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PAVEMENT DESIGN REPORT
STERLING RANCH, FILING NO. 5
EL PASO COUNTY, COLORADO

PCD File No. SF241

Prepared for:
Classic Communities
2138 Flying Horse Club Drive
Colorado Springs, CO 80904

Attn: Austin Lenz

October 17, 2024

Respectfully Submitted,

ENTECH ENGINEERING, INC.

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Entech Job No. 240368

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1 Introduction

Entech Engineering, Inc. (Entech) completed a subsurface exploration program, laboratory testing, and pavement design for roadways within the Sterling Ranch, Filing No. 5. This report describes the subsurface exploration program conducted for the proposed roadway improvements and provides pavement section alternatives and construction recommendations. Entech participated in this project as a subconsultant to Classic Communities. The contents of this report, including the pavement design recommendations, are subject to the limitations and assumptions presented in Section 7.

2 Project Description

The site is located northwest of the intersection of Sterling Ranch Road and Dines Boulevard within Sterling Ranch, Filing No. 5, in El Paso County, Colorado (Figure 1). The proposed improvements include the paving of sections of Abby House Lane, Manor House Way, and School House Way. The extent of our investigation is shown in Figure 2.

At the time of our subsurface exploration program, the existing roadway had been rough-graded and utilities had been installed. Surrounding properties are comprised of vacant land, land being developed for future residential lots, and an existing subdivision. Based on the development plans, the roadways are designated as urban local roadways.

3 Subsurface Explorations and Laboratory Testing

3.1 Subsurface Exploration Program

Subsurface conditions at the project site were explored by eleven test borings, designated TB-1 through TB-11, drilled on September 17, 2024. The locations of the test borings are shown on the Site and Exploration Plan (Figure 2). The borings were drilled to depths of 5 to 10 feet below the existing ground surface (bgs). The drilling was performed using a truck-mounted, continuous flight auger drill rig supplied and operated by Entech. Descriptive boring logs providing the lithologies of the subsurface conditions encountered during drilling are presented in Appendix A. Groundwater levels were measured in each of the open boreholes at the conclusion of drilling.

Soil and bedrock samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D1586) using a split-barrel California sampler. Results of the Standard Penetration Test (SPT) are included on the boring logs in terms of N-values expressed in blows per foot (bpf). Soil

and bedrock samples recovered from the borings were visually classified and recorded on the boring logs. The soil classifications were later verified utilizing laboratory testing and grouped by soil type. The soil type numbers are included on the boring logs. It should be understood that the soil descriptions shown on the boring logs may vary between boring location and sample depths. It should also be noted that the lines of stratigraphic separation shown on the boring logs represent approximate boundaries between soil types and the actual stratigraphic transitions may be more gradual or variable with location.

3.2 Geotechnical Index and Engineering Property Testing

Water content testing (ASTM D2216) was performed on the samples recovered from the borings, and the results are shown on the boring logs. Grain-Size Analysis (ASTM D422) and Atterberg Limits testing (ASTM D4318) were performed on selected samples to assist in classifying the materials encountered in the borings.

One-dimensional swell or collapse testing (ASTM D4546) was performed on select samples to determine the swell or collapse potential of the soil. For pavement design, a modified proctor (ASTM D1557) and California Bearing Ratio (CBR) test (ASTM D1883) were completed. Soluble sulfate testing was performed on select soil samples to evaluate the potential for below-grade degradation of concrete due to sulfate attack. The laboratory testing results are presented in Appendix B and summarized in Table B-1.

Strength testing was performed on two sets of soil/cement composite samples for each Soil Type 1 and Soil Type 2. Testing was performed on soil samples prepared with 2% and 4% Portland Cement Type 1L. A compression strength of 125 pounds per square inch (psi) is recommended for cement-stabilized subgrade. The 6-day average strength value of the 2% mix was 213 psi and 185 psi, respectively and the 6-day strength of the 4% mix was 243 psi and 210 psi, respectively. A 2% mix is recommended based on the laboratory test results. A summary of the testing results is attached in Appendix B, Table B-2.

4 Subgrade Conditions

Five primary soil types and one bedrock type were encountered in the test borings drilled for the subsurface investigation. Each soil type was classified in accordance with the Unified Soil Classification System (USCS) and the American Association of State Highway and Transportation

Officials (AASHTO) soil classification system using the laboratory testing results and the observations made during drilling.

4.1 Subsurface Conditions

Subsurface conditions along the proposed roadways consisted of loose to medium dense silty sand and sand with silt fill (Soil Type 1, AASHTO A-1-b, A-2-4, A-2-6, and A-4), loose to medium dense clayey sand fill (Soil Type 2, AASHTO A-1-b, A-6, and A-2-6), and stiff to very stiff sandy clay fill (Soil Type 3, AASHTO A-6). Soils also encountered in two of the test borings was native dense clayey sand (Soil Type 4, AASHTO A-1-b, A-2-6) and in one of the test borings was native hard sandy clay (Soil Type 5, AASHTO A-6). Extremely weak to very weak sandstone bedrock, or very dense silty to clayey sand when classified as a soil (Soil Type 6, AASHTO A-1-b, A-2-6) was encountered in two of the test borings. Water soluble sulfate testing results showed less than 0.01% sulfates which indicates that the soils exhibit a negligible potential for sulfate attack.

4.2 Groundwater

Groundwater was not encountered in the test borings. Groundwater fluctuations are possible and will depend on seasonal variations, local precipitation, runoff, and other factors, however, we do not anticipate groundwater to affect the proposed construction.

5 Pavement Design Recommendations

Pavement design recommendations were made in accordance with the *El Paso County Engineering Criteria Manual (ECM).*

5.1 Subgrade Conditions

California Bearing Ratio (CBR) testing was performed on representative samples of the Soil Type 1 silty sand fill subgrade from TB-10 and from Soil Type 2 clayey sand fill from TB-8 to determine the support characteristic of the subgrade soils. The results of the CBR testing are presented in Appendix B and summarized in Exhibit 1.

Exhibit 1: Subsurface Laboratory Testing Summary

Design Parameter	Value	
Soil Type	1 – Clayey Sand Fill	2 – Clayey Sand Fill
CBR at 95%	31.1	8.7
Design CBR	10	8.7
Liquid Limit	36	31
Plasticity Index	15	20
Percent Passing 200	23.8	41.9
AASHTO Classification	A-2-6	A-6
Unified Soils Classification	SC	SC

5.2 Swell Mitigation

El Paso County requires swell mitigation for soils with swell testing results greater than 2% under a 150 pounds per square foot (psf) surcharge. Based on the subgrade soils classification and swell testing, mitigation for expansive soils will not be required on this site.

5.3 Traffic Loading

Traffic data is not available for the future interior roads in the Sterling Ranch, Filing No. 5 subdivision; however, the roads are classified as local roadways based on current development plans. The *El Paso County Engineering Criteria Manual* provides default 18-kip equivalent single axle loadings (ESAL) based on the street classification. For design, a default ESAL value of 292,000 was used for the local urban road designation.

5.4 Pavement Design

The pavement sections were determined utilizing the *El Paso County Engineering Criteria Manual*, the CBR testing, and default ESALs. Design parameters used in the pavement analysis are presented in Exhibit 2.

Exhibit 2: Pavement Design Parameters

Design Parameter	Value
Reliability	80%
Standard Deviation	0.45
Serviceability Loss (Δ psi)	2.5
Design CBR	8.7
Resilient Modulus	13,050 psi
Structural Coefficients	
Hot Bituminous Pavement	0.44
Aggregate Base Course	0.11
Recycled Concrete Base	0.11
Cement Stabilized Subgrade	0.11

Pavement section alternatives recommended for the roadways included in this phase filing are summarized in Exhibit 3. The pavement design calculations are presented in Appendix C.

Exhibit 3: Recommended Pavement Sections

Pavement Area	Design ESAL	Alternative [1]
Abby House Lane, Manor House Way, School House Way	292,000	1. 3.0 inches HMA over 8.0 inches ABC/RCB
		2. 3.0 inches HMA over 8.0 inches CTS

ABC = Aggregate Base Course; ESAL = equivalent single axle loads; HMA = Hot Mix Asphalt; CTS = Cement Treated Soil; RCB= Recycled Concrete Base

Notes:

1. The use of CTS will require a deviation request approval.

6 Construction Recommendations

Pavement design recommendations provided herein are contingent on good construction practices, and poor construction techniques may result in poor performance. Our analyses assumed that this project will be constructed according to the *El Paso County Engineering Criteria Manual* and the *Pikes Peak Region Asphalt Paving Specifications*.

6.1 Earthwork Recommendations for Pavement Subgrade

Proper subgrade preparation is required for adequate pavement performance. Paving areas should be cleared of all deleterious materials including but not limited to: existing pavements, utility poles, and fence poles. Surface vegetation, if any, should be removed by stripping, with the depth to be field determined. Isolated pockets of high cohesive soils such as those encountered in boring TB-7 should be removed and replaced with granular fill. In addition, loose soils such as

those encountered in boring TB-8 should be overexcavated to underlying dense and unyielding subgrade. Granular soils can be replaced in accordance with Section 6.1.3.

6.1.1 Subgrade Preparation – Unbound Base Alternatives

If pavement section alternatives are selected utilizing aggregate base course (ABC) or Recycled Concrete Base (RCB), the final subgrade surface should be scarified to a depth of 8 inches, moisture conditioned within +/- 2% of the optimum water content, and recompacted to 95% of the Modified Proctor (ASTM 1557) maximum dry density.

The compacted surface below pavements should be proof-rolled with a fully loaded, tandem-axle, 10-yard dump truck or equivalent. Any areas, that are delineated to be soft, loose, or yielding during proof-rolling should be removed and reconditioned or replaced.

6.1.2 Subgrade Preparation – Cement Treated Subgrade

Prior to placement of cement stabilization a preliminary proof roll should be completed with a fully loaded, tandem-axle, 10-yard dump truck or equivalent. Any areas that are delineated to be soft, loose, or yielding during proof-rolling should be removed and reconditioned or replaced.

Following the preliminary proof roll, the subgrade shall be stabilized by the addition of cement. The amount of cement applied shall be a minimum of 2% (by weight) of the subgrade's maximum dry density as determined by the Modified Proctor (ASTM D1557) for granular soils or by the Standard Proctor (ASTM D698) for cohesive soils. The cement should be spread evenly on the subgrade surface and be thoroughly mixed into the subgrade such that a uniform blend of soil and cement is achieved to the CTS design depth. Densification of the cement-stabilized subgrade should be completed to obtain a compaction of at least 95% of the subgrade maximum dry density as determined by the Modified Proctor (ASTM D1557) or by the Standard Proctor (ASTM D698). Satisfactory compaction of the subgrade shall occur within 90 minutes from the time of mixing the cement into the subgrade.

The following conditions shall be observed as part of the subgrade stabilization:

- Type I/II or Type 1L cement as supplied; a local supplier shall be used. All cement used for stabilization should come from the same source. If cement sources are changed, a new laboratory mix design should be completed.
- Moisture conditioning of the subgrade and/or mixing of the cement into the subgrade shall not

occur when soil temperatures are below 40 degrees F. Cement treated subgrades should be maintained at a temperature of 40 degrees F or greater until the subgrade has been compacted as required.

- Cement placement, cement mixing, and compaction of the cement treated subgrade should be observed by Entech Engineering. Testing should include in-situ compaction tests and representative compacted specimens of the treated subgrade material for subsequent laboratory quality assurance testing. Testing reports will be provided to El Paso County as construction progresses.
- A minimum 7-day CTS compressive strength of 125 psi must be achieved.
- Soil strengths in excess of 275 psi will require microfracturing. Microfracturing will be completed using the Standard Method as defined by the *City of Colorado Springs Draft Standard Specification,* Section 305 – Chemically Treated Subgrade. Microfracturing will be performed with the same (or equivalent tonnage) steel drum vibratory roller used for compaction of the CTS. A minimum of 12-ton roller shall be used. Three full passes with the roller operating at maximum amplitude and traveling at 2- 3 mph shall be applied. If the treated material breaks up excessively at the surface, the vibration amplitude shall be decreased or eliminated.

6.1.3 Fill Placement and Compaction

Granular fill placed as part of the pavement subgrade shall consist of non-expansive, granular soil, free of organic matter, unsuitable materials, debris, and cobbles greater than 3 inches in diameter. Additionally, any granular fill placed as part of the roadway subgrade should have a minimum CBR of 8. All granular fill placed within the pavement subgrade should be compacted to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density at +/-2% of optimum moisture content. Fill material should be placed in horizontal lifts such that each finished lift has a compacted thickness of 6 inches or less. Entech should approve any imported fill to be used within the pavement subgrade area prior to delivery to the site.

6.1.4 Aggregate Base Course and Recycled Concrete Base

ABC or RCB materials shall conform to the *El Paso County Standard Specifications Manual*, Section 300 Aggregate Base Course. ABC or RCB materials should be compacted to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density within +/-2% of optimum moisture content.

6.2 Concrete Degradation Due to Sulfate Attack

Sulfate solubility testing was conducted on several samples recovered from the test borings to evaluate the potential for sulfate attack on concrete. The test results indicated less than 0.01% soluble sulfate (by weight). The test results indicate the sulfate component of the in-place soils presents a negligible to severe exposure threat to concrete placed below the site grade.

As presented in *Evaluation of Selected Pavement Specifications and Responses to Questions Relevant to Design and Construction of Cement-Treated Soil and Aggregate Layers in El Paso County, Colorado* report from Spencer Gutherie and Robert Stevens dated March 13, 2024 soils with less than 3,000 ppm (0.3%) do not require special construction practices.

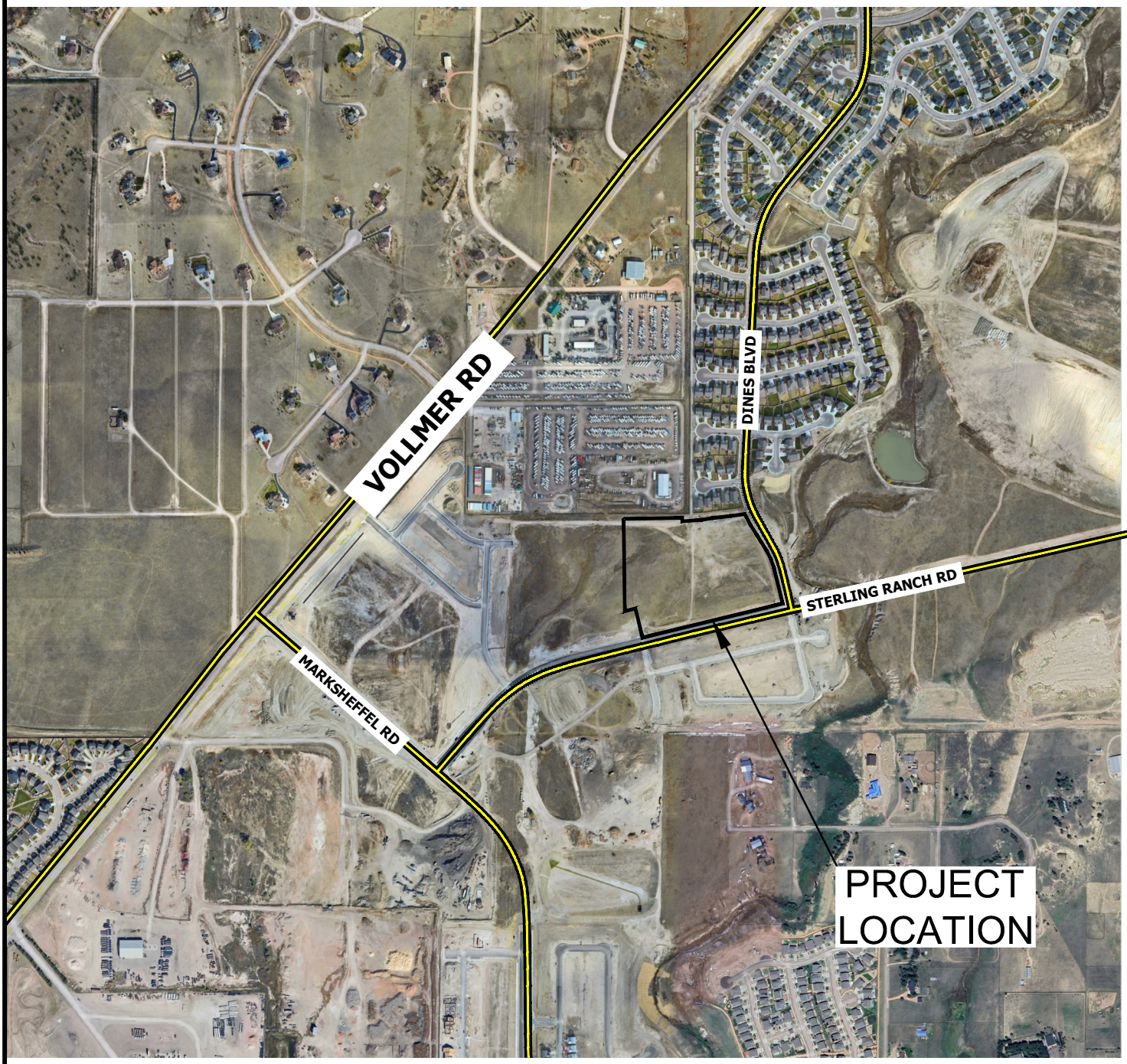
6.3 Construction Observation

Subgrade preparation for pavement structures should be observed by Entech in order to verify that (1) no anomalies are present, (2) materials similar to those described in this report have been encountered or placed, and (3) no soft spots, expansive or organic soil, or debris are present in the pavement subgrade prior to paving. Construction observation requirements as presented in the Use of CTS for Paving Season Memorandum should be followed.

7 Closure

The subsurface investigation, geotechnical evaluation, and recommendations presented in this report are intended for use by Classic Communities with application to the paving of the Sterling Ranch, Filing No. 5 project in El Paso County, Colorado. In conducting the subsurface investigation, laboratory testing, engineering evaluation, and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in the same locality and under similar conditions. No other warranty, expressed or implied, is made. During final design and/or construction, if conditions are encountered that appear different from those described in this report, Entech Engineering, Inc. requests to be notified so that the evaluation and recommendations presented herein can be reviewed and modified as appropriate.

If there are any questions regarding the information provided herein, or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.



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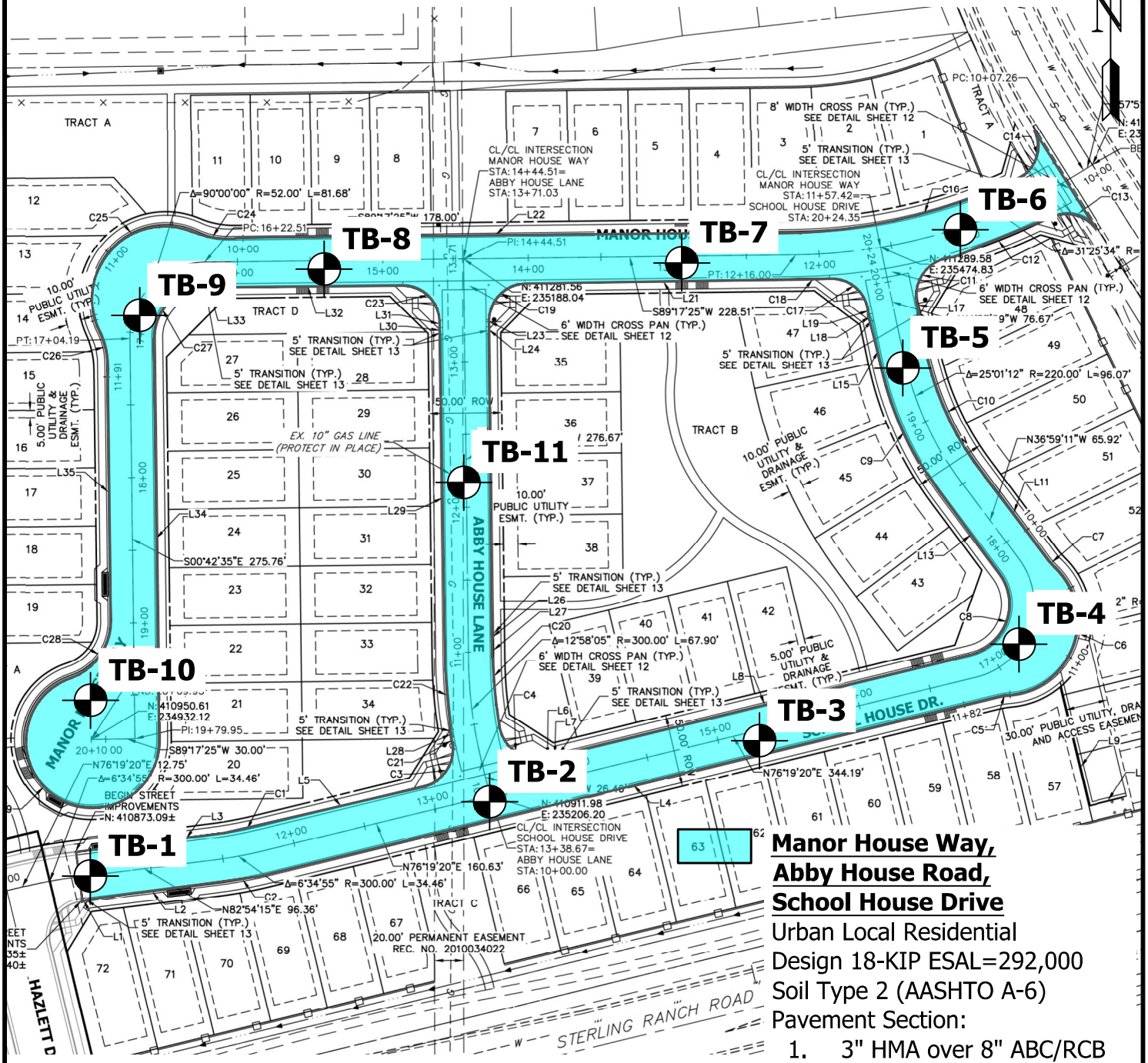
VICINITY MAP

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO.
240368

FIG. 1



Manor House Way,
Abby House Road,
School House Drive

Urban Local Residential
Design 18-KIP ESAL=292,000
Soil Type 2 (AASHTO A-6)
Pavement Section:

1. 3" HMA over 8" ABC/RCB
2. 3" HMA over 8" CTS

TB- APPROXIMATE TEST BORING LOCATION AND NUMBER

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SITE AND EXPLORATION MAP

STERLING RANCH, FILING NO. 5
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FIG. 2

APPENDIX A: Test Boring Logs

TEST BORING 1
DATE DRILLED 9/17/2024

REMARKS

DRY TO 10', 9/17/24

FILL 0-10', SAND, SILTY, TAN to BROWN, LOOSE to MEDIUM DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			6	8.1	1
5			16	9.5	1
10			4	13.7	1
15					
20					

TEST BORING 2
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

FILL 0-5', SAND, SILTY, BROWN to TAN, LOOSE to MEDIUM DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			7	6.5	1
5			12	5.9	1
10					
15					
20					

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TEST BORING LOGS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

FIG. A-1

TEST BORING 3
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			9	7.4	1
5			11	5.1	1
10					
15					
20					

FILL 0-5', SAND, SILTY, TAN to BROWN, LOOSE to MEDIUM DENSE, MOIST

TEST BORING 4
DATE DRILLED 9/17/2024

REMARKS

DRY TO 10', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			14	4.0	1
5			35	11.6	4
10			50/5"	7.4	5
15					
20					

FILL 0-4', SAND, WITH SILT, TAN, MEDIUM DENSE, MOIST

SAND, CLAYEY, TAN, DENSE, MOIST

SANDSTONE, WEAK, TAN, WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)

ENTECH ENGINEERING, INC.

TEST BORING LOGS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

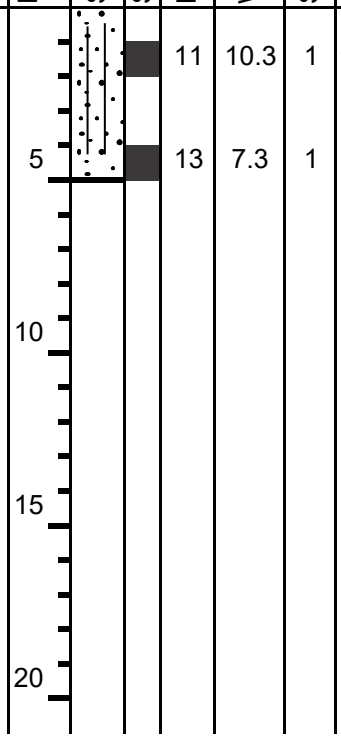
FIG. A-2

TEST BORING 5
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

FILL 0-5', SAND, SILTY, BROWN to TAN, MEDIUM DENSE, MOIST



Blows per foot	Watercontent %	Soil Type
11	10.3	1
13	7.3	1

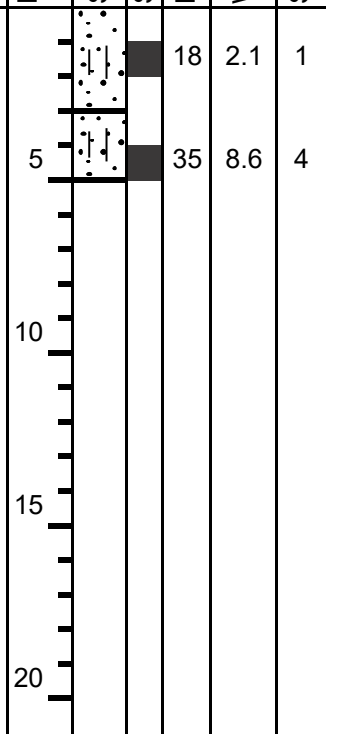
TEST BORING 6
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

FILL 0-3', SAND, SLIGHTLY SILTY, TAN, MEDIUM DENSE, DRY

SAND, SILTY, TAN, DENSE, MOIST



Blows per foot	Watercontent %	Soil Type
18	2.1	1
35	8.6	4

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TEST BORING LOGS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

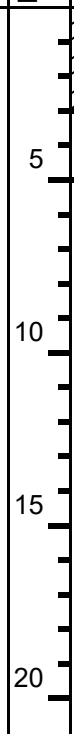
FIG. A-3

TEST BORING 7
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', CLAY, SLIGHTLY, SANDY, BROWN, STIFF, MOIST			10	17.7	3
FILL, SAND, SILTY, BROWN, MEDIUM DENSE, MOIST			19	3.5	1

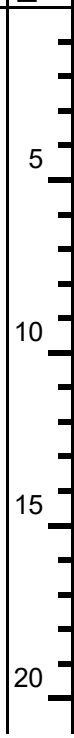


TEST BORING 8
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, BROWN to TAN, LOOSE, MOIST			4	8.6	2
			5	13.3	2



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TEST BORING LOGS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

FIG. A-4

TEST BORING 9
DATE DRILLED 9/17/2024

REMARKS

DRY TO 10', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			16	8.2	2
5			7	18.2	2
10			40	19.4	5
15					
20					

FILL 0-9', SAND, CLAYEY, TAN to BROWN, MEDIUM DENSE to LOOSE, MOIST

CLAY, SANDY, TAN, HARD, MOIST

TEST BORING 10
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			50/10"	11.0	6
5			50/11"	11.9	6
10					
15					
20					

FILL 0-1', SAND, CLAYEY, BROWN

SANDSTONE, VER YWEAK, TAN, MODERATELY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)

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TEST BORING LOGS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

FIG. A-5

TEST BORING 11
DATE DRILLED 9/17/2024

REMARKS

DRY TO 5', 9/17/24

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, BROWN to TAN, MEDIUM DENSE, MOIST					
			14	8.1	2
5			12	10.0	2
10					
15					
20					

ENTECH ENGINEERING, INC.

TEST BORING LOGS

STERLING RANCH, FILING NO. 5
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JOB NO. 240368

FIG. A-6

APPENDIX B: Laboratory Test Results

TABLE B-1
SUMMARY OF LABORATORY TEST RESULTS

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX	SULFATE (WT %)	SWELL/ COLLAPSE (%)	AASHTO CLASS. (GROUP INDEX)	USCS	SOIL DESCRIPTION
1, CBR	10	0-3	8.1	124.3	23.8	36	21	15		0.5	A-2-6 (0)	SC	FILL, SAND, CLAYEY
1	1	1-2	8.1		30.2	27	26	1			A-2-4 (0)	SM	FILL, SAND, SILTY
1	2	1-2	6.5		19.0	NV	NP	NP	<0.01		A-1-b (0)	SM	FILL, SAND, SILTY
1	3	1-2	7.4		19.9	NV	NP	NP			A-1-b (0)	SM	FILL, SAND, SILTY
1	4	1-2	4.0		9.4	NV	NP	NP			A-1-b (0)	SW-SM	FILL, SAND, WITH SILT
1	5	1-2	10.3		38.8	28	26	2			A-4 (0)	SM	FILL, SAND, SILTY
1	6	1-2	2.1		4.5	NV	NP	NP	0.00		A-1-b (0)	SW	FILL, SAND, SLIGHTLY SILTY
2, CBR	8	0-3	9.6	115.6	41.9	31	20	11		1.0	A-6 (1)	SC	FILL, SAND, CLAYEY
2	8	1-2	8.6		38.1	29	21	8			A-4 (0)	SC	FILL, SAND, CLAYEY
2	9	1-2	8.6	115.9	21.0	33	22	11		0.3	A-2-6 (0)	SC	FILL, SAND, CLAYEY
2	11	1-2	8.1		36.4	33	22	11			A-6 (0)	SC	FILL, SAND, CLAYEY
3	7	1-2	17.7	108.8	83.9	32	20	12		0.4	A-6 (10)	CL	FILL, CLAY, SLIGHTLY SANDY
5	9	10	16.9	99.8	69.9	35	13	22		0.9	A-6 (13)	CL	CLAY, SANDY
6	10	1-2	11.0		16.0	NV	NP	NP	<0.01		A-1-b (0)	SM	SANDSTONE (SAND, SILTY)
6	4	10	12.3	115.0	22.0	37	24	13	<0.01	-0.4	A-2-6 (0)	SC	SANDSTONE (SAND, CLAYEY)

Project: Sterling Ranch, Filing No. 5
Client: Classic Communities
Job No: 240368

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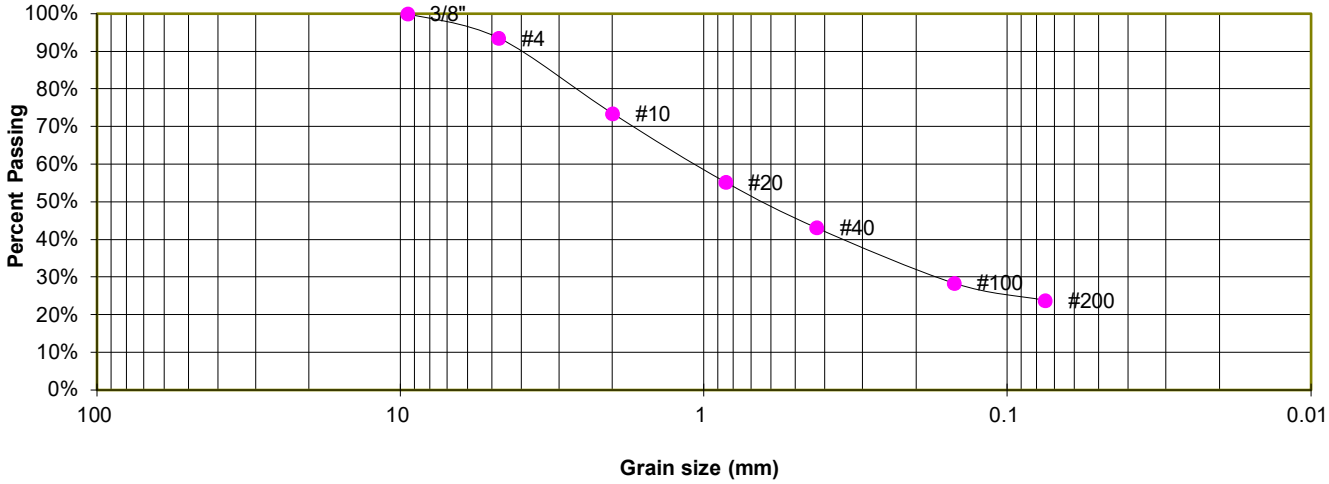
TABLE B-2
SUMMARY OF CTS TEST RESULTS

FIELD SAMPLE ID	*SOIL ADDITIVE*	*ADDITIVE PERCENTAGE (%)*	*WATER CONTENT (%)*	*DENSITY (dry)*	*AGE (days)*	*STRENGTH (psi)*
TB-10 @ 0-3'	TYPE IL CEMENT	2	7.3	121.9	5	219
				121.7		206
				121.8		214
					AVERAGE:	**213**
TB-10 @ 0-3'	TYPE IL CEMENT	4	7.3	121.8	5	238
				121.6		234
				121.5		257
					AVERAGE:	**243**
TB-8 @ 0-3'	TYPE IL CEMENT	2	9.2	115.3	5	193
				115.0		185
				114.8		177
					AVERAGE:	**185**
TB-8 @ 0-3'	TYPE IL CEMENT	4	9.2	115.1	5	212
				115.0		202
				114.7		216
					AVERAGE:	**210**

Notes:

1. CURING METHOD: 100° HUMIDIFIED OVEN

TEST BORING	10	SOIL DESCRIPTION	FILL, SAND, CLAYEY
DEPTH (FT)	0-3	SOIL TYPE	1, CBR



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	93.6%
10	73.4%
20	55.3%
40	43.1%
100	28.4%
200	23.8%

ATTERBERG LIMITS

Plastic Limit	21
Liquid Limit	36
Plastic Index	15

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-6
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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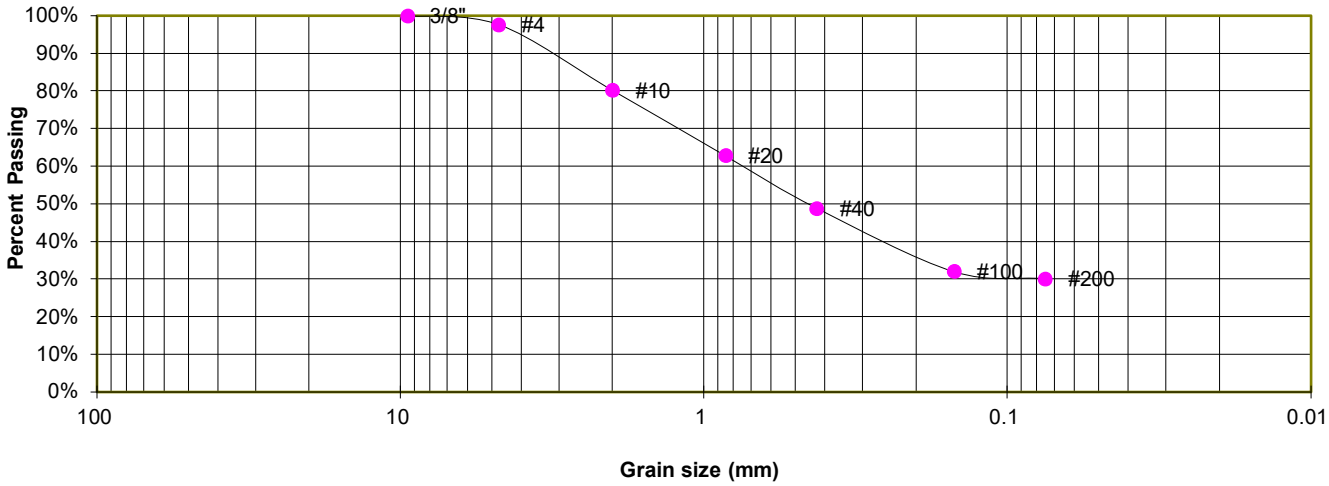
FIG. B-1

TEST BORING	1	SOIL DESCRIPTION	FILL, SAND, SILTY
DEPTH (FT)	1-2	SOIL TYPE	1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	97.7%
10	80.2%
20	62.8%
40	48.8%
100	32.1%
200	30.2%

ATTERBERG LIMITS

Plastic Limit	26
Liquid Limit	27
Plastic Index	1

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0

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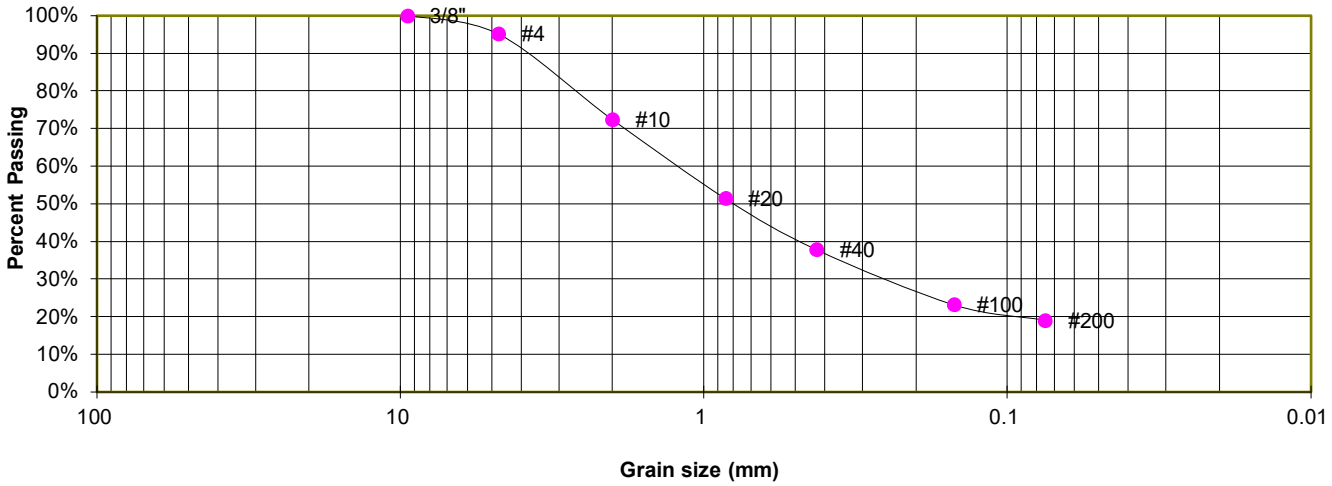
FIG. B-2

TEST BORING	2	SOIL DESCRIPTION	FILL, SAND, SILTY
DEPTH (FT)	1-2	SOIL TYPE	1

Sieve Analysis
Grain Size Distribution



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	95.2%
10	72.4%
20	51.4%
40	37.8%
100	23.2%
200	19.0%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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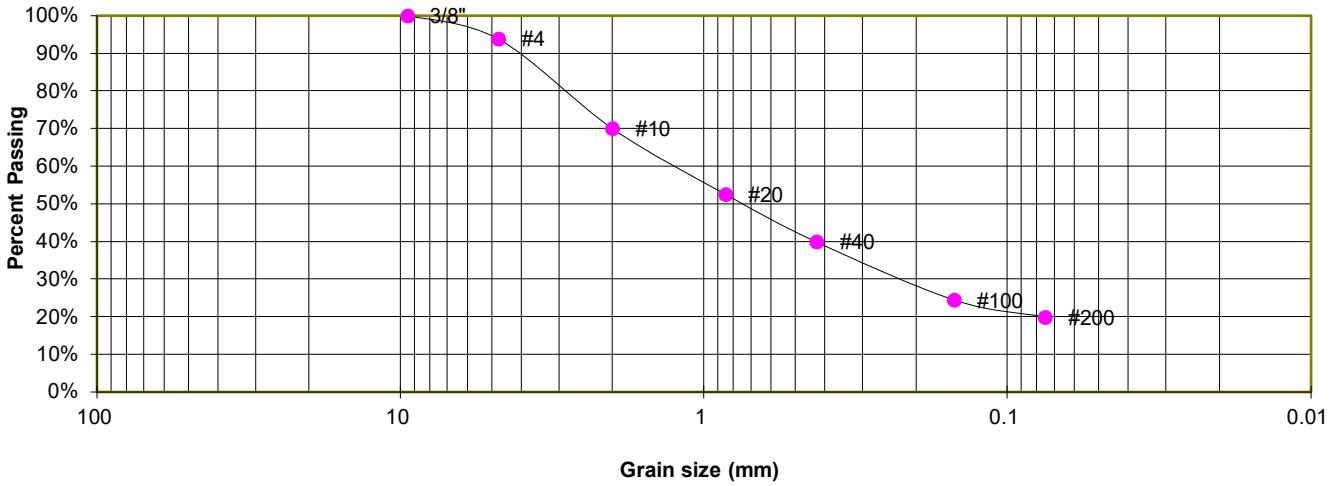
FIG. B-3

TEST BORING	3	SOIL DESCRIPTION	FILL, SAND, SILTY
DEPTH (FT)	1-2	SOIL TYPE	1

Sieve Analysis
Grain Size Distribution



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	93.8%
10	70.0%
20	52.5%
40	39.9%
100	24.5%
200	19.9%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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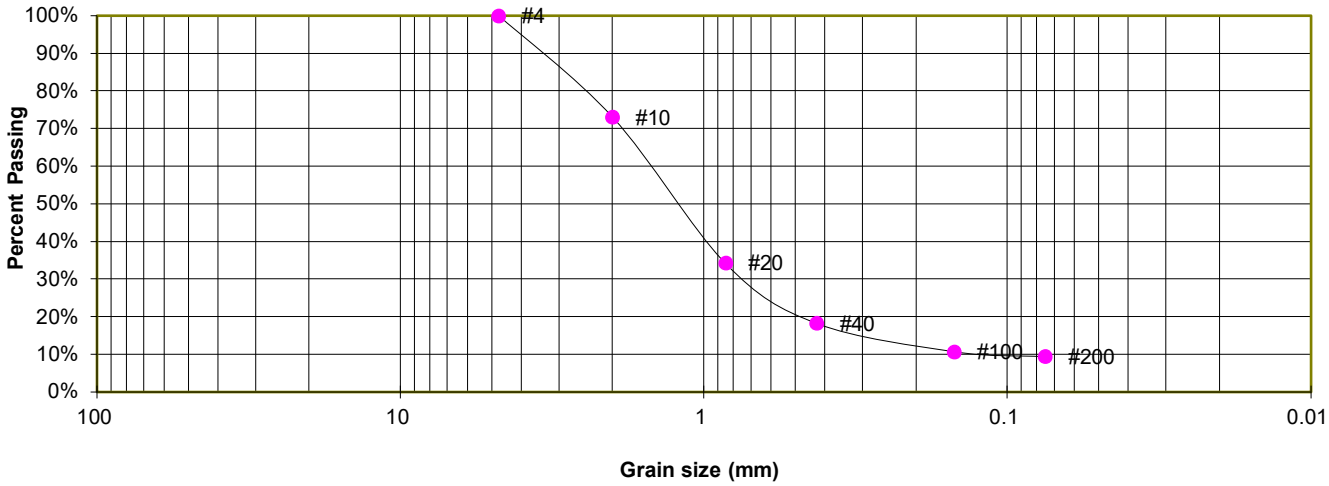
FIG. B-4

TEST BORING	4	SOIL DESCRIPTION	FILL, SAND, WITH SILT
DEPTH (FT)	1-2	SOIL TYPE	1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	73.1%
20	34.2%
40	18.3%
100	10.7%
200	9.4%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM
AASHTO CLASSIFICATION: A-1-b
AASHTO GROUP INDEX: 0

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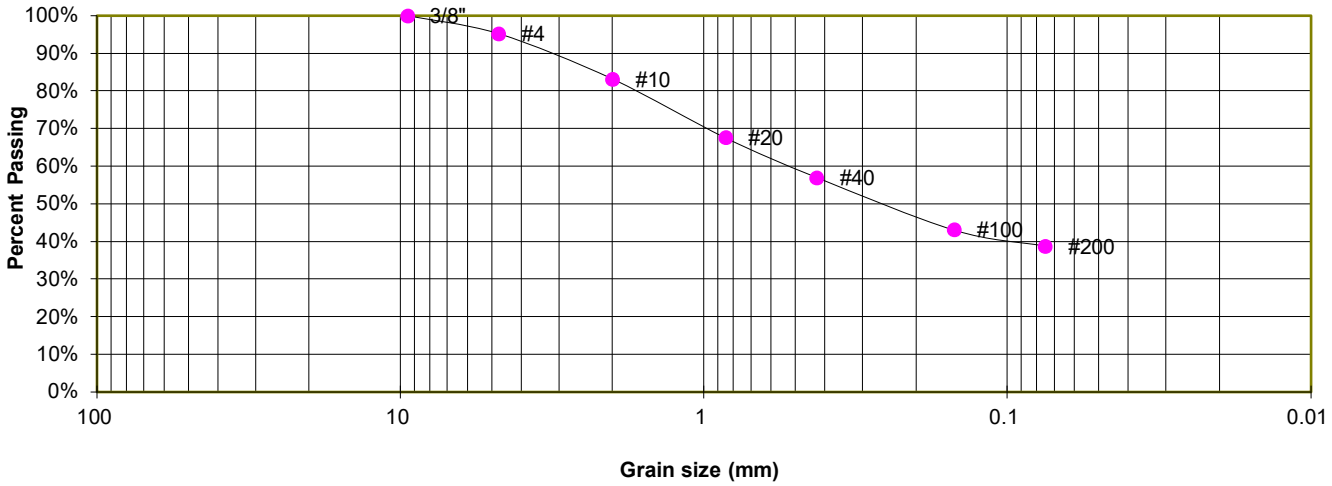
FIG. B-5

TEST BORING	5	SOIL DESCRIPTION	FILL, SAND, SILTY
DEPTH (FT)	1-2	SOIL TYPE	1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	95.3%
10	83.1%
20	67.7%
40	57.0%
100	43.1%
200	38.8%

ATTERBERG LIMITS

Plastic Limit	26
Liquid Limit	28
Plastic Index	2

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	0

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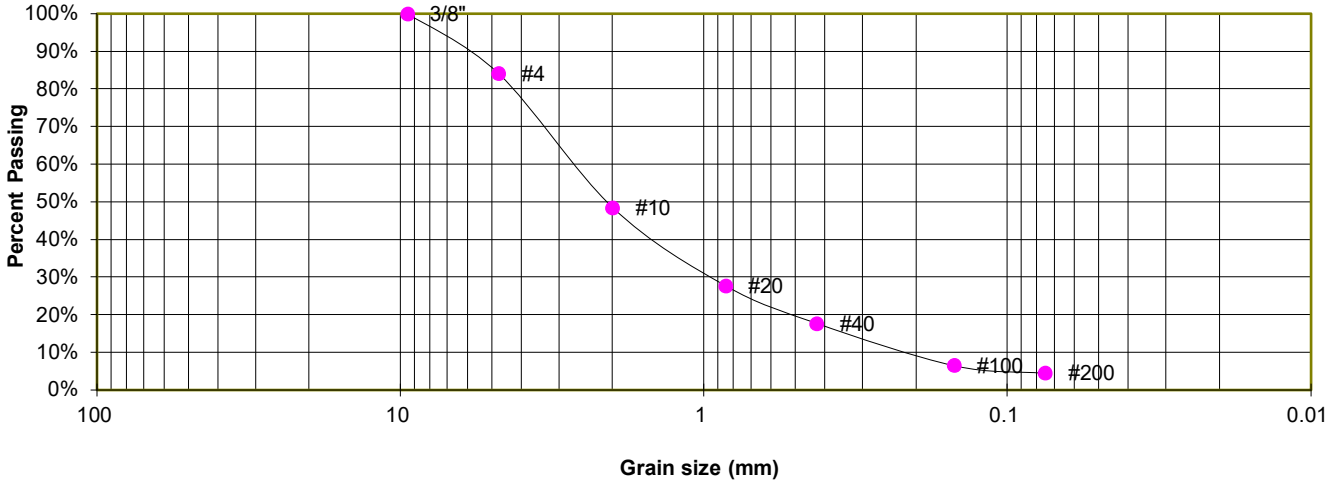
FIG. B-6

TEST BORING	6	SOIL DESCRIPTION	FILL, SAND, SLIGHTLY SILTY
DEPTH (FT)	1-2	SOIL TYPE	1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	84.1%
10	48.5%
20	27.8%
40	17.7%
100	6.6%
200	4.5%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SW
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0

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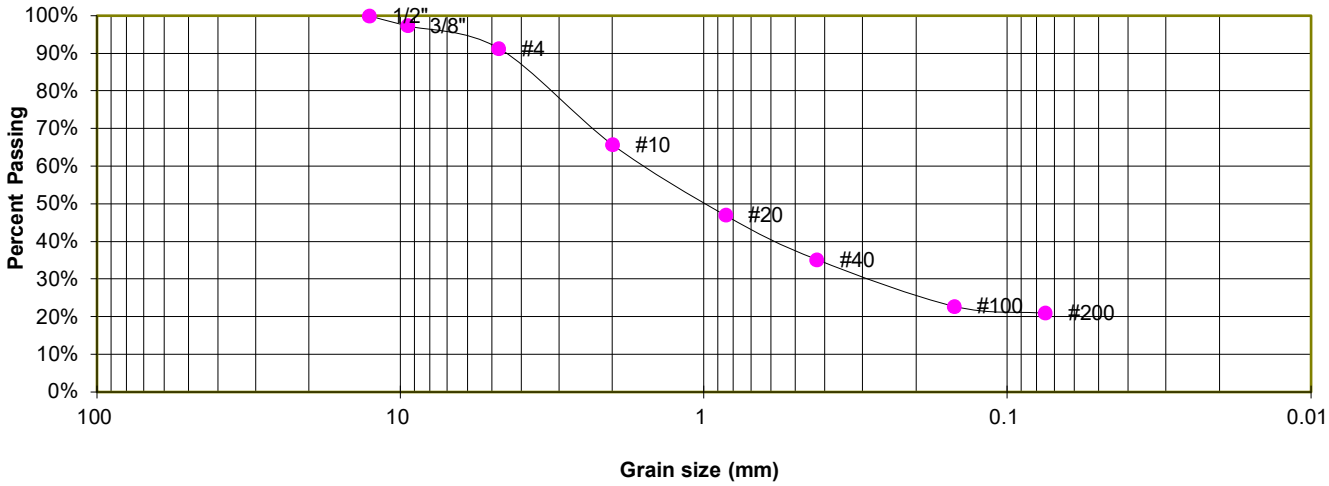
FIG. B-7

TEST BORING	9	SOIL DESCRIPTION	FILL, SAND, CLAYEY
DEPTH (FT)	1-2	SOIL TYPE	1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	97.4%
4	91.4%
10	65.8%
20	47.1%
40	35.2%
100	22.8%
200	21.0%

ATTERBERG LIMITS

Plastic Limit	22
Liquid Limit	33
Plastic Index	11

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-6
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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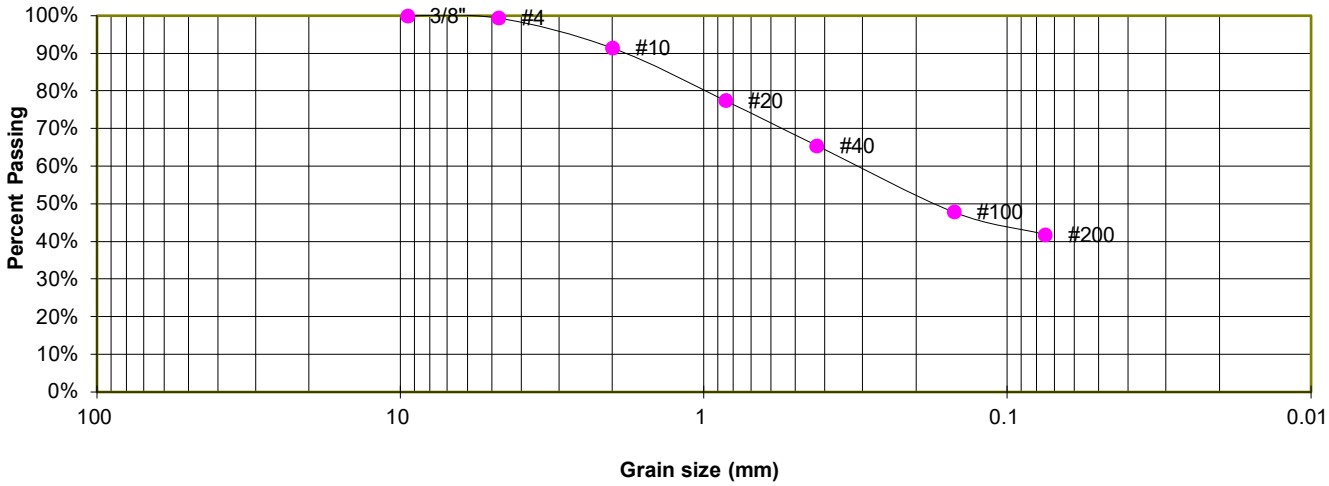
FIG. B-8

TEST BORING	8	SOIL DESCRIPTION	FILL, SAND, CLAYEY
DEPTH (FT)	0-3	SOIL TYPE	2, CBR



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.4%
10	91.4%
20	77.5%
40	65.5%
100	47.9%
200	41.9%

ATTERBERG LIMITS

Plastic Limit	20
Liquid Limit	31
Plastic Index	11

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	1

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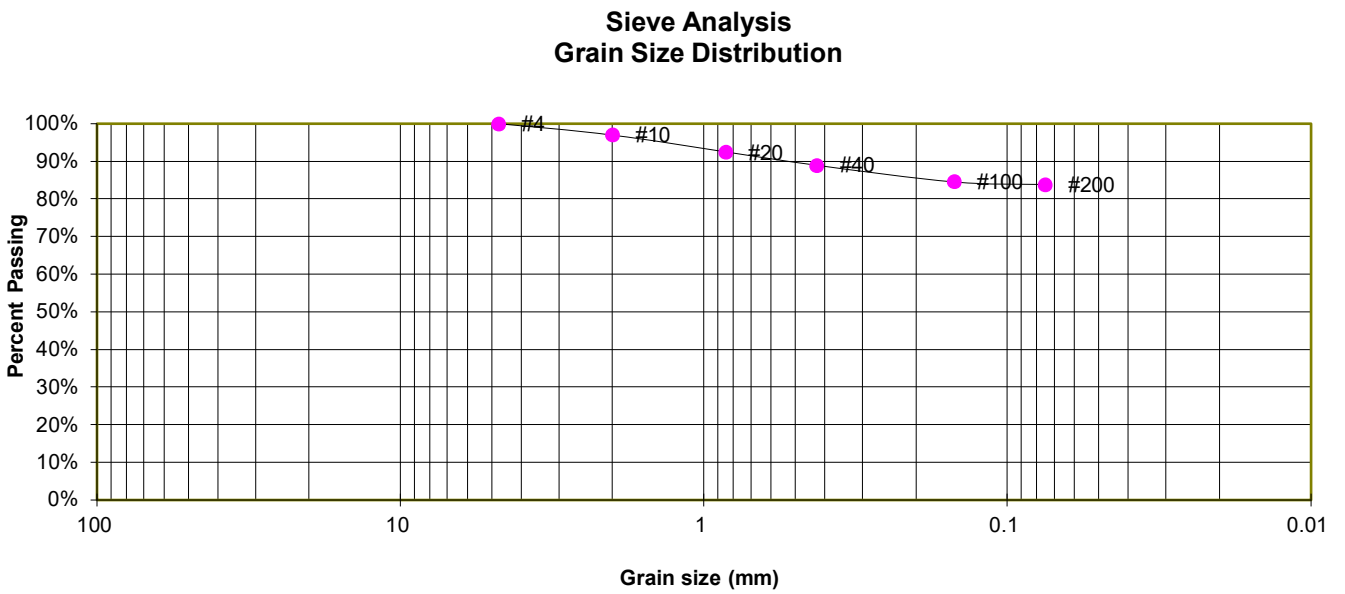
FIG. B-9

TEST BORING	7	SOIL DESCRIPTION	FILL, CLAY, SLIGHTLY SANDY
DEPTH (FT)	1-2	SOIL TYPE	2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	97.1%
20	92.5%
40	89.0%
100	84.6%
200	83.9%

ATTERBERG LIMITS

Plastic Limit	20
Liquid Limit	32
Plastic Index	12

SOIL CLASSIFICATION

USCS CLASSIFICATION:	CL
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	10

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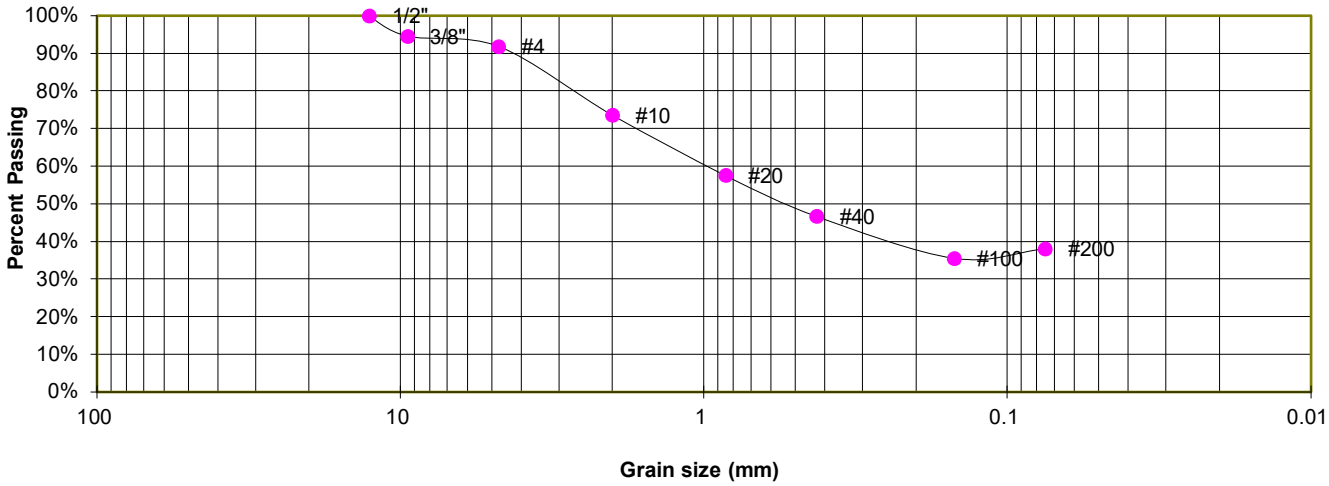
FIG. B-10

TEST BORING	8	SOIL DESCRIPTION	FILL, SAND, CLAYEY
DEPTH (FT)	1-2	SOIL TYPE	2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	94.6%
4	91.8%
10	73.5%
20	57.6%
40	46.7%
100	35.5%
200	38.1%

ATTERBERG LIMITS

Plastic Limit	21
Liquid Limit	29
Plastic Index	8

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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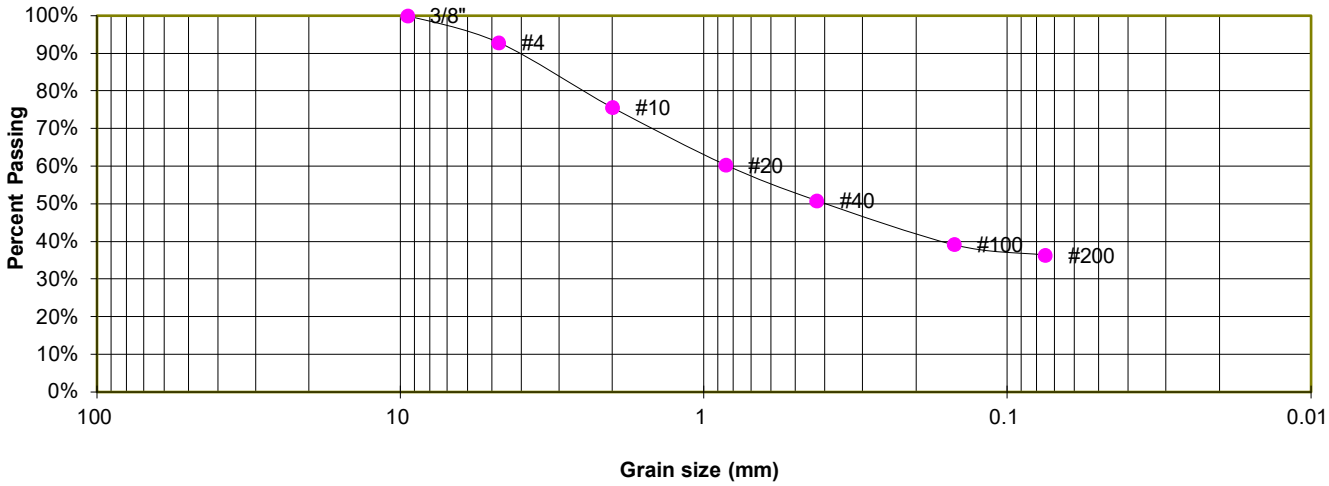
FIG. B-11

TEST BORING	11	SOIL DESCRIPTION	FILL, SAND, CLAYEY
DEPTH (FT)	1-2	SOIL TYPE	2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	92.9%
10	75.6%
20	60.4%
40	50.9%
100	39.3%
200	36.4%

ATTERBERG LIMITS

Plastic Limit	22
Liquid Limit	33
Plastic Index	11

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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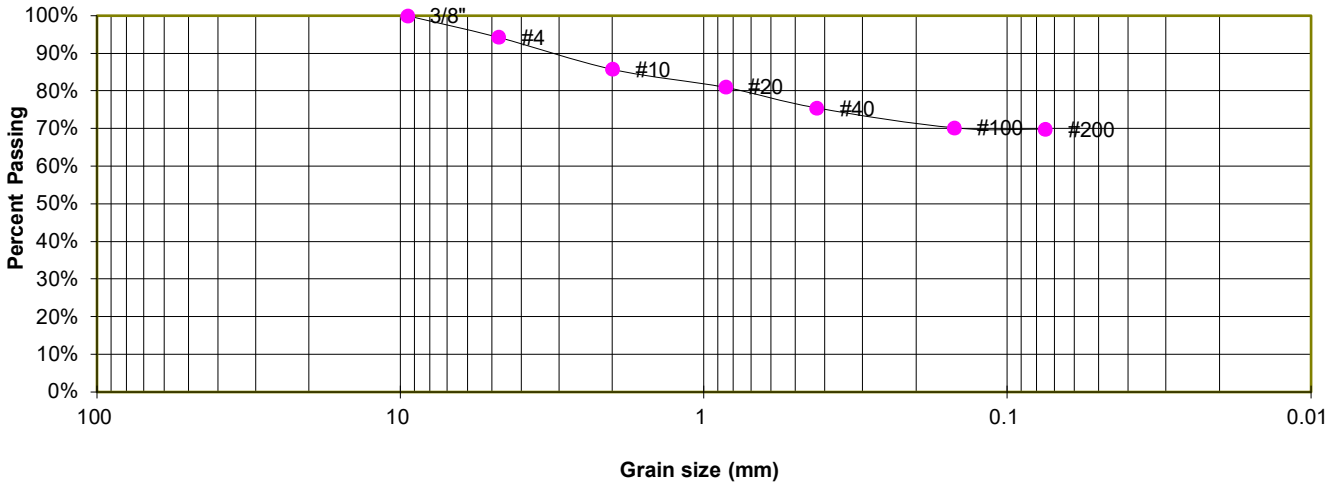
FIG. B-12

TEST BORING	9	SOIL DESCRIPTION	CLAY, SANDY
DEPTH (FT)	10	SOIL TYPE	4



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	94.3%
10	85.8%
20	81.0%
40	75.5%
100	70.2%
200	69.9%

ATTERBERG LIMITS

Plastic Limit	13
Liquid Limit	35
Plastic Index	22

SOIL CLASSIFICATION

USCS CLASSIFICATION:	CL
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	13

ENTECH ENGINEERING, INC.

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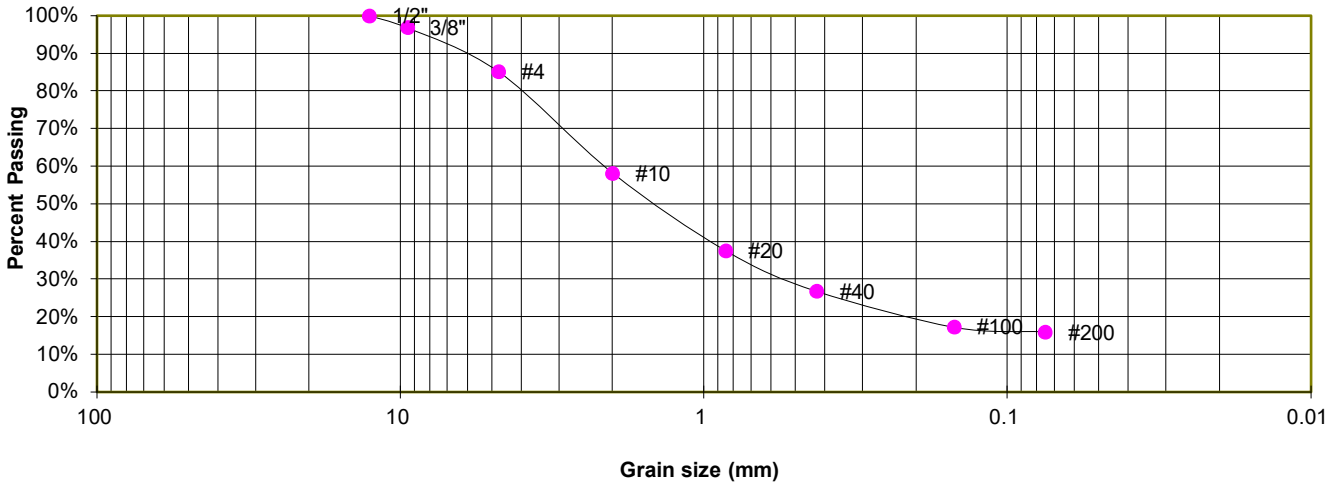
FIG. B-13

TEST BORING	10	SOIL DESCRIPTION	SANDSTONE (SAND, SILTY)
DEPTH (FT)	1-2	SOIL TYPE	5



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	96.8%
4	85.2%
10	58.2%
20	37.6%
40	26.8%
100	17.3%
200	16.0%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0

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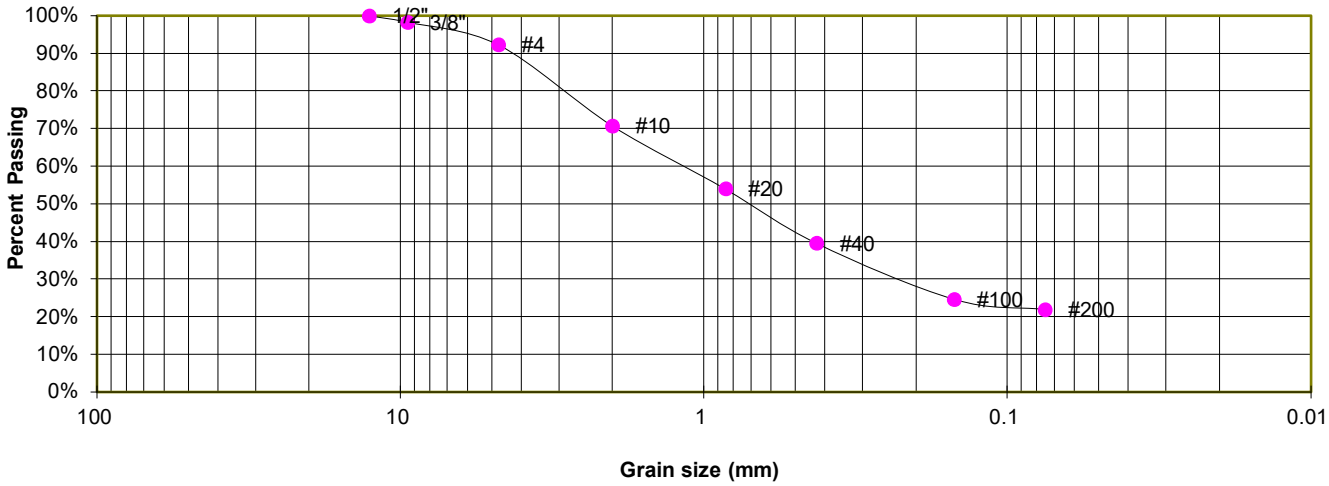
FIG. B-14

TEST BORING	4	SOIL DESCRIPTION	SANDSTONE (SAND, CLAYEY)
DEPTH (FT)	10	SOIL TYPE	5



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.3%
4	92.3%
10	70.7%
20	54.0%
40	39.5%
100	24.7%
200	22.0%

ATTERBERG LIMITS

Plastic Limit	24
Liquid Limit	37
Plastic Index	13

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-6
AASHTO GROUP INDEX:	0

ENTECH ENGINEERING, INC.

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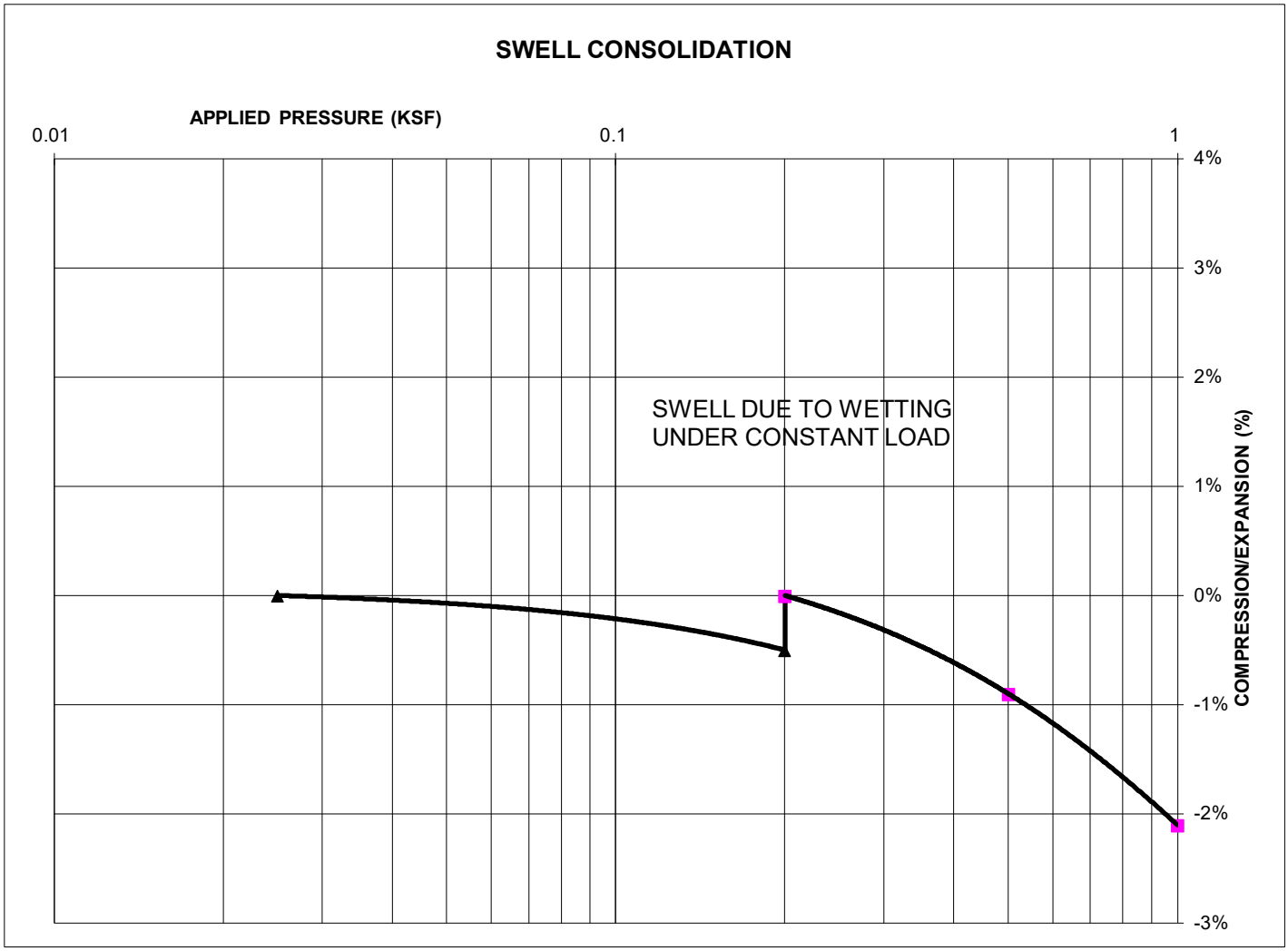
FIG. B-15

TEST BORING	10	SOIL DESCRIPTION	SAND, CLAYEY
DEPTH (FT)	0-3	SOIL TYPE	1, CBR



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	124
NATURAL MOISTURE CONTENT:	8.1%
SWELL/COLLAPSE (%):	0.5%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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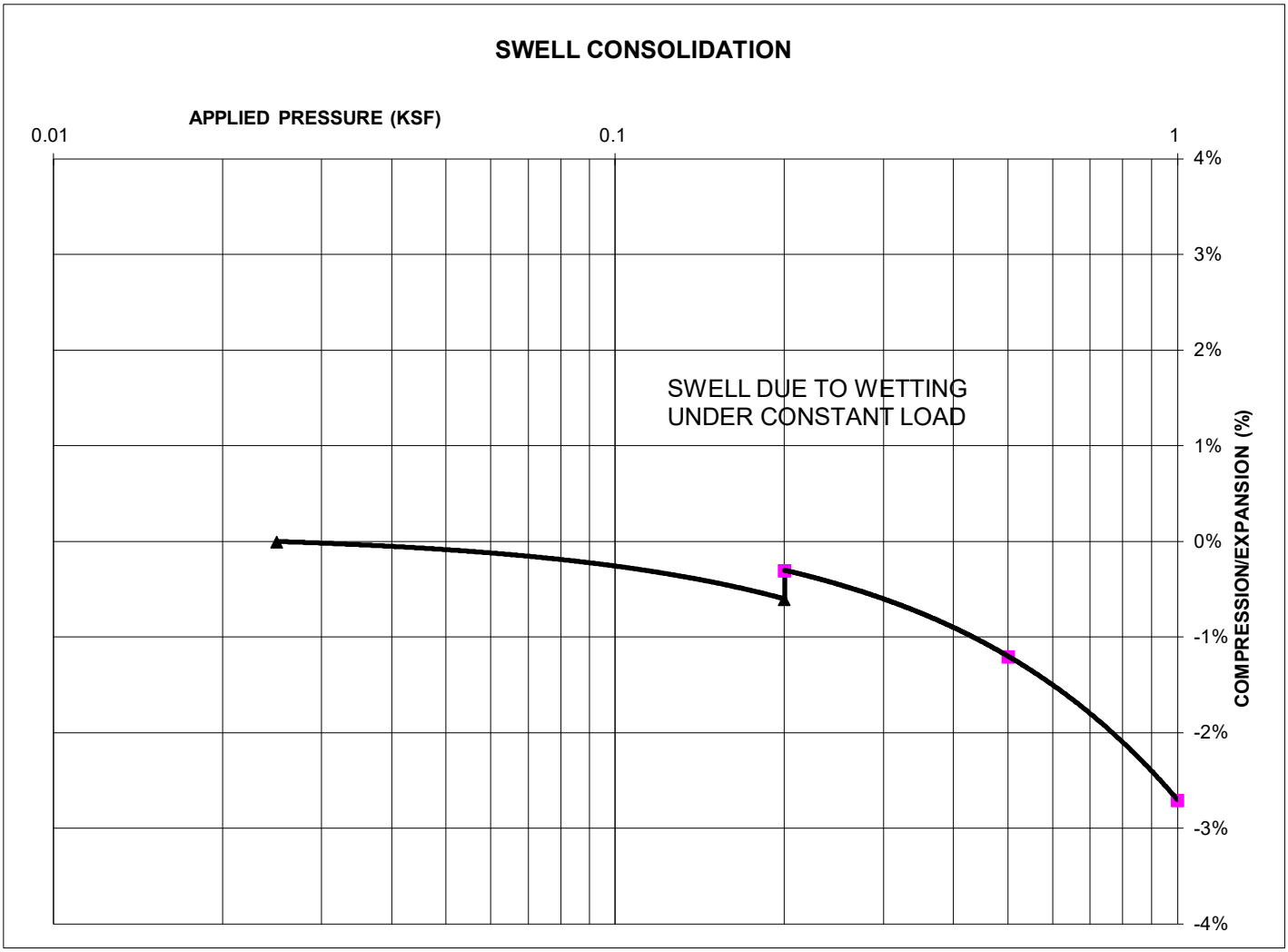
FIG. B-16

TEST BORING	9	SOIL DESCRIPTION	SAND, CLAYEY
DEPTH (FT)	1-2	SOIL TYPE	1



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	116
NATURAL MOISTURE CONTENT:	8.6%
SWELL/COLLAPSE (%):	0.3%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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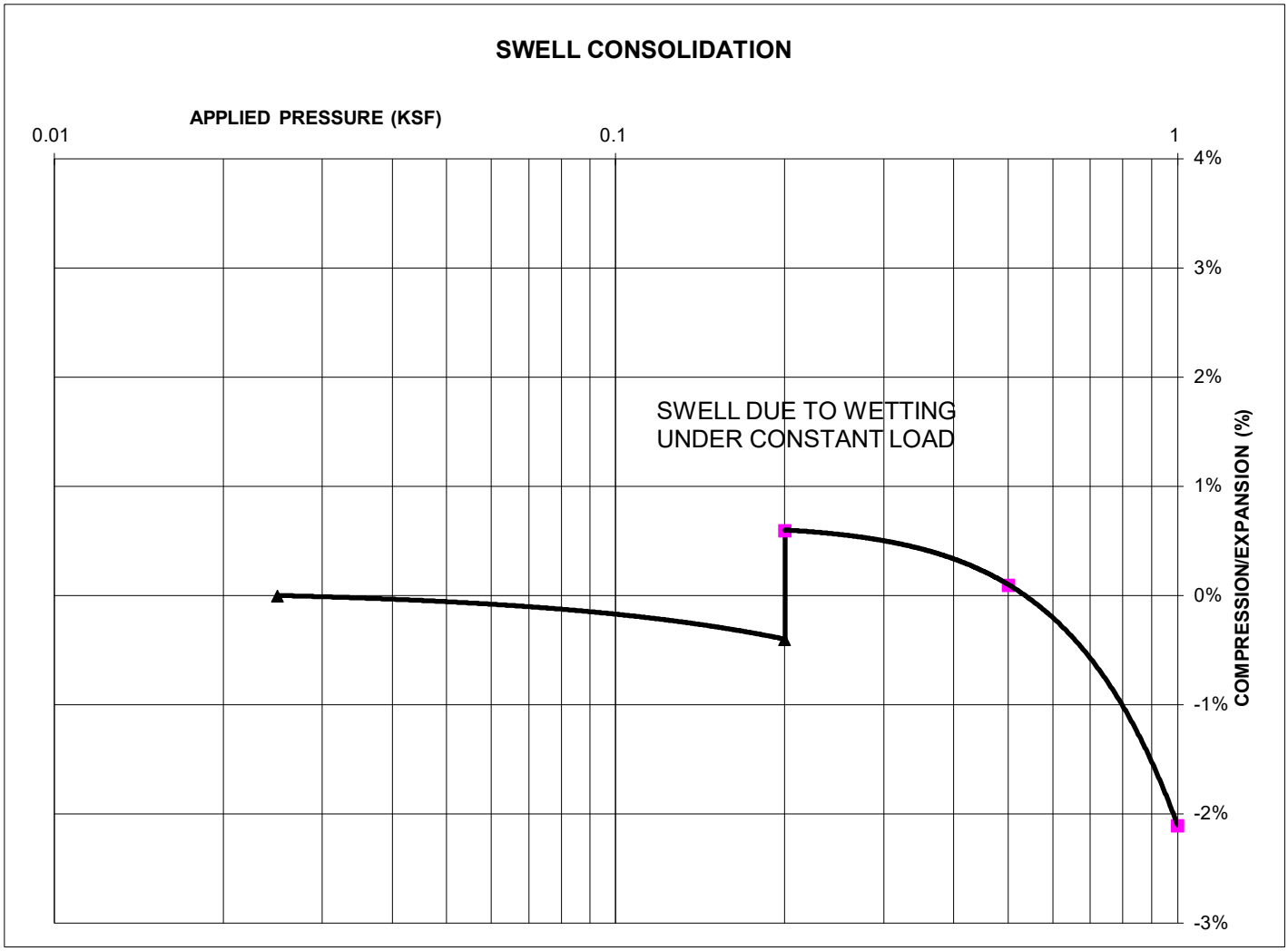
FIG. B-17

TEST BORING	8	SOIL DESCRIPTION	SAND, CLAYEY
DEPTH (FT)	0-3	SOIL TYPE	2, CBR



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	116
NATURAL MOISTURE CONTENT:	9.6%
SWELL/COLLAPSE (%):	1.0%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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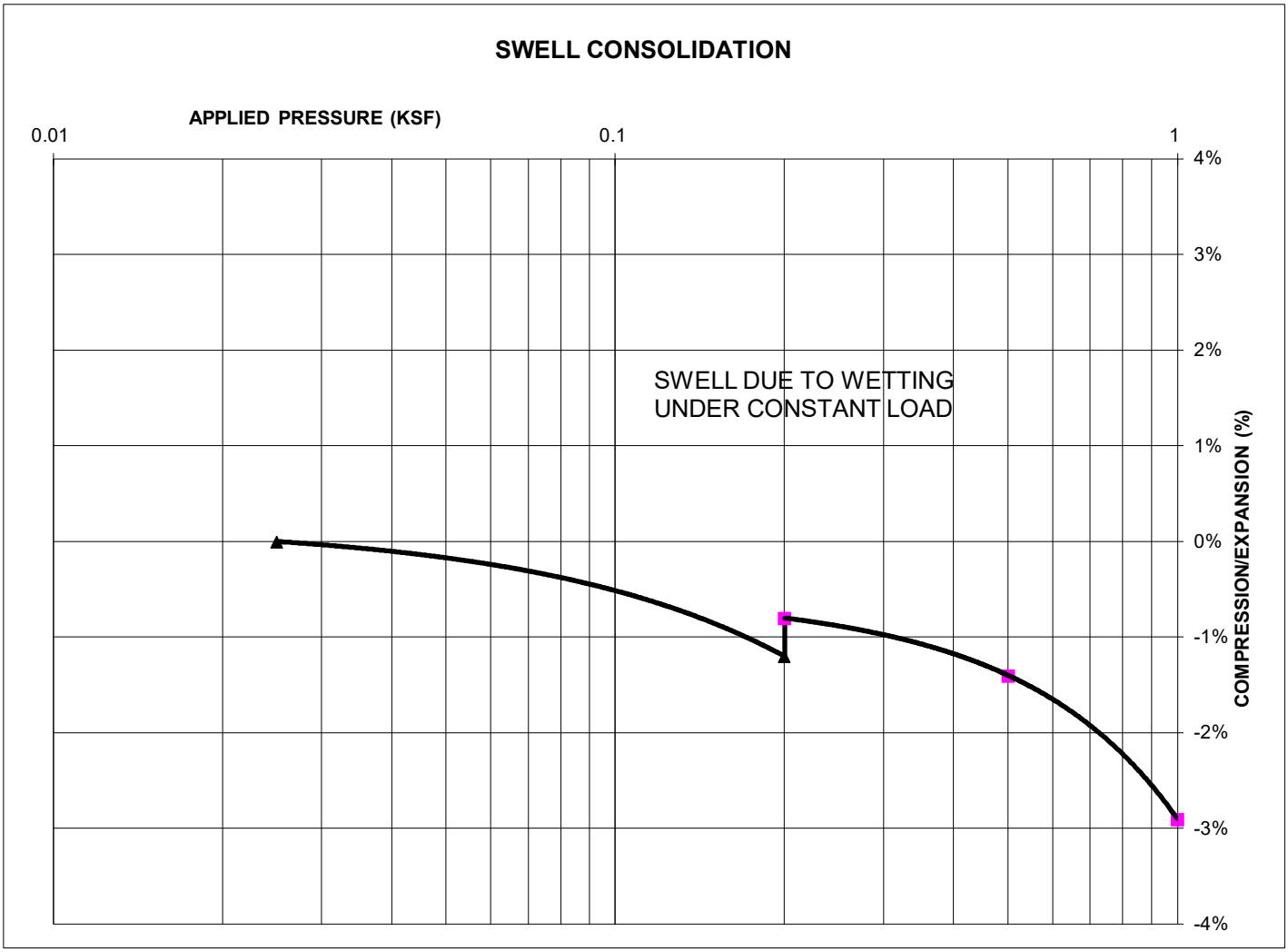
FIG. B-18

TEST BORING	7	SOIL DESCRIPTION	CLAY, SLIGHTLY SANDY
DEPTH (FT)	1-2	SOIL TYPE	2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF): 109
NATURAL MOISTURE CONTENT: 17.7%
SWELL/COLLAPSE (%): 0.4%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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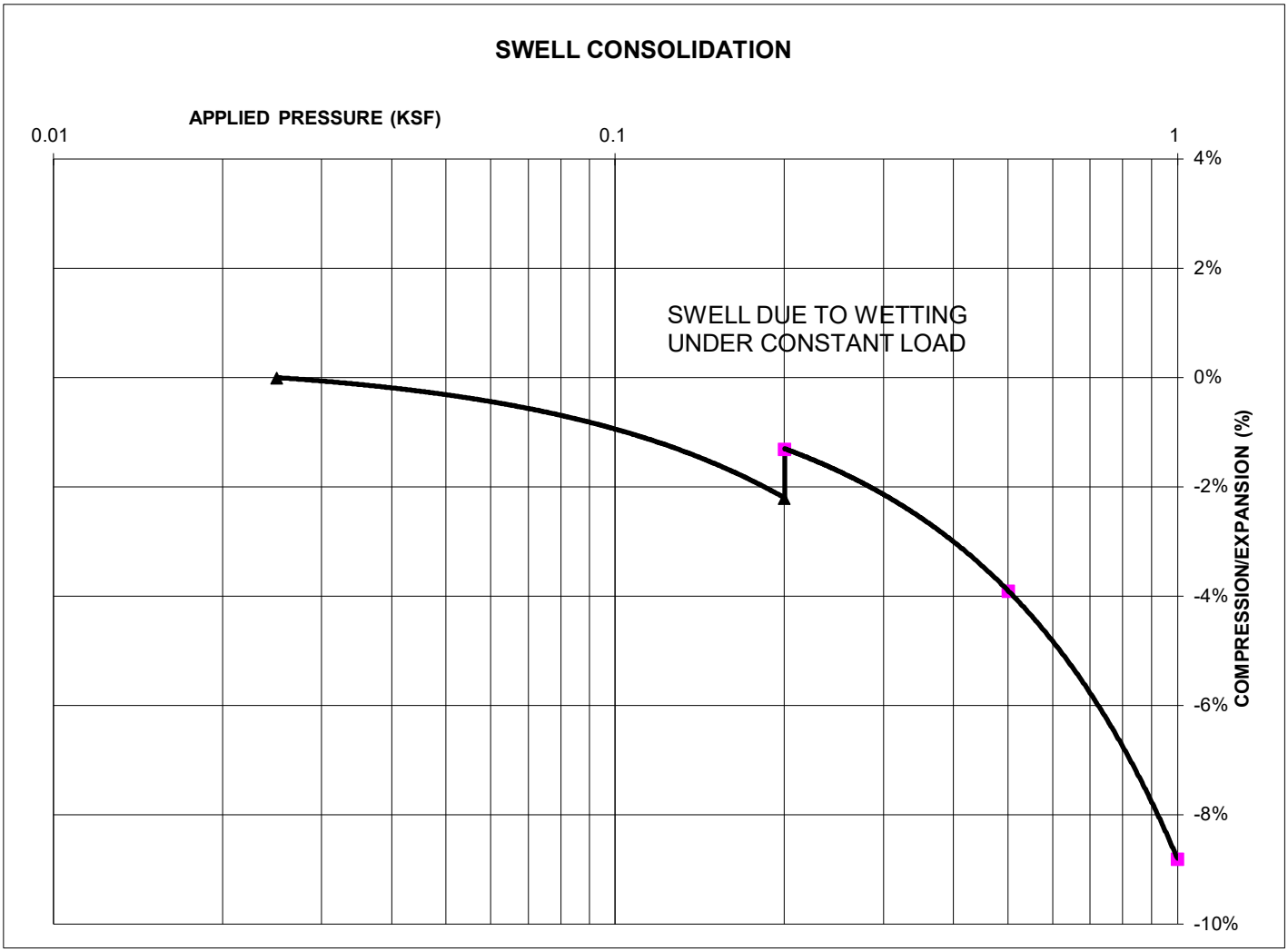
FIG. B-19

TEST BORING	9	SOIL DESCRIPTION	CLAY, SANDY
DEPTH (FT)	10	SOIL TYPE	4



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF): 100
NATURAL MOISTURE CONTENT: 16.9%
SWELL/COLLAPSE (%): 0.9%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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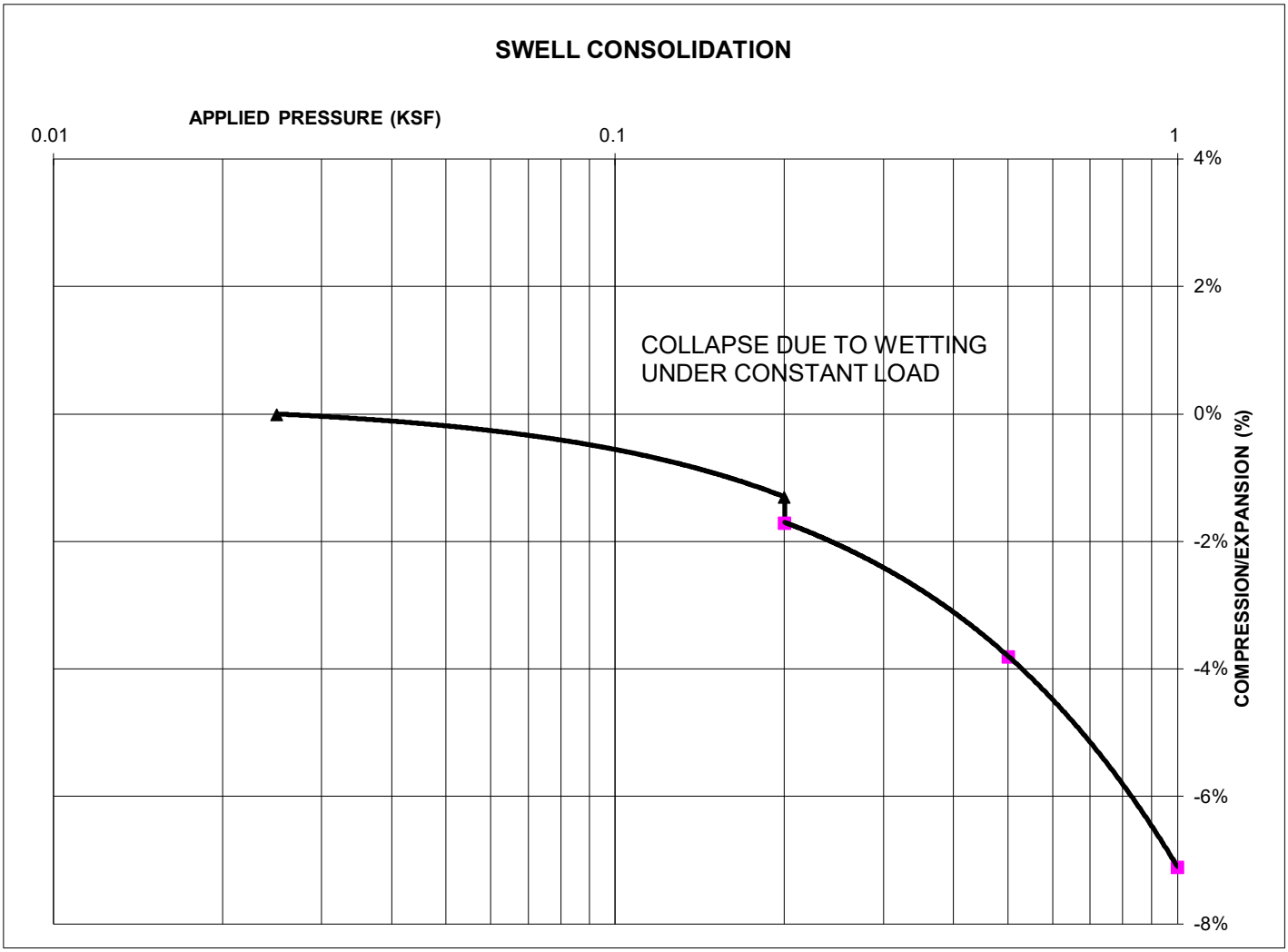
FIG. B-20

TEST BORING	4	SOIL DESCRIPTION	SANDSTONE (SAND, CLAYEY)
DEPTH (FT)	10	SOIL TYPE	5



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF): 115
NATURAL MOISTURE CONTENT: 12.3%
SWELL/COLLAPSE (%): -0.4%

ENTECH ENGINEERING, INC.

SWELL TEST RESULTS

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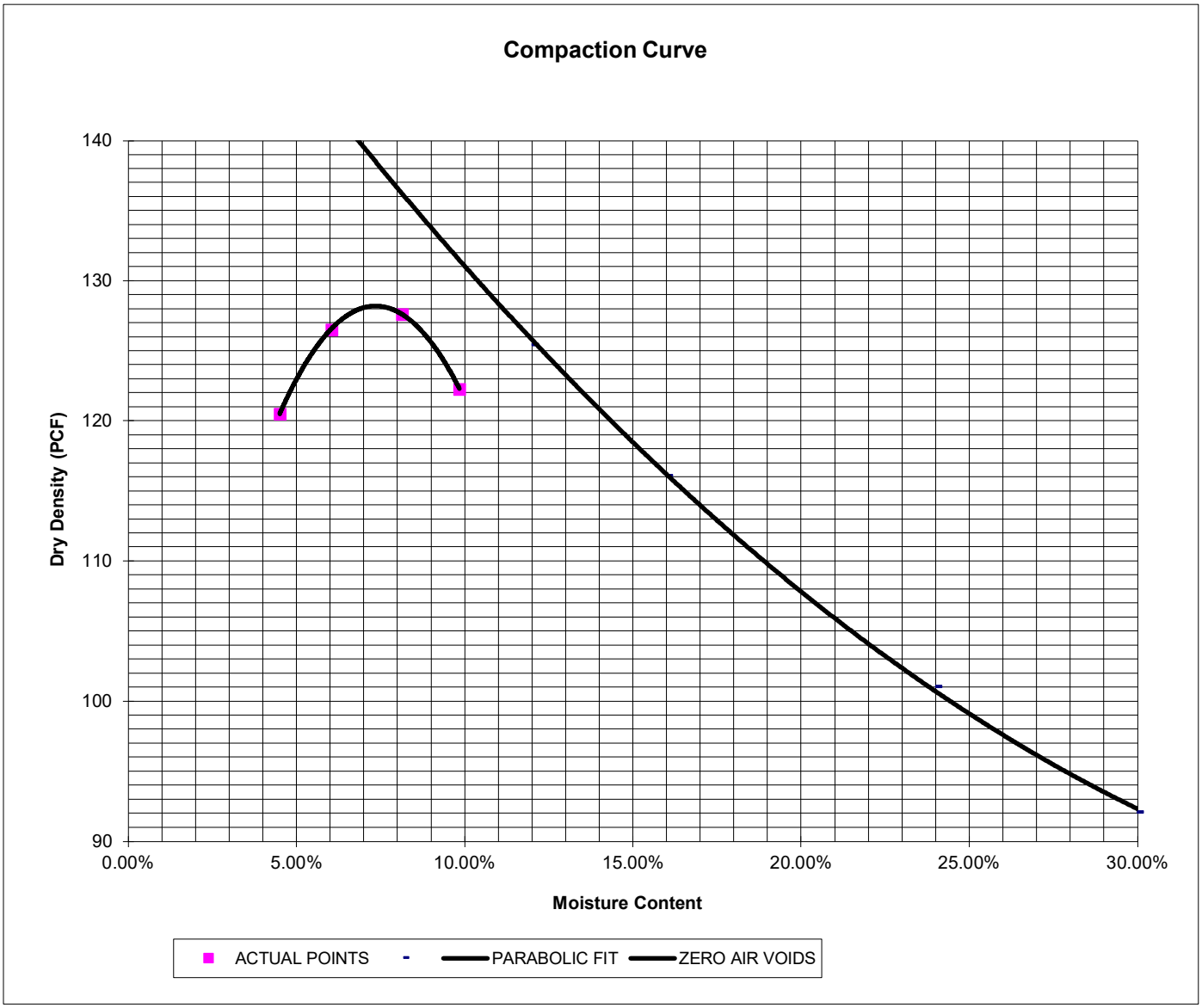
FIG. B-21

SAMPLE LOCATION TB-10 @ 0-3'	SOIL DESCRIPTION SAND, CLAYEY, BROWN
	SOIL TYPE 1

PROCTOR DATA

IDENTIFICATION:	SC
PROCTOR TEST #:	1
TEST BY:	BL
TEST DESIGNATION:	ASTM-1557-A
MAXIMUM DRY DENSITY (PCF):	128.1
OPTIMUM MOISTURE:	7.3



ENTECH ENGINEERING, INC.

LABORATORY TEST RESULTS

STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

FIG. B-22

SAMPLE LOCATION	TB-10 @ 0-3'	SOIL DESCRIPTION	SAND, CLAYEY, BROWN
		SOIL TYPE	1

CBR TEST LOAD DATA

Piston Diameter (cm): 4.958

Piston Area (in^2): 2.993

Penetration Depth (inches)	*10 BLOWS* Mold # 1 Load (lbs)	Stress (psi)	*25 BLOWS* Mold # 2 Load (lbs)	Stress (psi)	*56 BLOWS* Mold # 3 Load (lbs)	Stress (psi)
0.000	0	0.00	0	0.00	0	0.00
0.025	92	30.74	178	59.48	339	113.28
0.050	138	46.12	306	102.26	782	261.32
0.075	157	52.46	375	125.31	1102	368.25
0.100	172	57.48	450	150.38	1850	618.21
0.125	198	66.17	524	175.10	2239	748.20
0.150	218	72.85	662	221.22	2530	845.44
0.175	230	76.86	725	242.27	2825	944.02
0.200	236	78.86	801	267.67	3244	1084.04
0.300	261	87.22	952	318.13	4265	1425.23
0.400	296	98.91	1145	382.62	5045	1685.88
0.500	323	107.94	1265	422.72	5957	1990.64

MOISTURE AND DENSITY DATA

	Mold # 1	Mold # 2	Mold # 3
Can #	303	340	341
Wt. Can	9.28	6.85	6.76
Wt. Can+Wet	114.7	140.51	118.61
Wt. Can+Dry	100.55	124.52	107.25
Wt. H20	14.15	15.99	11.36
Wt. Dry Soil	91.27	117.67	100.49
Moisture Content	15.50%	13.59%	11.30%
Wet Density (PCF)	123.0	128.7	134.1
Dry Density (PCF)	114.6	120.0	125.0
% Compaction	89%	94%	98%
CBR	5.75	15.04	61.82

PROCTOR DATA

Maximum Dry Density (pcf)	128.1
Optimum Moisture	7.3
90% of Max. Dry Density (pcf)	115.3
95% of Max. Dry Density (pcf)	121.7

CBR at 90% of Max. Density = 6.9	~ R VALUE 14
CBR at 95% of Max. Density = 31.1	~ R VALUE 74

ENTECH ENGINEERING, INC.

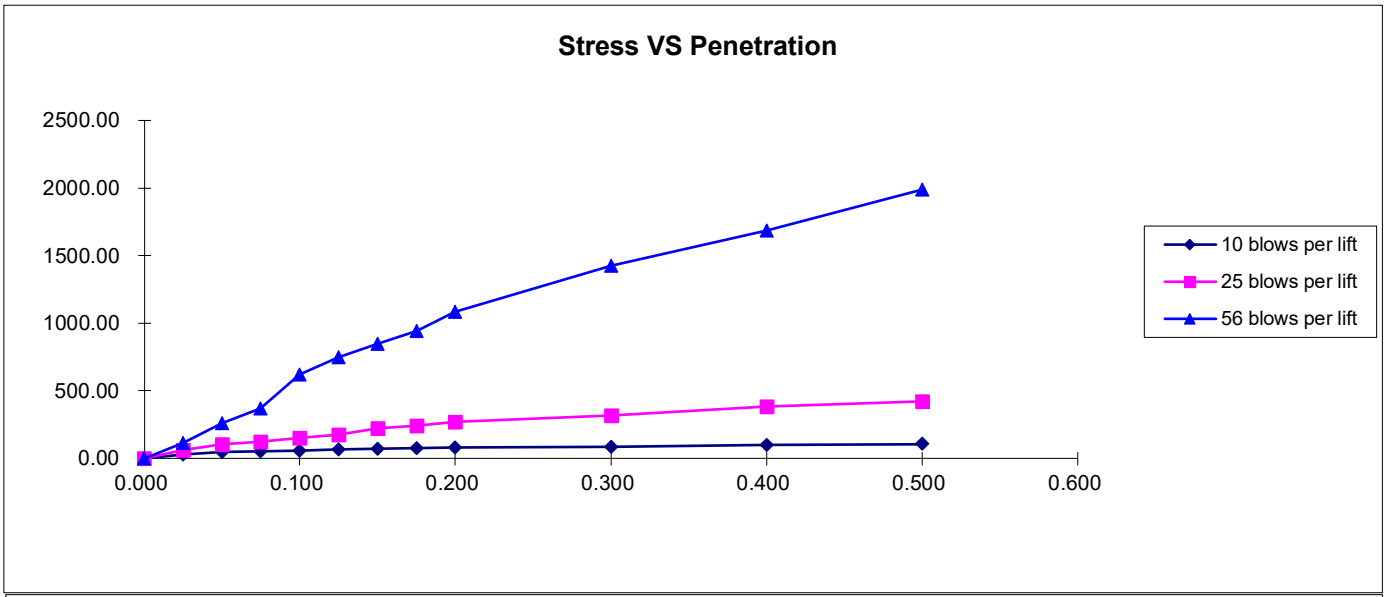
LABORATORY TEST RESULTS

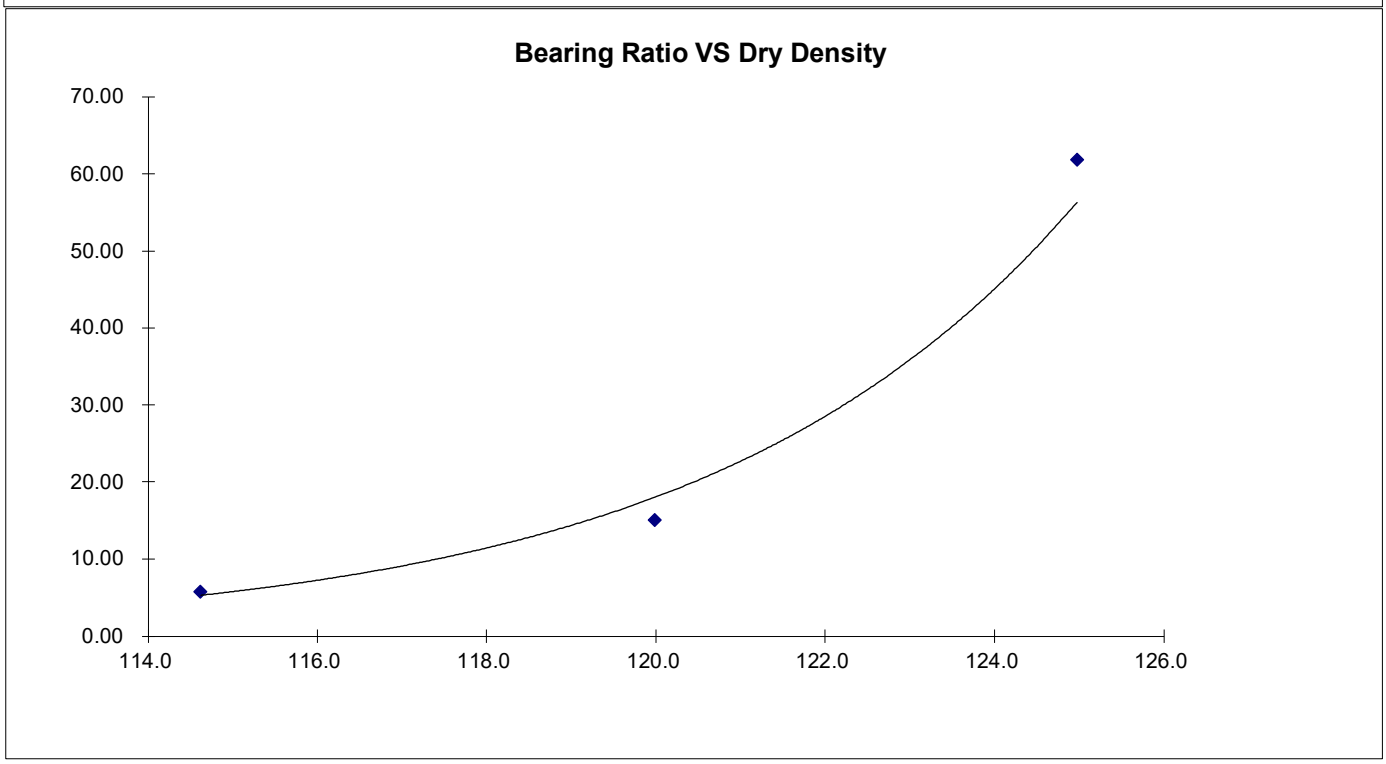
STERLING RANCH, FILING NO. 5
CLASSIC COMMUNITIES

JOB NO. 240368

FIG. B-23

SAMPLE LOCATION TB-10 @ 0-3'	SOIL DESCRIPTION SAND, CLAYEY, BROWN SOIL TYPE 1





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LABORATORY TEST RESULTS

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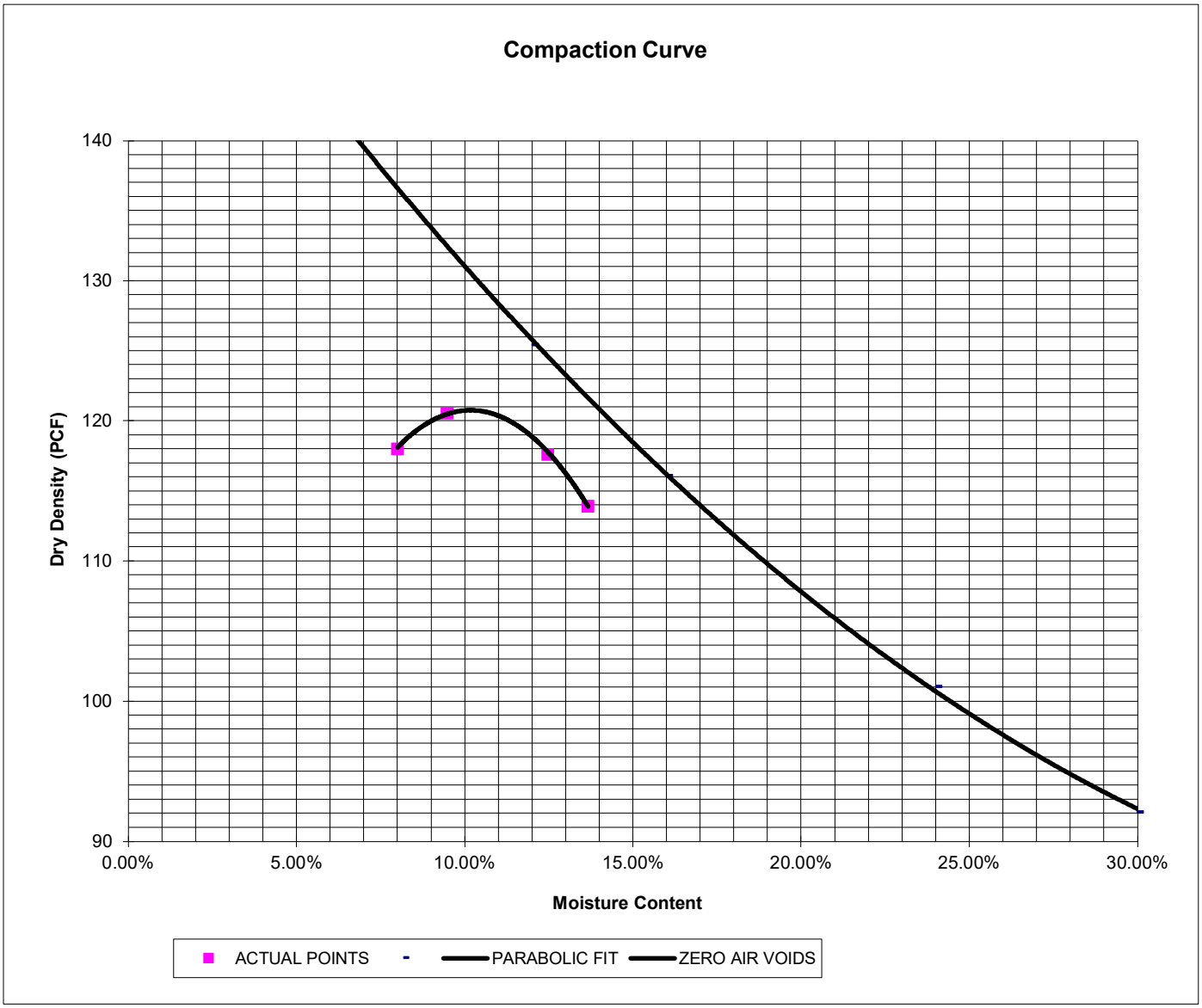
FIG. B-24

SAMPLE LOCATION TB-8 @ 0-3'	SOIL DESCRIPTION SAND, CLAYEY, BROWN
	SOIL TYPE 2

PROCTOR DATA

IDENTIFICATION:	SC
PROCTOR TEST #:	2
TEST BY:	PH
TEST DESIGNATION:	ASTM-698-A
MAXIMUM DRY DENSITY (PCF):	120.9
OPTIMUM MOISTURE:	9.2



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LABORATORY TEST RESULTS

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FIG. B-25

SAMPLE LOCATION	TB-8 @ 0-3'	SOIL DESCRIPTION	SAND, CLAYEY, BROWN
		SOIL TYPE	2

CBR TEST LOAD DATA

Piston Diameter (cm): 4.958

Piston Area (in^2): 2.993

Penetration Depth (inches)	*10 BLOWS* Mold # 1 Load (lbs)	Stress (psi)	*25 BLOWS* Mold # 2 Load (lbs)	Stress (psi)	*56 BLOWS* Mold # 3 Load (lbs)	Stress (psi)
0.000	0	0.00	0	0.00	0	0.00
0.025	41	13.70	109	36.42	148	49.46
0.050	51	17.04	169	56.47	236	78.86
0.075	60	20.05	196	65.50	292	97.58
0.100	81	27.07	212	70.84	339	113.28
0.125	93	31.08	241	80.53	410	137.01
0.150	101	33.75	262	87.55	478	159.73
0.175	114	38.10	281	93.90	535	178.78
0.200	123	41.10	296	98.91	588	196.49
0.300	133	44.44	352	117.63	820	274.02
0.400	145	48.45	402	134.34	955	319.13
0.500	165	55.14	462	154.39	1125	375.94

MOISTURE AND DENSITY DATA

	Mold # 1	Mold # 2	Mold # 3
Can #	369	420	352
Wt. Can	8.7	8.32	8.09
Wt. Can+Wet	197.1	152.42	136.19
Wt. Can+Dry	170.94	131.47	118.93
Wt. H20	26.16	20.95	17.26
Wt. Dry Soil	162.24	123.15	110.84
Moisture Content	16.12%	17.01%	15.57%
Wet Density (PCF)	120.1	123.1	129.1
Dry Density (PCF)	110.0	112.7	118.3
% Compaction	91%	93%	98%
CBR	2.71	7.08	11.33

PROCTOR DATA

Maximum Dry Density (pcf)	120.9
Optimum Moisture	9.2
90% of Max. Dry Density (pcf)	108.8
95% of Max. Dry Density (pcf)	114.9

CBR at 90% of Max. Density =	0.7	~ R VALUE	1
CBR at 95% of Max. Density =	8.7	~ R VALUE	22

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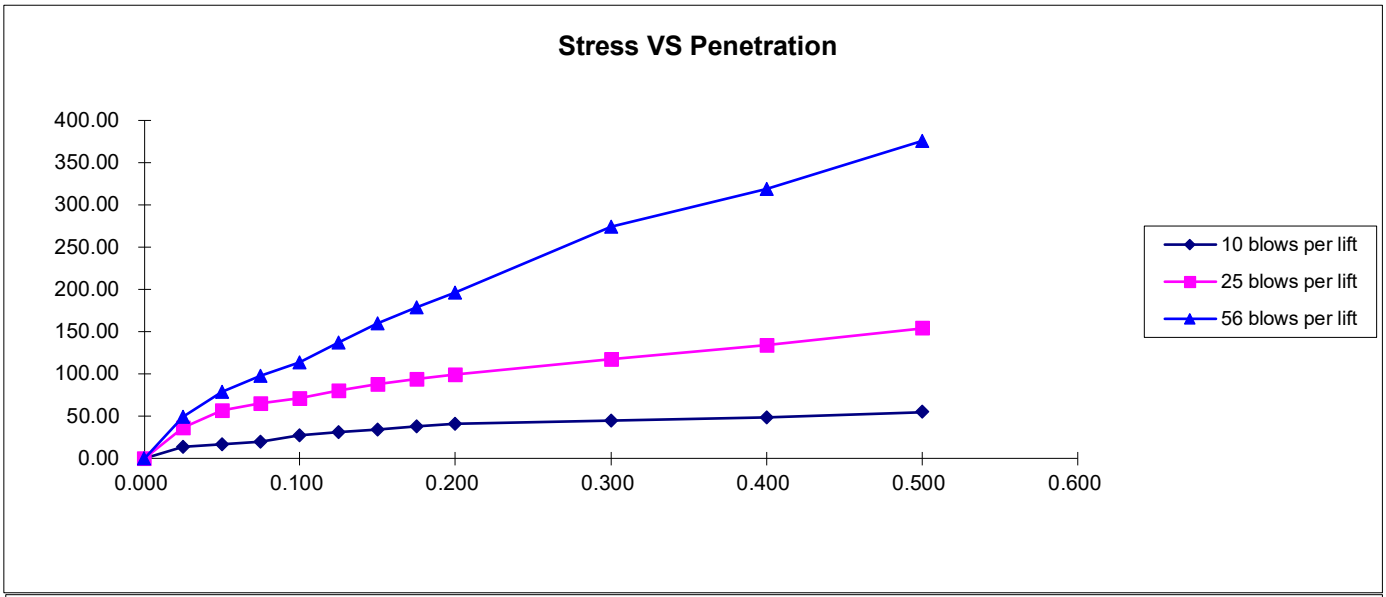
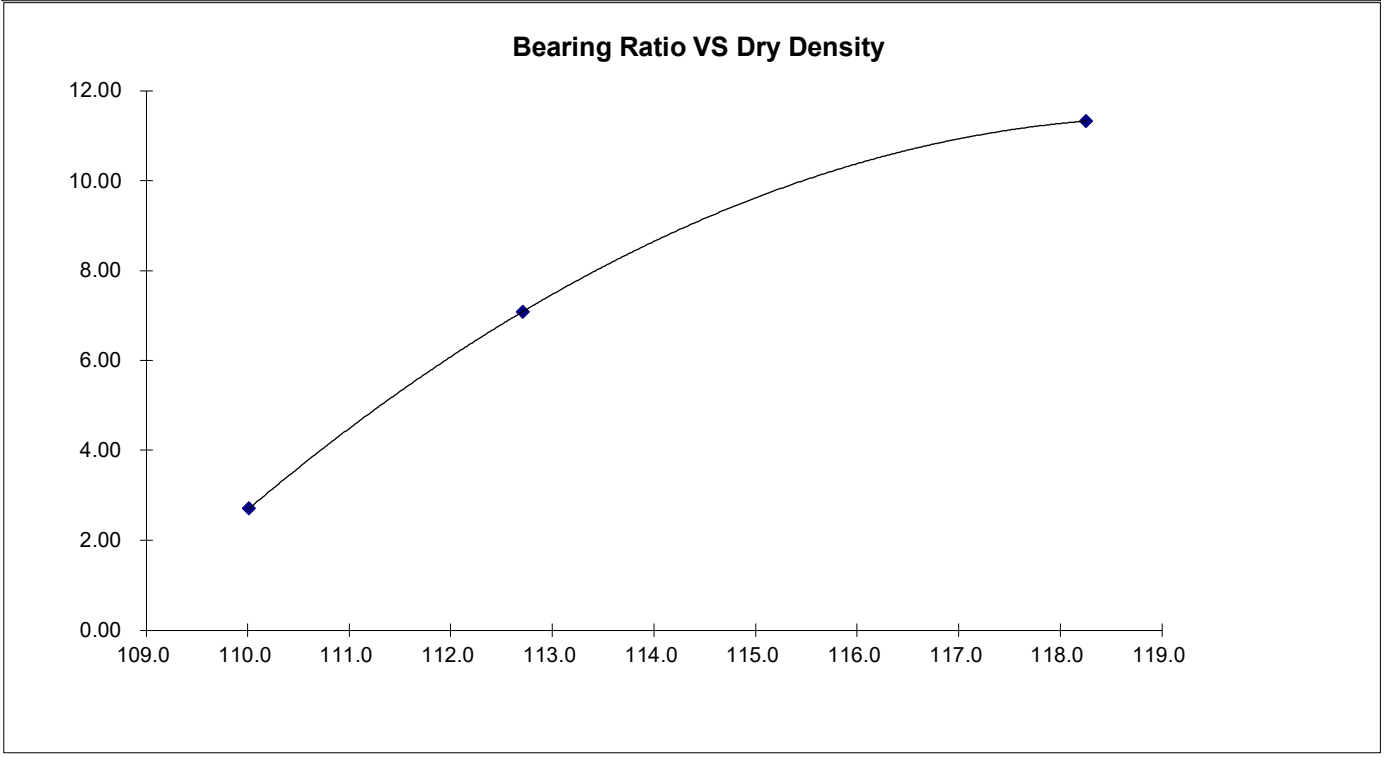
FIG. B-26

SAMPLE LOCATION TB-8 @ 0-3'	SOIL DESCRIPTION SAND, CLAYEY, BROWN SOIL TYPE 2





ENTECH ENGINEERING, INC.

LABORATORY TEST RESULTS

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CLASSIC COMMUNITIES

JOB NO. 240368

FIG. B-27

APPENDIX C: Pavement Design Calculations

FLEXIBLE PAVEMENT DESIGN

PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5

Job Number: 240368

DESIGN DATA

Equivalent (18-kip) Single Axle Load Applications (ESAL):	ESAL (W_{18}) =	292,000
Design CBR	CBR =	8.7
Standard Deviation	S_o =	0.45
Loss in Serviceability	Δpsi =	2.5
Reliability	Reliability =	80
Reliability (z-statistic)	Z_R =	-0.84
Soil Resilient Modulus	M_R =	13,050 psi

Required Structural Number (SN): → SN = 2.08

DESIGN EQUATIONS

Resilient Modulus

If using CBR: M_R = (CBR) x 1,500

If using R-Value: $M_R = 10^{[(S_1 + 18.72) / 6.24]}$ where S_1 = [(R-value - 5) / 11.29] + 3

Required Structural Number

$$\log_{10}W_{18} = Z_R * S_O + 9.36*\log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left[\frac{\Delta PSI}{4.2 - 1.5}\right]}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32*\log_{10}M_R - 8.07$$

Pavement Section Thickness

$SN^* = C_1D_1 + C_2D_2$ where:

C_1 = Strength Coefficient - HMA

C_2 = Strength Coefficient - ABC/RCB

D_1 = Depth of HMA (inches)

D_2 = Depth of ABC/RCB (inches)

RECOMMENED THICKNESSES

Layer	Material	Structural Layer	Thickness (D^*_i)	SN^*_i	SN
1	HMA	C_1 = 0.44	3.0 inches	1.320	-
2	ABC/RCB	C_2 = 0.11	8.0 inches	0.880	
			SN* =	2.200	2.08

Pavement SN > Required SN, Design is Acceptable

FIG. C-1

FLEXIBLE PAVEMENT DESIGN

PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5

Job Number: 240368

DESIGN DATA

Equivalent (18-kip) Single Axle Load Applications (ESAL):	ESAL (W_{18}) =	292,000
Design CBR	CBR =	8.7
Standard Deviation	S_o =	0.45
Loss in Serviceability	Δpsi =	2.5
Reliability	Reliability =	80
Reliability (z-statistic)	Z_R =	-0.84
Soil Resilient Modulus	M_R =	13,050 psi

Required Structural Number (SN): → SN = 2.08

DESIGN EQUATIONS

Resilient Modulus

If using CBR: $M_R = (CBR) \times 1{,}500$

If using R-Value: $M_R = 10^{[(S_1 + 18.72) / 6.24]}$ where $S_1 = [(R\text{-value} - 5) / 11.29] + 3$

Required Structural Number

$$\log_{10}W_{18} = Z_R * S_O + 9.36*\log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left[\frac{\Delta PSI}{4.2 - 1.5}\right]}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32*\log_{10}M_R - 8.07$$

Pavement Section Thickness

$SN^* = C_1D_1 + C_2D_2$ where:

C_1 = Strength Coefficient - HMA

C_2 = Strength Coefficient - CTS

D_1 = Depth of HMA (inches)

D_2 = Depth of CTS (inches)

RECOMMENED THICKNESSES

Layer	Material	Structural Layer	Thickness (D^*_i)	SN^*_i	SN
1	HMA	C_1 = 0.44	3.0 inches	1.320	-
2	CTS	C_2 = 0.11	8.0 inches	0.880	
			SN* =	2.200	2.08

Pavement SN > Required SN, Design is Acceptable

FIG. C-2