Update title to match the rest of the

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Website: http://www.Isctrans.com

## TRANSPORTATION application. <br> CONSULTANTS, IN "Struthers Ranch Subdivision Filing No. 5

# Struthers Ranch Tract B 

 Traffic Impact Study(LSC \#204110)
January 8, 2021

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.


$$
\frac{I-13-202}{\text { Date }}
$$

# Struthers Ranch Tract B Traffic Impact Study 

Prepared for:
Design and Development Consultants
T-Bone Construction
1310 Ford Street
Colorado Springs, Colorado 80915

Contact: Mr. Darin C. Weiss, AIA

JANUARY 8, 2021

LSC Transportation Consultants
Prepared by: Colleen Guillotte, P.E., PTOE, RSP
Reviewed by: Jeffrey C. Hodsdon, P.E.

Please add the
following: "PCD File
No. VR-2101"
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January 8, 2021

Darin C Weiss, AIA
Design and Development Consultants
T-Bone Construction
1310 Ford Street
Colorado Springs, CO 80915


LSC Transportation Consultants, Inc. has prepared this traffic impact study for the proposed Struthers Ranch Tract B development in El Paso County, Colorado. The development is planned to be located southeast of the intersection of Struthers Road/Struthers Ranch Road. The planned land use is for 19,740 square feet of retail, 5,200 square feet of office, and 5,200 square feet of restaurant. This report has been prepared for submittal to El Paso County.

## REPORT CONTENTS

The preparation of this report included the following:

- Inventory of the existing adjacent and nearby area street and roadway system. This includes functional classifications, street widths, lane configurations, intersection traffic control, posted speed limits, pavement markings, intersection and access spacing, roadway and intersection alignments, auxiliary left- and right-turn lanes, intersection sight distances, etc.;
- A review of the proposed site land use and access locations;
- Morning and evening peak-hour traffic counts at the intersection of Struthers Road/Struthers Ranch Road;
- Estimates of short- and long-term background traffic volumes and total traffic (site traffic plus background traffic). Forecasts include buildout of adjacent proposed developments;
- Estimates of the daily and peak-hour trip generation for the proposed land use;
- The estimated directional distribution of site-generated vehicle trips on the study area street and roadway system;
- Projections of peak-hour site-generated turning-movement traffic volumes at the study area intersections, which include:
- Site access point intersections
- Struthers Road/Struthers Ranch Road
- Level of service (LOS) analysis at the study area intersections;
- Evaluation of the shnrt-torm and Inno-torm nnniartad intorcortinn unlumac tn dotarmino the potential need for a Discuss this deviation request with your client. Staff does not see an undue hardship to justify a deviation request for access on Struthers Road. With access available at Struthers Ranch Rd, this deviation request will likely be denied.


## PREVIOUS TRAFFIC STUD

If the request is withdrawn then update the TIS analysis
Nearby properties have rt based on a single access from Struthers Ranch Road.

- Monument Ridge L
- Monument Ridge A Please note: If the applicant chooses to pursue the request,
- Cathedral Rock Chu staff encourages you to submit the deviation request form prior to the 2nd resubmittal. The determination to approve This report is consistent n or deny the deviation impacts the traffic report. background traffic volumes.


## LAND USE AND ACCESS

Figure 1 shows the site location relative to the adjacent and nearby roadways. The site plan is shown in Figure 2. The Cathedral Rock Church is planned for the adjacent Tract A on the north side of Struthers Ranch Road. As shown in Figure 2, two access points are proposed: a full movement access onto Struthers Ranch Road across from the proposed access to Cathedral Rock Church, and a right-in/right-out access onto Struthers Road.

Struthers Ranch Tract B is proposed to include 19,740 square feet of retail, 5,200 square feet of office, and 5,200 square feet of restaurant.

Please revise 18 inches to 30 inches above the flow line of the adjacent INTERSECTION SIGHT DISTANC road per ECM 2.3.6.G.2.

The required intersection sight distance for the intersection of Struthers Road/Struthers Ranch Road is 625 feet for passenger vehicles and $9 反 0$ feet for combination trucks. This intersection would be able to this criterion provided the intersection line of sight "triangles" are kept free of site improvements (thatwould limit the lihe of sight needed to maintain ECM prescribed sight distance). Examples of site improvements include landscaping, monument signs, parking areas, berms, etc. Obstruction height to maintain passenger car line of sight is about 18 inches. Obstruction height to maintain truck line of sight is higher as the truck "driver's eye" is significantly higher than the "driver's eye" for a passenger vehicle. Depending on the site grading, the combination truck driver line of sight may pas over vehicles parked along the west side of the site. Line of sight exhibits are attached.
Update the required intersection sight distance and update Exhibit 1. Intersection sight distance applies to Struthers Road/Struthers Ranch Road intersection.

The required sight distance for the right-in/right-out access point onto Struthers Road is 500 feet for passenger vehicles. The line of sight to arriving northbound through traffic on Struthers is over one quarter mile. $\longleftarrow \quad$ Please revise to include a description of the required sight distance for the access point on Struthers Ranch Rd.

## Area Roads

Figure 1 shows the streets in the vicinity of the site. The streets adjacent to the site are identified below followed by a brief description of each:

Struthers Road is a four-lane, median-divided road that extends north from North Gate Boulevard to the intersection of Baptist Road and Jackson Creek Parkway. Struthers Road is classified as a four-lane Urban Minor Arterial on the El Paso County Major Transportation Corridors Plan and has a speed limit of 45 miles per hour (mph) about 325 feet north of Air Garden Lane (adjacent to the south portion of the site). South of this point, the posted speed limit is 40 mph .

Struthers Ranch Road is classified as a local roadway. Struthers Ranch Road is an east/west road that extends from Struthers Road into the Struthers Ranch residential development. The roadway has a posted speed limit of 25 mph . The intersection with Struthers Road is unsignalized. The roadway at the intersection with Struthers Road is 32 feet wide, which only allows for a shared westbound left/right lane on the minor street approach. Struthers Road has a 340 -foot southbound left-turn deceleration lane and a 260 -foot northbound right-turn deceleration lane at the intersection with Struthers Ranch Road.

## Traffic Volumes

Morning and evening peak-hour turning-movement traffic counts were conducted March 2020 at the intersection of Struthers Road/Struthers Ranch Road. The morning and evening peak-hour volumes are shown in Figure 3. It should be noted that the recorded southbound through traffic during the evening peak was significantly higher than historical counts. Counts recorded by LSC on Struthers Road between 2011 and 2019 have shown that the southbound traffic is typically lower than the northbound traffic during the evening peak hour. It is believed that there may have been an incident on $1-25$ in the southbound direction during the countsthat caused a large number of vehirles to use Struthers Rnad as an alternate route. Traffic count/reports are attached for reference. Can this statement be verified? Explain how the future volumes were calibrated/modified.

## Crash History

Three years of crash data were collected at the intersection of Struthers Road/Struthers Ranch Road. There was only one crash during the study period. The only crash was a fixed object type crash that resulted in property damage only. No correctable crash patterns were identified.

## TRIP GENERATION

Estimates of the vehicle trips projected to be generated by the proposed development have been made using the nationally published trip-generation rates from Trip Generation, $10^{\text {th }}$ Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 1, below, presents a summary of the estimated site trip generation on a typical weekday. The detailed trip-generation estimate for the development, including ITE rates for the proposed land use, is presented in Table 3.

Approximately 2,986 total vehicle trips are projected to enter and exit the site at the access point ("driveway trips") on the average weekday during a 24 -hour period. During the morning peak hour, approximately 170 vehicles would enter and 98 vehicles would exit the site. During the evening peak hour, approximately 101 vehicles would enter and 112 vehicles would exit the site.

The proposed development is projected to generate approximately 1,776 (new/non-pass-by or diverted) vehicle trips on the average weekday during a 24 -hour period.

Table 1: Estimated External Site Vehicle-Trip Generation (Vehicles per Hour)

| Analysis Period | Total Trips |  |  | Pass-by Trips |  |  | New Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | In | Out | Total | In | Out | Total | In | Out | Total |
| A.M. Peak Hour | 170 | 98 | 268 | 53 | 53 | 106 | 117 | 45 | 162 |
| P.M. Peak Hour | 101 | 112 | 213 | 43 | 43 | 86 | 58 | 69 | 127 |
| Daily/24-Hour | 1,493 | 1,493 | 2,986 | 605 | 605 | 1,210 | 888 | 888 | 1,776 |

## Internal Trips

Internal trips are trips that occur within the site and do not impact the external roadways. Because the site is planned to have multiple retail, office, and restaurant pads, some of the generated trips will be traveling within the site. Table 3 includes estimates of internal trip capture to account for trips generated within the site as well as non-motorized trips from adjacent and nearby developments.

## Pass-by Trips

The trips generated by the site have also been aggregated by trip type to account for the pass-by phenomenon. A pass-by trip is one made by a motorist who would already be on an adjacent road regardless of the proposed development, but who stops in at the site while passing by. The pass-by motorist would then continue on his or her way to a final destination in the original direction. For purposes of this report, pass-by trips are trips by motorists already traveling through the intersection of Struthers Road/Struthers Ranch Road. Pass-by trips are shown in Table 3 and are based on Trip Generation Handbook - An ITE Proposed Recommended Practice, 3rd Edition, 2014 by ITE.

## BACKGROUND TRAFFIC

Background volumes do not include projected traffic to be generated by the proposed development. As noted in the existing conditions section, the southbound through traffic during the evening peak was unusually high. This volume was modified in the background to reflect expected volumes.

## Short-Term Background Traffic Volumes

Figure 4 shows the projected background traffic volumes. The projected volumes assume that the following nearby developments have been constructed:

- Monument Ridge Lots 7 \& 8
- Cathedral Rock Church
- Monument Ridge Apartments


## Long-Term Background Traffic Volumes

Figure 5 shows the projected 20 -year background traffic volumes for the year 2040. The long-term scenario includes the developments in the short-term background. In addition, the long-term background traffic assumes a growth of approximately 2 percent per year of through | traffic on Struthers Road. | $\begin{array}{l}\text { Describe the reason } \\ \text { behind the } 2 \% \text { traffic } \\ \text { growth. }\end{array}$ |
| :--- | :--- |
| TRIP DISTRIBUTION AND ASSIGNMENT |  |

## Trip Directional Distribution

Estimation of the directional distribution of site-generated vehicle trips to the study area roads and intersections is a necessary component in determining the site's traffic impacts. Figure 6 shows the short-term and long-term directional distribution estimates for the proposed development. Estimates were based on the following factors: existing area development, the area roadway system, and the site's proposed land use.

## Site-Generated Traffic

Site-generated traffic volumes at the study intersections have been calculated by applying the directional-distribution percentages estimated by LSC (from Figure 6) to the trip-generation estimates (from Table 1). Figure 7 shows the projected site-generated traffic volumes for the proposed development.

## Short-Term Total Traffic Volumes

Figure 8 shows the sum of the short-term background traffic volumes (from Figure 4) and the site-generated peak-hour traffic volumes (shown in Figure 7). These volumes represent the projected short-term total traffic following construction of the development. projected traffic volumes/conditions.

## Long-Term Total Traffic Volumes

 Reference ECM B.2.2.C.Figure 9 shows the projected 2040 total traffic volumes, which are the sum of 2040 background traffic volumes (from Figure 5) plus the site-generated traffic volumes (from Figure 7).

## LEVEL OF SERVICE ANALYSIS

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 2 shows the level of service delay ranges for signalized and unsignalized intersections.

Table 2: Intersection Levels of Service Delay Ranges

| Level of Service | Signalized Intersections | Unsignalized Intersections <br> (seconds per vehicle) |
| :---: | :---: | :---: |
|  | Average Control Delay <br> (seconds per vehicle) ${ }^{(1)}$ |  |
|  | 10.0 sec or less | 10.0 sec or less |
| C | $20.1-30.0 \mathrm{sec}$ | $10.1-15.0 \mathrm{sec}$ |
| D | $35.1-55.0 \mathrm{sec}$ | $15.1-25.0 \mathrm{sec}$ |
| E | $55.1-80.0 \mathrm{sec}$ | $25.1-35.0 \mathrm{sec}$ |
| F | 80.1 sec or more | $35.1-50.0 \mathrm{sec}$ |

(1) For unsignalized intersections if $\mathrm{V} / \mathrm{C}$ ratio is greater than 1.0 the level of service is LOS F regardless of the projected average control delay per

The intersections of Struthers Road/Struthers Ranch Road, as well as the site access points, have been analyzed to determine the projected control delay and corresponding levels of service for turning movements. Figure 3 provides the existing levels of service. Figure 4 and Figure 5 provide the background levels of service for the short-term and long-term scenarios, respectively. Figure 8 andFigure 9 provide the levels of service of the short-term and long-term total traffic scenarios, respectively.

## Struthers Road/Struthers Ranch Road

The yielding turning movements at the unsignalized intersection of Struthers Road/Struthers Ranch Road currently operate at LOS C or better during both the morning and evening peak hours. In the short-term future, the yielding turning movements are forecast to continue to operate at LOS C or better during both peak hours with and without the proposed development.

In the long-term future, the yielding turning movements are also forecast to continue to operate at LOS C or better. This intersection is not anticipated to meet signal warrants.

## Site Accesses

In all scenarios, the yielding turning movements operate at LOS B or better during both peak hours.

## AUXILIARY TURN LANES

As mentioned previously, there is a 340-foot southbound left-turn deceleration lane at the intersection of Struthers Road/Struthers Ranch Road. This lane meets the ECM auxiliary lane criteria and does not need to be modified with the development.

There is a 400-foot northbound right-turn deceleration lane (combined lane plus taper length) at this intersection of Struthers Road/Struthers Ranch Road. This auxiliary lane, although it exists, is not currently required per the ECM and is not expected to be required in the future with added site-generated traffic. The turning volume threshold could potentially be met on Sunday mornings with addition of future church traffic. The turn lane already exists and is about 400 feet (lane plus taper). The ECM requirement is 435 feet.

The right-in/right-out access on Struthers Road is anticipated to require a right-turn deceleration lane. This criterion calls for 370 -foot-long deceleration distance (lane plus taper) based on a $45-\mathrm{mph}$ design speed and a 435 -foot-long deceleration distance (lane plus taper) for a $50-\mathrm{mph}$ design speed. The speed limit currently changes just south of the access location. LSC recommends an approximately 200 -foot-long lane plus a 75 -foot-long reverse curve bay taper. This would likely allow the lane to be installed given the limited space adjacent to the developed property to the south. No acceleration lanes are required on Struthers Road.

Although not anticipated to be required based on projected volumes or levels of service, it is recommended that right-of-way be reserved in case Struthers Ranch Road needs minor widening in the future to allow for separate right- and left-turn lanes in the westbound direction.

## VEHICLE QUEUING

At the intersection of Struthers Road/Struthers Ranch Road, there are 250 feet available for vehicle queueing to the east prior to the site access. The $95^{\text {th }}$ percentile queue length for the westbound approach at the intersection is anticipated to be 100 feet, which will not impact the site access. If Struthers Ranch Road were widened in the future to provide separate right- and left-turn lanes on the westbound approach the intersection, then the $95^{\text {th }}$ percentile queue for the westbound left is forecast to be 75 feet, which also would not impact the access to the east.

## PEDESTRIAN AND BICYCLE ACCOMMODATION

A sidewalk exists along Struthers Road adjacent to the site. However, there are currently no sidewalks along Struthers Ranch Road adjacent to the site. It is recommended that a sidewalk be
constructed adjacent to the site on Struthers Ranch Road.
There are no bike lanes on Struthers Road and the roadway is not planned to have bike lanes. However, there are sections of Struthers Road that have paved outside shoulders to accommodate cyclists.

## COUNTY ROAD IMPROVEMENT FEE PROGRAM

## Transportation Impact Fees

Per ECM Appendix B: State what the current applicable Transportation Impact Fees are and what option the developer will be selecting for payment.

The applicant intends to opt out of the PID options and will pay the full fee amount at the time of building permit. The current "full-fee" is $\$ 4,958$ per 1,000 square feet of building floor area. The total fee amount for the 30,140 square foot of commercial buildings is $\$ 149,434$.

## Reimbursable MTCP Improvements

There are no apparent reimbursable improvements programmed in the MTCP in the general vicinity of this site.

## FINDINGS AND CONCLUSIONS

- The site is projected to generate approximately 2,986 external vehicle trips on the average weekday. Approximately half of this traffic is expected to be pass-by traffic.
- During the morning peak hour, approximately 170 vehicles would enter and 98 vehicles would exit the site. During the evening peak hour, approximately 101 vehicles would enter and 112 vehicles would exit the site at the access points.
- The site improvements, landscaping, signage etc. will need to accommodate the driver sight distance lines of sight necessary to meet the prescribed intersection sight distance at Struthers Road/Struthers Rangh Road. Please refer to the Sight Distance section for details.
- Turning movements at the site accesses are projected to operate at acceptable levels of service in all scenarios.
- The westbound approach at the intersection of Struthers Road/Struthers Ranch Road operates at LOS C or better in all scenarios.
- The $95^{\text {th }}$ percentile queues at all study intersection are not projected to impact adjacent intersections.
- See Table 4 for a/summary of recommended improvements.


Update the to identify that a sight distance easement where the line of sight encroaches into the private property

Table 4: Recommended Improvements


LSC TRANSPORTATION CONSULTANTS, INC.

By Colleen Guillotte, P.E., PTOE, RSP
Project Manager
CRG:JCH:jas

Enclosures: Table 3
Figures 1-9
Line of Sight Exhibits
Traffic Count Reports
Level of Service Reports

Please provide a list of references used to create this study.

Table 3


Figures








This meets the criteria for right turn lane.
Update the Auxiliary Turn Lane section and




## Line of Sight Exhibits



## $$
1
$$



## Traffic Counts

## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd-Struthers Ranch Rd AM
Site Code : 00204110
Start Date : 3/3/2020
Page No : 1

Groups Printed- Unshifted

|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| 06:30 AM | 0 | 23 | 0 | 0 | 23 | 2 | 0 | 3 | 0 | 5 | 0 | 22 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 50 |
| 06:45 AM | 0 | 22 | 0 | 0 | 22 | 2 | 0 | 3 | 0 | 5 | 0 | 27 | 0 | 0 | 27 | 0 | 0 | 0 | 0 | 0 | 54 |
| Total | 0 | 45 | 0 | 0 | 45 | 4 | 0 | 6 | 0 | 10 | 0 | 49 | 0 | 0 | 49 | 0 | 0 | 0 | 0 | 0 | 104 |
| 07:00 AM | 1 | 36 | 0 | 0 | 37 | 4 | 0 | 4 | 0 | 8 | 0 | 32 | 1 | 0 | 33 | 0 | 0 | 0 | 0 | 0 | 78 |
| 07:15 AM | 1 | 44 | 0 | 0 | 45 | 4 | 0 | 3 | 0 | 7 | 0 | 54 | 0 | 0 | 54 | 0 | 0 | 0 | 0 | 0 | 106 |
| 07:30 AM | 0 | 51 | 0 | 0 | 51 | 1 | 0 | 7 | 0 | 8 | 0 | 47 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 106 |
| 07:45 AM | 1 | 56 | 0 | 0 | 57 | 3 | 0 | 4 | 0 | 7 | 0 | 60 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 124 |
| Total | 3 | 187 | 0 | 0 | 190 | 12 | 0 | 18 | 0 | 30 | 0 | 193 | 1 | 0 | 194 | 0 | 0 | 0 | 0 | 0 | 414 |
| 08:00 AM | 4 | 47 | 0 | 0 | 51 | 6 | 0 | 1 | 0 | 7 | 0 | 51 | 1 | 0 | 52 | 0 | 0 | 0 | 0 | 0 | 110 |
| 08:15 AM | 1 | 41 | 0 | 0 | 42 | 1 | 0 | 5 | 0 | 6 | 0 | 58 | 1 | 0 | 59 | 0 | 0 | 0 | 0 | 0 | 107 |
| Grand Total | 8 | 320 | 0 | 0 | 328 | 23 | 0 | 30 | 0 | 53 | 0 | 351 | 3 | 0 | 354 | 0 | 0 | 0 | 0 | 0 | 735 |
| Apprch \% | 2.4 | 97.6 | 0 | 0 |  | 43.4 | 0 | 56.6 | 0 |  | 0 | 99.2 | 0.8 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| Total \% | 1.1 | 43.5 | 0 | 0 | 44.6 | 3.1 | 0 | 4.1 | 0 | 7.2 | 0 | 47.8 | 0.4 | 0 | 48.2 | 0 | 0 | 0 | 0 | 0 |  |

## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd-Struthers Ranch Rd AM
Site Code : 00204110
Start Date : 3/3/2020
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|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| Peak Hour Analysis From 6:30:00 AM to 8:15:00 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 7:30:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:30:00 AM | 0 | 51 | 0 | 0 | 51 | 1 | 0 | 7 | 0 | 8 | 0 | 47 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 106 |
| 7:45:00 AM | 1 | 56 | 0 | 0 | 57 | 3 | 0 | 4 | 0 | 7 | 0 | 60 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 124 |
| 8:00:00 AM | 4 | 47 | 0 | 0 | 51 | 6 | 0 | 1 | 0 | 7 | 0 | 51 | 1 | 0 | 52 | 0 | 0 | 0 | 0 | 0 | 110 |
| 8:15:00 AM | 1 | 41 | 0 | 0 | 42 | 1 | 0 | 5 | 0 | 6 | 0 | 58 | 1 | 0 | 59 | 0 | 0 | 0 | 0 | 0 | 107 |
| Total Volume | 6 | 195 | 0 | 0 | 201 | 11 | 0 | 17 | 0 | 28 | 0 | 216 | 2 | 0 | 218 | 0 | 0 | 0 | 0 | 0 | 447 |
| \% App. Total | 3 | 97 | 0 | 0 |  | 39.3 | 0 | 60.7 | 0 |  | 0 | 99.1 | 0.9 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 375 | . 871 | . 000 | . 000 | . 882 | . 458 | . 000 | . 607 | . 000 | . 875 | . 000 | . 900 | . 500 | . 000 | . 908 | . 000 | . 000 | . 000 | . 000 | . 000 | . 901 |

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Page No : 4

|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total |  |
| Peak Hour Analysis From 6:30:00 AM to 8:15:00 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 7:15:00 Am |  |  |  |  | 7:00:00 AM |  |  |  |  | 7:30:00 AM |  |  |  |  | 6:30:00 AM |  |  |  |  |  |
| +0 mins. | 1 | 44 | 0 | 0 | 45 | 4 | 0 | 4 | 0 | 8 | 0 | 47 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 |  |
| +5 mins. | 0 | 51 | 0 | 0 | 51 | 4 | 0 | 3 | 0 | 7 | 0 | 60 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 |  |
| +10 mins. | 1 | 56 | 0 | 0 | 57 | 1 | 0 | 7 | 0 | 8 | 0 | 51 | 1 | 0 | 52 | 0 | 0 | 0 | 0 | 0 |  |
| +15 mins. | 4 | 47 | 0 | 0 | 51 | 3 | 0 | 4 | 0 | 7 | 0 | 58 | 1 | 0 | 59 | 0 | 0 | 0 | 0 | 0 |  |
| Total Volume | 6 | 198 | 0 | 0 | 204 | 12 | 0 | 18 | 0 | 30 | 0 | 216 | 2 | 0 | 218 | 0 | 0 | 0 | 0 | 0 |  |
| \% App. Total | 2.9 | 97.1 | 0 | 0 |  | 40 | 0 | 60 | 0 |  | 0 | 99.1 | 0.9 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 375 | . 884 | . 000 | . 000 | . 895 | . 750 | . 000 | . 643 | . 000 | . 938 | . 000 | . 900 | . 500 | . 000 | . 908 | . 000 | . 000 | . 000 | . 000 | . 000 |  |

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd - Struthers Ranch Rd AM
Site Code : 00204110
Start Date : 3/3/2020
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## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd-Struthers Ranch Rd PM
Site Code : 00204110
Start Date : 3/3/2020
Page No : 1

Groups Printed- Unshifted

|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| 04:00 PM | 4 | 106 | 0 | 0 | 110 | 0 | 0 | 3 | 0 | 3 | 0 | 56 | 3 | 0 | 59 | 0 | 0 | 0 | 0 | 0 | 172 |
| 04:15 PM | 4 | 98 | 0 | 0 | 102 | 1 | 0 | 3 | 0 | 4 | 0 | 52 | 3 | 0 | 55 | 0 | 0 | 0 | 0 | 0 | 161 |
| 04:30 PM | 4 | 82 | 0 | 0 | 86 | 1 | 0 | 0 | 0 | 1 | 0 | 58 | 2 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 147 |
| 04:45 PM | 6 | 75 | 0 | 0 | 81 | 0 | 0 | 3 | 0 | 3 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 147 |
| Total | 18 | 361 | 0 | 0 | 379 | 2 | 0 | 9 | 0 | 11 | 0 | 227 | 10 | 0 | 237 | 0 | 0 | 0 | 0 | 0 | 627 |
| 05:00 PM | 6 | 120 | 0 | 0 | 126 | 1 | 0 | 1 | 0 | 2 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 191 |
| 05:15 PM | 5 | 101 | 0 | 0 | 106 | 3 | 0 | 6 | 0 | 9 | 0 | 53 | 4 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 172 |
| 05:30 PM | 6 | 109 | 0 | 0 | 115 | 2 | 0 | 2 | 0 | 4 | 0 | 57 | 3 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 179 |
| 05:45 PM | 7 | 77 | 0 | 0 | 84 | 0 | 0 | 1 | 0 | 1 | 0 | 51 | 1 | 0 | 52 | 0 | 0 | 0 | 0 | 0 | 137 |
| Total | 24 | 407 | 0 | 0 | 431 | 6 | 0 | 10 | 0 | 16 | 0 | 222 | 10 | 0 | 232 | 0 | 0 | 0 | 0 | 0 | 679 |
| Grand Total | 42 | 768 | 0 | 0 | 810 | 8 | 0 | 19 | 0 | 27 | 0 | 449 | 20 | 0 | 469 | 0 | 0 | 0 | 0 | 0 | 1306 |
| Apprch \% | 5.2 | 94.8 | 0 | 0 |  | 29.6 | 0 | 70.4 | 0 |  | 0 | 95.7 | 4.3 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| Total \% | 3.2 | 58.8 | 0 | 0 | 62 | 0.6 | 0 | 1.5 | 0 | 2.1 | 0 | 34.4 | 1.5 | 0 | 35.9 | 0 | 0 | 0 | 0 | 0 |  |

## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd - Struthers Ranch Rd PM
Site Code : 00204110
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|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 4:45:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:45:00 PM | 6 | 75 | 0 | 0 | 81 | 0 | 0 | 3 | 0 | 3 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 147 |
| 5:00:00 PM | 6 | 120 | 0 | 0 | 126 | 1 | 0 | 1 | 0 | 2 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 191 |
| 5:15:00 PM | 5 | 101 | 0 | 0 | 106 | 3 | 0 | 6 | 0 | 9 | 0 | 53 | 4 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 172 |
| 5:30:00 PM | 6 | 109 | 0 | 0 | 115 | 2 | 0 | 2 | 0 | 4 | 0 | 57 | 3 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 179 |
| Total Volume | 23 | 405 | 0 | 0 | 428 | 6 | 0 | 12 | 0 | 18 | 0 | 232 | 11 | 0 | 243 | 0 | 0 | 0 | 0 | 0 | 689 |
| \% App. Total | 5.4 | 94.6 | 0 | 0 |  | 33.3 | 0 | 66.7 | 0 |  | 0 | 95.5 | 4.5 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 958 | . 844 | . 000 | . 000 | . 849 | . 500 | . 000 | . 500 | . 000 | . 500 | . 000 | . 951 | . 688 | . 000 | . 964 | . 000 | . 000 | . 000 | . 000 | . 000 | . 902 |

## LSC Transportation Consultants, Inc.

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## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd-Struthers Ranch Rd PM
Site Code : 00204110
Start Date : 3/3/2020
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|  | Struthers Rd Southbound |  |  |  |  | Struthers Ranch Rd Westbound |  |  |  |  | Struthers Rd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 5:00:00 PM |  |  |  |  | 4:45:00 PM |  |  |  |  | 4:30:00 PM |  |  |  |  | 4:00:00 PM |  |  |  |  |  |
| +0 mins. | 6 | 120 | 0 | 0 | 126 | 0 | 0 | 3 | 0 | 3 | 0 | 58 | 2 | 0 | 60 | 0 | 0 | 0 | 0 | 0 |  |
| +5 mins. | 5 | 101 | 0 | 0 | 106 | 1 | 0 | 1 | 0 | 2 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 |  |
| +10 mins. | 6 | 109 | 0 | 0 | 115 | 3 | 0 | 6 | 0 | 9 | 0 | 61 | 2 | 0 | 63 | 0 | 0 | 0 | 0 | 0 |  |
| +15 mins. | 7 | 77 | 0 | 0 | 84 | 2 | 0 | 2 | 0 | 4 | 0 | 53 | 4 | 0 | 57 | 0 | 0 | 0 | 0 | 0 |  |
| Total Volume | 24 | 407 | 0 | 0 | 431 | 6 | 0 | 12 | 0 | 18 | 0 | 233 | 10 | 0 | 243 | 0 | 0 | 0 | 0 | 0 |  |
| \% App. Total | 5.6 | 94.4 | 0 | 0 |  | 33.3 | 0 | 66.7 | 0 |  | 0 | 95.9 | 4.1 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 857 | . 848 | . 000 | . 000 | . 855 | . 500 | . 000 | . 500 | . 000 | . 500 | . 000 | . 955 | . 625 | . 000 | . 964 | . 000 | . 000 | . 000 | . 000 | . 000 |  |

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Struthers Rd - Struthers Ranch Rd PM
Site Code : 00204110
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| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 0.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | F |  | 个个 | $\mathbf{F}$ | º | 个个 |
| Traffic Vol，veh／h | 11 | 17 | 216 | 2 | 6 | 195 |
| Future Vol，veh／h | 11 | 17 | 216 | 2 | 6 | 195 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 88 | 88 | 91 | 91 | 88 | 88 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 19 | 237 | 2 | 7 | 222 |


| Major／Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 362 | 119 | 0 | 0 | 239 | 0 |
| Stage 1 | 237 | － | － | － | － | － |
| Stage 2 | 125 | － | － | － | － | － |
| Critical Hdwy | 6.84 | 6.94 | － | － | 4.14 | － |
| Critical Hdwy Stg 1 | 5.84 | － | － | － | － | － |
| Critical Hdwy Stg 2 | 5.84 | － | － | － | － | － |
| Follow－up Hdwy | 3.52 | 3.32 | － | － | 2.22 | － |
| Pot Cap－1 Maneuver | 610 | 910 | － | － | 1325 | － |
| Stage 1 | 780 | － | － | － | － | － |
| Stage 2 | 887 | － | － | － | － | － |
| Platoon blocked，\％ |  |  | － | － |  | － |
| Mov Cap－1 Maneuver | 607 | 910 | － | － | 1325 | － |
| Mov Cap－2 Maneuver | 607 | － | － | － | － | － |
| Stage 1 | 780 | － | － | － | － | － |
| Stage 2 | 883 | － | － | － | － | － |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay，s | 9.9 |  | 0 |  | 0.2 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity（veh／h） |  | － | － | 761 | 1325 | － |
| HCM Lane V／C Ratio |  | － | － | 0.042 | 0.005 | － |
| HCM Control Delay（s） |  | － | － | 9.9 | 7.7 | － |
| HCM Lane LOS |  | － | － | A | A | － |
| HCM 95th \％tile Q（veh） |  | － | － | 0.1 | 0 | － |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 0.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | 个个 | $\mathbf{F}$ | i | 个个 |
| Traffic Vol，veh／h | 6 | 12 | 232 | 11 | 23 | 405 |
| Future Vol，veh／h | 6 | 12 | 232 | 11 | 23 | 405 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 50 | 50 | 92 | 92 | 85 | 85 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 24 | 252 | 12 | 27 | 476 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement W | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | ¢4 | F | \% | 个个 |
| Traffic Vol, veh/h | 14 | 22 | 222 | 7 | 14 | 204 |
| Future Vol, veh/h | 14 | 22 | 222 | 7 | 14 | 204 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 88 | 88 | 91 | 91 | 88 | 88 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 16 | 25 | 244 | 8 | 16 | 232 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 392 | 122 | 0 | 0 | 252 | 0 |
| Stage 1 | 244 | - | - | - | - | - |
| Stage 2 | 148 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 585 | 906 | - | - | 1310 | - |
| Stage 1 | 774 | - | - | - | - | - |
| Stage 2 | 864 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 578 | 906 | - | - | 1310 | - |
| Mov Cap-2 Maneuver | 578 | - | - | - | - | - |
| Stage 1 | 774 | - | - | - | - | - |
| Stage 2 | 854 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.1 |  | 0 |  | 0.5 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 742 | 1310 | - |
| HCM Lane V/C Ratio |  | - | - | 0.055 | 0.012 | - |
| HCM Control Delay (s) |  | - | - | 10.1 | 7.8 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.2 | 0 | - |



| Major/Minor | Major1 |  | Major2 |  |  | Minor1 |  |  | Minor2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 36 | 0 | 0 | 10 | 0 | 0 | 85 | 80 | 10 | 80 | 80 | 36 |  |
| Stage 1 | - | - | - | - | - |  | 44 | 44 | - | 36 | 36 |  |  |
| Stage 2 | - | - | - | - | - | - | 41 | 36 | - | 44 | 44 |  |  |
| Critical Hdwy | 4.12 | - | - | 4.12 | - |  | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | - 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1575 | - | - | 1610 | - | - | 901 | 810 | 1071 | 908 | 810 | 1037 |  |
| Stage 1 | - | - | - | - | - |  | 970 | 858 | - | 980 | 865 | - |  |
| Stage 2 | - | - | - | - | - |  | 974 | 865 | - | 970 | 858 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1575 | - | - | 1610 | - | - | 885 | 801 | 1071 | 901 | 801 | 1037 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 885 | 801 | - | 901 | 801 | - |  |
| Stage 1 | - | - | - | - | - |  | 959 | 849 | - | 969 | 865 | - |  |
| Stage 2 | - | - | - | - | - |  | 964 | 865 | - | 959 | 849 | - |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 4.5 |  |  | 0 |  |  | 0 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  |  | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | - | 1575 | - | - | 1610 | - | - | 1037 |  |  |  |  |
| HCM Lane V/C Ratio |  |  | 0.011 | - | - | - | - - | - | 0.01 |  |  |  |  |
| HCM Control Delay (s) |  | 0 | 7.3 | 0 | - | 0 | - | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | - | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | - | 0 | - | - | 0 | - | - | 0 |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.3 |  |  |  |  |  |
| Movement W | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * ${ }^{\text {F }}$ |  | 44 | 「 | ${ }^{1}$ | 44 |
| Traffic Vol, veh/h | 10 | 19 | 244 | 15 | 29 | 245 |
| Future Vol, veh/h | 10 | 19 | 244 | 15 | 29 | 245 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 50 | 50 | 92 | 92 | 85 | 85 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 38 | 265 | 16 | 34 | 288 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 477 | 133 | 0 | 0 | 281 | 0 |
| Stage 1 | 265 | - | - | - | - | - |
| Stage 2 | 212 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 517 | 892 | - | - | 1278 | - |
| Stage 1 | 755 | - | - | - | - | - |
| Stage 2 | 803 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 503 | 892 | - | - | 1278 | - |
| Mov Cap-2 Maneuver | 503 | - | - | - | - | - |
| Stage 1 | 755 | - | - | - | - | - |
| Stage 2 | 781 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.6 |  | 0 |  | 0.8 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 704 | 1278 | - |
| HCM Lane V/C Ratio |  | - | - | 0.082 | 0.027 | - |
| HCM Control Delay (s) |  | - | - | 10.6 | 7.9 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0.1 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  | \$ |  |  |
| Traffic Vol, veh/h | 10 | 34 | 0 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |  |
| Future Vol, veh/h | 10 | 34 | 0 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 13 | 44 | 0 | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 14 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | F |  | 个个 | $\mathbf{F}$ | 〒 | 个个 |
| Traffic Vol，veh／h | 14 | 22 | 321 | 7 | 14 | 290 |
| Future Vol，veh／h | 14 | 22 | 321 | 7 | 14 | 290 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 88 | 88 | 91 | 91 | 88 | 88 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 16 | 25 | 353 | 8 | 16 | 330 |


| Major／Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 550 | 177 | 0 | 0 | 361 | 0 |
| Stage 1 | 353 | － | － | － | － | － |
| Stage 2 | 197 | － | － | － | － | － |
| Critical Hdwy | 6.84 | 6.94 | － | － | 4.14 | － |
| Critical Hdwy Stg 1 | 5.84 | － | － | － | － | － |
| Critical Hdwy Stg 2 | 5.84 | － | － | － | － | － |
| Follow－up Hdwy | 3.52 | 3.32 | － | － | 2.22 | － |
| Pot Cap－1 Maneuver | 465 | 835 | － | － | 1194 | － |
| Stage 1 | 682 | － | － | － | － | － |
| Stage 2 | 817 | － | － | － | － | － |
| Platoon blocked，\％ |  |  | － | － |  | － |
| Mov Cap－1 Maneuver | 459 | 835 | － | － | 1194 | － |
| Mov Cap－2 Maneuver | 459 | － | － | － | － | － |
| Stage 1 | 682 | － | － | － | － | － |
| Stage 2 | 806 | － | － | － | － | － |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay，s | 11.1 |  | 0 |  | 0.4 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity（veh／h） |  | － | － | 633 | 1194 | － |
| HCM Lane V／C Ratio |  | － | － | 0.065 | 0.013 | － |
| HCM Control Delay（s） |  | － | － | 11.1 | 8.1 | － |
| HCM Lane LOS |  | － | － | B | A | － |
| HCM 95th \％tile Q（veh） |  | － | － | 0.2 | 0 | － |



| Major/Minor | Major1 |  | Major2 |  |  | Minor1 |  |  | Minor2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 36 | 0 | 0 | 10 | 0 | 0 | 85 | 80 | 10 | 80 | 80 | 36 |  |
| Stage 1 | - | - | - | - | - |  | 44 | 44 | - | 36 | 36 |  |  |
| Stage 2 | - | - | - | - | - | - | 41 | 36 | - | 44 | 44 |  |  |
| Critical Hdwy | 4.12 | - | - | 4.12 | - |  | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | - 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1575 | - | - | 1610 | - | - | 901 | 810 | 1071 | 908 | 810 | 1037 |  |
| Stage 1 | - | - | - | - | - |  | 970 | 858 | - | 980 | 865 | - |  |
| Stage 2 | - | - | - | - | - |  | 974 | 865 | - | 970 | 858 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1575 | - | - | 1610 | - | - | 885 | 801 | 1071 | 901 | 801 | 1037 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 885 | 801 | - | 901 | 801 | - |  |
| Stage 1 | - | - | - | - | - |  | 959 | 849 | - | 969 | 865 | - |  |
| Stage 2 | - | - | - | - | - |  | 964 | 865 | - | 959 | 849 | - |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 4.5 |  |  | 0 |  |  | 0 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  |  | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | - | 1575 | - | - | 1610 | - | - | 1037 |  |  |  |  |
| HCM Lane V/C Ratio |  |  | 0.011 | - | - | - | - - | - | 0.01 |  |  |  |  |
| HCM Control Delay (s) |  | 0 | 7.3 | 0 | - | 0 | - | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | - | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | - | 0 | - | - | 0 | - | - | 0 |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement W | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * ${ }^{\text {F }}$ |  | 44 | 「 | ${ }^{1}$ | 44 |
| Traffic Vol, veh/h | 10 | 19 | 345 | 15 | 29 | 350 |
| Future Vol, veh/h | 10 | 19 | 345 | 15 | 29 | 350 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 50 | 50 | 92 | 92 | 85 | 85 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 38 | 375 | 16 | 34 | 412 |


| Major/Minor | Minor1 |  | ajor1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 649 | 188 | 0 | 0 | 391 | 0 |
| Stage 1 | 375 | - | - | - | - | - |
| Stage 2 | 274 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 402 | 822 | - | - | 1164 | - |
| Stage 1 | 665 | - | - | - | - | - |
| Stage 2 | 747 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 390 | 822 | - | - | 1164 | - |
| Mov Cap-2 Maneuver | 390 | - | - | - | - | - |
| Stage 1 | 665 | - | - | - | - | - |
| Stage 2 | 725 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 11.7 |  | 0 |  | 0.6 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 595 | 1164 | - |
| HCM Lane V/C Ratio |  | - |  | 0.097 | 0.029 | - |
| HCM Control Delay (s) |  | - | - | 11.7 | 8.2 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0.1 | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 3.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | F |  | 个个 | $\mathbf{F}$ | 〒 | 个个 |
| Traffic Vol，veh／h | 61 | 30 | 237 | 15 | 102 | 180 |
| Future Vol，veh／h | 61 | 30 | 237 | 15 | 102 | 180 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 88 | 88 | 91 | 91 | 88 | 88 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 69 | 34 | 260 | 16 | 116 | 205 |




| Major/Minor | Major1 |  | Major2 |  |  | Minor1 |  |  | Minor2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 33 | 0 | 0 | 133 | 0 | 0 | 149 | 144 | 71 | 145 | 206 | 33 |  |
| Stage 1 | - | - | - | - | - |  | 105 | 105 |  | 39 | 39 |  |  |
| Stage 2 | - | - | - | - | - | - | 44 | 39 | - | 106 | 167 |  |  |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | - 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1579 | - | - | 1452 | - | - | 819 | 747 | 991 | 824 | 691 | 1041 |  |
| Stage 1 | - | - | - | - | - |  | 901 | 808 | - | 976 | 862 | - |  |
| Stage 2 | - | - | - | - | - |  | 970 | 862 | - | 900 | 760 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1579 | - | - | 1452 | - | - | 803 | 737 | 991 | 814 | 681 | 1041 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 803 | 737 | - | 814 | 681 | - |  |
| Stage 1 | - | - | - | - | - |  | 890 | 798 | - | 964 | 860 | - |  |
| Stage 2 | - | - | - | - | - | - | 959 | 860 | - | 888 | 751 | - |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.8 |  |  | 0.5 |  |  | 9.9 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | 806 | 1579 | - | - | 1452 | - | - | 1041 |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.091 | 0.011 | - | - | 0.002 | - | - | 0.01 |  |  |  |  |
| HCM Control Delay (s) |  | 9.9 | 7.3 | 0 | - | 7.5 | 0 | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | A | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.3 | 0 | - | - | 0 | - | - | 0 |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow \uparrow$ | 付 |  |  | $\mathbf{F}$ |
| Traffic Vol, veh/h | 0 | 241 | 212 | 67 | 0 | 40 |
| Future Vol, veh/h | 0 | 241 | 212 | 67 | 0 | 40 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | - | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 87 | 87 | 87 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 277 | 244 | 86 | 0 | 51 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 4.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  |  | $\uparrow \uparrow$ | $\mathbf{F}$ | 个 | 个个 |
| Traffic Vol，veh／h | 59 | 30 | 274 | 21 | 79 | 221 |
| Future Vol，veh／h | 59 | 30 | 274 | 21 | 79 | 221 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 50 | 50 | 92 | 92 | 85 | 85 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 118 | 60 | 298 | 23 | 93 | 260 |


| Major／Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 614 | 149 | 0 | 0 | 321 | 0 |
| Stage 1 | 298 | － | － | － | － | － |
| Stage 2 | 316 | － | － | － | － | － |
| Critical Hdwy | 6.84 | 6.94 | － | － | 4.14 | － |
| Critical Hdwy Stg 1 | 5.84 | － | － | － | － | － |
| Critical Hdwy Stg 2 | 5.84 | － | － | － | － | － |
| Follow－up Hdwy | 3.52 | 3.32 | － | － | 2.22 | － |
| Pot Cap－1 Maneuver | 424 | 871 | － | － | 1236 | － |
| Stage 1 | 727 | － | － | － | － | － |
| Stage 2 | 712 | － | － | － | － | － |
| Platoon blocked，\％ |  |  | － | － |  | － |
| Mov Cap－1 Maneuver | 392 | 871 | － | － | 1236 | － |
| Mov Cap－2 Maneuver | 392 | － | － | － | － | － |
| Stage 1 | 727 | － | － | － | － | － |
| Stage 2 | 659 | － | － | － | － | － |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay，s | 16.8 |  | 0 |  | 2.1 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity（veh／h） |  | － | － | 481 | 1236 | － |
| HCM Lane V／C Ratio |  | － | － | 0.37 | 0.075 | － |
| HCM Control Delay（s） |  | － | － | 16.8 | 8.1 | － |
| HCM Lane LOS |  | － | － | C | A | － |
| HCM 95th \％tile Q（veh） |  | － | － | 1.7 | 0.2 | － |



| Major/Minor | Major1 | Major2 |  |  |  |  | Minor1 | Minor2 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 23 | 0 | 0 | 115 | 0 | 0 | 135 | 128 | 79 | 128 | 164 | 23 |  |
| Stage 1 | - | - | - | - | - | - | 105 | 105 | - | 23 | 23 |  | - |
| Stage 2 | - | - | - | - | - | - | 30 | 23 | - | 105 | 141 | - | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 |  | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1592 | - | - | 1474 | - | - | 836 | 763 | 981 | 845 | 729 | 1054 |  |
| Stage 1 | - | - | - | - | - | - | 901 | 808 | - | 995 | 876 | - | - |
| Stage 2 | - | - | - | - | - | - | 987 | 876 | - | 901 | 780 |  | - |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1592 | - | - | 1474 | - | - | 819 | 756 | 981 | 838 | 722 | 1054 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 819 | 756 | - | 838 | 722 | - | - |
| Stage 1 | - | - | - | - | - | - | 893 | 801 | - | 986 | 876 |  | - |
| Stage 2 | - | - | - | - | - | - | 974 | 876 | - | 892 | 773 |  | - |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.7 |  |  | 0 |  |  | 9.9 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | 821 | 1592 | - | - | 1474 | - | - | 1054 |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.097 | 0.008 | - | - | - | - | - | 0.013 |  |  |  |  |
| HCM Control Delay (s) |  | 9.9 | 7.3 | 0 | - | 0 | - | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | - | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.3 | 0 | - | - | 0 | - | - | 0 |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow \uparrow$ | 付 |  |  | $\mathbf{F}$ |
| Traffic Vol, veh/h | 0 | 283 | 244 | 44 | 0 | 51 |
| Future Vol, veh/h | 0 | 283 | 244 | 44 | 0 | 51 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | - | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 87 | 87 | 87 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 325 | 280 | 56 | 0 | 65 |



| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |  |
| Lane Configurations | F |  | 个个 | $\mathbf{7}$ | 〒 | 个个 |  |
| Traffic Vol，veh／h | 61 | 30 | 336 | 15 | 102 | 266 |  |
| Future Vol，veh／h | 61 | 30 | 336 | 15 | 102 | 266 |  |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Free | Free | Free | Free |  |
| RT Channelized | - | None | - | None | - | None |  |
| Storage Length | 0 | - | - | 255 | 340 | - |  |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |  |
| Grade，\％ | 0 | - | 0 | - | - | 0 |  |
| Peak Hour Factor | 88 | 88 | 91 | 91 | 88 | 88 |  |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 69 | 34 | 369 | 16 | 116 | 302 |  |




| Major/Minor | Major1 | Major2 |  |  |  |  | Minor1 | Minor2 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 33 | 0 | 0 | 133 | 0 | 0 | 149 | 144 | 71 | 145 | 206 | 33 |  |
| Stage 1 | - | - | - | - | - | - | 105 | 105 | - | 39 | 39 |  | - |
| Stage 2 | - | - | - | - | - | - | 44 | 39 | - | 106 | 167 | - | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 |  | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 |  | - |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1579 | - | - | 1452 | - | - | 819 | 747 | 991 | 824 | 691 | 1041 |  |
| Stage 1 | - | - | - | - | - | - | 901 | 808 | - | 976 | 862 | - | - |
| Stage 2 | - | - | - | - | - | - | 970 | 862 | - | 900 | 760 |  | - |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1579 | - | - | 1452 | - | - | 803 | 737 | 991 | 814 | 681 | 1041 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 803 | 737 | - | 814 | 681 | - | - |
| Stage 1 | - | - | - | - | - | - | 890 | 798 | - | 964 | 860 |  | - |
| Stage 2 | - | - | - | - | - | - | 959 | 860 | - | 888 | 751 |  | - |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.8 |  |  | 0.5 |  |  | 9.9 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | 806 | 1579 | - | - | 1452 | - | - | 1041 |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.091 | 0.011 | - | - | 0.002 | - | - | 0.01 |  |  |  |  |
| HCM Control Delay (s) |  | 9.9 | 7.3 | 0 | - | 7.5 | 0 | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | A | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.3 | 0 | - | - | 0 | - | - | 0 |  |  |  |  |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 4.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  |  | $\uparrow \uparrow$ | $\mathbf{F}$ | 个 | 个个 |
| Traffic Vol，veh／h | 59 | 30 | 375 | 21 | 79 | 329 |
| Future Vol，veh／h | 59 | 30 | 375 | 21 | 79 | 329 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 255 | 340 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 50 | 50 | 92 | 92 | 85 | 85 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 118 | 60 | 408 | 23 | 93 | 387 |


| Major／Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 788 | 204 | 0 | 0 | 431 | 0 |
| Stage 1 | 408 | － | － | － | － | － |
| Stage 2 | 380 | － | － | － | － | － |
| Critical Hdwy | 6.84 | 6.94 | － | － | 4.14 | － |
| Critical Hdwy Stg 1 | 5.84 | － | － | － | － | － |
| Critical Hdwy Stg 2 | 5.84 | － | － | － | － | － |
| Follow－up Hdwy | 3.52 | 3.32 | － | － | 2.22 | － |
| Pot Cap－1 Maneuver | 328 | 803 | － | － | 1125 | － |
| Stage 1 | 640 | － | － | － | － | － |
| Stage 2 | 661 | － | － | － | － | － |
| Platoon blocked，\％ |  |  | － | － |  | － |
| Mov Cap－1 Maneuver | 301 | 803 | － | － | 1125 | － |
| Mov Cap－2 Maneuver | 301 | － | － | － | － | － |
| Stage 1 | 640 | － | － | － | － | － |
| Stage 2 | 606 | － | － | － | － | － |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay，s | 22.5 |  | 0 |  | 1.6 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity（veh／h） |  | － | － | 381 | 1125 | － |
| HCM Lane V／C Ratio |  | － | － | 0.467 | 0.083 | － |
| HCM Control Delay（s） |  | － | － | 22.5 | 8.5 | － |
| HCM Lane LOS |  | － | － | C | A | － |
| HCM 95th \％tile Q（veh） |  | － | － | 2.4 | 0.3 | － |



| Major/Minor | Major1 | Major2 |  |  |  |  | Minor1 | Minor2 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 23 | 0 | 0 | 115 | 0 | 0 | 135 | 128 | 79 | 128 | 164 | 23 |  |
| Stage 1 | - | - | - | - | - | - | 105 | 105 | - | 23 | 23 |  | - |
| Stage 2 | - | - | - | - | - | - | 30 | 23 | - | 105 | 141 | - | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 |  | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - | - |
| Follow-up Hdwy | 2.218 | - |  | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1592 | - | - | 1474 | - | - | 836 | 763 | 981 | 845 | 729 | 1054 |  |
| Stage 1 | - | - | - | - | - | - | 901 | 808 | - | 995 | 876 | - | - |
| Stage 2 | - | - | - | - | - | - | 987 | 876 | - | 901 | 780 |  | - |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1592 | - | - | 1474 | - | - | 819 | 756 | 981 | 838 | 722 | 1054 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 819 | 756 | - | 838 | 722 | - | - |
| Stage 1 | - | - | - | - | - | - | 893 | 801 | - | 986 | 876 |  | - |
| Stage 2 | - | - | - | - | - | - | 974 | 876 | - | 892 | 773 |  | - |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.7 |  |  | 0 |  |  | 9.9 |  |  | 8.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | A |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | 821 | 1592 | - | - | 1474 | - | - | 1054 |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.097 | 0.008 | - | - | - | - | - | 0.013 |  |  |  |  |
| HCM Control Delay (s) |  | 9.9 | 7.3 | 0 | - | 0 | - | - | 8.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | A | - | A | - | - | A |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.3 | 0 | - | - | 0 | - | - | 0 |  |  |  |  |




## Traffic Impact Study_V1.pdf Markup Summary

| dsdlaforce (22) |  |  |
| :---: | :---: | :---: |
|  | Subject: Highlight <br> Page Label: 6 <br> Author: dsdlaforce <br> Date: 3/10/2021 6:00:19 PM <br> Status: <br> Color: <br> Layer: <br> Space: | a right-in/right-out access onto Struthers Road |
|  | Subject: Callout <br> Page Label: 6 <br> Author: dsdlaforce <br> Date: 3/10/2021 6:10:36 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Discuss this deviation request with your client. Staff does not see an undue hardship to justify a deviation request for access on Struthers Road. With access available at Struthers Ranch Rd, this deviation request will likely be denied. <br> If the request is withdrawn then update the TIS analysis based on a single access from Struthers Ranch Road. <br> Please note: If the applicant chooses to pursue the request, staff encourages you to submit the deviation request form prior to the 2nd resubmittal. The determination to approve or deny the deviation impacts the traffic report. |
|  | Subject: Callout <br> Page Label: 7 <br> Author: dsdlaforce <br> Date: 3/10/2021 6:38:06 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Can this statement be verified? Explain how the future volumes were calibrated/modified. |
| site-generated traffic. The turnin mornings with addition of future ch (lane plus taper). The ECM requiren The right-in/right-out access on Strt lane. This criterion calls for $370-\mathrm{fc}$ design speed. The speed limit $c$ recommends an approximately 200 | Subject: Highlight <br> Page Label: 11 <br> Author: dsdlaforce <br> Date: 3/11/2021 2:50:19 PM <br> Status: <br> Color: <br> Layer: <br> Space: | e right-in/right-out access |
|  | Subject: Highlight <br> Page Label: 23 <br> Author: dsdlaforce <br> Date: 3/11/2021 2:51:07 PM <br> Status: <br> Color: <br> Layer: <br> Space: |  |




| Update | Subject: Callout <br> Page Label: 5 <br> Author: dsdlaforce <br> Date: 3/11/2021 5:20:02 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Update |
| :---: | :---: | :---: |
| $\square$ | Subject: Highlight <br> Page Label: 5 <br> Author: dsdlaforce <br> Date: 3/11/2021 5:20:07 PM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
|  | Subject: Highlight <br> Page Label: 5 <br> Author: dsdlaforce <br> Date: 3/11/2021 5:20:08 PM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
| $\longrightarrow$ | Subject: Highlight <br> Page Label: 5 <br> Author: dsdlaforce <br> Date: 3/11/2021 5:20:11 PM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
|  | Subject: Callout <br> Page Label: 15 <br> Author: dsdlaforce <br> Date: 3/11/2021 7:43:57 AM <br> Status: <br> Color: <br> Layer: <br> Space: | include the reference in the appendix. The percentages seems high. |
| Ipackman (9) |  |  |
| 景 | Subject: Callout <br> Page Label: 2 <br> Author: lpackman <br> Date: 2/25/2021 11:11:27 AM <br> Status: <br> Color: <br> Layer: <br> Space: | Please add the following: "PCD File No. VR-2101" |


| Livi piesuriven signs, parking out 18 inches. river's eye" is ie site grading | Subject: Highlight <br> Page Label: 6 <br> Author: lpackman <br> Date: 2/25/2021 11:33:56 AM <br> Status: <br> Color: <br> Layer: <br> Space: | 18 inches |
| :---: | :---: | :---: |
| $\square=\square$ $\square=\square$ | Subject: Callout <br> Page Label: 7 <br> Author: Ipackman <br> Date: 2/25/2021 11:57:43 AM <br> Status: <br> Color: <br> Layer: <br> Space: | Please revise to include a description of the required sight distance for the access point on Struthers Ranch Rd. |
|  | Subject: Text Box <br> Page Label: 13 <br> Author: Ipackman <br> Date: 2/25/2021 2:29:47 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Please provide a list of references used to create this study. |
| $\square$ $=$ $=\sim$ | Subject: Callout <br> Page Label: 9 <br> Author: Ipackman <br> Date: 2/25/2021 4:10:45 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Describe the reason behind the $2 \%$ traffic growth. |
|  | Subject: Callout <br> Page Label: 23 <br> Author: Ipackman <br> Date: 2/25/2021 4:36:18 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Per Struthers Ranch Filing No. 4 plat note number 14, there shall be no direct access to Struthers Rd. Please update the narrative and the analysis accordingly. |
|  | Subject: Text Box <br> Page Label: 18 <br> Author: Ipackman <br> Date: 2/25/2021 4:36:54 PM <br> Status: <br> Color: <br> Layer: <br> Space: | Per Struthers Ranch Filing No. 4 plat note number 14, there shall be no direct access to Struthers Rd. Please update the narrative and the analysis accordingly. |



