

# **FINAL DRAINAGE REPORT**

**for**

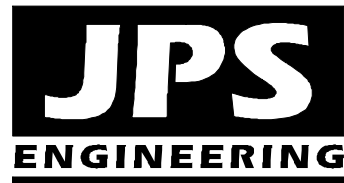
## **BERISFORD SUBDIVISION**

**Prepared for:**

**Christopher Berisford**  
17240 W. Goshawk Road  
Colorado Springs, CO 80908

April 25, 2024  
Revised September 12, 2024  
Revised November 22, 2024

**Prepared by:**



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**JPS Project No. 012401**  
**PCD File No.: SF2415**

**BERISFORD SUBDIVISION  
FINAL DRAINAGE REPORT  
TABLE OF CONTENTS**

	<u>PAGE</u>
DRAINAGE STATEMENT .....	i
I. GENERAL LOCATION AND DESCRIPTION.....	1
II. DRAINAGE BASINS AND SUB-BASINS.....	2
III. DRAINAGE DESIGN CRITERIA .....	3
IV. DRAINAGE PLANNING FOUR STEP PROCESS.....	3
V. GENERAL DRAINAGE RECOMMENDATIONS .....	4
VI. DRAINAGE FACILITY DESIGN .....	5
VII. EROSION / SEDIMENT CONTROL .....	8
VIII. STORMWATER DETENTION AND WATER QUALITY.....	8
IX. DRAINAGE FEES .....	8
X. SUMMARY .....	9

APPENDICES

APPENDIX A	Soils Information
APPENDIX B	Hydrologic Calculations
APPENDIX C	Hydraulic Calculations
APPENDIX D	Site Photographs
APPENDIX E	Figures
Figure A1	Vicinity Map
Figure FIRM	Floodplain Exhibit
Sheet EX1	Existing Conditions Drainage Plan
Sheet D1	Developed Drainage Plan

DRAINAGE STATEMENT

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for liability caused by negligent acts, errors or omissions on my part in preparing this report.

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John P. Schwab, P.E. #29891

Developer's Statement:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

By:

---

Printed Name: Christopher Berisford  
17240 W. Goshawk Road, Colorado Springs, CO 80908

Date

El Paso County's Statement

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2, and Engineering Criteria Manual as amended.

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Joshua Palmer, P.E.  
County Engineer / ECM Administrator

Date

Conditions:

## **I. GENERAL LOCATION AND DESCRIPTION**

### **A. Background**

Berisford Subdivision is a proposed 4-lot rural residential subdivision located in the Black Forest area of El Paso County, Colorado. The minor subdivision will create four residential lots on the existing 20-acre parcel (El Paso County Assessor's Number 51230-00-026) located along the north side of Goshawk Road. All of the proposed lots will have direct access to Goshawk Road along the south boundary of the subdivision. There are no subdivision roadway improvements or common infrastructure improvements proposed, and the site disturbance associated with subdivision improvements is anticipated to be less than one acre.

### **B. Scope**

This report will provide a summary of site drainage issues impacting the proposed residential minor subdivision. The report will analyze upstream drainage patterns, site-specific developed drainage patterns, and impacts on downstream facilities. This report is based on the guidelines and criteria presented in the El Paso County Drainage Criteria Manual, and the report is intended to fulfill the requirements for a "Final Drainage Report" in support of the Final Plat process for this property.

### **C. Site Location and Description**

Berisford Subdivision is located in the Northeast Quarter of Section 23, Township 11 South, Range 65 West of the 6th Principal Meridian. The 20-acre parcel is currently an undeveloped rural residential property vegetated with Ponderosa Pine trees. The property is zoned RR5 (rural residential), allowing for 5-acre minimum lot sizes, and the proposed minor subdivision is fully in conformance with the existing zoning of the site.

The site is bordered by existing unplatted rural ranch properties to the north, east, and south (Zoned RR5). The west boundary of the site adjoins a vacant 10-acre lot recently platted as Lot 4, Warner 4-Lot Subdivision. Goshawk Road is an existing gravel private street along the south boundary of the site. Access to the new lots will be provided by new private driveway connections to the existing Goshawk Road along the south boundary of the subdivision.

Ground elevations within the site range from approximately 7,430 to 7,490 feet above mean sea level.

The site is located in the West Kiowa Creek Drainage Basin, and the existing drainage swales downstream of the site flow northeasterly towards West Kiowa Creek. The terrain is rolling with average grades ranging from 2 to 10 percent. The site is vegetated with Ponderosa Pine trees along with clusters of native shrubbery and grasses.

## **D. General Soil Conditions**

According to the Custom Soil Resource Report for this site (see details in Appendix A) provided by the Natural Resources Conservation Service (NRCS), on-site soils are comprised of “Type 26: Elbeth sandy loam.” These soils are classified as hydrologic soils group “B” (moderate infiltration rate).

## **E. References**

City of Colorado Springs & El Paso County “Drainage Criteria Manual (DCM),” revised October 31, 2018.

City of Colorado Springs “Drainage Criteria Manual, Volumes 1 and 2,” revised October 31, 2018.

El Paso County “Engineering Criteria Manual,” revised December 13, 2016.

FEMA, Flood Insurance Rate Map (FIRM) Number 08041C0310G, December 7, 2018.

Forsgren Associates, Inc., “Drainage Letter for Warner 4-Lot Subdivision,” August, 2021 (EPC File No. MS-21-004).

## **II. DRAINAGE BASINS AND SUB-BASINS**

### **A. Major Basin Description**

The proposed development lies completely within the West Kiowa Creek Drainage Basin as classified by El Paso County. Stormwater runoff from the property generally drains easterly to existing natural swales flowing to the West Kiowa Creek channel, which ultimately flows to a downstream confluence with the South Platte River.

### **B. Floodplain Impacts**

The project site is located beyond the limits of any 100-year floodplain delineated by the Federal Emergency Management Agency (FEMA). The floodplain limits in the vicinity of the site are shown in Flood Insurance Rate Map (FIRM) Panel Number 08041C0310G dated December 7, 2018 (see Firmette Exhibit in Appendix C).

### **C. Sub-Basin Description**

The existing drainage basins lying in and around the proposed development are depicted on Figure EX1 (Appendix E). The property has been delineated as four on-site drainage basins (Basins A-D).

Developed runoff in this minor subdivision will continue to follow historic paths.

### III. DRAINAGE DESIGN CRITERIA

#### A. Development Criteria Reference

No Drainage Basin Planning Study (DBPS) has been completed for the West Kiowa Creek Drainage Basin. No Master Development Drainage Plan (MDDP) reports were found for any adjacent subdivisions.

Forsgren Associates, Inc. prepared a drainage report for the adjoining rural residential subdivision to the west entitled “Drainage Letter for Warner 4-Lot Subdivision,” dated August, 2021. The Forsgren report concluded that “proposed improvements will not have any negative impacts on the existing site conditions or downstream conditions as extra flows from site are minimal.”

#### B. Hydrologic Criteria

The tributary drainage basins impacting this site are all less than 100 acres, so Rational Method Hydrology procedures were utilized for calculation of peak flows. Rational Method hydrologic calculations were based on the following assumptions:

- |                                      |                            |             |
|--------------------------------------|----------------------------|-------------|
| • Design storm (minor)               | 5-year                     |             |
| • Design storm (major)               | 100-year                   |             |
| • Rainfall Intensities               | El Paso County I-D-F Curve |             |
| • Hydrologic soil type               | B                          |             |
|                                      | <u>C5</u>                  | <u>C100</u> |
| • Runoff Coefficients - undeveloped: |                            |             |
| Existing meadow / forest areas       | 0.08                       | 0.35        |
| • Runoff Coefficients - developed:   |                            |             |
| Proposed lot areas (5-acre lots)     | 0.137                      | 0.393       |

Hydrologic calculations are enclosed in Appendix B, and peak design flows are identified on the drainage basin drawings.

### IV. DRAINAGE PLANNING FOUR STEP PROCESS

El Paso County Drainage Criteria require drainage planning to include a Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long-term source controls.

As stated in DCM Volume 2, the Four Step Process is applicable to all new and re-development projects with construction activities that disturb 1 acre or greater or that disturb less than 1 acre but are part of a larger common plan of development. The Four Step Process has been implemented as follows in the planning of this project:

#### Step 1: Employ Runoff Reduction Practices

- **Minimize Impacts:** The proposed minor rural residential subdivision is an inherently low impact development. The proposed 5-acre minimum lot sizes will minimize drainage impacts in comparison to higher density development alternatives.

#### Step 2: Stabilize Drainageways

- There are no major drainageways within the site. Vegetated buffer strips will be maintained between developed areas of the site and downstream drainage swales.
- Drainage basin fees will be paid at the time of recording of the subdivision plat, and these fees provide the applicable cost contribution towards regional drainage improvements.

#### Step 3: Provide Water Quality Capture Volume (WQCV)

- Water quality detention is not required based on the low density of the rural residential development proposed (average lot size of 5-acres). According to ECM Appendix I Section I.7.1.B.5, a single-family residential lot greater than or equal to 2.5 acres in size per dwelling and having a total lot impervious area of less than 10 percent is excluded from permanent WQ control measures. The presumptive impervious area for 5-acre lots in El Paso County is 7 percent, which meets the criteria for exclusion from water quality requirements.

#### Step 4: Consider Need for Industrial and Commercial BMPs

- No industrial or commercial land uses are proposed as part of this development.

### **V. GENERAL DRAINAGE RECOMMENDATIONS**

The developed drainage plan for the site is to provide and maintain positive drainage away from structures and conform to the established drainage patterns for the overall site. JPS Engineering recommends that positive drainage be established and maintained away from all structures within the site in conformance with applicable building codes and geotechnical engineering recommendations.

Individual lot grading is the sole responsibility of the individual builders and property owners. Final grading of each home site should establish proper protective slopes and positive drainage in accordance with applicable housing industry standards, HUD guidelines, and building codes. In general, main floor elevations for each home should be established a minimum of 2 feet above the adjoining street.

In general, we recommend a minimum of 6 inches clearance from the top of concrete foundation walls to adjacent finished site grades. Positive drainage slopes should be maintained away from all structures, with a minimum recommended slope of 5 percent for the first 10 feet away from buildings in landscaped areas, a minimum recommended slope of 2 percent for the first 10 feet away from buildings in paved areas, and a minimum slope of 1 percent for paved areas beyond buildings.

The enclosed “Developed Drainage Plan” includes the following General Drainage Notes to Builders and Property Owners:

1. Individual builders shall provide positive drainage away from structures and account for potential cross-lot drainage impacts within each lot.
2. Builders and property owners shall implement and maintain erosion control measures for protection of downstream properties and facilities including protection of existing vegetated buffer strips along the downstream property boundaries.
3. Builders and property owners shall implement and maintain drainage control measures to minimize impacts of concentrated drainage from individual home sites including level spreaders along downstream grading limits or similar techniques.

## **VI. DRAINAGE FACILITY DESIGN**

### **A. General Concept**

Development of the proposed minor subdivision will not require any public improvements or common subdivision infrastructure improvements. The general concept for management of developed storm runoff is for the individual home site grading to provide positive drainage away from the building pads and divert runoff to stable drainage swales following historic drainage patterns.

### **B. Specific Details**

#### **1. Existing Drainage Conditions**

Historic drainage conditions are depicted on Figure EX1 (Appendix E). The parcel is a historically vacant forest area. There are no existing drainage facilities within the property. There are no existing irrigation facilities, major utilities, or significant encumbrances impacting the site.

Basin A comprises the southwest corner of the property, which sheet flows south towards Goshawk Road. Existing peak flows at Design Point #1 are calculated as  $Q_5 = 0.3$  cfs and  $Q_{100} = 2.4$  cfs.

Basin B comprises the southeast corner of the property, which sheet flows south towards Goshawk Road. Existing peak flows at Design Point #2 are calculated as  $Q_5 = 0.4$  cfs and  $Q_{100} = 3.2$  cfs.

Basin C comprises the majority of the property, which drains northeasterly to the eastern property boundary by sheet flow and existing natural drainage swales. Off-site Basin OC1 comprises a small upstream area to the west (within Lot 4, Warner Subdivision), which sheet flows northeasterly into Basin C. Runoff from Basins OC1 and C combines at Design Point #3, with existing peak flows calculated as  $Q_5 = 4.9$  cfs and  $Q_{100} = 32.9$  cfs.



Drainage from Design Point #3 flows northeasterly to an existing stable vegetated drainage swale downstream.

Basin D comprises the northwest corner of the property, which sheet flows towards the north boundary of the site. Existing peak flows at Design Point #4 are calculated as  $Q_5 = 0.5$  cfs and  $Q_{100} = 3.8$  cfs.

## **2. Developed Drainage Conditions**

The developed drainage basins and projected flows are shown on the Developed Drainage Plan (Figure D1, Appendix E).

Developed flows from Basin A will continue to sheet flow south to Goshawk Road at Design Point #1, with developed peak flows calculated as  $Q_5 = 0.6$  cfs and  $Q_{100} = 2.7$  cfs (minimal 100-year flow increase of 0.3 cfs compared to existing conditions). Goshawk Road is a private gravel road in relatively poor condition. Roadway drainage generally flows to the east, and there is a minimal ditch along the north side of the existing road. With appropriate measures to minimize impacts of concentrated flows from individual home sites, the drainage impact on Goshawk Road will be minimal.

Developed flows from Basin B will continue to sheet flow southeasterly to Goshawk Road at Design Point #2, with developed peak flows calculated as  $Q_5 = 0.8$  cfs and  $Q_{100} = 3.7$  cfs (minimal 100-year flow increase of 0.5 cfs compared to existing conditions). Goshawk Road is a private gravel road in relatively poor condition. Roadway drainage generally flows to the east, and there is a minimal ditch along the north side of the existing road. With appropriate measures to minimize impacts of concentrated flows from individual home sites, the drainage impact on Goshawk Road will be minimal.

Developed flows from Basins OC1 and C will continue to sheet flow northeasterly to Design Point #3 along the east boundary of the proposed Lot 3. Developed peak flows at Design Point #3 are calculated as  $Q_5 = 7.5$  cfs and  $Q_{100} = 36.2$  cfs (minimal 100-year flow increase of 3.3 cfs, or 10.0 percent, compared to existing conditions).

Drainage from Design Point #3 flows to an existing stable, grass-lined drainage swale at the east boundary of the site. As detailed in Appendix C, the hydraulic capacity of the existing drainage swales within the property has been evaluated, and the existing grass-lined swales (designated as Channels C1.1, C1.2, and C1.3 on Sh. D1) are adequate to convey the developed flows. Proposed drainage easements have been delineated on the subdivision plat to prohibit building within the existing drainage swale areas.

The existing vegetated drainage channel immediately downstream of the eastern site boundary has been identified as Channel C1.4 on Sh. D1. According to the hydraulic calculations in Appendix C, the existing channel has adequate hydraulic capacity to safely convey the developed flows from this minor subdivision. The calculated channel hydraulic

properties (velocities and Froude Numbers) summarized in the “Channel Calculations” table in Appendix C show a negligible increase between the existing and developed flow conditions, indicating no significant developed drainage impact from the proposed subdivision. As depicted in the photograph in Appendix D, the existing outfall channel is in stable, well-vegetated condition.

Recognizing the vegetated condition of the existing property and the minimal increase in calculated flows, no on-site stormwater detention is proposed as is most commonly the case with rural residential lots at this density. As previously noted, we recommend implementation of appropriate drainage control measures during construction of individual home sites to minimize future impacts of concentrated drainage.

Developed flows from Basin D will continue to sheet flow northerly to Design Point #4 along the north property boundary, with developed peak flows calculated as  $Q_5 = 0.9$  cfs and  $Q_{100} = 4.3$  cfs (minimal 100-year flow increase of 0.5 cfs compared to existing conditions).

Recognizing the rural residential nature of the proposed subdivision (5-acre minimum lot sizes in full compliance with existing zoning), the minor increase in developed flows will have no significant drainage impact in the West Kiowa Creek Drainage Basin. Future construction of the rural residential home sites should implement appropriate drainage and erosion control measures and maximize preservation of vegetated buffer strips along the downstream sides of the property.

### **C. On-Site Drainage Facility Design**

Developed drainage basins and drainage patterns are depicted on the enclosed Developed Drainage Plan (Sheet D1, Appendix E). No public drainage improvements are required for this minor subdivision. Based on the rural residential nature of this minor subdivision and the large lot sizes proposed, there will be no significant increase in developed flows, and there is no need for on-site flood control detention.

### **D. Analysis of Existing and Proposed Downstream Facilities**

The proposed subdivision area will drain to existing natural drainage swales flowing to the West Kiowa Creek Drainage Basin. Development of this property as a rural residential subdivision will have no significant impact on downstream drainage facilities.

### **E. Anticipated Drainage Problems and Solutions**

The drainage plan for this minor subdivision consists of maintaining positive drainage away from home sites and conveying surface drainage through the site in general conformance with historic drainage patterns. The primary drainage problems anticipated within this type of development consist of maintenance of proper drainage patterns and erosion control.

Care will need to be taken to implement proper erosion control measures associated with the proposed driveways, home sites, and drainage swales. Vegetated buffer strips should be maintained along downstream property boundaries to the greatest extent possible. Drainage facilities outside the public right-of-way will be owned and maintained by the individual lot owners unless otherwise noted.

## **VII. EROSION CONTROL / SEDIMENT CONTROL**

Contractors and Owners will need to implement and maintain proper Construction Control Measures (CCM's) for erosion and sediment control during and after construction. Erosion control measures should include installation of silt fence along the downstream edge of disturbed areas, sediment control logs protecting drainage ditches, vehicle tracking control pads at access points, riprap protection at culvert outlets, and revegetation of disturbed areas. Cut slopes will need to be stabilized during excavation as necessary and vegetation will need to be re-established as soon as possible for stabilization of graded areas.

## **VIII. STORMWATER DETENTION AND WATER QUALITY**

As previously stated, the proposed development will result in a minimal increase in developed flows based on the rural residential development plan. There is no need for on-site stormwater detention based on the minimal developed drainage impact.

Water quality facilities are not required as this site meets exclusions listed in the revised El Paso County Engineering Criteria Manual (ECM). Section I.7.1.B.5 of the ECM identifies "Large Lot Single Family Sites" as excluded sites under the following definition: "A single-family residential lot, or agricultural zoned lands, greater than or equal to 2.5 acres in size per dwelling and having a total lot impervious area of less than 10 percent." The proposed subdivision plat consists of 5-acre minimum lot sizes with a presumptive impervious area of 7 percent, which is below the "10 percent" threshold.

As noted on the enclosed "Developed Drainage Plan," builders and property owners shall implement and maintain appropriate drainage control measures to minimize impacts of concentrated drainage from individual home sites through use of level spreaders and similar techniques along the downstream edges of the disturbed home site areas.

## **IX. DRAINAGE FEES**

The developer will be responsible for all construction costs associated with subdivision infrastructure improvements. There are no reimbursable public drainage improvements required for this subdivision.

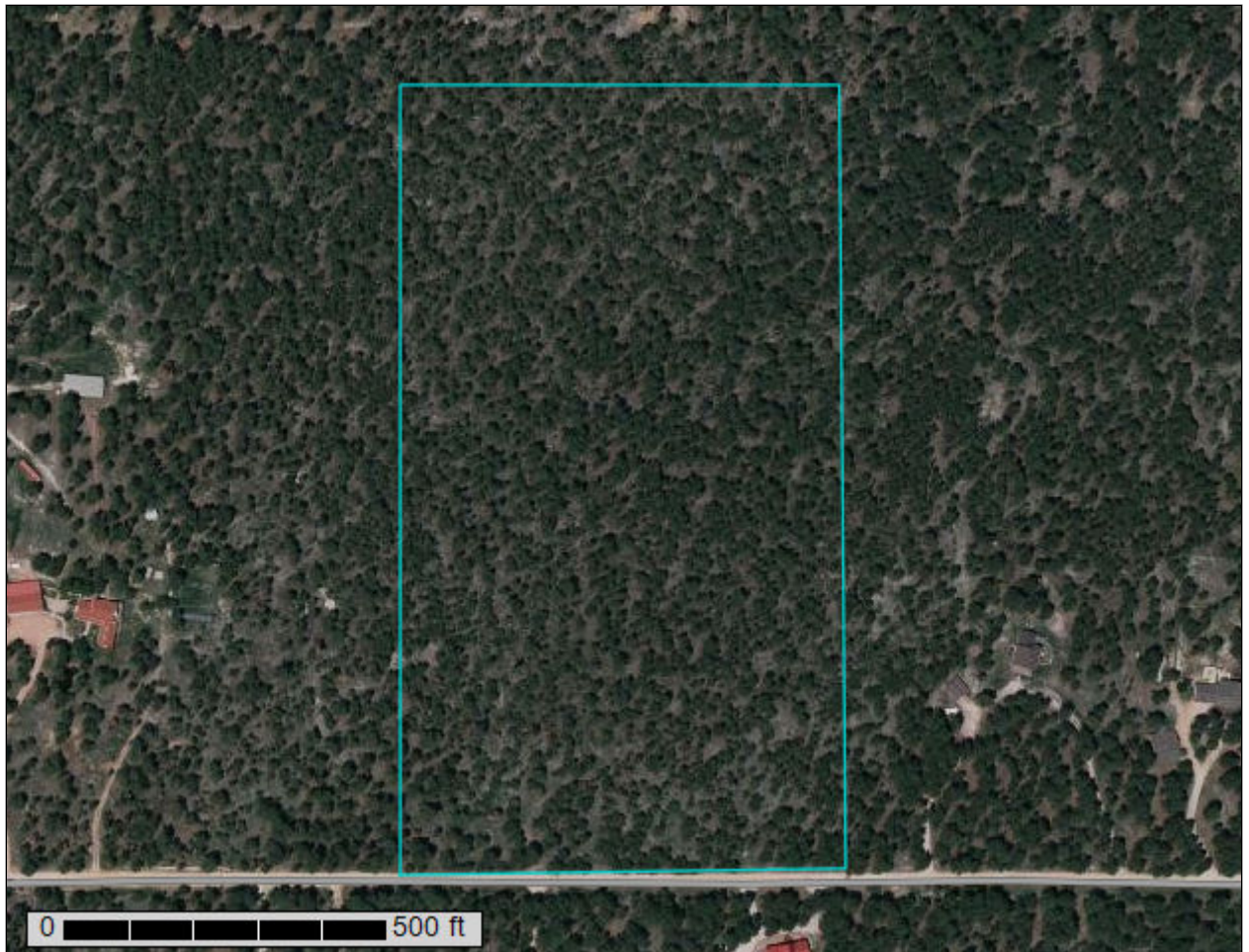
This parcel is located in the West Kiowa Creek Drainage Basin. No drainage and bridge fees will be due at the time of recordation of the final plat as the subject site is not located in a fee basin.

## **X. SUMMARY**

Berisford Subdivision is a proposed rural residential subdivision consisting of 4 lots on a 20-acre site. Development of the proposed minor subdivision is anticipated to result in a minimal increase in developed runoff from the site, and proper erosion and sediment control measures will be implemented by the individual owners and builders to mitigate developed drainage impacts. The proposed drainage patterns will remain consistent with historic conditions. Implementation and maintenance of proper erosion control measures will ensure that this minor subdivision has no significant adverse drainage impact on downstream properties or drainage facilities.

**APPENDIX A**  
**SOILS INFORMATION**

# Custom Soil Resource Report for El Paso County Area, Colorado



# Preface

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Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist ([http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\\_053951](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951)).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# Contents

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<b>Preface</b> .....	2
<b>How Soil Surveys Are Made</b> .....	5
<b>Soil Map</b> .....	8
Soil Map.....	9
Legend.....	10
Map Unit Legend.....	11
Map Unit Descriptions.....	11
El Paso County Area, Colorado.....	13
26—Elbeth sandy loam, 8 to 15 percent slopes.....	13
<b>References</b> .....	15

# How Soil Surveys Are Made

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Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

## Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

## Custom Soil Resource Report

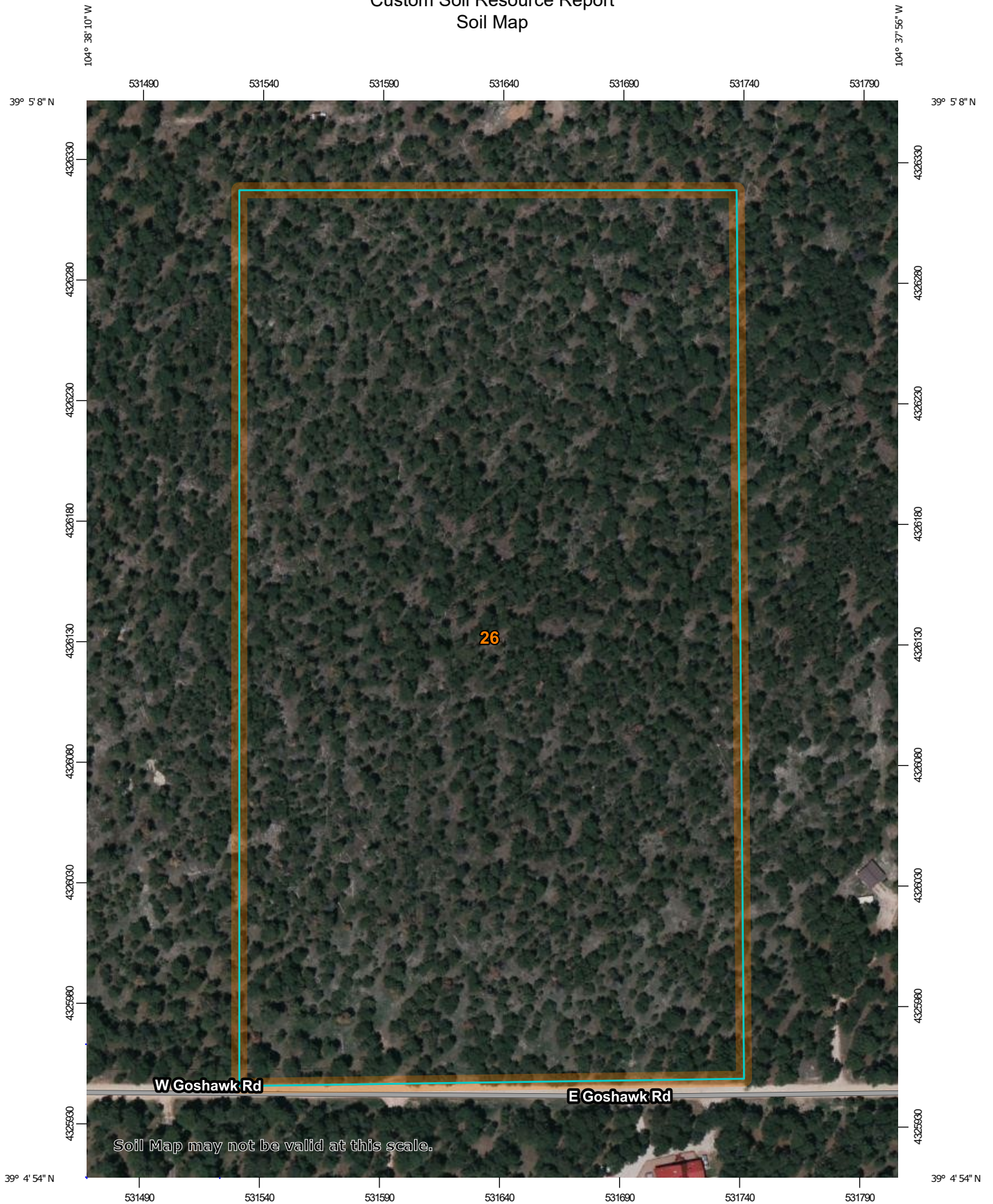
identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

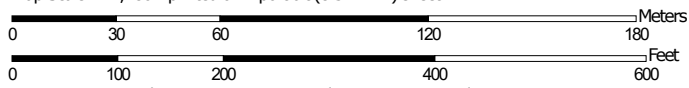
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The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

# Custom Soil Resource Report Soil Map




Map Scale: 1:2,180 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84


### MAP LEGEND


**Area of Interest (AOI)**

 Area of Interest (AOI)

**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features

**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
 Survey Area Data: Version 21, Aug 24, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 9, 2021—Jun 12, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
26	Elbeth sandy loam, 8 to 15 percent slopes	19.2	100.0%
<b>Totals for Area of Interest</b>		<b>19.2</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.



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An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## El Paso County Area, Colorado

### 26—Elbeth sandy loam, 8 to 15 percent slopes

#### Map Unit Setting

*National map unit symbol:* 367y  
*Elevation:* 7,300 to 7,600 feet  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Elbeth and similar soils:* 85 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Elbeth

##### Setting

*Landform:* Hills  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Alluvium derived from arkose

##### Typical profile

*A - 0 to 3 inches:* sandy loam  
*E - 3 to 23 inches:* loamy sand  
*Bt - 23 to 68 inches:* sandy clay loam  
*C - 68 to 74 inches:* sandy clay loam

##### Properties and qualities

*Slope:* 8 to 15 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high (0.20 to 0.60 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Moderate (about 7.1 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 4e  
*Hydrologic Soil Group:* B  
*Ecological site:* F048AY908CO - Mixed Conifer  
*Hydric soil rating:* No

#### Minor Components

##### Other soils

*Percent of map unit:*  
*Hydric soil rating:* No

##### Pleasant

*Percent of map unit:*  
*Landform:* Depressions

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*Hydric soil rating:* Yes

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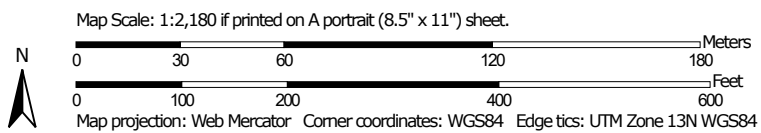
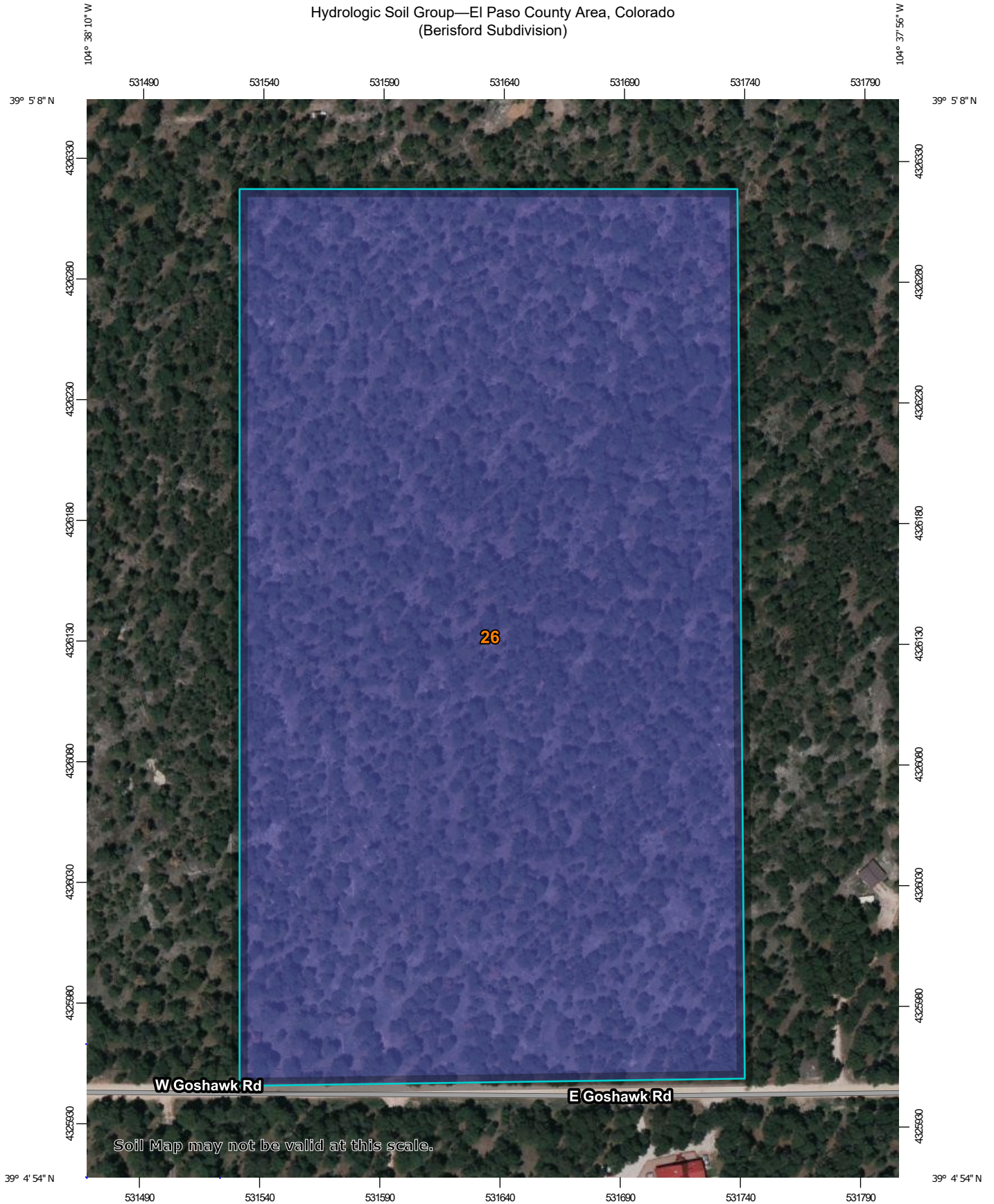
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Hydrologic Soil Group—El Paso County Area, Colorado  
(Berisford Subdivision)



Hydrologic Soil Group—El Paso County Area, Colorado  
(Berisford Subdivision)

### MAP LEGEND

**Area of Interest (AOI)**









 Area of Interest (AOI)

**Soils**

**Soil Rating Polygons**





-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

**Soil Rating Lines**


-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

**Soil Rating Points**






-  A
-  A/D
-  B
-  B/D

-  C
-  C/D
-  D
-  Not rated or not available

**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

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## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
26	Elbeth sandy loam, 8 to 15 percent slopes	B	19.2	100.0%
<b>Totals for Area of Interest</b>			<b>19.2</b>	<b>100.0%</b>

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### Rating Options

*Aggregation Method: Dominant Condition*

*Component Percent Cutoff: None Specified*



*Tie-break Rule:* Higher

**APPENDIX B**  
**HYDROLOGIC CALCULATIONS**

**Table 6-6. Runoff Coefficients for Rational Method**

(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

### 3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration ( $t_c$ ) consists of an initial time or overland flow time ( $t_i$ ) plus the travel time ( $t_r$ ) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time ( $t_i$ ) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion ( $t_r$ ) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

$$t_c = t_i + t_t \quad (\text{Eq. 6-7})$$

Where:

$t_c$  = time of concentration (min)

$t_i$  = overland (initial) flow time (min)

$t_t$  = travel time in the ditch, channel, gutter, storm sewer, etc. (min)

### 3.2.1 Overland (Initial) Flow Time

The overland flow time,  $t_i$ , may be calculated using Equation 6-8.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad (\text{Eq. 6-8})$$

Where:

$t_i$  = overland (initial) flow time (min)

$C_5$  = runoff coefficient for 5-year frequency (see Table 6-6)

$L$  = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)

$S$  = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

### 3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time,  $t_t$ , which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time,  $t_t$ , can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad (\text{Eq. 6-9})$$

Where:

$V$  = velocity (ft/s)

$C_v$  = conveyance coefficient (from Table 6-7)

$S_w$  = watercourse slope (ft/ft)

**Table 6-7. Conveyance Coefficient,  $C_v$** 

Type of Land Surface	$C_v$
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

\* For buried riprap, select  $C_v$  value based on type of vegetative cover.

The travel time is calculated by dividing the flow distance (in feet) by the velocity calculated using Equation 6-9 and converting units to minutes.

The time of concentration ( $t_c$ ) is then the sum of the overland flow time ( $t_i$ ) and the travel time ( $t_t$ ) per Equation 6-7.

### 3.2.3 First Design Point Time of Concentration in Urban Catchments

Using this procedure, the time of concentration at the first design point (typically the first inlet in the system) in an urbanized catchment should not exceed the time of concentration calculated using Equation 6-10. The first design point is defined as the point where runoff first enters the storm sewer system.

$$t_c = \frac{L}{180} + 10 \quad (\text{Eq. 6-10})$$

Where:

$t_c$  = maximum time of concentration at the first design point in an urban watershed (min)

$L$  = waterway length (ft)

Equation 6-10 was developed using the rainfall-runoff data collected in the Denver region and, in essence, represents regional “calibration” of the Rational Method. Normally, Equation 6-10 will result in a lesser time of concentration at the first design point and will govern in an urbanized watershed. For subsequent design points, the time of concentration is calculated by accumulating the travel times in downstream drainageway reaches.

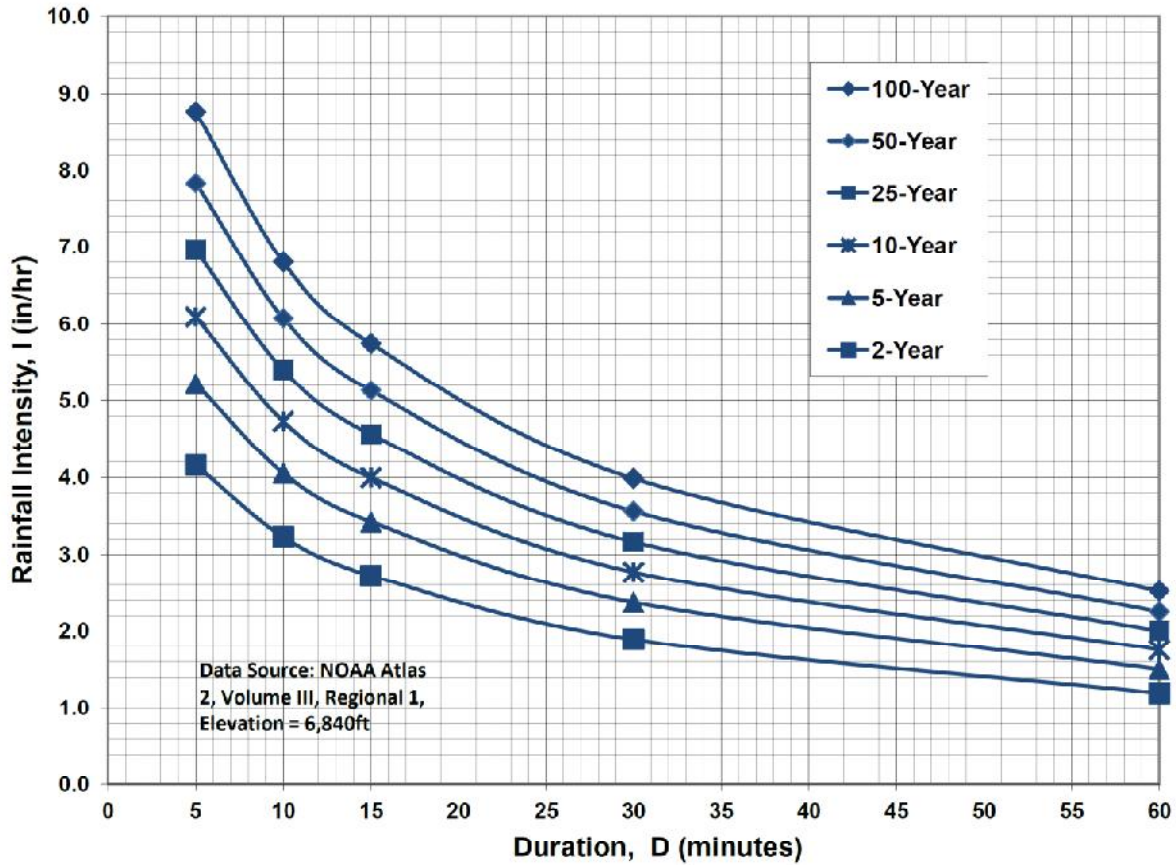
### 3.2.4 Minimum Time of Concentration

If the calculations result in a  $t_c$  of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum  $t_c$  for urbanized areas is 5 minutes.

### 3.2.5 Post-Development Time of Concentration

As Equation 6-8 indicates, the time of concentration is a function of the 5-year runoff coefficient for a drainage basin. Typically, higher levels of imperviousness (higher 5-year runoff coefficients) correspond to shorter times of concentration, and lower levels of imperviousness correspond to longer times of

**Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency**



**IDF Equations**

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

BERISFORD SUBDIVISION												
COMPOSITE RUNOFF COEFFICIENTS - TYPICAL 5-ACRE DEVELOPED RESIDENTIAL AREA												
DEVELOPED CONDITIONS												
100-YEAR C VALUES												
BASIN	TOTAL AREA (AC)	SOIL TYPE	AREA (%)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (%)	SUB-AREA 2 DEVELOPMENT/ COVER	C	AREA (%)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
5-ACRE LOTS	5.00	B	7.00	BLDG/DRIVEWAY	0.9	93.00	LAWN/MEADOW	0.08				0.137
100-YEAR C VALUES												
BASIN	TOTAL AREA (AC)	SOIL TYPE	AREA (%)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (%)	SUB-AREA 2 DEVELOPMENT/ COVER	C	AREA (%)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
5-ACRE LOTS	5.00	B	7.00	BLDG/DRIVEWAY	0.96	93.00	LAWN/MEADOW	0.35				0.393

**BERISFORD SUBDIVISION  
RATIONAL METHOD**

**EXISTING CONDITIONS FLOWS**

BASIN	DESIGN POINT	AREA (AC)	C		Overland Flow			Channel flow					TOTAL Tc <sup>(4)</sup> (MIN)	TOTAL Tc <sup>(4)</sup> (MIN)	INTENSITY <sup>(5)</sup>		PEAK FLOW	
			5-YEAR	100-YEAR	LENGTH (FT)	SLOPE (FT/FT)	Tco <sup>(1)</sup> (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT C	SLOPE (FT/FT)	SCS <sup>(2)</sup> VELOCITY (FT/S)	Tt <sup>(3)</sup> (MIN)			5-YR (IN/HR)	100-YR (IN/HR)	Q5 <sup>(6)</sup> (CFS)	Q100 <sup>(6)</sup> (CFS)
			A	1	1.1	0.080	0.350	100	0.030	13.0	120	15			0.042	3.07	0.7	13.6
B	2	1.6	0.080	0.350	100	0.020	14.8	150	15	0.033	2.72	0.9	15.7	15.7	3.45	5.79	0.44	3.24
OC1		3.3	0.137	0.393	300	0.057	17.1	185	15	0.081	4.27	0.7	17.8	17.8	3.26	5.48	1.47	7.10
Tt OC1 to DP3								750	15	0.045	3.18	3.9						
C		15.2	0.080	0.350	100	0.040	11.8	1020	15	0.053	3.45	4.9	16.7	16.7	3.36	5.64	4.09	30.01
OC1,C	3	18.5	0.090	0.358									21.8	21.8	2.96	4.97	4.94	32.91
D	4	1.9	0.080	0.350	100	0.020	14.8	310	15	0.048	3.29	1.6	16.4	16.4	3.39	5.69	0.51	3.78

**DEVELOPED FLOWS**

BASIN	DESIGN POINT	AREA (AC)	C		Overland Flow			Channel flow					TOTAL Tc <sup>(4)</sup> (MIN)	TOTAL Tc <sup>(4)</sup> (MIN)	INTENSITY <sup>(5)</sup>		PEAK FLOW	
			5-YEAR	100-YEAR	LENGTH (FT)	SLOPE (FT/FT)	Tco <sup>(1)</sup> (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT C	SLOPE (FT/FT)	SCS <sup>(2)</sup> VELOCITY (FT/S)	Tt <sup>(3)</sup> (MIN)			5-YR (IN/HR)	100-YR (IN/HR)	Q5 <sup>(6)</sup> (CFS)	Q100 <sup>(6)</sup> (CFS)
			A	1	1.1	0.137	0.393	100	0.030	12.2	120	15			0.042	3.07	0.7	12.9
B	2	1.6	0.137	0.393	100	0.020	14.0	150	15	0.033	2.72	0.9	14.9	14.9	3.53	5.93	0.77	3.73
OC1		3.3	0.137	0.393	300	0.057	17.1	185	15	0.081	4.27	0.7	17.8	17.8	3.26	5.48	1.47	7.10
Tt OC1 to DP3								750	15	0.045	3.18	3.9						
C		15.2	0.137	0.393	100	0.040	11.1	1020	15	0.053	3.45	4.9	16.0	16.0	3.42	5.74	7.12	34.31
OC1,C	3	18.5	0.137	0.393									21.8	21.8	2.96	4.97	7.51	36.16
D	4	1.9	0.137	0.393	100	0.020	14.0	310	15	0.048	3.29	1.6	15.6	15.6	3.47	5.82	0.90	4.34

1) OVERLAND FLOW Tco = (0.395\*(1.1-RUNOFF COEFFICIENT))\*(OVERLAND FLOW LENGTH^(0.5))/(SLOPE^(0.333))

- 2) SCS VELOCITY = C \* ((SLOPE(FT/FT))^0.5)  
 C = 2.5 FOR HEAVY MEADOW  
 C = 5 FOR TILLAGE/FIELD  
 C = 7 FOR SHORT PASTURE AND LAWNS  
 C = 10 FOR NEARLY BARE GROUND  
 C = 15 FOR GRASSED WATERWAY  
 C = 20 FOR PAVED AREAS AND SHALLOW PAVED SWALES

3) MANNING'S CHANNEL TRAVEL TIME = L/V (WHEN CHANNEL VELOCITY IS KNOWN)

4) Tc = Tco + Tt

\*\*\* IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED

5) INTENSITY BASED ON I-D-F EQUATIONS IN CITY OF COLORADO SPRINGS DRAINAGE CRITERIA MANUAL

$$I_5 = -1.5 * \ln(Tc) + 7.583$$

$$I_{100} = -2.52 * \ln(Tc) + 12.735$$

6) Q = CiA



**APPENDIX C**  
**HYDRAULIC CALCULATIONS**

**BERISFORD SUBDIVISION  
CHANNEL CALCULATIONS**

**CHANNELS - EXISTING FLOWS**

CHANNEL	DESIGN POINT	SLOPE (FT/FT)	BOTTOM WIDTH (B, FT)	SIDE SLOPE (Z)	CHANNEL DEPTH (FT)	FRICTION FACTOR (n)	DP/ BASIN	Q100 FLOW (CFS)	CHANNEL FLOW % OF BASIN	CHANNEL FLOW (CFS)	Q100 DEPTH (FT)	Q100 VELOCITY (FT/S)	TOP WIDTH (FT)	FROUDE NUMBER	EASEMENT WIDTH (FT)	CHANNEL LINING
CHANNEL C1.1	DP3	0.048	10	4:1	2.0	0.030	DP3	32.9	40	13.2	0.3	4.3	12.2	1.51	30.0	GRASS
CHANNEL C1.2	DP3	0.054	4	4:1	2.0	0.030	C	30.0	15	4.5	0.2	3.9	5.9	1.54	30.0	GRASS
CHANNEL C1.3	DP3	0.047	20	10:1	2.0	0.030	DP3	32.9	100	32.9	0.3	4.5	26.3	1.53	50.0	GRASS
CHANNEL C1.4	DP3	0.050	24	7.5	2.0	0.030	DP3	32.9	100	32.9	0.3	4.5	28.2	1.56	50.0	GRASS

**CHANNELS - DEVELOPED FLOWS**

CHANNEL	DESIGN POINT	SLOPE (FT/FT)	BOTTOM WIDTH (B, FT)	SIDE SLOPE (Z)	CHANNEL DEPTH (FT)	FRICTION FACTOR (n)	DP/ BASIN	Q100 FLOW (CFS)	CHANNEL FLOW % OF BASIN	CHANNEL FLOW (CFS)	Q100 DEPTH (FT)	Q100 VELOCITY (FT/S)	TOP WIDTH (FT)	FROUDE NUMBER	EASEMENT WIDTH (FT)	CHANNEL LINING
CHANNEL C1.1	DP3	0.048	10	4:1	2.0	0.030	DP3	36.2	40	14.5	0.3	4.5	12.3	1.53	30.0	GRASS
CHANNEL C1.2	DP3	0.054	4	4:1	2.0	0.030	C	34.3	15	5.1	0.3	4.0	6.0	1.55	30.0	GRASS
CHANNEL C1.3	DP3	0.047	20	10:1	2.0	0.030	DP3	36.2	100	36.2	0.3	4.7	26.6	1.54	50.0	GRASS
CHANNEL C1.4	DP3	0.050	24	7.5	2.0	0.030	DP3	36.2	100	36.2	0.3	4.7	28.4	1.57	50.0	GRASS

- 1) Channel flow calculations based on Manning's Equation
- 2) n = 0.03 for grass-lined non-irrigated channels (minimum)
- 3) Vmax = 5.0 fps for 100-year flows w/ grass-lined channels
- 4) Vmax = 8.0 fps for 100-year flows w/ Erosion Control Blankets / Turf Reinforcement Mats (Eronet SC150 or equal)

# Hydraulic Analysis Report

## Project Data

Project Title: Berisford Subdivision - Existing Flows  
Designer: JPS  
Project Date: Saturday, September 7, 2024  
Project Units: U.S. Customary Units  
Notes:

## Channel Analysis: Channel Analysis - C1.1

Notes:

## Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 4.0000 ft/ft  
Side Slope 2 (Z2): 4.0000 ft/ft  
Channel Width: 10.0000 ft  
Longitudinal Slope: 0.0480 ft/ft  
Manning's n: 0.0300  
Flow: 13.2000 cfs

## Result Parameters

Depth: 0.2761 ft  
Area of Flow: 3.0664 ft<sup>2</sup>  
Wetted Perimeter: 12.2771 ft  
Hydraulic Radius: 0.2498 ft  
Average Velocity: 4.3048 ft/s  
Top Width: 12.2091 ft  
Froude Number: 1.5137  
Critical Depth: 0.3597 ft  
Critical Velocity: 3.2079 ft/s  
Critical Slope: 0.0194 ft/ft  
Critical Top Width: 12.88 ft  
Calculated Max Shear Stress: 0.8271 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.7481 lb/ft<sup>2</sup>

## Channel Analysis: Channel Analysis - C1.2

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 4.0000 ft/ft  
Side Slope 2 (Z2): 4.0000 ft/ft  
Channel Width: 4.0000 ft  
Longitudinal Slope: 0.0540 ft/ft  
Manning's n: 0.0300  
Flow: 4.5000 cfs

### Result Parameters

Depth: 0.2350 ft  
Area of Flow: 1.1608 ft<sup>2</sup>  
Wetted Perimeter: 5.9377 ft  
Hydraulic Radius: 0.1955 ft  
Average Velocity: 3.8767 ft/s  
Top Width: 5.8798 ft  
Froude Number: 1.5376  
Critical Depth: 0.3053 ft  
Critical Velocity: 2.8227 ft/s  
Critical Slope: 0.0212 ft/ft  
Critical Top Width: 6.44 ft  
Calculated Max Shear Stress: 0.7918 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.6587 lb/ft<sup>2</sup>

## Channel Analysis: Channel Analysis - C1.3

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 10.0000 ft/ft  
Side Slope 2 (Z2): 10.0000 ft/ft  
Channel Width: 20.0000 ft  
Longitudinal Slope: 0.0470 ft/ft  
Manning's n: 0.0300  
Flow: 32.9000 cfs

### Result Parameters

Depth: 0.3129 ft  
Area of Flow: 7.2382 ft<sup>2</sup>  
Wetted Perimeter: 26.2901 ft  
Hydraulic Radius: 0.2753 ft  
Average Velocity: 4.5453 ft/s  
Top Width: 26.2589 ft  
Froude Number: 1.5257  
Critical Depth: 0.4078 ft  
Critical Velocity: 3.3511 ft/s  
Critical Slope: 0.0187 ft/ft  
Critical Top Width: 28.16 ft  
Calculated Max Shear Stress: 0.9178 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.8075 lb/ft<sup>2</sup>

## Channel Analysis: Channel Analysis - C1.4

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 7.5000 ft/ft  
Side Slope 2 (Z2): 7.5000 ft/ft  
Channel Width: 24.0000 ft  
Longitudinal Slope: 0.0500 ft/ft  
Manning's n: 0.0300  
Flow: 32.9000 cfs

### Result Parameters

Depth: 0.2802 ft  
Area of Flow: 7.3132 ft<sup>2</sup>  
Wetted Perimeter: 28.2400 ft  
Hydraulic Radius: 0.2590 ft  
Average Velocity: 4.4987 ft/s  
Top Width: 28.2028 ft  
Froude Number: 1.5569  
Critical Depth: 0.3725 ft  
Critical Velocity: 3.2960 ft/s  
Critical Slope: 0.0189 ft/ft  
Critical Top Width: 29.59 ft  
Calculated Max Shear Stress: 0.8742 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.8080 lb/ft<sup>2</sup>

# Hydraulic Analysis Report

## Project Data

Project Title: Berisford Subdivision - Developed Flows  
Designer: JPS  
Project Date: Saturday, September 7, 2024  
Project Units: U.S. Customary Units  
Notes:

## Channel Analysis: Channel Analysis - C1.1

Notes:

## Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 4.0000 ft/ft  
Side Slope 2 (Z2): 4.0000 ft/ft  
Channel Width: 10.0000 ft  
Longitudinal Slope: 0.0480 ft/ft  
Manning's n: 0.0300  
Flow: 14.5000 cfs

## Result Parameters

Depth: 0.2918 ft  
Area of Flow: 3.2580 ft<sup>2</sup>  
Wetted Perimeter: 12.4059 ft  
Hydraulic Radius: 0.2626 ft  
Average Velocity: 4.4505 ft/s  
Top Width: 12.3340 ft  
Froude Number: 1.5260  
Critical Depth: 0.3818 ft  
Critical Velocity: 3.2949 ft/s  
Critical Slope: 0.0190 ft/ft  
Critical Top Width: 13.05 ft  
Calculated Max Shear Stress: 0.8739 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.7866 lb/ft<sup>2</sup>

## Channel Analysis: Channel Analysis - C1.2

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 4.0000 ft/ft  
Side Slope 2 (Z2): 4.0000 ft/ft  
Channel Width: 4.0000 ft  
Longitudinal Slope: 0.0540 ft/ft  
Manning's n: 0.0300  
Flow: 5.1000 cfs

### Result Parameters

Depth: 0.2522 ft  
Area of Flow: 1.2633 ft<sup>2</sup>  
Wetted Perimeter: 6.0799 ft  
Hydraulic Radius: 0.2078 ft  
Average Velocity: 4.0369 ft/s  
Top Width: 6.0178 ft  
Froude Number: 1.5527  
Critical Depth: 0.3291 ft  
Critical Velocity: 2.9145 ft/s  
Critical Slope: 0.0208 ft/ft  
Critical Top Width: 6.63 ft  
Calculated Max Shear Stress: 0.8499 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.7002 lb/ft<sup>2</sup>



## Channel Analysis: Channel Analysis - C1.3

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 10.0000 ft/ft  
Side Slope 2 (Z2): 10.0000 ft/ft  
Channel Width: 20.0000 ft  
Longitudinal Slope: 0.0470 ft/ft  
Manning's n: 0.0300  
Flow: 36.2000 cfs

### Result Parameters

Depth: 0.3306 ft  
Area of Flow: 7.7061 ft<sup>2</sup>  
Wetted Perimeter: 26.6459 ft  
Hydraulic Radius: 0.2892 ft  
Average Velocity: 4.6976 ft/s  
Top Width: 26.6129 ft  
Froude Number: 1.5384  
Critical Depth: 0.4327 ft  
Critical Velocity: 3.4394 ft/s  
Critical Slope: 0.0184 ft/ft  
Critical Top Width: 28.65 ft  
Calculated Max Shear Stress: 0.9697 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.8482 lb/ft<sup>2</sup>

## Channel Analysis: Channel Analysis - C1.4

Notes:

### Input Parameters

Channel Type: Trapezoidal  
Side Slope 1 (Z1): 7.5000 ft/ft  
Side Slope 2 (Z2): 7.5000 ft/ft  
Channel Width: 24.0000 ft  
Longitudinal Slope: 0.0500 ft/ft  
Manning's n: 0.0300  
Flow: 36.2000 cfs

### Result Parameters

Depth: 0.2963 ft  
Area of Flow: 7.7690 ft<sup>2</sup>  
Wetted Perimeter: 28.4835 ft  
Hydraulic Radius: 0.2728 ft  
Average Velocity: 4.6595 ft/s  
Top Width: 28.4442 ft  
Froude Number: 1.5712  
Critical Depth: 0.3960 ft  
Critical Velocity: 3.3893 ft/s  
Critical Slope: 0.0185 ft/ft  
Critical Top Width: 29.94 ft  
Calculated Max Shear Stress: 0.9244 lb/ft<sup>2</sup>  
Calculated Avg Shear Stress: 0.8510 lb/ft<sup>2</sup>

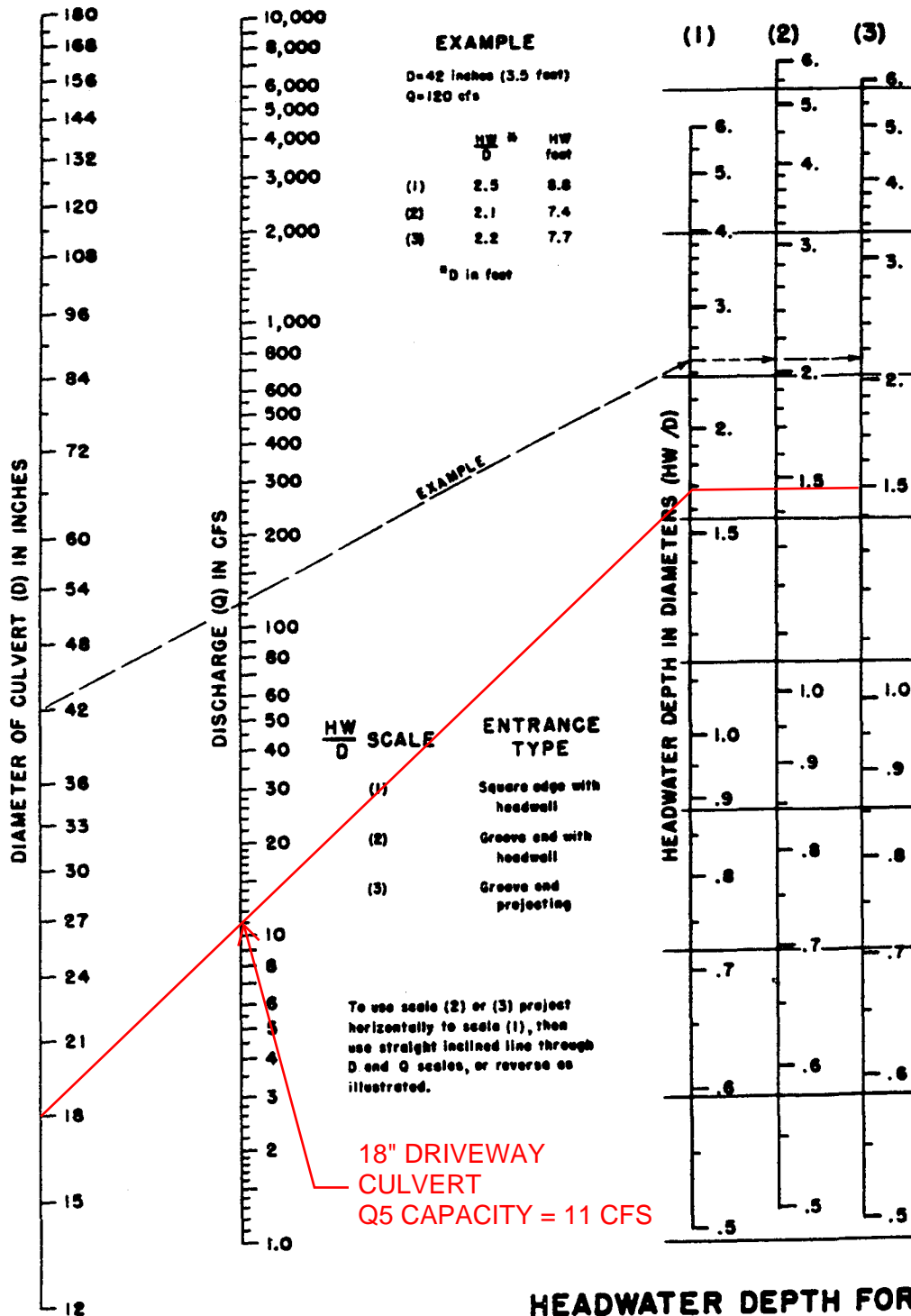
**BERISFORD SUBDIVISION  
DRIVEWAY CULVERT SIZING SUMMARY**

PRIVATE CULVERT	DP	Q5 FLOW (CFS)	CULVERT SIZE (IN)
C1.1	A1.1	3.5	18

\* CULVERT SIZING BASED ON EPC DCM, FIG. 9-34; ASSUMING MAX. ALLOWABLE HW/D = 1.5 FOR Q5

\* Q5 FLOW FOR CULVERT C1.1 CALCULATED AS 40% \* DP3 FLOW = (40% \* 7.5) - 3.0 CFS

# PRIVATE DRIVEWAY CULVERT C1.1



## HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 2&3  
REVISED MAY 1984

BUREAU OF PUBLIC ROADS JAN. 1963

The City of Colorado Springs / El Paso County  
Drainage Criteria Manual

Date  
**OCT. 1987**

Figure  
9-34



HDR Infrastructure, Inc.  
A Centerra Company

**APPENDIX D**  
**SITE PHOTOGRAPHS**



Typical vegetation within Berisford Subdivision property



Existing "Channel C1.3" facing downstream towards eastern site boundary





Facing east along Goshawk Road

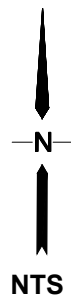
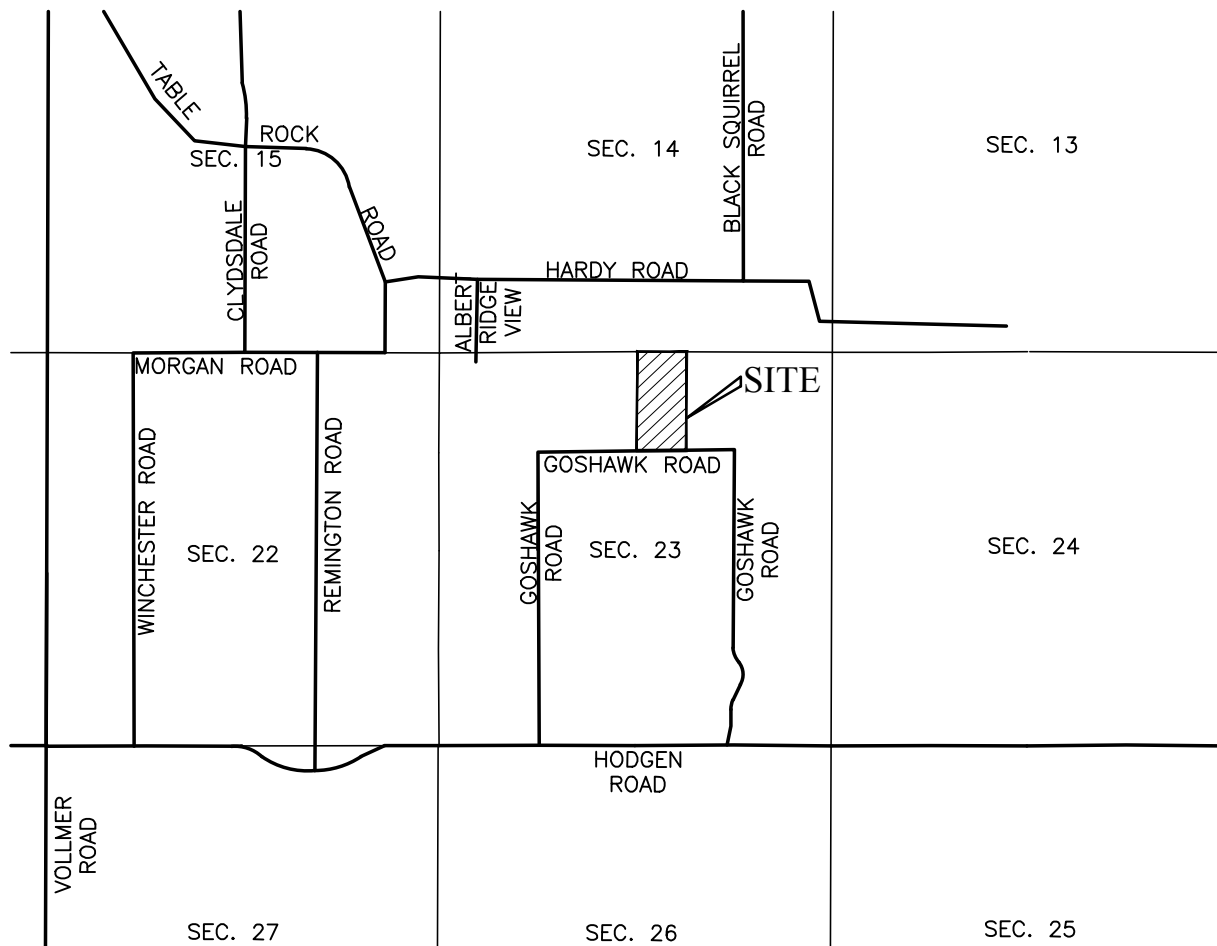


Facing west along Goshawk Road

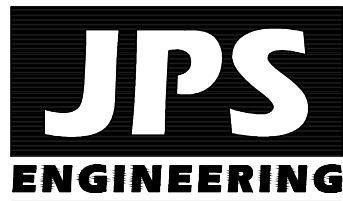
**APPENDIX E**

**FIGURES**





VICINITY MAP



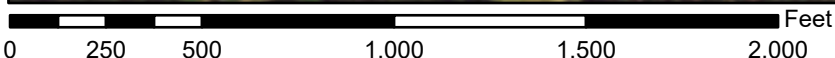
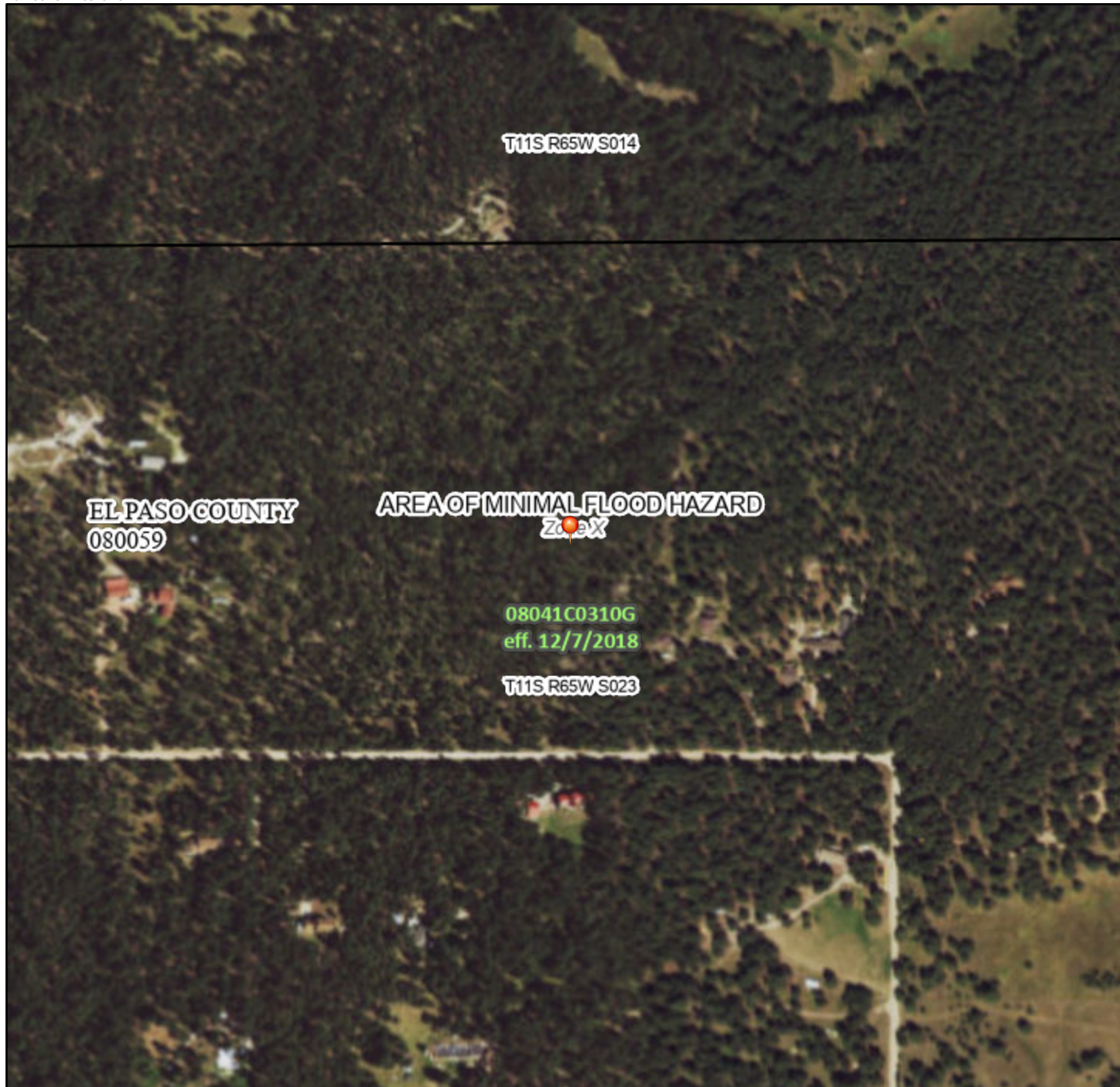
BERISFORD SUBDIVISION  
EL PASO COUNTY, CO

FIGURE A1  
JPS PROJ NO. 012401

# National Flood Hazard Layer FIRMette



104°38'19"W 39°5'15"N



1:6,000

104°37'41"W 39°4'47"N

Basemap Imagery Source: USGS National Map 2023

## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

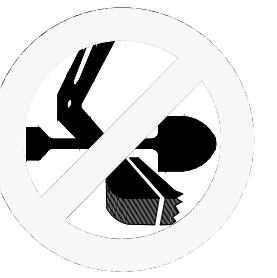
SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>
		With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
		Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
		Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
		Area with Flood Risk due to Levee <i>Zone D</i>
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i>
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard <i>Zone D</i>
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
MAP PANELS		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped
		The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 4/25/2024 at 1:20 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

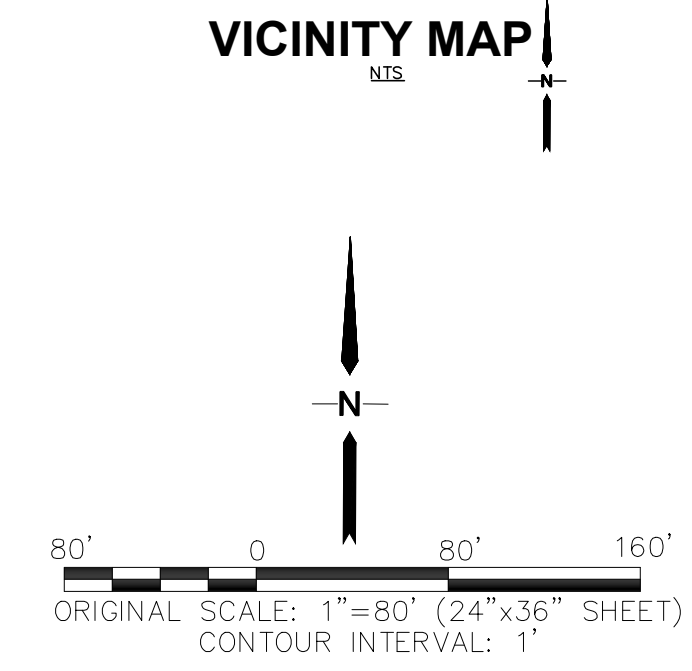
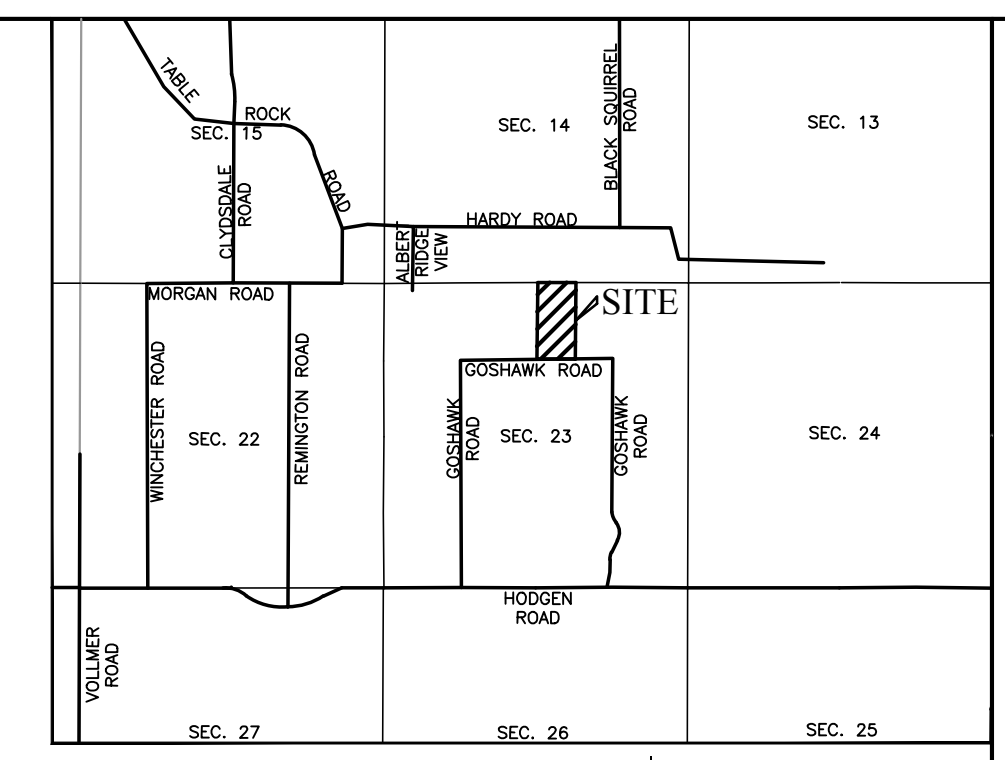


CALL UTILITY NOTIFICATION  
CENTER OF COLORADO  
**1-800-922-1987**  
CALL 2-BUSINESS DAYS IN ADVANCE  
BEFORE YOU DIG, GRADE, OR EXCAVATE  
FOR THE MARKING OF UNDERGROUND  
MEMBER UTILITIES.

No.	REVISION	BY	DATE

**EXISTING CONDITIONS  
DRAINAGE PLAN**

**BERISFORD SUBDIVISION  
EL PASO COUNTY, CO**



**LEGEND**

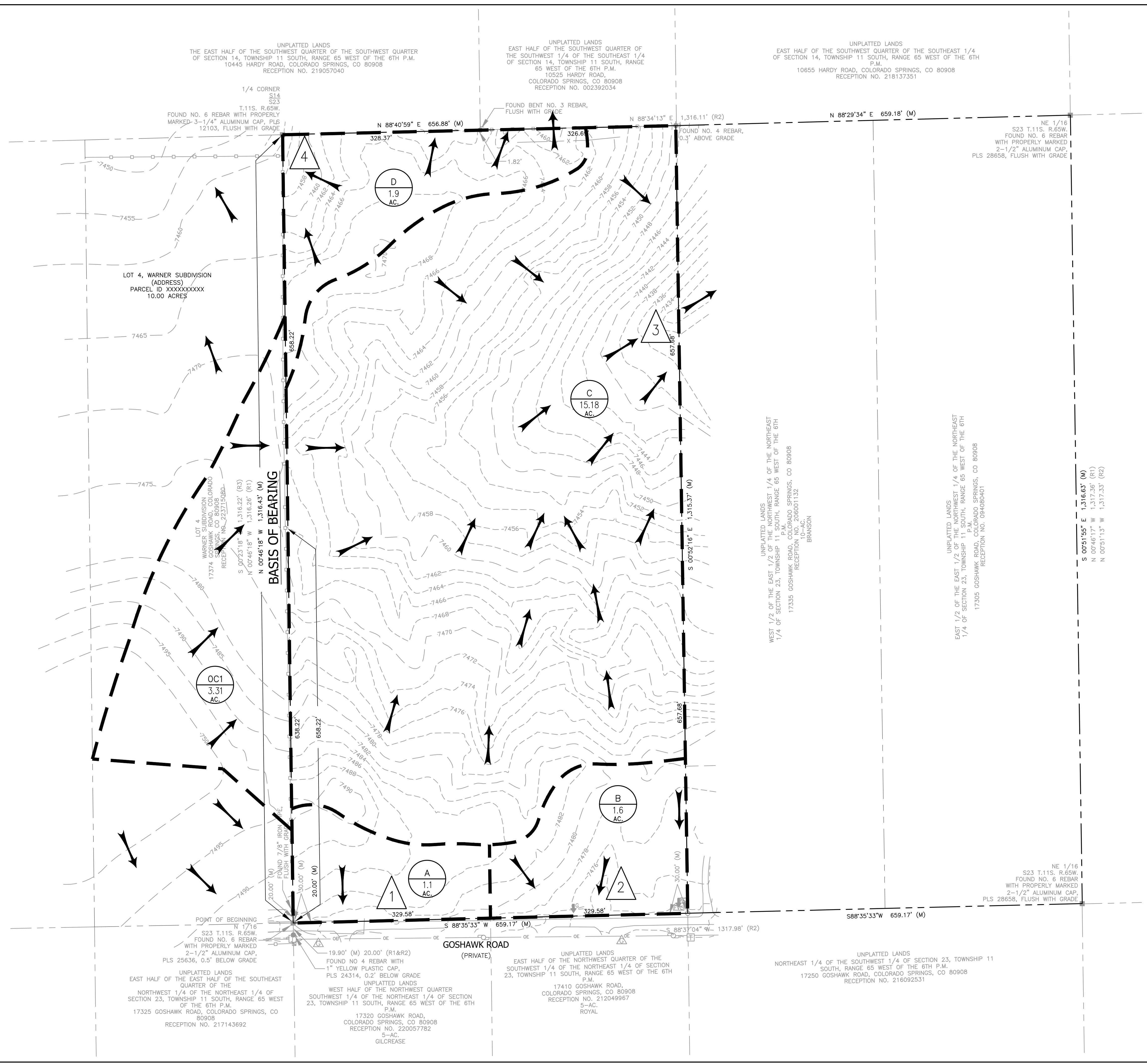
- LIMITS OF DISTURBANCE
- FEMA 100-YEAR FLOODWAY
- FEMA 100-YR FLOODPLAIN
- PROPERTY BOUNDARY
- DRAINAGE BASIN BOUNDARY
- PROPOSED CONTOUR
- EXISTING CONTOUR
- FLOWLINE
- FLOW DIRECTION ARROW
- DESIGN POINT
- BASIN DESIGNATION
- BASIN AREA (ACRES)

**BASIN SUMMARY TABLE**

BASIN	Q5 (CFS)	Q100 (CFS)
A	0.3	2.4
B	0.4	3.2
OC1	1.5	7.1
C	4.1	30.0
D	0.5	3.8

**SUMMARY HYDROLOGY TABLE**

DESIGN POINT	Q5 (CFS)	Q100 (CFS)
1	0.3	2.4
2	0.4	3.2
3	4.9	32.9
4	0.5	3.8



UNPLATTED LANDS  
THE EAST HALF OF THE SOUTHWEST QUARTER OF SECTION 14, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
10445 HARDY ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 219057040

UNPLATTED LANDS  
EAST HALF OF THE SOUTHWEST QUARTER OF THE SOUTHWEST 1/4 OF SECTION 14, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
10525 HARDY ROAD,  
COLORADO SPRINGS, CO 80908  
RECEPTION NO. 002392034

UNPLATTED LANDS  
EAST HALF OF THE SOUTHWEST QUARTER OF THE SOUTHEAST 1/4 OF SECTION 14, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
10655 HARDY ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 218137351

LOT 4, WARNER SUBDIVISION  
(ADDRESS)  
PARCEL ID XXXXXXXXX  
10.00 ACRES

LOT 4, WARNER SUBDIVISION  
17374 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 22371988-  
S 00°23'18" W 1,316.22' (R3)  
N 00°46'18" W 1,316.26' (R1)  
N 00°46'18" W 1,316.43' (M)

POINT OF BEGINNING  
N 171°6'  
S23 T.11S, R.65W.  
FOUND NO. 6 REBAR  
WITH PROPERLY MARKED  
2-1/2" ALUMINUM CAP,  
PLS 25636, 0.5' BELOW GRADE

UNPLATTED LANDS  
EAST HALF OF THE SOUTHWEST QUARTER OF THE NORTHWEST 1/4 OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17325 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 217143692

UNPLATTED LANDS  
WEST HALF OF THE NORTHWEST QUARTER OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17320 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 220057782  
5-AC.  
GILCREASE

UNPLATTED LANDS  
EAST HALF OF THE SOUTHWEST QUARTER OF THE SOUTHWEST 1/4 OF THE NORTHEAST 1/4 OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17410 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 212049967  
5-AC.  
ROYAL

UNPLATTED LANDS  
NORTHEAST 1/4 OF THE SOUTHWEST 1/4 OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17250 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 216092531

UNPLATTED LANDS  
WEST 1/2 OF THE EAST 1/2 OF THE NORTHWEST 1/4 OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17335 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 09001132  
BRANSON

UNPLATTED LANDS  
EAST 1/2 OF THE EAST 1/2 OF THE NORTHWEST 1/4 OF THE NORTHWEST 1/4 OF SECTION 23, TOWNSHIP 11 SOUTH, RANGE 65 WEST OF THE 6TH P.M.  
17305 GOSHAWK ROAD, COLORADO SPRINGS, CO 80908  
RECEPTION NO. 094086401

NE 1/16  
S23 T.11S, R.65W.  
FOUND NO. 6 REBAR  
WITH PROPERLY MARKED  
2-1/2" ALUMINUM CAP,  
PLS 28658, FLUSH WITH GRADE

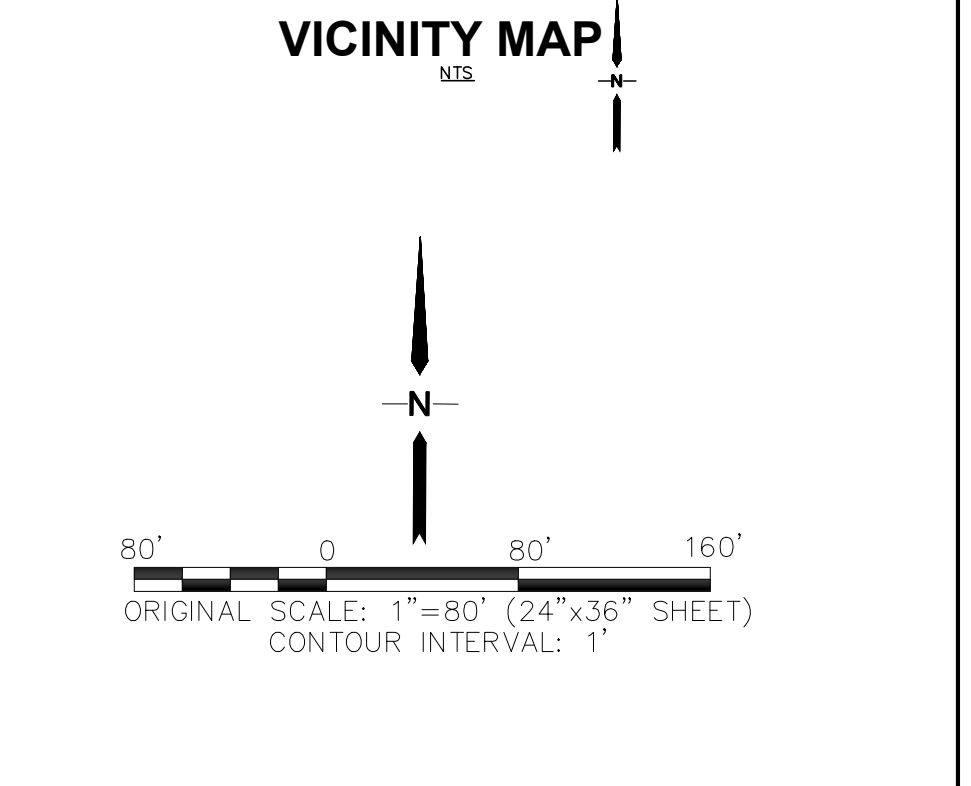
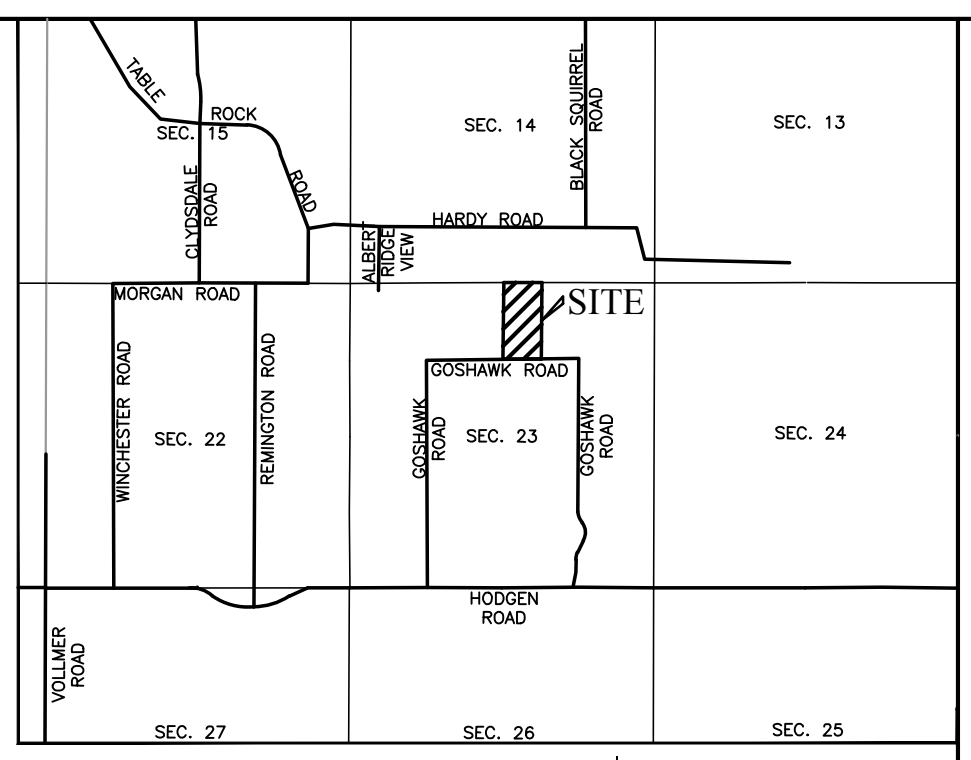
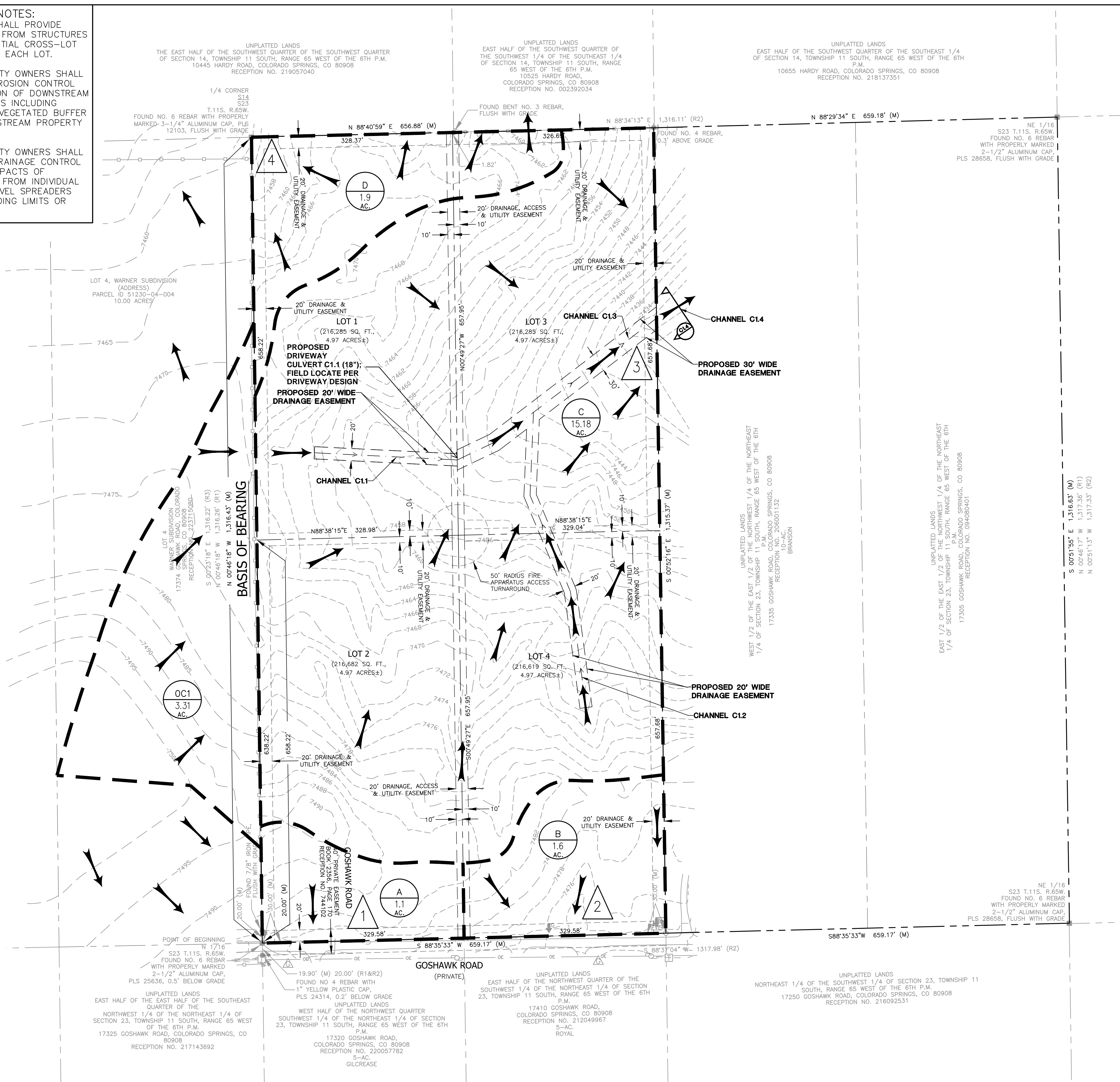
NE 1/16  
S23 T.11S, R.65W.  
FOUND NO. 6 REBAR  
WITH PROPERLY MARKED  
2-1/2" ALUMINUM CAP,  
PLS 28658, FLUSH WITH GRADE

HORIZ. SCALE: 1"=80'  
VERT. SCALE: N/A  
DRAWN: PV  
DESIGNED: JPS  
SURVEYED: APEX  
CHECKED: JPS  
CREATED: 08/29/22  
LAST MODIFIED: 11/14/24  
PROJECT NO: 012401  
MODIFIED BY: PV  
SHEET:



**GENERAL DRAINAGE NOTES:**

- INDIVIDUAL BUILDERS SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM STRUCTURES AND ACCOUNT FOR POTENTIAL CROSS-LOT DRAINAGE IMPACTS WITHIN EACH LOT.
- BUILDERS AND PROPERTY OWNERS SHALL IMPLEMENT & MAINTAIN EROSION CONTROL MEASURES FOR PROTECTION OF DOWNSTREAM PROPERTIES AND FACILITIES INCLUDING PROTECTION OF EXISTING VEGETATED BUFFER STRIPS ALONG THE DOWNSTREAM PROPERTY BOUNDARIES.
- BUILDERS AND PROPERTY OWNERS SHALL IMPLEMENT & MAINTAIN DRAINAGE CONTROL MEASURES TO MINIMIZE IMPACTS OF CONCENTRATED DRAINAGE FROM INDIVIDUAL HOME SITES INCLUDING LEVEL SPREADERS ALONG DOWNSTREAM GRADING LIMITS OR SIMILAR TECHNIQUES.



**LEGEND**

- LDD — LIMITS OF DISTURBANCE
- - - PROPOSED DRAINAGE EASEMENT
- — — PROPERTY BOUNDARY
- — — DRAINAGE BASIN BOUNDARY
- 7470 — PROPOSED CONTOUR
- 7470 — EXISTING CONTOUR
- FLOWLINE
- FLOW DIRECTION ARROW
- △ DESIGN POINT
- BASIN DESIGNATION
- BASIN AREA (ACRES)

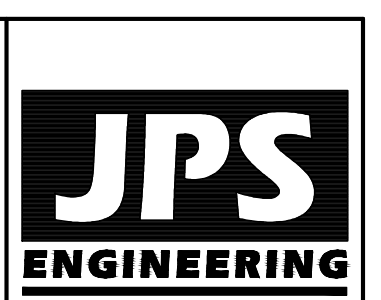
**BASIN SUMMARY TABLE**

BASIN	Q5 (CFS)	Q100 (CFS)
A	0.6	2.7
B	0.8	3.7
OC1	1.5	7.1
C	7.1	34.3
D	0.9	4.3

**SUMMARY HYDROLOGY TABLE**

DESIGN POINT	Q5 (CFS)	Q100 (CFS)
1	0.6	2.7
2	0.8	3.7
3	7.5	36.2
4	0.9	4.3

**BERISFORD SUBDIVISION  
EL PASO COUNTY, CO**



19 E. Wilamette Ave.  
 Colorado Springs, CO 80903  
 PH: 719-477-9429  
 FAX: 719-471-0766  
 www.jpseng.com



CALL UTILITY NOTIFICATION  
 CENTER OF COLORADO  
 1-800-922-1987  
 CALL 2-BUSINESS DAYS IN ADVANCE  
 BEFORE YOU DIG, GRADE, OR EXCAVATE  
 FOR THE MARKING OF UNDERGROUND  
 MEMBER UTILITIES.

No.	REVISION	BY	DATE

**DEVELOPED  
DRAINAGE PLAN**

HORIZ. SCALE: 1"=80'	DRAWN: PV
VERT. SCALE: N/A	DESIGNED: JPS
SURVEYED: APEX	CHECKED: JPS
CREATED: 08/29/22	LAST MODIFIED: 11/14/24
PROJECT NO: 012401	MODIFIED BY: PV
SHEET:	<b>D1</b>