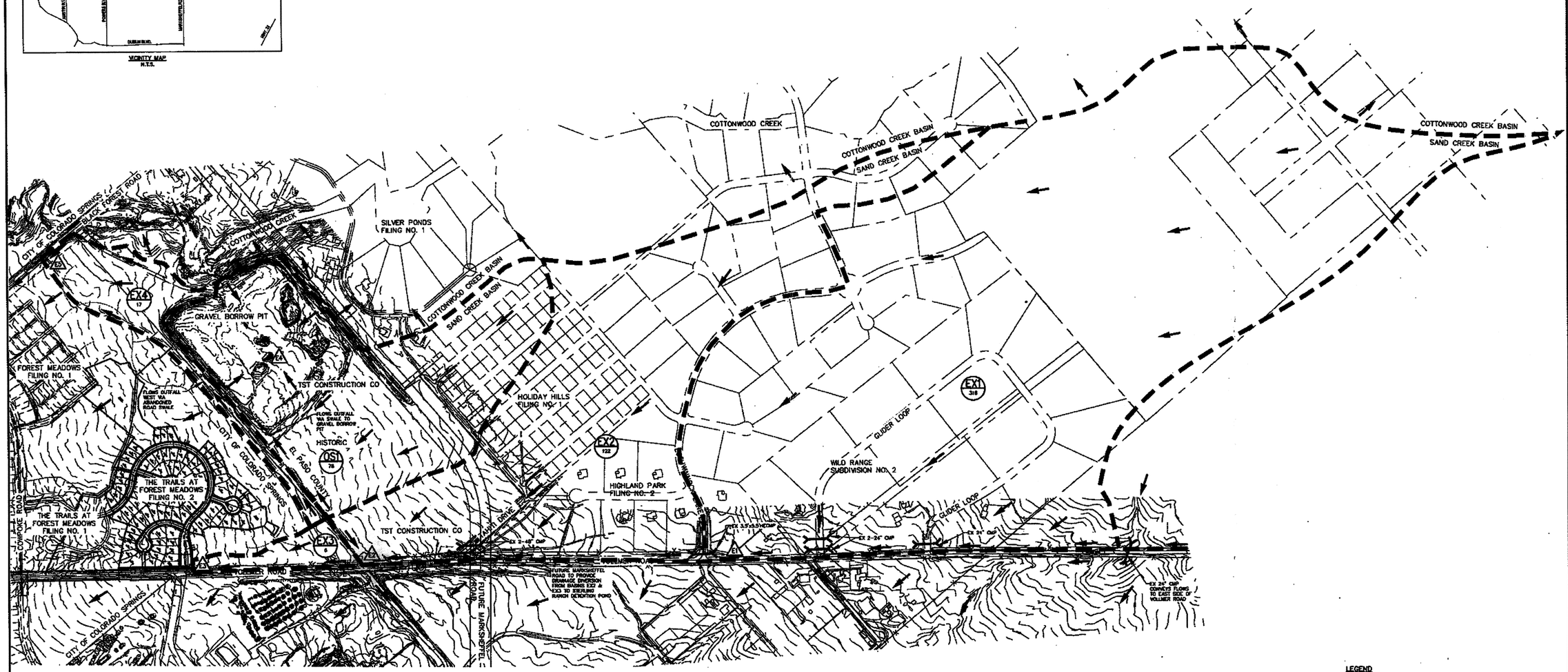
PROJECT NO. 08-025	FILE: *\dwg\Dev. Plan\ 08025 - DRAINAGE		
DESIGNED BY: VAS	SCALE	DATE: 04/09/2014	
DRAWN BY: BLB	HORIZ: 1"=100'	SHEET 2 OF 2	DP2
CHECKED BY: VAS	VERT: N/A		



VICINITY MAP
N.T.S.

HISTORIC BASIN SUMMARY			
BASIN	AREA (acres)	Q_5 (cfs)	Q_{100} (cfs)
EX1	318	66.6	300.7
EX2	122	23.5	106.1
EX3	6	1.8	8.0
OS1	78	34.4	84.1
EX5	17	5.1	22.9

HISTORIC SURFACE ROUTING SUMMARY			
DESIGN POINT	CONTRIBUTING BASINS/DESIGN POINTS	Q_5 (cfs)	Q_{100} (cfs)
E1	EX1	66.6	300.7
E2	EX2 & DP E1	84.7	382.6
E3	EX3 & DP E2	85.8	387.8
E4	OS1	34.4	84.1
E5	EX4	5.1	22.9



1" = 300

0 75 150 300 600

Scale in Feet

LEGEND

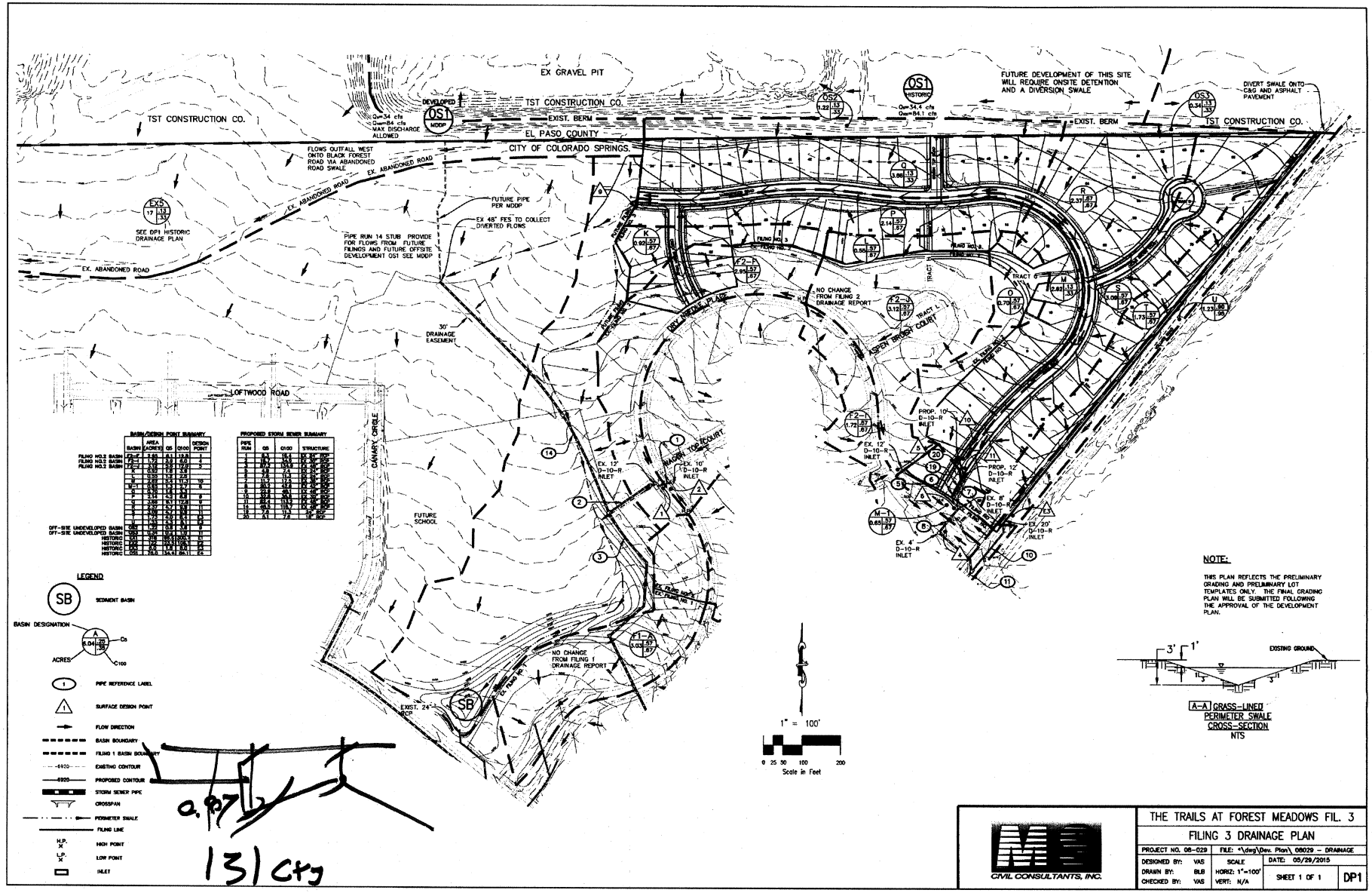
- EX1 / 33.2 — EXISTING MDDP BASIN / ACREAGE
- E1 — EXISTING FLOW RELEASE POINT
- FLOW DIRECTION
- BASIN BOUNDARY
- 6920 — EXISTING CONTOUR
- CULVERT PIPE

CIVIL CONSULTANTS, INC.
E. PIKES PEAK AVE., STE 300
COLORADO SPRINGS, CO 80901
(719) 235-5249, FAX (719) 444-8427

THE TRAILS AT FOREST MEADOWS FIL. NO. 2

HISTORIC – DRAINAGE MAP

PROJECT NO. 08-025	FILE: *\dwg\Dev Plan\08-025-HISTORIC		
DESIGNED BY: ET	SCALE	DATE: 05/01/14	
DRAWN BY: ET	HORIZ: 1"=300'	SHEET 1 OF 2	DP1
CHECKED BY:	VERT: N/A		



BASIN/DESIGN POINT SUMMARY

	BASIN	AREA (ACRES)	Q5	Q100	DESIGN POINT
FILING NO.2 BASIN	F2-1	2.88	4.1	13.8	1
FILING NO.2 BASIN	F2-2	1.72	3.0	8.8	2
FILING NO.2 BASIN	F2-3	3.12	3.9	12.0	3
	A	6.04	1.3	3.8	4
	B	1.00	1.1	2.5	
	C	3.86	1.3	11.0	15
	M-1	0.65	1.1	1.7	8
	E	0.92	1.1	2.7	
	F	2.14	1.3	6.8	9
	G	1.00	1.1	11.20	9
	H	2.37	1.1	6.8	9
	I	2.06	1.1	6.1	11
	J	2.82	1.3	8.1	12
	K	0.92	1.1	2.7	13
	L	3.06	1.1	9.1	14
	M	2.82	1.3	8.1	16
	N	0.70	1.1	2.1	17
	O	3.08	1.1	9.2	18
	P	2.14	1.1	6.4	19
	Q	1.73	1.1	5.1	20
	R	2.37	1.1	7.0	21
	S	3.08	1.1	9.2	22
	T	1.73	1.1	5.1	23
	U	1.23	1.1	3.6	24
OFF-SITE UNDEVELOPED BASIN	OS1	1.22	0.5	1.8	5
OFF-SITE UNDEVELOPED BASIN	OS2	1.22	0.5	1.8	10
HISTORIC	OS1	34.4	MEASURED		
HISTORIC	OS2	1.22	0.5	1.8	25
HISTORIC	OS3	0.34	0.1	0.5	26
HISTORIC	OS4	33.0	(A-1-a) SW.		27

PROPOSED STORM SEWER SUMMARY

PIPE RUN	Q5	Q100	STRUCTURE
1	4.1	16.1	24" RCP
2	8.2	34.5	24" RCP
3	12.0	53.5	24" RCP
4	3.8	7.4	24" RCP
5	7.5	17.1	24" RCP
6	11.3	17.5	24" RCP
7	20.1	42.9	24" RCP
8	23.8	60.1	24" RCP
9	27.8	68.1	24" RCP
10	32.8	83.5	24" RCP
11	36.6	119.0	24" RCP
14	49.5	118.7	24" RCP
18	7.8	17.5	24" RCP
20	4.1	7.8	18" RCP

LEGEND

- SB — SEDIMENT BASIN
- BASIN DESIGNATION — A, 6.04 ac, Q5, Q100
- PIPE REFERENCE LABEL
- SURFACE DESIGN POINT
- FLOW DIRECTION
- BASIN BOUNDARY
- FILING 1 BASIN BOUNDARY
- 6920 — EXISTING CONTOUR
- 6920 — PROPOSED CONTOUR
- STORM SEWER PIPE
- CROSSPAN
- PERIMETER SWALE
- FILING LINE
- H.P. — HIGH POINT
- L.P. — LOW POINT
- INLET

CIVIL CONSULTANTS, INC.

THE TRAILS AT FOREST MEADOWS FIL. 3

FILING 3 DRAINAGE PLAN

PROJECT NO. 08-029	FILE: *\dwg\Dev. Plan\ 08029 - DRAINAGE		
DESIGNED BY: VAS	SCALE	DATE: 05/29/2015	
DRAWN BY: BLB	HORIZ: 1"=100'	SHEET 1 OF 1	DP1
CHECKED BY: VAS	VERT: N/A		

TRAILS AT FOREST MEADOWS FILING NO. 3

CITY OF COLORADO SPRINGS, EL PASO COUNTY, STATE OF COLORADO

STREET IMPROVEMENT PLANS

(INCLUDING STORM SEWER)

AUGUST 2015

GENERAL NOTES

1. ALL MATERIALS AND INSTALLATION PROCEDURES SHALL BE IN COMPLIANCE WITH THE CITY OF COLORADO SPRINGS, DEPARTMENT OF PUBLIC WORKS, SUBDIVISION POLICY MANUAL AND DIVISION "RULES FOR THE INSTALLATION OF SEWER MAINS AND SERVICES".
2. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE AND LOCATION OF ALL UNDERGROUND UTILITIES ALONG THE ROUTE OF THE WORK. THE OMISSION FROM OR THE INCLUSION OF UTILITY LOCATIONS ON THE PLANS IS NOT TO BE CONSIDERED AS THE NONEXISTENCE OF OR A DEFINITE LOCATION OF EXISTING UNDERGROUND UTILITIES.
3. THE CONTRACTOR WILL TAKE THE NECESSARY PRECAUTIONS TO PROTECT EXISTING UTILITIES FROM DAMAGE DUE TO THIS OPERATION. ANY DAMAGE TO THE UTILITIES WILL BE REPAIRED AT THE CONTRACTOR'S EXPENSE, AND ANY SERVICE DISRUPTION WILL BE SETTLED BY THE CONTRACTOR.
4. CONCRETE USED IN CURB AND GUTTER, SIDEWALK, AND CROSSPAN CONSTRUCTION WILL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS.
5. ALL BACKFILL, SUB-BASE, AND/OR BASE COURSE (CLASS 6) MATERIAL SHALL BE COMPACTED PER THE SOILS ENGINEER'S RECOMMENDATIONS.
6. ALL STATIONING IS CENTERLINE OF IMPROVEMENTS UNLESS OTHERWISE INDICATED. ALL ELEVATIONS ARE FLOW LINE UNLESS OTHERWISE INDICATED AS TOP BACK OF CURB (TBC), ASPHALT (ASP), OR TOP OF INLET OR BOX (TOB).
7. ALL CURB RETURNS AND 10' EITHER SIDE OF CURB RETURNS SHALL BE 8" VERTICAL CURB, CITY OF COLORADO SPRINGS TYPE 1 CURB WITH AND ADDITIONAL 10' OF TRANSITION TO 6" RAMP CURB. CITY OF COLORADO SPRINGS MODIFIED TYPE 5 CURB, UNLESS OTHERWISE INDICATED.
8. PEDESTRIAN RAMPS SHALL BE INSTALLED AT INTERSECTIONS AS SHOWN AND CONFORM TO THE CITY OF COLORADO SPRINGS, DEPARTMENT OF PUBLIC WORKS STANDARDS AND SPECIFICATIONS.
   NOTE: WIDTH OF PEDESTRIAN RAMPS MUST MATCH WIDTH OF SIDEWALK.
9. IF A DISCREPANCY OCCURS BETWEEN THE CONSTRUCTION DOCUMENTS AND THE CITY OF COLORADO SPRINGS STANDARD SPECIFICATIONS, THE ENGINEER WILL BE NOTIFIED IMMEDIATELY FOR RESOLUTION.
10. THE CONTRACTOR SHALL SECURE ALL APPLICABLE LICENSES AND PERMITS TO COMPLETE THE CONSTRUCTION IN COMPLIANCE WITH ALL LOCAL, STATE, AND FEDERAL REGULATIONS.
11. CONTRACTOR TO OBTAIN COPIES OF THE SOILS REPORT FROM THE GEOTECHNICAL ENGINEER AND TO BE KEPT ON-SITE DURING ALL EARTHWORK OPERATIONS.

CONCRETE: CONCRETE REINFORCEMENT:

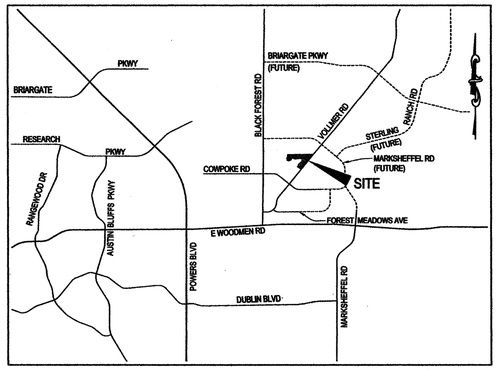
1. ALL CAST IN PLACE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE YIELD STRENGTH OF 4,000 PSI UNLESS OTHERWISE NOTED. HIGHER COMPRESSIVE STRENGTH CONCRETE IS ACCEPTABLE TO ACHIEVE EARLY CONCRETE STRENGTH THAT MAY BE DEEMED NECESSARY TO MEET CONSTRUCTION SCHEDULING PRIORITIES.
2. ALL CAST IN PLACE CONCRETE REINFORCEMENT SHALL HAVE A MINIMUM TENSILE YIELD STRENGTH OF 6,000 PSI UNLESS OTHERWISE NOTED, AND CONFORMANCE WITH CITY OF COLORADO SPRINGS SPECIFICATIONS, SECTION 603.
3. CONCRETE TESTING SHALL BE IN CONFORMANCE WITH CITY OF COLORADO SPRINGS SPECIFICATIONS, SECTION 506.

STORM SEWER NOTES:

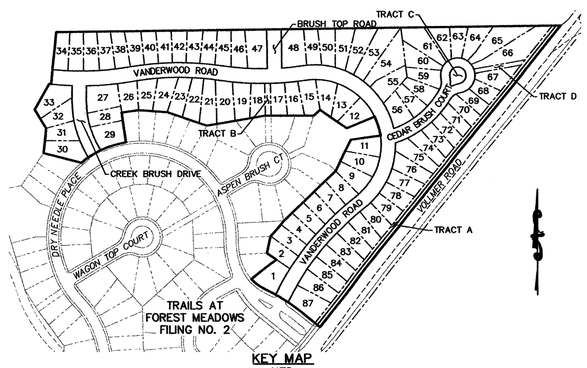
1. CONSTRUCT AND INSTALL D-10-R INLETS PER CITY OF COLORADO SPRINGS SHEET D-10-R 1, 2, AND 3.
2. CONSTRUCT AND INSTALL TYPE I MANHOLES PER CITY OF COLORADO SPRINGS SHEETS D-20A (1) AND D-20D (4).
3. THE MINIMUM CLASS OF REINFORCED CONCRETE PIPE SHALL BE CLASS III.

TRAFFIC ENGINEERING GENERAL NOTES:

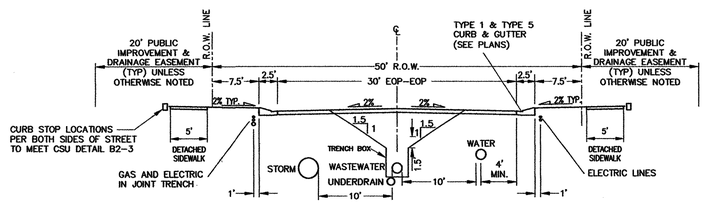
1. CALL BEFORE EXCAVATING, CONTRACTOR SHALL VERIFY LOCATION OF UNDERGROUND UTILITIES.
2. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY MONUMENTATION AND/OR BENCHMARKS WHICH WILL BE DISTURBED OR DESTROYED BY CONSTRUCTION. SUCH POINTS SHALL BE REFERENCED AND REPLACED WITH APPROPRIATE MONUMENTATION BY A REGISTERED CIVIL ENGINEER AUTHORIZED TO PRACTICE LAND SURVEYING.
3. APPROVAL OF THESE PLANS BY THE CITY ENGINEER DOES NOT AUTHORIZE ANY WORK TO BE PERFORMED UNTIL A PERMIT HAS BEEN ISSUED.
4. THE APPROVAL OF THESE PLANS OR ISSUANCE OF A PERMIT BY THE CITY OF COLORADO SPRINGS DOES NOT AUTHORIZE THE SUBDIVIDER AND OWNER TO VIOLATE ANY FEDERAL, STATE, OR CITY LAWS, ORDINANCES, REGULATIONS, OR POLICIES.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NEW, TEMPORARY AND EXISTING TRAFFIC SIGNS FROM THE START OF THE CONSTRUCTION PROJECT UNTIL ACCEPTANCE BY CITY TRAFFIC ENGINEERING.
6. ALL TRAFFIC SIGNS, PAVEMENT MARKINGS, AND TRAFFIC SIGNALS SHALL MEET OR EXCEED M.T.U.C.D. STANDARDS.
7. THE CONTRACTOR SHALL NOT REMOVE ANY EXISTING SIGNS, PAVEMENT MARKINGS OR TRAFFIC SIGNALS DURING THE PROJECT WITHOUT SIGNED AUTHORIZATION OF THE CITY TRAFFIC ENGINEERING INSPECTOR ASSIGNED TO THE PROJECT.
8. CONTRACTOR SHALL PREPARE A DETAILED TRAFFIC CONTROL PLAN, SUBMIT TO CITY TRAFFIC ENGINEERING FOR APPROVAL, AND OBTAIN APPROPRIATE PERMITS IN ACCORDANCE WITH THE "TRAFFIC CONTROLS FOR STREET CONSTRUCTION, UTILITY WORK AND MAINTENANCE OPERATIONS", M.U.T.C.D. SUPPLEMENT FOR THE CITY OF COLORADO SPRINGS. AUGUST 1992.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK ZONE TRAFFIC CONTROL. CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING, INSTALLING AND MAINTAINING THE TEMPORARY TRAFFIC CONTROL DEVICES THROUGHOUT THE DURATION OF THE PROJECT.
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NEW, TEMPORARY, AND EXISTING TRAFFIC SIGNAL MODIFICATIONS.



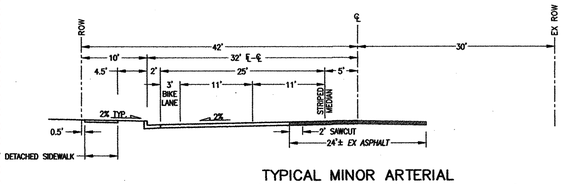
VICINITY MAP
N.T.S.



KEY MAP
NTS



TYPICAL RESIDENTIAL STREET
SCALE: NTS



TYPICAL MINOR ARTERIAL
STREET/UTILITY SECTION (VOLLMER ROAD)
SCALE: NTS

AGENCIES

OWNER:	CHALLENGER HOMES, INC. 13570 NORTHGATE ESTATES DRIVE COLORADO SPRINGS, CO 80921 ROGER MILLER (719) 598-5192 ROGER@MYCHALLENGERHOMES.COM
CIVIL ENGINEER:	M & S CIVIL CONSULTANTS, INC. 20 BOULDER CRESCENT, SUITE 110 COLORADO SPRINGS, CO 80903 VIRGIL A. SANCHEZ P.E. (719) 955-5485
ENGINEERING DIVISION:	CITY OF COLORADO SPRINGS 30 S. NEVADA AVE., SUITE 401 COLORADO SPRINGS, CO 80903 ELIZABETH NIJKAMP, P.E. (719) 385-5410
TRAFFIC ENGINEERING:	CITY OF COLORADO SPRINGS 30 S. NEVADA AVE., SUITE 401 COLORADO SPRINGS, CO 80903 KATHLEEN KRAGER (719) 385-7628
DEVELOPMENT SERVICES:	COLORADO SPRINGS UTILITIES 1521 HANCOCK EXPRESSWAY COLORADO SPRINGS, CO 80903 AL JUVERA (719) 668-8769
GAS DEPARTMENT:	COLORADO SPRINGS UTILITIES 7710 DURANT DR. COLORADO SPRINGS, CO 80920 TIM WENDT (719) 668-3556
ELECTRIC DEPARTMENT:	COLORADO SPRINGS UTILITIES 7710 DURANT DR. COLORADO SPRINGS, CO 80920 SARAH LABARRE (719) 668-4933
COMMUNICATIONS:	QWEST COMMUNICATIONS (U.N.C.C. LOCATORS) (800) 922-1987 AT&T (LOCATORS) (719) 635-3674

DETAILED DRAINAGE CONSTRUCTION PLANS AND SPECIFICATIONS ENGINEER'S STATEMENT:

THESE DETAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECTION AND SUPERVISION. SAID DETAILED PLANS AND SPECIFICATIONS HAVE BEEN PREPARED ACCORDING TO THE ESTABLISHED CRITERIA FOR DETAILED DRAINAGE PLANS AND SPECIFICATIONS, AND SAID DETAILED PLANS AND SPECIFICATIONS ARE IN CONFORMITY WITH THE MASTER PLAN OF THE DRAINAGE BASIN. SAID DETAILED DRAINAGE PLANS AND SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR DRAINAGE FACILITY(S) IS DESIGNED. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARATION OF THE DETAILED DRAINAGE PLANS AND SPECIFICATIONS."

VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160
FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

9/16/15
DATE

PLAN REVIEW BY CITY OF COLORADO SPRINGS IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH DESIGN CRITERIA. THE CITY OF COLORADO SPRINGS IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE CITY OF COLORADO SPRINGS, THROUGH THE APPROVAL OF THIS DOCUMENT, ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

SHEET INDEX

SHEET 1	TITLE SHEET
SHEET 2	PLAN & PROFILE - VANDERWOOD ROAD
SHEET 3	PLAN & PROFILE - VANDERWOOD ROAD
SHEET 4	PLAN & PROFILE - CEDAR BRUSH COURT
SHEET 5	PLAN & PROFILE - CREEK BRUSH DRIVE & BRUSH TOP ROAD
SHEET 6	PLAN & PROFILE - VOLLMER ROAD
SHEET 7	VANDERWOOD ROAD STORM DRAIN AND INLETS
SHEET 8	SIGNAGE AND STRIPING PLAN

PROJECT DATUM
VERTICAL - NATIONAL GEODETIC VERTICAL DATUM 1929 (NGVD29)
HORIZONTAL - NORTH AMERICAN DATUM 1983 (NAD83)
CONTROL - COLORADO STATE PLANE CENTRAL ZONE

STATEMENT:

THE CITY OF COLORADO SPRINGS RECOGNIZES THE DESIGN ENGINEER AS HAVING RESPONSIBILITY FOR THE DESIGN; THE CITY HAS LIMITED ITS SCOPE OF REVIEW ACCORDINGLY. RESUBMITTAL REQUIRED IF CONSTRUCTION HAS NOT COMMENCED WITHIN 180 DAYS AFTER REVIEW DATE.

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES
FOR BURIED UTILITY INFORMATION
48 HRS BEFORE YOU DIG
CALL 1-800-922-1987

STREET DESIGN:
REVIEW:
TRAFFIC ENGINEERING: _____ DATE: _____
CURB & GUTTER: _____ DATE: 9/18/15
FINAL REVIEW: _____ DATE: _____
DRAINAGE DESIGN:
DRAINAGE REVIEW: _____ DATE: _____
FILED IN ACCORDANCE WITH SECTION 7-7-906 OF COLORADO SPRINGS CODE 2001, AS AMENDED.

DESIGN DATA:

* FULL DEPTH ASPHALT

SIDEWALKS: WIDTH 5'
LOCATION: Attached ☐
Detached ☐ CENTERED IN 5' EASEMENT
CURB TYPE 1 ☒
CURB TYPE 5 ☒
ROW WIDTH 50' EOP-EOP 30'
STREET TYPE RES HVEEM

ASPHALT THICKNESS:
AC Surface _____
AC Base _____
AGG. BASE THICKNESS:
Class 6 _____
Class 5 _____
Class 2 _____

REVISIONS: NO. | DATE: | BY: | DESCRIPTION:

APPR'D. BY: | DATE:

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

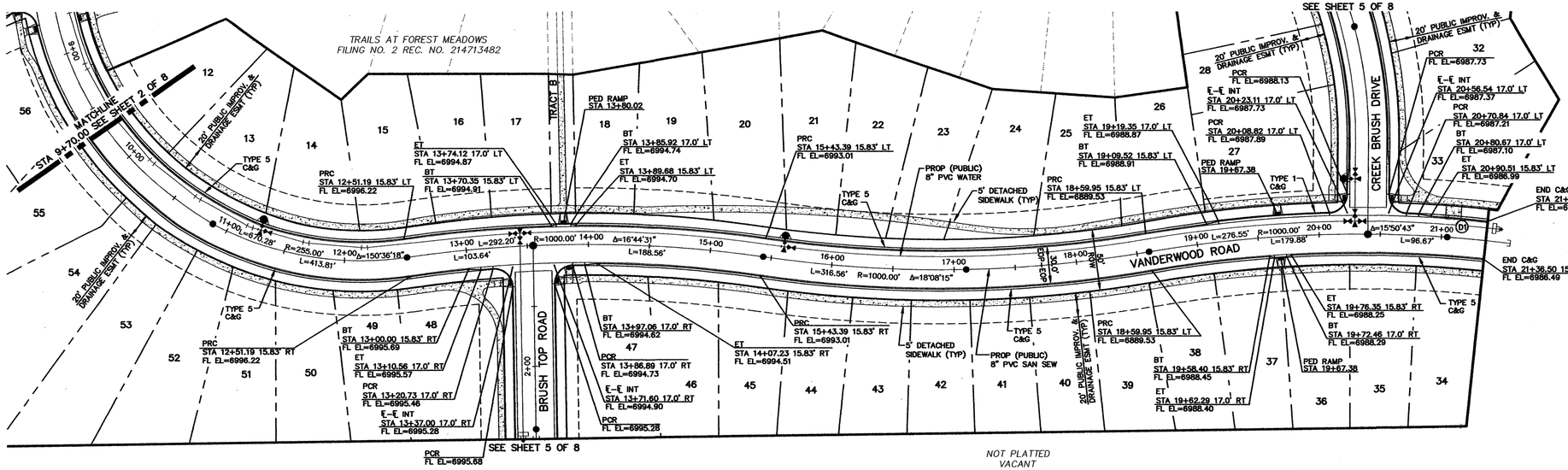
CAUTION

TRAILS AT FOREST MEADOWS FILING NO. 3
STREET IMPROVEMENT PLANS COVER SHEET
PROJECT NO. 08-029 | FILE: \dwg\Cons...Drgs...\Street Improv...\SI01.dwg
DESIGNED BY: GW | SCALE
DRAWN BY: BB | HORIZ: N/A
CHECKED BY: VAS | VERT: N/A
DATE: 8-05-15
SHEET 1 OF 8
SI01

20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485
M&S CIVIL CONSULTANTS, INC.

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.
VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160

21326.20



VANDERWOOD ROAD (VR)

1" = 50'

0 25 50 100

Scale in Feet

STREET ABBREVIATIONS

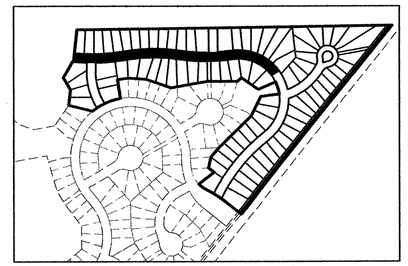
VR	VANDERWOOD ROAD
VR-2	VOLLMER ROAD
CBC	CEDAR BRUSH COURT
BTR	BRUSH TOP ROAD
CBD	CREEK BRUSH DRIVE
DNP	DRY NEEDLE PLACE

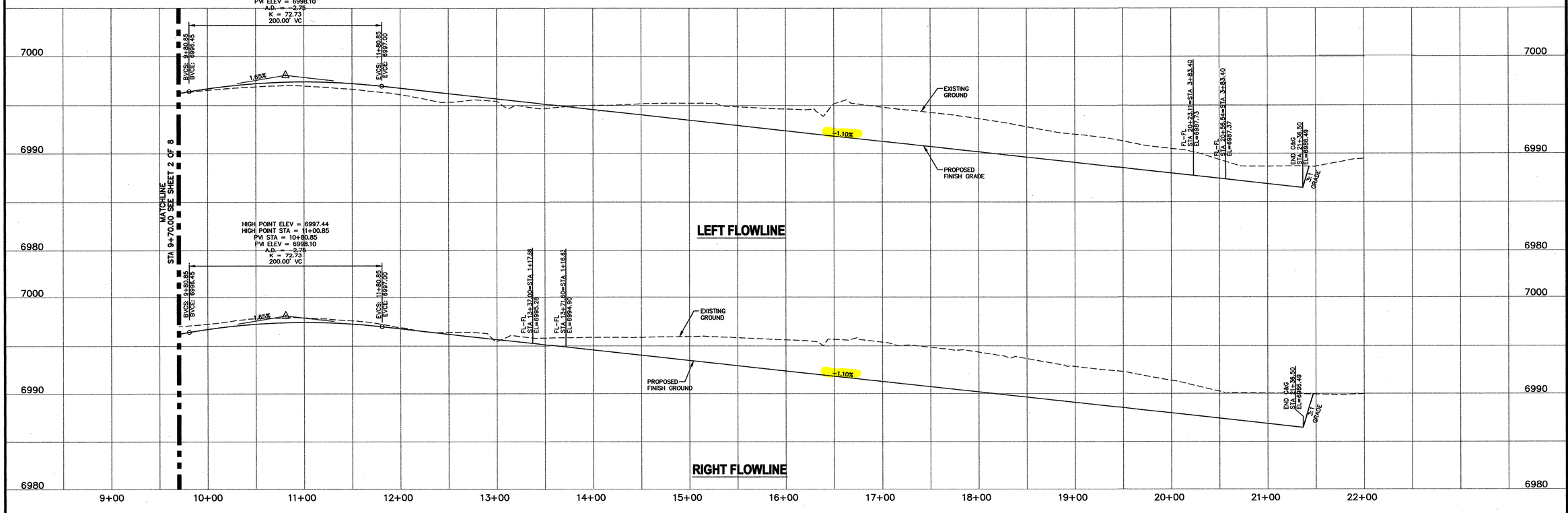
NOTES:

1. ALL CURB RETURN RADIUS SHALL BE 15.00'. CURB RETURNS WILL ALWAYS USE TYPE I VERTICAL CURB. WHEN TRANSITIONING FROM RAMP CURB TO VERTICAL CURB A 20 FOOT TRANSITION SHALL BE USED AS FOLLOWS: BEGINNING AT THE P.T.C.R., TEN FEET OF VERTICAL CURB AND 10 FEET OF TRANSITION SECTION.
2. ALL SIDEWALKS SHALL BE 5' WIDE AND DETACHED UNLESS OTHERWISE STATED.
3. ALL CROSS PANS SHALL BE CITY STANDARD D-7 (6' PAN) AS SHOWN.
4. ALL PED RAMPS LOCATED AT INTERSECTIONS SHALL BE CITY STD D-8A AND ALL MID STREET PED RAMPS SHALL BE CITY STD D-8D.
5. DRIVEWAYS FOR LOTS 1, 33, 75, & 87 SHALL BE PER CITY STANDARD D-16

SPECIAL DRIVEWAY LOCATIONS

LOT 33 (D1) 16' DRIVEWAY CUT
STA 20+98.87, 15.83' LT
STA 21+14.62, 15.83' LT





20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

CIVIL CONSULTANTS, INC.

TRAILS AT FOREST MEADOWS FILING NO. 3

STREET IMPROVEMENT PLANS ~ VANDERWOOD ROAD

PROJECT NO. 06-029	FILE: \dwg\Cons...Drwn...\STREET IMPR...\SI03.dwg		SI03
DESIGNED BY: GW	SCALE	DATE: 8-05-15	SHEET 3 OF 8
DRAWN BY: BB	HORIZ: 1"=50'		
CHECKED BY: VAS	VERT: 1"=5'		

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160

REVISIONS:

NO.	DATE:	BY:	DESCRIPTION:	APPRO. BY:	DATE:

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CAUTION

STATEMENT:

THE CITY OF COLORADO SPRINGS RECOGNIZES THE DESIGN ENGINEER AS HAVING RESPONSIBILITY FOR THE DESIGN; THE CITY HAS LIMITED ITS SCOPE OF REVIEW ACCORDINGLY. RESUBMITTAL REQUIRED IF CONSTRUCTION HAS NOT COMMENCED WITHIN 180 DAYS AFTER REVIEW DATE.

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES

FOR BURIED UTILITY INFORMATION
48 HRS BEFORE YOU DIG
CALL 1-800-922-1987

STREET DESIGN:

REVIEW:

TRAFFIC ENGINEERING: _____ DATE: _____

CURB & GUTTER: [signature] DATE: 9/18/15

FINAL REVIEW: [signature] DATE: 10/29/15

DRAINAGE DESIGN:

DRAINAGE REVIEW: _____ DATE: _____

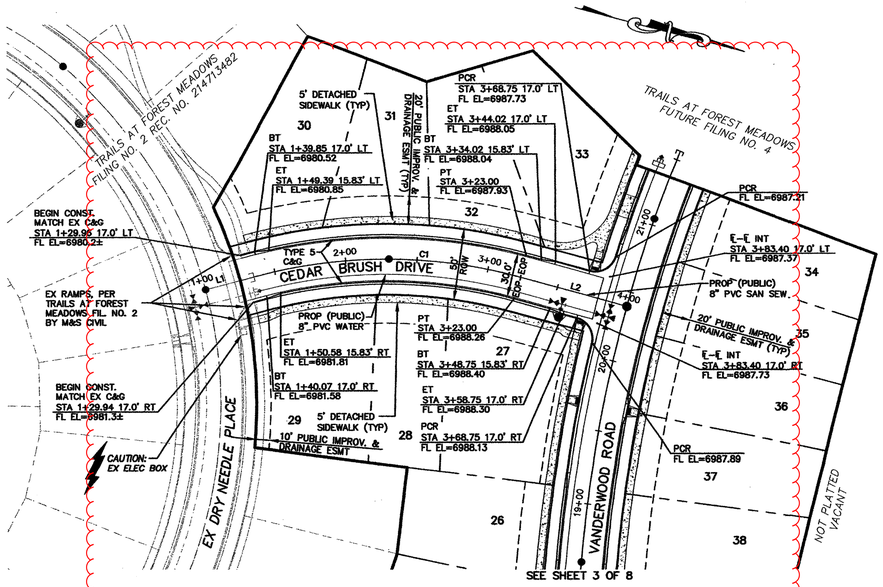
FILED IN ACCORDANCE WITH SECTION 7-7-906 OF COLORADO SPRINGS CODE 2001, AS AMENDED.

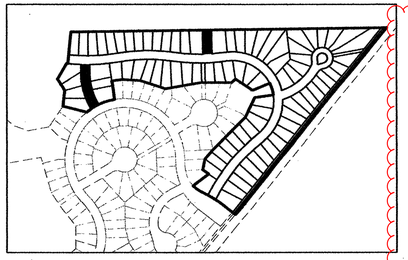
DESIGN DATA:

SIDEWALKS: WIDTH 5'
LOCATION: Attached ☐
Detached ☐ CENTERED IN 5' EASEMENT
CURB TYPE 1 ☒
CURB TYPE 5 ☒
ROW WIDTH 50' EOP-EOP 30'
STREET TYPE RES HVEEM

" FULL DEPTH ASPHALT

ASPHALT THICKNESS: 4"
AC Surface
AC Base
AGG. BASE THICKNESS: 6"
Class 6
Class 5
Class 2





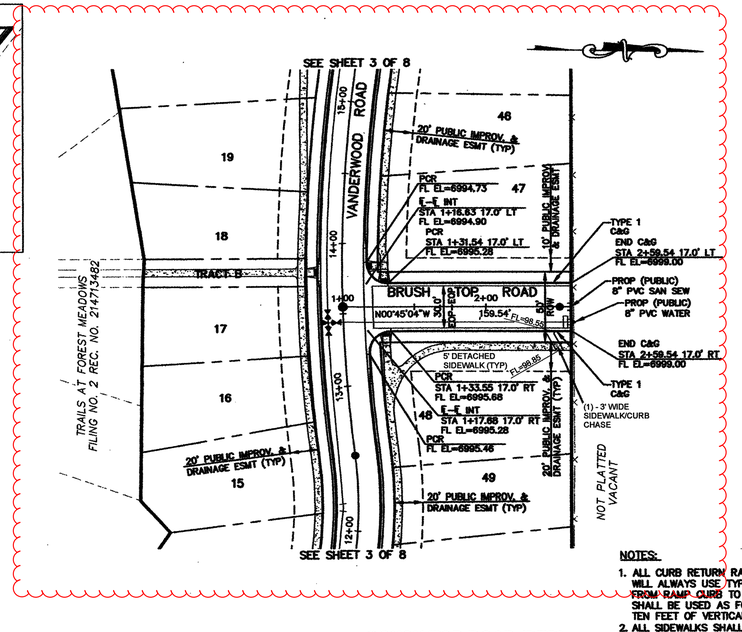
LINE TABLE		
LINE	LENGTH	BEARING
L1	37.44'	N29°53'25"W
L2	77.26'	N00°29'11"E

CURVE TABLE			
CURVE	DELTA	RADIUS	LENGTH
C1	30°22'38"	350.00	185.56'

1" = 50'

0 25 50 100

Scale in Feet



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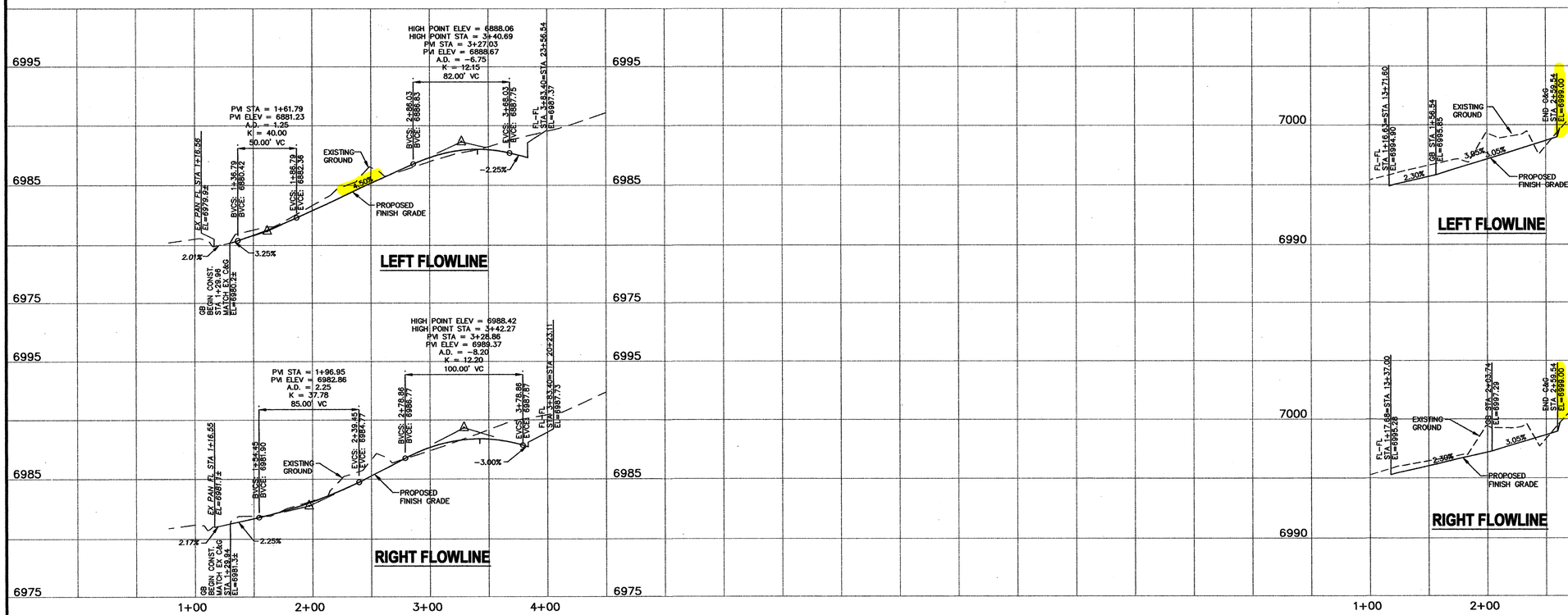

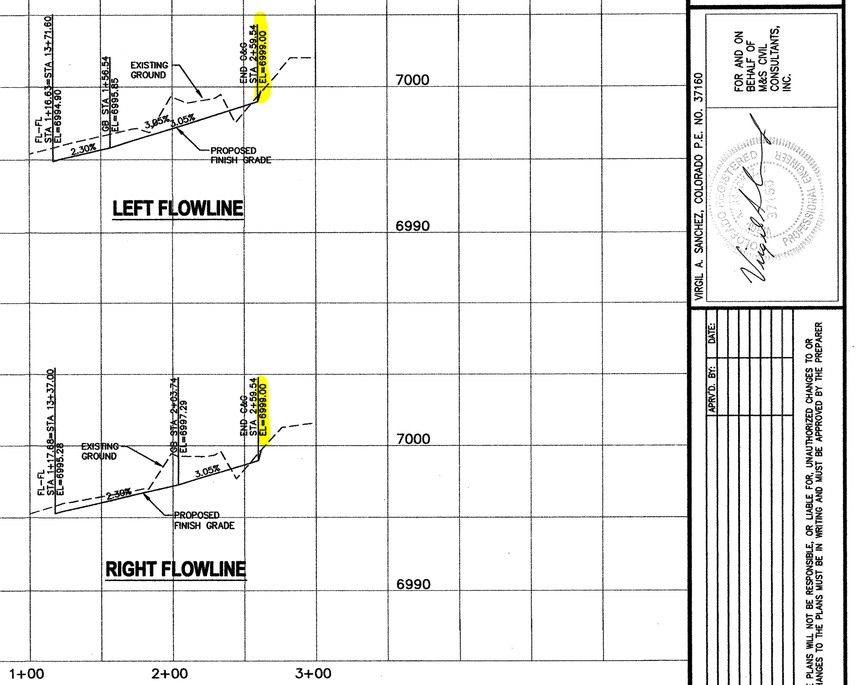

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STREET DESIGN:
REVIEW:
TRAFFIC ENGINEERING: _____ DATE: -
CURB & GUTTER: _____ DATE: 9/18/15
FINAL REVIEW: _____ DATE: 10/29/15

DRAINAGE DESIGN:
DRAINAGE REVIEW: _____ DATE: _____
FILED IN ACCORDANCE WITH SECTION 7-7-906 OF COLORADO SPRINGS CODE 2001, AS AMENDED.

DESIGN DATA:

SIDEWALKS: WIDTH 5'
LOCATION: Attached ☐ Detached ☐ CENTERED IN 5' EASEMENT
CURB TYPE 1 ☒
CURB TYPE 5 ☒
ROW WIDTH 50' EOP-EOP 30'
STREET TYPE RES HVEEM

" FULL DEPTH ASPHALT
ASPHALT THICKNESS: 6"
AC Surface ___
AC Base ___
AGG. BASE THICKNESS: 11"
Class 6 6
Class 5 ___
Class 2 ___

REVISIONS:

NO.	DATE:	BY:	DESCRIPTION:

DATE:	APPR'D. BY:

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CAUTION

TRAILS AT FOREST MEADOWS FILING NO. 3

STREET IMPROVEMENT ~ CREEK BRUSH CT/BRUSH TOP RD

PROJECT NO. 08-029 | DESIGNED BY: GW | DRAWN BY: BB | CHECKED BY: VAS
FILE: \eng\Cons...Drwn.\Street Improv...\SI05.dwg | SCALE | DATE: 8-05-15 | HORIZ: 1"=50' | VERT: 1"=5'

SHEET 5 OF 8 — SI05

20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

CIVIL CONSULTANTS, INC.

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160

TRAILS AT FOREST MEADOWS
FILING NO. 4
FINAL DRAINAGE REPORT

AMENDMENT TO:
MASTER DEVELOPMENT DRAINAGE PLAN UPDATE FOR WOODMEN HEIGHTS AND FINAL DRAINAGE REPORT FOR FOREST MEADOWS FILING NO.1AND NO.4

April 2016

Prepared for:

Challenger Homes, Inc.
13530 Northgate Estates Drive, Suite 200
Colorado Springs, CO 80921
(719) 598-5192

Prepared by:

CIVIL CONSULTANTS, INC.
20 Boulder Crescent, Suite 110
Colorado Springs, CO 80903
(719) 955-5485

Project #08-032

ranges from 12 to 14 inches per year, with the majority of this moisture occurring in the spring and summer in the form of rainfall. Thunderstorms are common during the summer months.

FLOODPLAIN STATEMENT

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0529, effective date March 17, 1997, no portion of the site lies within a designated floodplain.

DRAINAGE CRITERIA

The drainage analysis has been prepared in accordance with the current City of Colorado Springs/El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities during the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres.

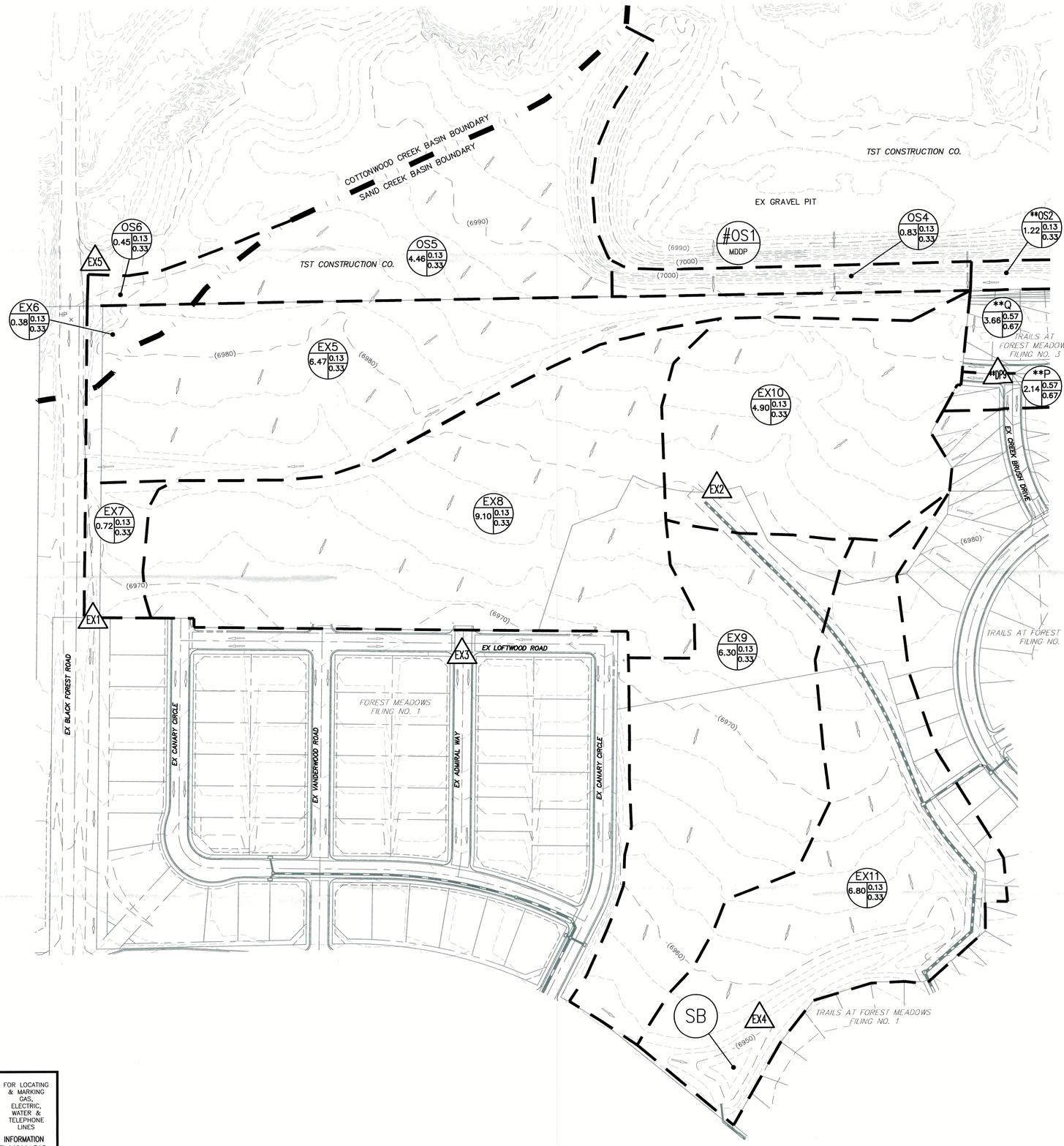
EXISTING DRAINAGE CONDITIONS

The overall site consists of 20.514 acres and is currently undeveloped, with the initial grading project proposed to disturb 20.514 acres for Filing No. 4 residential subdivision. Filing No.4 proposes 90 single family residential lots. All of the streets, curb, gutter, sidewalk and utilities for the development of Vanderwood Road, Oak Vine Court, Salt Brush Road, Leaf Wood Court and Admiral Way, will be constructed in Filing No. 4.

In order to compare past studies, a portion of the drainage basins within this study are denoted by asterisks. The drainage basins labels preceded (or followed) by an single hashtag (#) are referencing watersheds previously illustrated and/or described within the "Master Development Drainage Plan Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No.1 & No.4" (MDDP), by Classic Consulting Engineers, & Surveyors, dated February 2006. Those drainage basins labels preceded (or followed) by two asterisks (**) are referencing watersheds previously illustrated and/or described within the "Trails at Forest Meadows Filing No. 3 Final Drainage Report"(TFM 3 FDR), by M&S Civil Consultants, Inc., dated September 2015.

Per review of the existing topographic drainage patterns, the off-site historic flows, north of the property (OSl), are conveyed west by an existing berm and swale to an existing gravel borrow pit, as evident by the eroded embankment at the southeast comer of the gravel borrow pit and evaluation of grade during onsite visits. Per the "Master Development Drainage Plan for Woodmen Heights Master ·Plan" prepared by Classic Consulting Engineers and Surveyors, dated June 2004, it was inferred that the entire watershed (historic flows of Q5=65 cfs and Q100=162 cfs) would be transported to the future detention facility through our site. This report was amended by the "Master Development Drainage Plan Update for Woodmen Heights and Final Drainage Report for Forest Meadows Filing No.1 and No. 4" (MDDP) by Engineering and Surveying, Inc., dated February 2006, in which it states that 50% of the flow from OS1 (Q5=34 cfs, Q100==84 cfs) will be collected by a proposed concrete pipe and will be conveyed through and combined with the flows of Trails at Forest Meadows Filings to Sand Creek Regional Detention Facility No. 6. Per historic drainage patterns and per the "Preliminary/Final Drainage Report for Highland Park Filing No.2", prepared by Law and Mariotti Consultants, dated June 2002, which is referenced in the MDDP, the remainder of the flows will be transported via road side swale on the west side of Vollmer Road to the proposed drainage infrastructure in Vollmer Road. The flows on Vollmer Road were addressed in the "Trails at Forest Meadows Filing No. 3 Final Drainage Report"(TFM 3 FDR). Any increase in flows due to future development of Basin OS1 will require the construction of a proposed detention facility, as per the Sand Creek DBPS.

In the interim, Historic flows **OS2-Q_5=0.8 cfs and Q_{100}=3.6 cfs, OS4-Q_5=0.4 cfs and Q_{100}=1.8 cfs, OS5-Q_5=2.1 cfs and Q_{100}=9.0 cfs, EX5- Q_5=1.8 cfs and Q_{100}=7.5 cfs, EX7- Q_5=0.3 cfs and Q_{100}=1.1 cfs, see



1" = 100'

0 50 100 200

Scale in Feet

LEGEND

- SB — SEDIMENT BASIN
- 200 — PIPE RUN REFERENCE LABEL
- 2A — SURFACE DESIGN POINT
- BASIN BOUNDARY
- 6920 — EXISTING CONTOUR
- 6920 — PROPOSED CONTOUR
- STORM SEWER PIPE
- CROSSPAN
- INLET

BASIN DESIGNATION: Z | 25 | .25 | .35 | C5 | C100 | ACRES

- EXISTING FLOW DIRECTION ARROW
- EMERGENCY OVERFLOW DIRECTION
- FLOW DIRECTION
- FLARED END SECTION
- PERIMETER SWALE
- FILING LINE
- 11 — LOT NUMBER
- 2.5% — STREET GRADE W/DIRECTION
- H.P. — HIGH POINT
- L.P. — LOW POINT

BASIN SUMMARY			
BASIN	AREA (ACRES)	Q5	Q100
#OS1	78	34.4	84.1
**OS2	1.22	0.8	3.6
OS4	0.83	0.4	1.8
OS5	4.46	2.1	9.0
OS6	0.45	0.1	0.6
EX5	6.47	1.8	7.5
EX6	0.38	0.2	0.6
EX7	0.72	0.3	1.1
EX8	9.10	3.1	13.3
EX9	6.30	1.7	7.4
EX10	4.90	1.5	6.4
EX11	6.80	1.5	6.3
**P	2.14	4.2	8.8
**Q	3.66	6.1	12.8

DESIGN POINT SUMMARY			
DESIGN POINT	Q5	Q100	BASIN
#DP9	10.2	22.4	**DP9Q,**DP9P
EX1	3.7	16.7	**OS2,OS4,OS5,EX5,EX7
EX2	11.5	28.5	**DP9,EX10
EX3	3.1	13.3	EX8
EX4	3.0	13.4	EX9,EX11
EX5	0.3	1.2	OS6,EX6

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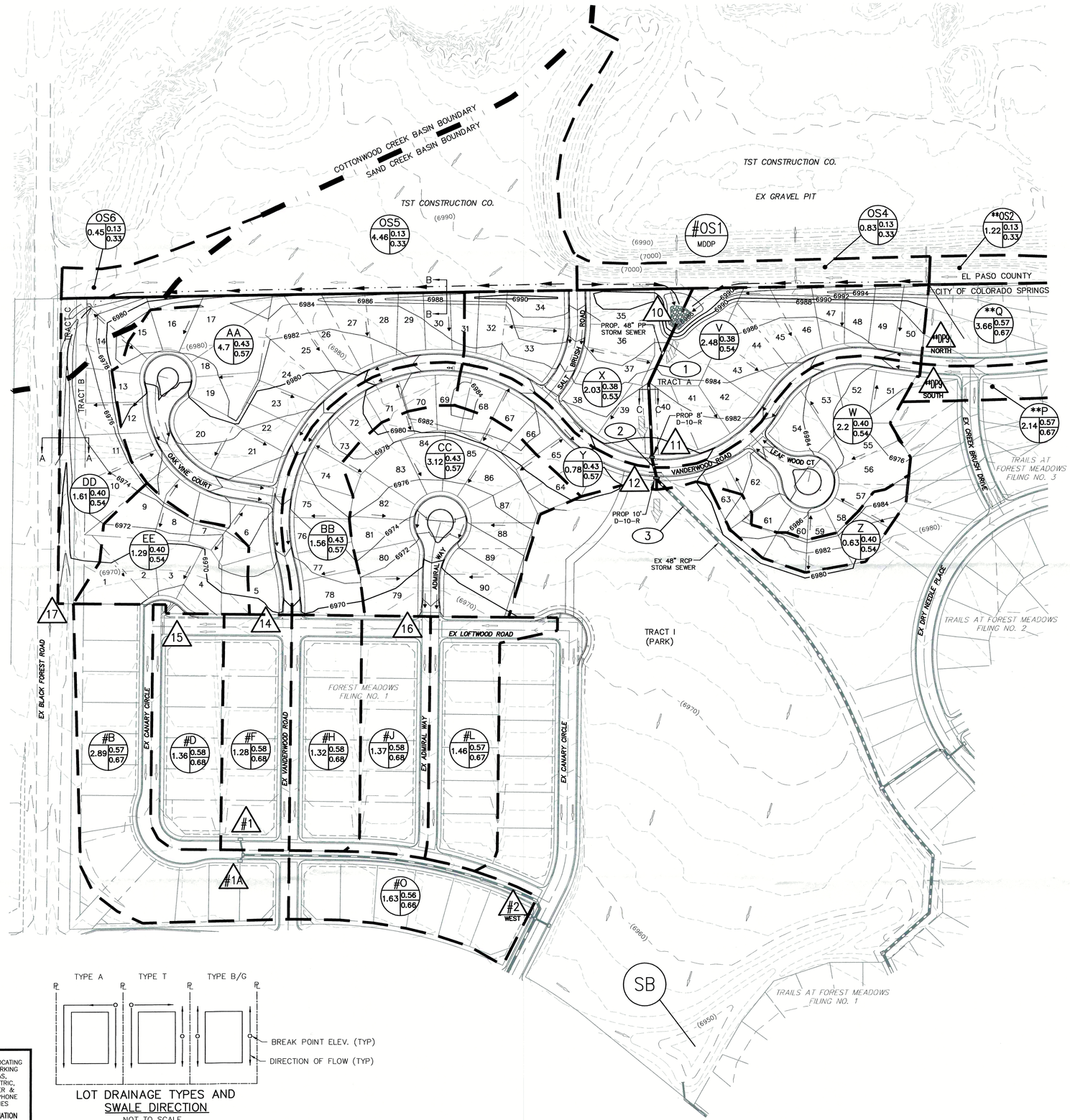
20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

TRAILS AT FOREST MEADOWS FILING NO. 4

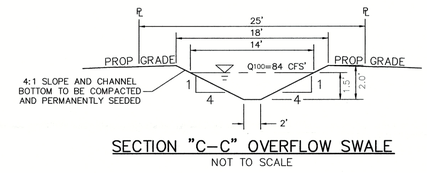
EXISTING DRAINAGE PLAN

PROJECT NO. 08-032
FILE: \dwg\Eng Exhibits\DP1.dwg
DESIGNED BY: ET
DRAWN BY: BB
CHECKED BY: ET
SCALE
HORIZ: 1"=100'
VERT: N/A
DATE: 3/28/16
SHEET 1 OF 2
DP1

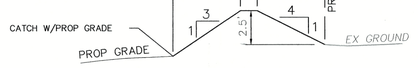


LEGEND

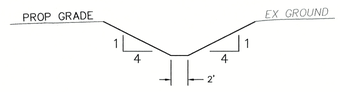
- SB — SEDIMENT BASIN
- 200 — PIPE RUN REFERENCE LABEL
- 2A — SURFACE DESIGN POINT
- BASIN BOUNDARY
- (6920) — EXISTING CONTOUR
- 6920 — PROPOSED CONTOUR
- STORM SEWER PIPE
- CROSSPAN
- INLET
- BASIN DESIGNATION: Z; 25 ACRES; .25 C5; .35 C100
- EXISTING FLOW DIRECTION ARROW
- EMERGENCY OVERFLOW DIRECTION
- FLOW DIRECTION
- FLARED END SECTION
- PERIMETER SWALE
- FILING LINE
- 11 — LOT NUMBER
- 2.5% — STREET GRADE W/DIRECTION
- H.P. — HIGH POINT
- L.P. — LOW POINT



SECTION "C–C" OVERFLOW SWALE
NOT TO SCALE



SECTION "B–B" BERM
NOT TO SCALE



SECTION "A–A" SWALE
NOT TO SCALE

BASIN SUMMARY			
BASIN	AREA (ACRES)	Q5	Q100
V	2.48	3.2	7.6
W	2.20	3.1	7.1
X	2.03	2.7	6.4
Y	0.78	1.2	2.8
Z	0.63	1.2	2.7
AA	4.70	7.2	15.9
BB	1.56	2.4	5.3
CC	3.12	5.1	11.3
DD	1.61	2.6	5.8
EE	1.29	2.3	5.3
OS4	0.83	0.4	1.8
OS5	4.46	2.1	9.0
OS6	0.45	0.1	0.6
#OS1	78.0	34.4	84.1
**OS2	1.22	0.8	3.6
**P	2.14	4.2	8.8
**Q	3.66	6.1	12.8
#B	2.89	6.4	13.4
#D	1.36	3.3	7.0
#F	1.28	3.0	6.3
#H	1.32	3.1	6.5
#J	1.37	3.2	6.8
#L	1.48	3.4	7.0
#O	1.63	3.6	7.6

DESIGN POINT SUMMARY			
DESIGN POINT	Q5	Q100	BASIN
**DP9 NORTH	6.1	12.8	**DP9Q
**DP9 SOUTH	3.6	7.5	**DP9P
10	1.0	4.7	OS4,**OS2
11	11.0	25.2	**DP9Q,V,X
12	7.5	16.8	**DP9P,W,Y
11–12	18.1	41.1	**DP9Q,**DP9P,V,X,W,Y
14	9.4	22.1	AA,BB
15	2.3	5.3	EE
#1&#1A	22.9	50.7	AA,BB,EE,#B,#D,#F,#H
16	5.0	11.9	CC
#2 WEST	12.9	28.2	CC,#J,#L,#O
17	4.2	14.5	OS5,OS6,DD

STORM SEWER SUMMARY			
PIPE RUN	Q5	Q100	PIPE SIZE
1	34.4	84.1	48" PP
2	41.3	100.0	48" PP
3	45.8	110.0	EX 48" RCP

LOT DRAINAGE TYPES AND SWALE DIRECTION
NOT TO SCALE

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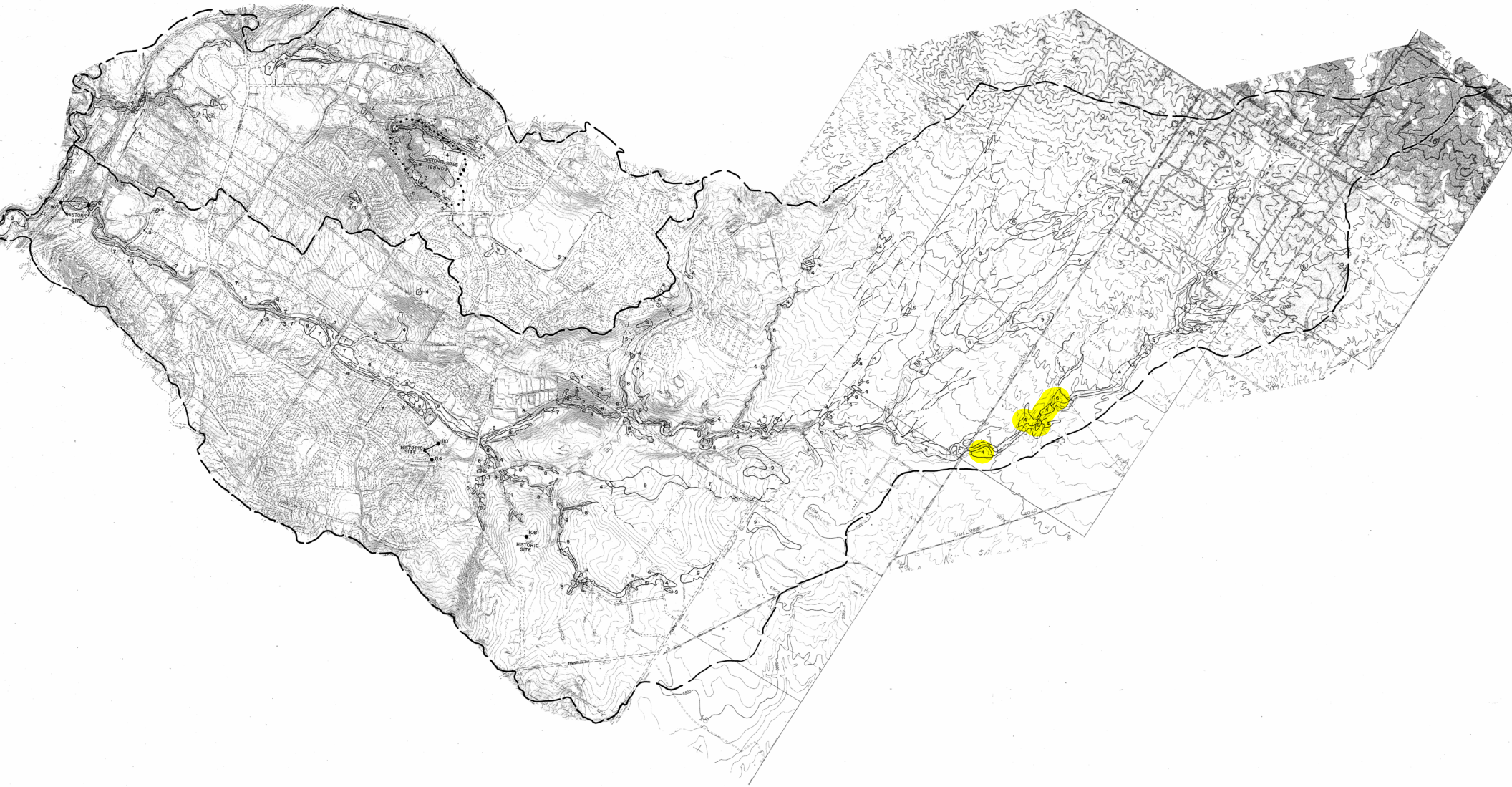
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

TRAILS AT FOREST MEADOWS FILING NO. 4

PROPOSED DRAINAGE PLAN

PROJECT NO. 08-032	FILE: \dwg\Eng Exhibits\DP2.dwg		
DESIGNED BY: ET	SCALE	DATE: 3/28/16	
DRAWN BY: BB	HORIZ: 1"=100'	SHEET 2 OF 2	DP2
CHECKED BY: ET	VERT: N/A		



NOTE: CULTURAL/HISTORICAL INVENTORIES ARE ALSO SHOWN ON THIS FIGURE.

1000 0

LEGEND:

1 AGRICULTURAL CHANNEL – drainage transecting pastures and fields
2 BACKWATER WETLAND – expanded areas of drainage above obstructions
3 ENGINEERED FLOODWAY – concrete lined channels
4 HERBACEOUS WETLAND – marshes, pondmargins and protected places along channels
5 MODIFIED CHANNEL – channelized, eroded (earth bottom)
6 OPEN WATER – ponds, reservoirs (excludes flowing channel)
7 RIPARIAN FOREST – cottonwood and conifers along channel.
8 SHRUB WETLAND – willow in and along channel (no tree overstory)
9 PRAIRIE SWALE – low lying grassy and weedy depressions along drainages

NOTE: Drainageways that are not classified have been inventoried but were not considered to be significant in terms of vegetation or wildlife habitat. Examples of this are bare bank & bottom channels, channels incised in rock, or soil, or fully man-made(streets & storm sewer), etc.)

SHEET INDEX:

MAPPING:
LANDMARK MAPPING

ENGINEER:
URS CONSULTANTS
MAKING TECHNOLOGY WORK
1040 SOUTH EIGHTH STREET
COLORADO SPRINGS, CO. 80906
(719)634-6699

PROJECT:
COTTONWOOD CREEK DRAINAGE BASIN PLANNI
ENVIRONMENTAL INVENTORY
SCALE: 1 =1000' CONTOUR INTERVAL=2'
FIGURE 5 SHEET

MASTER DEVELOPMENT DRAINAGE PLAN AMENDMENT FOR STERLING RANCH

Prepared For:
SR Land, LLC
20 Boulder Crescent, 2nd Floor
Colorado Springs, CO 80903
(719) 491-3024

March 29th, 2023
Project No. 25188.04

Prepared By:
JR Engineering, LLC
5475 Tech Center Drive
Colorado Springs, CO 80919
(303)-267-6240
Contact: Mike Bramlett, PE

PURPOSE

This document is an amendment to the approved Master Development Drainage Plan (MDDP) for Sterling Ranch. The purpose of this MDDP Amendment report is to:

1. Amend the approved MDDP to account for completed and planned on-site development within Sterling Ranch, including detention from completed filings.
2. Document all differences between the analysis and conclusions reached in the MDDP and this Amendment.
3. Provide analysis and conceptual design information for the on-line Detention Pond at Sterling Ranch Road (PNDW3).
4. This amendment is not intended to address drainage and bridge fees or credits for DBPS improvements. Subsequent submittals will address this.

GENERAL LOCATION AND DESCRIPTION

Location

Sterling Ranch, known as "the site" from herein, is a parcel of land located in Section 27, 28, 33 and 34, Township 12 South, and Section 4, Township 13 South, Range 65 West of the 6th Principal Meridian in El Paso County, Colorado. To the west the site is bound by Vollmer Road. To the north and east, the site is bounded by undeveloped land. To the south, the site is bound by the Pawnee Rancheros and Woodmen Heights developments. A vicinity map is presented in Appendix A.

Description of Property

Sterling Ranch is 1444 acres and is a Planned Unit Development to be built in multiple phases. The site is currently in various stages of development, with portions already in construction, with others unoccupied and undeveloped. The existing ground cover is sparse vegetation and open space, typical of a Colorado rolling range land condition. In general, the site slopes from north to south and the existing drainageways follow this topography.

Per a NRCS web soil survey of the area, the site is made up of Type A and B soils. Type A soils cover roughly 65% of the site while Type B soils cover the remaining 35% of the site. Group A soils have a high infiltration rate when thoroughly wet. Type B soils have a moderate infiltration when thoroughly wet. Type D soils have a very slow infiltration rate when thoroughly wet and have a high shrink-swell potential. A NRCS soil survey map has been presented in Appendix A.

Floodplain Statement

Based on the FEMA FIRM Map numbers 08041C0533G and 08041C0535G, dated December 7, 2018, the site lies within Zone AE and Zone X of the Sand Creek floodplain. Zone AE is defined as area subject to inundation by the 1-percent-annual-chance flood event. Zone X is defined as area outside the Special Flood Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. All proposed development within the site occurs in Zone X. The current FIRM Map has been presented in Appendix A.

DRAINAGE BASINS AND SUBBASINS

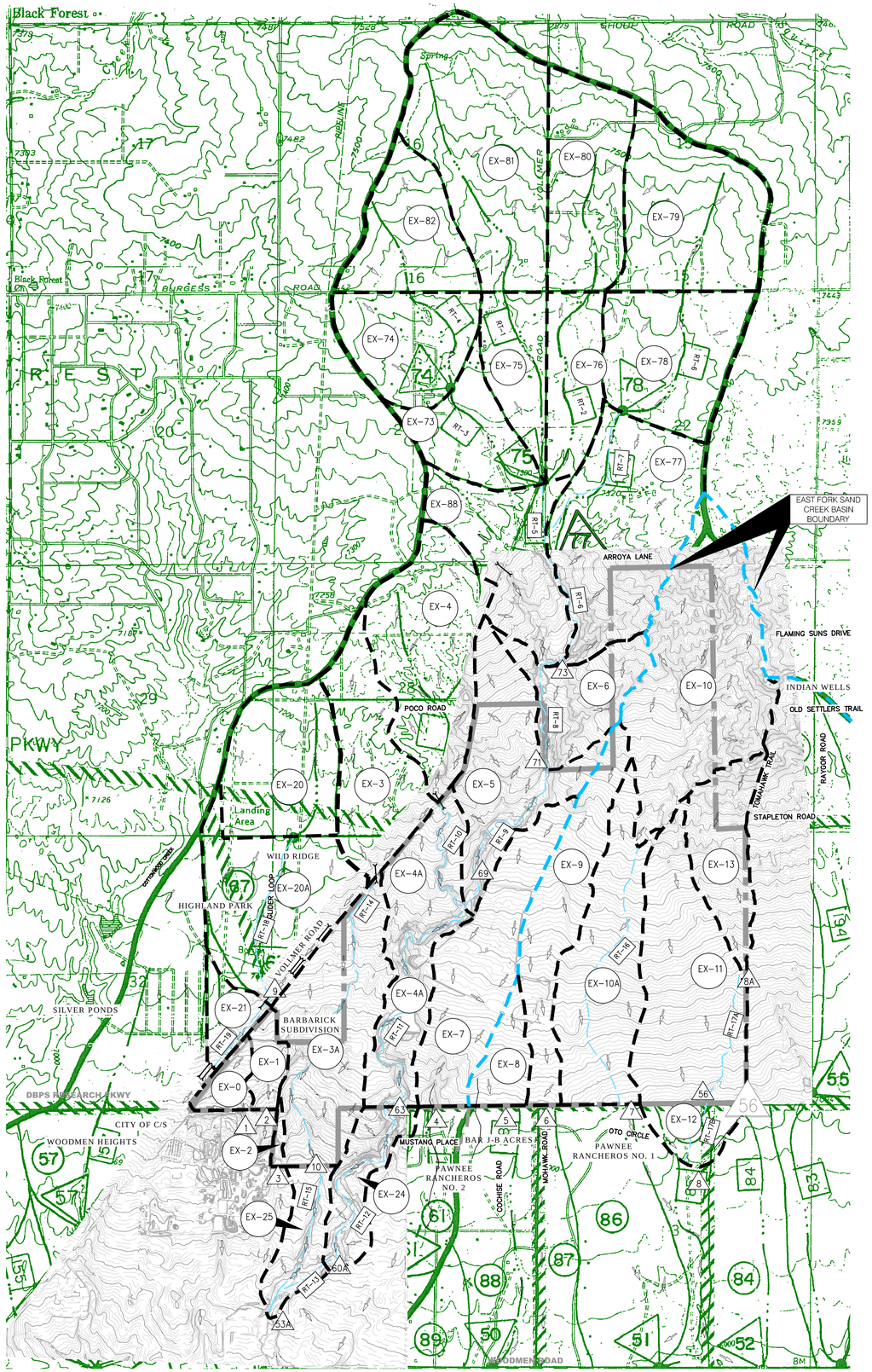
Major Basin Descriptions

The site lies within two major drainage basins: the Sand Creek Drainage Basin and the East Fork of Sand Creek Drainage Basin. Both Basins have been previously studied firstly in the 1996 Drainage Basin Planning Study (DBPS) and again in the 2018 Sterling Ranch MDDP by M&S, and finally in the 2021 DBPS by Stantec which has not been adopted by the El Paso County.

The Sand Creek Drainage Basin covers approximately 22 square miles and begins approximately five miles northeast of the Town of Falcon and travels approximately 15 miles to the southeast. The majority of the area within the basin is undeveloped and is characterized as rolling range land typically associated with Colorado's semi-arid climates. Anticipated land use for the basin includes residential, industrial, agricultural and commercial development. Residential developments will range from 0.125 – 5 acre lots with a mix of low, medium and high density developments.

As part of its drainage research, JR Engineering reviewed the following drainage studies and reports:

- Sand Creek Drainage Basin Study prepared by Kiowa in 1996
- Upper Sand Creek Basin Drainage Study prepared by Wilson in 2011
- Sand Creek Drainage Basin Study prepared by Stantec in 2021
- Sterling Ranch Master Development Drainage Plan prepared by M&S in 2018
- Sterling Ranch Filing 1 Final Drainage report prepared by M&S in 2016
- Upper Sand Creek Basin Detention Evaluation Report by Wilson in 2009
- Branding Iron at Sterling Ranch Filing 1 Drainage Report prepared by M&S in 2018
- Branding Iron at Sterling Ranch Filing 2 Drainage Report prepared by M&S in 2020
- Sterling Ranch East Preliminary Drainage Report prepared by Classic Consultants in 2022



0 600 1200 2400

Scale in Feet

LEGEND

BASIN ID – SC3–77

DESIGN POINT – 87

REACH IDENTIFIER – RT–17A

BASIN BOUNDARY –

EAST FORK SAND CREEK BASIN BOUNDARY –

FLOW DIRECTION –

BASIN SUMMARY

BASIN	CN	AREA (ACRES)	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)
EX–0	62	23.8	0.037	5.0	8.2	13.0	19.6	25.7	32.2
EX–1	62	25.7	0.040	4.8	7.9	12.4	18.7	24.5	30.9
EX–2	62	5.5	0.009	1.1	1.8	2.8	4.3	5.6	7.1
EX–3	62	136.8	0.214	22.0	36.4	57.6	86.9	114.0	143.1
EX–3A	61	188.1	0.294	28.3	47.4	75.7	115.1	152.2	192.6
EX–4	62	192.0	0.300	30.1	49.9	79.1	119.5	157.0	197.3
EX–4A	62	151.5	0.237	24.7	40.8	64.4	97.0	127.2	160.1
EX–5	62	153.9	0.240	24.2	40.0	63.4	95.9	125.9	158.2
EX–6	62	90.2	0.141	15.3	25.5	40.1	60.7	79.9	100.5
EX–7	56	165.0	0.258	11.6	21.5	37.5	60.9	83.1	107.4
EX–8	45	42.0	0.066	0.5	1.7	4.5	9.4	14.5	20.5
EX–9	54	131.9	0.206	12.2	23.9	43.1	70.9	97.0	125.2
EX–10	60	270.7	0.423	32.7	56.0	91.1	140.1	185.9	236.1
EX–10A	41	179.3	0.280	0.6	2.2	7.3	17.4	29.1	43.1
EX–11	43	209.3	0.327	18.0	29.8	47.7	73.4	98.3	126.1
EX–12	51	39.5	0.062	2.2	5.1	10.1	17.7	25.1	33.3
EX–13	55	89.3	0.139	7.7	15.2	27.1	44.2	60.5	78.4
EX–20	62	143.4	0.224	25.4	42.1	66.7	100.7	132.3	166.2
EX–20A	64	179.7	0.281	32.2	51.9	80.5	119.8	155.9	194.6
EX–21	65	33.3	0.052	8.6	13.5	20.7	30.5	39.4	49.0
EX–24	59	63.1	0.099	9.5	16.6	27.5	42.9	57.4	73.0
EX–25	43	54.4	0.085	0.3	1.5	4.8	10.7	17.2	25.1
EX–73	63	90.0	0.141	16.4	26.4	41.3	62.1	81.3	102.0
EX–74	63	119.7	0.187	22.3	36.5	57.3	85.9	112.3	140.7
EX–75	63	79.3	0.124	13.1	21.5	33.7	50.5	66.1	82.8
EX–76	63	86.4	0.135	14.2	23.1	36.4	54.6	71.4	89.6
EX–77	62	230.6	0.360	34.7	56.9	90.6	137.5	180.9	227.7
EX–78	63	155.6	0.243	28.1	45.3	70.6	106.2	139.1	174.5
EX–79	63	189.0	0.295	34.9	57.0	89.5	134.3	175.6	220.1
EX–80	63	147.7	0.231	27.3	44.3	69.6	104.5	136.8	171.4
EX–81	62	262.9	0.411	42.6	70.2	111.0	167.4	219.6	275.7
EX–82	62	117.8	0.184	20.0	33.2	52.8	80.0	105.1	132.3
EX–88	62	139.2	0.217	22.2	36.7	58.0	87.6	115.0	144.4

DESIGN POINT SUMMARY (PEAK FLOW)

DESIGN POINT	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)	LOCATION
DP–74	0.371	39.3	65.3	104.8	158.9	209.1	262.8	
DP–75	1.413	141.2	235.1	376.6	566.6	750.9	950.5	
DP–78	0.538	59.7	98.4	154.0	232.6	306.2	385.3	
DP–73	2.528	225.9	380.7	618.0	957.0	1260.4	1582.3	
DP–71	2.669	229.3	388.9	629.7	978.8	1277.3	1637.9	STERLING RANCH NORTHERN BNDRY
DP–69	3.209	253.0	434.8	707.7	1100.0	1453.3	1870.4	
DP–63	3.446	251.4	430.7	713.1	1113.2	1496.2	1911.5	STERLING RANCH SOUTHERN BNDRY
DP–10	0.508	36.5	56.0	106.4	162.9	220.6	287.2	COLORADO SPRINGS/EL PASO BNDRY
DP–9A	0.557	55.3	94.3	150.3	227.7	299.5	380.5	VOLLMER/TAHITI DRIVE
DP–9	0.505	52.8	88.8	142.1	214.2	281.0	351.4	VOLLMER/LOCHWINNOCH LN
DP–8A	0.139	7.7	15.2	27.1	44.2	60.5	78.4	D/S STERLING RANCH EASTERN BNDRY
DP–8	0.528	24.2	45.1	77.8	124.4	169.5	220.9	D/S STERLING RANCH SOUTHERN BNDRY
DP–7	0.703	32.4	57.1	97.3	156.1	213.8	277.9	STERLING RANCH SOUTHERN BNDRY
DP–6	0.206	12.2	23.9	43.1	70.9	97.0	125.2	STERLING RANCH SOUTHERN BNDRY
DP–5	0.066	0.5	1.7	4.5	9.4	14.5	20.5	STERLING RANCH SOUTHERN BNDRY
DP–4	0.258	11.6	21.5	37.5	60.9	83.1	107.4	STERLING RANCH SOUTHERN BNDRY
DP–3	0.009	1.1	1.8	2.8	4.3	5.6	7.1	STERLING RANCH SOUTHERN BNDRY
DP–2	0.040	4.8	7.9	12.4	18.7	24.5	30.9	STERLING RANCH SOUTHERN BNDRY
DP–1	0.037	5.0	8.2	13.0	19.6	25.7	32.2	STERLING RANCH SOUTHERN BNDRY
DP–60A	3.545	247.7	430.2	707.1	1113.0	1496.6	1913.5	FUTURE MARKSHEFFEL X–ING
DP–56	0.466	23.2	42.5	71.9	115.6	157.4	202.9	STERLING RANCH SOUTHERN BNDRY
DP–53A	4.138	262.1	454.0	763.2	1196.5	1609.8	2061.5	SAND CREEK AND POND 3

DESIGN POINT SUMMARY (VOLUME)

DESIGN POINT	AREA (SQ MI)	V_2 (AC–FT)	V_5 (AC–FT)	V_{10} (AC–FT)	V_{25} (AC–FT)	V_{50} (AC–FT)	V_{100} (AC–FT)	LOCATION
DP–74	0.371	5.9	9.0	13.6	19.8	25.5	31.6	
DP–75	1.413	22.7	34.5	51.7	75.4	97.1	120.5	
DP–78	0.538	8.9	13.5	20.1	29.3	37.7	46.7	
DP–73	2.528	40.4	61.5	92.1	134.3	173.1	214.9	
DP–71	2.669	42.5	64.9	97.1	141.6	182.5	226.6	STERLING RANCH NORTHERN BNDRY
DP–69	3.209	50.7	77.4	116.1	169.4	218.6	271.4	
DP–63	3.446	54.1	82.5	123.8	180.8	233.3	289.9	STERLING RANCH SOUTHERN BNDRY
DP–10	0.508	7.6	11.7	17.6	25.8	33.4	41.6	COLORADO SPRINGS/EL PASO BNDRY
DP–9A	0.557	9.3	14.1	21.1	30.7	39.4	48.8	VOLLMER/TAHITI DRIVE
DP–9	0.505	8.4	12.7	19.0	27.6	35.5	44.0	VOLLMER/LOCHWINNOCH LN
DP–8A	0.139	1.3	2.1	3.4	5.2	7.0	8.9	D/S STERLING RANCH EASTERN BNDRY
DP–8	0.528	4.4	7.0	11.1	16.8	22.3	28.4	D/S STERLING RANCH SOUTHERN BNDRY
DP–7	0.703	6.1	10.0	15.9	24.3	32.4	41.3	STERLING RANCH SOUTHERN BNDRY
DP–6	0.206	2.4	4.0	6.3	9.6	12.7	16.0	STERLING RANCH SOUTHERN BNDRY
DP–5	0.066	0.2	0.4	0.8	1.4	1.9	2.6	STERLING RANCH SOUTHERN BNDRY
DP–4	0.258	2.6	4.2	6.7	10.2	13.5	17.2	STERLING RANCH SOUTHERN BNDRY
DP–3	0.009	0.1	0.2	0.3	0.5	0.6	0.8	STERLING RANCH SOUTHERN BNDRY
DP–2	0.040	0.6	0.9	1.4	2.1	2.7	3.4	STERLING RANCH SOUTHERN BNDRY
DP–1	0.037	0.6	0.9	1.3	1.9	2.5	3.1	STERLING RANCH SOUTHERN BNDRY
DP–60A	3.545	55.3	84.4	126.4	184.8	238.5	296.6	FUTURE MARKSHEFFEL X–ING
DP–56	0.466	4.0	6.3	9.9	14.9	19.8	25.1	SAND CREEK AND POND 3
DP–53A	4.138	63.0	96.4	144.7	211.8	273.9	340.9	SAND CREEK AND POND 3

EFSC DBPS DESIGN POINT SUMMARY (PEAK FLOW)

DBPS DESIGN POINT	AREA (SQ MI)	Q_{10} (CFS)	Q_{100} (CFS)
DP–50	0.32	47.0	195.7
DP–51 (BASIN 86)	0.33	17.7	74.1
DP–52	1.67	80.5	456.5
DP–56	0.79	63.6	265.0

Values reported from SCDBPS
(DP 50, 51, 52 Not analyzed as a part of this study)
DBPS Reach 85(Basin91)=Q10=28.8cfs Q100=115.2cfs

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2018 STERLING RANCH MDDP

EXISTING HYDROLOGIC CONDITIONS MAP

PROJECT NO. 09–002
FILE: \dwg\Eng Exhibits\2018–MDDP–ExistCondWSWrkMap.dwg
DESIGNED BY: DLM
DRAWN BY: DLM
CHECKED BY: VAS
SCALE
HORIZ: NTS
VERT: NTS
DATE: 08–22–18

DM1

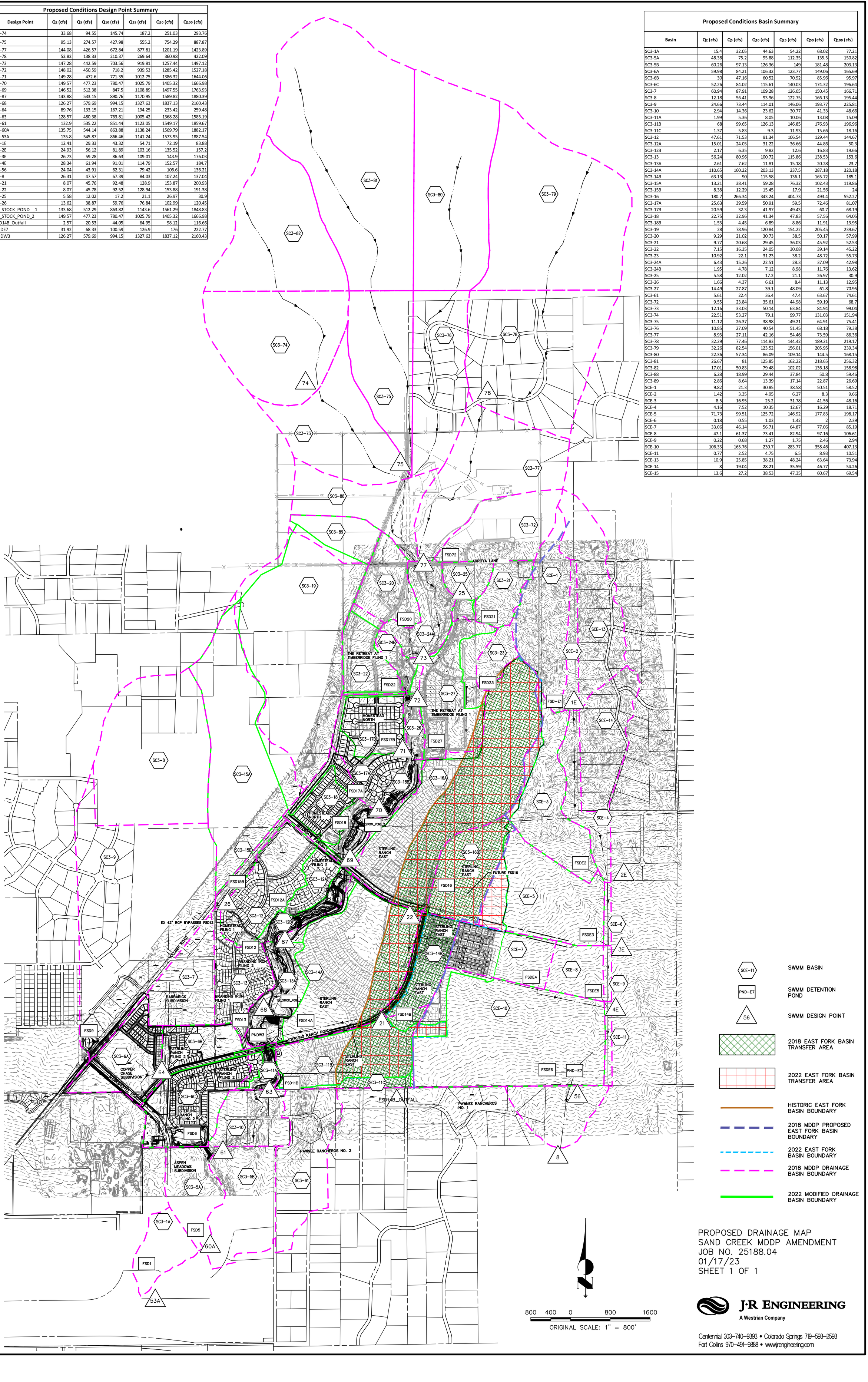
File: O:\09002A\Sterling Ranch District\dwg\Eng Exhibits\2018 MDDP\2018–MDDP–ExistCondWSWrkMap.dwg Plotstamp: 11/13/2018 1:52 PM

Proposed Conditions Design Point Summary

Design Point	Q_2 (cfs)	Q_5 (cfs)	Q_{10} (cfs)	Q_{25} (cfs)	Q_{50} (cfs)	Q_{100} (cfs)
DP-74	33.68	94.55	145.74	187.2	251.03	293.76
DP-75	95.13	274.57	427.98	555.2	754.29	887.87
DP-77	144.08	426.57	672.84	877.81	1201.19	1423.89
DP-78	52.82	138.33	210.37	269.64	360.98	422.09
DP-73	147.28	442.59	703.56	919.81	1257.44	1497.12
DP-72	148.02	450.59	718.2	939.53	1285.42	1527.18
DP-71	149.28	472.6	771.35	1012.75	1386.32	1644.06
DP-70	149.57	477.23	780.47	1025.79	1405.32	1666.98
DP-69	146.52	512.38	847.5	1108.89	1497.55	1763.93
DP-87	143.88	533.15	890.76	1170.95	1589.82	1880.39
DP-68	126.27	579.69	994.15	1327.63	1837.13	2160.43
DP-64	89.76	133.15	167.21	194.25	233.42	259.48
DP-63	128.57	480.38	763.81	1005.42	1368.28	1585.19
DP-61	132.9	535.22	851.44	1123.05	1549.17	1859.67
DP-60A	135.75	544.14	863.88	1138.24	1569.79	1882.17
DP-53A	135.8	545.87	866.46	1141.24	1573.95	1887.54
DP-1E	12.41	29.33	43.32	54.71	72.19	83.88
DP-2E	24.93	56.12	81.89	103.16	135.52	157.2
DP-3E	26.73	59.28	86.63	109.01	143.9	176.03
DP-4E	28.34	61.94	91.01	114.79	152.57	184.7
DP-56	24.04	43.91	62.31	79.42	106.6	136.21
DP-8	26.31	47.57	67.39	84.03	107.24	137.04
DP-21	8.07	45.76	92.48	128.9	153.87	200.93
DP-22	8.07	45.78	92.52	128.94	153.88	191.38
DP-25	5.58	12.02	17.2	21.1	26.97	30.9
DP-26	13.62	38.87	59.76	76.84	102.99	120.45
EX_STOCK_POND _1	133.68	512.29	863.82	1143.6	1561.29	1848.83
EX_STOCK_POND_2	149.57	477.23	780.47	1025.79	1405.32	1666.98
FSD14B_Outfall	2.57	20.53	44.05	64.95	98.12	116.66
PNDE7	31.92	68.33	100.59	126.9	176	222.77
PNDW3	126.27	579.69	994.15	1327.63	1837.12	2160.43

Proposed Conditions Basin Summary

Basin	Q_2 (cfs)	Q_5 (cfs)	Q_{10} (cfs)	Q_{25} (cfs)	Q_{50} (cfs)	Q_{100} (cfs)
SC3-1A	15.4	32.05	44.63	54.22	68.02	77.21
SC3-5A	48.38	75.2	95.88	112.35	135.5	150.82
SC3-5B	60.26	97.13	126.36	149	181.48	203.13
SC3-6A	59.98	84.21	106.32	123.77	149.06	165.69
SC3-6B	30	47.16	60.52	70.92	85.96	95.97
SC3-6C	52.26	84.02	115.61	140.03	174.32	196.64
SC3-7	60.94	87.91	109.28	126.05	150.45	166.71
SC3-8	12.18	56.41	93.96	122.75	166.13	195.44
SC3-9	24.66	73.44	114.01	146.06	193.77	225.81
SC3-10	2.94	14.36	23.62	30.77	41.33	48.66
SC3-11A	1.99	5.36	8.05	10.06	13.08	15.09
SC3-11B	68	99.65	126.13	146.85	176.93	196.96
SC3-11C	1.37	5.83	9.3	11.93	15.66	18.16
SC3-12	47.61	71.53	91.34	106.54	129.44	144.67
SC3-12A	15.01	24.03	31.22	36.66	44.86	50.3
SC3-12B	2.17	6.35	9.82	12.6	16.83	19.66
SC3-13	56.24	80.96	100.72	115.86	138.53	153.6
SC3-13A	2.61	7.62	11.81	15.18	20.28	23.7
SC3-14A	110.65	160.22	203.13	237.5	287.18	320.18
SC3-14B	63.13	90	115.58	136.1	165.72	185.1
SC3-15A	13.21	38.41	59.28	76.32	102.43	119.86
SC3-15B	8.38	12.29	15.45	17.9	21.56	24
SC3-16	180.7	266.34	343.24	404.73	493.4	552.27
SC3-17A	25.63	39.59	50.91	59.5	72.46	81.07
SC3-17B	20.59	32.3	41.97	49.43	60.7	68.19
SC3-18	22.75	32.96	41.34	47.83	57.56	64.05
SC3-18B	1.53	4.45	6.89	8.86	11.91	13.95
SC3-19	28	78.96	120.84	154.22	205.45	239.67
SC3-20	9.29	21.02	30.73	38.5	50.17	57.99
SC3-21	9.77	20.68	29.45	36.03	45.92	52.53
SC3-22	7.15	16.35	24.05	30.08	39.14	45.22
SC3-23	10.92	22.1	31.23	38.2	48.72	55.73
SC3-24A	6.43	15.26	22.51	28.3	37.09	42.98
SC3-24B	1.95	4.78	7.12	8.98	11.76	13.62
SC3-25	5.58	12.02	17.2	21.1	26.97	30.9
SC3-26	1.66	4.37	6.61	8.4	11.13	12.95
SC3-27	14.49	27.87	39.1	48.09	61.8	70.95
SC3-61	5.61	22.4	36.4	47.4	63.67	74.61
SC3-72	9.55	23.84	35.61	44.98	59.19	68.7
SC3-73	12.16	33.03	50.14	63.84	84.94	99.04
SC3-74	22.51	53.27	79.1	99.77	131.03	151.94
SC3-75	11.12	26.37	38.98	49.21	64.91	75.41
SC3-76	10.85	27.09	40.54	51.45	68.18	79.38
SC3-77	8.93	27.11	42.16	54.46	73.59	86.36
SC3-78	32.29	77.46	114.83	144.42	189.21	219.17
SC3-79	32.26	82.54	123.52	156.01	205.95	239.34
SC3-80	22.36	57.34	86.09	109.14	144.5	168.15
SC3-81	26.67	81	125.85	162.22	218.65	256.32
SC3-82	17.01	50.83	79.48	102.02	136.18	158.98
SC3-88	6.28	18.99	29.44	37.84	50.8	59.46
SC3-89	2.86	8.64	13.39	17.14	22.87	26.69
SCE-1	9.82	21.3	30.85	38.58	50.51	58.52
SCE-2	1.42	3.35	4.95	6.27	8.3	9.66
SCE-3	8.5	16.95	25.2	31.78	41.56	48.16
SCE-4	4.16	7.52	10.35	12.67	16.29	18.71
SCE-5	71.73	99.51	125.72	146.92	177.83	198.17
SCE-6	0.18	0.55	1.03	1.42	2	2.39
SCE-7	33.06	46.14	56.71	64.87	77.06	85.19
SCE-8	47.1	61.37	73.41	82.94	97.16	106.61
SCE-9	0.22	0.68	1.27	1.75	2.46	2.94
SCE-10	106.33	165.76	230.7	283.77	358.46	407.13
SCE-11	0.77	2.52	4.75	6.5	8.93	10.51
SCE-13	10.9	25.85	38.21	48.24	63.64	73.94
SCE-14	8	19.04	28.21	35.59	46.77	54.26
SCE-15	13.6	27.2	38.53	47.35	60.67	69.54



PROPOSED DRAINAGE MAP
SAND CREEK MDDP AMENDMENT
JOB NO. 25188.04
01/17/23
SHEET 1 OF 1

J·R ENGINEERING
A Westrian Company

Centennial 303-740-9393 • Colorado Springs 719-593-2593
Fort Collins 970-491-9888 • www.jrengineering.com

Marksheffel Road – Segment M3 CDRs

Final Drainage Report

PCD File No: CDR2413

All Terrain Engineering Project No: 24013

April 2026

PREPARED FOR:

Turkey Canon Quarry Inc

Contact: James Morley

20 Boulder Crescent Street, Suite 200

Colorado Springs, CO 80903

PREPARED BY:

All Terrain Engineering LLC

Contact: Ryan Burns

rburns@allterraineng.com

DBPS, reach RUC160 runs west of the site and has been identified as being in stable condition. Cottonwood Creek was also studied in the June 1994, "Cottonwood Creek Drainage Basin Planning Study" by URS Consultants, and identified the on-site reach as the segment between Design Points 6 & 7. The study characterized the reach as a naturalized channel with ecological value including wetlands and riparian vegetation.

The channel will be analyzed for stability and required improvements with the forthcoming Schmidt PUD and Preliminary Plan and any required creek improvements will be identified with that project.

The Sand Creek Basin was studied many times over the years by various companies and in various reports. One of the most recent reports adopted by the County is the "Master Development Drainage Plan Amendment for Sterling Ranch" prepared by JR Engineering in March 2023. This report did not include the subject site in the in the Sand Creek Basin, and neither did the original 2018 "Sterling Ranch Master Development Drainage Plan" prepared by M&S in 2018. Although not adopted by the County, the site was partially included in the "Sand Creek Drainage Basin Planning Study" (Sand DBPS) completed by Stantec in January 2021, however it only showed approximately 17 acres of the eastern most portion of the site as tributary to Sand Creek. The Stantec Sand Creek DBPS assumed the Schmidt Parcel property to have an "Open Space" use which is consistent with the property at this time and the Basin line they showed, is generally consistent with the existing site topography.

b. Existing Subbasin Description

The existing condition describes the current state of the site after the early grading for the Schmidt Parcel. The existing condition consists of six on-site basins and one off-site basin. The existing sub-basin delineation is shown on the drainage map within Appendix F and is described as follows:

Basin OSI4 is 27.16 acres of Silver Ponds Subdivision Filing 1 & the existing Colorado Springs R.O.W. reserved for the future Marksheffel Road extension. Values for this basin were taken from "Silver Ponds Subdivision Filing No.1 Final Drainage Report". Runoff from this basin (Q_5=19.0 cfs, Q_{100}=44.2 cfs) flows south and is intercepted by the existing swale/berm that directs runoff around the site. Flows are diverted to the west towards DPI4 and enter Cottonwood Creek.

Basin EXA is 0.25 acres of an existing berm with stabilized vegetation. Runoff from this basin (Q_5=0.0 cfs, Q_{100}=0.3 cfs) flows along the berm west and enters an existing swale at DP1 and flows offsite and into basin OSI4. Flows in the existing swale combine with a portion of Basin OSI4 at DP14 (Q_5 = 19.0 cfs, Q_{100} = 44.2 cfs) and direct flows to the west towards Cottonwood Creek.

Basin EXB is 1.51 acres of an existing berm with stabilized vegetation, and includes a portion of the "Tahiti Road" R.O.W. Although the R.O.W. exists, the road is shut down and no longer active and generally consists of only a dirt 2-track or varying width drive. Runoff from this basin (Q_5=0.6 cfs, Q_{100}=2.4 cfs) flows east along the berm and enters existing dual 48" RCP culverts at DP2. Culvert flows are directed south along Vollmer Road. There is no drainage infrastructure downstream, therefore; runoff that enters Vollmer Road right-of-way (R.O.W.) flows per existing drainage patterns southeast towards Sand Creek.

Basin EXC is 11.5 acres of native and stabilized vegetation except for a temporary vehicle tracking pad and access that was installed with the CDR22-007 project. Runoff from this basin (Q_5=2.0 cfs, Q_{100}=13.2 cfs) flows overland southeast and enters an existing swale also installed with the CDR22-007 project. The existing swale enters Basin EXD at DP3 and follows the drainage patterns of that Basin EXD.

Basin EXD is 21.8 acres of native and stabilized vegetation. Runoff from this basin (Q_5=3.7 cfs, Q_{100}=23.3 cfs) flows overland southwest and enters an existing swale at DP4. Flows from DP3 and DP4 combine at DP4.1 (Q_5=5.5 cfs, Q_{100}=35.6 cfs) and are conveyed via an existing swale through Basin EX-G and towards the existing Temporary Sediment Basin. The sediment basin outfalls to Cottonwood Creek at DP7.1. See Basin EXG description below.

Basin EXE is 3.96 acres of undeveloped land with native vegetation. Runoff from this basin (Q_5=0.9 cfs, Q_{100}=5.7 cfs) flows overland southeast to DP5, where flow enters Vollmer Road R.O.W. There is no drainage infrastructure downstream, therefore; runoff that enters Vollmer Road R.O.W flows per existing drainage patterns southeast towards Sand Creek.

Basin EXF is 2.58 acres of native and stabilized vegetation. Runoff from this basin (Q_5=0.6 cfs, Q_{100}=4.1 cfs) flows overland south to DP6. Basin EXF overland flows south to Trails at Forest Meadows Filing No. 3. Basin EXF flow was accounted for in "Trails at Forest Meadows Filing No. 3 Final Drainage Report" (Trails No. 3 FDR) as Basins OS2 and OS3. Basin OS2 and OS3 total 1.56 acres and have a total flow of Q_5=1.0 cfs and Q_{100}=3.6 cfs.

Basin EXG is 39.7 acres of native and stabilized vegetation, although, disturbance from the CDR22-007 project may be present at the time of writing this report. Runoff from this basin (Q_5=4.9 cfs, Q_{100}=32.9 cfs) flows overland southwest and enters an existing sediment basin installed with the CDR22-007 project, at DP7. Flows are then either released through the sediment basin outlet, to DP7.1 or over-top the spillway, and flow to DP7.1 (Q5 = 9.2 cfs, Q100 = 60.8 cfs), where they combine with flows from DP4.1 and enter Cottonwood Creek and continue off-site. Total flow in Cottonwood Creek, including the site flows, is 854 CFS per the "Cottonwood Creek DBPS" by URS, dated 1994.

Historically, this basin flowed to a low point in roughly the same location as the existing sediment basin and flows either infiltrated or over-topped the low-point and entered Cottonwood Creek. The site was previously an active quarry, and therefore the drainage patterns were altered with mining activities over the years. Construction of the Trails at Forest Meadows development to the south stated that the construction company running the mine constructed a large berm to ensure flows did not enter the Trails Subdivision and instead were diverted west.

Basin EXI is 10.3 acres of native and stabilized vegetation, including wetland and riparian vegetation within the Cottonwood Creek Flood Plain. Runoff from this basin (Q_5=2.4 cfs, Q_{100}=16.3 cfs) flows overland south and towards Cottonwood Creek. Flows enter the Creek and combine with flows from upstream and DP7.1 flows and continue south off-site. Total flow in Cottonwood Creek, including the site flows, is 854 CFS per the "Cottonwood Creek DBPS" by URS, dated 1994.

Basin EXJ is 0.33 acres of native and stabilized vegetation and includes just a small sliver of land from the southern property line to the top of a small berm that keeps the majority of the site flows from entering the "Trails at Forest Meadows Filing No. 4 Subdivision. Runoff from this basin (Q_5=0.1 cfs, Q_{100}=0.5cfs) flows overland south to DP10. Flows are distributed over the length of the basin, and are not concentrated. This area was analyzed as part of Trails at Forest Meadows Filing No. 4 drainage report and was delineated as basins OS4 and Basin OS2 which had a combined flow of Q5 = 1.0 cfs & Q100 = 4.7 cfs.

c. Proposed Subbasin Description

Proposed Sub-basin Drainage

The proposed condition consists of 13 on-site basins and 1 off-site basin. The proposed sub-basin delineation is shown on the drainage map within Appendix F and is described as follows:

Basin OSI4 is 27.16 acres of Silver Ponds Subdivision Filing 1. Values for this basin were taken from "Silver Ponds Subdivision Filing No.1 Final Drainage Report". Runoff from this basin (Q_5=19.0 cfs, Q_{100}=44.2 cfs) flows south and is intercepted by the existing swale/berm that directs runoff around the site. Flows are diverted to the west towards DP14 and enter Cottonwood Creek. Basin OSI4 will not be captured nor detained in Pond A. No exclusion is necessary for Basin OSI4 as it is an offsite, undisturbed basin shown for reference only.

Basin A is 0.25 acres of an existing berm with stabilized vegetation. Runoff from this basin (Q_5=0.0 cfs, Q_{100}=0.2 cfs) flows along the berm west and enters an existing swale at DP1 and enters an existing swale at DP1 and flows offsite and into basin OSI4.Flows in the existing swale combine with a portion of Basin OSI4 at DP14 (Q_5 = 19.0 cfs, Q_{100} = 44.2 cfs) and direct flows west to Cottonwood Creek. Basin A will not be captured or detained in Pond A. Water quality is not required for Basin A as it will remain in its existing condition and will not be disturbed.

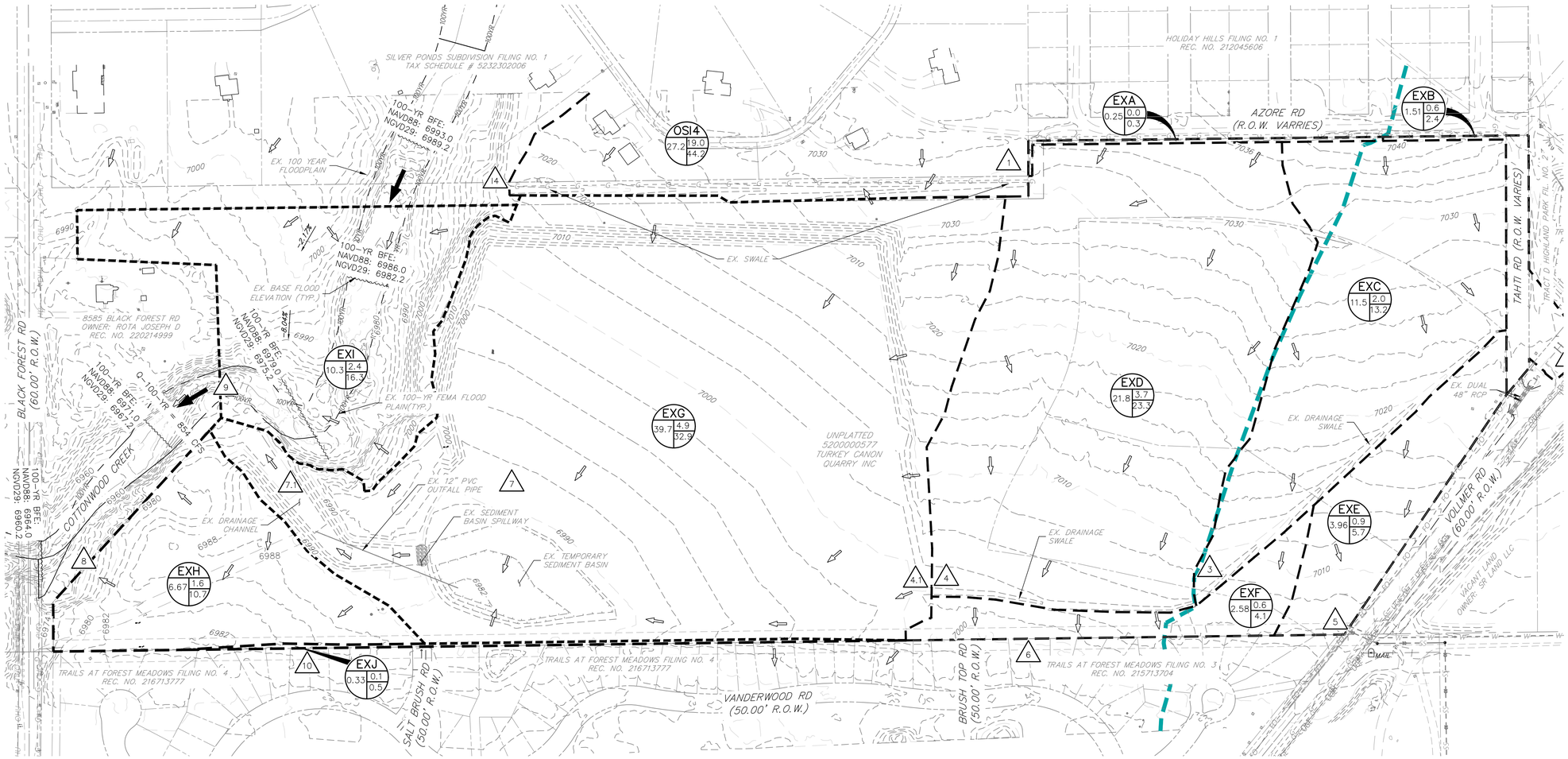
Basin B is 0.09 acres of an existing berm with stabilized vegetation. Runoff from this basin (Q_5=0.0 cfs, Q_{100}=0.1 cfs) flows along the berm east and enters an existing pond adjacent to Vollmer Road. Basin B will follow historic drainage patterns. Water quality is not required for Basin B as it will remain in its existing condition and will not be disturbed.

Basin C is 1.12 acres of undeveloped land and a portion of the north side of Marksheffel Roadway. Runoff from this basin (Q_5=1.6 cfs, Q_{100}=4.4 cfs) flows overland to the south to the CS Type 1 C&G which transports flows west to a 5' Type R sump inlet (CS Type 2 Inlet) at DP3. All flows are captured by the inlet and are piped to DP4.1 where they combine with flows Captured at DP4. This inlet was sized to capture all flow in the 5 and 100-year storm and in the event of inlet failure at DP3, the flow will overtop the roadway crown and flow either to the inlet at DP4 or south in Brushtop Road curb and gutter. See Basin D description below. WQ and detention will be provided for this basin in Interim Pond A.

Basin D is 1.18 acres of proposed Marksheffel Road and proposed sidewalk. Runoff from this basin (Q_5=1.8 cfs, Q_{100}=4.0 cfs) flows toward the CS Type 1 C&G and are transported west to a 5' Type R sump inlet (CS Type 2 Inlet at DP4. All flows at DP4 are captured and piped to DP4.1(Q5 = 3.2 cfs, Q100 = 7.9 cfs), where they combine with DP3 flows. This inlet was sized to capture all flow in the 5 and 100-year storm. In the event of inlet failure at DP4, the flow will overtop the roadway crown and flow south in Brushtop Road curb and gutter.

SCHMIDT PUD

PREDEVELOPMENT CONDITIONS DRAINAGE MAP



LEGEND

	EXISTING	PROPOSED
SECTION LINE		
BOUNDARY LINE		
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
CENTERLINE		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
INDEX CONTOUR	6100	6100
INTERMEDIATE CONTOUR		
FLOW DIRECTION		

BASIN ID — ID | AC | Q5 | Q100

DESIGN POINT DESIGNATION — 1

SUB-BASIN DRAINAGE AREA

SAND CREEK/COTTONWOOD CREEK DRAINAGE BASIN LINE

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
EXA	0.25	2%	0.09	0.36	51.8	0.0	0.3
EXB	1.51	16%	0.18	0.42	34.8	0.6	2.4
EXC	11.55	2%	0.09	0.36	44.5	2.0	13.2
EXD	21.78	3%	0.10	0.36	48.7	3.7	23.3
EXE	3.96	2%	0.09	0.36	31.9	0.9	5.7
EXF	2.58	2%	0.09	0.36	27.1	0.6	4.1
EXG	39.67	2%	0.09	0.36	62.7	4.9	32.9
EXH	6.67	2%	0.09	0.36	26.6	1.6	10.7
EXI	10.29	2%	0.09	0.36	27.3	2.4	16.3
EXJ	0.33	2%	0.09	0.36	25.7	0.1	0.5
OSI4	27.16	-	0.30	0.40	29.2	19.0	44.2

Values in BLUE indicate they are from the "Silver Ponds Subdivision Filing No. 1 Final Drainage Report", by M.V.E. Inc. revised May 5th, 1996.

DESIGN POINT SUMMARY TABLE

DP#	$Q_{5\text{-}YR}$	$Q_{100\text{-}YR}$
1	0.0	0.3
2	0.6	2.4
3	2.0	13.2
4	3.7	23.3
4.1	5.5	35.6
5	0.9	5.7
6	0.6	4.1
7	4.9	32.9
7.1	9.2	60.8
8	1.6	10.7
9	2.4	16.3
10	0.1	0.5
I4	19.0	44.2

Values in BLUE indicate they are from the "Silver Ponds Subdivision Filing No. 1 Final Drainage Report", by M.V.E. Inc. revised May 5th, 1996.

N

150 75 0 150 300

ORIGINAL SCALE: 1" = 150'

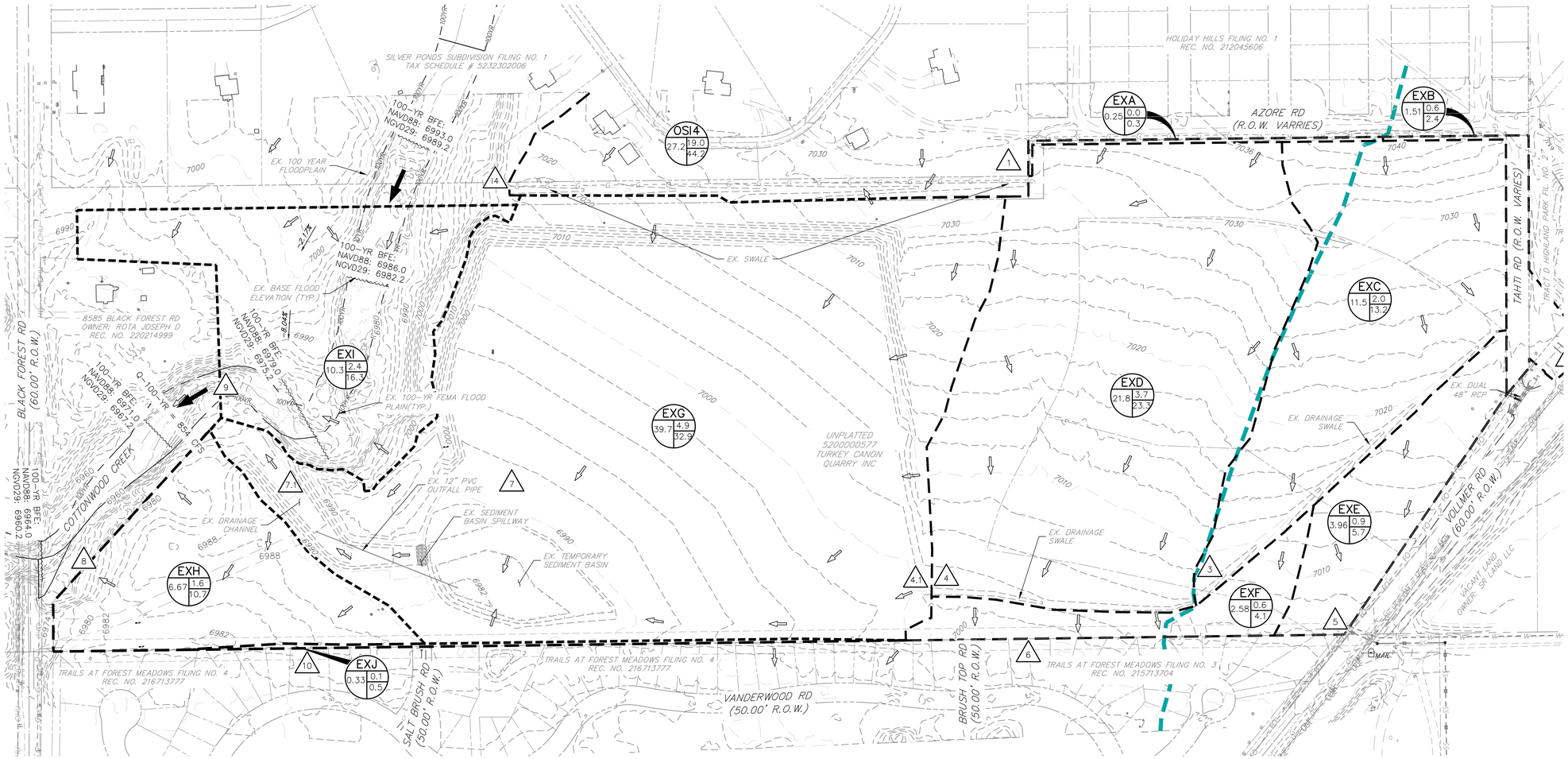
SCHMIDT PUD	
PREDEV. CONDITIONS DRAINAGE MAP	
JOB NO. 24013	SHEET 1
LOCATION: EPC	
06/09/2026	

ALL TERRAIN ENGINEERING

APPENDIX F – DRAINAGE MAPS

SCHMIDT PUD

PREDEVELOPMENT CONDITIONS DRAINAGE MAP



LEGEND

	EXISTING	PROPOSED
SECTION LINE		
BOUNDARY LINE		
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
CENTERLINE		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
INDEX CONTOUR	6100	6100
INTERMEDIATE CONTOUR		
FLOW DIRECTION		

BASIN ID — ID | AC | Q5 | Q100

DESIGN POINT DESIGNATION — 1

SUB-BASIN DRAINAGE AREA

SAND CREEK/COTTONWOOD CREEK DRAINAGE BASIN LINE

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
EXA	0.25	2%	0.09	0.36	51.8	0.0	0.3
EXB	1.51	16%	0.18	0.42	34.8	0.6	2.4
EXC	11.55	2%	0.09	0.36	44.5	2.0	13.2
EXD	21.78	3%	0.10	0.36	48.7	3.7	23.3
EXE	3.96	2%	0.09	0.36	31.9	0.9	5.7
EXF	2.58	2%	0.09	0.36	27.1	0.6	4.1
EXG	39.67	2%	0.09	0.36	62.7	4.9	32.9
EXH	6.67	2%	0.09	0.36	26.6	1.6	10.7
EXI	10.29	2%	0.09	0.36	27.3	2.4	16.3
EXJ	0.33	2%	0.09	0.36	25.7	0.1	0.5
OSI4	27.16	-	0.30	0.40	29.2	19.0	44.2

Values in BLUE indicate they are from the "Silver Ponds Subdivision Filing No. 1 Final Drainage Report", by M.V.E. Inc. revised May 5th, 1996.

DESIGN POINT SUMMARY TABLE

DP#	$Q_{5\text{-}YR}$	$Q_{100\text{-}YR}$
1	0.0	0.3
2	0.6	2.4
3	2.0	13.2
4	3.7	23.3
4.1	5.5	35.6
5	0.9	5.7
6	0.6	4.1
7	4.9	32.9
7.1	9.2	60.8
8	1.6	10.7
9	2.4	16.3
10	0.1	0.5
I4	19.0	44.2

Values in BLUE indicate they are from the "Silver Ponds Subdivision Filing No. 1 Final Drainage Report", by M.V.E. Inc. revised May 5th, 1996.

N

150 75 0 150 300

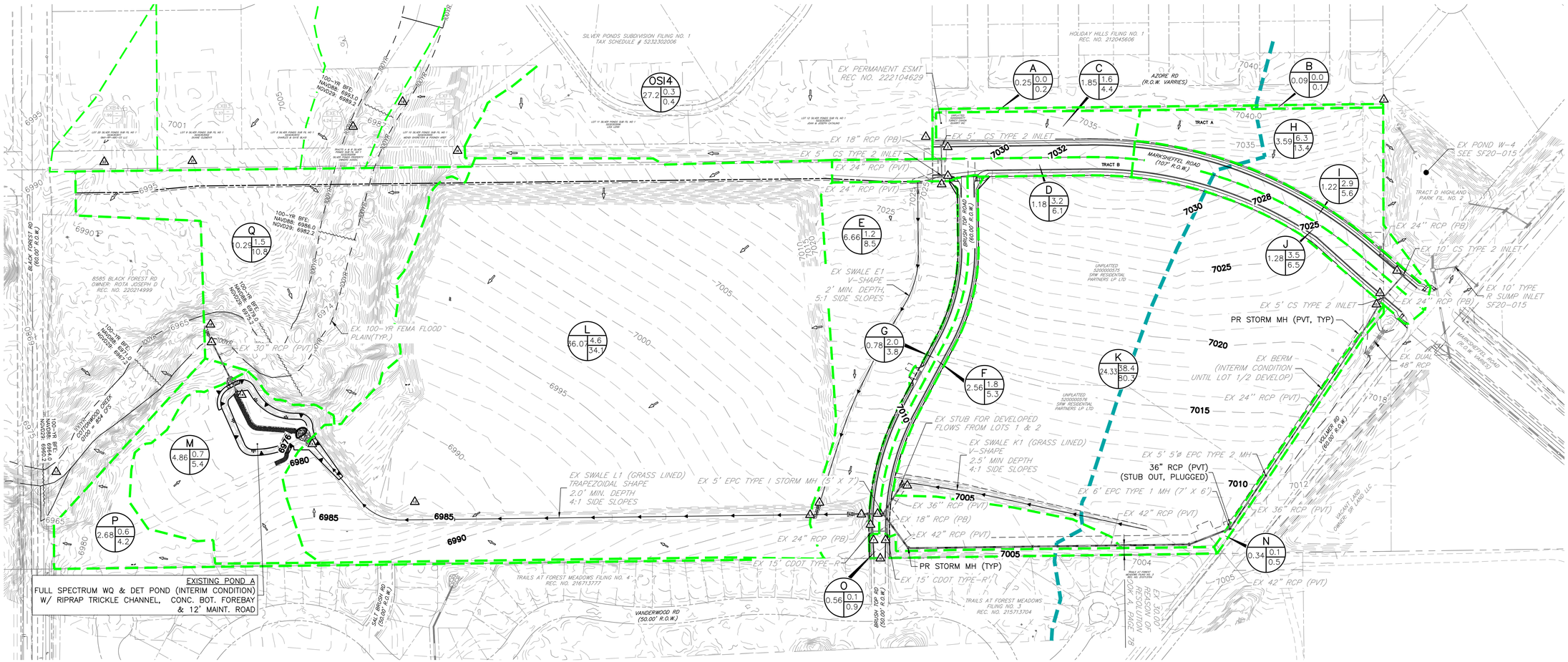
ORIGINAL SCALE: 1" = 150'

SCHMIDT PUD	
PREDEV. CONDITIONS DRAINAGE MAP	
JOB NO. 24013	SHEET 1
LOCATION: EPC	
06/09/2026	

ALL TERRAIN ENGINEERING

SCHMIDT PUD

EXISTING CONDITIONS DRAINAGE MAP



	EXISTING	PROPOSED
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
CONTOUR	6100	6100
BASIN LIMITS		
FLOW DIRECTION		

BASIN ID: ID / AC / Q5 / Q100

DESIGN POINT DESIGNATION: 1

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A	0.25	0%	0.08	0.35	53.0	0.0	0.2
B	0.09	0%	0.08	0.35	36.1	0.0	0.1
C	1.85	77%	0.61	0.73	17.2	3.7	7.5
D	1.18	86%	0.78	0.87	15.1	3.2	6.1
E	6.66	0%	0.08	0.35	36.8	1.2	8.5
F	2.56	25%	0.29	0.50	29.5	1.8	5.4
G	0.78	85%	0.78	0.87	16.9	2.0	3.8
H	3.59	72%	0.51	0.65	15.9	6.3	13.4
I	1.22	80%	0.73	0.84	17.5	2.9	5.6
J	1.28	87%	0.79	0.88	16.0	3.5	6.5
K	24.33	75%	0.53	0.66	30.3	31.8	66.5
L	36.07	0%	0.08	0.35	53.6	4.6	34.1
M	4.86	0%	0.08	0.35	44.3	0.7	5.4
N	0.34	0%	0.08	0.35	26.1	0.1	0.5
O	0.56	0%	0.08	0.35	26.7	0.1	0.9
P	2.68	0%	0.08	0.35	26.1	0.6	4.2
Q	10.29	0%	0.08	0.35	47.7	1.5	10.8

DESIGN POINT SUMMARY TABLE

DP#	$Q_{5\text{-YR}}$	$Q_{100\text{-YR}}$
1	0.0	0.2
2	0.0	0.1
3	3.7	7.5
4	3.2	6.1
4.1	6.7	13.7
5	5.6	17.2
5.1	44.8	99.4
6	1.8	5.4
7	2.0	3.8
7.1	3.4	8.3
9	2.9	18.5
10	3.5	6.5
10.1	12.3	24.6
11	31.8	66.5
11.1	40.9	84.8
11.2	44.2	92.8
13	4.6	34.1
13.1	37.8	107.6
14	38.4	112.2
14.1	0.1	40.9
15	0.1	0.5
16	0.1	0.9
17	0.6	4.2
18	1.5	10.8
19	6.6	14.4
20	5.5	12.1
21	35.7	83.1
22	39.2	89.8
14	19.0	44.2

Values in BLUE indicate they are from the "Silver Ponds Subdivision Filing No. 1 Final Drainage Report", by M.V.E. Inc. revised May 5th, 1996.

NOTES:
1. SILVER PONDS F1 DRAINAGE BASINS SHOWN FOR REFERENCE ONLY. REFER TO SILVER PONDS F1 FDR FOR HYDROLOGIC DETAIL.

N

150 75 0 150 300

ORIGINAL SCALE: 1" = 150'

SCHMIDT PUD

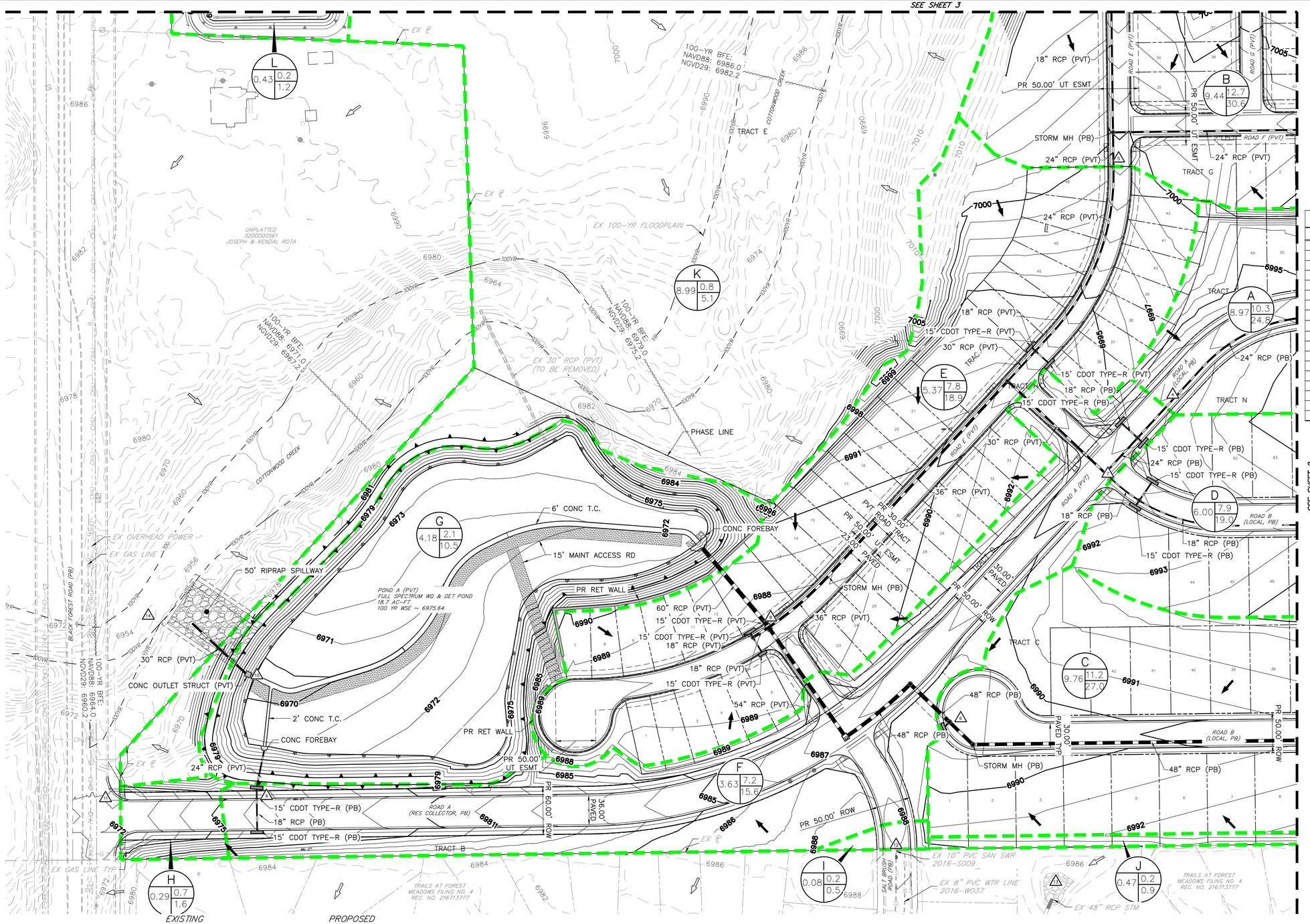
EXISTING CONDITIONS DRAINAGE MAP

JOB NO. 24013 | SHEET 1

LOCATION: EPC

06/12/2026

ALL TERRAIN ENGINEERING



NOTES:

1. SEE SCHMIDT SEGMENT 3 FDR FOR DRAINAGE BASINS EAST OF BRUSH TOP ROAD. EPC PROJECT CDR2413.
2. EXISTING BASIN TAGS SHOWN FOR REFERENCE ONLY. REFER TO APPENDIX E FOR EXISTING DRAINAGE REPORT REFERENCE MATERIAL & CALCULATIONS.

PROPOSED CONDITIONS - BASIN SUMMARY TABLE

Sub-basin	Area (ac)	Impervious	C_5	C_{100}	t_c (min)	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
MS1	0.28	79%	0.73	0.83	5.4	1.0	2.0
MS2	1.76	64%	0.60	0.74	10.4	4.3	8.9
MS3	0.34	83%	0.76	0.85	5.2	1.3	2.5
MS4	1.41	81%	0.74	0.84	9.2	4.4	8.5
MS5	0.82	64%	0.60	0.74	6.0	2.4	5.0
MS6	2.57	76%	0.70	0.81	11.4	7.1	13.8
A	8.97	65%	0.41	0.59	24.3	10.3	24.8
B	9.44	65%	0.41	0.59	17.6	12.7	30.6
C	9.76	65%	0.41	0.59	24.4	11.2	27.0
D	6.00	65%	0.41	0.59	18.7	7.9	19.0
E	5.37	65%	0.41	0.59	14.7	7.8	18.9
F	3.63	56%	0.54	0.69	13.3	7.2	15.6
G	4.18	7%	0.13	0.39	12.0	2.1	10.5
H	0.29	53%	0.51	0.67	5.6	0.7	1.6
I	0.08	63%	0.60	0.74	5.0	0.2	0.5
J	0.47	12%	0.18	0.42	26.0	0.2	0.9
K	8.99	2%	0.09	0.36	86.4	0.7	4.8
L	0.43	2%	0.09	0.36	7.0	0.2	1.2

DESIGN POINT SUMMARY TABLE

Design Point	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
I4	19.0	44.2
EX3	1.0	2.0
EX4	1.3	2.5
EX4.1	6.7	13.1
EX11.2	44.2	92.8
1	4.3	8.9
2	8.5	17.0
3	9.2	18.9
4	10.3	24.8
5	26.3	57.4
6	54.0	116.5
7	17.2	41.5
8	94.2	208.9
9	7.2	15.6
10	2.1	10.5
11	0.7	1.6
12	0.2	0.5
13	0.2	0.9
14	3.5	58.1

	EXISTING	PROPOSED
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
CONTOUR	6100	6100
BASIN LIMITS		
FLOW DIRECTION		

BASIN ID: ID / AC / Q5 / Q100

DESIGN POINT DESIGNATION: 1

60 30 0 60 120

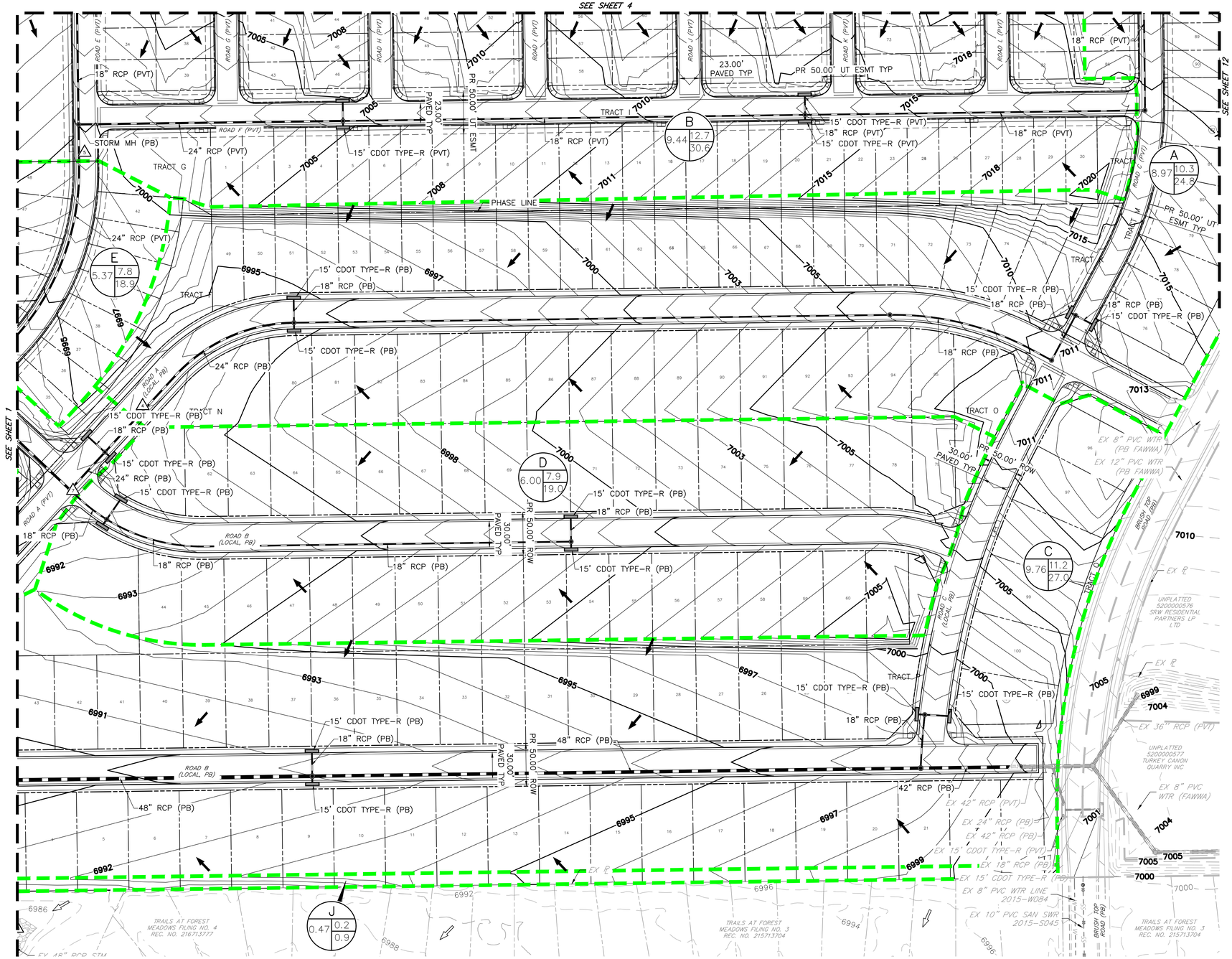
ORIGINAL SCALE: 1" = 60'

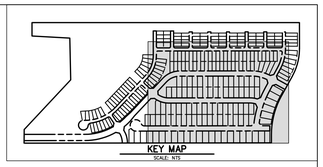
PROPOSED CONDITIONS DRAINAGE MAP

SCHMIDT PUD

JOB NO. 24013
LOCATION: EPC
05/12/2026
SHEET 1

ALL TERRAIN ENGINEERING





NOTES:

1. SEE SCHMIDT SEGMENT 3 FDR FOR DRAINAGE BASINS EAST OF BRUSH TOP ROAD. EPC PROJECT CDR2413.
2. EXISTING BASIN TAGS SHOWN FOR REFERENCE ONLY. REFER TO APPENDIX E FOR EXISTING DRAINAGE REPORT REFERENCE MATERIAL & CALCULATIONS.

PROPOSED CONDITIONS - BASIN SUMMARY TABLE

Sub-basin	Area (ac)	Impervious	C_5	C_{100}	t_c (min)	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
MS1	0.28	79%	0.73	0.83	5.4	1.0	2.0
MS2	1.76	64%	0.60	0.74	10.4	4.3	8.9
MS3	0.34	83%	0.76	0.85	5.2	1.3	2.5
MS4	1.41	81%	0.74	0.84	9.2	4.4	8.5
MS5	0.82	64%	0.60	0.74	6.0	2.4	5.0
MS6	2.57	76%	0.70	0.81	11.4	7.1	13.8
A	8.97	65%	0.41	0.59	24.3	10.3	24.8
B	9.44	65%	0.41	0.59	17.6	12.7	30.6
C	9.76	65%	0.41	0.59	24.4	11.2	27.0
D	6.00	65%	0.41	0.59	18.7	7.9	19.0
E	5.37	65%	0.41	0.59	14.7	7.8	18.9
F	3.63	56%	0.54	0.69	13.3	7.2	15.6
G	4.18	7%	0.13	0.39	12.0	2.1	10.5
H	0.29	53%	0.51	0.67	5.6	0.7	1.6
I	0.08	63%	0.60	0.74	5.0	0.2	0.5
J	0.47	12%	0.18	0.42	26.0	0.2	0.9
K	8.99	2%	0.09	0.36	86.4	0.7	4.8
L	0.43	2%	0.09	0.36	7.0	0.2	1.2

DESIGN POINT SUMMARY TABLE

Design Point	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
I4	19.0	44.2
EX3	1.0	2.0
EX4	1.3	2.5
EX4.1	6.7	13.1
EX11.2	44.2	92.8
1	4.3	8.9
2	8.5	17.0
3	9.2	18.9
4	10.3	24.8
5	26.3	57.4
6	54.0	116.5
7	17.2	41.5
8	94.2	208.9
9	7.2	15.6
10	2.1	10.5
11	0.7	1.6
12	0.2	0.5
13	0.2	0.9
14	3.5	58.1

	EXISTING	PROPOSED
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
CONTOUR	6100	6100
BASIN LIMITS		
FLOW DIRECTION		

BASIN ID — ID / AC / Q5 / Q100

DESIGN POINT DESIGNATION — 1

60 30 0 60 120

ORIGINAL SCALE: 1" = 60'

PROPOSED CONDITIONS DRAINAGE MAP

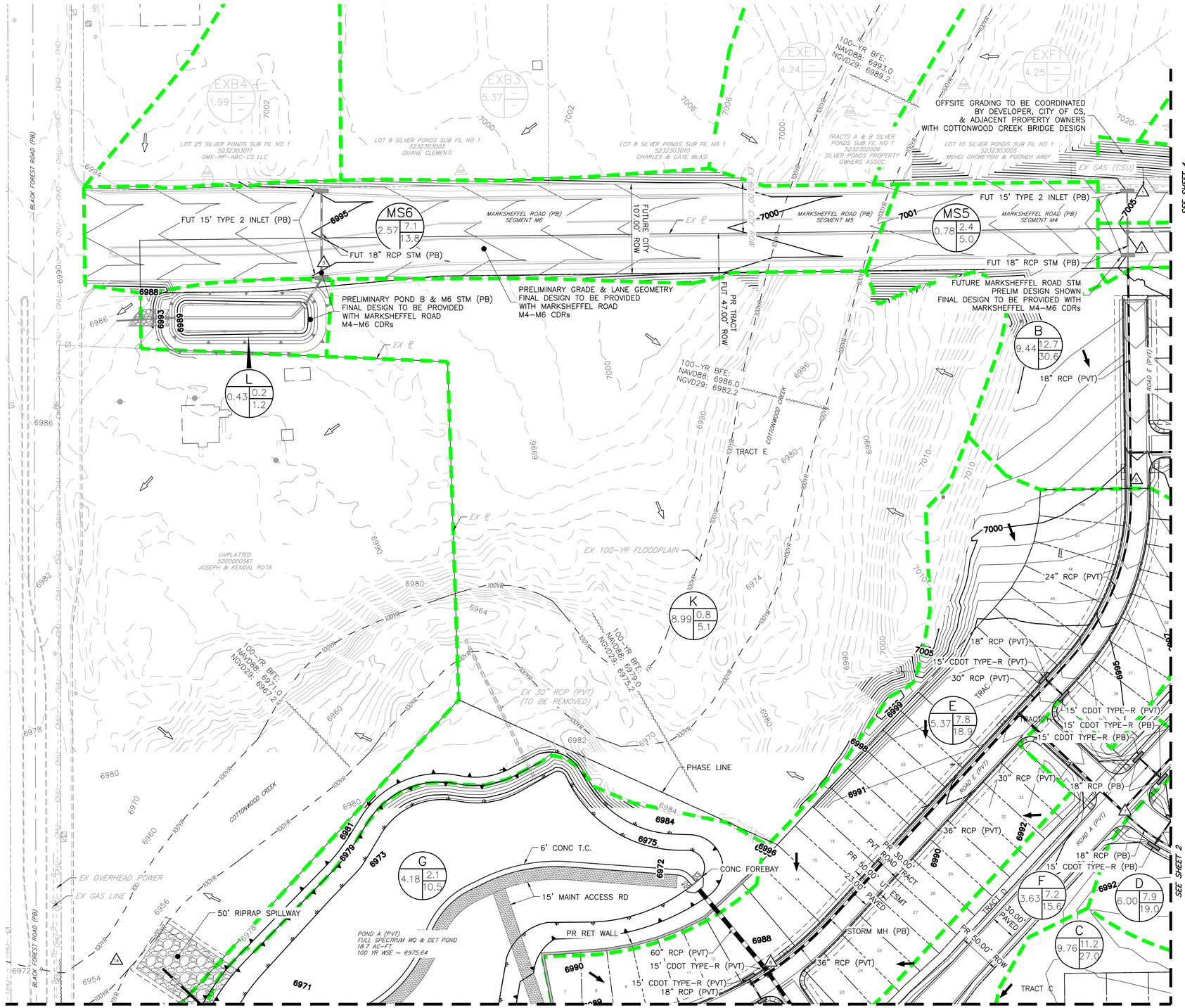
SCHMIDT PUD

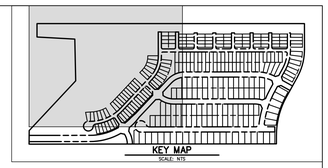
JOB NO. 24013 | SHEET 1

LOCATION: EPC

06/12/2026

ALL TERRAIN ENGINEERING





NOTES:

1. SEE SCHMIDT SEGMENT 3 FDR FOR DRAINAGE BASINS EAST OF BRUSH TOP ROAD. EPC PROJECT CDR2413.
2. EXISTING BASIN TAGS SHOWN FOR REFERENCE ONLY. REFER TO APPENDIX E FOR EXISTING DRAINAGE REPORT REFERENCE MATERIAL & CALCULATIONS.

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MS6	2.57	76%	0.70	0.81	11.4	7.1	13.8
A	8.97	65%	0.41	0.59	24.3	10.3	24.8
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C	9.76	65%	0.41	0.59	24.4	11.2	27.0
D	6.00	65%	0.41	0.59	18.7	7.9	19.0
E	5.37	65%	0.41	0.59	14.7	7.8	18.9
F	3.63	56%	0.54	0.69	13.3	7.2	15.6
G	4.18	7%	0.13	0.39	12.0	2.1	10.5
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K	8.99	2%	0.09	0.36	86.4	0.7	4.8
L	0.43	2%	0.09	0.36	7.0	0.2	1.2

DESIGN POINT SUMMARY TABLE

Design Point	$Q_{5\text{-YR}}$ (cfs)	$Q_{100\text{-YR}}$ (cfs)
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EX3	1.0	2.0
EX4	1.3	2.5
EX4.1	6.7	13.1
EX11.2	44.2	92.8
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13	0.2	0.9
14	3.5	58.1

	EXISTING	PROPOSED
PROPERTY LINE		
EASEMENT LINE		
RIGHT OF WAY		
STORM SEWER		
SWALE/WATERWAY FLOWLINE		
CONTOUR	6100	6100
BASIN LIMITS / FLOW DIRECTION		

BASIN ID: ID / AC / Q5 / Q100

DESIGN POINT DESIGNATION: 1

60 30 0 60 120

ORIGINAL SCALE: 1" = 60'

PROPOSED CONDITIONS DRAINAGE MAP

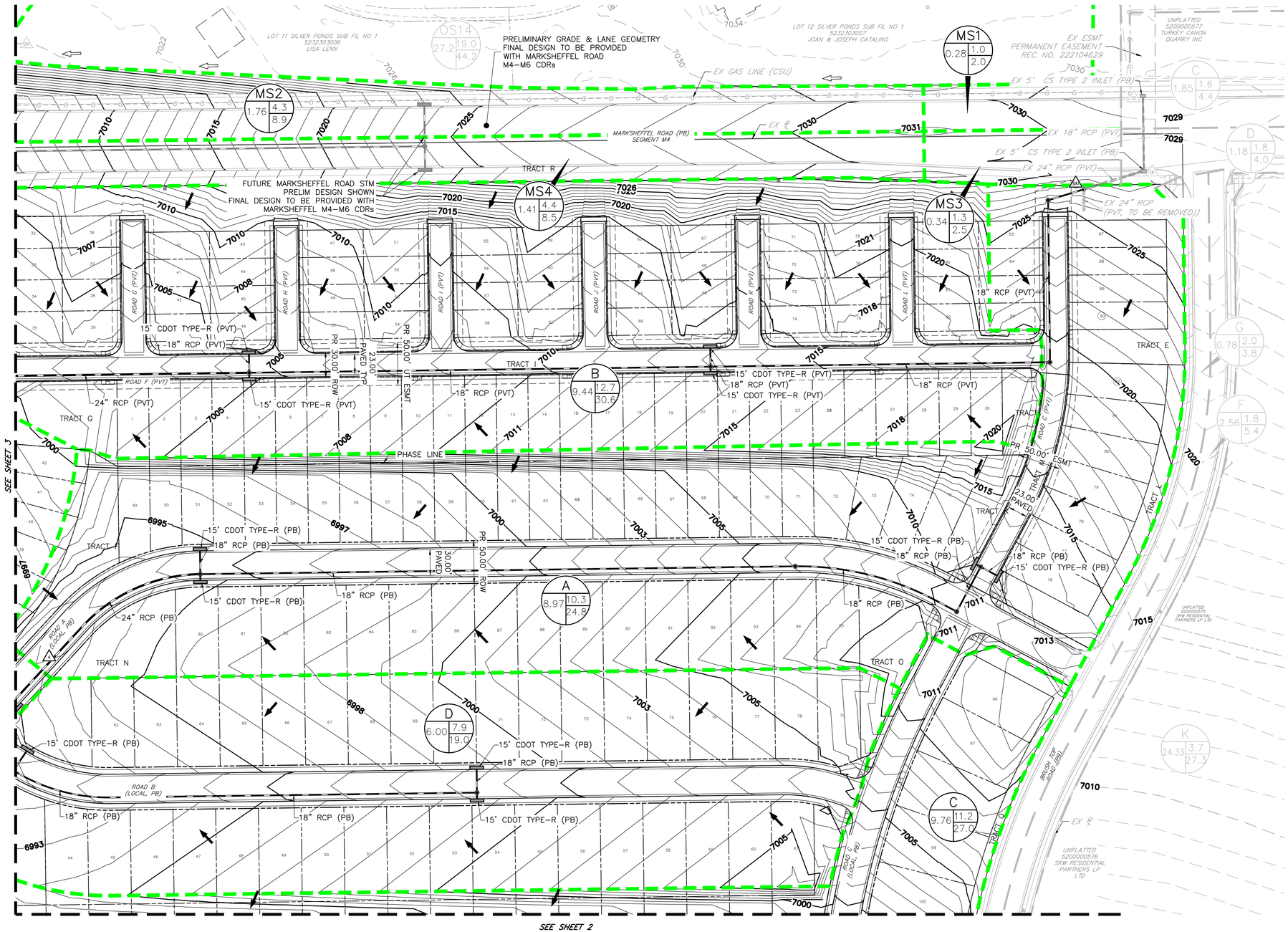
SCHMIDT PUD

JOB NO. 24013 | SHEET 3

LOCATION: EPC

06/12/2026

ALL TERRAIN ENGINEERING


PRELIMINARY GRADE & LANE GEOMETRY
FINAL DESIGN TO BE PROVIDED
WITH MARKSHEFFEL ROAD
M4–M6 CDRs
FUTURE MARKSHEFFEL ROAD STM
PRELIM DESIGN SHOWN
FINAL DESIGN TO BE PROVIDED WITH
MARKSHEFFEL M4–M6 CDRs
MARKSHEFFEL ROAD (PB)
SEGMENT M4
EX GAS LINE (CSU)
EX ESMT
PERMANENT EASEMENT
REC. NO. 222104629
EX 5' CS TYPE 2 INLET (PB)
EX 18" RCP (PVT)
EX 24" RCP (PVT)
EX 24" RCP
(PVT, TO BE REMOVED)
LOT 11 SILVER PONDS SUB FIL NO 1
5232303006
LISA LENN
LOT 12 SILVER PONDS SUB FIL NO 1
5232303007
JOAN & JOSEPH CATALINO
UNPLATTED
5200000577
TURKEY CANON
QUARRY INC
UNPLATTED
5200000576
SPW RESIDENTIAL
PARTNERS LP LTD
OS14
27.2 19.0 44.2
MS1
0.28 1.0 2.0
MS2
1.76 4.3 8.9
MS3
0.34 1.3 2.5
MS4
1.41 4.4 8.5
A
8.97 10.3 24.8
B
9.44 12.7 30.6
C
9.76 11.2 27.0
D
6.00 7.9 19.0
C
1.85 1.6 4.4
D
1.18 1.8 4.0
G
0.78 2.0 3.8
F
2.56 1.8 5.4
K
24.33 3.7 27.3
TRACT E
TRACT F
TRACT G
TRACT I
TRACT J
TRACT K
TRACT L
TRACT M
TRACT N
TRACT O
TRACT R
ROAD A (LOCAL, PB)
ROAD B (LOCAL, PB)
ROAD C (PVT)
ROAD C (LOCAL, PB)
ROAD D (PVT)
ROAD F (PVT)
ROAD G (PVT)
ROAD H (PVT)
ROAD J (PVT)
ROAD K (PVT)
ROAD L (PVT)
15" CDOT TYPE–R (PVT)
18" RCP (PVT)
24" RCP (PVT)
15" CDOT TYPE–R (PB)
18" RCP (PB)
24" RCP (PB)
PR 50.00' ROW
PR 50.00' UT ESMT
30.00' PAVED
23.00' PAVED TYP
PHASE LINE
EX 18" RCP (PVT)
EX 5' CS TYPE 2 INLET (PB)
EX 24" RCP (PVT)
EX 24" RCP
(PVT, TO BE REMOVED))
EX 18" RCP (PVT)
EX 15" RCP (PVT)
EX 5' CS TYPE 2 INLET (PB)
EX 18" RCP (PVT