

1845.00 Vollmer Substation  
Utility Yard Interior Cutoff Swale Drain Pipe Outfall  
Riprap Sizing (pipe that drains into sand filter from yard)  
Max Pipe Q=0.5 cfs, 6" PVC pipe

$$H_a = \frac{(H + Y_n)}{2}$$

Equation 9-19

Where the maximum value of  $H_a$  shall not exceed  $H$ , and:

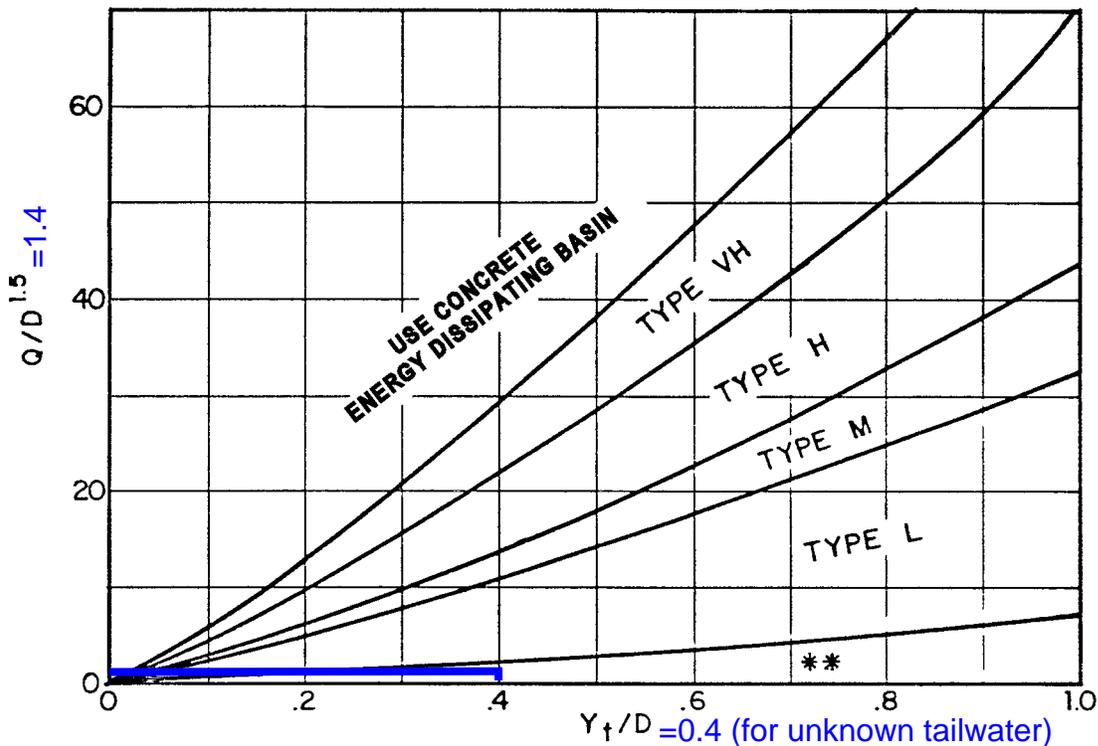
$D_a$  = parameter to use in place of  $D$  in Figure 9-38 when flow is supercritical (ft)

$D_c$  = diameter of circular culvert (ft)

$H_a$  = parameter to use in place of  $H$  in Figure 9-39 when flow is supercritical (ft)

$H$  = height of rectangular culvert (ft)

$Y_n$  = normal depth of supercritical flow in the culvert (ft)



Use  $D_a$  instead of  $D$  whenever flow is supercritical in the barrel.  
\*\* Use Type L for a distance of  $3D$  downstream.

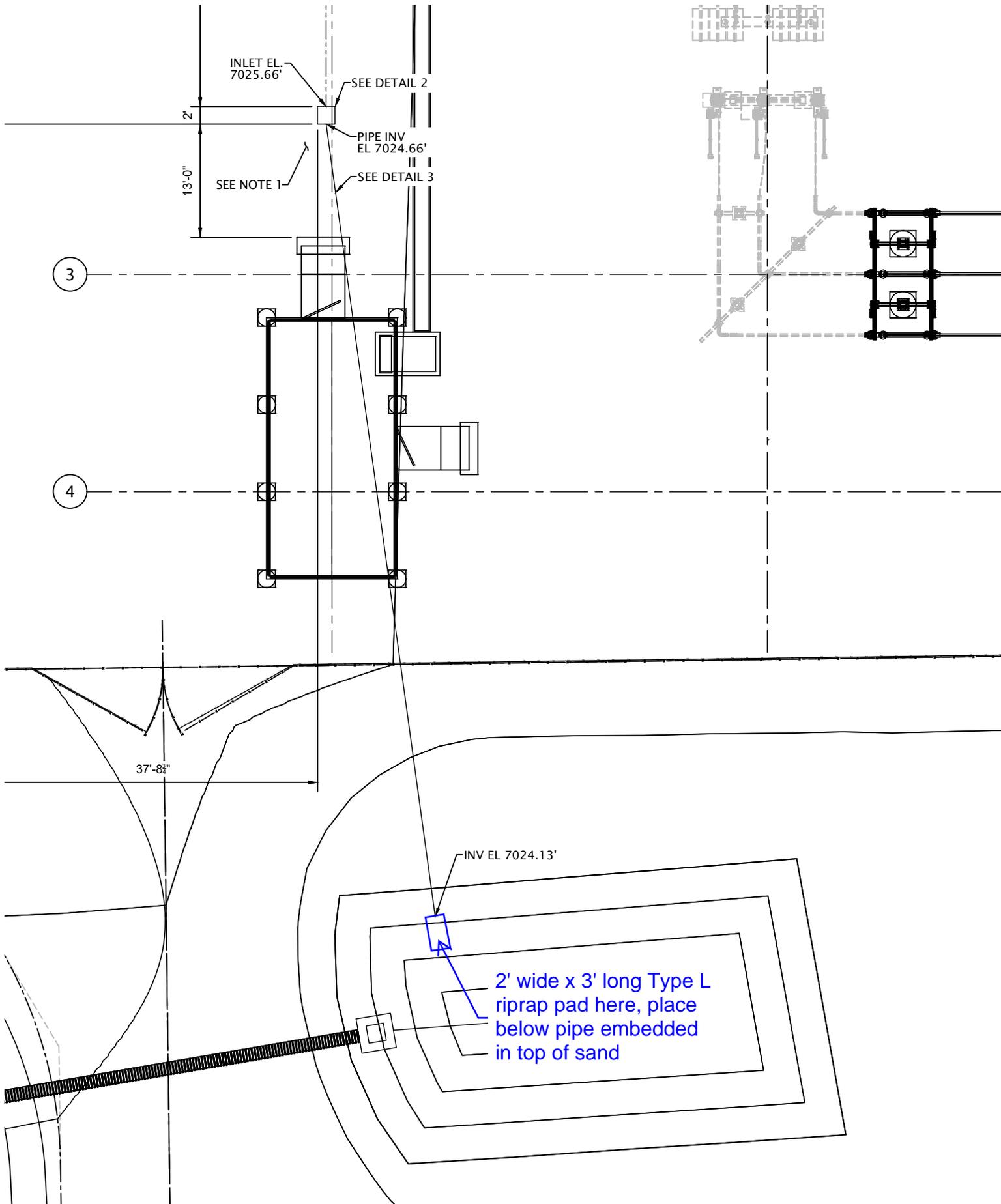
Result: Type L, D50=9" riprap

Note: Pipe outfall invert is 0.13' above top of sand.

Use: Single layer of Type L riprap below pipe, 2' wide x 3' long.

Type VL = 6"  
Type L = 9"  
Type M = 12"  
Type H = 18"  
Type VH = 24"

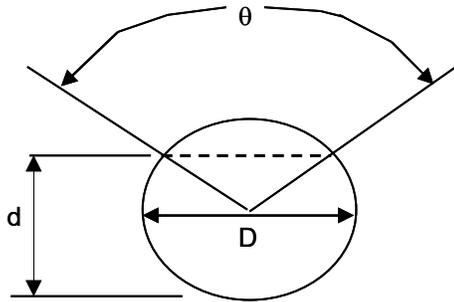
Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for  $Q/D2.5 \leq 6.0$ )



**MANNING'S EQUATION FOR PIPE FLOW**

Project: Vollmer Substation Location: Utility Yard Interior Cutoff Swale Drain Pipe Cap  
 By: Dane Frank Date: 5/2/2023  
 Chk. By: Date: mdo version 12.8.00

Clear Data  
Entry Cells



Mannings Formula

$$Q = (1.486/n) A R_h^{2/3} S^{1/2}$$

R=A/P  
 A=cross sectional area  
 P=wetted perimeter  
 S=slope of channel  
 n=Manning's roughness coefficient

INPUT

D= 6 inches  
 d= 5.5 inches  
 n= 0.012 mannings coeff  
 theta= 67.1 degrees  
 S= 0.0058 slope in/in

$$V = (1.49/n) R_h^{2/3} S^{1/2}$$

$$Q = V \times A$$

Solution to Mannings Equation					Manning's n-values	
Area, ft <sup>2</sup>	Wetted Perimeter, ft	Hydraulic Radius, ft	velocity ft/s	flow, cfs		
0.19	1.28	0.15	2.63	0.50	PVC	0.01
					PE (<9"dia)	0.015
					PE (>12"dia)	0.02
					PE(9-12"dia)	0.017
					CMP	0.025
					ADS N12	0.012
					HCMP	0.023
					Conc	0.013

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