

LSC TRANSPORTATION CONSULTANTS, INC. 2504 East Pikes Peak Avenue, Suite 304 Colorado Springs, CO 80909 (719) 633-2868 FAX (719) 633-5430

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Struthers Ranch Tract B
Traffic Impact Study
(LSC #204110)
January 8, 2021

Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

Vens HCl

1-13-2021

Date

update

Struthers Ranch Tract B Traffic Impact Study

Prepared for:
Design and Development Consultants
T-Bone Construction
1310 Ford Street
Colorado Springs, Colorado 80915

Contact: Mr. Darin C. Weiss, AIA

JANUARY 8, 2021

LSC Transportation Consultants

Prepared by: Colleen Guillotte, P.E., PTOE, RSP

Reviewed by: Jeffrey C. Hodsdon, P.E.

LSC #204110

Please add the following: "PCD File No. VR-2101"



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January 8, 2021

Darin C Weiss, AIA
Design and Development Consultants
T-Bone Construction
1310 Ford Street
Colorado Springs, CO 80915

RE: Struthers Ranch Tract B
Traffic Impact Study
El Paso County, Colorado
LSC #204110

Dear Mr. Weiss:

LSC Transportation Consultants, Inc. has prepared this traffic impact study for the proposed Struthers Ranch Tract B development in El Paso County, Colorado. The development is planned to be located southeast of the intersection of Struthers Road/Struthers Ranch Road. The planned land use is for 19,740 square feet of retail, 5,200 square feet of office, and 5,200 square feet of restaurant. This report has been prepared for submittal to El Paso County.

REPORT CONTENTS

The preparation of this report included the following:

- Inventory of the existing adjacent and nearby area street and roadway system. This includes functional classifications, street widths, lane configurations, intersection traffic control, posted speed limits, pavement markings, intersection and access spacing, roadway and intersection alignments, auxiliary left- and right-turn lanes, intersection sight distances, etc.;
- A review of the proposed site land use and access locations;
- Morning and evening peak-hour traffic counts at the intersection of Struthers Road/Struthers Ranch Road;
- Estimates of short- and long-term background traffic volumes and total traffic (site traffic plus background traffic). Forecasts include buildout of adjacent proposed developments;
- Estimates of the daily and peak-hour trip generation for the proposed land use;
- The estimated directional distribution of site-generated vehicle trips on the study area street and roadway system;
- Projections of peak-hour site-generated turning-movement traffic volumes at the study area intersections, which include:

- Site access point intersections
- Struthers Road/Struthers Ranch Road
- Level of service (LOS) analysis at the study area intersections;
- Evaluation of the short-term and long-term projected intersection volumes to determine the potential need for a determine this deviation request with your client.

 Staff does not see an undue hardship to justify a deviation

Findings and recom request for access on Struthers Road. With access available at Struthers Ranch Rd, this deviation request will likely be denied.

PREVIOUS TRAFFIC STUD

Nearby properties have rebased on a single access from Struthers Ranch Road.

- Monument Ridge L
- Monument Ridge A Please note: If the applicant chooses to pursue the request,
- Cathedral Rock Chu staff encourages you to submit the deviation request form prior to the 2nd resubmittal. The determination to approve

This report is consistent v or deny the deviation impacts the traffic report. he background traffic volumes.

LAND USE AND ACCESS

Figure 1 shows the site location relative to the adjacent and nearby roadways. The site plan is shown in Figure 2. The Cathedral Rock Church is planned for the adjacent Tract A on the north side of Struthers Ranch Road. As shown in Figure 2, two access points are proposed: a full movement access onto Struthers Ranch Road across from the proposed access to Cathedral Rock Church, and a right-in/right-out access onto Struthers Road.

Struthers Ranch Tract B is proposed to include 19,740 square feet of retail, 5,200 square feet of office, and 5,200 square feet of restaurant.

INTERSECTION SIGHT DISTANC road per ECM 2.3.6.G.2.

Please revise 18 inches to 30 inches above the flow line of the adjacent road per FCM 2.3.6.G.2.

The required intersection sight distance for the intersection of Struthers Road/Struthers Ranch Road is 625 feet for passenger vehicles and 950 feet for combination trucks. This intersection would be able to meet this criterion provided the intersection line of sight "triangles" are kept free of site improvements (that would limit the line of sight needed to maintain ECM prescribed sight distance). Examples of site improvements include landscaping, monument signs, parking areas, berms, etc. Obstruction height to maintain passenger car line of sight is about 18 inches. Obstruction height to maintain truck line of sight is higher as the truck "driver's eye" is significantly higher than the "driver's eye" for a passenger vehicle. Depending on the site grading, the combination truck driver line of sight may pass over vehicles parked along the west side of the site. Line of sight exhibits are attached.

Update the required intersection sight distance and update Exhibit 1. Intersection sight distance applies to Struthers Road/Struthers Ranch Road intersection.

Intersection sight distance is ECM 2.3.6.G Table 2-21. Driveway access sight distance is ECM ECM 2.4.1.D.

The required sight distance for the right-in/right-out access point onto Struthers Road is 500 feet for passenger vehicles. The line of sight to arriving northbound through traffic on Struthers is over one quarter mile.

ROAD AND TRAFFIC CONDITIONS

Please revise to include a description of the required sight distance for the access point on Struthers Ranch Rd.

Area Roads

Figure 1 shows the streets in the vicinity of the site. The streets adjacent to the site are identified below followed by a brief description of each:

Struthers Road is a four-lane, median-divided road that extends north from North Gate Boulevard to the intersection of Baptist Road and Jackson Creek Parkway. Struthers Road is classified as a four-lane Urban Minor Arterial on the El Paso County Major Transportation Corridors Plan and has a speed limit of 45 miles per hour (mph) about 325 feet north of Air Garden Lane (adjacent to the south portion of the site). South of this point, the posted speed limit is 40 mph.

Struthers Ranch Road is classified as a local roadway. Struthers Ranch Road is an east/west road that extends from Struthers Road into the Struthers Ranch residential development. The roadway has a posted speed limit of 25 mph. The intersection with Struthers Road is unsignalized. The roadway at the intersection with Struthers Road is 32 feet wide, which only allows for a shared westbound left/right lane on the minor street approach. Struthers Road has a 340-foot southbound left-turn deceleration lane and a 260-foot northbound right-turn deceleration lane at the intersection with Struthers Ranch Road.

Traffic Volumes

Morning and evening peak-hour turning-movement traffic counts were conducted March 2020 at the intersection of Struthers Road/Struthers Ranch Road. The morning and evening peak-hour volumes are shown in Figure 3. It should be noted that the recorded southbound through traffic during the evening peak was significantly higher than historical counts. Counts recorded by LSC on Struthers Road between 2011 and 2019 have shown that the southbound traffic is typically lower than the northbound traffic during the evening peak hour. It is believed that there may have been an incident on I-25 in the southbound direction during the counts that caused a large number of vehicles to use Struthers Road as an alternate route. Traffic count reports are attached for reference. Can this statement be verified? Explain how the future volumes were calibrated/modified.

Crash History

Three years of crash data were collected at the intersection of Struthers Road/Struthers Ranch Road. There was only one crash during the study period. The only crash was a fixed object type crash that resulted in property damage only. No correctable crash patterns were identified.

TRIP GENERATION

Estimates of the vehicle trips projected to be generated by the proposed development have been made using the nationally published trip-generation rates from *Trip Generation*, 10th Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 1, below, presents a summary of the estimated site trip generation on a typical weekday. The detailed trip-generation estimate for the development, including ITE rates for the proposed land use, is presented in Table 3.

Approximately 2,986 total vehicle trips are projected to enter and exit the site at the access point ("driveway trips") on the average weekday during a 24-hour period. During the morning peak hour, approximately 170 vehicles would enter and 98 vehicles would exit the site. During the evening peak hour, approximately 101 vehicles would enter and 112 vehicles would exit the site.

The proposed development is projected to generate approximately 1,776 (new/non-pass-by or diverted) vehicle trips on the average weekday during a 24-hour period.

						, , , , , , , , , , , , , , , , , , , ,		,	
Analysis Period		Total Tr	ips	F	Pass-by Tri	ps	N	ew Trip	s
•	In	Out	Total	In	Out	Total	In	Out	Total
A.M. Peak Hour	170	98	268	53	53	106	117	45	162
P.M. Peak Hour	101	112	213	43	43	86	58	69	127
Daily/24-Hour	1,493	1,493	2,986	605	605	1,210	888	888	1,776

Table 1: Estimated External Site Vehicle-Trip Generation (Vehicles per Hour)

Internal Trips

Internal trips are trips that occur within the site and do not impact the external roadways. Because the site is planned to have multiple retail, office, and restaurant pads, some of the generated trips will be traveling within the site. Table 3 includes estimates of internal trip capture to account for trips generated within the site as well as non-motorized trips from adjacent and nearby developments.

Pass-by Trips

The trips generated by the site have also been aggregated by trip type to account for the pass-by phenomenon. A pass-by trip is one made by a motorist who would already be on an adjacent road regardless of the proposed development, but who stops in at the site while passing by. The pass-by motorist would then continue on his or her way to a final destination in the original direction. For purposes of this report, pass-by trips are trips by motorists already traveling through the intersection of Struthers Road/Struthers Ranch Road. Pass-by trips are shown in Table 3 and are based on *Trip Generation Handbook - An ITE Proposed Recommended Practice*, 3rd Edition, 2014 by ITE.

BACKGROUND TRAFFIC

Background volumes do not include projected traffic to be generated by the proposed development. As noted in the existing conditions section, the southbound through traffic during the evening peak was unusually high. This volume was modified in the background to reflect expected volumes.

Short-Term Background Traffic Volumes

Figure 4 shows the projected background traffic volumes. The projected volumes assume that the following nearby developments have been constructed:

- Monument Ridge Lots 7 & 8
- Cathedral Rock Church
- Monument Ridge Apartments

Long-Term Background Traffic Volumes

Figure 5 shows the projected 20-year background traffic volumes for the year 2040. The long-term scenario includes the developments in the short-term background. In addition, the long-term background traffic assumes a growth of approximately 2 percent per year of through traffic on Struthers Road.

TRIP DISTRIBUTION AND ASSIGNMENT

Describe the reason behind the 2% traffic growth.

Trip Directional Distribution

Estimation of the directional distribution of site-generated vehicle trips to the study area roads and intersections is a necessary component in determining the site's traffic impacts. Figure 6 shows the short-term and long-term directional distribution estimates for the proposed development. Estimates were based on the following factors: existing area development, the area roadway system, and the site's proposed land use.

Site-Generated Traffic

Site-generated traffic volumes at the study intersections have been calculated by applying the directional-distribution percentages estimated by LSC (from Figure 6) to the trip-generation estimates (from Table 1). Figure 7 shows the projected site-generated traffic volumes for the proposed development.

Short-Term Total Traffic Volumes

Figure 8 shows the sum of the short-term background traffic volumes (from Figure 4) and the site-generated peak-hour traffic volumes (shown in Figure 7). These volumes represent the projected short-term total traffic following construction of the development.

Please describe if MTCP was used for projected traffic volumes/conditions. Reference ECM B.2.2.C.

Long-Term Total Traffic Volumes

Figure 9 shows the projected 2040 total traffic volumes, which are the sum of 2040 background traffic volumes (from Figure 5) plus the site-generated traffic volumes (from Figure 7).

LEVEL OF SERVICE ANALYSIS

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 2 shows the level of service delay ranges for signalized and unsignalized intersections.

Table 2: Intersection Levels of Service Delay Ranges

	Signalized Intersections	Unsignalized Intersections
	Average Control Delay	Average Control Delay
Level of Service	(seconds per vehicle)	(seconds per vehicle) ⁽¹⁾
А	10.0 sec or less	10.0 sec or less
В	10.1-20.0 sec	10.1-15.0 sec
С	20.1-35.0 sec	15.1-25.0 sec
D	35.1-55.0 sec	25.1-35.0 sec
Е	55.1-80.0 sec	35.1-50.0 sec
F	80.1 sec or more	50.1 sec or more

⁽¹⁾ For unsignalized intersections if V/C ratio is greater than 1.0 the level of service is LOS F regardless of the projected average control delay per

The intersections of Struthers Road/Struthers Ranch Road, as well as the site access points, have been analyzed to determine the projected control delay and corresponding levels of service for turning movements. Figure 3 provides the existing levels of service. Figure 4 and Figure 5 provide the background levels of service for the short-term and long-term scenarios, respectively. Figure 8 and Figure 9 provide the levels of service of the short-term and long-term total traffic scenarios, respectively.

Struthers Road/Struthers Ranch Road

The yielding turning movements at the unsignalized intersection of Struthers Road/Struthers Ranch Road currently operate at LOS C or better during both the morning and evening peak hours. In the short-term future, the yielding turning movements are forecast to continue to operate at LOS C or better during both peak hours with and without the proposed development.

In the long-term future, the yielding turning movements are also forecast to continue to operate at LOS C or better. This intersection is not anticipated to meet signal warrants.

Site Accesses

In all scenarios, the yielding turning movements operate at LOS B or better during both peak hours.

AUXILIARY TURN LANES

As mentioned previously, there is a 340-foot southbound left-turn deceleration lane at the intersection of Struthers Road/Struthers Ranch Road. This lane meets the ECM auxiliary lane criteria and does not need to be modified with the development.

There is a 400-foot northbound right-turn deceleration lane (combined lane plus taper length) at this intersection of Struthers Road/Struthers Ranch Road. This auxiliary lane, although it exists, is not currently required per the ECM and is not expected to be required in the future with added site-generated traffic. The turning volume threshold could potentially be met on Sunday mornings with addition of future church traffic. The turn lane already exists and is about 400 feet (lane plus taper). The ECM requirement is 435 feet.

The right-in/right-out access on Struthers Road is anticipated to require a right-turn deceleration lane. This criterion calls for 370-foot-long deceleration distance (lane plus taper) based on a 45-mph design speed and a 435-foot-long deceleration distance (lane plus taper) for a 50-mph design speed. The speed limit currently changes just south of the access location. LSC recommends an approximately 200-foot-long lane plus a 75-foot-long reverse curve bay taper. This would likely allow the lane to be installed given the limited space adjacent to the developed property to the south. No acceleration lanes are required on Struthers Road.

Although not anticipated to be required based on projected volumes or levels of service, it is recommended that right-of-way be reserved in case Struthers Ranch Road needs minor widening in the future to allow for separate right- and left-turn lanes in the westbound direction.

VEHICLE QUEUING

At the intersection of Struthers Road/Struthers Ranch Road, there are 250 feet available for vehicle queueing to the east prior to the site access. The 95th percentile queue length for the westbound approach at the intersection is anticipated to be 100 feet, which will not impact the site access. If Struthers Ranch Road were widened in the future to provide separate right- and left-turn lanes on the westbound approach the intersection, then the 95th percentile queue for the westbound left is forecast to be 75 feet, which also would not impact the access to the east.

PEDESTRIAN AND BICYCLE ACCOMMODATION

A sidewalk exists along Struthers Road adjacent to the site. However, there are currently no sidewalks along Struthers Ranch Road adjacent to the site. It is recommended that a sidewalk be

constructed adjacent to the site on Struthers Ranch Road.

There are no bike lanes on Struthers Road and the roadway is not planned to have bike lanes. However, there are sections of Struthers Road that have paved outside shoulders to accommodate cyclists.

COUNTY ROAD IMPROVEMENT FEE PROGRAM

Transportation Impact Fees

Per ECM Appendix B: State what the current applicable Transportation Impact Fees are and what option the developer will be selecting for payment.

The applicant intends to opt out of the PID options and will pay the full fee amount at the time of building permit. The current "full-fee" is \$4,958 per 1,000 square feet of building floor area. The total fee amount for the 30,140 square foot of commercial buildings is \$149,434.

Reimbursable MTCP Improvements

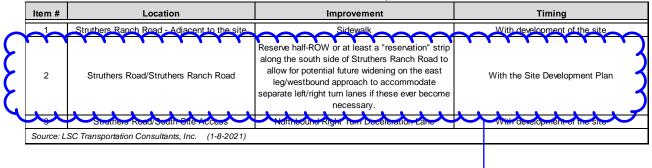
There are no apparent reimbursable improvements programmed in the MTCP in the general vicinity of this site.

FINDINGS AND CONCLUSIONS

- The site is projected to generate approximately 2,986 external vehicle trips on the average weekday. Approximately half of this traffic is expected to be pass-by traffic.
- During the morning peak hour, approximately 170 vehicles would enter and 98 vehicles would exit the site. During the evening peak hour, approximately 101 vehicles would enter and 112 vehicles would exit the site at the access points.
- The site improvements, landscaping, signage etc. will need to accommodate the driver sight distance lines of sight necessary to meet the prescribed intersection sight distance at Struthers Road/Struthers Rangh Road. Please refer to the Sight Distance section for details.
- Turning movements at the site accesses are projected to operate at acceptable levels of service in all scenarios.
- The westbound approach at the intersection of Struthers Road/Struthers Ranch Road operates at LOS C or better in all scenarios.
- The 95th percentile queues at all study intersection are not projected to impact adjacent intersections.
- See Table 4 for a summary of recommended improvements.

Update the to identify that a sight distance easement where the line of sight encroaches into the private property

Table 4: Recommended Improvements



Please contact me if you have any questions regarding thi the resubmittal.

Respectfully Submitted,

Will be reviewed on the resubmittal. Recommendation may be different with the removal of RIRO.

LSC TRANSPORTATION CONSULTANTS, INC.

By Colleen Guillotte, P.E., PTOE, RSP Project Manager

CRG:JCH:jas

Enclosures: Table 3

Figures 1-9

Line of Sight Exhibits Traffic Count Reports Level of Service Reports

Please provide a list of references used to create this study

Table 3



Show the Morning/Afternoon new external trip generated

Table 3: Detailed Trip Generation Estimate

Land	Land	- Trip	T Average	Fotal Trij Mori	ps Gener	ated After	noon	_	Int	ernal Tr Morr	ips Gen		noon	Ex	ternal 1	Trips Ger		noon	ζ	New External Trips Generated Average
Use	Use	Generation	Weekday		Hour		Hour	Internal	Weekday	Peak	•		Hour	Weekday		Hour		Hour	Pass-By	Weekday
Code	Description	Units	Traffic	In	Out	In	Out	Trips	Traffic	In	Out	In	Out	Traffic	In	Out	ln	Out	Trips (3)	Traffic
820	Shopping Center	19.74 KSF ⁽⁴⁾	1,995	100	61	79	85	21%	423	7	8	32	22	1,571	93	53	47	63	34%	1,037
712	Small Office Building	5.2 KSF	84	8	2	4	9	26%	22	1	1	2	2	62	7	1	2	7	€ %	62
933	Fast Food w/o Drive-Thru	5.2 KSF	1,800	78	52	74	74	25%	447	9	8	21	31	1,353	69	44	53	43	50%	677
		Total Trip Generation Estimate	3,879	187	115	156	167	_	892	17	17	55	55	2,987	170	98	101	112	۲	1,776

Notes:

- (1) Source: "Trip Generation, 10th Edition, 2017" by the Institute of Transportation Engineers (ITE)
- (2) NCHRP 684 Internal Trip Capture Estimate Tool Sheets
- (3) Source: "Trip Generation Handbook An ITE Proposed Recommended Practice, Third Edition September 2017" by ITE
- (4) KSF = one thousand square feet of floor space

Source: LSC Transportation Consultants, Inc.

1-8-2021

include the reference in the appendix. The percentages seems high.

Figures



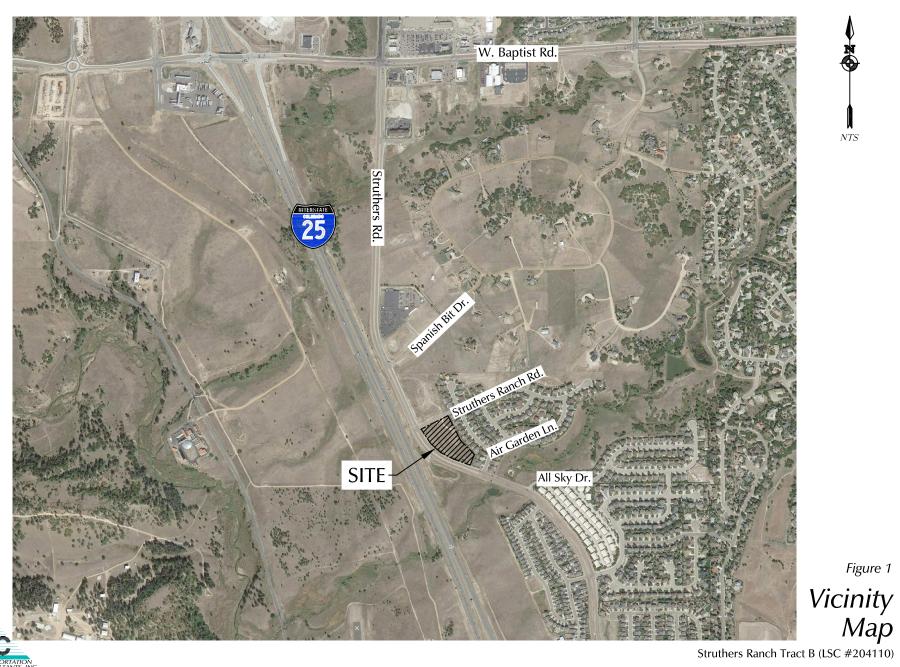
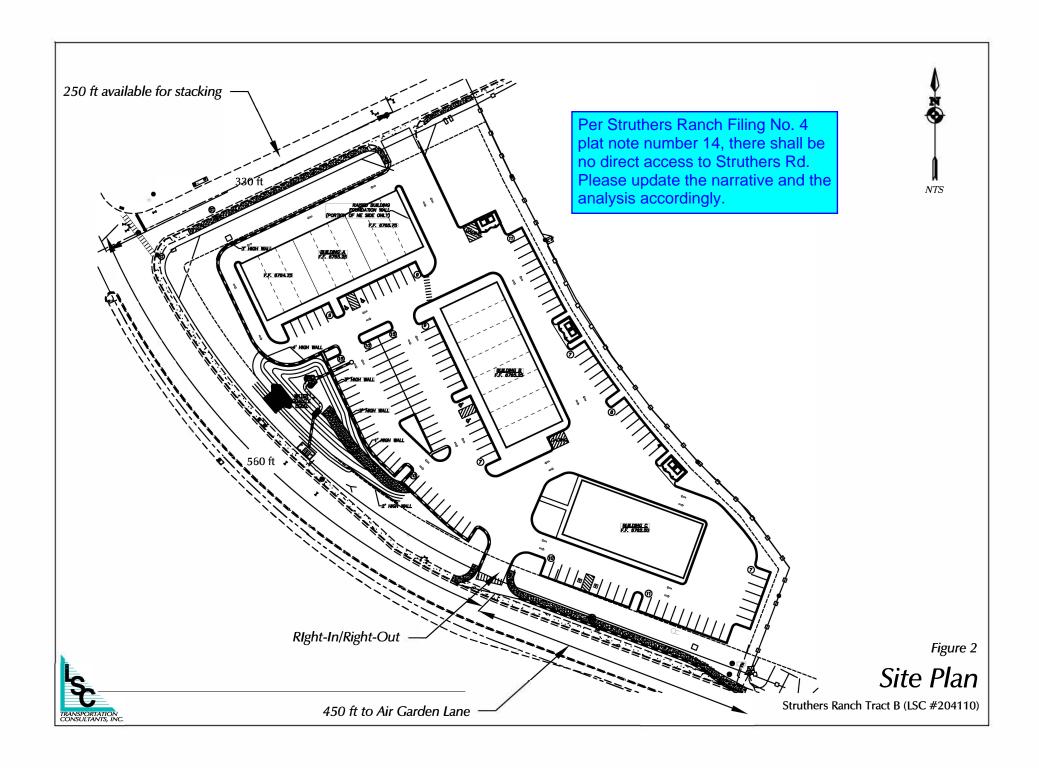
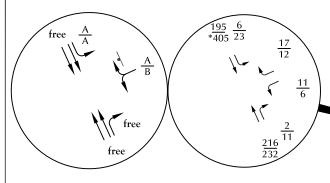


Figure 1

NTS







*The SB PM volume recorded was high compared to historical counts. It is assumed this was due to construction or an incident on I-25. Future volumes have been modified accordingly.



LEGEND:

= Stop Sign

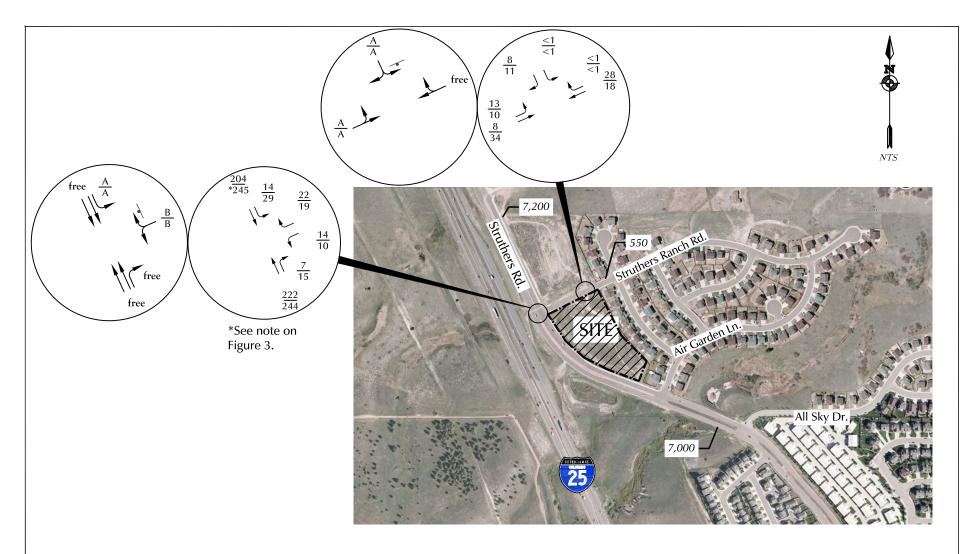
 $\frac{XX}{XX} = \begin{array}{c} \frac{AM}{XX} = & \frac{AM}{XX} & \frac{AM}{XX}$

 $X_{x}XXX$ = Average Weekday Traffic (vehicles per day)

Figure 3

Existing Traffic Conditions





LEGEND:

• = Stop Sign

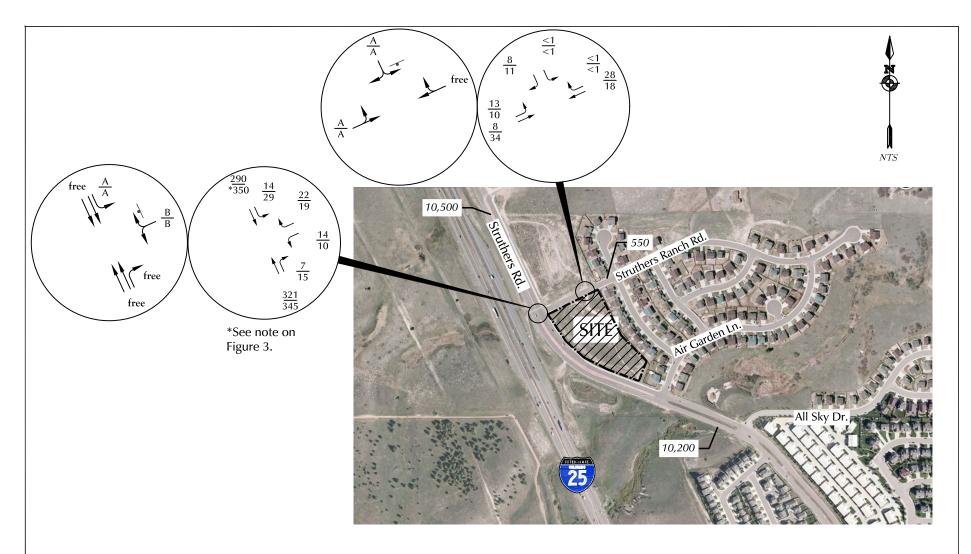
 $\frac{XX}{XX} = \begin{array}{c} \frac{AM}{XX} = & \frac{AM}{XX} & \frac{AM}{XX}$

 $X_{x}XXX$ = Average Weekday Traffic (vehicles per day)



Short-Term Background Traffic Conditions





LEGEND:

= Stop Sign

 $\frac{XX}{XX} = \frac{AM}{PM} \frac{\text{Weekday Peak-Hour Traffic (vehicles per hour)}}{PM} \frac{A}{PM} \frac{A}{PM} \frac{AM}{PM} \frac{Peak-Hour Peak-Hour Level of Service}{PM} \frac{AM}{PM} \frac{Peak-Hour Peak-Hour Level of Service}{PM} \frac{AM}{PM} \frac{Peak-Hour Peak-Hour Level of Service}{PM} \frac{Peak-Hour Peak-Hour Level of Service}{PM} \frac{Peak-Hour Peak-Hour Peak-Hour$

 $X_{x}XXX$ = Average Weekday Traffic (vehicles per day)



Long-Term Background Traffic Conditions







Figure 6

Directional Distribution of Site-Generated Traffic

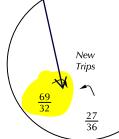
Struthers Ranch Tract B (LSC #204110)

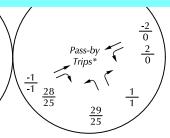
LEGEND:

 $rac{XX}{XX} = rac{AM}{PM} rac{Percent}{Pm} rac{Directional}{Distribution} rac{Distribution}{Distribution}$

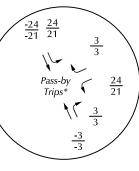
This meets the criteria for right turn lane. Update the Auxiliary Turn Lane section and the conclusion/recommendation section to discuss.

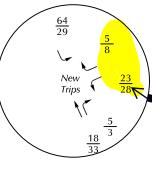
Per Struthers Ranch Filing No. 4 plat note number 14, there shall be no direct access to Struthers Rd. Please update the narrative and the analysis accordingly.

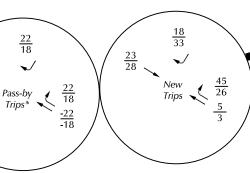












Without the RIRO this it seems separate left turn/right turn lanes are warranted. Provide auxiliary turn lane analysis and recommendation

* Pass-by trips are existing trips on the adjacent roadways that stop at the site on the way to their planned destination. These trips are not new to the roadway, but are new turns at the access points.

LEGEND:



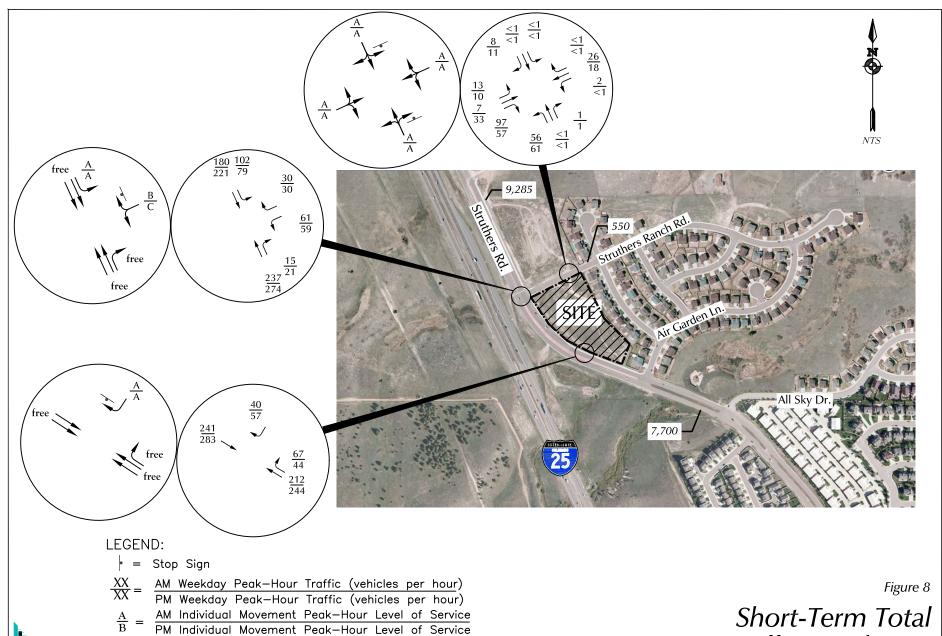
 $\frac{XX}{XX} = \frac{\text{AM Weekday Peak-Hour Traffic (vehicles per hour)}}{\text{PM Weekday Peak-Hour Traffic (vehicles per hour)}}$ $X_{,XXX} = \text{Average Weekday Traffic (vehicles per day)}$

Figure 7

Site-Generated Traffic Volumes

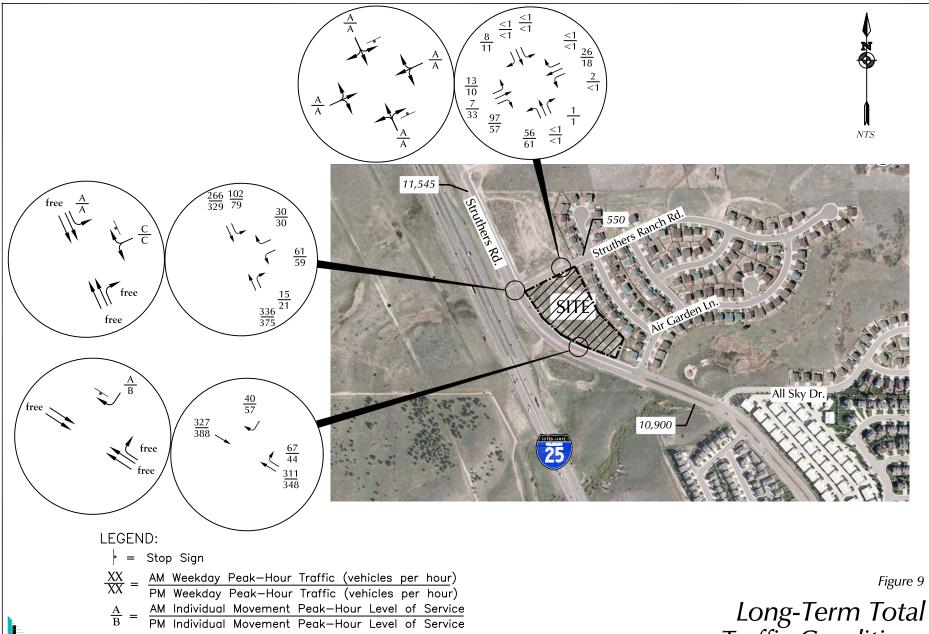
Struthers Ranch Tract B (LSC #204110)

All Sky Dr



 $X_{x}XXX = Average Weekday Traffic (vehicles per day)$

Short-Term Total **Traffic Conditions**



 $X_{x}XXX$ = Average Weekday Traffic (vehicles per day)

Long-Term Total Traffic Conditions

Line of Sight Exhibits







Traffic Counts



545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd AM

Site Code : 00204110 Start Date : 3/3/2020

Page No : 1

Groups Printed-Unshifted

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		S	<u>outhbou</u>	ınd				Vestbou	ınd			N	orthbou	ınd			Ę	stboun	<u>ıd</u>		
Start Time	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	T	R	U	App. Total	L	Т	R	U	App. Total	Int. Total
06:30 AM	0	23	0	0	23	2	0	3	0	5	0	22	0	0	22	0	0	0	0	0	50
06:45 AM	0	22	0	0	22	2	0	3	0	5	0	27	0	0	27	0	0	0	0	0	54
Total	0	45	0	0	45	4	0	6	0	10	0	49	0	0	49	0	0	0	0	0	104
07:00 AM	1	36	0	0	37	4	0	4	0	8	0	32	1	0	33	0	0	0	0	0	78
07:15 AM	1	44	0	0	45	4	0	3	0	7	0	54	0	0	54	0	0	0	0	0	106
07:30 AM	0	51	0	0	51	1	0	7	0	8	0	47	0	0	47	0	0	0	0	0	106
07:45 AM	1	56	0	0	57	3	0	4	0	7	0	60	0	0	60	0	0	0	0	0	124
Total	3	187	0	0	190	12	0	18	0	30	0	193	1	0	194	0	0	0	0	0	414
08:00 AM	4	47	0	0	51	6	0	1	0	7	0	51	1	0	52	0	0	0	0	0	110
08:15 AM	1	41	0	0	42	1	0	5	0	6	0	58	1	0	59	0	0	0	0	0	107
Grand Total	8	320	0	0	328	23	0	30	0	53	0	351	3	0	354	0	0	0	0	0	735
Apprch %	2.4	97.6	0	0		43.4	0	56.6	0		0	99.2	8.0	0		0	0	0	0		
Total %	1.1	43.5	0	0	44.6	3.1	0	4.1	0	7.2	0	47.8	0.4	0	48.2	0	0	0	0	0	

545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd AM

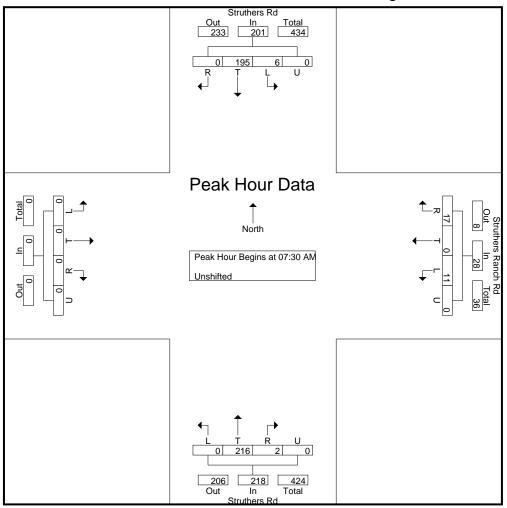
Site Code : 00204110 Start Date : 3/3/2020

		St	ruthers	Rd		Struthers Ranch Rd						St	ruthers	Rd							
		Sc	outhbou	nd			W	estbour	nd			No	orthbou	nd			E	astbour	nd		
Start Time	L	Т	R	U	App. Total	L	Т	R	C	App. Total	L	Т	R	U	App. Total	L	Т	R	U	App. Total	Int. Total
Peak Hour Anal	ysis Fro	m 6:30	00 AM t	o 8:15:	:00 AM - P	eak 1 of	1														
Peak Hour for En	ntire Inte	rsection	Begins a	at 7:30:	:00 AM																
7:30:00 AM	0	51	0	0	51	1	0	7	0	8	0	47	0	0	47	0	0	0	0	0	106
7:45:00 AM	1	56	0	0	57	3	0	4	0	7	0	60	0	0	60	0	0	0	0	0	124
8:00:00 AM	4	47	0	0	51	6	0	1	0	7	0	51	1	0	52	0	0	0	0	0	110
8:15:00 AM	1	41	0	0	42	1	0	5	0	6	0	58	1	0	59	0	0	0	0	0	107
Total Volume	6	195	0	0	201	11	0	17	0	28	0	216	2	0	218	0	0	0	0	0	447
% App. Total	3	97	0	0		39.3	0	60.7	0		0	99.1	0.9	0		0	0	0	0		
PHF	.375	.871	.000	.000	.882	.458	.000	.607	.000	.875	.000	.900	.500	.000	.908	.000	.000	.000	.000	.000	.901

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File Name: Struthers Rd - Struthers Ranch Rd AM

Site Code : 00204110 Start Date : 3/3/2020



545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd AM

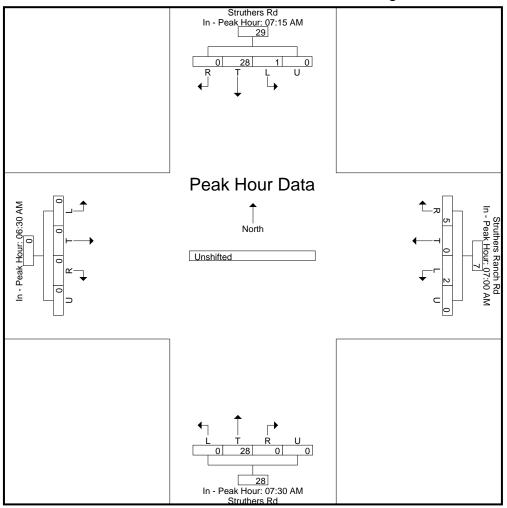
Site Code : 00204110 Start Date : 3/3/2020

	Struthers Rd Struthers Ranch Rd									St	ruthers	Rd									
		So	uthbou	nd			W	estbour	nd			No	orthbou	nd			E	astbour	nd		
Start Time	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	Т	R	U	App. Total	Int. Tota
Peak Hour Ana	lysis Fro	m 6:30:	00 AM t	o 8:15:	00 AM - P	eak 1 of	1	•					·	•							•
Peak Hour for E	ach Appr	oach Be	gins at:																		_
	7:15:00 AM					7:00:00 AM					7:30:00 AM					6:30:00 AM					
+0 mins.	1	44	0	0	45	4	0	4	0	8	0	47	0	0	47	0	0	0	0	0	
+5 mins.	0	51	0	0	51	4	0	3	0	7	0	60	0	0	60	0	0	0	0	0	
+10 mins.	1	56	0	0	57	1	0	7	0	8	0	51	1	0	52	0	0	0	0	0	
+15 mins.	4	47	0	0	51	3	0	4	0	7	0	58	1	0	59	0	0	0	0	0	
Total Volume	6	198	0	0	204	12	0	18	0	30	0	216	2	0	218	0	0	0	0	0	
% App. Total	2.9	97.1	0	0		40	0	60	0		0	99.1	0.9	0		0	0	0	0		
PHF	.375	.884	.000	.000	.895	.750	.000	.643	.000	.938	.000	.900	.500	.000	.908	.000	.000	.000	.000	.000	

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File Name: Struthers Rd - Struthers Ranch Rd AM

Site Code : 00204110 Start Date : 3/3/2020



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File Name: Struthers Rd - Struthers Ranch Rd PM

Site Code : 00204110 Start Date : 3/3/2020

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Groups Printed- Unshifted

			truthers					ners Ra				_	truthers				-	- 11			
_		S	outhbou	ınd				/estbou	nd			N	orthbo	und			<u> </u>	<u>istboun</u>	d		
Start	L	т	R	U	App. Total	L	т	R	U	App. Total	L	т	R	U	App. Total	L	т	R	U	App. Total	Int. Total
Time		_				_								_			-				
04:00 PM	4	106	0	0	110	0	0	3	0	3	0	56	3	0	59	0	0	0	0	0	172
04:15 PM	4	98	0	0	102	1	0	3	0	4	0	52	3	0	55	0	0	0	0	0	161
04:30 PM	4	82	0	0	86	1	0	0	0	1	0	58	2	0	60	0	0	0	0	0	147
04:45 PM	6	75	0	0	81	0	0	3	0	3	0	61	2	0	63	0	0	0	0	0	147
Total	18	361	0	0	379	2	0	9	0	11	0	227	10	0	237	0	0	0	0	0	627
05:00 PM	6	120	0	0	126	1	0	1	0	2	0	61	2	0	63	0	0	0	0	0	191
05:15 PM	5	101	0	0	106	3	0	6	0	9	0	53	4	0	57	0	0	0	0	0	172
05:30 PM	6	109	0	0	115	2	0	2	0	4	0	57	3	0	60	0	0	0	0	0	179
05:45 PM	7	77	0	0	84	0	0	1	0	1	0	51	1	0	52	0	0	0	0	0	137
Total	24	407	0	0	431	6	0	10	0	16	0	222	10	0	232	0	0	0	0	0	679
Grand Total	42	768	0	0	810	8	0	19	0	27	0	449	20	0	469	0	0	0	0	0	1306
Apprch %	5.2	94.8	0	0		29.6	0	70.4	0		0	95.7	4.3	0		0	0	0	0		
Total %	3.2	58.8	0	0	62	0.6	0	1.5	0	2.1	0	34.4	1.5	0	35.9	0	0	0	0	0	

545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd PM

Site Code : 00204110 Start Date : 3/3/2020

	Struthers Rd						Struth	ers Ran	ch Rd			St	ruthers	Rd							
		Sc	outhbou	nd			W	estbour/	nd			No	orthbou	nd			E	astbour	nd		
Start Time	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	Т	R	U	App. Total	Int. Total
Peak Hour Anal	ysis Fro	m 4:00:	00 PM t	o 5:45:	00 PM - P	eak 1 of	1														
Peak Hour for En	ntire Inte	rsection	Begins a	at 4:45:	00 PM																
4:45:00 PM	6	75	0	0	81	0	0	3	0	3	0	61	2	0	63	0	0	0	0	0	147
5:00:00 PM	6	120	0	0	126	1	0	1	0	2	0	61	2	0	63	0	0	0	0	0	191
5:15:00 PM	5	101	0	0	106	3	0	6	0	9	0	53	4	0	57	0	0	0	0	0	172
5:30:00 PM	6	109	0	0	115	2	0	2	0	4	0	57	3	0	60	0	0	0	0	0	179
Total Volume	23	405	0	0	428	6	0	12	0	18	0	232	11	0	243	0	0	0	0	0	689
% App. Total	5.4	94.6	0	0		33.3	0	66.7	0		0	95.5	4.5	0		0	0	0	0		
PHF	.958	.844	.000	.000	.849	.500	.000	.500	.000	.500	.000	.951	.688	.000	.964	.000	.000	.000	.000	.000	.902

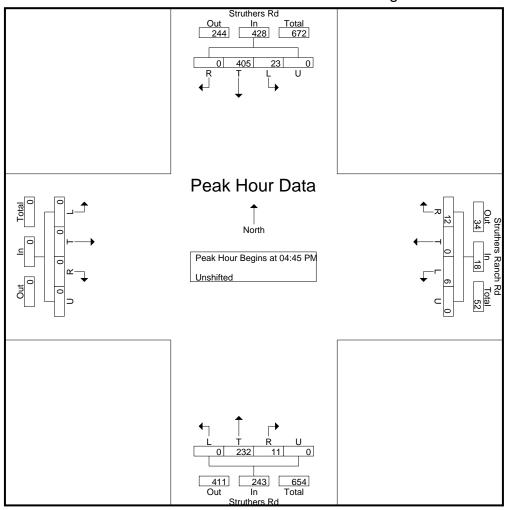
LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd PM

Site Code : 00204110 Start Date : 3/3/2020

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File Name: Struthers Rd - Struthers Ranch Rd PM

Site Code : 00204110 Start Date : 3/3/2020

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			ruthers outhbou					ers Ran estbour					ruthers orthbou				E	astbour	nd		
Start Time	L	Т	R	U	App. Total	L	Т	R	U	App. Total	L	Т	R	U	pp. Total	L	Т	R	U	App. Total	Int. Tot
Peak Hour Ana	lysis Fro	m 4:00:	00 PM t	o 5:45:0	00 PM - P	eak 1 of	1														
Peak Hour for E	ach Appr	oach Be	gins at:																		
	5:00:00 PM					4:45:00 PM					4:30:00 PM					4:00:00 PM					1
+0 mins.	6	120	0	0	126	0	0	3	0	3	0	58	2	0	60	0	0	0	0	0	1
+5 mins.	5	101	0	0	106	1	0	1	0	2	0	61	2	0	63	0	0	0	0	0	
+10 mins.	6	109	0	0	115	3	0	6	0	9	0	61	2	0	63	0	0	0	0	0	1
+15 mins.	7	77	0	0	84	2	0	2	0	4	0	53	4	0	57	0	0	0	0	0]
Total Volume	24	407	0	0	431	6	0	12	0	18	0	233	10	0	243	0	0	0	0	0	1
% App. Total	5.6	94.4	0	0		33.3	0	66.7	0		0	95.9	4.1	0		0	0	0	0		1
PHF	.857	.848	.000	.000	.855	.500	.000	.500	.000	.500	.000	.955	.625	.000	.964	.000	.000	.000	.000	.000	

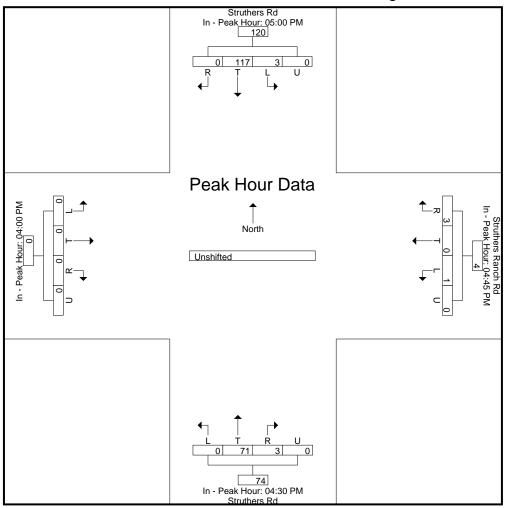
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545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905 719-633-2868

File Name: Struthers Rd - Struthers Ranch Rd PM

Site Code : 00204110 Start Date : 3/3/2020

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Levels of Service



Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		† †	7	ሻ	† †
Traffic Vol, veh/h	11	17	216	2	6	195
Future Vol, veh/h	11	17	216	2	6	195
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	88	88	91	91	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	19	237	2	7	222
		.0		_	•	
	Minor1		//ajor1		Major2	
Conflicting Flow All	362	119	0	0	239	0
Stage 1	237	-	-	-	-	-
Stage 2	125	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	_	-	_	_	_
Critical Hdwy Stg 2	5.84	_	_	_	_	_
Follow-up Hdwy	3.52	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	610	910	_	_	1325	_
Stage 1	780	-	_	_	-	_
Stage 2	887	_	_	_	_	_
Platoon blocked, %	007	_	_	_	_	_
	607	910		_	1325	-
Mov Cap-1 Maneuver			-	-		
Mov Cap-2 Maneuver	607	-	-	-	-	-
Stage 1	780	-	-	-	-	-
Stage 2	883	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.9		0		0.2	
HCM LOS	Α		U		0.2	
TIOWI LOO						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	761	1325	_
HCM Lane V/C Ratio		_	-	0.042		-
HCM Control Delay (s)		-	-	9.9	7.7	-
HCM Lane LOS		_	-	Α	Α	-
HCM 95th %tile Q(veh))	_	_	0.1	0	_
TOW JOHN JOHN Q(VEI)				J. 1	U	

Intersection						
Int Delay, s/veh	0.7					
-		14/55			0-1	05-
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		^	7	ሻ	† †
Traffic Vol, veh/h	6	12	232	11	23	405
Future Vol, veh/h	6	12	232	11	23	405
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	92	92	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	24	252	12	27	476
Mainu/Minnu	1:1		1-:1		4-:0	
	linor1		//ajor1		Major2	
Conflicting Flow All	544	126	0	0	264	0
Stage 1	252	-	-	-	-	-
Stage 2	292	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	469	901	-	-	1297	-
Stage 1	767	-	-	-	-	-
Stage 2	732	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	459	901	-	-	1297	-
Mov Cap-2 Maneuver	459	-	-	-	-	-
Stage 1	767	-	-	-	-	-
Stage 2	717	_	_	_	_	_
	145					
Approach	WB		NB		SB	
HCM Control Delay, s	10.6		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		1101	אוטויי	682	1297	051
HCM Lane V/C Ratio			-	0.053		
		-				-
HCM Lang LOS			-	10.6	7.8	-
HCM Of the Political Columbia		-	-	В	Α	-
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Intersection						
Int Delay, s/veh	1					
	WDL	WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		00	† †	7	<u>ነ</u>	↑ ↑
Traffic Vol, veh/h	14	22	222	7	14	204
Future Vol, veh/h	14	22	222	7	14	204
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	- 04	-	0
Peak Hour Factor	88	88	91	91	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	25	244	8	16	232
Major/Minor N	Minor1	N	//ajor1		Major2	
Conflicting Flow All	392	122	0	0	252	0
Stage 1	244	-	-	-	-	-
Stage 2	148	-	-	-	-	_
Critical Hdwy	6.84	6.94	_	-	4.14	-
Critical Hdwy Stg 1	5.84	-	_	_	_	_
Critical Hdwy Stg 2	5.84	-	-	_	-	-
Follow-up Hdwy	3.52	3.32	_	-	2.22	_
Pot Cap-1 Maneuver	585	906	_	_	1310	-
Stage 1	774		_	_	-	_
Stage 2	864	_	_	_	_	_
Platoon blocked, %	- 00 T		_	_		_
Mov Cap-1 Maneuver	578	906	_	_	1310	
Mov Cap-1 Maneuver	578	300	_		1010	_
Stage 1	774	-	-	-	-	<u>-</u>
•	854	-	-	-	•	-
Stage 2	004	-	_	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.1		0		0.5	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-		1310	-
HCM Lane V/C Ratio		_		0.055		_
HCM Control Delay (s)			_		7.8	_
HCM Lane LOS		_	_	В	Α.	_
HCM 95th %tile Q(veh)		_		0.2	0	<u>-</u>
			-	U.Z	U	_

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	8	0	0	28	0	0	0	0	0	0	8
Future Vol, veh/h	13	8	0	0	28	0	0	0	0	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	10	0	0	36	0	0	0	0	0	0	10
Major/Minor	Major1		N	Major2			Minor1			Minor2		
	36	0		<u>viajoi 2</u> 10	^	0		80	10		80	36
Conflicting Flow All		0	0	10	0	U	85			80 36		
Stage 1	-	-	-	-	-	-	44 41	44	-	36 44	36 44	-
Stage 2	4 4 9	-	-	1 10	-	-		36	6 22			6 22
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	- 0.040	-	-	0.040	-	-	6.12	5.52	-	6.12	5.52	2 240
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1575	-	-	1610	-	-	901	810	1071	908	810	1037
Stage 1	-	-	-	-	-	-	970	858	-	980	865	-
Stage 2	-	-	-	-	-	-	974	865	-	970	858	-
Platoon blocked, %	4575	-	-	1010	-	-	005	004	4074	004	004	4007
Mov Cap-1 Maneuver	1575	-	-	1610	-	-	885	801	1071	901	801	1037
Mov Cap-2 Maneuver	-	-	-	-	-	-	885	801	-	901	801	-
Stage 1	-	-	-	-	-	-	959	849	-	969	865	-
Stage 2	-	-	-	-	-	-	964	865	-	959	849	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.5			0			0			8.5		
HCM LOS							A			A		
							, ,			,,		
Minor Lane/Major Mvn	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBI n1			
Capacity (veh/h)		-		-	-	1610	-	-				
HCM Lane V/C Ratio			0.011	_	-	1010	-	-	0.01			
HCM Control Delay (s)		0	7.3	0	-	0			8.5			
HCM Lane LOS												
	1	Α	A 0	Α	-	A	-	-	A 0			
HCM 95th %tile Q(veh)	-	U	-	-	0	-	-	U			

Intersection						
Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		^	7	ሻ	^
Traffic Vol, veh/h	10	19	244	15	29	245
Future Vol, veh/h	10	19	244	15	29	245
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	_	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage		_	0		_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	50	50	92	92	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	20	38	265	16	34	288
					•	
NA ' /NA'	\ A' \ A					
	Minor1		//ajor1		Major2	
Conflicting Flow All	477	133	0	0	281	0
Stage 1	265	-	-	-	-	-
Stage 2	212	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	517	892	-	-	1278	-
Stage 1	755	-	-	-	-	-
Stage 2	803	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	503	892	-	-	1278	-
Mov Cap-2 Maneuver	503	-	-	-	-	-
Stage 1	755	-	-	-	-	-
Stage 2	781	-	-	-	-	-
Approach	WB		NB		SB	
	10.6		0		0.8	
HCM Control Delay, s HCM LOS	10.0		U		0.0	
HCIVI LOS	D					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	704	1278	-
HCM Lane V/C Ratio		-	-	0.082	0.027	-
HCM Control Delay (s)		-	-	10.6	7.9	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh))	-	-	0.3	0.1	-
.,						

latana atian												
Intersection	2.3											
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	10	34	0	0	18	0	0	0	0	0	0	11
Future Vol, veh/h	10	34	0	0	18	0	0	0	0	0	0	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	44	0	0	23	0	0	0	0	0	0	14
Major/Minor I	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	23	0	0	44	0	0	100	93	44	93	93	23
Stage 1	-	-	-	-	-	-	70	70	-	23	23	-
Stage 2	_	_	_	_	<u>-</u>	<u>-</u>	30	23	<u>-</u>	70	70	_
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	_	_		_	_	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	_	_	_	_	_	6.12	5.52	_	6.12	5.52	_
Follow-up Hdwy	2.218	_	_	2.218	_	_	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1592	_	_	1564	-	_	881	797	1026	891	797	1054
Stage 1	-	_	_	-	_	_	940	837	-	995	876	- 30 7
Stage 2	-	-	-	-	-	-	987	876	-	940	837	-
Platoon blocked, %		-	-		-	-	301	3.0			J U.	
Mov Cap-1 Maneuver	1592	-	_	1564	-	-	864	791	1026	886	791	1054
Mov Cap-2 Maneuver	-	_	-	-	_	_	864	791	-	886	791	-
Stage 1	-	-	_	-	-	-	932	830	-	987	876	_
Stage 2	_	_	-	-	_	_	974	876	_	932	830	_
- 1-13 							J	. .		JU-	300	
Annroach	ED			WD			ND			CD		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.7			0			0			8.5		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		-	1592	-	-	1564	-		1054			
HCM Lane V/C Ratio		-	0.008	-	-	-	-		0.013			
HCM Control Delay (s)		0	7.3	0	-	0	-	-	8.5			
HCM Lane LOS		A	A	A	-	A	-	-	Α			
HCM 95th %tile Q(veh))	-	0	-	-	0	-	-	0			
.,												

Intersection						
Int Delay, s/veh	0.8					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	00	^	7	7	^
Traffic Vol, veh/h	14	22	321	7	14	290
Future Vol, veh/h	14	22	321	7	14	290
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	88	88	91	91	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	25	353	8	16	330
Major/Minor N	Minor1	N	Major1		Major2	
			Major1			^
Conflicting Flow All	550	177	0	0	361	0
Stage 1	353	-	-	-	-	-
Stage 2	197	-	_	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	465	835	-	-	1194	-
Stage 1	682	-	-	-	-	-
Stage 2	817	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	459	835	-	-	1194	-
Mov Cap-2 Maneuver	459	-	-	-	-	-
Stage 1	682	-	-	-	-	-
Stage 2	806	-	-	-	-	-
<u> </u>						
A	\A/D		ND		OB	
Approach	WB		NB		SB	
HCM Control Delay, s	11.1		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1194	-
HCM Lane V/C Ratio		<u>-</u>		0.065		_
HCM Control Delay (s)			_		8.1	
HCM Lane LOS		_	_	В	Α	-
HCM 95th %tile Q(veh)		<u>-</u>	-	0.2	0	-
			_	U.Z	U	

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	8	0	0	28	0	0	0	0	0	0	8
Future Vol, veh/h	13	8	0	0	28	0	0	0	0	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	_
Veh in Median Storage	. # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	_	0	_	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	10	0	0	36	0	0	0	0	0	0	10
Major/Minor	Major1		1	Major2		1	Minor1			Minor2		
Conflicting Flow All	36	0	0	10	0	0	85	80	10	80	80	36
Stage 1	-	-	-	-	-	-	44	44	-	36	36	-
Stage 2	_	_	_	_	_	_	41	36	_	44	44	_
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1575	-	-	1610	-	-	901	810	1071	908	810	1037
Stage 1	-	-	-	-	-	-	970	858	-	980	865	-
Stage 2	-	-	-	-	-	-	974	865	-	970	858	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1575	-	-	1610	-	-	885	801	1071	901	801	1037
Mov Cap-2 Maneuver	-	-	-	-	-	-	885	801	-	901	801	-
Stage 1	-	-	-	-	-	-	959	849	-	969	865	-
Stage 2	-	-	-	-	-	-	964	865	-	959	849	-
Ü												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.5			0			0			8.5		
HCM LOS							A			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		-	1575	-	-	1610	-	-	1037			
HCM Lane V/C Ratio		_	0.011	-	-	-	-	-	0.01			
HCM Control Delay (s)		0	7.3	0	-	0	-	-	8.5			
HCM Lane LOS		A	Α	A	-	A	-	-	Α			
HCM 95th %tile Q(veh))	-	0	-	-	0	-	-	0			
111 11(101)						-						

Intersection						
Int Delay, s/veh	1.1					
		WED	NET	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	· W	4.0	^	7		^
Traffic Vol, veh/h	10	19	345	15	29	350
Future Vol, veh/h	10	19	345	15	29	350
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	92	92	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	20	38	375	16	34	412
		_		-		
	Minor1		//ajor1		Major2	
Conflicting Flow All	649	188	0	0	391	0
Stage 1	375	-	-	-	-	-
Stage 2	274	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	_	-	-	-
Follow-up Hdwy	3.52	3.32	-	_	2.22	_
Pot Cap-1 Maneuver	402	822	_	_	1164	_
Stage 1	665	-	_	_	-	_
Stage 2	747	_	_	_	_	_
Platoon blocked, %	171		_	_		_
Mov Cap-1 Maneuver	390	822	_	_	1164	_
	390					
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	665	-	-	-	-	-
Stage 2	725	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.7		0		0.6	
HCM LOS	В		U		0.0	
1 TOWN LOO	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	595	1164	-
HCM Lane V/C Ratio		_	-	0.097		_
HCM Control Delay (s)		_	_	11.7	8.2	_
HCM Lane LOS		-	_	В	A	-
HCM 95th %tile Q(veh))	_	_	0.3	0.1	_
How Jour Joure Wiveri)			0.0	0.1	

Intersection												
Int Delay, s/veh	2.3											
• •												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	10	34	0	0	18	0	0	0	0	0	0	11
Future Vol, veh/h	10	34	0	0	18	0	0	0	0	0	0	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	э,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	44	0	0	23	0	0	0	0	0	0	14
Major/Minor	Major1		ı	Major2			Minor1			Minor2		
Conflicting Flow All	23	0	0	44	0	0	100	93	44	93	93	23
Stage 1	23	-	U	44	-	U	70	70	44	23	23	- 23
Stage 2	_	_		-	-		30	23	_	70	70	_
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	4.12	_	-	4.12	-	_	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218		-	2.218	-	_	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1592	_	-	1564	<u>-</u>	_	881	797	1026	891	797	1054
Stage 1	1092	_	-	1504	_	_	940	837	1020	995	876	1004
Stage 2	-	_	-	<u>-</u>	<u>-</u>	_	987	876		940	837	_
Platoon blocked, %	_	_	-	-	_	_	307	070	_	340	037	
Mov Cap-1 Maneuver	1592	-	-	1564	-	-	864	791	1026	886	791	1054
Mov Cap-1 Maneuver	1092	_		1504	_	_	864	791	1020	886	791	1004
Stage 1	-	<u>-</u>	-	<u>-</u>	-	-	932	830		987	876	-
Stage 2		_		_	_	_	974	876	- -	932	830	_
Olaye Z	_	_	-	<u>-</u>	<u>-</u>	-	314	010	_	332	000	_
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.7			0			0			8.5		
HCM LOS							Α			Α		
Minor Lane/Major Mvn	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		-	4=00	-	-	1564	-		1054			
HCM Lane V/C Ratio		_	0.008	<u> </u>	_	1304	<u> </u>		0.013			
HCM Control Delay (s)	\	0	7.3	0	_	0	-	<u>-</u>	8.5			
HCM Lane LOS		A	7.3 A	A	_	A	<u> </u>	_	0.5 A			
HCM 95th %tile Q(veh	1)	- -	0	- -	-	0		-	0			
)	-	U	-	-	U	-	-	U			

Intersection						
Int Delay, s/veh	3.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	אופוז	†	T T) T	↑ ↑
Traffic Vol, veh/h	61	30	237	15	102	180
Future Vol, veh/h	61	30	237	15	102	180
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Otop	None	-		-	None
Storage Length	0	-	_	255	340	-
Veh in Median Storage		_	0	-	-	0
Grade, %	, # 0	<u>-</u>	0	<u>-</u>	_	0
Peak Hour Factor	88	88	91	91	88	88
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	69	34	260	16	116	205
Major/Minor N	Minor1	N	Major1	N	//ajor2	
Conflicting Flow All	595	130	0	0	276	0
Stage 1	260	-	-	-		-
Stage 2	335	_	_	_	_	_
Critical Hdwy	6.84	6.94	_	_	4.14	-
Critical Hdwy Stg 1	5.84	0.0 -	_	_		_
Critical Hdwy Stg 2	5.84	_	_	_	_	_
Follow-up Hdwy	3.52	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	436	896	-	-	1284	
•	760	090	-	_	1204	-
Stage 1			-	-	-	
Stage 2	697	-	-	-	-	-
Platoon blocked, %	207	000	-	-	4004	-
Mov Cap-1 Maneuver	397	896	-	-	1284	-
Mov Cap-2 Maneuver	397	-	-	-	-	-
Stage 1	760	-	-	-	-	-
Stage 2	634	-	-	-	-	-
Approach	WB		NB		SB	
	14.4		0		2.9	
HCM Control Delay, s			U		2.9	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	486	1284	-
HCM Lane V/C Ratio		_		0.213	0.09	_
HCM Control Delay (s)		-	_	14.4	8.1	-
HCM Lane LOS		_	_	В	A	_
HCM 95th %tile Q(veh)		-	-	0.8	0.3	-

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	7	97	2	26	0	56	0	1	0	0	8
Future Vol, veh/h	13	7	97	2	26	0	56	0	1	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	9	124	3	33	0	72	0	1	0	0	10
Major/Minor N	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	33	0	0	133	0	0	149	144	71	145	206	33
Stage 1	-	-	-	-	-	-	105	105	-	39	39	-
Stage 2	_	_	-	_	_	_	44	39	_	106	167	_
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	_	-	_	-	-	6.12	5.52	-	6.12	5.52	_
Critical Hdwy Stg 2	_	_	_	-	_	_	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	_	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1579	-	-	1452	-	-	819	747	991	824	691	1041
Stage 1	-	_	-	-	-	-	901	808	-	976	862	_
Stage 2	-	-	-	-	-	-	970	862	-	900	760	-
Platoon blocked, %		_	-		-	-						
Mov Cap-1 Maneuver	1579	-	-	1452	-	-	803	737	991	814	681	1041
Mov Cap-2 Maneuver	-	-	-	-	-	-	803	737	-	814	681	-
Stage 1	-	-	-	-	-	-	890	798	-	964	860	-
Stage 2	-	-	-	-	-	-	959	860	-	888	751	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.8			0.5			9.9			8.5		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		806	1579		-	1452	_	_				
HCM Lane V/C Ratio		0.091		_		0.002	_	_	0.01			
HCM Control Delay (s)		9.9	7.3	0	_	7.5	0	_	8.5			
HCM Lane LOS		Α	Α.	A	_	Α.	A	_	A			
HCM 95th %tile Q(veh)		0.3	0	-	_	0	- '.	-	0			
		3.0										

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	† 1>			7
Traffic Vol, veh/h	0	241	212	67	0	40
Future Vol, veh/h	0	241	212	67	0	40
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		_	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	87	87	87	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	277	244	86	0	51
Major/Minor	Major1	ı	Major2	N	/linor2	
Conflicting Flow All	- Iviajoi i	0	- viajoiz	0	-	165
Stage 1	_	-	_	-	_	-
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	_	_	_	_	6.94
Critical Hdwy Stg 1	_	_		_	_	- 0.34
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	<u>-</u>	_	_	_	_	3.32
Pot Cap-1 Maneuver	0	_	_	_	0	850
Stage 1	0	_	_	_	0	-
Stage 2	0	_	_	_	0	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	_	_	_	-	_	850
Mov Cap 1 Maneuver	_	_	_	_	_	-
Stage 1	_	_	_	_	_	_
Stage 2	<u>-</u>	_	_	_	<u>-</u>	_
Jugo 2						
A			14/5		0.5	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		9.5	
HCM LOS					Α	
Minor Lane/Major Mvn	nt	EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		-	-	-	850	
HCM Lane V/C Ratio		_	_	_	0.06	
HCM Control Delay (s)		_	-	-	9.5	
HCM Lane LOS		-	-	-	Α	
HCM 95th %tile Q(veh)	-	-	-	0.2	
	,					

Intersection						
Int Delay, s/veh	4.4					
		WED	NET	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩	00	^	7	70	^
Traffic Vol, veh/h	59	30	274	21	79	221
Future Vol, veh/h	59	30	274	21	79	221
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	92	92	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	118	60	298	23	93	260
Major/Minor N	/linor1	N	Major1	ı	Major2	
Conflicting Flow All	614	149	0	0	321	0
Stage 1	298	-	-	_	-	-
Stage 2	316	<u>-</u>	_	_	_	_
Critical Hdwy	6.84	6.94			4.14	_
Critical Hdwy Stg 1	5.84	0.34	_	_	7.17	_
Critical Hdwy Stg 2	5.84	_	_	_	_	
Follow-up Hdwy	3.52	3.32	_	_	2.22	-
Pot Cap-1 Maneuver	424	871		_	1236	-
•	727	- 071	-	_	1230	_
Stage 1	712		_	_	_	
Stage 2	112	-	-	-	-	-
Platoon blocked, %	202	074	-	-	1000	-
Mov Cap-1 Maneuver	392	871	-	-	1236	-
Mov Cap-2 Maneuver	392	-	-	-	-	-
Stage 1	727	-	-	-	-	-
Stage 2	659	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	16.8		0		2.1	
HCM LOS	C		U		۷.۱	
	J					
Minor Lane/Major Mvmt	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	101	1236	-
HCM Lane V/C Ratio		-	-		0.075	-
HCM Control Delay (s)		-	-		8.1	-
HCM Lane LOS		-	-	С	Α	-
LIONA OF U. O/ UL O/ LA		_	_	1.7	0.2	_
HCM 95th %tile Q(veh)				1.7	0.2	

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	10	33	57	0	18	0	61	0	1	0	0	11
Future Vol, veh/h	10	33	57	0	18	0	61	0	1	0	0	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	42	73	0	23	0	78	0	1	0	0	14
Major/Minor I	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	23	0	0	115	0	0	135	128	79	128	164	23
Stage 1	-	-	-	-	-	-	105	105	-	23	23	-
Stage 2	-	-	-	_	_	-	30	23	_	105	141	-
Critical Hdwy	4.12	_	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1592	-	-	1474	-	-	836	763	981	845	729	1054
Stage 1	-	-	-	-	-	-	901	808	-	995	876	-
Stage 2	-	-	-	-	-	-	987	876	-	901	780	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1592	-	-	1474	-	-	819	756	981	838	722	1054
Mov Cap-2 Maneuver	-	-	-	-	-	-	819	756	-	838	722	-
Stage 1	-	-	-	-	-	-	893	801	-	986	876	-
Stage 2	-	-	-	-	-	-	974	876	-	892	773	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0			9.9			8.5		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		821	1592	-	-	1474	_	-	1054			
HCM Lane V/C Ratio			0.008	-	_	-	-	_	0.013			
HCM Control Delay (s)		9.9	7.3	0	-	0	_	-	8.5			
HCM Lane LOS		A	A	A	_	A	-	-	A			
HCM 95th %tile Q(veh))	0.3	0	-	-	0	-	-	0			
/ (/ 011)												

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	1 1			7
Traffic Vol, veh/h	0	283	244	44	0	51
Future Vol, veh/h	0	283	244	44	0	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	87	87	87	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	325	280	56	0	65
Major/Minor N	/lajor1	N	Major2		/linor2	
						160
Conflicting Flow All	-	0	-	0	-	168
Stage 1			-	-		-
Stage 2	-	-	-	-	-	6.94
Critical Hdwy	-	-	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	2 22
Follow-up Hdwy	-	-	-	-	-	3.32 847
Pot Cap-1 Maneuver	0	-	-	-	0	847
Stage 1	0	-	-	-	0	
Stage 2	U	-	-	-	0	-
Platoon blocked, %		-	-	-		0.47
Mov Cap-1 Maneuver	-	-	-	-	-	847
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		9.6	
HCM LOS					Α	
Minor Long/Major Maren		EDT	WDT	WDD	DI ~1	
Minor Lane/Major Mym		EBT	WBT	WBR S		
Capacity (veh/h)		-	-	-	847	
HCM Lane V/C Ratio		-	-	-	0.077	
HCM Lora LOS		-	-	-	9.6	
HCM Lane LOS		_	-	-	Α	
HCM 95th %tile Q(veh)					0.2	

Intersection						
Int Delay, s/veh	3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	TTD.T.	^	7	ሻ	^
Traffic Vol, veh/h	61	30	336	15	102	266
Future Vol, veh/h	61	30	336	15	102	266
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	88	88	91	91	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	69	34	369	16	116	302
mmer ion	00	Ų i	000		110	002
	Minor1		Major1		Major2	
Conflicting Flow All	752	185	0	0	385	0
Stage 1	369	-	-	-	-	-
Stage 2	383	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	346	826	-	-	1170	-
Stage 1	670	-	-	-	-	-
Stage 2	659	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	312	826	-	-	1170	-
Mov Cap-2 Maneuver	312	-	-	-	-	-
Stage 1	670	-	-	-	-	-
Stage 2	594	-	-	-	-	-
Annroach	MD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	17.4		0		2.3	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	393	1170	_
HCM Lane V/C Ratio		_	_	0.263		_
HCM Control Delay (s))	_	_	17.4	8.4	_
HCM Lane LOS		_	_	С	A	_
HCM 95th %tile Q(veh)	-	-	1	0.3	_
	,					

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	7	97	2	26	0	56	0	1	0	0	8
Future Vol, veh/h	13	7	97	2	26	0	56	0	1	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	9	124	3	33	0	72	0	1	0	0	10
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	33	0	0	133	0	0	149	144	71	145	206	33
Stage 1	-	_	-	-	-	-	105	105	-	39	39	-
Stage 2	_	_	-	_	_	-	44	39	_	106	167	_
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	_	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1579	_	-	1452	-	-	819	747	991	824	691	1041
Stage 1	-	-	-	-	-	-	901	808	-	976	862	-
Stage 2	-	-	-	-	-	-	970	862	-	900	760	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1579	-	-	1452	-	-	803	737	991	814	681	1041
Mov Cap-2 Maneuver	-	-	-	-	-	-	803	737	-	814	681	-
Stage 1	-	-	-	-	-	-	890	798	-	964	860	-
Stage 2	-	-	-	-	-	-	959	860	-	888	751	-
Ŭ												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.8			0.5			9.9			8.5		
HCM LOS							A			A		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		806	1579			1452	-	-				
HCM Lane V/C Ratio		0.091		_		0.002	_	_	0.01			
HCM Control Delay (s)		9.9	7.3	0	_	7.5	0	_	8.5			
HCM Lane LOS		A	A	A	_	A	A	_	A			
HCM 95th %tile Q(veh))	0.3	0	-	_	0	-	_	0			
		5.5										

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	† †	↑ ↑	אטול	ODL	7
Traffic Vol, veh/h	0	327	311	67	0	40
Future Vol, veh/h	0	327	311	67	0	40
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	_	-	_	0
Veh in Median Storage,	.# -	0	0	-	0	-
Grade, %	, -	0	0	_	0	-
Peak Hour Factor	87	87	87	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	376	357	86	0	51
Maing/Minn	1-1-4		4-1-0		A: C	
	//ajor1		Major2		/linor2	000
Conflicting Flow All	-	0	-	0	-	222
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	782
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	_					
		-	-	-	-	782
Mov Cap-2 Maneuver	-	-	-	-	-	782 -
Stage 1		- - -	- -			
	-	- - -	- - -			
Stage 1	-	-	-	-	-	-
Stage 1 Stage 2	-	-	-	-	- - -	-
Stage 1 Stage 2 Approach	- - - EB	-	- - WB	-	- - - SB	-
Stage 1 Stage 2 Approach HCM Control Delay, s	- - -	-	- -	-	- - - SB 9.9	-
Stage 1 Stage 2 Approach	- - - EB	-	- - WB	-	- - - SB	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	- - - EB 0	-	- - WB 0	-	- - - SB 9.9 A	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	- - - EB 0	-	- - WB	-	- - - SB 9.9 A	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	- - - EB 0	-	- - WB 0	WBR S	SB 9.9 A SBLn1 782	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	- - - EB 0	-	- - WB 0	- - - WBR \$	SB 9.9 A SBLn1 782 0.066	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	- - - EB 0	-	WB 0	WBR S	SB 9.9 A SBLn1 782 0.066 9.9	-
Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	- - - 0	-	WB 0	- - - WBR \$	SB 9.9 A SBLn1 782 0.066	-

Intersection						
Int Delay, s/veh	4.4					
		WED	NET	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		^	7	ř	† †
Traffic Vol, veh/h	59	30	375	21	79	329
Future Vol, veh/h	59	30	375	21	79	329
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	255	340	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	92	92	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	118	60	408	23	93	387
	Minor1		//ajor1		Major2	
Conflicting Flow All	788	204	0	0	431	0
Stage 1	408	-	-	-	-	-
Stage 2	380	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	_	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	328	803	_	_	1125	_
Stage 1	640	-	_	_	-	_
Stage 2	661	-	-	-	_	-
Platoon blocked, %	- 001		_	_		_
Mov Cap-1 Maneuver	301	803	-		1125	
Mov Cap-1 Maneuver	301	- 003		_	1123	_
	640	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	606	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	22.5		0		1.6	
HCM LOS	C				1.0	
TIOWI LOO	J					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	381	1125	_
HCM Lane V/C Ratio		_	-	0.467		-
HCM Control Delay (s))	-	-		8.5	-
HCM Lane LOS		_	-	С	A	_
HCM 95th %tile Q(veh)	_	_	2.4	0.3	_
HOW JOHN JUNE Q(VEI)	7			۷.٦	0.0	

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	10	33	57	0	18	0	61	0	1	0	0	11
Future Vol, veh/h	10	33	57	0	18	0	61	0	1	0	0	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	42	73	0	23	0	78	0	1	0	0	14
Major/Minor I	Major1			Major2		1	Minor1			Minor2		
Conflicting Flow All	23	0	0	115	0	0	135	128	79	128	164	23
Stage 1	-	-	-	-	_	-	105	105	-	23	23	
Stage 2	-	-	_	-	-	-	30	23	-	105	141	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	_	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1592	-	_	1474	-	-	836	763	981	845	729	1054
Stage 1	-	-	-	-	-	-	901	808	-	995	876	-
Stage 2	-	-	-	-	-	-	987	876	-	901	780	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1592	-	-	1474	-	-	819	756	981	838	722	1054
Mov Cap-2 Maneuver	-	-	-	-	-	-	819	756	-	838	722	-
Stage 1	-	-	-	-	-	-	893	801	-	986	876	-
Stage 2	-	-	-	-	-	-	974	876	-	892	773	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0			9.9			8.5		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		821	1592	-	-	1474	-	-	1054			
HCM Lane V/C Ratio		0.097		-	-	-	-	_	0.013			
HCM Control Delay (s)		9.9	7.3	0	-	0	-	_	8.5			
HCM Lane LOS		A	Α	A	-	A	-	-	A			
HCM 95th %tile Q(veh))	0.3	0	-	-	0	-	-	0			

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	\\/DT	WBR	SBL	SBR
	EDL	<u></u> ★↑	WBT	WDK	ODL	
Lane Configurations	•		† ‡		•	7
Traffic Vol, veh/h	0	388	348	44	0	51
Future Vol, veh/h	0	388	348	44	0	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	_	0	0	-	0	-
Peak Hour Factor	87	87	87	78	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	446	400	56	0	65
WWITH FIOW	U	440	400	50	U	03
Major/Minor	Major1	N	Major2	N	/linor2	
Conflicting Flow All		0		0	-	228
Stage 1	-	-	_	-	_	
Stage 2	_	<u>-</u>	_	_	_	_
Critical Hdwy	_	_		_	_	6.94
Critical Hdwy Stg 1	_	_	_	_	<u>-</u>	0.34
			-			
Critical Hdwy Stg 2	-	-	-	-	_	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	775
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	_	-	-	-	775
Mov Cap-2 Maneuver		_	_	_	_	_
Stage 1	_	_	_	_	_	_
Stage 2	_	<u>-</u>	_	_	_	_
Olage 2						
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		10.1	
HCM LOS					В	
TIOWI LOO					U	
Minor Lane/Major Mvr	nt	EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		_	_	-	775	
HCM Lane V/C Ratio		_	_	_	0.084	
HCM Control Delay (s)	_	_	_		
Jili John Doldy (0	1				В	
HCM Lane LOS		_	-	-		
HCM Lane LOS HCM 95th %tile Q(veh	1)	-	-	-	0.3	

Traffic Impact Study_V1.pdf Markup Summary

dsdlaforce (22)

IND USE AND ACCESS

gure 1 shows the site location relative to the adjacent and ne own in Figure 2. The Cathedrial Rock Church is planned for the 6 of Struthers Runch Road. As Johnson in Figure 2, these access ownered access onto Struthers Ranch Road across from the go survity, and a printly-drived two scores Struthers Readnuthers Rench Tract 8 is proposed to include 12,740 square for

TERSECTION SIGHT DISTANCE

se required intersection sight distance for the intersection of Strut

Subject: Highlight Page Label: 6
Author: dsdlaforce

Date: 3/10/2021 6:00:19 PM

Status: Color: Layer: Space: a right-in/right-out access onto Struthers Road



Subject: Callout Page Label: 6 Author: dsdlaforce

Date: 3/10/2021 6:10:36 PM

Status: Color: Layer: Space: Discuss this deviation request with your client. Staff does not see an undue hardship to justify a deviation request for access on Struthers Road. With access available at Struthers Ranch Rd, this deviation request will likely be denied.

If the request is withdrawn then update the TIS analysis based on a single access from Struthers Ranch Road.

Please note: If the applicant chooses to pursue the request, staff encourages you to submit the deviation request form prior to the 2nd resubmittal. The determination to approve or deny the deviation impacts the traffic report.

And to determine a second prior for the region of the second to the second prior for the seco

Subject: Callout Page Label: 7 Author: dsdlaforce

Date: 3/10/2021 6:38:06 PM

Status: Color: Layer: Space: Can this statement be verified? Explain how the future volumes were calibrated/modified.

site-generated traffic. The turning mornings with addition of future ch (lane plus taper). The ECM requiren

The right-in/right-out access on Str. lane. This criterion calls for 370-for 45-mph design speed and a 435-fo design speed. The speed limit c recommends an approximately 200

Subject: Highlight Page Label: 11 Author: dsdlaforce

Date: 3/11/2021 2:50:19 PM

Status: Color: Layer: Space: e right-in/right-out access



Subject: Highlight Page Label: 23 Author: dsdlaforce

Date: 3/11/2021 2:51:07 PM

Status: Color: Layer: Space:



Subject: Highlight Page Label: 23 Author: dsdlaforce

Date: 3/11/2021 2:53:00 PM

Status: Color: Layer: Space:



Subject: Callout Page Label: 23 Author: dsdlaforce

Date: 3/11/2021 2:53:05 PM

Status: Color: Layer: Space: This meets the criteria for right turn lane. Update the Auxiliary Turn Lane section and the conclusion/recommendation section to discuss.



Subject: Cloud+ Page Label: 13 Author: dsdlaforce

Date: 3/11/2021 3:03:16 PM

Status: Color: Layer: Space: Will be reviewed on the resubmittal.

Recommendation may be different with the

removal of RIRO.



Subject: Text Box Page Label: 28 Author: dsdlaforce

Date: 3/11/2021 3:40:55 PM

Status: Color: Layer: Space: Show the sight distance to the north and update the narrative to state whether or not it meets criteria. If it doesn't, provide recommendations to mitigate.



Subject: Callout Page Label: 28 Author: dsdlaforce

Date: 3/11/2021 3:49:55 PM

Status: Color: Layer: Space: update.



Subject: Text Box Page Label: 28 Author: dsdlaforce

Date: 3/11/2021 3:59:35 PM

Status: Color: Layer: Space: Provide an exhibit for the driveway entering sight

distance



Subject: Callout Page Label: 23 Author: dsdlaforce

Date: 3/11/2021 4:15:58 PM

Status: Color: Layer: Space: Without the RIRO this it seems separate left turn/right turn lanes are warranted. Provide auxiliary turn lane analysis and recommendation



Subject: Callout Page Label: 6 Author: dsdlaforce

Date: 3/11/2021 5:12:52 PM

Status: Color: Layer: Space: Update the required intersection sight distance and update Exhibit 1. Intersection sight distance applies to Struthers Road/Struthers Ranch Road intersection.

Intersection sight distance is ECM 2.3.6.G Table

2-21.

Driveway access sight distance is ECM ECM

2.4.1.D.



Subject: Callout Page Label: 12 Author: dsdlaforce

Date: 3/11/2021 5:15:47 PM

Status: Color: Layer: Space: Update the to identify that a sight distance easement where the line of sight encroaches into

the private property



Subject: Cloud+ Page Label: 15 Author: dsdlaforce

Date: 3/11/2021 5:16:17 PM

Status: Color: Layer: Space: Show the Morning/Afternoon new external trip

generated



Subject: Callout Page Label: 1 Author: dsdlaforce

Date: 3/11/2021 5:19:04 PM

Status: Color: Layer: Space: Update title to match the rest of the application. "Struthers Ranch Subdivision Filing No. 5

ch Tract B

Study

Subject: Callout Page Label: 2 Author: dsdlaforce

Date: 3/11/2021 5:19:43 PM

Status: Color: Layer: Space: update

Subject: Callout Page Label: 5 Author: dsdlaforce

Date: 3/11/2021 5:20:02 PM

Status: Color: Layer: Space:

.....

Update

Subject: Highlight Page Label: 5 Author: dsdlaforce

Date: 3/11/2021 5:20:07 PM

Status: Color: Layer: Space:

.....

Subject: Highlight Page Label: 5 Author: dsdlaforce

Date: 3/11/2021 5:20:08 PM

Status: Color: Layer: Space:

.....

Subject: Highlight Page Label: 5
Author: dsdlaforce

Date: 3/11/2021 5:20:11 PM

Status: Color: Layer: Space:

.....



Subject: Callout Page Label: 15 Author: dsdlaforce

Date: 3/11/2021 7:43:57 AM

Status: Color: Layer: Space: include the reference in the appendix. The

percentages seems high.

lpackman (9)



Subject: Callout Page Label: 2 Author: lpackman

Date: 2/25/2021 11:11:27 AM

Status: Color: Layer: Space: Please add the following: "PCD File No. VR-2101"

civi prescribea signs, parking out 18 inches. river's eve" is ne site grading

Subject: Highlight Page Label: 6 Author: lpackman

Date: 2/25/2021 11:33:56 AM

Status: Color: Layer: Space:

18 inches

Subject: Callout Page Label: 7 Author: lpackman

Date: 2/25/2021 11:57:43 AM

Status: Color: Layer: Space:

Please revise to include a description of the required sight distance for the access point on Struthers Ranch Rd.

Subject: Text Box Page Label: 13 Author: lpackman

Date: 2/25/2021 2:29:47 PM

Status: Color: Layer: Space:

Please provide a list of references used to create this study.

Subject: Callout Page Label: 9 Author: lpackman

Date: 2/25/2021 4:10:45 PM

Status: Color: Layer: Space:

Describe the reason behind the 2% traffic growth.



Subject: Callout Page Label: 23 Author: lpackman

Date: 2/25/2021 4:36:18 PM

Status: Color: Layer: Space:

Per Struthers Ranch Filing No. 4 plat note number 14, there shall be no direct access to Struthers Rd. Please update the narrative and the analysis

accordingly.



Subject: Text Box Page Label: 18 Author: lpackman

Date: 2/25/2021 4:36:54 PM

Status: Color: Layer: Space:

Per Struthers Ranch Filing No. 4 plat note number 14, there shall be no direct access to Struthers Rd. Please update the narrative and the analysis

accordingly.



Subject: Callout Page Label: 10 Author: Ipackman

Date: 3/1/2021 7:13:14 AM

Status: Color: Layer: Space: Please describe if MTCP was used for projected traffic volumes/conditions. Reference ECM

B.2.2.C.

3. The Control of the Country against the text of the Country against the Country again

Subject: Callout Page Label: 6 Author: lpackman

Date: 3/1/2021 8:08:28 AM

Status: Color: Layer: Space: Please revise 18 inches to 30 inches above the flow line of the adjacent road per ECM 2.3.6.G.2.