# Homestead at Sterling Ranch Filing 2 <br> Traffic Technical Memorandum 

(LSC \#184283)
PCD File No.: SF-19-004
March 3, 2020

ACCEPTED for FILE
Engineering Review 10/21/2020 12:20:59 PM dsdnijkamp
EPC Planning \& Community Development Department

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


Developer's Statement
April 22, 202

I, the Developer, have read and will comply with all commitments made on my behalf within this report.


March 3, 2020

Mr. Jim Morley
Morley-Bentley Investments, LLC
20 Boulder Crescent, 1st Floor
Colorado Springs, CO 80903

RE: Homestead at Sterling Ranch Filing 2<br>El Paso County, CO<br>Transportation Memorandum<br>LSC \#184283

Dear Mr. Morley:

LSC Transportation Consultants, Inc. has prepared this updated Transportation Memorandum for Homestead at Sterling Ranch Filing 2. As shown on Figure 1, Sterling Ranch is located east of Vollmer Road near Lochwinnoch Lane between the future extensions of Marksheffel Road and Stapleton Drive in El Paso County, Colorado. This report is intended as a site-specific, final plat traffic report for the currently proposed filing.

## REPORT CONTENTS

This report presents:

- A list of previous Sterling Ranch traffic reports and the context of this project
- A summary of the proposed land use and access plan
- The existing roadway and traffic conditions in the site's vicinity including the roadway widths, surface conditions, lane geometries, traffic controls, and posted speed limits
- Existing (2017) traffic volume data
- Estimates of projected short-term and intermediate-term traffic volumes
- The projected average weekday and peak-hour vehicle-trips to be generated by the proposed development
- The assignment of the projected site-generated traffic volumes to the area roadways
- The projected short-term total traffic volumes on the area roadways
- The projected levels of service at the key intersections in the vicinity of the site
- The recommended street classifications for the internal streets within the proposed development
- The project's obligation to the County roadway improvement fee program


## PREVIOUS STERLING RANCH TRAFFIC REPORTS AND MEMORANDUM

LSC prepared a traffic impact study (TIS) for the entire Sterling Ranch development dated June 5, 2008. LSC also prepared a traffic impact analysis for the first phase of the Sterling Ranch development dated March 16, 2015; a memorandum for Phases 1-3 dated October 2, 2017; and a traffic impact analysis for the Sterling Ranch Phase 2 Preliminary Plan dated December 20, 2018. The following site-specific, final plat traffic reports have also been prepared:

- Branding Iron at Sterling Ranch Filing No. 1 and Homestead at Sterling Ranch Filing No. 1 dated December 19, 2017
- Sterling Ranch Filing No. 2 dated April 3, 2018
- Copper Chase at Sterling Ranch dated December 20, 2018 Homestead at Sterling Ranch Filing No. 2 dated March 1, 2019


## LAND USE AND ACCESS

## Land Use

Figure 2 shows the location of the Sterling Ranch developments in the vicinity of the site that are either approved or currently under review. Branding Iron at Sterling Ranch Filing No. 1 and Homestead at Sterling Ranch Filing No. 1 have both been approved but no homes have been constructed in either filing. Applications to plat both Branding Iron at Sterling Ranch Filing No. 2 and Homestead at Sterling Ranch Filing No. 2 have been submitted and are currently in the review process. It is our understanding that Sterling Ranch Filing No. 2, Copper Chase at Sterling Ranch and Sterling Ranch Phase 2 are all currently on hold, however, for the purposes of this report these developments were assumed to occur in the intermediate-term future.

This site-specific, final traffic report is for The Homestead at Sterling Ranch Filing 2. The currently proposed filing is planned to include 104 lots for single-family homes. Four full-movement access points are proposed to Dines Boulevard and Wheatland Drive. The site plan is shown in Figure 3.

## Access

Figure 4 shows the proposed short-term street connection plan. As shown on Figure 4 Dines Boulevard is planned to be constructed south from Vollmer Road to the future Sterling Ranch Road. A short, half section of Briargate Parkway is planned to be constructed between Vollmer Road and Wheatland Drive and Wheatland Drive is planned to be constructed south from Briargate Parkway adjacent to and through the Homestead at Sterling Ranch Filings 1 and 2 to Dines Boulevard.. The section of Sterling Ranch Road between Dines Boulevard and Marksheffel Road and the section of Marksheffel Road between Vollmer Road and Sterling Ranch Road are planned to be constructed with Sterling Ranch Filing 2, currently being processed through the

County. An emergency access road is constructed southwest from the terminus of Dines Boulevard to Vollmer Road.

Figure 4 also shows the street connection plan assumed for the purposes of this report. As shown in Figure 4 it was assumed that Marksheffel Road would be constructed from its current terminus just north of Woodmen Road to Vollmer Road and Sterling Ranch Road would be constructed from Marksheffel Road to Dines Boulevard in the intermediate-term future. If areas of Sterling Ranch other than those identified on Figure 2 as "intermediate-term" are developed prior to this occurring or if other intermediate-term street connections are constructed this report may need to be revised.

## ROADWAY AND TRAFFIC CONDITIONS

The roadways in the site's vicinity are shown on Figure 1 and are described below.

Vollmer Road is currently a five-lane urban street within the City of Colorado Springs limits between Black Forest Road and Cowpoke Road; and a two-lane, rural, paved roadway north of Cowpoke Road extending to north of Hodgen Road. In the southbound direction, Vollmer Road has a posted speed limit of 45 miles per hour (mph). South of Cowpoke Road, Vollmer Road has a $40-\mathrm{mph}$ posted speed limit. The 2040 El Paso County Major Transportation Corridors Plan (MTCP) and the Sterling Ranch master traffic study show Vollmer Road as a four-lane Urban Minor Arterial in the vicinity of the site. In the interim, auxiliary turn lanes will be completed on Vollmer Road as shown in the memos by LSC dated October 2, 2017 and February 2, 2019.

Marksheffel Road is a Principal Arterial extending north from the City of Fountain to Woodmen Road. Marksheffel Road is planned to ultimately be widened to six lanes and extended north and west from Woodmen Road to connect to Research Parkway at Black Forest Road. Marksheffel Road is shown as a six-lane Principal Arterial through the site on the EI Paso County MTCP.

Briargate Parkway is a six-lane, Principal Arterial that extends east from I-25 to Grand Lawn Circle (about one-half mile east of Powers Boulevard). Briargate Parkway is planned to ultimately extend to Towner Drive. With the Sterling Ranch Phase 1 development, Stapleton Road is planned to be constructed as a two-lane roadway between Vollmer Road and the proposed first site access intersection 750 feet east of Vollmer (Wheatland Drive). For this report of short-term conditions, it was assumed that only this section of Briargate Parkway would be constructed in the vicinity of the site.

Sterling Ranch Road is a planned Non-Residential Collector shown extending through the Sterling Ranch development between Marksheffel Road and Briargate Parkway.

## EXISTING (2017) TRAFFIC VOLUMES

Figure 5 shows the existing (2017) daily and peak-hour traffic volumes on Vollmer Road in the vicinity of the site. The traffic volumes are from the attached traffic counts conducted adjacent to the site in September 2017 and June 2018.

## BACKGROUND TRAFFIC

Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation of site-generated traffic volumes. Background traffic includes the through traffic and the traffic generated by nearby developments, but assumes zero traffic generated by the site.

Figure 6 shows the projected short-term background traffic volumes. The short-term background volumes assume only the short-term street connections shown in Figure 4. The short-term background traffic includes the existing traffic volumes (from Figure 5) with increases in through traffic due to regional growth plus traffic estimated to be generated by buildout of the Homestead at Sterling Ranch Filings 1, Branding Iron at Sterling Ranch Filings 1 and 2, and the proposed Retreat at Timber Ridge development to be located generally northeast of the intersection of Vollmer Road and Poco Road.

Figure 7 shows the projected intermediate term background traffic volumes. These volumes assume Marksheffel Road has been extended northwest from Woodmen Road to Vollmer Road, Sterling Ranch Road has been constructed northeast from Marksheffel Road to Dines Boulevard, and Dines Boulevard has been completed between Sterling Ranch Road and Vollmer Road. The intermediate traffic volumes are based on the short-term background traffic volumes shown in Figure 6 with some changes in traffic patterns due to the new street connections plus traffic estimated to be generated by buildout of Sterling Ranch Filing 2, Copper Chase at Sterling Ranch and the residential portion of Sterling Ranch Phase 2.

## TRIP GENERATION

The site-generated vehicle-trips were estimated using the nationally-published trip generation rates from Trip Generation, 10th Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 1 shows the current trip generation estimate.

As shown in Table 1, Homestead at Sterling Ranch is projected to generate about 982 new vehicle-trips on the average weekday, with about one-half of the vehicles entering and one-half of the vehicles exiting in a 24 -hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 19 vehicles would enter and 58 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between 4:30 and 6:30 p.m., about 65 vehicles would enter and 38 vehicles would exit the site.

## SHORT-TERM DIRECTIONAL DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the street and roadway system serving the site is one of the most important factors in determining the site's traffic impacts. The specific short-term and intermediate-term distribution estimates are shown in Figure 8. The directional distribution estimates are based on the following factors: the location of the site with respect to the Colorado Springs metropolitan area, the planned access system for the site, the street and roadway system serving the site, and the land uses proposed for the site.

The short-term distribution estimate shown in Figure 8 assumes:

- Dines Boulevard has been constructed south of Vollmer Road to the southern-most access for the Branding Iron at Sterling Ranch Filing 1 (Kintla Court) only.
- No sections of Sterling Ranch Road have been constructed.
- Only the short section of Briargate Parkway between Vollmer Road and Wheatland Drive constructed in the vicinity of the site.
- The section Marksheffel Road east of Vollmer Road has not been constructed.

The intermediate-term distribution estimate shown in Figure 8 assumes:

- Marksheffel Road has been constructed between Woodmen Road and Vollmer Road, but not west of Vollmer Road.
- Sterling Ranch Road has been constructed between Marksheffel Road and Dines Boulevard, but not north of Dines Boulevard.
- The completion of Dines Boulevard between Sterling Ranch Road and Vollmer Road.
- Only the short section of Briargate Parkway between Vollmer Road and Wheatland Drive has been constructed in the vicinity of the site.

When the distribution percentages (from Figure 8) are applied to the trip generation estimates (from Table 1), the resulting site-generated traffic volumes can be determined. Figure 9 shows the short-term site-generated traffic volume estimate and Figure 10 shows the intermediateterm site-generated traffic volumes estimate.

## TOTAL TRAFFIC

Figure 11 shows the projected short-term total traffic. The short-term total traffic volumes include short-term background traffic volumes (from Figure 6) plus the short-term site-generated traffic volumes (from Figure 9). The short-term total traffic volumes assume only the short-term street connections shown in Figure 4.

Figure 12 shows the projected intermediate-term total traffic. The intermediate-term total traffic volumes include intermediate-term background traffic volumes (from Figure 7) plus the intermediate-term site-generated traffic volumes (from Figure 10). The intermediate-term total traffic volumes assume buildout of the residential portion of the Sterling Ranch development located in the vicinity of Marksheffel Road and Sterling Ranch Road. Applications for these developments have been previously submitted, but it is our understanding that these projects are currently on hold. If the areas east of the currently proposed short-term Sterling Ranch filings are developed prior to those assumed in the intermediate-term, the analysis may need to be updated.

## LONG-TERM TRAFFIC

Please refer to the master traffic report—the June 5, 2008 Sterling Ranch Updated Traffic Impact Analysis by LSC-for the long-term peak-hour traffic volume projections and level of service analysis. The original report is for the entire Sterling Ranch Sketch Plan.

## PROJECTED INTERSECTION LEVELS OF SERVICE

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. Table 2 shows the level of service delay ranges.

Table 2: Intersection Levels of Service Delay Ranges

| Level of Service | Signalized Intersections | Unsignalized Intersections <br> Average Control Delay <br> (seconds per vehicle) |
| :---: | :---: | :---: |
|  |  |  |
|  |  |  |$|$

The key area intersection and site access points were analyzed to determine the projected levels of service for the short-term and intermediate-term background and total traffic volumes based on the unsignalized intersection analysis procedures from the Highway Capacity Manual $6^{\text {th }}$ Edition. Figures 6, 7, 11, and 12 show the level of service analysis results. The level of service reports are attached.

The intersections of Dines/Vollmer and Briargate/Vollmer are projected to operate LOS B or better during the peak hours for all movements as stop-sign controlled intersections based on the projected short-term and intermediate-term total traffic volumes.

All of the site access points are projected to operate at a Level of Service A for all movements during the peak hours as stop sign-controlled intersections based on the projected intermediateterm total traffic volumes.

## SUBDIVISION STREET CLASSIFICATIONS

Figure 13 shows the recommended street classifications for the internal streets within Homestead at Sterling Ranch Filing No. 2.

## ROADWAY IMPROVEMENTS

## Vollmer Road

Road improvements to Vollmer Road including auxiliary turn lanes as discussed in our October 2, 2017 transportation memorandum are required as part of the Subdivision Improvements Agreement (SIA) for Homestead at Sterling Ranch Filing No. 1 and Branding Iron at Sterling Ranch Filing No. 1. The applicant will be constructing an interim cross-section for Vollmer Road between Marksheffel Road and Briargate Parkway no later than May 30, 2021. The interim road improvement would widen the roadway to the east side. There would continue to be one through lane in each direction, but the interim road improvements would allow for southbound left-turn and northbound right-turn lanes at the Briargate Parkway/Vollmer, and Dines/Vollmer intersections. The developer will be responsible for funding all road improvements.

## Dines Boulevard

Based on the projected intermediate-term total traffic volumes, the criteria contained in the El Paso County Engineering Criteria Manual, and the classification of Dines Boulevard as an Urban Collector, a northwest-bound right-turn deceleration lane would be required approaching Vollmer Road.

Based on the projected 2025 total traffic volumes, the criteria contained in the El Paso County Engineering Criteria Manual and the classification of Dines Boulevard as an Urban Collector,
northbound left-turn lanes and southbound right-turn lanes would not be required approaching Cut Banks Drive and Wheatland Drive.

## Wheatland Drive

Based on the projected 2025 total traffic volumes, the criteria contained in the El Paso County Engineering Criteria Manual and the classification of Wheatland Drive as an Urban Collector, a southbound left-turn lane and northbound right-turn lane would not be required approaching Niarada Way.

## TRANSPORTATION IMPROVEMENT FEE PROGRAM

The applicant will be required to participate in the Countywide Transportation Improvement Fee Program. This project will annex into the 10 mil PID. Based on a per-lot upfront building permit fee of $\$ 1,221$ per dwelling unit, the total building permit fee amount for the 104 lots would be $\$ 126,984$.

Please contact me if you have any questions regarding this report.

Respectfully Submitted,
LSC TRANSPORTATION CONSULTANTS, INC.

JCH/KDF/jas

## Enclosures: Table 1

Figures 1-13
Traffic Count Reports
Level of Service Reports
Branding Iron at Sterling Ranch Fil. No. 1 Emergency Access Rd - Alt. Route Plan

Tables and Figures

|  |  |  | Trip <br> omestead | Table eratio Sterlin | Estim <br> Ranc | ling 2 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Gen | tion | $\mathrm{s}^{(1)}$ |  |  | xte | Trips | nera |  |
| Land Use | Land Use | Trip Generation | Average Weekday |  |  |  | ng lour | Average <br> Weekday |  | ng lour |  |  |
| Code | Description | Units | Traffic | In | Out | In | Out | Traffic | In | Out | In | Out |
| 210 | Single-Family Detached Housing | $104 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 982 | 19 | 58 | 65 | 38 |
| Notes: <br> (1) Sour <br> (2) DU = | e: "Trip Generation, 10th Edition, dwelling unit | " by the Instit | of Transp | tion | ineers |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |










Figure 8
LEGEND:
$5 \frac{5 \%}{5 \%}=\begin{aligned} & \text { Intermediate-term Percent Directional Distribution }\end{aligned}$
$5 \frac{5 \%}{5 \%}=\begin{aligned} & \text { Short-Term Percent Directional Distribution } \\ & \text { Intermediate-term Percent Directional Distribution }\end{aligned}$
Directional Distribution of Site-Generated Traffic


LEGEND:
$26=\frac{\text { AM Weekday Peak-Hour Traffic (vehicles per hour) }}{}$ $31=\overline{\text { PM Weekday Peak-Hour Traffic (vehicles per hour) }}$ $500=$ Average Weekday Traffic (vehicles per day)

Assignment of Short-Term Site-Generated Traffic


Figure 10
Assignment of Intermediate-Term Site-Generated Traffic




## Traffic Counts

Page 1
Location: VOLLMER RD S/O POCO RD

COUNTER MEASURES INC.
1889 YORK STREET
DENVER,COLORADO 80206
303-333-7409
Site Code: 092712
Station ID: 092712


## LSC Transportation Consultants, Inc.

Colorado Springs, CO 80905
719-633-2868

## Default Comments

Change These in The Preferences Window
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Then Click the Comments Tab

Groups Printed- Unshifted

|  | Vollmer Rd Southbound |  |  |  |  | Cowpoke Rd Westbound |  |  |  |  | Vollmer Rd Northbound |  |  |  |  | Cowpoke Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| 06:30 | 0 | 45 | 1 | 0 | 46 | 7 | 4 | 0 | 0 | 11 | 0 | 16 | 1 | 0 | 17 | 0 | 0 | 1 | 0 | 1 | 75 |
| 06:45 | 1 | 47 | 1 | 0 | 49 | 9 | 5 | 0 | 0 | 14 | 1 | 18 | 2 | 0 | 21 | 0 | 0 | 2 | 0 | 2 | 86 |
| Total | 1 | 92 | 2 | 0 | 95 | 16 | 9 | 0 | 0 | 25 | 1 | 34 | 3 | 0 | 38 | 0 | 0 | 3 | 0 | 3 | 161 |


| $07: 00$ | 0 | 54 | 1 | 0 | 55 | 9 | 3 | 0 | 0 | 12 | 0 | 20 | 1 | 0 | 21 | 0 | 0 | 2 | 0 | 2 | 90 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $07: 15$ | 0 | 56 | 2 | 0 | 58 | 12 | 7 | 1 | 0 | 20 | 1 | 23 | 3 | 0 | 27 | 1 | 1 | 2 | 0 | 4 | 109 |
| $07: 30$ | 0 | 56 | 4 | 0 | 60 | 10 | 3 | 1 | 0 | 14 | 0 | 25 | 5 | 0 | 30 | 1 | 2 | 4 | 0 | 7 | 111 |
| $07: 45$ | 0 | 45 | 2 | 0 | 47 | 15 | 2 | 0 | 0 | 17 | 1 | 33 | 4 | 0 | 38 | 4 | 2 | 2 | 0 | 8 | 110 |
| Total | 0 | 211 | 9 | 0 | 220 | 46 | 15 | 2 | 0 | 63 | 2 | 101 | 13 | 0 | 116 | 6 | 5 | 10 | 0 | 21 | 420 |
| $08: 00$ | 0 | 36 | 0 | 0 | 36 | 10 | 5 | 1 | 0 | 16 | 3 | 22 | 9 | 0 | 34 | 2 | 0 | 0 | 0 | 2 | 88 |
| $08: 15$ | 0 | 55 | 3 | 0 | 58 | 11 | 2 | 1 | 0 | 14 | 1 | 29 | 1 | 0 | 31 | 1 | 1 | 2 | 0 | 4 | 107 |

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|  | Vollmer Rd Southbound |  |  |  |  | Cowpoke Rd Westbound |  |  |  |  | Vollmer Rd Northbound |  |  |  |  | Cowpoke Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 06:30 to 08:15-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:00 | 0 | 54 | 1 | 0 | 55 | 9 | 3 | 0 | 0 | 12 | 0 | 20 | 1 | 0 | 21 | 0 | 0 | 2 | 0 | 2 | 90 |
| 07:15 | 0 | 56 | 2 | 0 | 58 | 12 | 7 | 1 | 0 | 20 | 1 | 23 | 3 | 0 | 27 | 1 | 1 | 2 | 0 | 4 | 109 |
| 07:30 | 0 | 56 | 4 | 0 | 60 | 10 | 3 | 1 | 0 | 14 | 0 | 25 | 5 | 0 | 30 | 1 | 2 | 4 | 0 | 7 | 111 |
| 07:45 | 0 | 45 | 2 | 0 | 47 | 15 | 2 | 0 | 0 | 17 | 1 | 33 | 4 | 0 | 38 | 4 | 2 | 2 | 0 | 8 | 110 |
| Total Volume | 0 | 211 | 9 | 0 | 220 | 46 | 15 | 2 | 0 | 63 | 2 | 101 | 13 | 0 | 116 | 6 | 5 | 10 | 0 | 21 | 420 |
| \% App. Total | 0 | 95.9 | 4.1 | 0 |  | 73 | 23.8 | 3.2 | 0 |  | 1.7 | 87.1 | 11.2 | 0 |  | 28.6 | 23.8 | 47.6 | 0 |  |  |
| PHF | . 000 | . 942 | . 563 | . 000 | . 917 | . 767 | . 536 | . 500 | . 000 | . 788 | . 500 | . 765 | . 650 | . 000 | . 763 | . 375 | . 625 | . 625 | . 000 | . 656 | . 946 |



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Then Click the Comments Tab

Groups Printed- Unshifted

|  | Vollmer Rd Southbound |  |  |  |  | Cowpoke Rd Westbound |  |  |  |  | Vollmer Rd Northbound |  |  |  |  | Cowpoke Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| 16:00 | 1 | 42 | 4 | 0 | 47 | 12 | 0 | 1 | 0 | 13 | 2 | 36 | 12 | 0 | 50 | 2 | 2 | 0 | 0 | 4 | 114 |
| 16:15 | 0 | 37 | 2 | 0 | 39 | 4 | 2 | 1 | 0 | 7 | 5 | 50 | 7 | 0 | 62 | 5 | 5 | 1 | 0 | 11 | 119 |
| 16:30 | 1 | 53 | 2 | 0 | 56 | 9 | 1 | 1 | 0 | 11 | 8 | 44 | 13 | 0 | 65 | 3 | 7 | 2 | 0 | 12 | 144 |
| 16:45 | 1 | 45 | 3 | 0 | 49 | 10 | 3 | 0 | 0 | 13 | 2 | 51 | 7 | 0 | 60 | 4 | 6 | 2 | 0 | 12 | 134 |
| Total | 3 | 177 | 11 | 0 | 191 | 35 | 6 | 3 | 0 | 44 | 17 | 181 | 39 | 0 | 237 | 14 | 20 | 5 | 0 | 39 | 511 |


| $17: 00$ | 0 | 44 | 4 | 0 | 48 | 6 | 3 | 0 | 0 | 9 | 5 | 61 | 15 | 0 | 81 | 3 | 5 | 4 | 0 | 12 | 150 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $17: 15$ | 0 | 43 | 4 | 0 | 47 | 8 | 1 | 0 | 0 | 9 | 4 | 45 | 9 | 0 | 58 | 5 | 10 | 0 | 0 | 15 | 129 |
| $17: 30$ | 0 | 42 | 3 | 0 | 45 | 7 | 1 | 0 | 0 | 8 | 5 | 56 | 12 | 0 | 73 | 4 | 6 | 5 | 0 | 15 | 141 |
| $17: 45$ | 0 | 41 | 2 | 0 | 43 | 6 | 1 | 0 | 0 | 7 | 4 | 48 | 9 | 0 | 61 | 3 | 5 | 3 | 0 | 11 | 122 |
| Total | 0 | 170 | 13 | 0 | 183 | 27 | 6 | 0 | 0 | 33 | 18 | 210 | 45 | 0 | 273 | 15 | 26 | 12 | 0 | 53 | 542 |

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|  | Vollmer Rd Southbound |  |  |  |  | Cowpoke Rd Westbound |  |  |  |  | Vollmer Rd Northbound |  |  |  |  | Cowpoke Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 16:00 to 17:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 16:30 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 16:30 | 1 | 53 | 2 | 0 | 56 | 9 | 1 | 1 | 0 | 11 | 8 | 44 | 13 | 0 | 65 | 3 | 7 | 2 | 0 | 12 | 144 |
| 16:45 | 1 | 45 | 3 | 0 | 49 | 10 | 3 | 0 | 0 | 13 | 2 | 51 | 7 | 0 | 60 | 4 | 6 | 2 | 0 | 12 | 134 |
| 17:00 | 0 | 44 | 4 | 0 | 48 | 6 | 3 | 0 | 0 | 9 | 5 | 61 | 15 | 0 | 81 | 3 | 5 | 4 | 0 | 12 | 150 |
| 17:15 | 0 | 43 | 4 | 0 | 47 | 8 | 1 | 0 | 0 | 9 | 4 | 45 | 9 | 0 | 58 | 5 | 10 | 0 | 0 | 15 | 129 |
| Total Volume | 2 | 185 | 13 | 0 | 200 | 33 | 8 | 1 | 0 | 42 | 19 | 201 | 44 | 0 | 264 | 15 | 28 | 8 | 0 | 51 | 557 |
| \% App. Total | 1 | 92.5 | 6.5 | 0 |  | 78.6 | 19 | 2.4 | 0 |  | 7.2 | 76.1 | 16.7 | 0 |  | 29.4 | 54.9 | 15.7 | 0 |  |  |
| PHF | . 500 | . 873 | . 813 | . 000 | . 893 | . 825 | . 667 | . 250 | . 000 | . 808 | . 594 | . 824 | . 733 | . 000 | . 815 | . 750 | . 700 | . 500 | . 000 | . 850 | . 928 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | 1 | $\mathbf{4}$ |
| Traffic Vol, veh/h | 94 | 5 | 110 | 33 | 2 | 210 |
| Future Vol, veh/h | 94 | 5 | 110 | 33 | 2 | 210 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 102 | 5 | 120 | 36 | 2 | 259 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ri | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{4}$ |
| Traffic Vol, veh/h | 61 | 3 | 260 | 102 | 5 | 150 |
| Future Vol, veh/h | 61 | 3 | 260 | 102 | 5 | 150 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 66 | 3 | 283 | 111 | 6 | 185 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 3 | 3.2 |  |  |  |  |  |
| Movement WBL | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * | ¢ | 4 | 7 | ${ }^{*}$ | 4 |
| Traffic Vol, veh/h 131 | 131 | 7 | 116 | 45 | 2 | 228 |
| Future Vol, veh/h 131 | 131 | 7 | 116 | 45 | 2 | 228 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Stop | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length 205 | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 142 | 142 | 8 | 126 | 49 | 2 | 281 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | A |
| Traffic Vol, veh/h | 18 | 1 | 117 | 6 | 1 | 212 |
| Future Vol, veh/h | 18 | 1 | 117 | 6 | 1 | 212 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 1 | 127 | 7 | 1 | 262 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{T}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | A |
| Traffic Vol, veh/h | 86 | 4 | 280 | 144 | 6 | 161 |
| Future Vol, veh/h | 86 | 4 | 280 | 144 | 6 | 161 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 93 | 4 | 304 | 157 | 7 | 199 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{T}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | A |
| Traffic Vol, veh/h | 11 | 1 | 264 | 20 | 2 | 156 |
| Future Vol, veh/h | 11 | 1 | 264 | 20 | 2 | 156 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 1 | 287 | 22 | 2 | 193 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | 4 |
| Traffic Vol, veh/h | 8 | 11 | 135 | 5 | 4 | 212 |
| Future Vol, veh/h | 8 | 11 | 135 | 5 | 4 | 212 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 12 | 147 | 5 | 5 | 262 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement EBL | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | * |  |  | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 6 | 0 | 2 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 6 | 3 |  |
| Future Vol, veh/h | 6 | 0 | 2 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 6 | 3 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control S | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 7 | 0 | 2 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 7 | 3 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | A | F |  |
| Traffic Vol, veh/h | 1 | 3 | 0 | 12 | 7 | 1 |
| Future Vol, veh/h | 1 | 3 | 0 | 12 | 7 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 3 | 0 | 13 | 8 | 1 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | 1 | $\mathbf{4}$ |
| Traffic Vol, veh/h | 5 | 7 | 273 | 18 | 12 | 192 |
| Future Vol, veh/h | 5 | 7 | 273 | 18 | 12 | 192 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 5 | 8 | 297 | 20 | 15 | 237 |







| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{4}$ |
| Traffic Vol, veh/h | 29 | 13 | 138 | 12 | 4 | 222 |
| Future Vol, veh/h | 29 | 13 | 138 | 12 | 4 | 222 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 32 | 14 | 150 | 13 | 5 | 274 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{F}$ | $\mathbf{1}$ | 4 |
| Traffic Vol, veh/h | 10 | 2 | 148 | 3 | 1 | 216 |
| Future Vol, veh/h | 10 | 2 | 148 | 3 | 1 | 216 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 11 | 2 | 161 | 3 | 1 | 267 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 0 | 4 | 4 | 0 | 12 | 12 |
| Future Vol, veh/h | 0 | 4 | 4 | 0 | 12 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 4 | 4 | 0 | 13 | 13 |







| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{T}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{F}$ | $\mathbf{1}$ | A |
| Traffic Vol, veh/h | 20 | 9 | 285 | 42 | 13 | 198 |
| Future Vol, veh/h | 20 | 9 | 285 | 42 | 13 | 198 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 10 | 310 | 46 | 16 | 244 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{r}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | A |
| Traffic Vol, veh/h | 7 | 1 | 282 | 12 | 2 | 204 |
| Future Vol, veh/h | 7 | 1 | 282 | 12 | 2 | 204 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 205 | 0 | - | 235 | 285 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 1 | 307 | 13 | 2 | 252 |









## Additional Attachments

Branding Iron at Sterling Ranch Fil. No. 1 Emergency Access Rd - Alt. Route Plan

GENERAL NOTES:






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BRANDING IRON AT STERLING RANCH FIL. NO. 1
BETWEEN VOLLMER ROAD - DINES BOULEVARD COUNTY OF EL PASO, STATE OF COLORADO
EMERGENCY ACCESS RD - ALT. ROUTE PLAN
FEBRUARY 2019


EMERGENCY ACCESS ROAD ALT. ROUTE PLAN


APPROVALS:
BLACK FOREST FIRE PROTECTION DISTRICT: tiHs document fane deen reveneo ano approve:



OWNER/DEVELOPER STATEMENT:


## 

STERLING RANCH METROPOLITAN DISTRICT:
tins document fave been revened and approveo.

For ANO ON BEHALF OF THE STERLING RANCH MERO DISTRC
CONDITIONS:

