

Mike Bramlett

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From: Mike Bramlett  
Sent: Tuesday, March 29, 2022 6:47 AM  
To: 'Wodiuk - DNR, Jessica'  
Cc: 'Doug.Hollister@state.co.us'; 'brian.c.mccormick@state.co.us'  
Subject: RE: Non-Jurisdictional Water Impoundment Structure Notice - Homestead North at Sterling Ranch Filing 2 - Full Spectrum Detention Pond B  
Attachments: NOI Pond B Homestead North at Sterling Ranch F2.pdf

Jessica,

I have attached a notice of non-jurisdictional water impoundment structure for the full spectrum detention pond associated with Homestead North at Sterling Ranch Filing 2. This pond provides detention and water quality for the single family lots and roadways that are part of the filing 1 development.

Please let me know if you need additional information / drawings.

Thanks

Mike Bramlett, PE  
Client Manager  
JR Engineering, LLC  
5475 Tech Center Drive, Suite 235, Colorado Springs, CO 80919  
Phone: (719) 593-2593 Cell: (719) 659-7679

[mbramlett@jrengineering.com](mailto:mbramlett@jrengineering.com)

March 28, 2022



**Colorado Division of Water Resources**

310 E. Abriendo Ave., Suite B  
Pueblo, CO 81004

RE: Non-Jurisdictional Water Impoundment Structure Notice  
Homestead North at Sterling Ranch Filing 2, El Paso County

JR Engineering is performing civil engineering services for the proposed Homestead North at Sterling Ranch Filing 2 development northeast of the intersection of Vollmer Road and future Briargate Parkway in El Paso County. The development is comprised of urban residential lots, tracts roadways and utilities.

As part of this development, one (1) full spectrum detention pond is proposed. The pond is identified as Pond B. The pond will have an embankment on the downhill side but, in my opinion, the pond is non-jurisdictional and provide no public exposure in the event of embankment failure as they are adjacent to the Sand Creek drainageway. Groundwater is not anticipated to be encountered based on the depth of excavation and soils report completed by Entech Engineering. In the event groundwater is encountered, your office will be notified.

I have attached the NOI and the grading/pond plans for the pond. I have also attached an overall vicinity map to help define the location of the proposed pond.

If additional information or clarification is needed to support this submittal, please feel free to contact me.

Respectfully submitted,

**JR ENGINEERING, LLC**

A handwritten signature in blue ink that reads "Mike Bramlett".

Mike Bramlett, PE  
Client Manager  
Ph: (303) 267-6240  
Cell: (719) 659-7679  
Email: [mbramlett@jrengineering.com](mailto:mbramlett@jrengineering.com)



**NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE<sup>1</sup>**

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

**OWNER INFORMATION**

Name: SR LAND, LLC Telephone/E-Mail: (719) 491-3024 / JMORLEY3870@AOL.COM  
 Address: 20 BOULDER CRESCENT, SUITE 200 COLORADO SPRINGS CO 80903  
 Street / P.O. Box/ Rural Route City State Zip Code  
 Responsible Person: JIM MORLEY Telephone/E-Mail: (719) 491-3024 / JMORLEY3870@AOL.COM  
 Address: 20 BOULDER CRESCENT, SUITE 200 COLORADO SPRINGS CO 80903  
 Street / P.O. Box/ Rural Route City State Zip Code  
 Contractor: TO BE DETERMINED. Telephone/E-Mail: (TO BE DETERMINED.)

**STRUCTURE INFORMATION**

Name of Dam: HOMESTEAD NORTH F2 - POND B Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, and GPS Point taken at crest of dam above streamline/outlet)

- Section: 34, Township: 12S, Range: 65W, 6th P.M.  
 - Northing 4313521.56 meters, Easting 528793.89 meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height<sup>2</sup>: 7.1 ft., Length: 270 ft., Crest Width: 10 ft., Slopes: U/S: 4 (H:1V), D/S 4 (H:1V)

Reservoir:

- Surface Area<sup>1</sup>: 0.67 acres, Capacity<sup>1</sup>: 2.495 acre-feet, Drainage Area\*: 27.86 acres  
 \*(If drainage area is unknown leave blank and a spillway size will be assigned):

Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 75 ft., Side Slopes: 4 H:1V, Freeboard<sup>3</sup>: 2.2' ft

Outlet Conduit Type: RCP, Size: 24" inches, Location: SAND CREEK

Stream Name or Water Source<sup>4</sup>: SAND CREEK Proposed Water Use: FULL SPECTRUM DETENTION POND

Water Court Case or WDID : \_\_\_\_\_  
 (Water District Identification Number)

  
 Signature of Owner

3/28/22  
 Date

**Office Use Only**

**DIVISION ENGINEER'S REQUIREMENTS:** \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

**Dam I.D.** \_\_\_\_\_ **Signature of Division Engineer** \_\_\_\_\_ **Date** \_\_\_\_\_

<sup>1</sup> A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less and a surface area of 20 acres or less and a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.  
<sup>2</sup> "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.  
<sup>3</sup> "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.  
<sup>4</sup> If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at [www.water.state.co.us](http://www.water.state.co.us))



Table 1 DAM SAFETY BRANCH Spillway Sizing Guidelines for Non-Jurisdictional Dams

Drainage Area (Acres)	Minimum Recommended Bottom Width <sup>1</sup> (Feet) Low Intensity Rainfall Zone	Minimum Recommended Bottom Width <sup>1</sup> (Feet) High Intensity Rainfall Zone
175	8	8
225	8	10
275	8	12
325	8	15
375	10	17
425	11	19
475	12	21
525	13	24
575	15	26
625	16	28
675	17	30
725	19	33
775	20	35
825	21	37
875	22	39
925	24	42
975	25	44
1025	26	46
1075	28	48
1125	29	51
1175	30	53
1225	31	55
1275	33	57
1325	34	59
1375	35	62
1425	37	64
1475	38	66

<sup>1</sup>Minimum recommended bottom width for drainage areas less than 175 acres is 8 feet



Spillway Section



Map Key - - - - - Rainfall Divide Line



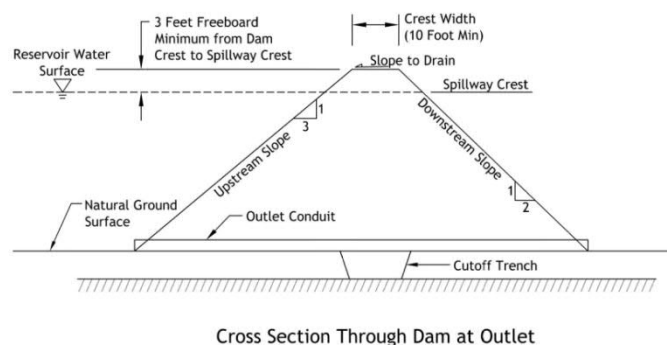
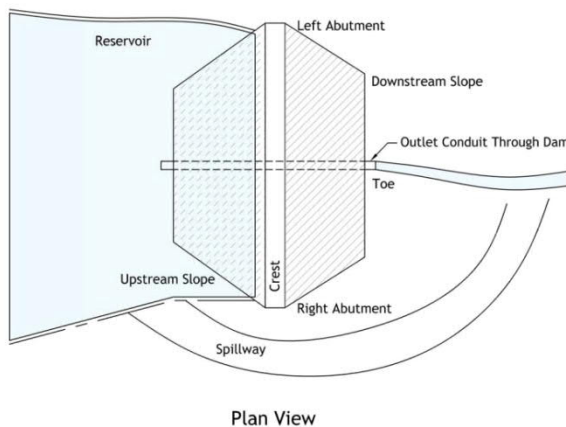
**COLORADO**  
 Division of Water Resources  
 Department of Natural Resources

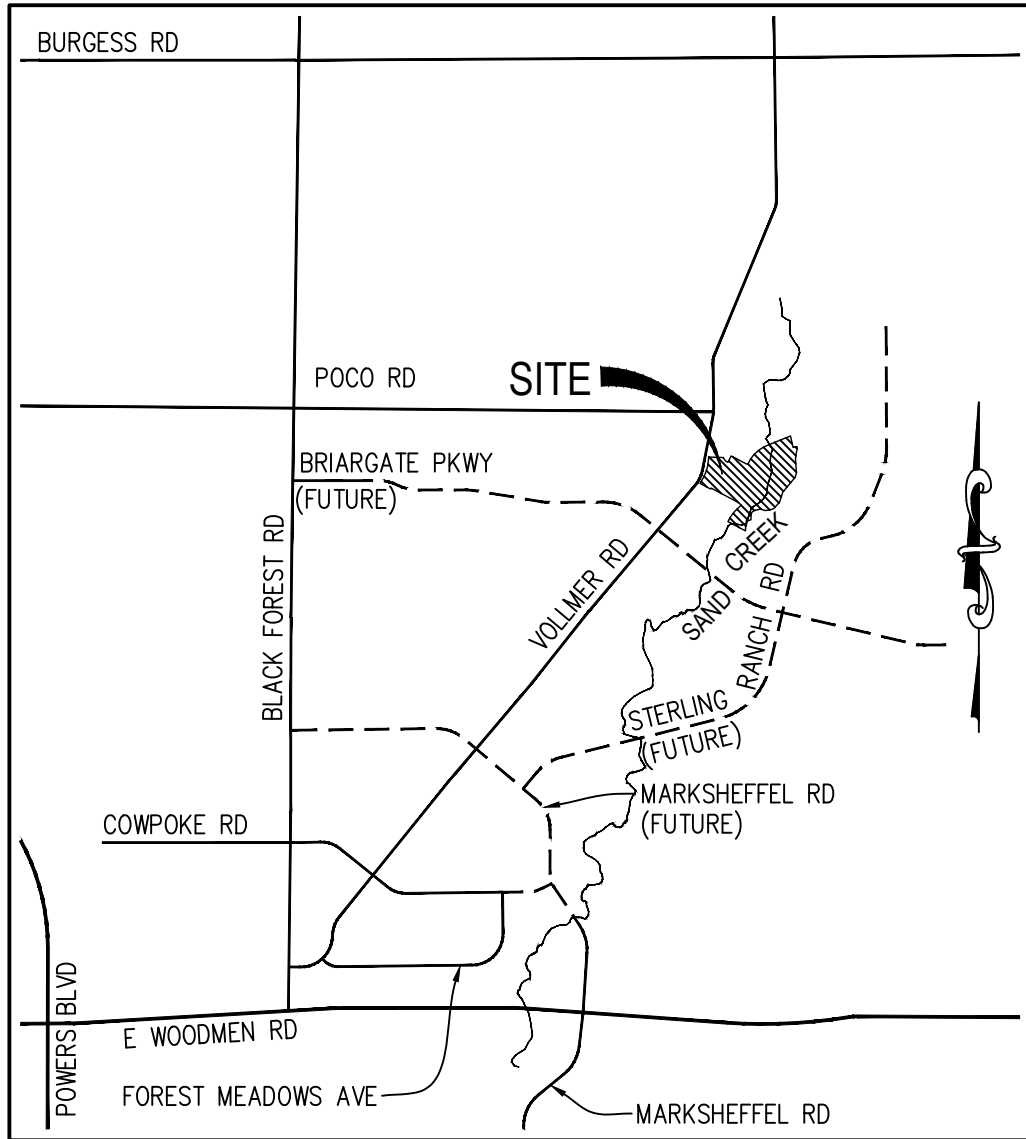
## Rainfall Intensity Zones for Non-Jurisdictional Dam Spillway Sizing



## DAM SAFETY BRANCH Specifications for Construction of Non-Jurisdictional Dams

- Site Selection:
  - Foundation soils should be firm to provide adequate support for the embankment and should have low permeability to allow for water retention. Site selection should consider potential downstream property damage in the event of a dam failure. Construction of dams in boggy areas, areas with non-uniform fractured rock, or sands/gravels is not recommended and an engineer should be hired to evaluate the site conditions. Any part of the reservoir basin excavated below grade cannot expose groundwater.
- Embankment Design:
  - Backfill material to be used for construction of the cutoff trench and embankment should be a suitable clay material and contain no material larger than 6 inches in diameter.
  - The upstream slope should be constructed with a slope no steeper than 3:1, and the downstream slope should be no steeper than 2:1 (see cross section below). The dam crest should have a minimum width of 10 feet and the surface should be graded with positive drainage toward the reservoir basin.
  - It is recommended that rock rip rap or other suitable material be placed on the upstream slope of the embankment to protect it from wave action. A suitable gravel or geosynthetic material should be placed under the rip rap to prevent fine material from washing out from behind the larger rock.
  - The embankment should be fenced to restrict livestock from accessing the dam since they damage the protective vegetation and increase erosion.
- Embankment Construction
  - The topsoil and all organic material should be removed from the foundation of the proposed dam site. Organic soil should only be reused for placement on the completed embankment to promote the re-growth of vegetation.
  - A cutoff trench should be excavated under the full length of the centerline of the dam with sloping sides (1:1 min.), a minimum bottom width of 3 feet and a depth of 3 feet.
  - The foundation of the dam should be scarified/ripped to a depth of 6-inches to provide proper contact between the native foundation and embankment. This surface should then be moisture treated before placement of fill.
  - Fill material should be placed in layers not exceeding 12 inches in thickness prior to compaction. Suitable backfill material should have enough clay and moisture content to roll a small ball by hand. If this cannot be done, the soil is likely too dry or does not have adequate clay content.
  - Each lift should be thoroughly compacted using a sheeps foot compactor. Care should be taken not to allow the top layers of the soil to dry out between placement of lifts.
  - Fill should be placed in uniform lifts that cover the entire embankment length and width.
- Outlet
  - Unless a waiver is granted in writing by the Division Engineer, all non-jurisdictional dams require an outlet conduit positioned at the natural low point of the reservoir basin. A minimum diameter of 12 inches is recommended and should be controlled at the upstream end by a valve and trash rack.
- Emergency Spillway
  - The spillway should have sufficient width to provide capacity to route the runoff from the drainage basin above the dam during rainfall/runoff events.
  - The emergency spillway should be located on natural ground far enough away to prevent erosion of the dam embankment. A spillway over the dam embankment is not acceptable.
  - A minimum of 3 feet of freeboard is required from the bottom of the emergency spillway to the top of the dam.
  - To determine the minimum spillway width, see the attached table for your area and drainage basin size.
- Example Plan View and Cross Section





## VICINITY MAP

N.T.S.

VICINITY MAP  
 HOMESTEAD NORTH AT  
 STERLING RANCH FILING NO. 2  
 JOB NO. 25188.00  
 02-16-2022



**J·R ENGINEERING**

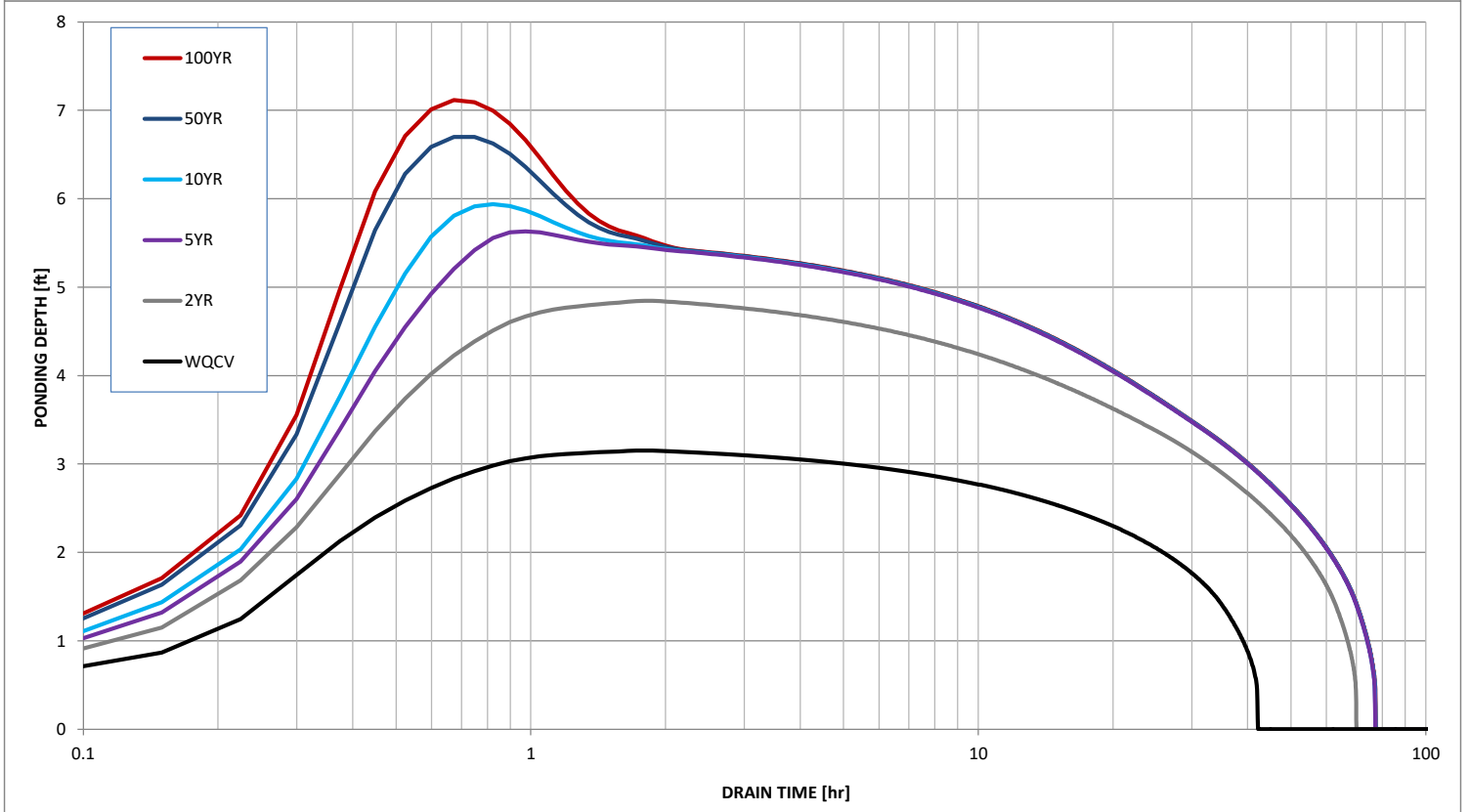
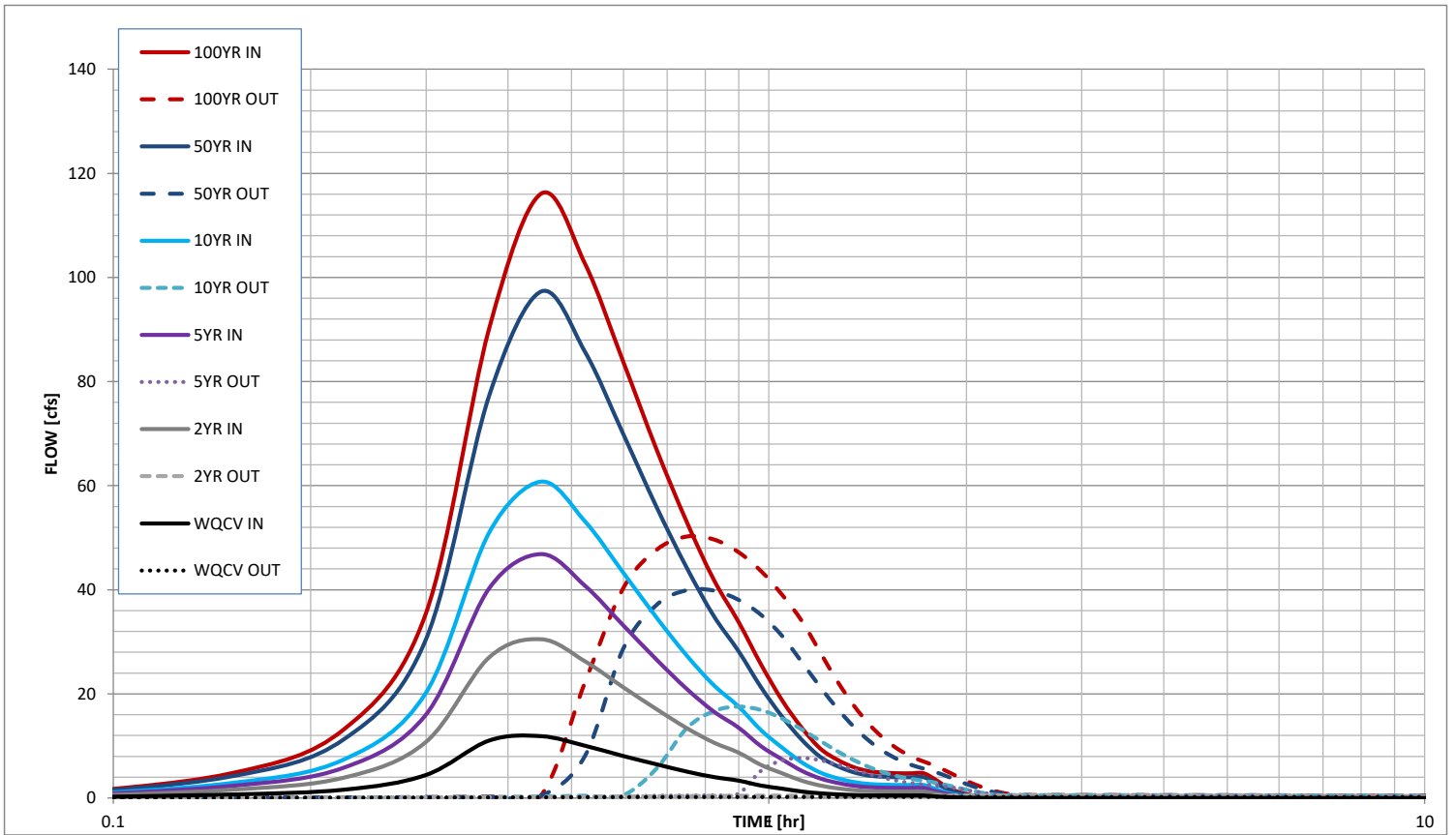
A Westrian Company

Centennial 303-740-9393 • Colorado Springs 719-593-2593  
 Fort Collins 970-491-9888 • [www.jrengineering.com](http://www.jrengineering.com)





# Stormwater Detention and Infiltration Design Data Sheet

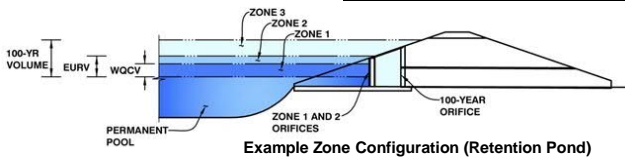


# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: 25188.10 Homestead North Filing No. 2

Basin ID: \_\_\_\_\_



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WOCV)	3.12	0.442	Orifice Plate
Zone 2 (EURV)	5.09	0.860	Orifice Plate
Zone 3 (100-year)	6.94	1.078	Weir&Pipe (Restrict)
Total (all zones)		2.380	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth =  ft (distance below the filtration media surface)  
 Underdrain Orifice Diameter =  inches

Calculated Parameters for Underdrain:  
 Underdrain Orifice Area =  ft<sup>2</sup>  
 Underdrain Orifice Centroid =  feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =  ft (relative to basin bottom at Stage = 0 ft)  
 Depth at top of Zone using Orifice Plate =  ft (relative to basin bottom at Stage = 0 ft)  
 Orifice Plate: Orifice Vertical Spacing =  inches  
 Orifice Plate: Orifice Area per Row =  inches

Calculated Parameters for Plate:  
 WO Orifice Area per Row =  ft<sup>2</sup>  
 Elliptical Half-Width =  feet  
 Elliptical Slot Centroid =  feet  
 Elliptical Slot Area =  ft<sup>2</sup>

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.79	3.57	4.00				
Orifice Area (sq. inches)	2.00	2.00	2.00	4.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	<input type="text" value="N/A"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Vertical Orifice:  
 Vertical Orifice Area =  ft<sup>2</sup>  
 Vertical Orifice Centroid =  feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	<input type="text" value="5.60"/>	<input type="text" value="N/A"/>	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	<input type="text" value="5.00"/>	<input type="text" value="N/A"/>	feet
Overflow Weir Grate Slope =	<input type="text" value="0.00"/>	<input type="text" value="N/A"/>	H:V
Horiz. Length of Weir Sides =	<input type="text" value="5.00"/>	<input type="text" value="N/A"/>	feet
Overflow Grate Type =	<input type="text" value="Type C Grate"/>	<input type="text" value="N/A"/>	
Debris Clogging % =	<input type="text" value="0%"/>	<input type="text" value="N/A"/>	%

Calculated Parameters for Overflow Weir:  
 Height of Grate Upper Edge, H<sub>1</sub> =  ft  
 Overflow Weir Slope Length =  feet  
 Grate Open Area / 100-yr Orifice Area =  ft<sup>2</sup>  
 Overflow Grate Open Area w/o Debris =  ft<sup>2</sup>  
 Overflow Grate Open Area w/ Debris =  ft<sup>2</sup>

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	<input type="text" value="0.00"/>	<input type="text" value="N/A"/>	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	<input type="text" value="24.00"/>	<input type="text" value="N/A"/>	inches
Restrictor Plate Height Above Pipe Invert =	<input type="text" value="18.00"/>	<input type="text" value="N/A"/>	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction:  
 Outlet Orifice Area =  ft<sup>2</sup>  
 Outlet Orifice Centroid =  feet  
 Half-Central Angle of Restrictor Plate on Pipe =  degrees

User Input: Emergency Spillway (Rectangular or Trapezoidal)

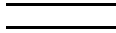
Spillway Invert Stage =  ft (relative to basin bottom at Stage = 0 ft)  
 Spillway Crest Length =  feet  
 Spillway End Slopes =  H:V  
 Freeboard above Max Water Surface =  feet

Calculated Parameters for Spillway:  
 Spillway Design Flow Depth =  feet  
 Stage at Top of Freeboard =  feet  
 Basin Area at Top of Freeboard =  acres  
 Basin Volume at Top of Freeboard =  acre-ft

## Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WOCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =								
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft) =	0.442	1.302	1.255	1.836	2.352	3.073	3.644	4.379
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.255	1.836	2.352	3.073	3.644	4.379
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	3.0	8.2	12.5	22.4	28.1	36.0
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.11	0.30	0.45	0.81	1.02	1.30
Peak Inflow Q (cfs) =	N/A	N/A	18.7	28.3	35.4	47.4	56.3	67.3
Peak Outflow Q (cfs) =	0.2	0.5	0.5	3.1	10.7	23.5	28.5	30.0
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.4	0.9	1.0	1.0	0.8
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	0.2	0.6	1.3	1.6	1.7
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	68	67	75	72	70	68	66
Time to Drain 99% of Inflow Volume (hours) =	40	72	72	80	79	78	77	76
Maximum Ponding Depth (ft) =	3.12	5.09	4.85	5.72	5.89	6.10	6.30	6.92
Area at Maximum Ponding Depth (acres) =	0.35	0.51	0.49	0.56	0.57	0.59	0.61	0.65
Maximum Volume Stored (acre-ft) =	0.442	1.303	1.182	1.635	1.737	1.860	1.979	2.370



ain



Orifice

ft<sup>2</sup>  
feet

Weir

feet  
feet

ft<sup>2</sup>  
ft<sup>2</sup>

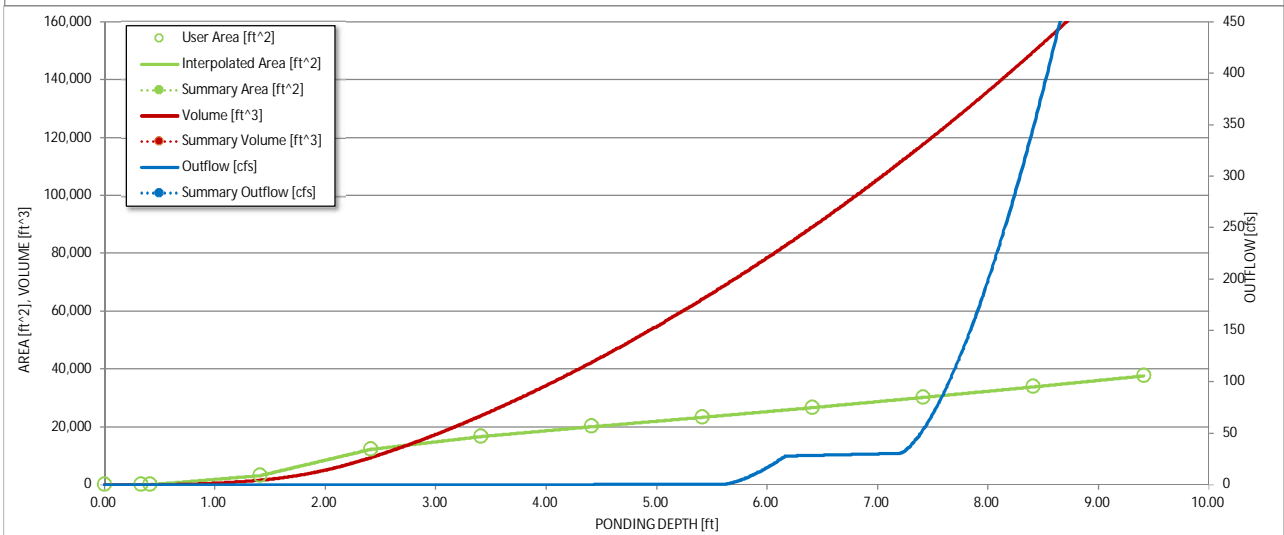
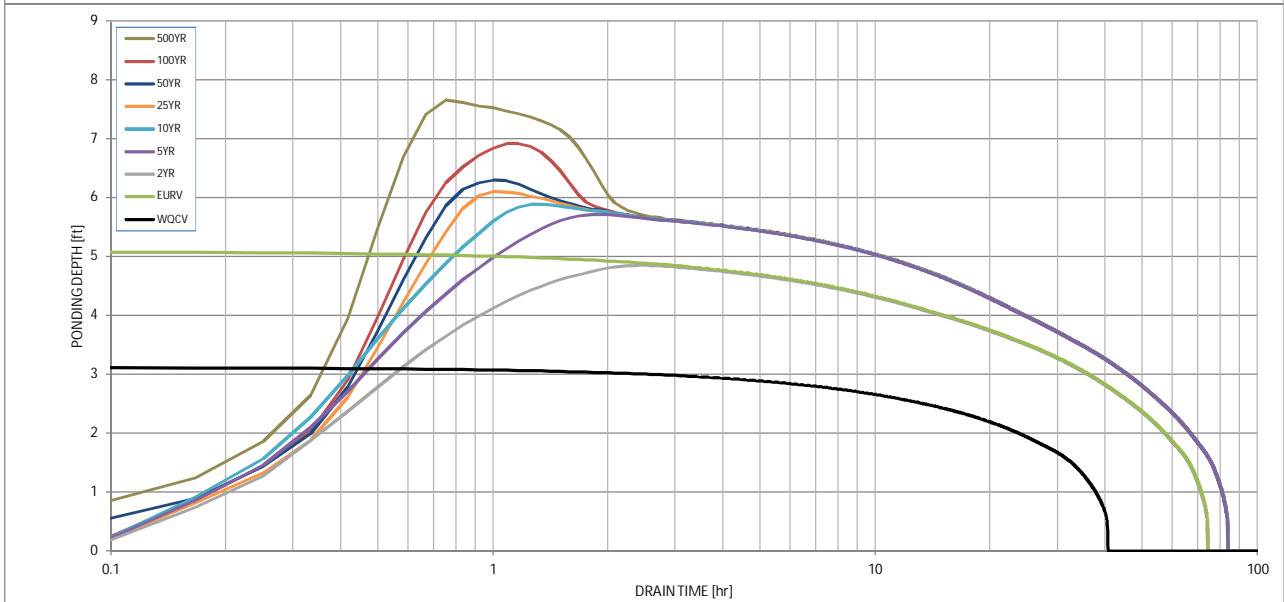
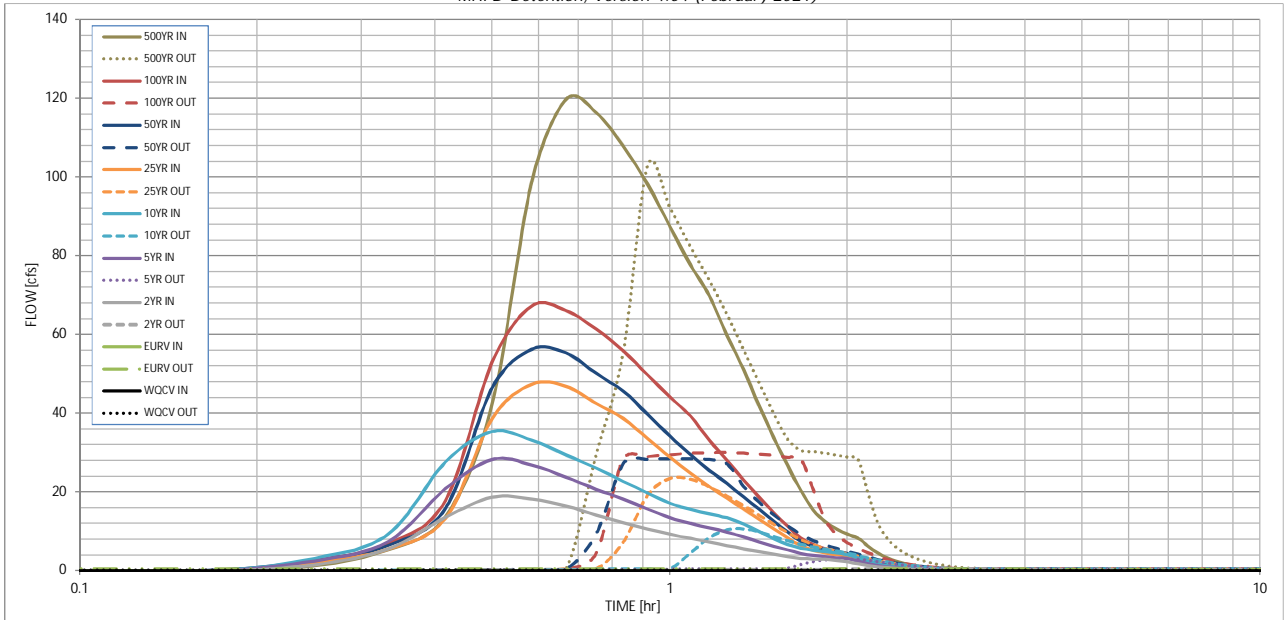
Plate

ft<sup>2</sup>  
feet  
radians

500 Year
4.00
7.939
7.939
70.5
2.55
119.7
102.9
1.5
Spillway
1.8
N/A
57
71
7.66
0.71
2.869

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.04 (February 2021)*



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

# DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: \_\_\_\_\_

**Inflow Hydrographs**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

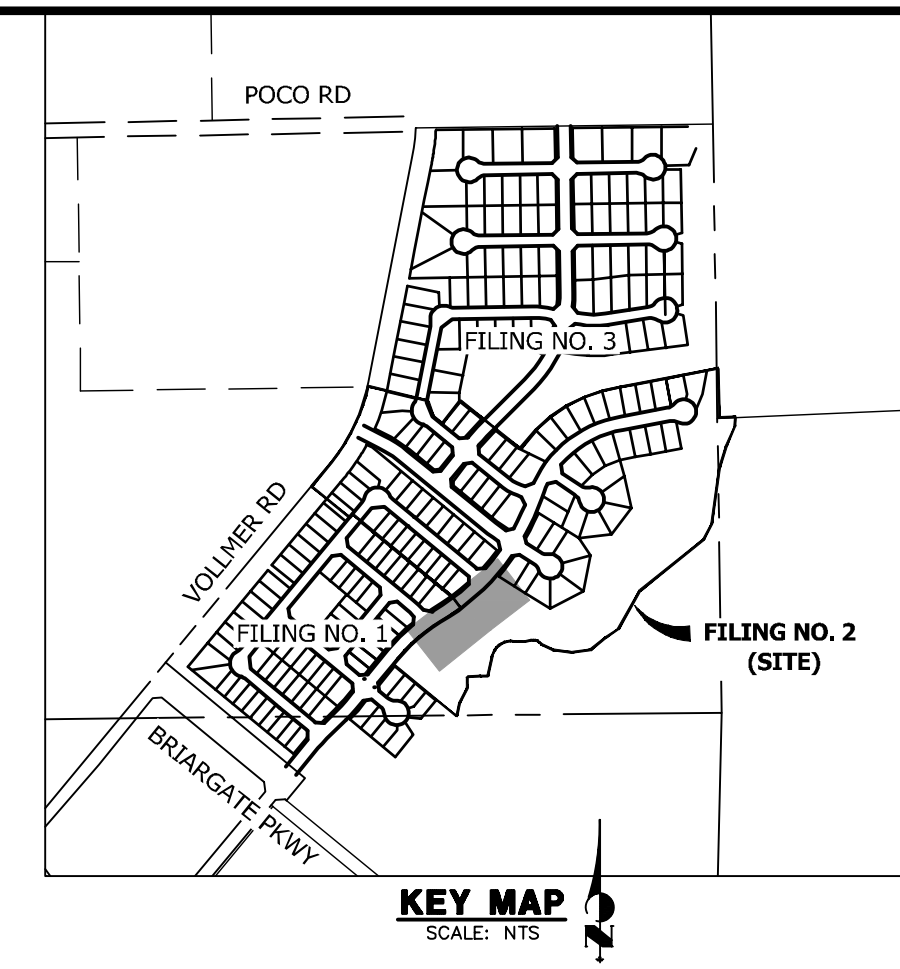
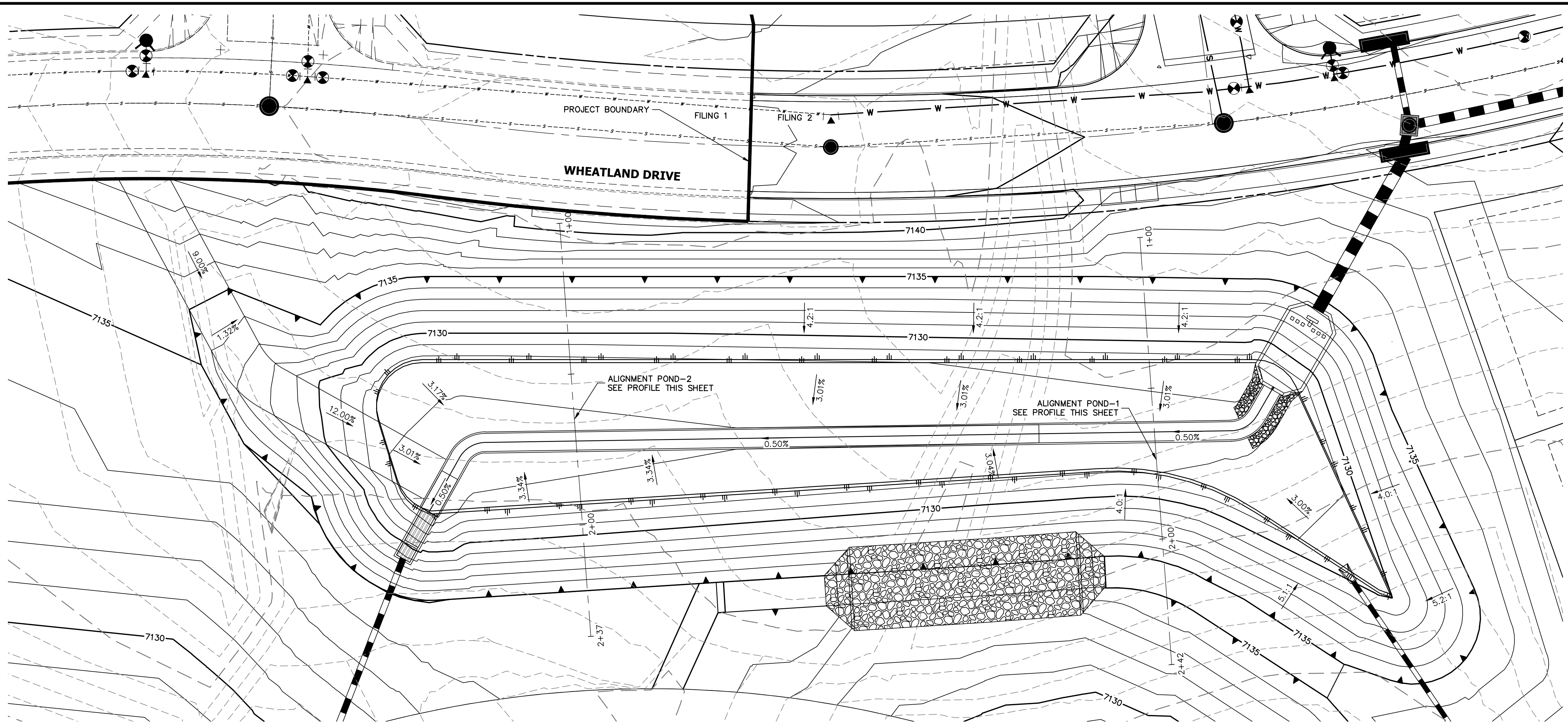
Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.18	0.02	1.35
	0:15:00	0.00	0.00	1.56	2.56	3.17	2.13	2.67	2.60	5.41
	0:20:00	0.00	0.00	5.63	7.46	9.36	5.56	6.49	6.94	13.86
	0:25:00	0.00	0.00	13.61	20.88	27.68	13.40	15.94	17.90	42.45
	0:30:00	0.00	0.00	18.74	28.32	35.41	38.74	46.60	53.06	98.82
	0:35:00	0.00	0.00	18.24	26.90	33.22	47.40	56.32	67.31	119.75
	0:40:00	0.00	0.00	16.49	23.82	29.49	46.91	55.38	66.10	116.43
	0:45:00	0.00	0.00	14.22	20.74	26.12	42.52	50.15	61.47	107.92
	0:50:00	0.00	0.00	12.28	18.25	22.76	38.74	45.69	55.91	98.24
	0:55:00	0.00	0.00	10.66	15.77	19.78	33.65	39.79	49.78	87.53
	1:00:00	0.00	0.00	9.26	13.52	17.19	28.90	34.24	44.19	77.77
	1:05:00	0.00	0.00	8.28	12.02	15.59	24.85	29.55	39.37	69.87
	1:10:00	0.00	0.00	7.29	11.01	14.49	21.36	25.49	33.22	59.77
	1:15:00	0.00	0.00	6.42	9.86	13.48	18.52	22.16	28.04	51.17
	1:20:00	0.00	0.00	5.65	8.60	11.89	15.74	18.80	23.07	42.03
	1:25:00	0.00	0.00	4.91	7.38	9.95	13.18	15.71	18.67	33.87
	1:30:00	0.00	0.00	4.20	6.28	8.19	10.69	12.69	14.81	26.74
	1:35:00	0.00	0.00	3.60	5.33	6.71	8.38	9.90	11.32	20.41
	1:40:00	0.00	0.00	3.19	4.47	5.79	6.43	7.54	8.39	15.45
	1:45:00	0.00	0.00	3.00	3.94	5.26	5.27	6.19	6.70	12.54
	1:50:00	0.00	0.00	2.90	3.59	4.90	4.56	5.34	5.62	10.62
	1:55:00	0.00	0.00	2.59	3.34	4.55	4.11	4.79	4.88	9.29
	2:00:00	0.00	0.00	2.30	3.07	4.12	3.79	4.40	4.34	8.32
	2:05:00	0.00	0.00	1.82	2.43	3.24	2.97	3.43	3.29	6.31
	2:10:00	0.00	0.00	1.40	1.86	2.47	2.23	2.58	2.39	4.59
	2:15:00	0.00	0.00	1.08	1.42	1.87	1.68	1.93	1.75	3.36
	2:20:00	0.00	0.00	0.83	1.08	1.40	1.27	1.45	1.32	2.52
	2:25:00	0.00	0.00	0.63	0.81	1.04	0.95	1.08	0.99	1.87
	2:30:00	0.00	0.00	0.48	0.60	0.77	0.70	0.80	0.74	1.40
	2:35:00	0.00	0.00	0.35	0.43	0.57	0.51	0.58	0.55	1.03
	2:40:00	0.00	0.00	0.26	0.32	0.42	0.39	0.44	0.41	0.77
	2:45:00	0.00	0.00	0.18	0.22	0.30	0.28	0.31	0.29	0.55
	2:50:00	0.00	0.00	0.12	0.15	0.19	0.19	0.21	0.20	0.36
	2:55:00	0.00	0.00	0.07	0.09	0.11	0.11	0.13	0.12	0.22
	3:00:00	0.00	0.00	0.03	0.05	0.06	0.06	0.07	0.06	0.11
	3:05:00	0.00	0.00	0.01	0.02	0.02	0.02	0.02	0.02	0.03
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
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4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
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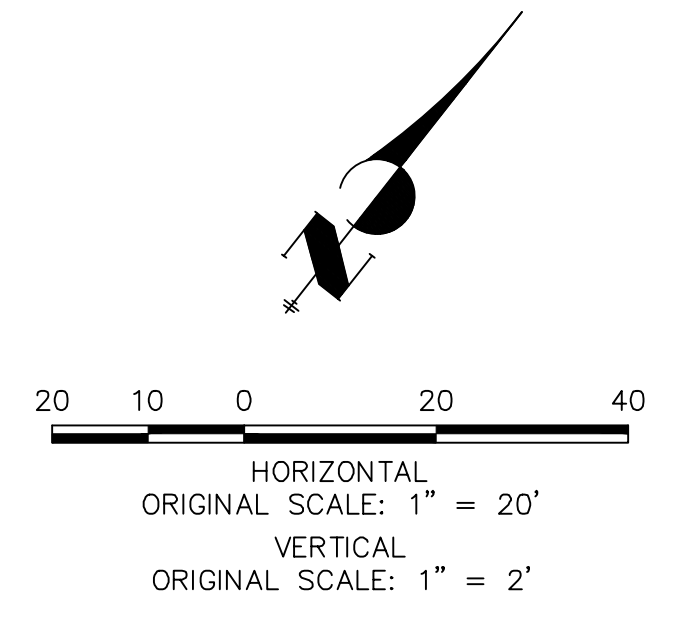
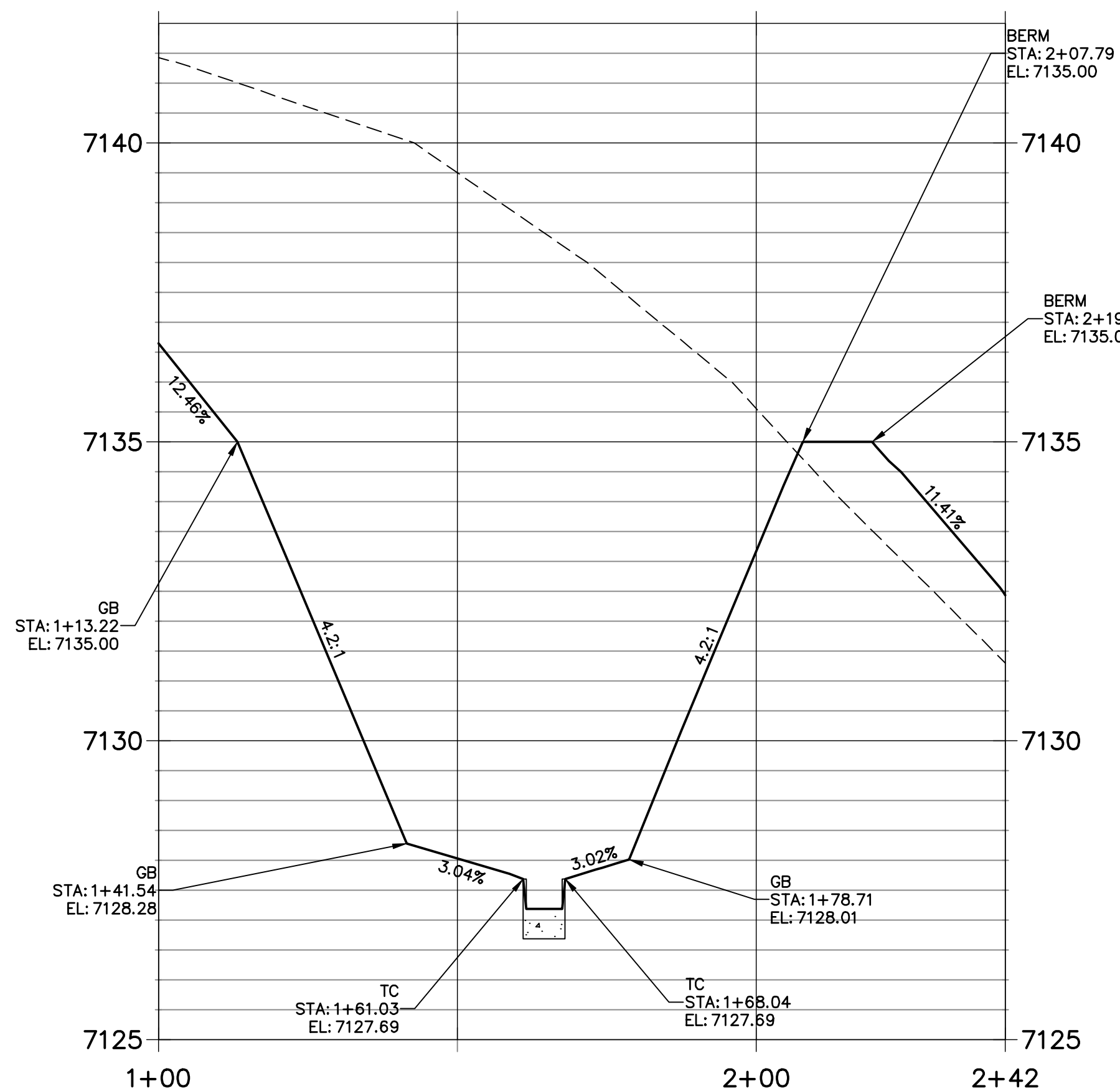
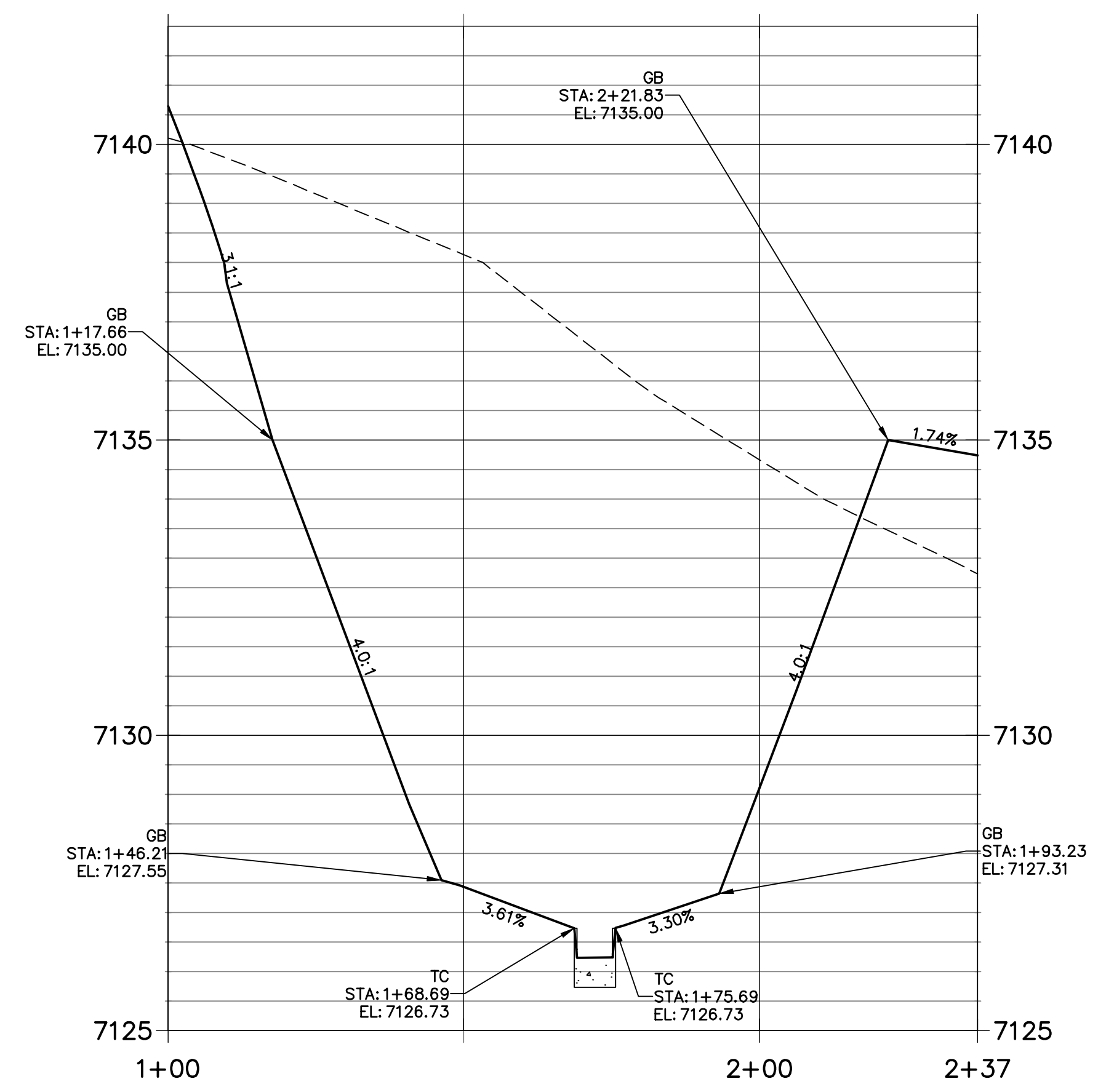






**POND-2 PROFILE  
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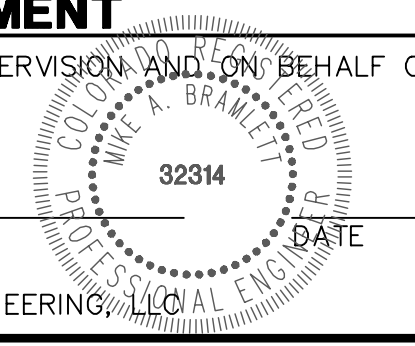
**POND-1 PROFILE  
STA 1+00.00 TO 2+41.67**



**ENGINEER'S STATEMENT**

PREPARED UNDER MY DIRECT SUPERVISION AND ON BEHALF OF JR ENGINEERING

MIKE A. BRAMLETT, P.E.  
COLORADO P.E. 32314  
FOR AND ON BEHALF OF JR ENGINEERING, LLC



UNTIL SUCH TIME AS THESE DRAWINGS ARE APPROVED BY THE AGENCIES, OR ENGINEERING APPROVES THEIR USE, THESE DRAWINGS ARE DESIGNATED BY WRITTEN AUTHORIZATION.

PREPARED FOR  
**SR LAND, LLC**  
20 BOULDER CRESCENT  
SUITE 201  
COLORADO SPRINGS, CO 80903  
JAMES F. MORLEY  
(719) 471-1742

**J.R. ENGINEERING**  
A Westman Company  
Central 303-740-9888 • Colorado Springs 719-583-2583  
Fort Collins 970-491-9888 • www.jrengineering.com

No.	REVISION	BY	DATE

HOMESTEAD NORTH AT  
STERLING RANCH FILING NO. 2

POND PLANS

SHEET 9 OF 14

JOB NO. 25188.10



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