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**SUBSURFACE SOIL INVESTIGATION
STERLING RANCH BRIDGES
STERLING RANCH ROAD OVER SAND CREEK
EL PASO COUNTY, COLORADO**

Prepared for:

**C&C Land
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Attn: Chaz Collins

November 4, 2021
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Respectfully Submitted,

ENTECH ENGINEERING, INC.

Stuart Wood
Geologist

Reviewed by:



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SW/bs

Encl.

Entech Job No. 200045

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**SUBSURFACE SOIL INVESTIGATION
STERLING RANCH BRIDGES
STERLING RANCH ROAD OVER SAND CREEK
EL PASO COUNTY, COLORADO**

1.0 INTRODUCTION

C&C Land is planning the construction of a vehicular bridge over Sand Creek for the proposed Sterling Ranch Road in El Paso County northeast of Colorado Springs, Colorado. The approximate location of the site is shown on the Vicinity Map, Figure 1. The planned layout of the proposed bridge is shown on Figure 2, Test Boring Location Map.

This report describes the subsurface investigation conducted for the planned bridge and provides recommendations for foundation design and construction. The subsurface soil investigation included drilling test borings at four (4) locations, in the creek bed near the bridge crossing and in the footprint of the proposed bridge abutments, collecting samples of soil, and conducting a geotechnical evaluation of the investigation findings. All drilling and subsurface investigation activities were performed by Entech Engineering, Inc. (Entech). The contents of this report, including the geotechnical evaluation and recommendations, are subject to the limitations and assumptions presented in Section 6.0.

2.0 PROJECT AND SITE DESCRIPTION

It is Entech's understanding that the project will consist of the construction of a vehicular bridge spanning Sand Creek with shallow spread footings and associated site improvements. At the time of drilling, the site for the proposed bridge was vacant. The crossing for the proposed Sterling Ranch Road had been rough graded at the time of drilling. Sand Creek flows to the south and east. Current vegetation on the site consisted of grasses and small shrubs.

3.0 SUBSURFACE EXPLORATIONS AND LABORATORY TESTING

The subsurface conditions were investigated by drilling four (4) exploratory test borings, in the creek bed near the proposed bridge crossing and in the footprint of the proposed bridge abutments. The borings were drilled to depths of 20 and 30 feet below the existing ground surface using a truck-mounted continuous flight auger-drilling rig supplied and operated by Entech Engineering, Inc. Boring Logs descriptive of the subsurface conditions encountered during drilling and subsequent to drilling are presented in Appendix A. At the conclusion of drilling, observations of groundwater levels were made in each of the open borings. The approximate locations of the test borings are indicated on Figure 2.

Soil samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D-1586) using a California Sampler. Results of the Standard Penetration Test (SPT) are included on the Test Boring Logs in terms of N-values expressed in blows per foot (bpf). Soil samples recovered from the borings were visually classified and recorded on the Test Boring Logs. The soil classifications were later verified utilizing laboratory testing and grouped by soil type. The soil type numbers are included on the Test Boring Logs. It should be understood that the soil descriptions shown on the Test Boring Logs may vary between boring location and sample depth.

It should also be noted that the lines of stratigraphic separation shown on the Test Boring Logs represent approximate boundaries between soil types and the actual stratigraphic transitions may be more gradual and vary with location. The Test Boring Logs are presented in Appendix A.

Moisture Content, ASTM D-2216, was obtained in the laboratory for all recovered samples. Grain-Size, ASTM D-422, and Atterberg Limits, ASTM D-4318, were determined for various samples for the purpose of classification and to obtain pertinent engineering characteristics. Volume change testing was performed on selected samples using the Swell/Consolidation Test (ASTM D-4546) in order to evaluate potential expansion/consolidation characteristics of the bedrock. Sulfate testing was performed on select samples to determine the corrosive characteristics of the soils. The Laboratory Test Results are included in Appendix B and summarized in Table 1.

4.0 SUBSURFACE CONDITIONS

One (1) soil type and two (2) bedrock types were encountered in the borings drilled for the subsurface investigation: Type 1: slightly silty to well graded sand (SM-SW, SW), Type 2: silty to slightly silty sandstone (SM, SM-SW), and Type 3: very sandy claystone to siltstone bedrock (CL, ML). The soils were classified in accordance with the Unified Soil Classification System (USCS) using the laboratory testing results and the observations made during drilling.

4.1 Soil and Rock

Soil Type 1 is a slightly silty to well graded clean sand (SM-SW, SW). The sand was encountered in all the test borings at the existing ground surface extending to a depth of up to 9 feet. Standard Penetration Testing conducted on the sand resulted in N-values of 8 to 21 blows per foot (bpf), which indicates loose to medium dense states. Moisture content and grain size testing resulted in moisture contents of 10 to 15 percent with approximately 5 to 10 percent of the soil size particles passing the No. 200 sieve. Atterberg limit testing was performed on a sample of native sand and resulted in a liquid limit of no value with a plastic index of non-plastic.

Sulfate testing on the sand resulted in 0.00 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 2 is a silty to slightly silty sandstone (SM, SM-SW). The sandstone was encountered in three of the test borings at depths ranging from 2 to 4 feet bgs and extending to depths of 25 feet or the depths explored (20 feet). Standard Penetration Testing conducted on the sandstone resulted in N-values of 50 to greater than 50 blows per foot (bpf), indicating the sandstone is very dense in terms of density. Moisture content and grain size testing resulted in moisture contents of 9 to 21 percent with approximately 10 percent of the soil size particles passing the No. 200 sieve in the sample tested. Atterberg limit testing resulted in liquid limits of no value and plastic indexes of non-plastic. Sulfate testing on the sandstone resulted in 0.00 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 3 is very sandy claystone to very sandy siltstone (CL, ML). The claystone/siltstone was encountered in Test Borings 1 and 4 at a depth of 9 to 25 feet bgs and extending to the termination of the borings (20 to 30 feet). Standard Penetration Testing conducted on the bedrock resulted in N-values of greater than 50 blows per foot (bpf), indicating the bedrock is hard in terms of consistency. Moisture content and grain size testing resulted in moisture contents of 10 to 22 percent with approximately 52 and 59 percent of the soil size particles passing the No. 200 sieve. Atterberg limit testing on the siltstone resulted in a liquid limit no value and a plastic index of non-plastic. Swell/Consolidation Testing on samples of the claystone/siltstone resulted in volume changes of 0.0 to 0.1 percent, indicating a low potential for expansion/consolidation. Sulfate testing on the sandstone resulted in 0.00 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

Additional descriptions and engineering properties of the soil encountered during drilling are included on the boring logs. Laboratory Testing Results are summarized on Table 1 and presented in Appendix B. It should be understood that the soil descriptions reported on the boring logs may vary between boring locations and sampling depths. Similarly, the lines of

stratigraphic separation shown on the boring logs represent approximate boundaries between soil types and the actual transitions between types may be more gradual or variable.

4.2 Groundwater

Groundwater was encountered at depths of 1 foot during and subsequent to drilling. Groundwater will affect development of significant foundation excavations or during installation of deep utilities depending on the final grading plans. Creek flow will vary due to rainfall, drainage, and other factors not readily apparent at this time. It should be noted that groundwater levels, observed at the time of the subsurface investigation, could change due to seasonal variations, changes in land runoff characteristics and future development including nearby areas.

5.0 GEOTECHNICAL EVALUATION AND RECOMMENDATIONS

The following discussion is based on the subsurface conditions encountered in the borings drilled in the creek bed near the planned bridge. If subsurface conditions different from those described herein are encountered during construction or if the project elements change from those described, Entech Engineering, Inc. should be notified so that the evaluation and recommendations presented can be reviewed and revised if necessary.

The site will be developed by constructing a vehicle traffic bridge over Sand Creek and associated site improvements on Sterling Ranch Road. The proposed bridge is expected to utilize shallow spread footings.

Subsurface soil conditions encountered in the test borings drilled for the planned bridge structure consisted of fine to coarse grained to well graded sand overlying silty to slightly silty sandstone and very sandy claystone and siltstone. Bedrock was encountered at depths of 2 to 9 feet in the test borings. The surficial sands were encountered at loose to medium dense states. The underlying sandstone was encountered in very dense states, and the underlying claystone/siltstone were encountered at hard consistencies.

5.1 Foundation Recommendations

The main purpose of the subsurface investigation was to gather soil and bedrock information for the proposed bridge abutments for use in providing foundation recommendations and design values. Recommendations for bridge supports using shallow spread footings and parameters for retaining walls are provided.

5.1.1 Shallow Foundation Parameters

Structures associated with the bridges can be supported with shallow foundations resting on the native sands, recompacted loose sands, or sandstone. It should be noted that due to potential shallow groundwater on this site (due to the proximity to Sand Creek), extensive subgrade improvements are anticipated to support shallow foundations. The foundation members should bear on the native site sands, sandstone, or granular soils compacted according to the "Structural Fill" paragraph. Any topsoil must be removed and the existing subgrade cleared of any debris to expose suitable native soils prior to fill placement. Loose soils or uncontrolled fill material beneath foundation components will require removal and recompaction. Any expansive soils encountered beneath the foundation will require removal and replacement with non-expansive structural fill compacted according to the "Structural Fill" paragraph. Any new fill should be placed to the requirements of the "Structural Fill" paragraph. On-site granular sands may be used as structural fill as approved by Entech. Any import material should be approved by Entech prior to hauling to the site.

Provided the above recommendations are followed, a maximum ultimate bearing pressure (LRFD) of 4000 psf is recommended for the native sands. For recompacted sands or imported granular structural fill, a maximum ultimate bearing pressure of 5000 psf is recommended. A maximum ultimate bearing capacity of 5800 psf is recommended for undisturbed sandstone. For footings bearing a minimum of 5 feet below adjacent grades on undisturbed sandstone, a maximum ultimate bearing capacity of 14,500psf is recommended. A resisting factor of 0.45 is recommended for the sand and structural fill referenced above and 0.6 is recommended for the above referenced bedrock (sandstone) soils. Additional bearing capacity information, including allowable stress design recommendations (ASD) can be found in Table 2.

Footings should extend a minimum of 30 inches below the adjacent exterior surface grade for frost protection. Following the above foundation subgrade preparation recommendations, and adhering to the recommended maximum allowable bearing pressure, it is expected to result in foundation designs which should limit total and differential vertical movements.

Foundation excavations are recommended to extend at least 3 feet horizontally beyond the foundation limits in order to provide adequate space for installation of drain materials (if necessary) and placement of controlled fill. All foundation excavation side slopes should be inclined at angles of 1¹/₂ horizontal to 1 vertical or flatter, as necessary, to provide for excavation sidewall stability during construction or as required by OSHA regulations.

Entech should observe overexcavated subgrades as well as the overall foundation excavation subgrade and evaluate if the exposed conditions are consistent with those described in this report. Entech should also provide recommendations for overexcavation depth and other subgrade improvements, if necessary, and the need for drain systems and stabilization methods based on the excavation conditions observed at that time.

5.1.2 Retaining Wall Parameters

The following values are recommended for use in designing retaining walls with unbalanced lateral loading that may be associated with this project. Roadway/Vehicle surcharge loading is required for wall design.

Recommended Design Values – Lateral Loading

Equivalent fluid density for lateral earth pressure (active), pcf (site granular soils)	45
Equivalent fluid density for lateral earth pressure (passive), pcf	300
Equivalent fluid density for lateral earth pressure (at rest), pcf	60
Soil density (compacted sand), pcf	125
Angle of Internal Friction (loose silty sand and sandy clay-silt)	26°
Angle of Internal Friction (compacted silty sand)	34°
Coefficient of sliding between concrete and silty gravelly sand	0.35
Bearing capacity of sand (ultimate), psf	4000 psf

Bearing capacity of sandstone (ultimate), psf	5800 psf
Resisting Factor (Sand)	0.45
Resisting Factor (Sandstone)	0.60

*Note: The above lateral loading design values are for level back slope angles and no surcharge loads. If wall backfill is submerged, water pressures must be taken into account as additional wall loading. If backfill slope angles are greater than zero degrees, or if the backfill is surcharged, the design values must be adjusted to account for additional lateral loading.

5.2 Site Seismic Classification

Based on the subsurface conditions encountered at the site and in accordance with Section 1613 of the 2015 International Building Code (IBC), the site meets the conditions of a Site Class C.

5.3 Surface and Subsurface Drainage

Positive surface drainage must be maintained around structures to minimize infiltration of surface water. A minimum gradient of 5 percent in the first 10 feet adjacent to foundation components is recommended. A minimum gradient of 2 percent is recommended for paved areas. All grades should be directed away from structures.

To help minimize infiltration of water into foundation zones, vegetative plantings placed close to foundation components should be limited to those species having low watering requirements and irrigated grass should not be located within 5 feet of foundation components. Similarly, sprinklers are not recommended to discharge water within 5 feet of foundation components. Irrigation near foundations should be limited to the minimum amount sufficient to maintain vegetation. Application of more irrigation water than necessary can increase the potential for foundation movement.

5.4 Concrete

Soluble sulfate testing was conducted on three samples of the site soils to evaluate the potential for sulfate attack on concrete placed below the surface grade. The test results indicated 0.00

percent soluble sulfate by weight for the site soils. The test results indicate the sulfate component of the in-place site soils present a negligible exposure threat to concrete placed below grade that comes into contact with the site soils.

Type II cement is recommended for concrete at this site. To further avoid concrete degradation during construction it is recommended that concrete not be placed on frozen or wet ground. Care should be taken to prevent the accumulation or ponding of water in foundation excavations prior to the placement of concrete. If standing water is present in the foundation excavations, it should be removed by ditching to sumps and pumping the water away from the foundation area prior to concrete placement. If concrete is placed during periods of cold temperatures, the concrete must be kept from freezing. This may require covering the concrete with insulated blankets and adding heat to prohibit freezing.

5.5 Foundation Excavation Observations

Subgrade preparation for bridge foundations and associated improvements should be observed by Entech Engineering prior to construction of the foundation elements in order to verify that (1) no anomalies are present, (2) materials of the proper bearing capacity have been encountered or placed, and (3) no soft, loose, uncontrolled fill material, expansive soil or debris are present in the foundation area prior to concrete placement or backfilling. Pile driving should be observed to verify proper embedment or refusal. Piles should be driven 10 feet into bedrock or refusal. Entech should make final recommendations for over-excavation or stabilization, if required, at the time of excavation observation, if necessary.

5.6 Structural Fill

Areas to receive fill should have all topsoil, organic material or debris removed. Fill must be properly benched. The surface should be scarified and moisture conditioned to within ± 2 percent of its optimum moisture content and compacted to 95 percent of its maximum Modified Proctor Dry Density (ASTM D-1557) beneath footings or floor slabs prior to placing new fill. New fill beneath footings should be non-expansive and be placed in thin lifts not to exceed 6 inches after compaction while maintaining at least 95 percent of its maximum Modified Proctor Dry Density (ASTM D-1557). These materials should be placed at a moisture content

conducive to compaction, usually ± 2 percent of Proctor optimum moisture content. The placement and compaction of fill should be observed and tested by Entech Engineering, Inc. Imported soils should be approved by Entech Engineering, Inc. prior to being hauled to the site and on-site granular soils prior to placement.

Compacted, non-expansive granular soil, free of organics, debris and cobbles greater than 3-inches in diameter, is recommended for filling foundation components. All fill placed within the foundation areas should be non-expansive and be compacted to a minimum of 95 percent of the soils maximum dry density as determined by the Modified Proctor Test (ASTM D-1557). Fill material placed beneath floor slabs should be compacted to a minimum of 95 percent of its maximum Modified Proctor Dry Density, ASTM D-1557. Fill material should be placed in horizontal lifts such that each finished lift has a compacted thickness of six inches or less. Fill should be placed at water contents conducive to achieving adequate compaction, usually within ± 2 percent of the optimum water content as determined by ASTM D-1557. Mechanical methods can be used for placement and compaction of fill; however, heavy equipment should be kept at distance from foundation walls and below slab infrastructure to avoid overstressing. No water flooding techniques of any type should be used for compaction or placement of foundation or floor slab fill material.

5.7 Utility Trench Backfill

Fill placed in utility trenches should be compacted to a minimum of 95 percent of its maximum dry density as determined by the Standard Proctor Test (ASTM D-698) for cohesive soils and 95 percent as determined by the Modified Proctor Test (ASTM D-1557) for cohesionless soils. Fill should be placed in horizontal lifts having a compacted thickness of six inches or less and at a water content conducive to adequate compaction, within ± 2 percent of the optimum water content. Mechanical methods should be used for fill placement; however, heavy equipment should be kept at a distance from foundation walls. No water flooding techniques of any type should be used for compaction or placement of utility trench fill.

Trench backfill placement should be performed in accordance with El Paso County specifications. All excavation and excavation shoring/bracing should be performed in accordance with OSHA guidelines.

5.8 General Backfill

Any areas to receive fill outside the foundation limits should have all topsoil, organic material, and debris removed. Fill must be properly benched into existing slopes in order to be adequately compacted. The fill receiving surface should be scarified to a depth of 12-inches and moisture conditioned to ± 2 percent of the optimum water content, and compacted to a minimum of 95 percent of the ASTM D-1557 maximum dry density before the addition of new fill. Fill should be placed in thin lifts not to exceed 6 inches in thickness after compaction while maintaining at least 95 percent of the ASTM D-1557 maximum dry density. Fill material should be free of vegetation and other unsuitable material and shall not contain rocks or fragments greater than 3-inches. Topsoil and strippings should be segregated from all other fill sources on the site. Fill placement and compaction beneath and around foundations, in utility trenches, beneath roadways or other structural features of the project should be observed and tested by Entech during construction.

5.9 Excavation Stability

Excavation sidewalls must be properly sloped, benched and/or otherwise supported in order to maintain stable conditions. All excavation openings and work completed therein shall conform to OSHA Standards as put forward in CFR 29, Part 1926.650-652, (Subpart P).

5.10 Winter Construction

In the event construction of the planned facility occurs during winter, foundations and subgrades should be protected from freezing conditions. Concrete should not be placed on frozen soil and once concrete has been placed, it should not be allowed to freeze. Similarly, once exposed, the foundation subgrade should not be allowed to freeze. During site grading and subgrade preparation, care should be taken to eliminate burial of snow, ice or frozen material within the planned construction area.

5.11 Construction Observations

It is recommended that Entech observe and document the following activities during construction of the building foundations.

- Excavated subgrades and subgrade preparation.
- Placement of drains (if installed).
- Placement/compaction of fill material for the foundation components and retaining walls.
- Placement/compaction of utility bedding and trench backfill.

6.0 CLOSURE

The subsurface investigation, geotechnical evaluation and recommendations presented in this report are intended for use of C&C Land with application to the proposed bridge over Sand Creek at Sterling Ranch Road and the associated site improvements, in El Paso County northeast of Colorado Springs, Colorado. In conducting the subsurface investigation, laboratory testing, engineering evaluation and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in same locality and under similar conditions. No other warranty, expressed or implied is made. During final design and/or construction, if conditions are encountered which appear different from those described in this report, Entech Engineering, Inc. requests that it be notified so that the evaluation and recommendations presented herein can be reviewed and modified as appropriate.

If there are any questions regarding the information provided herein or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.

TABLE

TABLE 1
SUMMARY OF LABORATORY TEST RESULTS

CLIENT C&C LAND
 PROJECT STERLING RANCH BRIDGES
 JOB NO. 200045

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	1	5			10.0	NV	NP	0.00			SM-SW	SAND, SLIGHTLY SILTY
1	2	10			7.5						SM-SW	SAND, SLIGHTLY SILTY
1	4	2-3			4.5						SW	SAND
2	3	20			9.9	NV	NP	0.00			SM-SW	SANDSTONE, SLIGHTLY SILTY
3	4	15	16.1	114.3	59.0					0.1	CL	CLAYSTONE, VERY SANDY
3	1	30	14.4	113.5	51.9	NV	NP	0.00		0.0	ML	SILTSTONE, VERY SANDY

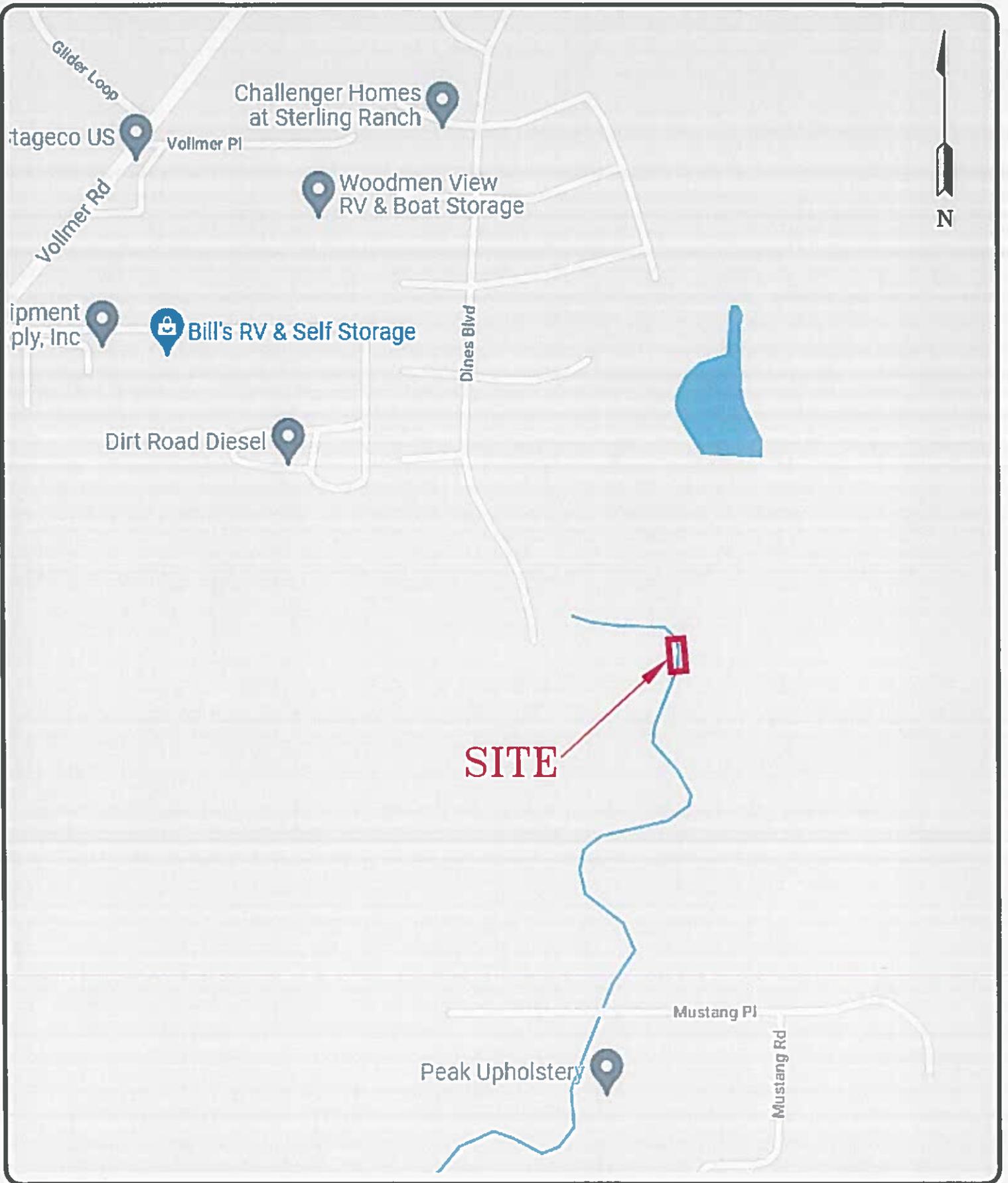
TABLE 2
BEARING CAPACITY

CLIENT C & C Land
PROJECT Sterling Ranch Road Over Sand Creek
JOB NO. 200045

Soil Type	Minimum Embedment (from adjacent slab/grade)	Design Methodology		
		ASD Allowable Bearing (psf)	LRFD Ultimate Bearing (psf)	Resisting Factor
Sand (Native)	3 feet	2400	4000	0.45
Structural Fill	3 feet	3000	5000	0.45
Sandstone	3 feet	3500	5800	0.6
Sandstone*	5 feet	8500	14,500	0.6

*Foundation component must bear on undisturbed sandstone

FIGURES



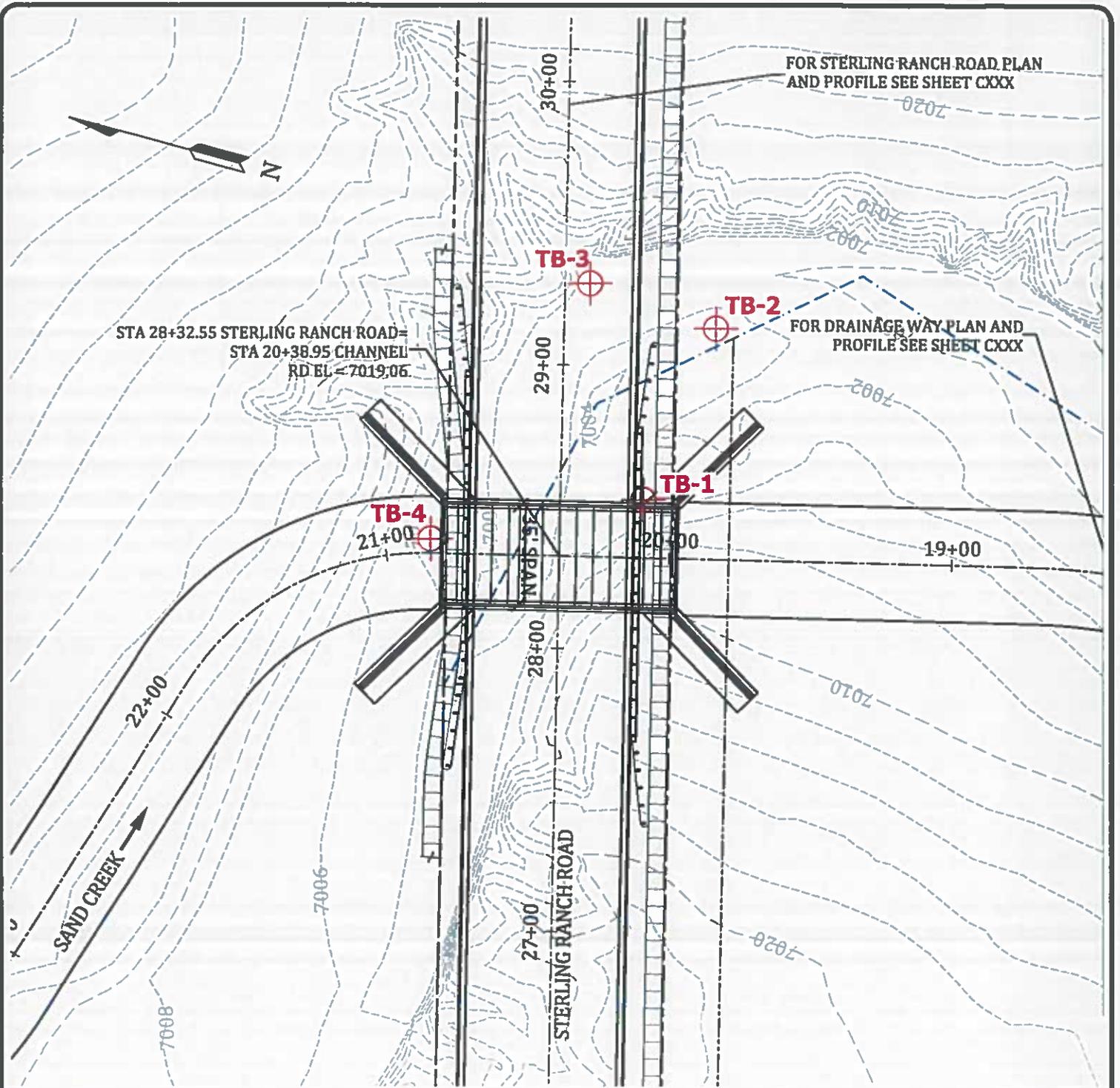

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VICINITY MAP
STERLING RANCH ROAD BRIDGE
COLORADO SPRINGS, CO.
FOR: C&C LAND

DRAWN: JAC	DATE: 10/25/21	CHECKED: DPS	DATE: 10/25/21
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JOB NO.:
200045

FIG NO.:
1



 **TB- APPROXIMATE TEST BORING LOCATION AND NUMBER**



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TEST BORING LOCATION MAP
STERLING RANCH ROAD BRIDGE
COLORADO SPRINGS, CO.
FOR: C&C LAND

DRAWN: JAC	DATE: 10/25/21	CHECKED: DPS	DATE: 10/25/21
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JOB NO.:
200045

FIG NO.:
2

APPENDIX A: Test Boring Logs

TEST BORING NO. 1
 DATE DRILLED 10/2/2021
 Job # 200045

TEST BORING NO. 2
 DATE DRILLED 10/2/2021
 CLIENT C&C LAND
 LOCATION STERLING RANCH BRIDGES

REMARKS

REMARKS

WATER @ 1', 10/2/21

SAND, SLIGHTLY SILTY, FINE TO COARSE GRAINED, GRAY BROWN, MEDIUM DENSE, WET

SANDSTONE, SLIGHTLY SILTY, FINE TO COARSE GRAINED, GRAY, VERY DENSE, WET

SILTSTONE, VERY SANDY, GRAY BROWN, HARD, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
0					
1			21	13.8	1
5			50	11.1	2
10			50 6"	13.8	2
15			50 6"	13.0	2
20			50 7"	8.8	2
25					
30			50 5"	12.3	3

WATER @ 1', 10/2/21

SAND, SILTY, TAN

SANDSTONE, SLIGHTLY SILTY WITH THIN SILTSTONE INTERBEDS, FINE TO COARSE GRAINED, GRAY, VERY DENSE, WET

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
0					
1					1
5			50 6"	5.2	2
5			50 6"	20.5	2
10			50 8"	14.9	2
15			50 7"	9.7	2
20			50 6"	11.6	2



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505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED: SW

DATE: 10-15-21

JOB NO:
 200045

FIG NO:
 A-1

TEST BORING NO. 3
 DATE DRILLED 10/2/2021
 Job # 200045

TEST BORING NO. 4
 DATE DRILLED 10/2/2021
 CLIENT C&C LAND
 LOCATION STERLING RANCH BRIDGES

REMARKS

WATER @ 1', 10/2/21

SAND, SLIGHTLY SILTY, FINE TO
 COARSE GRAINED, GRAY BROWN,
 MEDIUM DENSE, WET

SANDSTONE, SILTY TO SLIGHTLY
 SILTY, FINE TO COARSE GRAINED,
 GRAY, VERY DENSE, WET

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
1			11	10.2	1
5			50 8"	11.8	2
10			50 8"		2
15			50 6"	19.3	2
20			50 3"	18.6	2

REMARKS

WATER @ 1', 10/2/21

SAND, FINE GRAINED, GRAY
 BROWN, MEDIUM DENSE TO
 LOOSE, WET

CLAYSTONE, VERY SANDY, DARK
 GRAY BROWN, HARD, WET

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
1			12	10.9	1
5			8	14.6	1
10			50 5"	22.1	3
15			50 4"	15.8	3
20			50 4"	9.8	3



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 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED: *SW*

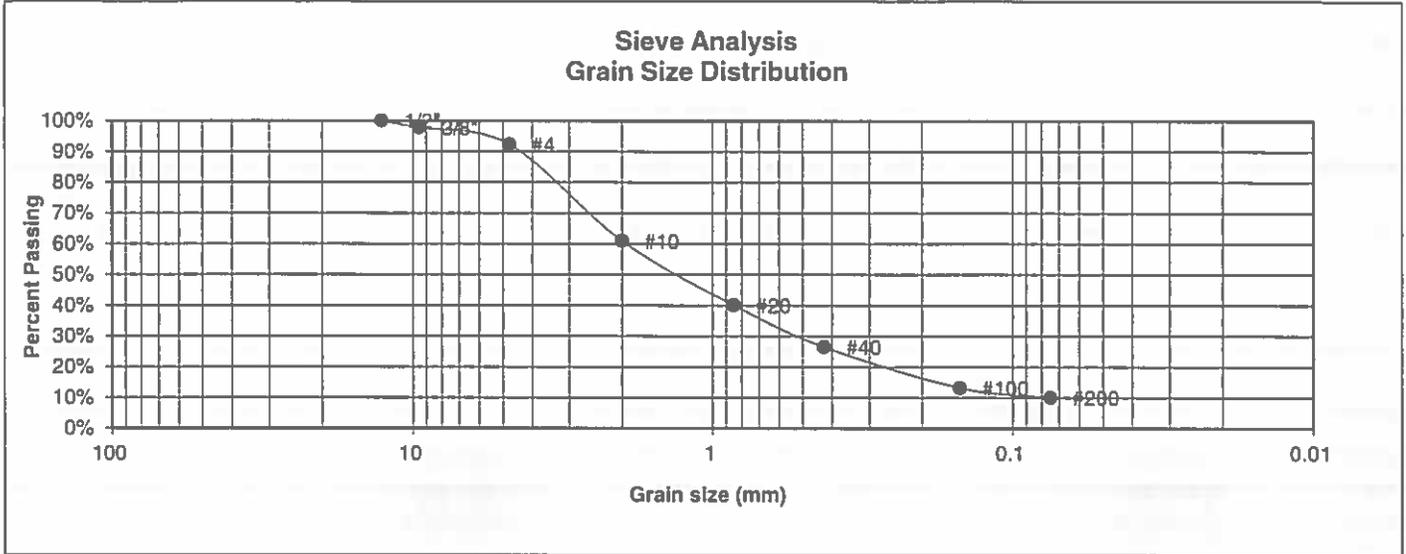
DATE: *11-9-21*

JOB NO:
 200045

FIG NO:
 A- 2

APPENDIX B: Laboratory Testing Results

UNIFIED CLASSIFICATION	SM-SW	CLIENT	C&C LAND
SOIL TYPE #	1	PROJECT	STERLING RANCH BRIDGES
TEST BORING #	1	JOB NO.	200045
DEPTH (FT)	5	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	97.8%
4	92.3%
10	61.0%
20	40.1%
40	26.5%
100	13.2%
200	10.0%

Atterberg Limits

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

Swell

Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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COLORADO SPRINGS, COLORADO 80907

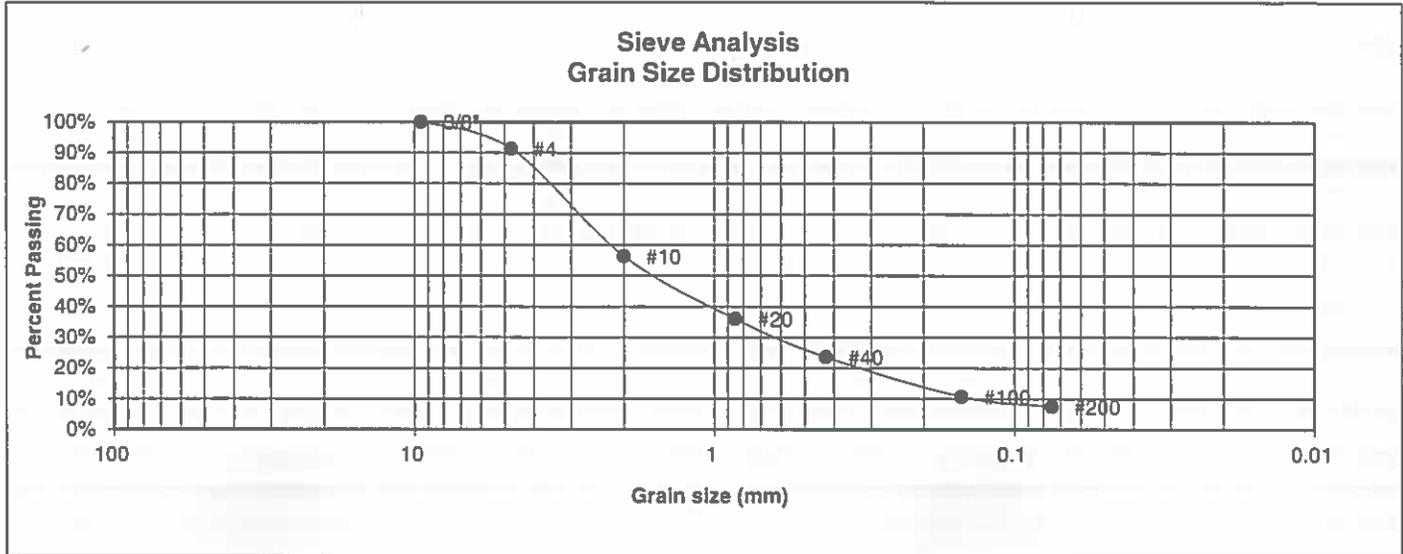
**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED: <i>SW</i>	DATE: <i>10-18-21</i>
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JOB NO.:
200045

FIG NO.:
B-1

UNIFIED CLASSIFICATION	SM-SW	CLIENT	C&C LAND
SOIL TYPE #	1	PROJECT	STERLING RANCH BRIDGES
TEST BORING #	2	JOB NO.	200045
DEPTH (FT)	10	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	91.3%
10	56.4%
20	36.0%
40	23.5%
100	10.7%
200	7.5%

Atterberg Limits
 Plastic Limit
 Liquid Limit
 Plastic Index

Swell
 Moisture at start
 Moisture at finish
 Moisture increase
 Initial dry density (pcf)
 Swell (psf)



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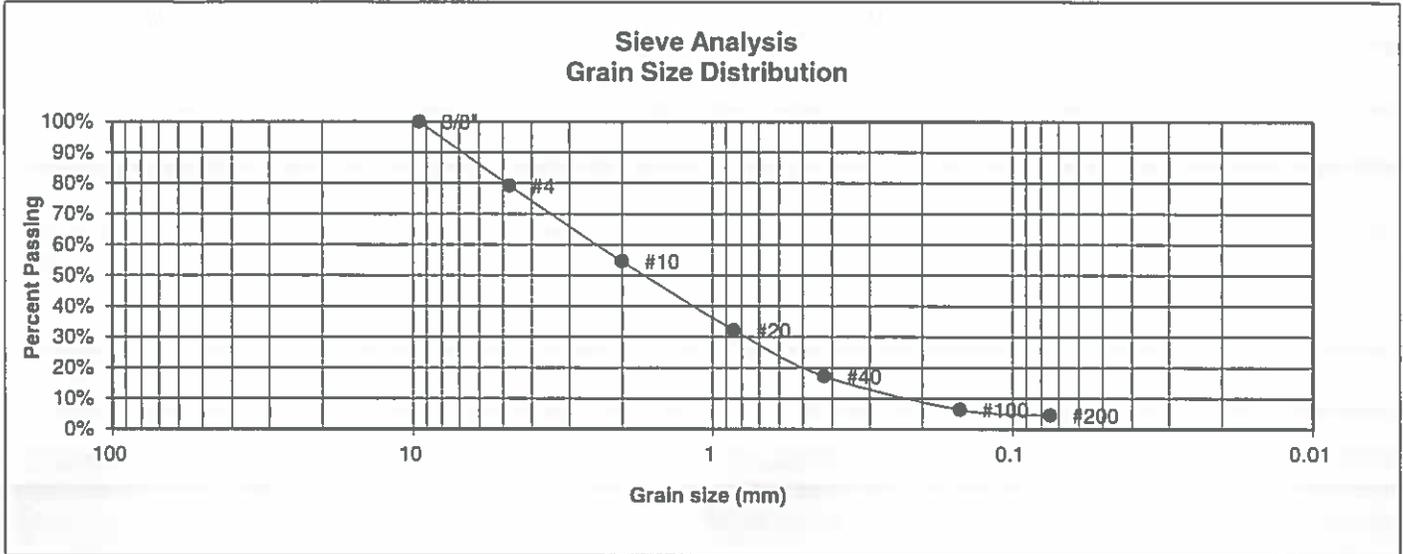
**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED:	DATE:
		SW	10-18-21

JOB NO.:
200045

FIG NO:
B-2

<u>UNIFIED CLASSIFICATION</u>	SW	<u>CLIENT</u>	C&C LAND
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	STERLING RANCH BRIDGES
<u>TEST BORING #</u>	4	<u>JOB NO.</u>	200045
<u>DEPTH (FT)</u>	2-3	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	79.2%
10	54.5%
20	32.2%
40	17.2%
100	6.4%
200	4.5%

Atterberg Limits
 Plastic Limit
 Liquid Limit
 Plastic Index

Swell
 Moisture at start
 Moisture at finish
 Moisture increase
 Initial dry density (pcf)
 Swell (psf)



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**LABORATORY TEST
RESULTS**

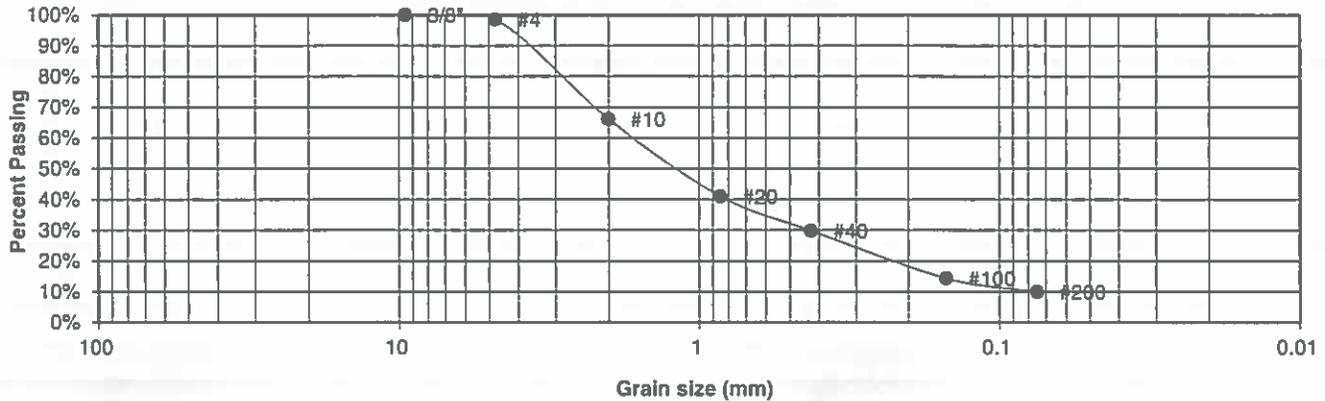
<u>DRAWN:</u>	<u>DATE:</u>	<u>CHECKED:</u> SW	<u>DATE:</u> 10-18-21
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JOB NO.:
200045

FIG NO.:
B-3

UNIFIED CLASSIFICATION	SM-SW	CLIENT	C&C LAND
SOIL TYPE #	2	PROJECT	STERLING RANCH BRIDGES
TEST BORING #	3	JOB NO.	200045
DEPTH (FT)	20	TEST BY	BL

**Sieve Analysis
Grain Size Distribution**



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.6%
10	66.1%
20	41.0%
40	29.8%
100	14.4%
200	9.9%

Atterberg Limits	
Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

Swell	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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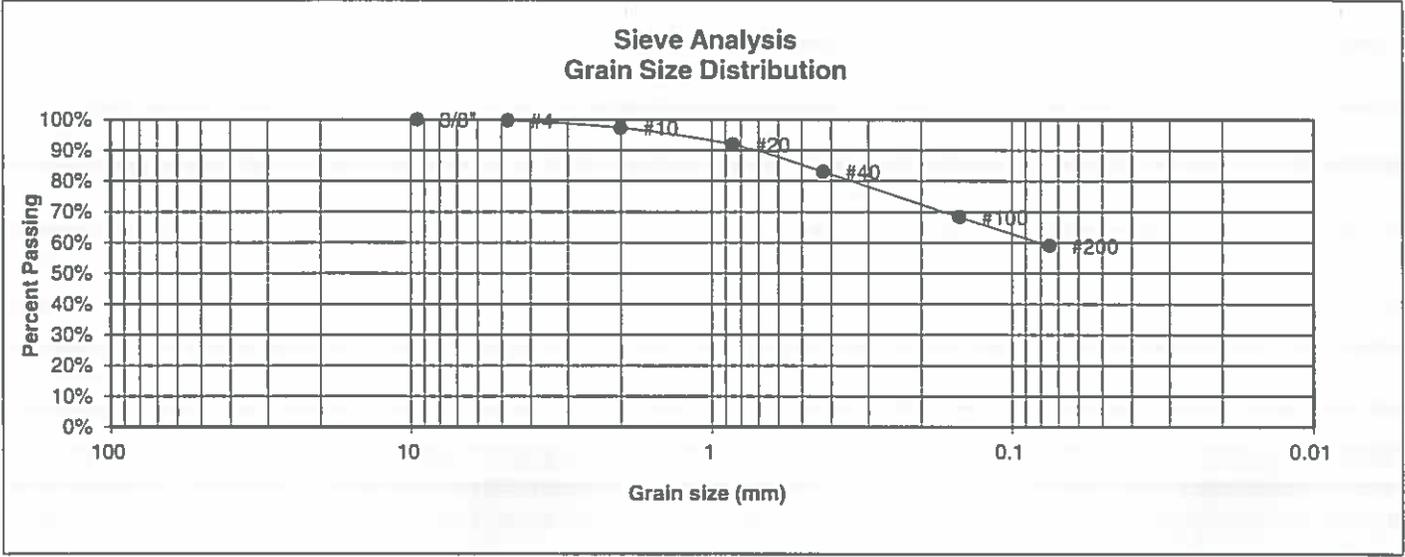
**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED: <i>SW</i>	DATE: <i>10-18-21</i>
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JOB NO.:
200045

FIG NO.:
B-4

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	C&C LAND
<u>SOIL TYPE #</u>	3	<u>PROJECT</u>	STERLING RANCH BRIDGES
<u>TEST BORING #</u>	4	<u>JOB NO.</u>	200045
<u>DEPTH (FT)</u>	15	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.7%
10	97.3%
20	91.9%
40	83.0%
100	68.3%
200	59.0%

Atterberg Limits
 Plastic Limit
 Liquid Limit
 Plastic Index

Swell
 Moisture at start
 Moisture at finish
 Moisture increase
 Initial dry density (pcf)
 Swell (psf)



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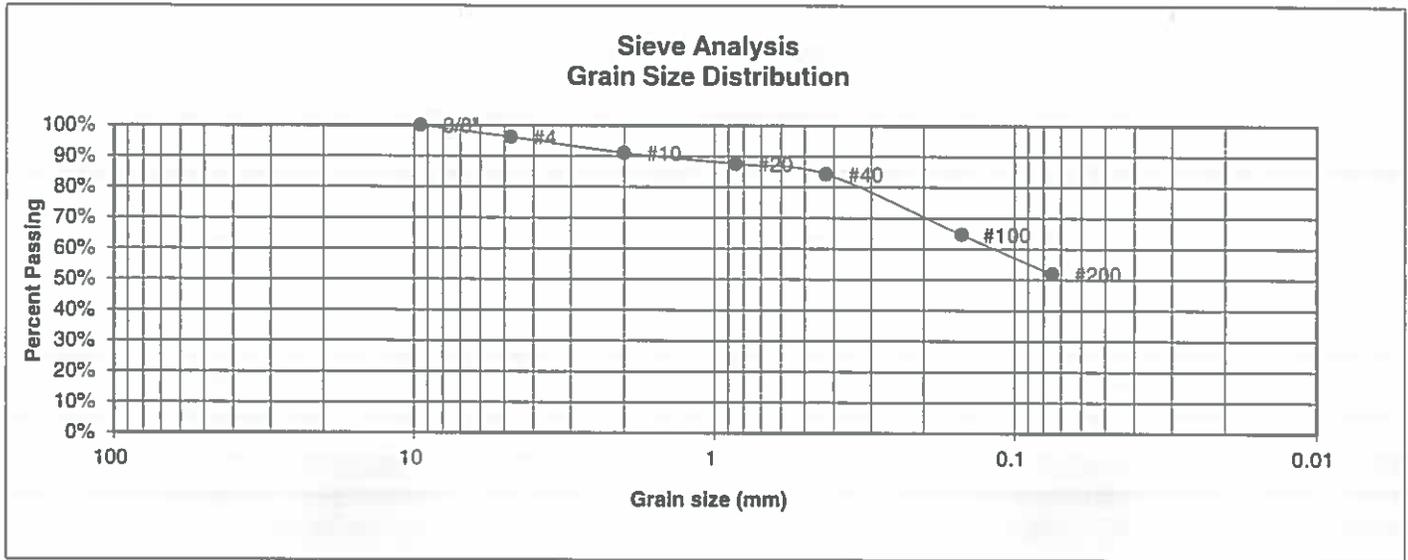
LABORATORY TEST RESULTS

<u>DRAWN:</u>	<u>DATE:</u>	<u>CHECKED:</u> SW	<u>DATE:</u> 70-18-21
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JOB NO.:
200045

FIG NO.:
B-5

<u>UNIFIED CLASSIFICATION</u>	ML	<u>CLIENT</u>	C&C LAND
<u>SOIL TYPE #</u>	3	<u>PROJECT</u>	STERLING RANCH BRIDGES
<u>TEST BORING #</u>	1	<u>JOB NO.</u>	200045
<u>DEPTH (FT)</u>	30	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	96.2%
10	91.0%
20	87.6%
40	84.2%
100	64.7%
200	51.9%

<u>Atterberg Limits</u>	
Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

<u>Swell</u>	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED: <i>SW</i>	DATE: <i>10-18-21</i>
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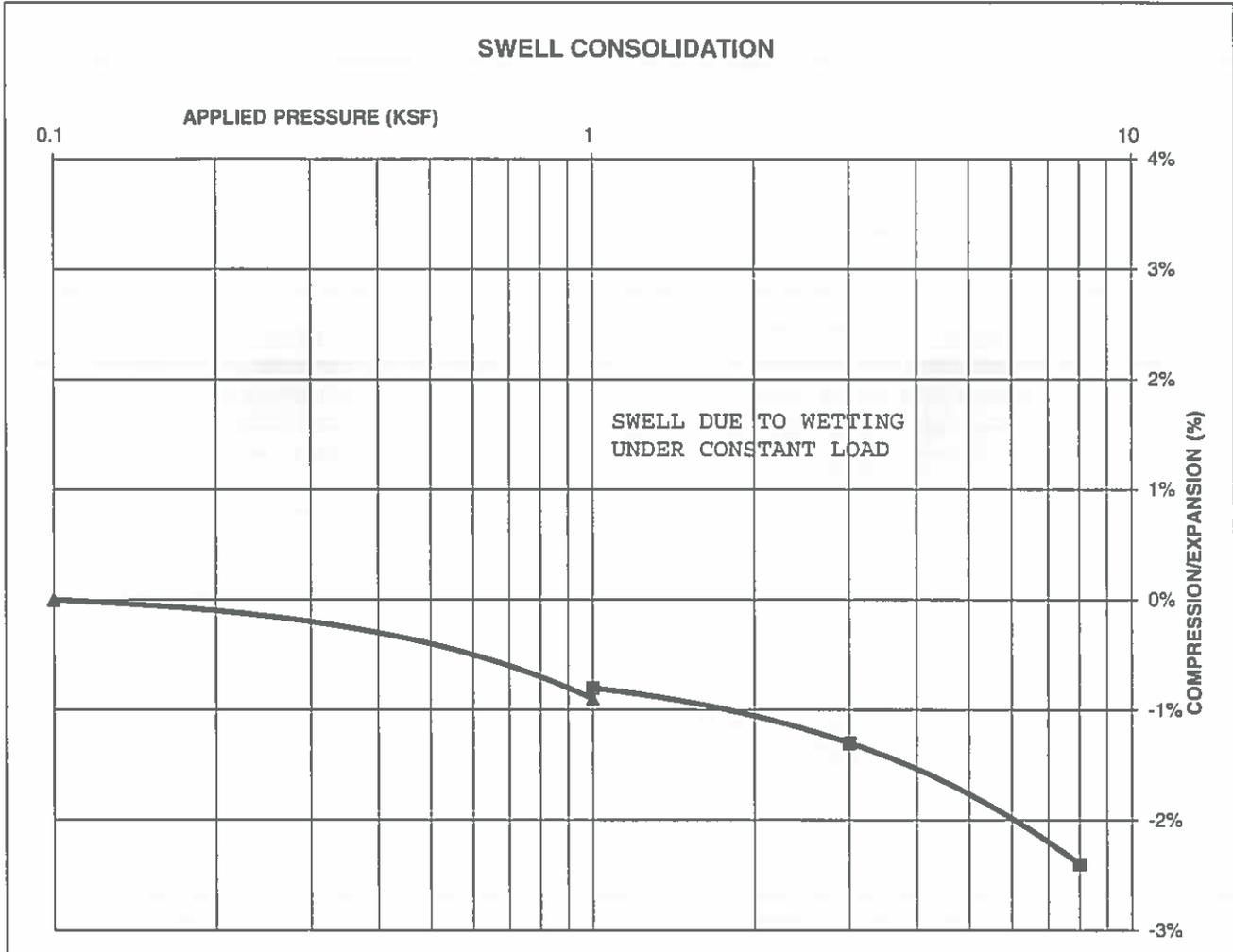
JOB NO.:
200045

FIG NO.:
B-6

CONSOLIDATION TEST RESULTS

TEST BORING #	4	DEPTH(ft)	15
DESCRIPTION	CL	SOIL TYPE	3
NATURAL UNIT DRY WEIGHT (PCF)			114
NATURAL MOISTURE CONTENT			16.1%
SWELL/CONSOLIDATION (%)			0.1%

JOB NO. 200045
 CLIENT C&C LAND
 PROJECT STERLING RANCH BRIDGES



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**SWELL CONSOLIDATION
 TEST RESULTS**

DRAWN:

DATE:

CHECKED:

SW

DATE:

10-18-21

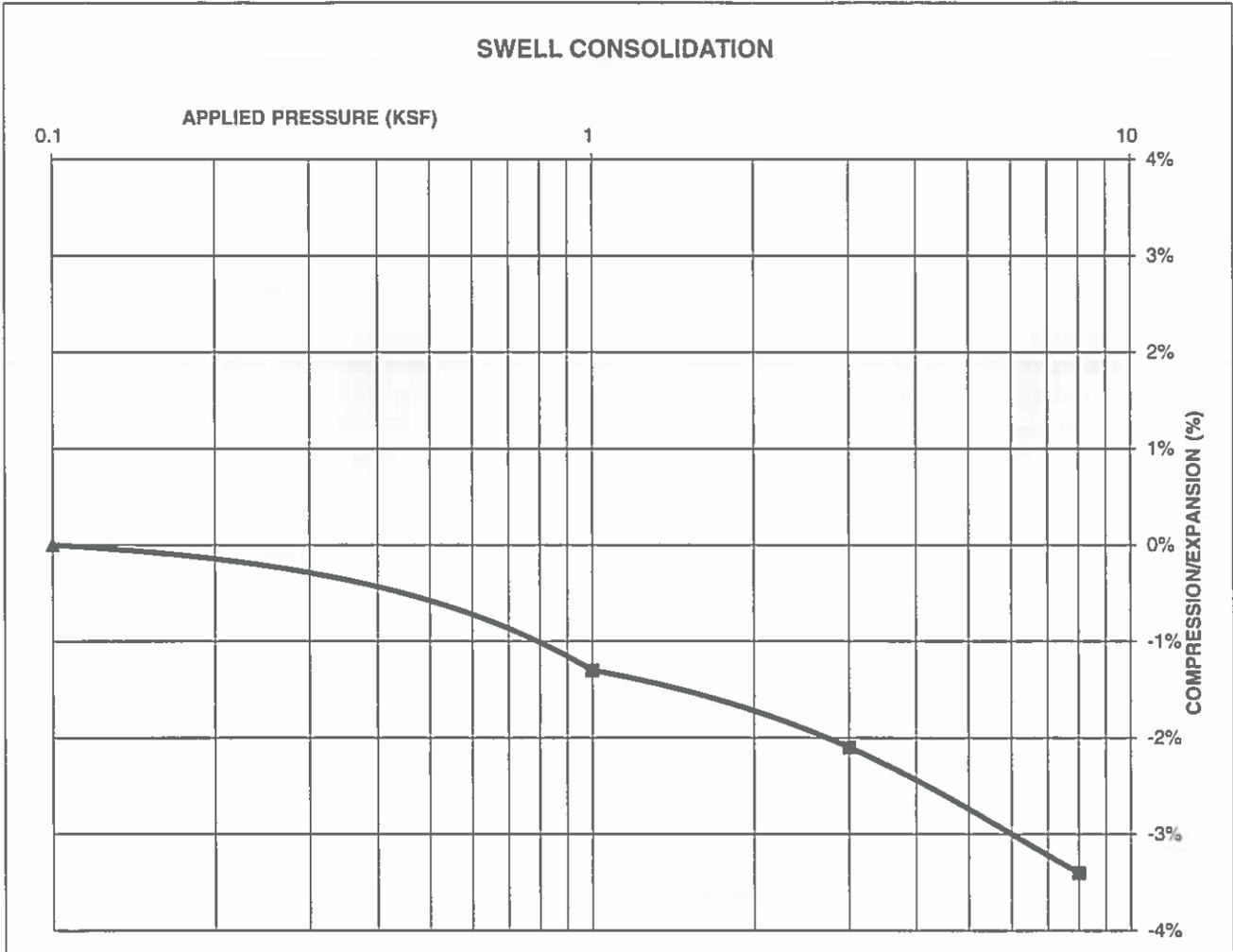
JOB NO.:
 200045

FIG NO.:
 B-7

CONSOLIDATION TEST RESULTS

TEST BORING #	1	DEPTH(ft)	30
DESCRIPTION	ML	SOIL TYPE	3
NATURAL UNIT DRY WEIGHT (PCF)			113
NATURAL MOISTURE CONTENT			14.4%
SWELL/CONSOLIDATION (%)			0.0%

JOB NO. 200045
 CLIENT C&C LAND
 PROJECT STERLING RANCH BRIDGES



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**SWELL CONSOLIDATION
 TEST RESULTS**

DRAWN:

DATE:

CHECKED:

SW

DATE:

10-21-21

JOB NO.:
 200045

FIG NO:
 B-8

