

LSC TRANSPORTATION CONSULTANTS, INC.

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Settlers View Subdivision Updated Transportation Memorandum (LSC #164720)

February 14, 2018

Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.

Jeffrey C. Hodsdon, P.E. #31684 31684 5 Date

Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

Date



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February 14, 2018

Mr. Jerry Hannigan Jerome W. Hannigan and Associates, Inc. 19360 Spring Valley Road Monument, CO 80132

> RE: Settlers View Subdivision El Paso County, CO Transportation Memorandum

Transportation Memorandum

LSC #164720

Dear Jerry:

LSC Transportation Consultants, Inc. has prepared this transportation memorandum for the proposed Settlers View subdivision. The site is located generally northwest of the intersection of Hodgen Road and Steppler Road in El Paso County, Colorado. The site's location is shown in Figure 1. Site access would be through adjacent subdivisions as the site is not directly adjacent to Steppler Road. This analysis has been prepared in conjunction with the proposed Abert Estates subdivision, which is adjacent to Settlers View. LSC has prepared a separate traffic report for Abert Estates.

REPORT CONTENTS

The report contains the following:

- Existing roadway and traffic conditions in the vicinity of the site, including the intersection lane geometries, traffic controls, posted speed limits, functional classifications, intersection spacing and alignment, etc.
- Existing peak-hour turning movement traffic counts and/or estimates of future background traffic volumes at the intersections of:
 - o Steppler Road at Silver Nell Drive
 - Steppler Road at Settler's Ranch Road (future)
- Description of the proposed land use
- Estimates of the average weekday and peak-hour vehicle-trips to be generated by the site
- Projected site-generated traffic volumes on roadways and intersections to provide access to the site
- Analysis of the resulting traffic impacts from the site including the development's relative average daily traffic volume impacts and intersection level of service analysis
- Findings and recommendations

LAND USE AND ACCESS

Site Land Use and Access

Settlers View is a proposed single-family residential subdivision consisting of 14 lots, each a minimum of 2.5 acres. The location of the site is shown in Figure 1. Figure 1 also shows the proposed adjacent Abert Ranch site. The existing Grandview subdivision is located to the north of the Settlers View and Abert Ranch sites and the eastern portion of Settlers Ranch is located to the south. The Settlers View site plan/subdivision plat is shown in Figure 2.

Site access to Steppler Road would be via a proposed extension of Silver Nell Drive. Future access is also planned through Abert Ranch to the planned future extension of Settlers Ranch Road. Settlers Ranch Road will ultimately connect to Steppler Road and will provide the secondary access for the Settlers View subdivision.

Adjacent Subdivisions – Existing and Proposed

Abert Ranch

Abert Ranch is a proposed single-family residential subdivision consisting of 10 lots, each a minimum of 2.5 acres. Site access to Steppler Road would initially be through the Settlers View subdivision and the extension of Silver Nell Drive. A second access would be available via the proposed future Settlers Ranch Road once constructed by the developer of Settlers Ranch.

Settlers Ranch

Settlers Ranch is located south and southeast of the site. Filing 1 to the southwest has been developed. The Settlers Ranch Road extension to Steppler Road will be added with future Filing 2. This future road connection will provide secondary access to both Abert Ranch and this site (via the proposed Abert Ranch subdivision roads).

Grandview

Grandview is located to the north of the Settlers View and Abert Ranch sites. It is partially developed, but Silver Nell Drive through Grandview has been completed and provides access to Steppler Road. Silver Nell Drive will provide the initial access to both the Settlers View and Abert Ranch subdivisions.

EXISTING ROADWAY AND TRAFFIC CONDITIONS

Area Roadways

Major roadways in the vicinity of the site are summarized below:

State Highway (SH) 83 extends from Colorado Springs north to Parker and areas of southeast Denver. In the vicinity of the site, SH 83 is classified as a Regional Highway (R-A). At this

location, SH 83 is a two-lane rural highway with two- to four-foot shoulders and a speed limit of 60 miles per hour (mph). The intersection with Hodgen Road is signalized.

Hodgen Road is a two-lane paved Rural Minor Arterial that extends east from the intersection of Roller Coaster Road/Baptist Road to Eastonville Road. The speed limit on Hodgen Road is generally 55 mph east of SH 83.

Walker Road/SH 105. Highway 105 west of SH 83 is a Principal Arterial, while Walker Road east of SH 83 is a Collector roadway. Both are currently two-lane roadways, but the *Major Transportation Corridors Plan (MTCP)* shows a future four-lane cross section on SH 105 west of SH 83. The intersection with SH 83 is unsignalized.

Steppler Road is a local roadway extending north from Hodgen Road to Walker Road. The posted speed limit on Steppler Road is 30 mph.

Traffic Volumes

Turning movement counts were conducted on Tuesday, August 30, 2016 from 4:00 to 6:00 p.m. and on September 1, 2016 from 6:30 to 8:30 a.m. at the intersection of Steppler Road at Silver Nell Drive. Count reports are attached. Based on these count data, existing morning and evening weekday peak-hour traffic volumes at this intersection are shown in Figure 3. Estimates of the average daily traffic volumes on Steppler Road based on these peak-hour counts are also shown in Figure 3.

Level of Service

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 1 shows the level of service delay ranges for signalized and unsignalized intersections.

Table 1: Intersection Levels of Service Delay Ranges

Level of	Signalized Intersections	Unsignalized Intersections
Service	Control Delay (se	conds per vehicle)
A	10 sec or less	10 sec or less
В	10-20 sec	10-15 sec
С	20-35 sec	15-25 sec
D	35-55 sec	25-35 sec
Е	55-80 sec	35-50 sec
F	80 sec or more	50 sec or more

The intersection of Steppler Road at Silver Nell Drive has been analyzed in Synchro to determine the current level of service using the unsignalized method of analysis procedures from the *Highway Capacity Manual*, 2010 Edition. The level of service is A.

TRIP GENERATION

Estimates of the vehicle-trips projected to be generated by Settlers View have been made using the nationally published trip generation rates from *Trip Generation*, 9th Edition, 2012 by the Institute of Transportation Engineers (ITE). Land use code 210 – Single-Family Detached Housing was categorized using the *Trip Generation Manual*, 9th Edition, 2012 by the Institute of Transportation Engineers (ITE) and used for trip generation estimates. The proposed Settlers View subdivision is projected to generate about 133 total vehicle-trips on the average weekday during a 24-hour period, with about half entering the site and half exiting the site during the evening peak hour. The peak-hour trip generation is also summarized. A detailed trip generation estimate for the development, including ITE rates for the proposed land use, is presented in Table 6 (attached).

Trip Distribution and Assignment

Distribution of the site-generated vehicle-trips to the study area streets and intersections is a necessary component in determining the site's traffic impacts. Figure 4 shows the directional distribution estimate for the site-generated trips. The figure shows the percentages of the site-generated vehicle-trips projected to be oriented to and from the site's major approaches. Estimates were based on the following factors: the proposed land use and access plan, the area street system, and anticipated area future development.

Site-Generated Traffic

When the directional distribution percentages (from Figure 4) were applied to the trip generation estimates (from Table 6), the site-generated traffic volumes on the adjacent streets were determined. Figure 5 shows the projected site-generated traffic volumes.

EXISTING VS. EXISTING PLUS SITE-GENERATED TRAFFIC/LOS

Traffic Volumes

Figure 7 shows the sum of the existing weekday traffic volumes (from Figure 3) and site-generated weekday traffic volumes (from Figure 4). The existing plus site-generated trips identify the site's short-term traffic impacts assuming buildout of all three aforementioned subdivisions.

Levels of Service

Morning Peak Hour

All approaches at the intersections of Steppler Road at Silver Nell Drive currently operate at and are projected to remain at LOS A during the morning peak hour upon site buildout. A summary of existing and projected short-term background plus site-generated LOS and control delays during the morning peak hour is shown in Table 2.

Table 2: Projected Peak-Hour LOS and Control Delays by Intersection (2016 a.m.)

Intersection	Traffic Control*	Scenario	NBL	EBL
		LOS		
Steppler Road @	TWSC	Existing	A	A
Silver Nell Dr	TWSC	Short-term BG + Site (short-term)	A	A
	Co	ntrol Delay (seconds)		
Steppler Road @	TWSC	Existing	7.3	8.5
Silver Nell Dr	1 WSC	Short-term BG + Site (short-term)	7.3	8.5
* TWSC = two-way	stop sign-control			

Evening Peak Hour

All approaches at the intersections of Steppler Road at Silver Nell Drive currently operate at and are projected to remain at LOS A during the evening peak hour upon site buildout. A summary of existing and projected short-term background plus site-generated LOS and control delays during the morning peak hour is shown in Table 3.

Table 3: Projected Peak-Hour LOS and Control Delays by Intersection (2016 p.m.)

Tubic Di Troje	ctcu i cum mour b	os and control belays by intersection	(=010 P	,,,,,
Intersection	Traffic Control*	Scenario	NBL	EBL
		LOS		
Steppler Road @	TWSC	Existing	A	A
Silver Nell Dr	1 WSC	Short-term BG + Site (short-term)	A	A
	Co	ntrol Delay (seconds)		
Steppler Road @	TWSC	Existing	7.3	8.4
Silver Nell Dr	1 WSC	Short-term BG + Site (short-term)	7.3	8.5
* TWSC = two-way	stop sign-control			

2040 BACKGROUND VS. 2040 TOTAL TRAFFIC/LOS

Traffic Volumes

Figure 8 shows the projected 2040 background traffic volumes based on existing turning movement counts (from Figure 3), the historic growth rate, and projected future development. Projected 2040 background plus site-generated weekday traffic volumes are shown in Figure 9.

Levels of Service

Morning Peak Hour

All approaches at the intersections of Steppler Road/Silver Nell Drive and Steppler Road/Settlers Ranch Road are projected to operate at LOS A during the 2040 morning peak hour with and without considering site-generated trips. A summary of projected 2040 background plus site-generated LOS and control delays during the morning peak hour is shown in Table 4.

Table 4: Projected Peak-Hour LOS and Control Delays by Intersection (2040 a.m.)

Intersection	Traffic Control*	Scenario	NBL	EBL
		LOS		
Steppler Road @	TWSC	2040 Background	A	A
Silver Nell Dr	1 W SC	2040 Background + Site	A	A
Steppler Road @	TWSC	2040 Background	A	A
Settler's Ranch Rd	1 W SC	2040 Background + Site	A	A
	Con	trol Delay (seconds)		
Steppler Road @	TWSC	2040 Background	7.3	8.5
Silver Nell Dr	1 W SC	2040 Background + Site	7.3	8.6
Steppler Road @	TWSC	2040 Background	7.3	8.6
Settlers Ranch Rd	1 W 5C	2040 Background + Site	7.3	8.6
* TWSC = two-way	stop sign-control			

Evening Peak Hour

All approaches at the intersections of Steppler Road/Silver Nell Drive and Steppler Road/Settlers Ranch Road are projected to operate at LOS A during the 2040 morning peak hour with and without considering site-generated trips. A summary of projected 2040 background plus site-generated LOS and control delays during the evening peak hour is shown in Table 5.

Table 5: Projected Peak-Hour LOS and Control Delays by Intersection (2040 p.m.)

Intersection	Traffic Control*	Scenario	NBL	EBL
		LOS		
Steppler Road @	TWSC	2040 Background	A	A
Silver Nell Dr	1 W SC	2040 Background + Site	A	A
Steppler Road @	TWSC	2040 Background	A	A
Settler's Ranch Rd	1 W SC	2040 Background + Site	A	A
	Con	trol Delay (seconds)		
Steppler Road @	TWSC	2040 Background	7.3	8.5
Silver Nell Dr	1 W SC	2040 Background + Site	7.3	8.6
Steppler Road @	TWSC	2040 Background	7.3	8.6
Settler's Ranch Rd	1 W SC	2040 Background + Site	7.3	8.6
* TWSC = two-way	stop sign-control			

CONCLUSIONS AND RECOMMENDATIONS

Trip Generation

• The proposed Settlers View subdivision is projected to generate about 133 new vehicle-trips on the average weekday with about half entering and half exiting the site. The projected morning **peak-hour** trip generation for the site (total "driveway" trips) is 3 entering and 8 exiting trips. The projected evening **peak-hour** trip generation for the site (total "driveway" trips) is 9 entering and 5 exiting trips.

Level of Service Analysis

• Levels of service at the intersections analyzed are projected to be A. Please refer to the Level of Service sections above for detailed findings and results of the intersection level of service analysis.

Auxiliary Turn Lanes

• Neither Silver Nell/Steppler nor Settlers Ranch Road/Steppler will exceed *Engineering Criteria Manual* thresholds requiring auxiliary left- and right-turn lanes.

Street Classification

The streets within this proposed subdivision should be classified as Rural Local streets.

County Road Improvement Fee Program

This project will need to participate in the County Road Improvement Fee Program.

* * *

Please contact me if you have any questions regarding this report.

Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

Вy

Jeffrey C. Hodsdon, P.E., PTOE

Principal Principal

JCH/JAB:bjwb

Enclosures: Table 6

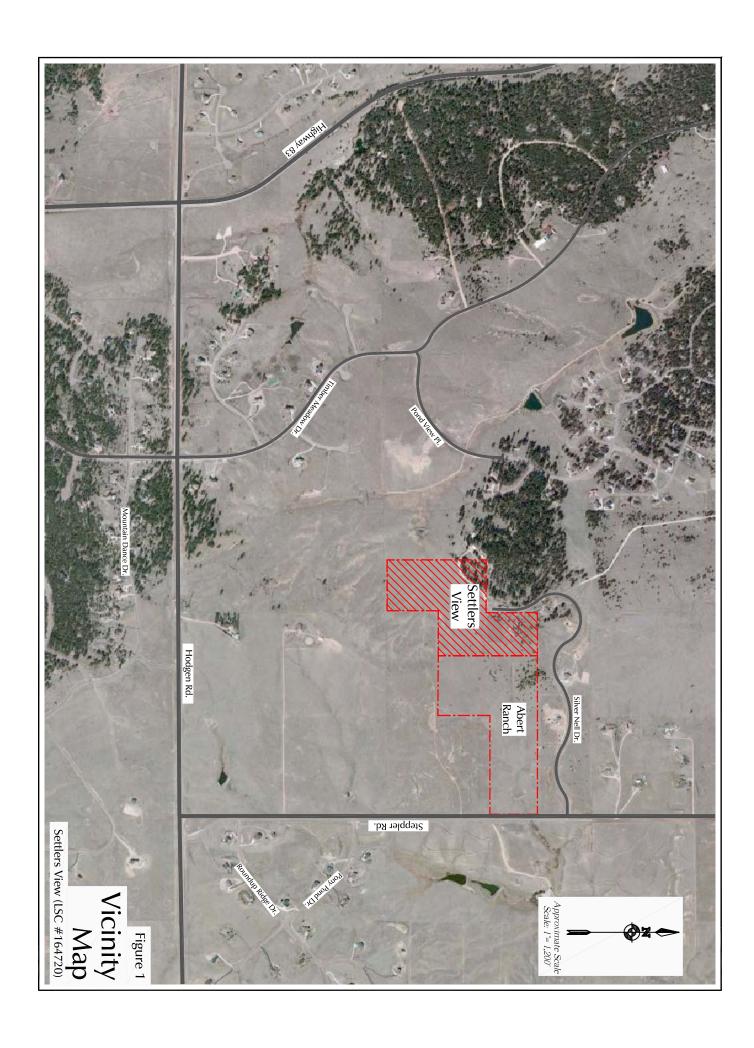
Figure 1-Figure 9
Appendix Figures 1-3
Traffic Count Reports
Level of Service Reports

Table 6: Trip Generation Estimate and Comparison

	ITE Land				Trip	Gener	ation R	Rates (1)	Total T	Trips	Gene	rateo	1
Lots	Use	Land Use Description	Value	Units	Average	A.	M.	P.	M.	Average	A	.M.	P.	.M.
200	Code	Zunu (30 Z 300 provi	, 4140		Weekday Traffic	In	Out	In	Out	Weekday Traffic	In	Out	In	Out
Abert Ranch Only														
1-10	210	Single-Family Detached Housing	10	DU (2)	9.52	0.19	0.56	0.63	0.37	95	2	6	6	4
Settler's View Only														
1-14	210	Single-Family Detached Housing	14	DU	9.52	0.19	0.56	0.63	0.37	133	3	8	9	5
		Total								228	5	14	15	9

⁽¹⁾ Source: "Trip Generation, 9th Edition, 2012" by the Institute of Transportation Engineers (ITE)

⁽²⁾ DU = dwelling units



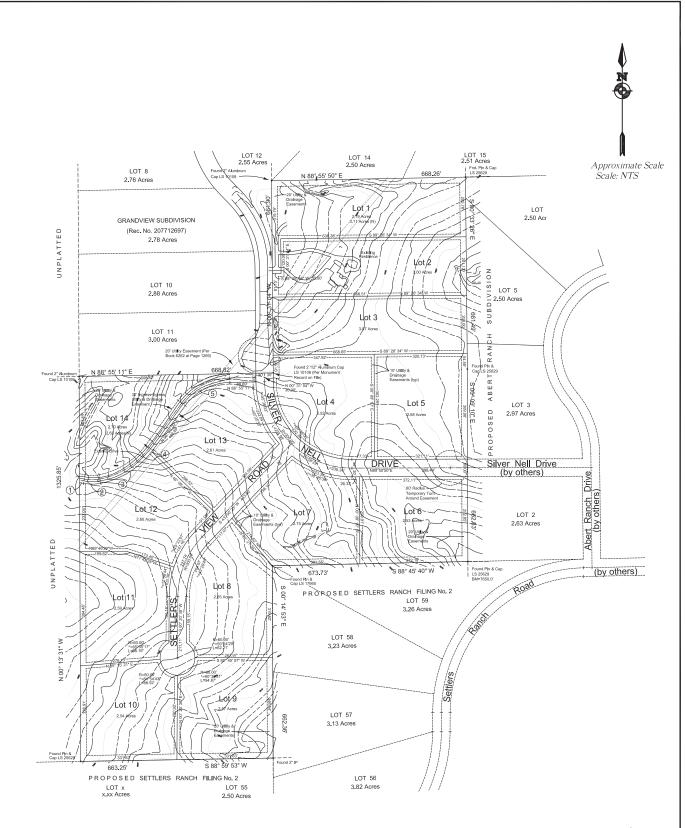
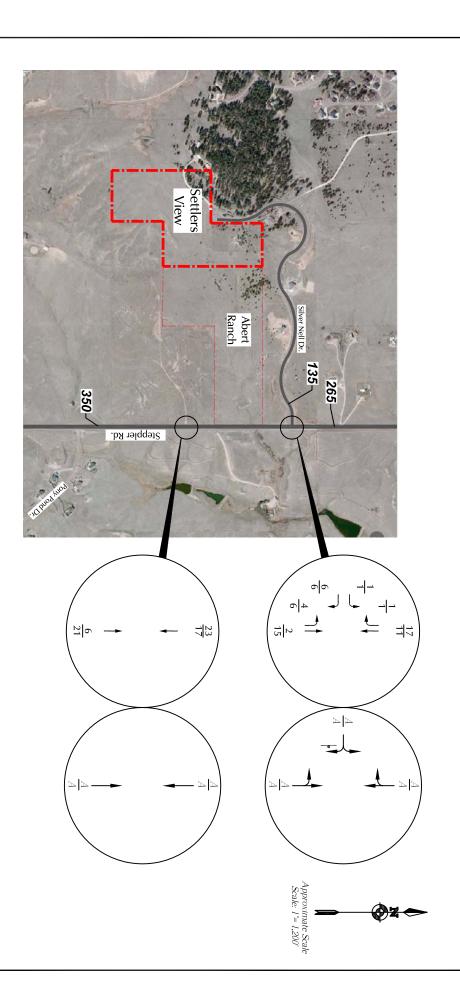


Figure 2

Settlers View Site Plan





LEGEND:

Stop Sign

XXX = Average Weekday Traffic (vehicles per day)
Estimates by LSC

AM Weekday Peak-Hour Traffic (vehicles per hour)

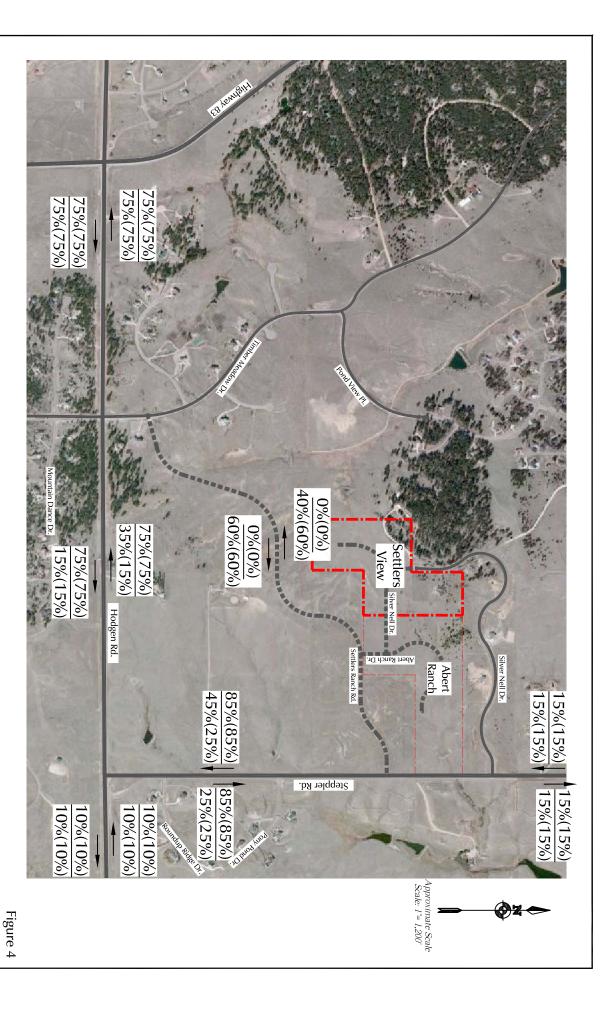
PM Weekday Peak-Hour Traffic (vehicles per hour)

PM Weekday Peak-Hour Traffic (vehicles per hour)

AM Individual Movement Peak-Hour Level of Service

PM Individual Movement Peak-Hour Level of Service

Traffic Control & Level of Service



X%(X%)

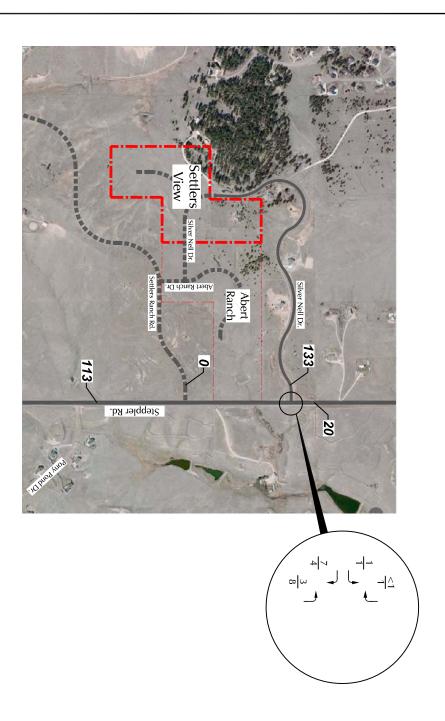
X%(X%)

X%(X%)

LEGEND:

Short—Term Percent Directional Distribution AM(PM) Long—Term Percent Directional Distribution AM(PM)

Directional Distribution of Site-Generated Traffic



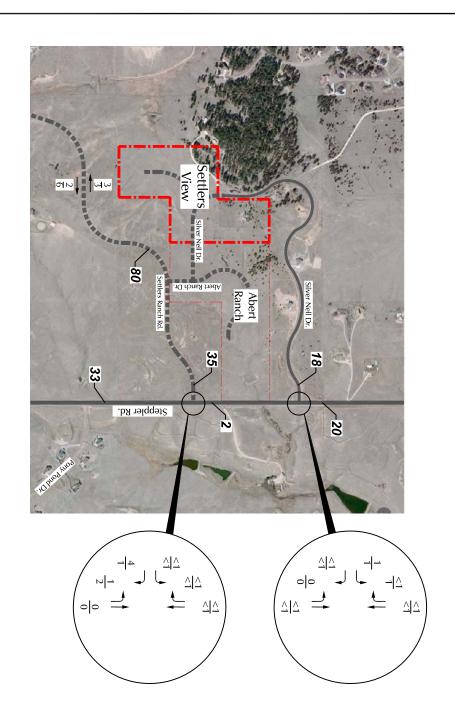
Approximate Scale
Scale: I'= 1,200'

LEGEND:

 $\frac{XX}{XX} = \frac{AM \text{ Weekday Peak-Hour Traffic (vehicles per hour)}}{PM \text{ Weekday Peak-Hour Traffic (vehicles per hour)}}$

XXX = Average Weekday Traffic (vehicles per day)

Short-Term Assignment of Site-Generated Traffic



Approximate Scale Scale: I'= 1,200'

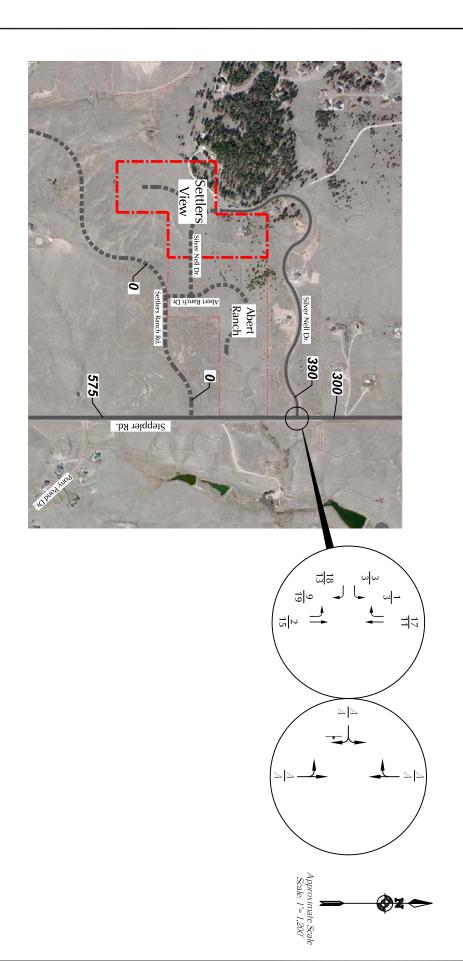
LEGEND:

AM Weekday Peak—Hour Traffic (vehicles per hour) PM Weekday Peak—Hour Traffic (vehicles per hour)

XXX = Average Weekday Traffic (vehicles per day)

Figure 6

Long-Term Assignment of Site-Generated Traffic Settlers View (LSC #164720)



* Includes buildout of the site plus Abert Ranch plus Grandview but not Settlers Ranch. Assumes Settlers Ranch Road not built adjacent to Abert Ranch east of Albert Ranch.

LEGEND:

Stop Sign

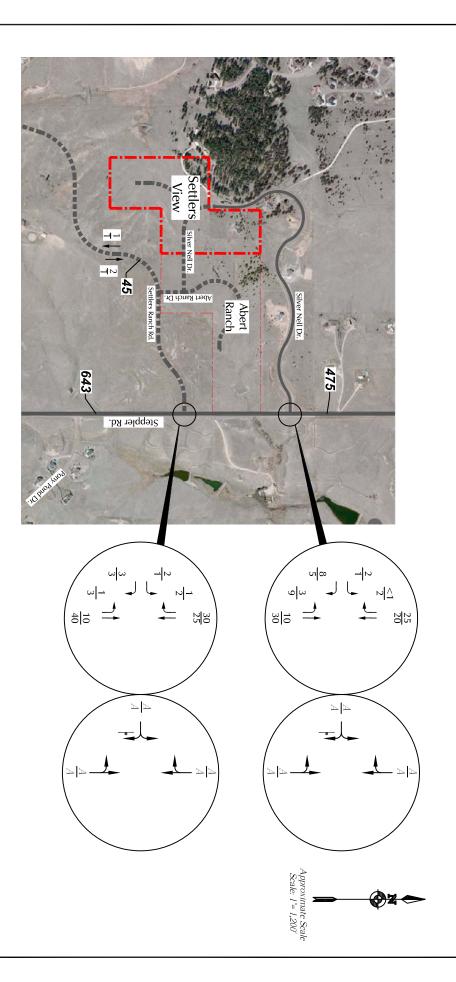
AM Weekday Peak—Hour Traffic (vehicles per hour)
PM Weekday Peak—Hour Traffic (vehicles per hour)

XXX = Average Weekday Traffic (vehicles per day)

AM Individual Movement Peak-Hour Level of Service PM Individual Movement Peak-Hour Level of Service Geometry, Traffic Control & Level of Service

Settlers View (LSC #164720) Short-Term

Figure 7



*Not including Settlers Ranch or Abert Ranch.

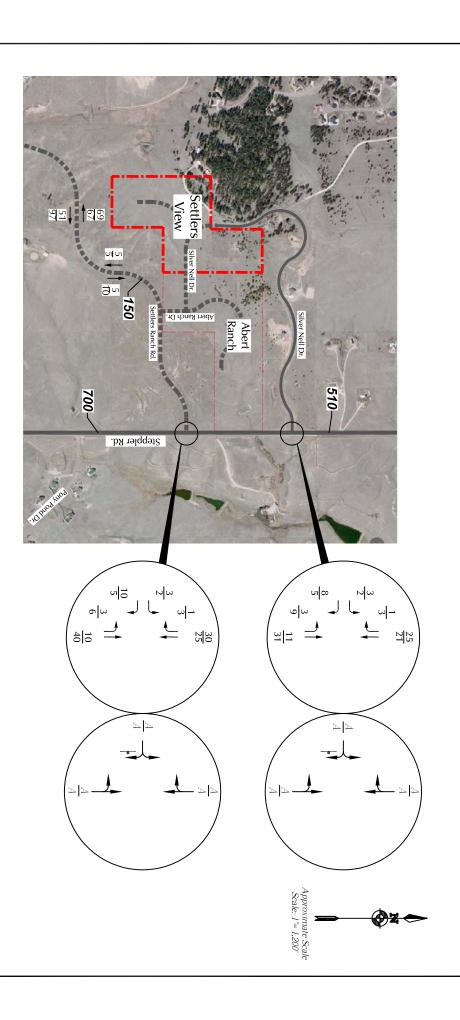
LEGEND:

Stop Sign

AM Weekday Peak—Hour Traffic (vehicles per hour)
PM Weekday Peak—Hour Traffic (vehicles per hour)

Figure 8

= AM Individual Movement Peak-Hour Level of Service Year 2040 Background Traffic, Lane PM Individual Movement Peak-Hour Level of Service Year 2040 Background Traffic, Lane Service Year 2040 Background Traffic, Lane Year PM Individual Movement Peak-Hour Level of Service Year 2040 Background Traffic, Lane Year 2040 Background Traffic, Lane



LEGEND:

Stop Sign

AM Weekday Peak—Hour Traffic (vehicles per hour)
PM Weekday Peak—Hour Traffic (vehicles per hour)

XXX = Average Weekday Traffic (vehicles per day)

Figure 9

545 E. Pikes Peak Ave., #210

 $\hbox{LSC Transportation Consultants, Inc.} \quad \hbox{\bf Colorado Springs, CO~80903} \hbox{Name} \quad \hbox{: Steppler Rd-Silver NeII Dr~AM}$

Site Code : 00164720 (719) 633-2868

Start Date : 09/01/2016

Page No

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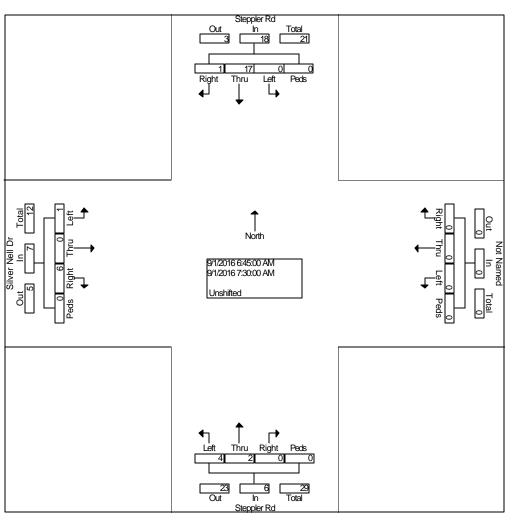
		Steppl	er Rd							Stepple	er Rd		5	Silver Ne	ell Dr		
		From	North			From	East			From S	outh			From V	Vest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	0	1	0	0	0	0	0	0	0	1	0	0	1	0	1	0	4
06:45 AM	0	3	0	0	0	0	0	0	0	0	0	0	1	0	0	0	4
Total	0	4	0	0	0	0	0	0	0	1	0	0	2	0	1	0	8
					1				ı				ı				1
07:00 AM	1	4	0	0	0	0	0	0	0	1	1	0	2	0	1	0	10
07:15 AM	0	5	0	0	0	0	0	0	0	1	2	0	1	0	0	0	9
07:30 AM	0	5	0	0	0	0	0	0	0	0	1	0	2	0	0	0	8
07:45 AM	0	2	0	0	0	0	0	0	0	0	1	0	0	0	0	0	3
Total	1	16	0	0	0	0	0	0	0	2	5	0	5	0	1	0	30
08:00 AM	0	0	0	0	0	0	0	0	0	3	1	0	1	0	2	0	7
08:15 AM	0	1	0	0	0	0	0	0	0	4	0	0	1	0	1	0	7
Grand Total	1	21	0	0	0	0	0	0	0	10	6	0	9	0	5	0	52
Apprch %	4.5	95.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	62.5	37.5	0.0	64.3	0.0	35.7	0.0	
Total %	1.9	40.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	19.2	11.5	0.0	17.3	0.0	9.6	0.0	

545 E. Pikes Peak Ave., #210

Colorado Springs, CO 809@3Name : Steppler Rd - Silver Nell Dr AM (719) 633-2868 Site Code : 00164720 Start Date : 09/01/2016

Page No : 2

		Sto	eppler	Rd								S	tepple	r Rd			S	ilver N	lell Dr		7
		Fr	om No	orth			F	rom E	ast			F	rom S	outh			F	rom '	West		
Start	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour	From (06:30	AM to	08:1	5 AM - I	Peak	1 of 1														
Intersecti on	06:4	5 AM																			
Volume	1	17	0	0	18	0	0	0	0	0	0	2	4	0	6	6	0	1	0	7	31
Percent	5.6	94. 4	0.0	0.0		0.0	0.0	0.0	0.0		0.0	33. 3	66. 7	0.0		85. 7	0.0	14. 3	0.0		
07:00 Volume	1	4	0	0	5	0	0	0	0	0	0	1	1	0	2	2	0	1	0	3	10
Peak															•						0.775
Factor																					
High Int.	07:0	MA 0				6:15	:00 A	M			07:1	5 AM	1			07:	00 AN	Λ			
Volume	1	4	0	0	5	0	0	0	0	0	0	1	2	0	3	2	0	1	0	3	·
Peak					0.90										0.50					0.58	
Factor					0										0					3	



545 E. Pikes Peak Ave., #210

LSC Transportation Consultants, Inc. Colorado Springs, CO 80'90'3 Name : Steppler Rd - Silver Nell Dr PM

(719) 633-2868 Site Code : 00164720 Start Date : 08/30/2016

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Page No : 1

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Ī			Steppl	erRd							Stepple	er Rd		5	Silver Ne	ell Dr		
			From	North			From	East			From S	South			From V	Vest		
	Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
	Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
	04:00 PM	1	4	0	0	0	0	0	0	0	3	1	0	2	0	0	0	11
	04:15 PM	0	1	0	0	0	0	0	0	0	7	1	0	1	0	0	0	10
	04:30 PM	0	4	0	0	0	0	0	0	0	2	1	0	2	0	1	0	10
	04:45 PM	0	2	0	0	0	0	0	0	0	3	1	0	1	0	0	0	7
Ī	Total	1	11	0	0	0	0	0	0	0	15	4	0	6	0	1	0	38
										_								
	05:00 PM	0	1	0	0	0	0	0	0	0	3	1	0	1	0	1	0	7
	05:15 PM	0	3	0	0	0	0	0	0	0	2	3	0	1	0	0	0	9
	05:30 PM	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	4
	05:45 PM	0	2	0	0	0	0	0	0	0	3	1	0	1	0	1	0	8
	Total	0	6	0	0	0	0	0	0	0	12	5	0	3	0	2	0	28
	Grand Total	1	17	0	0	0	0	0	0	0	27	9	0	9	0	3	0	66
	Apprch %	5.6	94.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	75.0	25.0	0.0	75.0	0.0	25.0	0.0	
	Total %	1.5	25.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	40.9	13.6	0.0	13.6	0.0	4.5	0.0	

545 E. Pikes Peak Ave., #210

Colorado Springs, CO 80903Name : Steppler Rd - Silver Nell Dr PM (719) 633-2868 Site Code : 00164720

(719) 633-2868

Start Date : 08/30/2016

Page No : 2

			epple om N				г	rom E	=ac+				tepple					ilver N From			
Start	Rig	Thr		Pe	Арр.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht	u	t	ds	Total	ht	u	t		Total	ht	u	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour I		04:00 0 PM	PM to	05:45	5 PM - F	Peak 1	of 1														
on Volume	1	11	0	0	12	0	0	0	0	0	0	15	4	0	19	6	0	1	0	7	38
Percent	8.3	91. 7	0.0	0.0		0.0	0.0	0.0	0.0		0.0	78. 9	21. 1	0.0		85. 7	0.0	14. 3	0.0		
04:00 Volume Peak	1	4	0	0	5	0	0	0	0	0	0	3	1	0	4	2	0	0	0	2	11 0.864
Factor High Int. Volume Peak Factor	04:00 1	0 PM 4	0	0	5 0.60 0	3:45 0	:00 PN 0	И О	0	0	04:1	5 PM 7	1	0	8 0.59 4	04:3	30 PM 0	l 1	0	3 0.58 3	
									[Out 16	Stepple In 11	12 -	Total 28 0 Peds								
			Silver Nell Dr Out In Total		Peds Right Thru Left	•				8/30/2016 8/30/2016 Unshifte	6 4:45:00	PM					↑	Right Thru Left Peds	0	Out In Total	
										4	hru 1	Right 0	Peds 0								

Intersection						
	2.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			स	4	
Traffic Vol, veh/h	1	6	4	2	17	1
Future Vol, veh/h	1	6	4	2	17	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-		-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	7	4	2	18	1
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	30	19	20	0	-	0
Stage 1	19		-	-		-
Stage 2	11	<u>-</u>		-		-
Critical Hdwy	6.42	6.22	4.12	-		-
Critical Hdwy Stg 1	5.42	-	-	-	-	_
Critical Hdwy Stg 2	5.42	-	-	-	_	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	
Pot Cap-1 Maneuver	984	1059	1596	-	-	-
Stage 1	1004	-	-	-	-	-
Stage 2	1012	-	-	-	-	-
Platoon blocked, %				-		-
Mov Cap-1 Maneuver	981	1059	1596	-		-
Mov Cap-2 Maneuver	981	-	-	-		-
Stage 1	1004	-	-	-	-	-
Stage 2	1009	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		4.8		0	
HCM LOS	A		1.0		U	
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR			
Capacity (veh/h)	1596	- 1047				
HCM Lane V/C Ratio	0.003	- 0.007				
HCM Control Delay (s)	7.3	0 8.5				
HCM Lane LOS	7.5 A	A A				
HCM 95th %tile Q(veh)	0	- 0				
113.11 70.11 70.110 (2(1011)	0	- 0				

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	4	
Traffic Vol, veh/h	1	6	6	15	11	1
Future Vol, veh/h	1	6	6	15	11	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	7	7	16	12	1
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	41	12	13	0	-	0
Stage 1	12	-	-	-	-	-
Stage 2	29	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	970	1069	1606	-	-	-
Stage 1	1011	-	-	-	-	-
Stage 2	994	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	966	1069	1606	-	-	-
Mov Cap-2 Maneuver	966	-	-	-	-	-
Stage 1	1011	-	-	-	-	-
Stage 2	990	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.4		2.1		0	
HCM LOS	А					
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR			
Capacity (veh/h)	1606	- 1053				
HCM Lane V/C Ratio	0.004	- 0.007				
HCM Control Delay (s)	7.3	0.007				
HCM Lane LOS	7.5 A	A A				
HCM 95th %tile Q(veh)	0	- 0				
1101VI 70111 701110 Q(VCII)	- 0	- 0				

Intersection						
Int Delay, s/veh	4.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩			4	₽	
Traffic Vol, veh/h	3	18	9	2	17	1
Future Vol, veh/h	3	18	9	2	17	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
	2	2	2	2	2	2
Heavy Vehicles, %	3	20	10	2	18	1
Mvmt Flow	3	20	10	2	Ιδ	ı
Major/Minor	Minor2	1	Major1	Ν	Major2	
Conflicting Flow All	41	19	19	0	-	0
Stage 1	19	-	-	-	-	-
Stage 2	22	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	2.218	-	-	-
Pot Cap-1 Maneuver	970	1059	1597	-	-	-
Stage 1	1004	-	_	_	-	_
Stage 2	1001	-	_	-	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	964	1059	1597	_	_	_
Mov Cap 1 Maneuver	964	-	-	_	_	_
Stage 1	998	_	_	_	_	
Stage 2	1001		_		_	_
Staye 2	1001	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		5.9		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBL	NDT	EDI n1	SBT	SBR
	ш			EBLn1		SBR
Capacity (veh/h)		1597		1044	-	-
HCM Lane V/C Ratio		0.006		0.022	-	-
HCM Control Delay (s))	7.3	0	8.5	-	-
110141 100						
HCM Lane LOS HCM 95th %tile Q(veh		A 0	Α -	A 0.1	-	-

Intersection						
Int Delay, s/veh	4.3					
					05=	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	, A			4	₽	
Traffic Vol, veh/h	3	13	19	15	11	3
Future Vol, veh/h	3	13	19	15	11	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	14	21	16	12	3
IVIVIII I IOVV	3	17	21	10	12	3
Major/Minor	Minor2	1	Major1	Λ	/lajor2	
Conflicting Flow All	72	14	15	0	-	0
Stage 1	14	-	-	-	-	-
Stage 2	58	-		-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-		-	-	-
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy		3.318	2 218	_	_	_
Pot Cap-1 Maneuver	932	1066	1603	_	_	_
Stage 1	1009	1000	1003	_		
Stage 2	965	_		-		-
Platoon blocked, %	700	-	-	_	-	_
	020	10//	1400	-	-	-
Mov Cap-1 Maneuver	920	1066	1603	-	-	-
Mov Cap-2 Maneuver	920	-	-	-	-	-
Stage 1	996	-	-	-	-	-
Stage 2	965	-	-	-	-	-
Approach	EB		NB		SB	
	8.5		4.1		0	
HCM Control Delay, s HCM LOS	6.5 A		4.1		U	
HOW LOS	А					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1603		1035	-	-
HCM Lane V/C Ratio		0.013		0.017	_	_
HCM Control Delay (s)		7.3	0	8.5	-	_
HCM Lane LOS		7.5 A	A	Α	_	_
HCM 95th %tile Q(veh)	0		0.1		
HOIVI 95111 %IIIE Q(VEN)	U	-	U. I	-	-

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EBK	INDL			SBR
Lane Configurations	Ă	0	2	ર્ ન	∱	0
Traffic Vol, veh/h	2	8	3	10	25	0
Future Vol, veh/h	2	8	3	10	25	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	9	3	11	27	0
	_	•	J		_,	· ·
				_		
	Minor2		Major1		/lajor2	
Conflicting Flow All	44	27	27	0	-	0
Stage 1	27	-	-	-	-	-
Stage 2	17	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	_
Pot Cap-1 Maneuver	967	1048	1587	_	-	_
Stage 1	996	-	-	_	_	_
Stage 2	1006	_	_	_	_	_
Platoon blocked, %	1000			_	_	_
Mov Cap-1 Maneuver	965	1048	1587	-	_	
Mov Cap-1 Maneuver	965	1040	1507	-	-	-
		-	-	-	-	-
Stage 1	994	-	-	-	-	-
Stage 2	1006	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		1.7		0	
			1.7		U	
HCM LOS	А					
Minor Lane/Major Mvm	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1587		1030	-	_
HCM Lane V/C Ratio		0.002		0.011	_	_
HCM Control Delay (s)		7.3	0	8.5	-	_
HCM Lane LOS		Α.	A	Α	_	_
HCM 95th %tile Q(veh))	0	-	0	_	
HOW /JULY JULIE CE(VEH)		U		U		

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	₽	
Traffic Vol, veh/h	2	3	1	10	30	1
Future Vol, veh/h	2	3	1	10	30	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	3	1	11	33	1
IVIVIIIL FIOW		3	ļ	- 11	აა	1
	Minor2		Major1	Λ	/lajor2	
Conflicting Flow All	47	34	34	0	-	0
Stage 1	34	-	-	-	-	-
Stage 2	13	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	2.218	-	-	-
Pot Cap-1 Maneuver	963	1039	1578	-	-	-
Stage 1	988	-	-	-	-	-
Stage 2	1010	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	962	1039	1578	_	_	_
Mov Cap-2 Maneuver	962	-	-	_	_	_
Stage 1	987	_	_	_	_	_
Stage 2	1010	_				
Stage 2	1010					
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0.7		0	
HCM LOS	Α					
Minor Lang/Major Mum	\t	MDI	MDT	EDI n1	CDT	CDD
Minor Lane/Major Mvm	ı	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1578		1007	-	-
HCM Lane V/C Ratio		0.001		0.005	-	-
HCM Control Delay (s)		7.3	0	8.6	-	-
		Λ	Λ	Λ	-	-
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	Α -	A 0		_

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7/	LDI	NDL	4	<u>361</u>	JUIN
Traffic Vol, veh/h		5	9	30	20	2
Future Vol, veh/h	1	5	9	30	20	2
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	5	10	33	22	2
IVIVIII I IOVV	•	3	10	33	22	
Major/Minor 1	Minor2		Major1	Λ	/lajor2	
Conflicting Flow All	76	23	24	0	-	0
Stage 1	23	-	-	-	-	-
Stage 2	53	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	_	-
Critical Hdwy Stg 1	5.42	-	-	_	_	_
Critical Hdwy Stg 2	5.42					_
Follow-up Hdwy		3.318	2 210	-	_	-
				-	-	-
Pot Cap-1 Maneuver	927	1054	1591	-	-	-
Stage 1	1000	-	-	-	-	-
Stage 2	970	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	921	1054	1591	-	-	-
Mov Cap-2 Maneuver	921	-	-	-	-	-
Stage 1	994	-	-	-	-	-
Stage 2	970	_	-	-	_	-
olago 2	7.0					
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		1.7		0	
HCM LOS	Α					
Minardana/Maiar Muni		NDI	NDT	EDI1	CDT	CDD
Minor Lane/Major Mvm	nt	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1591		1029	-	-
HCM Lane V/C Ratio		0.006	-	0.006	-	-
HCM Control Delay (s)		7.3	0	8.5	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh))	0	-	0	-	-

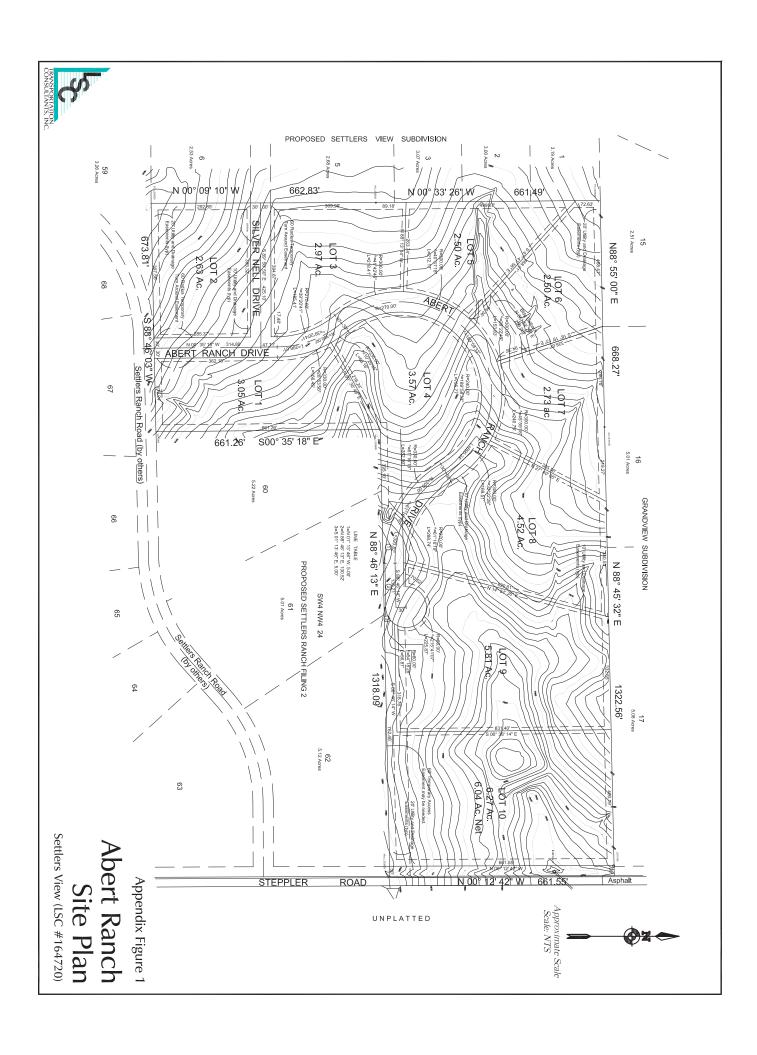
Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EBK	INRL			SBK
Lane Configurations	Y	2	า	ન	}	1
Traffic Vol, veh/h	1	3	3	40	25	2
Future Vol, veh/h	1	3	3	40	25	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	3	3	43	27	2
Major/Minor N	Minor		Major1		10ior2	
	Minor2		Major1		/lajor2	
Conflicting Flow All	77	28	29	0	-	0
Stage 1	28	-	-	-	-	-
Stage 2	49	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318		-	-	-
Pot Cap-1 Maneuver	926	1047	1584	-	-	-
Stage 1	995	-	-	-	-	-
Stage 2	973	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	924	1047	1584	-	-	-
Mov Cap-2 Maneuver	924	-	-	-	-	-
Stage 1	993	-	_	_	-	_
Stage 2	973	_	_	_	_	_
J. J.	713					
Approach	EB		NB		SB	
Approach HCM Control Delay, s			NB 0.5		SB 0	
Approach	EB					
Approach HCM Control Delay, s	EB 8.6					
Approach HCM Control Delay, s HCM LOS	8.6 A	NRI	0.5	FRI n1	0	SRD
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	8.6 A	NBL 1504	0.5 NBT	EBLn1	0 SBT	SBR
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	8.6 A	1584	0.5 NBT	1013	0 SBT	-
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	EB 8.6 A	1584 0.002	0.5 NBT -	1013 0.004	O SBT -	-
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	EB 8.6 A	1584 0.002 7.3	0.5 NBT 0	1013 0.004 8.6	0 SBT - -	- - -
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	EB 8.6 A	1584 0.002	0.5 NBT -	1013 0.004	O SBT -	-

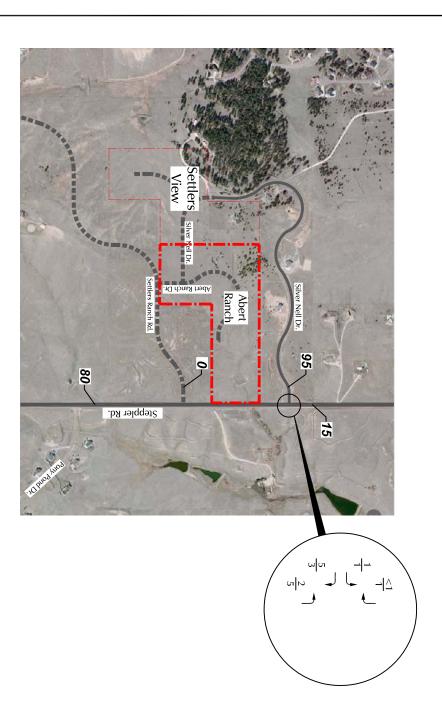
Intersection						
Int Delay, s/veh	2.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	LDIN		4	♣	JJIV
Traffic Vol, veh/h	3	8	3	11	25	1
Future Vol, veh/h	3	8	3	11	25	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storag		_	_	0	0	_
Grade, %	0	-	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
				2	2	2
Heavy Vehicles, %	2	2	2			
Mvmt Flow	3	9	3	12	27	1
Major/Minor	Minor2		Major1	Λ	/lajor2	
Conflicting Flow All	46	28	28	0	-	0
Stage 1	28	-	-	-	-	-
Stage 2	18	_	_	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	_	_	-
Critical Hdwy Stg 2	5.42	-	_	_	_	_
Follow-up Hdwy	3.518	3.318		_	_	_
Pot Cap-1 Maneuver	964	1047	1585	_	_	_
Stage 1	995	-	-	_	_	_
Stage 2	1005	_	_	_	_	_
Platoon blocked, %	1003				_	
Mov Cap-1 Maneuver	962	1047	1585	-		-
		1047	1363	-	-	
Mov Cap-2 Maneuver			-			-
Stage 1	993	-	-	-	-	-
Stage 2	1005	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			1.6		0	
HCM LOS	A		1.0			
	,,					
N 41 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ND	Not	EDL 1	ODT	000
Minor Lane/Major Mvi	nt	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1585		1022	-	-
HCM Lane V/C Ratio		0.002	-	0.012	-	-
HCM Control Delay (s	5)	7.3	0	8.6	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(vel	1)	0	-	0	-	-

Intersection						
Int Delay, s/veh	2.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	î»	
Traffic Vol, veh/h	3	10	3	10	30	1
Future Vol, veh/h	3	10	3	10	30	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	11	3	11	33	1
					- 00	
	Minor2		Major1		/lajor2	
Conflicting Flow All	51	34	34	0	-	0
Stage 1	34	-	-	-	-	-
Stage 2	17	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	958	1039	1578	-	-	-
Stage 1	988	-	-	-	-	-
Stage 2	1006	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	956	1039	1578	-	-	-
Mov Cap-2 Maneuver	956	-	-	_	_	_
Stage 1	986	_	_	_	_	_
Stage 2	1006					
Staye 2	1000	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		1.7		0	
HCM LOS	Α					
Minor Long /Martin Ma		NDI	NDT	EDL 4	CDT	CDD
Minor Lane/Major Mvm	11	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1578		1019	-	-
HCM Lane V/C Ratio		0.002		0.014	-	-
HCM Control Delay (s)		7.3	0	8.6	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Intersection						
Int Delay, s/veh	1.7					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	-	•	4	(0
Traffic Vol, veh/h	2	5	9	31	21	3
Future Vol, veh/h	2	5	9	31	21	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	5	10	34	23	3
N A a i a w/N Ai w a w	Alm c		1-1-1		Ania - C	
	Minor2		Major1		/lajor2	
Conflicting Flow All	79	25	26	0	-	0
Stage 1	25	-	-	-	-	-
Stage 2	54	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	924	1051	1588	-	-	-
Stage 1	998	-	-	-	-	-
Stage 2	969	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	918	1051	1588	-	-	-
Mov Cap-2 Maneuver	918	-	-	-	-	-
Stage 1	992	_	-	-	-	-
Stage 2	969	_	_	_	_	_
Jiago Z	,07					
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		1.6		0	
LICMIOC	Α					
HCM LOS						
HCIVI LUS						
	ıt	NRI	NRT	FRI n1	SRT	SRR
Minor Lane/Major Mvm	t	NBL 1500		EBLn1	SBT	SBR
Minor Lane/Major Mvm Capacity (veh/h)	ıt	1588	-	1009	-	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio		1588 0.006	-	1009 0.008	- -	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1588 0.006 7.3	- - 0	1009 0.008 8.6	- - -	- - -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio		1588 0.006	-	1009 0.008	- -	-

Intersection						
Int Delay, s/veh	1.3					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	-	,	4	4	0
Traffic Vol, veh/h	2	5	6	40	25	3
Future Vol, veh/h	2	5	6	40	25	3
Conflicting Peds, #/hr	0	0	0	_ 0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	5	7	43	27	3
Major/Minor I	Minor2		Major1	Λ.	/lajor2	
Conflicting Flow All	86	29	30	0	-	0
Stage 1	29	-	-	-	-	-
Stage 2	57	- (22	- 4.10	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318		-	-	-
Pot Cap-1 Maneuver	915	1046	1583	-	-	-
Stage 1	994	-	-	-	-	-
Stage 2	966	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	910	1046	1583	-	-	-
Mov Cap-2 Maneuver	910	-	-	-	-	-
Stage 1	989	-	-	-	-	-
Stage 2	966	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		1		0	
HCM LOS	А					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1583	-	1003	_	-
HCM Lane V/C Ratio		0.004		0.008	-	_
HCM Control Delay (s)		7.3	0	8.6	_	-
HCM Lane LOS		A	A	A	_	_
HCM 95th %tile Q(veh))	0	-	0	-	-
1.5W 75W 75W 76W 2(VCH	,	U		U		





Approximate Scale Scale: I"= 1,200'

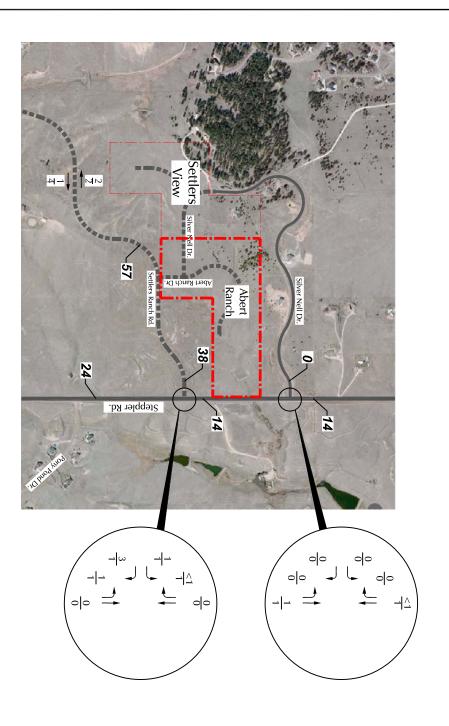
LEGEND:

XX = AM Weekday Peak—Hour Traffic (vehicles per hour)
XX = PM Weekday Peak—Hour Traffic (vehicles per hour)

XXX = Average Weekday Traffic (vehicles per day)

Appendix Figure 2

Abert Ranch Short-Term Traffic



Approximate Scale Scale: I'= 1,200'

LEGEND:

Appendix Figure 3

Long-Term Assignment of Abert Ranch Site-Generated Traffic