
DRAINAGE REPORT

FOR PROPERTY AT

**2727 Evergreen Road
Colorado Springs
El Paso County, Colorado**

PREPARED FOR

**Harry Studer
Studer Construction
2727 Evergreen Road
Colorado Springs, Colorado**

PREPARED BY JESIK CONSULTING
PROJECT NUMBER: 18-7882



**Jared Perea
Civil Engineer**



**Joseph A. Jesik, P.E.
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Executive Summary

Jesik Consulting (JESIK) has completed a Drainage Report for an approximate 15.23-acre property located in Black Forest, El Paso County, Colorado. The property will be divided into 3 parcels, each approximately 5 acres. The parcels will be used as single-family residences.

There will be minimal impact from the proposed development and no storm water improvements are proposed in this report.

1.0 Subdivision Description

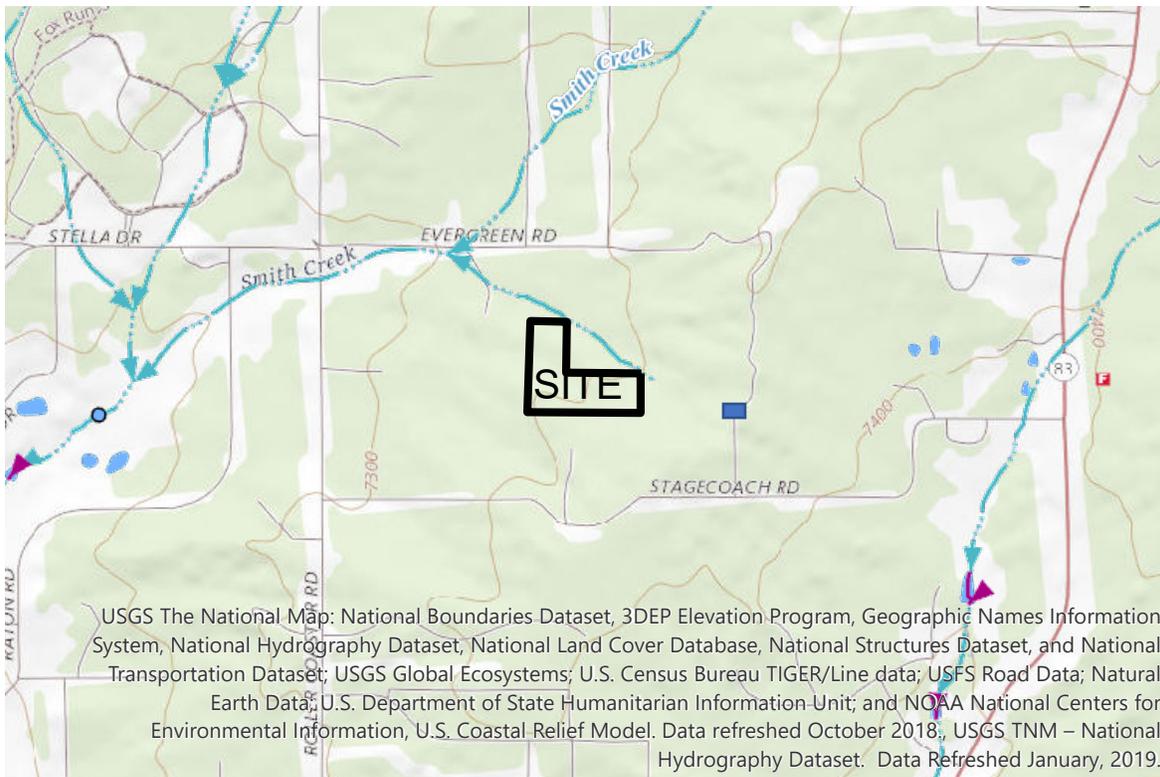
The project is in the Black Forest northeast of Colorado Springs in El Paso County, Colorado. Surrounding developments are Tall Pines Estates, and Stagecoach Spring Estates. The El Paso County schedule number for the property is 6133000043. The parcel is heavily forested with one single-family home. The proposed subdivision will divide the property into three, approximate 5-acre parcels which will be used for single-family homes.

1.1 Location

The project address is 2727 Evergreen Road, Colorado Springs, El Paso County, Colorado. An unnamed tributary to Smith Creek is adjacent to the north east corners of the property. The tributary drains the surrounding area northwesterly into Smith Creek.

Local streets north of the site are Fools Gold Lane, Evergreen Road, and Park Avenue. Roller Coaster Road is about 1,350 feet west of the property. The project location is Section 33, Township 11, Range 66, NE Quarter Section. Refer to Figure 1 below for the vicinity map.

Figure 1-Vicinity Map



1.2 Description

The Studer subdivision project will subdivide El Paso County, schedule no. 6133000043 into approximately 3, 5-acre parcels. There is a current home on the property in the northeast corner. The area of the Studer Subdivision is approximately 15.23-acres. The ground cover is trees, grasses, and shrubs. The ground elevation within the site ranges from approximately 7,428 above sea level on the east side of the site down to approximately 7,388 at the western property boundary. Generally surface water runs northeasterly into a tributary to Smith Creek then northwesterly into Smith Creek. The average grade ranges from 32 percent for side slopes and 13 percent for ridges and valleys.

1.3 Soils

A Natural Resources Conservation Service (NRCS) soils map is attached as Appendix A to this report. The map indicates the soil as Type "B" soil (moderate infiltration rate) for the entire site. Soils on-site are "Type 41: Kettle gravelly loamy sand with 8 to 40 percent slopes".

1.4 Climate

Black Forest averages 21 inches of rain per year, and an average of 40 inches of snow, per year. There is an average of 251 sunny days each year with an average summer high of 81 degrees and the winter low is around 13 degrees.

1.5 Site Impacts

An additional 2 single-family homes may be constructed on the property after subdivision. Increases in runoff will be minimal due to the increase in the relatively small amount of impervious area. Runoff will sheet flow across heavily forested and vegetated ground before leaving the property. There will be minimal off-site impacts.

1.6 Floodplain Designations

The property is not located in a 100-year floodplain. The National Flood Hazard Insurance map for the site is attached to the end of this report.

2.0 Drainage Basins

Generally, surface water flows northeast to a tributary to Smith Creek, then northwest into Smith Creek. Smith Creek drains the area in a south westerly direction and empties into Monument Creek approximately 4.2 miles southwest of the property.

2.1 Major Basin Descriptions

The site drains northeast into a tributary to Smith Creek. Smith Creek flows southwest into Monument Creek. The Smith Creek tributary is developed with single family homes on lots larger than an acre. The tributary will drain into Smith Creek approximately 950 feet from the project site, and Smith Creek will drain into Monument Creek approximately 4.2 miles from the project site.

2.2 Minor Basin Descriptions

Surface water from the site sheet flows northeasterly across about 30% slopes into the Smith Creek tributary. Tributary slopes average 13 percent.

3.0 Hydrologic Criteria

The tributary drainage basins impacting this site are all less than 100 acres, therefore, the Rational Method Hydrology procedures were used to calculate the peak flows. Presentation of existing and proposed hydrologic conditions including approximate flow rates entering and exiting the subdivision with all necessary preliminary calculations. The calculations were based on the following assumptions

• Design storm (minor)	2-year		
• Design storm (minor)	10-year		
• Design storm (major)	100-year		
• Rainfall intensities	El Paso County I-D-F Curve		
• Hydraulic Soil Type	B		
		C2	C10
			C100
• Runoff Coefficients-Undeveloped			
○ Roof, Gravel (packed), 15.23-acres	0.03	0.17	0.36
• Runoff Coefficients-Developed			
○ Roof, Gravel (packed), 15.23-acres	0.12	0.27	0.44

Hydrologic calculations show the largest increase in Basin 1 at 3.18cfs, Basin 2 at 1.48cfs, and Basin 3 0.40cfs when fully developed. Calculations are enclosed in Appendix B.

4.0 Reference

City of Colorado Springs & El Paso County “Drainage Manual,” October 31, 2108

El Paso County “Engineering Criteria Manual.” October 31, 2018

FEMA, Flood Insurance Rate Map (FIRM), January 16, 2018

USDA Natural Resources Conservation Service, “Custom Soil Resource Report for El Paso County Area,” January 16, 2019

5.0 CONCLUSION

The proposed development will have minimal storm water impacts to adjacent properties. The increased flow as seen in the rational method calculations sheet in Appendix B for the 100-year storm for Basin 1 is 3.18 cfs, Basin 2 is 1.48 cfs, and Basin 3 is 0.40 cfs. The increased flows from 2 single-family homes and driveways will not affect the surrounding areas. Water will sheet flow across vegetated and forested ground before leaving each of the new parcels.

APPENDIX A
CURRENT CONDITIONS

Hydrologic Soil Group—El Paso County Area, Colorado
(Studer)



Soil map may not be valid at this scale.

Map Scale: 1:1,560 if printed on A portrait (8.5" x 11") sheet.

0 20 40 80 120 Meters

0 50 100 200 300 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

1/11/2019
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MAP LEGEND

- Area of Interest (AOI)
 - Area of Interest (AOI)
- Soils
 - Soil Rating Polygons
 - A
 - A/D
 - B
 - B/D
 - C
 - C/D
 - D
 - Not rated or not available
 - Soil Rating Lines
 - A
 - A/D
 - B
 - B/D
 - C
 - C/D
 - D
 - Not rated or not available
 - Soil Rating Points
 - A
 - A/D
 - B
 - B/D
- Water Features
 - Streams and Canals
- Transportation
 - Rails
 - Interstate Highways
 - US Routes
 - Major Roads
 - Local Roads
- Background
 - Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 16, Sep 10, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 4, 2010—Oct 16, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
41	Kettle gravelly loamy sand, 8 to 40 percent slopes	B	12.2	100.0%
Totals for Area of Interest			12.2	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

APPENDIX B
HYDROLOGIC CALCULATIONS

Table 6-6 Coefficients for Rational Method

(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients					
		2-year		10-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business							
Commercial Areas	95	0.79	0.80	0.83	0.84	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.53	0.57	0.62	0.68
Residential							
1/2 Acre or less	65	0.41	0.45	0.49	0.54	0.59	0.65
1/4 Acre	40	0.23	0.28	0.36	0.42	0.50	0.58
1/3 Acre	30	0.18	0.22	0.32	0.38	0.47	0.57
1/2 Acre	25	0.15	0.20	0.30	0.36	0.46	0.56
1 Acre	20	0.12	0.17	0.27	0.34	0.44	0.55
Industrial							
Light Areas	80	0.57	0.60	0.63	0.66	0.70	0.74
Heavy Areas	90	0.71	0.73	0.75	0.77	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.20	0.29	0.39	0.52
Playgrounds	13	0.07	0.13	0.24	0.31	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.36	0.42	0.50	0.58
Undeveloped Areas							
Historic Flow Analysis-Greenbelts, Agriculture	2	0.03	0.05	0.17	0.26	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.15	0.25	0.35	0.50
Forest	0	0.02	0.04	0.15	0.25	0.35	0.50
Exposed Rock	100	0.89	0.89	0.92	0.92	0.96	0.96
Offsite Flow Analysis (when land use is undefined)	45	0.26	0.31	0.38	0.44	0.51	0.59
Streets							

Paved	100	0.89	0.89	0.92	0.92	0.96	0.96
Gravel	80	0.57	0.60	0.63	0.66	0.70	0.74
Drive and Walks	100	0.89	0.89	0.92	0.92	0.96	0.96
Roofs	90	0.71	0.73	0.75	0.77	0.81	0.83
Lawns	0	0.02	0.04	0.15	0.25	0.35	0.50

City of Colorado Springs

Drainage Criteria Manual, Volume 1

3.2. - Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_t) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_t) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

$$t_c = t_i + t_t \tag{Eq. 6-7}$$

Where:

t_c = time of concentration (min)

t_i = overland (initial) flow time (min)

t_t = travel time in the ditch, channel, gutter, storm sewer, etc. (min) ;0h5; 3.2.1.\Overland (Initial)

Flow Time

The overland flow time, t_i , may be calculated using Equation 6-8.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \tag{Eq. 6-8}$$

Where:

t_i = overland (initial) flow time (min)

C_5 = runoff coefficient for 5-year frequency (see Table 6-6)

L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)

S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize. ;0h5; 3.2.2.\Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t , which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t , can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad (\text{Eq. 6-9})$$

Where:

V = velocity (ft/s)

C_v = conveyance coefficient (from Table 6-7)

S_w = watercourse slope (ft/ft)

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried) *	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15

Paved areas and shallow paved swales	20
*For buried riprap, select Cv value on type or vegetative cover.	

The travel time is calculated by dividing the flow distance (in feet) by the velocity calculated using Equation 6-9 and converting units to minutes.

The time of concentration (t_c) is then the sum of the overland flow time (t_o) and the travel time (t_t) per Equation 6-7. ;0h5; 3.2.3.\First Design Point Time of Concentration in Urban Catchments

Using this procedure, the time of concentration at the first design point (typically the first inlet in the system) in an urbanized catchment should not exceed the time of concentration calculated using Equation 6-10. The first design point is defined as the point where runoff first enters the storm sewer system.

$$t_c = L / 180 + 10 \quad (\text{Eq. 6-10})$$

Where:

t_c = maximum time of concentration at the first design point in an urban watershed (min)

L = waterway length (ft)

Equation 6-10 was developed using the rainfall-runoff data collected in the Denver region and represents regional "calibration" of the Rational Method. Normally, Equation 6-10 will result in a lesser time of concentration at the first design point and will govern in an urbanized watershed. For subsequent design points, the time of concentration is calculated by accumulating the travel times in downstream drainage reaches.

Peak Flows

	5-year (cfs)	10-year (cfs)	100-year (cfs)
Basin 1	1.85	4.26	15.66
Basin 1-Developed	4.11	6.76	19.14
Change in flow	2.26	2.50	3.48
Basin 2	1.86	4.28	15.75
Basin 2-Developed	4.13	6.80	19.25
Change in flow	2.27	2.52	3.50
Basin 3	1.20	2.78	10.21

Basin 3-Developed	2.68	4.41	12.48
Change in flow	1.48	1.63	2.27

National Flood Hazard Layer FIRMette



39°37.44'N
104°48'58.93'W



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS

- Without Base Flood Elevation (BFE) Zone A, V, AE9
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD

- 0.2% Annual Chance Flood Hazard. Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Level. See Notes, Zone X
- Area with Flood Risk due to Levee Zone D

OTHER AREAS GENERAL STRUCTURES

- Area of Minimal Flood Hazard Zone X Effective LOMs
- Area of Undetermined Flood Hazard Zone D
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES

- Cross sections with 1% Annual Chance
- Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS

- Digital Data Available
- No Digital Data Available
- Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **1/16/2019 at 10:33:38 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Rational Method Calculations

Calculation of Peak Runoff																	
Designer: Jared J. Perera Company: Jessa Consulting Date: 2/22/2019 Project: 18.7882 Location: 2727 Evergreen Road		Version 2.00 released May 2017 Cells of this color are for required user-input Cells of this color are for optional override values Cells of this color are for calculated results based on overrides			$t_1 = \frac{0.395(L_1 - C_1)\sqrt{L_1}}{S^{0.33}}$ $t_2 = \frac{L_1}{60K\sqrt{S_2}} = \frac{L_1}{50V_t}$				Computed $t_c = t_1 + t_2$ Regional $t_c = (26 - 17I) + \frac{L_1}{60(14I + 9)\sqrt{S_2}}$		$t_{\text{minimum}} = 5$ $t_{\text{maximum}} = 3t$ Selected $t_c =$						
Subcatchment Name	Area (ac)	NRC S Hydrologic Soil Group	Percent Imperviousness	Runoff Coefficient, C							Overland (initial) Flow Time				Channel		
				2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr	Overland Flow Length L ₁ (ft)	U/S Elevation (ft) (Optional)	D/S Elevation (ft) (Optional)	Overland Flow Slope S ₁ (ft/ft)	Overland Flow Time t ₁ (min)	Channelized Flow Length L ₁ (ft)	U/S Elevation (ft) (Optional)
Basin 1	5.30	B	2.0	0.01	0.01	0.07	0.26	0.34	0.44	0.54	500.00			7.200	5.01	0.00	
				0.03	0.09	0.17	0.26	0.31	0.36						4.65		
Basin 1-Dev	5.30	B	12.0	0.07	0.09	0.15	0.32	0.39	0.48	0.58	500.00			7.200	4.67	0.00	
				0.12	0.20	0.27	0.35	0.40	0.44						4.14		
Basin 2	5.90	B	2.0	0.01	0.01	0.07	0.26	0.34	0.44	0.54	400.00			1.600	7.36	0.00	
				0.03	0.09	0.17	0.26	0.31	0.36						6.83		
Basin 2-Dev	5.90	B	12.0	0.07	0.09	0.15	0.32	0.39	0.48	0.58	400.00			1.600	6.86	0.00	
				0.12	0.20	0.27	0.35	0.40	0.44						6.09		
Basin 3	4.00	B	2.0	0.01	0.01	0.07	0.26	0.34	0.44	0.54	391.00			1.000	8.50	0.00	
				0.03	0.09	0.17	0.26	0.31	0.36						7.89		
Basin 3-Dev	4.00	B	12.0	0.07	0.09	0.15	0.32	0.39	0.48	0.58	391.00			1.000	7.93	0.00	
				0.12	0.20	0.27	0.35	0.40	0.44						7.03		

Using Rational Method

Select UDFCD location for NOAA Atlas 14 Rainfall Depths from the pull-down list OR enter your own depths obtained from the NOAA website (click this link)

1-hour rainfall depth, P1 (in) =		2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr																
		0.63	1.09	1.33	1.69	1.99	2.31	3.14																
max(t _{minimum} , min(Computed t _c , Regional t _c))		a	b	c	$I(in/hr) = \frac{a + P_1}{(b + t_c)^2}$																			
Rainfall Intensity Equation Coefficients =		28.50	10.00	0.786	Q(cfs) = CIA																			
Ized (Travel) Flow Time				Time of Concentration							Rainfall Intensity, I (in/hr)							Peak Flow, Q (cfs)						
Channelized Flow Slope S ₁ (ft/ft)	NRC S Conveyance Factor K	Channelized Flow Velocity V ₁ (ft/sec)	Channelized Flow Time t ₁ (min)	Computed t _c (min)	Regional t _c (min)	Selected t _c (min)	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr				
7.200	10	26.83	0.00	5.01	25.66	10.00	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.10	0.19	1.39	6.34	9.88	14.42	24.46				
				4.65		4.14	2.93	3.87	4.73	6.00	7.07	8.21	11.16	0.47	1.85	4.26	8.27	11.62	15.66	32.13				
7.200	10	26.83	0.00	4.67	23.96	4.14	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.63	1.33	2.93	7.96	11.27	15.98	26.11				
				7.36		4.14	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.11	0.21	1.55	7.06	10.77	16.05	27.23				
1.600	10	12.65	0.00	6.83	25.66	6.09	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.47	1.86	4.28	8.32	11.68	15.75	32.31				
				6.86		6.09	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.92	1.48	3.27	8.75	12.55	17.77	29.07				
1.600	10	12.65	0.00	6.09	23.96	6.09	2.23	2.95	3.60	4.57	5.38	6.25	8.50	1.87	4.13	6.80	11.20	15.07	19.25	34.49				
				8.50		6.09	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.08	0.14	1.05	4.78	7.30	10.88	18.46				
1.000	10	10.00	0.00	7.89	25.66	7.03	2.53	3.35	4.08	5.19	6.11	7.09	9.64	0.30	1.20	2.78	5.40	7.58	10.21	20.95				
				7.93		7.03	2.23	2.95	3.60	4.57	5.38	6.25	8.50	0.63	1.01	2.21	5.93	8.51	12.04	19.71				
1.000	10	10.00	0.00	7.03	23.96	7.03	2.53	3.35	4.08	5.19	6.11	7.09	9.64	1.22	2.68	4.41	7.26	9.77	12.48	22.36				