

August 23, 2017

El Paso County 2880 International Circle, Suite 110 Colorado Springs, CO 80910

RE: Lot 2, Academy Gateway Subdivision, Filing No. 1 Northgate & Struthers, El Paso County, CO Drainage Letter

Vertical Construction Management (VCM) is proposing a new restaurant within the Academy Gateway Subdivision, Filing No.1. The scope of the project will be to construct a new 2,200 sq. ft. restaurant with drive-thru pickup window and associated parking, drive, and landscape areas on a pad-ready site.

This project will be on Lot 2, Academy Gateway Subdivision Filing No. 1, which is 0.985 acres. The site is bound by Gleneagle Gate View (50.0 foot private R.O.W.), on the north, shared private drives on the west and south, and Struthers Road (120' R.O.W.) on the east. Lot 2 will be a vacant, pad-ready site that will be constructed by the overall developer with utilities stubbed to the site. The site will generally slope from north to south at approximately 1.0% to 5.0%.

The project area lies within Basins B and D, as defined by the *Preliminary / Final Drainage Report for Academy Gateway Subdivision Filing No. 1*, prepared by Classic Consulting Engineers & Surveyors, March 2017 (El Paso County Job No. SF-16-018). Basin D is 2.16 acres and is assumed to be 100% impervious per the Classic Consulting report. A copy of the drainage map from the *Preliminary / Final Drainage Report for Academy Gateway Subdivision Filing No. 1* has been provided in Appendix B.

It should be noted that the original Classic Consulting report did not account for the storm sewer stub to serve Lot 2. Classic Consultant has submitted a subsequent addendum to the approved Final Drainage Report, which now includes calculations and design for the storm sewer stub for Lot 2. The *Drainage Letter Addendum for Academy Gateway Subdivision Filing No.* 1, prepared by Classic Consulting, dated August 11, 2017 has been provided in Appendix C. The Drainage Letter addendum further divides the site into Basins B, D1, D2, and D4.

It should also be noted that the Classic Consulting report states that the detention facility in Tract C is temporary, and will be removed once the El Paso County Regional Facility is constructed southwest of the site. Although not explicitly stated in the drainage report for the overall development, it is our understanding that detention and water quality for Lot 2 will be provided by detention facility in Tract C, or by the El Paso County Regional Facility once it is constructed. Per discussion with the County, no water quality features are proposed or will be required on Lot 2, since water quality will be handled by detention facility in Tract C, or the regional facility.

The proposed development on Lot 2 will consist of roughly five onsite basins as depicted by the drainage plan in Appendix D:

- Basin A-1 (0.33 acres, 69% impervious) consists of parking, drive, patio and landscape areas and will drain to a proposed inlet in the southeast corner of the site where runoff will be conveyed via storm sewer to the detention facility in Tract C (design point 1).
- Basin A-2 (0.19 acres, 73% impervious) consists of parking, drive, and landscape areas and will drain to a proposed inlet in the southwest corner of the site where runoff will be conveyed via storm sewer to the detention facility in Tract C (design point 2).

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- Basin R-1 (0.05 acres, 90% impervious) consists of roof area for the proposed restaurant and will drain to
 proposed storm sewer along the south edge of Lot 2 and ultimately to the detention facility in Tract C
 (design point 2).
- Basin O-1 (0.32 acres, 44% impervious) consists of the north portion of the shared access drive south of the site, a portion of the south access point, and landscape areas in the south and east portions of the site. Due to grading constraints and the design of the internal access roads and storm sewer systems, runoff from Basin O-1 cannot be conveyed to the detention facility in Tract C. Runoff from Basin O-1 will drain to the private access drive south of the site where it will continue to the east to Struthers Road. (design point 3).
- Basin O-2 (0.10 acres, 47% impervious) consists of the east portion of the shared access drive west of the site, and landscape areas in the west portion of the site. Due to grading and layout constraints, this basin cannot be conveyed to the storm sewer system in Lot 2 and will ultimately outfall to Gleneagle Gate View. Runoff from Basin O-2 will be conveyed via curb and gutter and storm sewer in Gleneagle Gate View to the detention facility within Tract C (design point 4).

Proposed development on Lot 2 complies with the drainage concept outlined in the Classic Consulting report and subsequent addendum. Proposed development on Lot 2 will total approximately 60% impervious, which is significantly less than the 100% impervious assumed in the Classic Consulting report. As such, the proposed development is in conformance with El Paso County drainage criteria.

Sincerely,

Joe Schiel, PE For and on behalf of Sterling Design Associates, LLC

Attachments (3):

- Appendix A Tributary Basin and Percent Impervious Calculations
- Appendix B Selected Drainage Map from Academy Gateway Subdivision Filing No. 1 Drainage Report
- Appendix C Drainage Letter Addendum for Academy Gateway Subdivision Filing No. 1
- Appendix D Drainage Plan

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Drainage Letter Signature Blocks

Design Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissionaron my part in preparing this report.

Naple, P.E. # Joseph schiel, #48332

Owner/Developer's Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan. 10/16/17 Jason Keen, Manager Name. Title Date **Business Name** VRE Northgate, LLC. 1211 South White Chapel Blvd. Address Southlake, TX 76092

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Criteria Manual, as amended.

Jennifer Irvine, P.E.,

County Engineer / ECM Administrator

Date

jay M. Newell, PE

0/3/2017

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Appendix A

Tributary Basin and Percent Impervious Calculations

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Weighted Imperviousness

Job Name: Starbucks, Northgate & Struthers, El Paso County, CO

| | ¹ I (%) | ¹ C ₅ | ¹ C ₁₀ | ¹ C ₁₀₀ |
|----------------|--------------------|-----------------------------|------------------------------|-------------------------------|
| Roof | 90 | 0.73 | 0.75 | 0.81 |
| Lawn | 0 | 0.08 | 0.15 | 0.35 |
| Drives & Walks | 100 | 0.90 | 0.92 | 0.96 |
| | | | | |

¹City of Colorado Springs Drainage Criteria Manual, Volume 1, May 2014

PROPOSED SITE

| Basin | Roof | Lawn | Drives & Walks | Total | Total | l (%) | C ₅ | C ₁₀ | C ₁₀₀ |
|------------|-------|--------|----------------|--------|-------|-------|----------------|-----------------|------------------|
| | (sf) | (sf) | (sf) | (sf) | (ac) | | | | |
| A-1 | 0 | 4489 | 9791 | 14280 | 0.33 | 69 | 0.64 | 0.68 | 0.77 |
| A-2 | 0 | 2214 | 5911 | 8125 | 0.19 | 73 | 0.68 | 0.71 | 0.79 |
| R-1 | 2257 | 0 | 0 | 2257 | 0.05 | 90 | 0.73 | 0.75 | 0.81 |
| O-1 | 0 | 7824 | 6045 | 13869 | 0.32 | 44 | 0.44 | 0.49 | 0.62 |
| 0-2 | 0 | 2327 | 2049 | 4376 | 0.10 | 47 | 0.46 | 0.51 | 0.64 |
| Total Site | 2,257 | 16,854 | 23,796 | 42,907 | 0.99 | 60 | 0.57 | 0.61 | 0.71 |

²Assumed square footages from aerial documentation ³Assumed square footage of future development

Stormwater Runoff - 5 Year

Job Name: Starbucks, Northgate & Struthers, El Paso County, CO

Date: 5/16/17 Calculated by: JGS

Design Storm:

Project:

5-yr

| | | | Direct Rur | noff | | | | | Total Run | off | | Street | | | Pipe | | Tra | avel Time | | |
|--------|----------|------|---------------------|-----------------|------|----------------|-------|-------|-----------|--------------|-------|--------|--------|--------|-------|------|--------|-----------|-------|---------|
| Design | Basin ID | Area | ¹ Runoff | ³ TC | | ² I | Q | Tc | Total | ² | Q | Slope | Street | Design | Slope | Pipe | Length | Vel | Tt | |
| Point | | (Ac) | Coeff | (min) | CA | (in/hr) | (cfs) | (min) | CA | (in/hr) | (cfs) | % | Flow | Flow | % | Size | (Ft) | (fps) | (min) | Remarks |
| | | | | | | | | | | | | | | | | | | | | |
| | A-1 | 0.33 | 0.64 | 5.00 | 0.21 | 5.17 | 1.09 | | | | | | | | | | | | | |
| | A-2 | 0.19 | 0.68 | 5.00 | 0.13 | 5.17 | 0.65 | | | | | | | | | | | | | |
| | R-1 | 0.05 | 0.73 | 5.00 | 0.04 | 5.17 | 0.20 | | | | | | | | | | | | | |
| | O-1 | 0.32 | 0.44 | 5.00 | 0.14 | 5.17 | 0.72 | | | | | | | | | | | | | |
| 4 | 0-2 | 0.10 | 0.46 | 5.00 | 0.05 | 5.17 | 0.24 | | | | | | | | | | | | | |
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¹City of Colorado Springs Drainage Criteria Manual, Volume 1, May 2014
 ²Intensity from City of Colorado Springs Drainage Criteria Manual, Volume 1, May 2014
 ³5-Minute Time of Concentration Conservatively Assumed

Stormwater Runoff - 100 Year

Job Name: Starbucks, Northgate & Struthers, El Paso County, CO

Date: 5/16/17 Dunty, CO Calculated by: JGS

100-yr

Project:

Design Storm:

| | | | Direct Rur | noff | | | | | Total Run | off | | Street | | | Pipe | | Tra | avel Time | | |
|--------|----------|------|---------------------|-----------------|------|----------------|-------|-------|-----------|----------------|-------|--------|--------|--------|-------|------|--------|-----------|-------|---------|
| Design | Basin ID | Area | ¹ Runoff | ³ TC | | ² I | Q | Тс | Total | ² I | Q | Slope | Street | Design | Slope | Pipe | Length | Vel | Tt | |
| Point | | (Ac) | Coeff | (min) | CA | (in/hr) | (cfs) | (min) | CA | (in/hr) | (cfs) | % | Flow | Flow | % | Size | (Ft) | (fps) | (min) | Remarks |
| | | | | | | | | | | | | | | | | | | | | |
| 1 | A-1 | 0.33 | 0.77 | 5.00 | 0.25 | 8.68 | 2.19 | | | | | | | | | | | | | |
| 2 | A-2 | 0.19 | 0.79 | 5.00 | 0.15 | 8.68 | 1.29 | | | | | | | | | | | | | |
| 1 | R-1 | 0.05 | 0.81 | 5.00 | 0.04 | 8.68 | 0.36 | | | | | | | | | | | | | |
| 3 | O-1 | 0.32 | 0.62 | 5.00 | 0.20 | 8.68 | 1.70 | | | | | | | | | | | | | |
| 4 | 0-2 | 0.10 | 0.64 | 6.00 | 0.06 | 9.68 | 0.62 | | | | | | | | | | | | | |
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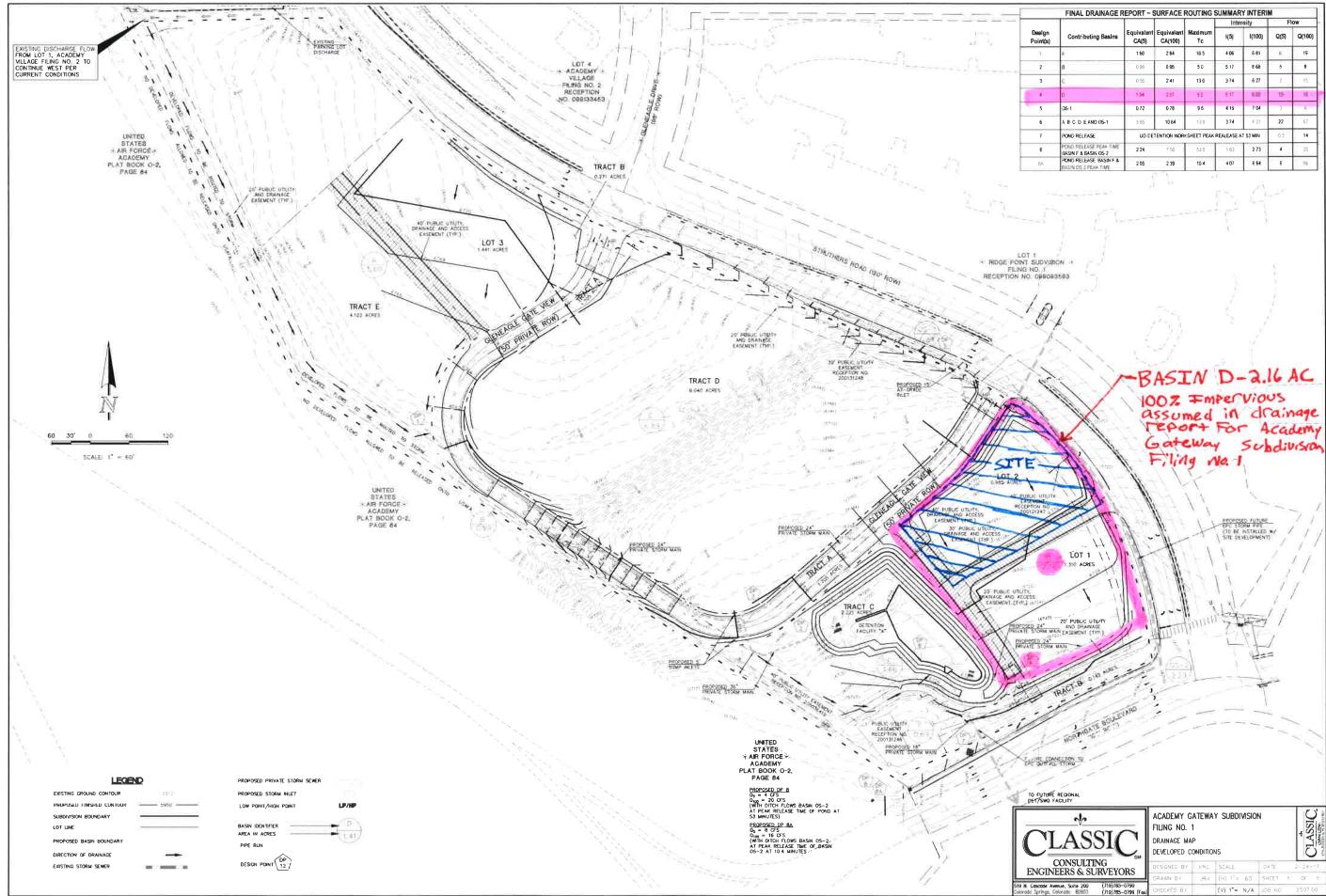
¹City of Colorado Springs Drainage Criteria Manual, Volume 1, May 2014
 ²Intensity from City of Colorado Springs Drainage Criteria Manual, Volume 1, May 2014
 ³5-Minute Time of Concentration Conservatively Assumed

Appendix B

Selected Drainage Map from Academy Gateway Subdivision Filing No. 1 Drainage Report

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| | | | | | inter | nsity | F | ow |
|------------------|--|---------------------|-----------------------|---------------|-----------|---------------|------|--------|
| esign sint(s) | Contributing Basins | Equivalent CA(5) | Equivalent CA(100) | Maximum Tc | 1(5) | l(100) | Q(5) | Q(100) |
| 18 | £ | 1 60 | 2.84 | 10.5 | 4 06 | 6 81 | e | 19 |
| 2 | β | 6 B9 | 0.95 | 50 | 5 17 | E 68 | 5 | 8 |
| 3 | C, | 0.55 | 2.41 | 13.0 | 374 | 6 27 | 2 | 15 |
| 4 | D | 1.94 | 267 | 50 | 517 | 1.00 - | 10- | 18 1 |
| 5 | 06-1 | 0 72 | 0 78 | 96 | 4 19 | 7 04 | 3 | 6 |
| 6 | A B C D E AND OS-1 | 5 85 | 10.64 | 13.0 | 374 | 6.27 | 22 | 67 |
| 7 | POND RELEASE | UD-DE | TENTION WOR | SHEET PEAK R | EALEASE A | T 53 MIN | 03 | 14 |
| 8 | PONO RELEASE PEAK TIME BASIN F & BASIN 05-2 | 2 24 | 7 50 | 53 0 | 1 63 | 2 73 | 4 | 20 |
| 8A | POND RELEASE BASIN F & BASIN DS 2 PEAK TIME | 2 05 | 2 39 | 10.4 | 4 07 | 6 64 | 8 | 201 |

Appendix C

Drainage Letter Addendum for Academy Gateway Subdivision Filing No. 1

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DRAINAGE LETTER ADDENDUM FOR ACADEMY GATEWAY SUBDIVISION NO. 1

DRAINAGE LETTER STATEMENT

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Kyle R Campbell, Colorado P.E. #29794

Date

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

| Business Name: | <u>Academy Gateway II, LLC</u> . | _ |
|----------------|----------------------------------|---|
| By: | | |
| Title: | | _ |
| Address: | 403 S. Tejon Street | |
| | Colorado Springs, CO 80903 | |

EL PASO COUNTY ONLY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Jennifer Irvine, P.E. County Engineer / ECM Administrator Date

Conditions:



August 11, 2017

City of Colorado Springs Engineering Development Review Division 30 South Nevada, Suite 401 Colorado Springs, CO 80903

ATTN: Mrs. Jennifer Irvine

RE: Drainage Letter Addendum for Academy Gateway Subdivision Filing No. 1

Dear Jennifer:

Please consider this the Drainage Letter Addendum for Academy Gateway Subdivision Filing No. 1 This letter is being written to further define the storm system required for Lot 1 and Lot 2.

This letter will update storm sewer routing inlet sizes and alignment for Academy Gateway Subdivision Filing No. 1 Lot 1 and Lot 2. More specifically Basin D. Please see the "Preliminary/Final Drainage Report for Academy Gateway Subdivision Filing No. 1" by CCES approved July 6, 2017 report for drainage information. All basins and drainage patterns from the original report remain the same as previously approved with the exception of Design Points 2 & 4 and Basins B & D on attached drainage map excerpt.

Design Point 2 ($Q_5 = 5$ cfs, $Q_{100} = 8$ cfs) is the proposed developed flow from Basin B which is predominately the paved surface of Academy Gate View. A proposed private 5' Type R sump inlet will intercept these flows in their entirety. Basin B area has increased from 0.99 acres to 1.08 acres with the addition of a high point in the access roadway adjacent to Lot 1 and Lot 2. No impact to proposed inlet sizing or developed flows.

Design Point 4 and Basin D from the approved report is further broken down based upon future use of Lot 1 and Lot 2. Basin D is broken down into Basins D, D1, D2 D3 and D4 with associated Design Points 4 and 4A for inlet and pipe sizing. Design Point 4 ($Q_5 = 0.1$ cfs, $Q_{100}= 3$ cfs) consists of flows from Basin D4 which is private drive aisle. A proposed 5' Type-R inlet will intercept flows. Design Point 4A ($Q_5 = 0.1$ cfs, $Q_{100}= 3$ cfs) consists of flows from Basin D2 which is private drive aisle between Lots 1 and 2. A proposed 5' Type-R-at grade inlet will intercept flows. Basin D ($Q_5 = 4$ cfs, $Q_{100}= 6$ cfs) consists of fluture commercial use on Lot 1. Flows will be captured onsite and routed to a provided 18'' storm sewer stub (Pipe 5). Basin D3 ($Q_5 = 2$ cfs, $Q_{100}= 3$ cfs) consists of fluture commercial use on Lot 1. Flows will be captured on site and routed to a provided 18.'' storm sewer stub (Pipe 5). Basin

routed to a provided 12" storm stub. Basin D1 ($Q_5 = 4 \text{ cfs}$, $Q_{100} = 7 \text{ cfs}$) consists of future commercial use on Lot 2. Flows will be captured onsite and routed to a provided 18" storm sewer stub (Pipe 3). All flows will be routed to the proposed Detention Facility A as previously designed. Per original report Basin Design Point 4 was ($Q_5 = 10 \text{ cfs}$, $Q_{100} = 18 \text{ cfs}$) and was routed to the detention facility in a proposed 24" Storm system. This is comparable to Pipe 6 ($Q_5 = 10 \text{ cfs}$, $Q_{100} = 18 \text{ cfs}$) in this addendum. No increase in developed flows is anticipated.

SUMMARY

The original drainage report for Academy Gateway Subdivision Filing No. 1 calculations and report details remain the same. This letter further defines the storm sewer system for Lot 1 and Lot 2 from the previous report.

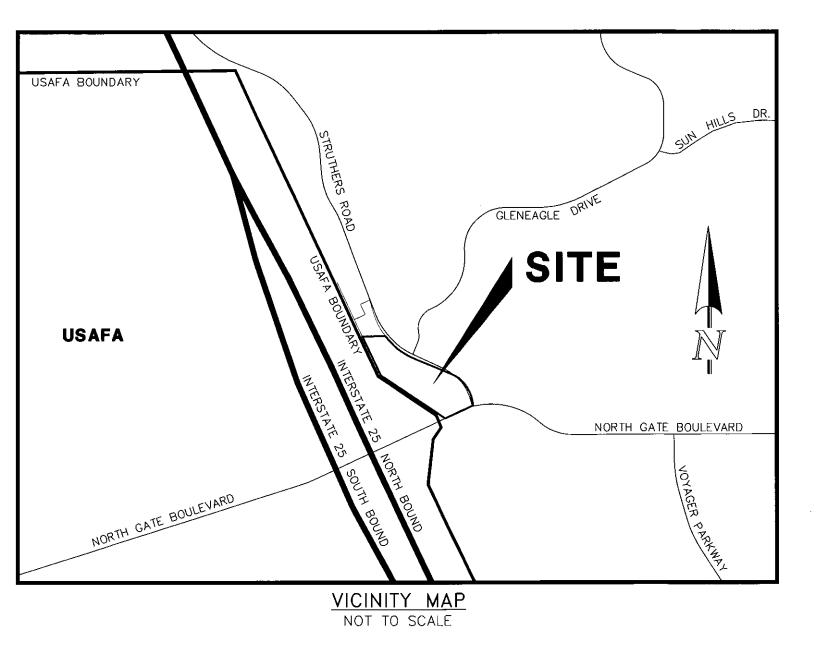
If you have any questions or comments regarding this drainage, please do not hesitate to call. Respectfully submitted,

David L. Gibson, P.E. Project Engineer

dlg/250700/drainage letter addendum.doc



VICINITY MAP





REVISED CALCULATIONS

| | | | ED CA | | CA(100) | 0.98 | 0.74 | 0.84 | 0.07 | 0.34 | 0.10 |
|---|-------------------------|---|-----------------------------|-------|-----------|------|------|------|------|------|------|
| | | | WEIGHTED CA | | CA(5) | 0:00 | 0.69 | 0.78 | 0.06 | 0.32 | 60'0 |
| | | RIM | ТЕР | | C(100) | 0.91 | 0.96 | 96.0 | 96.0 | 0.96 | 0.96 |
| | | RY - INTEI | WEIGHTED | | C(5) | 0.83 | 0.00 | 06.0 | 0:00 | 06.0 | 06.0 |
| | | BASIN RUNOFF COEFFICIENT SUMMARY - INTERIM | REAS | | C(100) | 0.35 | 0.35 | 0.35 | 0.35 | 0.35 | 0.35 |
| | | FFICIENT | LANDSCAPE/UNDEVELOPED AREAS | | C(5) | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 |
| | | DFF COE | SCAPE/UND | | C(2) | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 |
| | | SIN RUNC | LAND | | AREA (AC) | 0.09 | 0.00 | 0.00 | 0.00 | 0.00 | 00.0 |
| | | | TS | | C(100) | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| | | FINAL DRAINAGE REPORT ~ | IMPERVIOUS AREA / STREETS | | C(5) | 0:90 | 0:90 | 0:90 | 0:00 | 0.90 | 0.90 |
| Fil. No. 1 | | DRAINA(| ERVIOUS AF | | C(2) | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| teway Subd. | | FINAL | IMP | | AREA (AC) | 0.99 | 0.77 | 0.87 | 0.07 | 0.35 | 0.10 |
| Academy Gateway Subd. Fil. No. 1 2507.00 | 08/11/17 KRC | | | TOTAL | AREA (AC) | 1.08 | 0.77 | 0.87 | 0.07 | 0.35 | 0,10 |
| JOB NAME: JOB NUMBER: | DATE: CALCULATED BY: | | | | BASIN | ф | ۵ | D1 | D2 | B3 | 5 |

Page lof 7

| | WEIGHTED CA | | (5) CA(100) | 0.98 | 9 0.74 | 8 0.84 | 6 0.07 | 2 0.34 | 9 0.10 |
|---|---------------------------------|-------|-------------|------|--------|--------|--------|--------|--------|
| | WEIGH | | CA(5) | 06.0 | 0.69 | 0.78 | 0.06 | 0.32 | 0.09 |
| | | | CA(2) | 0.88 | 0.69 | 0.77 | 90'0 | 0.31 | 0.09 |
| ATE | | | C(100) | 0.91 | 96:0 | 96:0 | 96.0 | 0.96 | 0.96 |
| | WEIGHTED | | C(5) | 0.83 | 06.0 | 06'0 | 06'0 | 06.0 | 0:00 |
| SUMMARY | | | C(2) | 0.82 | 68.0 | 68.0 | 68.0 | 0.89 | 0.89 |
| BASIN RUNOFF COEFFICIENT SUMMARY - ULTIMATE | AREAS | | C(100) | 0.35 | 0.35 | 0.35 | 0.35 | 0.35 | 0.35 |
| OFF COE | ANDSCAPE/UNDEVELOPED AREAS | | C(5) | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 |
| SIN RUN | SCAPE/UND | | C(2) | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 |
| | LAND | | AREA (AC) | 60.0 | 0.00 | 0.00 | 00:0 | 0.00 | 0.00 |
| GE REP(| ITS | | C(100) | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| E No. 7 FINAL DRAINAGE REPORT ~ | MPERVIOUS AREA / STREETS | | C(5) | 0:00 | 06.0 | 06.0 | 06.0 | 0.90 | 06.0 |
| Fit No. T | ERVIOUS AF | | C(2) | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| tteway Subd. | IMP | | AREA (AC) | 66:0 | 22.0 | 0.87 | 20:0 | 0.35 | 0.10 |
| Academy Gateway Subd. Fil. No. 1 2507.00 08/11/17 KRC FINAI | | TOTAL | AREA (AC) | 1.08 | 0.77 | 0.87 | 0.07 | 0.35 | 0.10 |
| JOB NAME: JOB NUMBER: DATE: CALCULATED BY: | | | BASIN | в | ۵ | Б | D2 | B | 4 |

| | | | () () () | | | | | | |
|---|---|-----------------------|---|------|------|------|------|------|------|
| F | | SWO | Q(100) (cfs) | 6 | 9 | 1 | - | 3 | |
| | | TOTAL FLOWS | Q(5) (cfs) | 5 | 4 | 4 | 0 | 2 | 0 |
| | | 101 | Q(2) (cfs) | 4 | 3 | 3 | 0 | 1 | 0 |
| | | 7 | l(100) (in/hr) | 8.68 | 8.68 | 8.68 | 8.68 | 8.68 | 8.68 |
| | M | NTENSITY | l(5) (in/hr) | 5.17 | 5.17 | 5.17 | 5.17 | 5.17 | 5.17 |
| | NTERI | Z | 1(2) (in/hr) | 4.12 | 4.12 | 4.12 | 4.12 | 4.12 | 4.12 |
| | AGE REPORT ~ BASIN RUNOFF SUMMARY - INTERIM | ЧC | TOTAL (min) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| | AMMU | -row | Tc (min) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | FF SL | STREET / CHANNEL FLOW | Length Slope Velocity (ft) (%) (fbs) (| 0:0 | 0.0 | 0.0 | 0.0 | 0.0 | 0:0 |
| | RUNO | T / CH | Slope (%) | 0.0% | 0.0% | %0.0 | %0.0 | %0.0 | 0.0% |
| | ASIN | STREE | Length (#) | • | 0 | 0 | 0 | 0 | 0 |
| | RT ~ B | | Tc (min) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| | EPOF | AND | Height (#) | 0 | 0 | 0 | 0 | 0 | 0 |
| No. 1 | AGE R | OVERLAND | Length Height (ft) (ft) | 0 | 0 | 0 | 0 | 0 | 0 |
| d. Fil. N | RAIN | | C(5) | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 |
| Academy G <u>ateway Sub</u> d. Fil. 2507.00 08/11/17 KRC | FINAL DRAIN | | CA(100) | 0.98 | 0.74 | 0.84 | 0.07 | 0.34 | 0.10 |
| Academy G 2507.00 08/11/17 KRC | | WEIGHTED | CA(5) | 06.0 | 69.0 | 82.0 | 90'0 | 0.32 | 60:0 |
| | | | CA(2) | 0.88 | 69:0 | 0.77 | 0.06 | 0.31 | 60:0 |
| JOB NAME: JOB NUMBER: DATE: CALC'D BY: | | | BASIN | ш | G | 10 | D2 | D3 | D4 |

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| 0 0 5.0 0. 0.0% 0.0 5.0 4.12 5.17 8.68 3 4 6 0 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 3 4 6 0 0 5.0 0.0 0.0 5.0 4.12 5.17 8.68 3 4 7 0 0 5.0 0.0 0.0 5.0 4.12 5.17 8.68 0 7 0 0 5.0 0.0 0.0 5.0 4.12 5.17 8.68 1 7 0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 1 2 3 4 7 0 0 5.0 5.0 4.12 5.17 8.68 1 2 3 3 4 7 0 0 0.0 0.0 5.0 |
|---|
| 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 3 4 4 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 3 4 4 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 0 |
| 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 0 0 0 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 0 0 0 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 1 2 0 5.0 0 0.0 0.0 0.0 5.0 4.12 5.17 8.68 1 2 |
| 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 1 2 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 1 2 |
| 0 5.0 0 0.0% 0.0 0.0 5.0 4.12 5.17 8.68 0 |
| |

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Academy Gateway Subd. Fil. No. 1 2507.00 08/11/17 JOB NAME: JOB NUMBER: DATE:

| CALCULATED BY: KRC | KRC | | | | | | | |
|--------------------|---|---------------------|---|------------------|-----------|---------|------|--------|
| | FINAL DRAINAGE REPORT ~ SURFACE ROUTING SUMMARY INTERIM | REPORT ~ (| SURFACE F | SOUTING S | UMMARY | INTERIM | | |
| | | | | | Intensity | sity | Flow | W |
| Design Point(s) | Contributing Basins | Equivalent CA(5) | Equivalent Equivalent Maximum CA(5) CA(100) Tc | Maximum Tc | l(5) | I(100) | Q(5) | Q(100) |
| 2 | В | 0.89 | 0.95 | 5.0 | 5.17 | 8.68 | 5 | 8 |
| 4 | D4 | 0.09 | 0.10 | 5.0 | 5.17 | 8.68 | 0.5 | 1 |
| 4A | 02 | 0.06 | 0.07 | 5.0 | 5.17 | 8.68 | 0.3 | ÷ |

Classic Consulting FDR AMEND calcs

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Academy Gateway Subd. Fil. No. 1 2507,00 08/11/17 JOB NAME: Job Number: Date:

| CALCULATED BY: KRC | KRC | | | | | | | |
|--------------------|--|---------------------|---|---------------|-----------|---------|------|--------|
| | FINAL DRAINAGE REPORT ~ SURFACE ROUTING SUMMARY - ULTIMATE | PORT ~ SL | JRFACE RC | UTING SU | MMARY - 1 | ULTIMAT | ш | Γ |
| | | | | | Intensity | sity | Flow | N |
| Design Point(s) | Contributing Basins | Equivalent CA(5) | Equivalent Equivalent Maximum CA(5) CA(100) Tc | Maximum Tc | l(5) | l(100) | Q(5) | Q(100) |
| 2 | В | 68.0 | 0.95 | 5.0 | 5.17 | 8.68 | 5 | 8 |
| 4 | D4 | 60.0 | 0.10 | 5.0 | 5.17 | 8.68 | 0.5 | - |
| 4A | D2 | 0.06 | 0.07 | 5.0 | 5.17 | 8.68 | 0.3 | - |

Classic Consulting FDR AMEND calcs

| JOB NAME: JOB NUMBER: DATE: Calculated BY: | Academy Gateway Subd. Fil. No. 2507.00 08/11/17 KRC | Va. 1 | | | | | | | |
|---|---|--------------------------------|-------------------------------|--|-------------|-----------|------|--------|------------|
| * | * PIPES ARE LISTED AT MAXIMUM SIZE REQUIRED TO ACCOMMODATE Q100 FLOWS AT MINIMUM GRADE. REFER TO INDIVIDUAL PIPE SHEETS FOR HYDRAULIC INFORMATION. | A SIZE REQUIRI EETS FOR HYD | ED TO ACCOMA RAULIC INFORM | AODATE Q100 F MATION. | LOWS AT MIN | IMUM GRAD | ш | | |
| | FINAL | . DRAINAG | E REPORT | FINAL DRAINAGE REPORT ~ PIPE ROUTING SUMMARY | UTING SU | MMARY | | | |
| | | | | | Intensity | sity | FIc | Flow | |
| Pipe Run | Contributing Basins | Equivalent CA(5) | Equivalent CA(100) | Maximum Tc | I(5) | l(100) | Q(5) | Q(100) | Pipe Size* |
| ţ. | BASIN D3 | 0.32 | 0.34 | 5.00 | 5,17 | 8.68 | 2 | 3 | 12" STORM |
| 2 | PIPE 1 DP 4A | 0.38 | 0.40 | 5.00 | 5.17 | 8.68 | 2 | 3 | 18" STORM |
| 3 | BASIN D1 | 0.78 | 0.84 | 5.00 | 5.17 | 8.68 | 4 | 7 | 18" STORM |
| 4 | PIPE 2 & PIPE 3 | 1.16 | 1.24 | 5.00 | 5.17 | 8.68 | 6 | 11 | 24" STORM |
| 5 | BASIN D | 0.69 | 0.74 | 5.00 | 5.17 | 8.68 | 4 | 9 | 18" STORM |
| 9 | PIPE 4, PIPE 5 & DP 4 | 1.94 | 2.07 | 5.00 | 5.17 | 8.68 | 10 | 18 | 24" STORM |

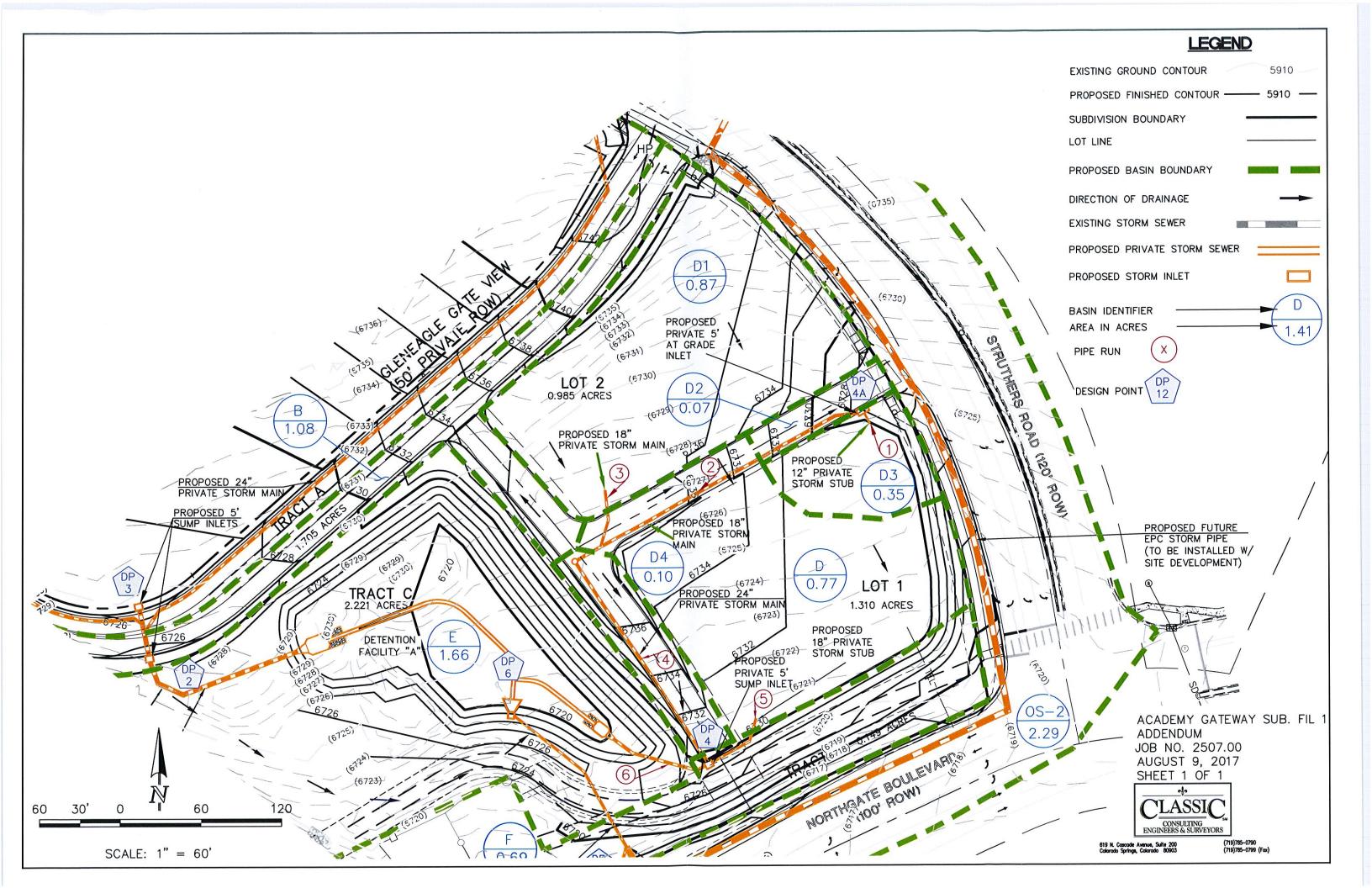
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| JOB NAME: | Academy G | ater | vay Subd. Fil. No. 1 | | | | | |
|---|-------------------|------|--|--|--|--|--|--|
| JOB NUMBER: | 2507.00 | | | | | | | |
| DATE: | 08/11/17 | | - | | | | | |
| CALCULATED BY: | KRC | | | | | | | |
| | 4 | | | | | | | |
| DESIGN POINT | 4 | | | | | | | |
| | | | | | | | | |
| Total Flow | r: Q ₅ | = | 0 cfs | | | | | |
| | Q ₁₀₀ | = | 1 cfs | | | | | |
| Maximum allowable ponding depth at sump: | | | | | | | | |
| | D_5 | = | 0.50 | | | | | |
| | D ₁₀₀ | = | 1.00 (dmax) | | | | | |
| | Qi | = | 1.7(Li+1.8(W))(dmax + w/12)^1.85 | | | | | |
| Clo | gging Factor | = | 1,25 | | | | | |
| | | | Length of inlet opening | | | | | |
| | | | | | | | | |
| 5-Year Event: | 4 | | foot inlet required | | | | | |
| 100-Year Event: | 4 | | foot inlet required | | | | | |
| INSTALL A PUBLIC 100 YR DEVELOPED FLOW | 4 S AT THIS D | ESI | FT D-10-R INLET TO ACCEPT BOTH 5YR & GN POINT. | | | | | |

| | Academy Gateway Subd. Fil. No. 1 | | | | | | | |
|----------------|----------------------------------|--------|------|------------------------|------|--|--|--|
| JOB NUMBER: | 2507.00 08/11/17 KRC | | | | | | | |
| DATE: | | | | | | | | |
| CALCULATED BY: | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| DESIGN POINT | ESIGN POINT 4A 100 YEAR FLOW | | | | | | | |
| | | | | | | | | |
| Q(100) | 1 | l(100) | 8.7 | | | | | |
| ····· | | | | | | | | |
| DEPTH | 0.20 | Fr | 1.83 | Inlet size ? L(i) = | 5 | | | |
| | | | | | | | | |
| SPREAD | 3.5 | L(1) | 4.9 | If Li < L(2) then Qi = | 1 | | | |
| | | | | | | | | |
| CROSS SLOPE | 2.0% | L(2) | 3.0 | If Li > L(2) then Qi = | 0.4 | | | |
| | | | | | | | | |
| STREET SLOPE | 4.0% | L(3) | 10.5 | FB = | 0 | | | |
| | | ., | | | | | | |
| | | | | CA(eqv.)= | 0.00 | | | |
| | | 1 | | | | | | |
| | | | | | | | | |
| | - | | | | | | | |
| | | | | 5 YEAR FLOW | | | | |
| Q(5) | 0.3 | l(5) | 5.2 | | | | | |
| | | | | | | | | |
| DEPTH | 0.20 | Fr | 1.87 | Inlet size ? L(i) = | 5 | | | |
| | | | | | | | | |
| SPREAD | 3.8 | L(1) | 5.4 | If Li < L(2) then Qi = | 0 | | | |
| | · · · · | | | | | | | |
| CROSS SLOPE | 2.0% | L(2) | 3.2 | If Li > L(2) then Qi = | 0 | | | |
| | | ·' | | | | | | |
| STREET SLOPE | 4.0% | L(3) | 11.6 | FB = | 0 | | | |
| | | | | | | | | |
| | | | | CA(eqv.)= | 0.00 | | | |



DRAINAGE MAP



Appendix D

Drainage Plan

Jay M. Newell, PE Wayne T. Sterling, RLA, LEED AP

> 2009 W. Littleton Blvd. #300 Littleton, CO 80120

