

FINAL DRAINAGE REPORT FOR ZINDORF — MCDANIEL'S SUBDIVISION

PCD FILE NO. MS-22-006

PREPARED BY Richard Gallegos, P.E. RESPEC 121 S. Tejon St., Suite 1110 Colorado Springs, CO 80903

PREPARED FOR Greg Zindorf Z Investments LLC PO Box 50005 Colorado Springs, CO

January 2022





ENGINEER'S STATEMENT

This report and plan for the drainage design of Zindorf - McDaniels Subdivision, was prepared by me (or under my direct supervision) and is correct to the best of my knowledge and belief. Said report and plan has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I understand that El Paso County does not, and will not, assume liability for drainage facilities designed by others. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

	1/3/20	23
Richard Gallegos, P.E.	Date	
Registered Professional Engineer State of Cold	orado No. 36247	OR ADO LICEN OF

DEVELOPER'S STATEMENT

Greg Zindorf hereby certifies that the drainage facilities for the Zindorf – McDaniels Subdivision shall be constructed according to the design presented in this report. I understand that El Paso County does not, and will not, assume liability for the drainage facilities designed and/or certified by my engineer and that are submitted to El Paso County; and cannot, on behalf of the Zindorf – McDaniels Road guarantee that final drainage design review will absolve Greg Zindorf and/or their successors and/or assigns of future liability for improper design. I further understand that approval of the final plat does not imply approval of my engineer's drainage design.

GREG ZINDORF

Authorized Signature Greg Zindorf Printed Name 12-20-2022 Date

<u>Owner</u> Title Address:

PO Box 50005 Colorado Springs, CO

EL PASO COUNTY STATEMENT

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.



Joshua Palmer, P.E. County Engineer/ECM Administrator

Date



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1.0 PURPOSE

This drainage report is for the design of Zindorf – McDaniels Subdivision (McDaniels Tract). The site is located at 22755 McDaniels Road, Calhan CO in eastern El Paso County. See Vicinity Map in the Appendix below for reference. It is further described as the Northeast One-Quarter of the Northeast One-Quarter of Section 11, Township 14 South, Range 63 West of the 6th P.M.

This site is located in the Ellicott Consolidation – CHBS1200 Drainage Basin. Work will include subdividing the 39.7-acre site into four residential lots. An existing home will remain, and the driveway will be reconstructed. On the other three lots, home pads and gravel driveways will be constructed.

2.0 SOIL CONDITIONS

According to the El Paso County Area Soil Survey, the soil on the site is classified as follows:

SOIL #	SOIL TYPE	HYDROLOGIC CLASSIFICATION
19	Columbine Gravelly Sandy Loam	Α
28	Ellicot Loamy Coarse Sand	А
95	Truckton Loamy Sand	Α

The Columbine soil can be described as having a very high permeability, very low surface runoff, and slight hazard of erosion. The Ellicot soil also can be described as very low surface runoff and slight erosion hazard. The Truckton soil includes a moderate hazard of erosion and low water surface runoff. The soil classification used for this study is 'A'. See Soils Map below in the Appendix for reference.

3.0 DRAINAGE CRITERIA

The methodology utilized for this report is in accordance with the *El Paso County Drainage Criteria Manual.* The Rational Method for computation of runoff was used.

Q = cia

Where

Q = maximum rate of runoff in cubic feet per second

- c = runoff coefficient representing drainage area characteristics
- i = average rainfall intensity, in inches per hour, for the duration required for the runoff to become established

a = drainage basin size in acres

The storm recurrence intervals used for this study were the 5-year storm and the 100-year storm. ManningSolver Version 1.019 was used in this analysis to calculate the Manning's normal depth within the proposed swale.

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Please clarify if 1.03 acres of land has been disturbed and how (i.e. grading, stockpiling, existing home & driveway). Is there active work taking place on the parcel?



4.0 EXISTING DRAINAGE CONDITIONS

The overall site consists of 39.7 acres, of this area only approximately 1.03 acres are currently disturbed. An existing gravel driveway is located off of McDaniels Road within the site to service an existing home. The areas of the site not covered by gravel road or the residential home are covered with short grass pastures. These undeveloped areas include slopes that range from 0.5% to 2.4%. The overall existing site is approximately 2.1% impervious. See Existing Drainage Map in Appendix for reference.

Flows from Sub-basins EX1 through EX3 and OS1 through OS3 are tributary to the Ellicot Consolidated Drainage Basin (CHBS1200).

Sub-basin OS1 contains 8.67 acres and drains southeast into Subbasin EX2. It produces flows of 1.1 cfs for the 5-year storm and 8.2 cfs for the 100-year storm. These flows sheetflow and continue towards the FEMA 100-year floodway in the middle of the site.

Sub-basin OS2 contains 0.49 acres and drain southwest into the Haegler Ranch floodway that runs through the middle of the site. It produces flows of 0.1 cfs for the 5-year storm and 1.0 cfs for the 100-year storm. These flows sheetflow to the southwest.

Sub-basin OS3 contains 1.02 acres and drains the east along McDaniels Road. It contains half of McDaniels Road that drains south into the ditch. It produces flows of 0.5 cfs for the 5-year storm and 1.5 cfs for the 100-year storm. All flows north of the site are directed into a drainage ditch that flows into a culvert under McDaniels Road. These flows bypass the development occurring on the site in a ditch along North Log Road. Flows overtopping the intersection of McDaniels Road and North Log Road sheetflow to the east.

Sub-basin EX1 contains 1.06 acres and drains due southwest. It produces flows of 0.2 cubic feet per second (cfs) for the 5-year storm and 1.4 cfs for the 100-year storm. These flows are directed to the southerly direction.

Sub-basin EX2 contains 14.59 acres and drains southeast into the Haeglar Ranch floodway in the middle of the site. It produces flows of 1.8 cfs for the 5-year storm and 14.0 cfs for the 100-year storm. These flows sheetflow to the southeast.

Sub-basin EX3 contains 22.89 acres and sheetflows to the southwest into the Haegler Ranch floodway in the middle of the site. It produces flows of 3.5 cfs for the 5-year storm and 22.8 cfs for the 100-year storm. These flows will combine with flows from Sub-basin EX2, Sub-basin EX3, Sub-basin OS1, Sub-basin OS2, and Sub-basin OS3 to produce total flows of 7.1 cfs for the 5-year storm and 48.0 cfs for the 100-year storm at Design Point 1. FEMA Zone 'A' 100-year floodplain flows entering the site are approximately 1,900 cfs.





The total flow leaving the site at Design Point 1 (DP1) is 7.1 cfs for the 5-year storm and 48.0 cfs for the 100-year storm. The estimated runoff amounts produced for the project under Existing Conditions are shown in Table 1 below.

TABI	E 1 – EXISTING CONDITIONS	
Sub-basin	Q₅(CFS)	Q ₁₀₀ (CFS)
EX1	0.2	1.4
EX2	1.8	14.0
EX3	3.5	22.8
0S1	1.1	8.2
O\$2	0.1	1.0
O\$3	0.5	1.5
DP1 (EX2 + EX3 + OS1 + OS2 + OS3)	7.1	48.0

5.0 DEVELOPED DRAINAGE CONDITIONS

Although the overall site consists of 39.7 acre only approximately 1.37 acres will be disturbed. The site will be subdivided into four separate lots with three house and gravel driveway being constructed on the four lots. The existing house will remain and the gravel driveway will be reconstructed. See Proposed Conditions Map below in Appendix for reference.

The overall drainage pattern will remain the same as existing conditions with developed flows directed to the same locations as described in the Existing Conditions Section. Proposed site imperviousness is 2.8%, versus 2.1% in the existing conditions.

Sub-basin OS1 contains 8.67 acres and drains southeast into Subbasin PP2. It produces flows of 1.1 cfs for the 5-year storm and 8.2 cfs for the 100-year storm. These flows continue towards the FEMA 100-year floodway in the middle of the site.

Sub-basin OS2 contains 0.49 acres and drain southwest into the Haegler Ranch floodway that runs through the middle of the site. It produces flows of 0.1 cfs for the 5-year storm and 1.0 cfs for the 100-year storm. These flows sheetflow to the southwest and will combine with flows from Design Point 1, described above.

Sub-basin OS3 contains 1.02 acres and drains the east along McDaniels Road. It contains half of McDaniels Road that drains south into the ditch. It produces flows of 0.5 cfs for the 5-year storm and 1.5 cfs for the 100-year storm. All flows north of the site are directed into a drainage ditch that flows into a culvert under McDaniels Road. These flows bypass the development occurring on the site in a ditch along North Log Road. Flows overtopping the intersection of McDaniels Road and North Log Road sheetflow to the east.

Sub-basin PP1 contains 1.06 acres and drains due southwest. It produces flows of 0.2 cfs for the 5-year storm and 1.4 cfs for the 100-year storm. These flows sheetflow in a southerly direction.

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Sub-basin PP2 contains 14.59 acres and drains southeast into the Haeglar Ranch floodway in the middle of the site. It produces flows of 1.8 cfs for the 5-year storm and 14.0 cfs for the 100-year storm. These flows sheetflow to the southeast.

Sub-basin PP3 contains 22.89 acres and sheetflows to the southwest into the Haegler Ranch floodway in the middle of the site. It produces flows of 3.9 cfs for the 5-year storm and 22.9 cfs for the 100-year storm. These flows will combine with flows from Sub-basin PP2, Sub-basin PP3, Sub-basin OS1, Sub-basin OS2, and Sub-basin OS3 to produce total flows of 7.5 cfs for the 5-year storm and 48.0 cfs for the 100-year storm at Design Point 1.

All flows north of the site are directed into a drainage ditch that flows into a culvert under McDaniels Road. These flows bypass the development occurring on the site in a ditch along North Log Road. Flows overtopping the intersection of McDaniels Road and North Log Road sheetflow to the east.

The total flow leaving the site under Developed Conditions at Design Point 1 (DP1) is 7.5 cfs for the 5year storm and 48.0 cfs for the 100-year storm. The estimated runoff amounts produced for the project for Developed Conditions are shown in Table 2 below.

	OS1 OS2 OS3 DP1 (PP2 + PP3 + OS1 + OS2	2 – DEVELOPED CONDITIONS	
	. .	Q₅(CFS)	Q ₁₀₀ (CFS)
	.	0.2	1.4
required or why not.		1.8	14.0
roquirou		3.9	22.9
	0S1	1.1	8.2
	0S2	0.1	1.0
	0S3	0.5	1.5
	DP1 (PP2 + PP3 +0S1 + 0S2 + 0S3)	7.5	48.0

6.0 WATER QUALITY

The total disturbance for this development will be 1.37 acres. According to the El Paso County Engineering Criteria Manual (ECM), "The following types of sites and associated land disturbances are excluded from the requirements of this Section 1.7". Furthermore, in El Paso County ECM Appendix I.7, 1.B, a "Large Lot Singe Family Site" is excluded from the requirements defined in Section 1.7. Since this site will be divided in single-family residential lots greater than 2.5 acres in size per dwelling and having a total lot imperviousness of less than 10 percent, the site can be excluded from water quality control measure requirements. Although permanent stormwater quality control measures and ESQCP are not required. Each site is still responsible for providing the appropriate temporary control measures (i.e. silt fence, VTC, etc). Please revise statement.

UUU RESPEC

7.0 EROSION CONTROL PLAN

The site construction consists of four single family residential lots, which includes four houses and gravel driveways that all occupy greater than 2.5 acres in size per dwelling and a total lot imperviousness of less than 10 percent. With single family residential lots greater than 2.5 acres in size and less than 10 percent imperviousness, the site is excluded from erosion control requirements.

8.0 FLOODPLAIN STATEMENT

Portions of the site are within the designated FEMA 100-year floodplain and designated FEMA 100-year floodway as designated on Map No. 08041C0810G and Map No. 08041C0807G, both dated December 7th, 2018. A large portion of the site consists of a FEMA Flood Zone 'AE'. A small portion on the north end of the site is considered a FEMA Flood Zone 'A'.

No development will occur within the FEMA 100-year floodplain and the FEMA 100-year floodway. There is currently not an approved drainage basin planning study for the Ellicott Consolidated (CHBS1200) Drainage Basin. For this reason, drainageway improvements and channel stabilization requirements have not been identified for the drainageway shown in the Drainage Maps below. No improvements are proposed for this drainageway as a result.

9.0 DRAINAGE BASIN FEES

	The proposed development is located within the Ellicott Consolidated Drai	nage Basin.
	Review 1 comment:	
	Please also discuss/provide analysis of the floodplain. What are the conditions of the channel? are improvements required to stabilize the drainageway? etc. Per DCMV1 1.4.2 "Developers in and along the drainage way are required to implement the proper	ed Drainage Basin.
	measures to maintain or create stable characteristics of the drainageway. The principal objective is to limit excessive erosion	Drainage Basin.
	in and along the channel	
	Review 2 : Unresolved. Please provide discussion and analysis of the floodplain. Identify conditions, characteristics, pictures, bud public analysis of the draine power.	
	of similar project) for an example of what is expected. Please be	no engineer's estimate of
	sure to identify any improvements that may be needed and provide any necessary construction drawings.	
	https://epcdevplanreview.com/Public/ProjectDetails/174808	
NO.		

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The existing conditions section states 1.07 acres of land has been disturbed. Please verify total land disturbance and update accordingly.

For this 39.7-acre site, the site will be divided into four separate lots. The existing gravel driveway will be removed and reconstructed, while the 1850 sf house will remain. Three additional home pads and gravel driveways will be constructed on the other lots. The total anticipated disturbance of the site will be 1.37 acres. Construction will occur within drainage sub-basin PP3. The development increases total routed flows exiting the site at Design Point 1 (DP1) from 7.1 cfs to 7.5 cfs for the 5-year storm, while the 100-year storm flow remains unchanged at 48.0 cfs. This is a 5.6% increase for the 5-year storm. These increases do not warrant the need for detention. All developed flows will continue to flow along existing drainage patterns. All areas disturbed by construction will be repaired, and erosion control measures will be installed during construction of the proposed site.

Unresolved Review 1 Comment: Please provide a statement confirming the proposed development will not adversely impact adjacent and downstream properties.

Municipal Code Corporation (2018). Engineering Criteria Manual of El Paso County, Colorado (ECM)

Municipal Code Corporation (2018). Drainage Criteria Manual of El Paso County, Colorado (DPM)

USDA, NRCS. Soil Survey of El Paso County Area, Colorado.

Haegler Ranch Drainage Basin Planning Study, URS Corporation, Dated May 2009.

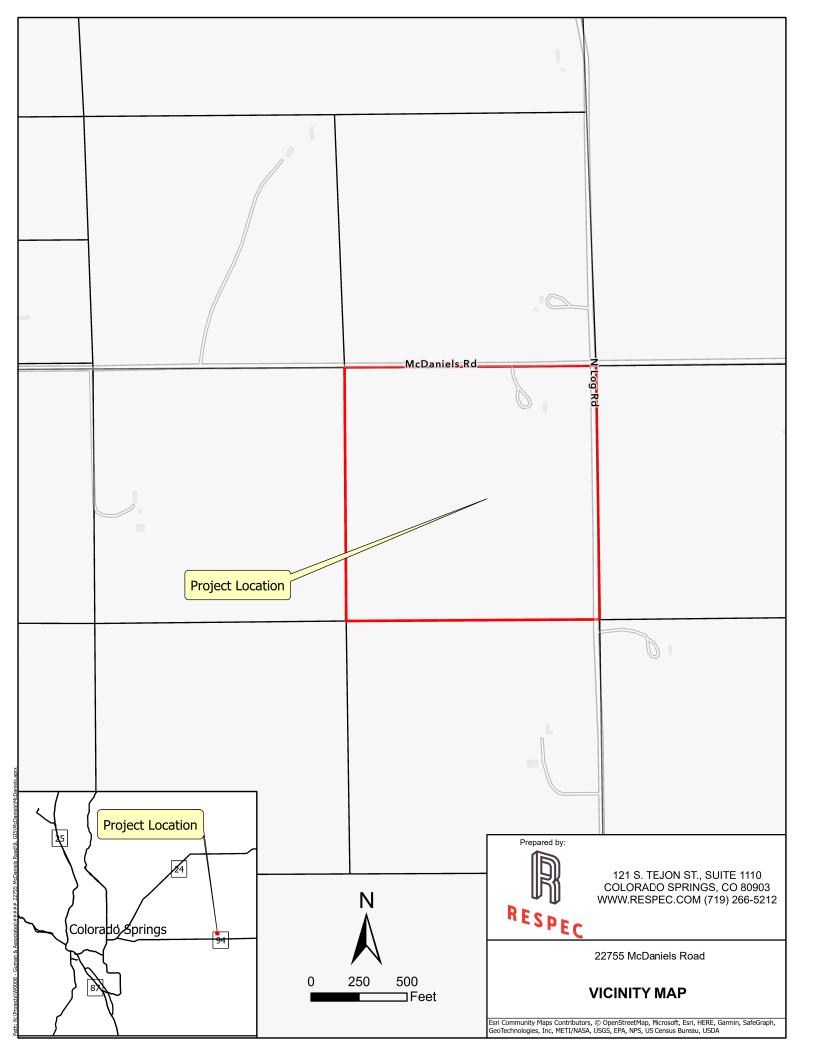
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APPENDIX A MAPS

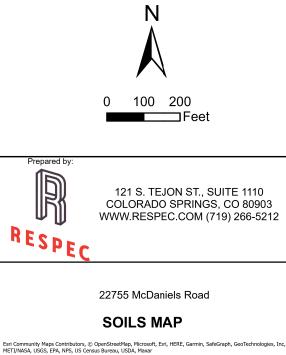




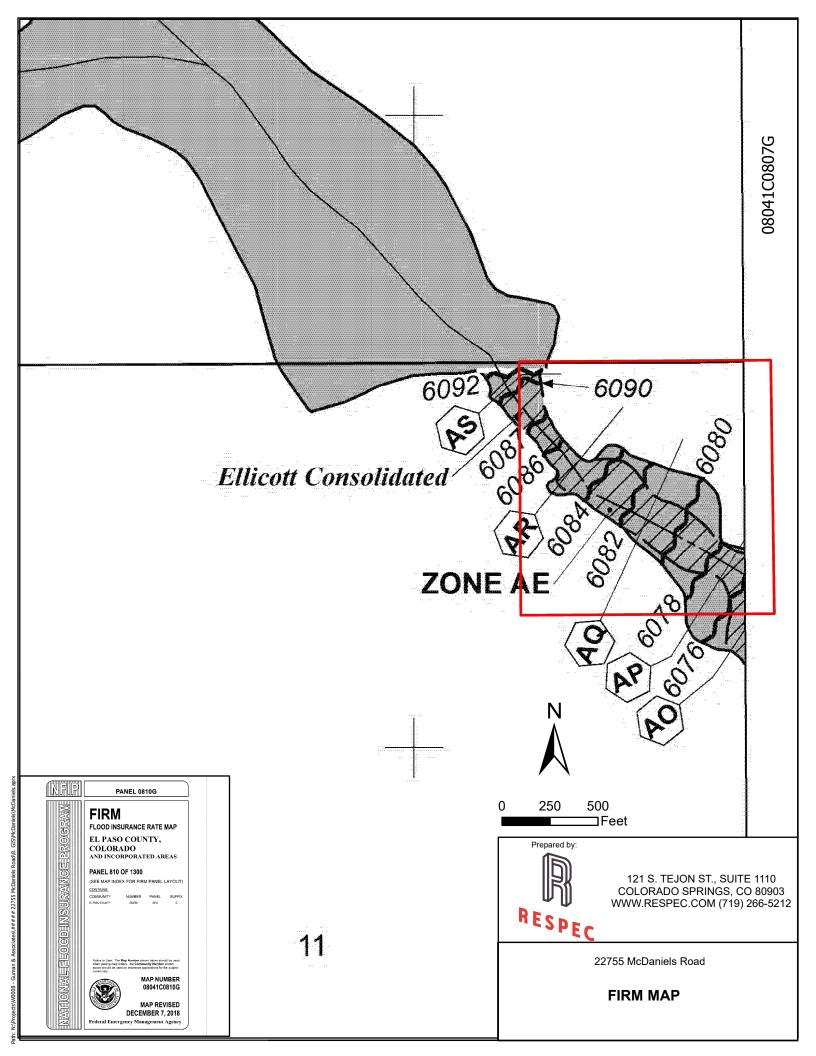


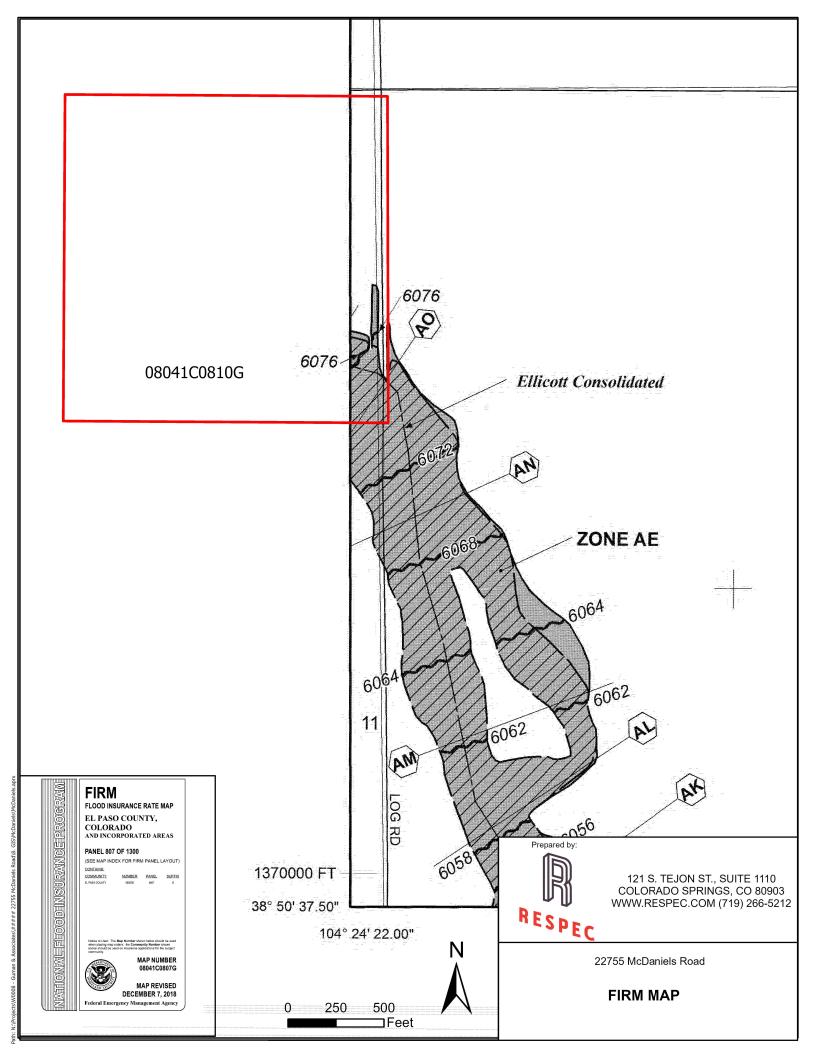


Map Unit Symbol	Map Unit Name	Rating
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A
28	Ellicot loamy coarse sand, 0 to 5 percent slopes	A
95	Truckton loamy sand, 1 to 9 percent slopes	A



Path: N:\Projects\W0008 - Guman & Associates\#### 22755 McDaniels Road\8. GIS\McDaniels\McDaniels.aprx







APPENDIX B CALCULATIONS





McDaniels S	Subdivision								
C FACTOR	CALCULATIO	ON SHEET							
EXISTING C									
RUNOFF CC									
TYPE A/B S	OILS								
LAND USE		Imperv %	5 YR	100 YR					
UNDEV		0	0.08						
GRAVEL RC		80	0.59	0.7					
ASPHALT R	OAD	100	0.9	0.96					
ROOFS		90	0.73	0.81					
				EXISTING C					
	TOTAL		E CONDITIOI				ATED C		
AREA	AREA		GRAVEL	ASPHALT	ROOFS	5	100	% IMPE	RVIOUS
	(acre)		ROAD	ROAD	10010	YR	YR	70 HVH L	
EX1	1.06		0.00	0.00	0.00	0.08	0.35		0.00
EX2	14.59	14.59	0.00	0.00	0.00	0.08	0.35		0.00
EX3	22.86	21.83	0.98	0.00	0.05	0.10	0.37		3.63
OS1	8.67	8.67	0.00	0.00	0.00	0.08	0.35		0.00
OS2	0.49	0.49	0.00	0.00	0.00	0.08	0.35		0.00
OS3	1.02	0.60	0.42	0.00	0.00	0.29	0.49		32.94
Site Percent	Impervious	2.11							

DEVELOPE	D CONDITIO	NS							
RUNOFF CO	DEFICIENT								
TYPE A/B S	OILS								
LAND USE		Imperv %	5 YR	100 YR					
UNDEV		0	0.08	0.35					
GRAVEL RC	DAD	80	0.59	0.7					
ASPHALT R		100	0.9	0.96					
ROOFS		90	0.73	0.81					
				Developed	Conditions				
	TOTAL	SURFAC	E CONDITIO			CALCUL	ATED C		
AREA	AREA	UNDEV	GRAVEL	ASPHALT	ROOFS	5	100	% IMPE	RVIOUS
DESIG.	(acre)		ROAD	ROAD		YR	YR		
PP1	1.06	1.06		0.00	0.00				0.00
PP2	14.59	14.59	0.00	0.00	0.00		0.35		0.00
PP3	22.86	21.49		0.00	0.21	0.11	0.37		4.89
Total	38.51	37.14	1.16	0.00	0.21	TOTAL SITE	IMPERVIOUS	NESS	2.90
OS1	8.67	8.67	0.00	0.00	0.00				0.00
OS2	0.49	0.49		0.00	0.00				0.00
OS3	1.02	0.60	0.42	0.00	0.00	0.29	0.49		32.94
Site Percent	Impervious	2.83							

PROJ.W008		-																
RAINAGE CALCULA	FION SHEE	Γ																
le:McDaniels Rd.xlsx																		
06/14/22																		
		-					Initial Tci			Travel Ti								
AREA	AREA	C5		C5 X A	C100 X A		Slope	ti		Slope	V	Tt	TC	15	1100	Q5	Q100	AREA
DESIG.	(acre)	(5 yr)	(100 yr)	i i i i i i i i i i i i i i i i i i i		L (ft)	(%)	(min)	L (ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(cfs)	(cfs)	DESIG.
EXISTING CONDITIONS																		
EX1	1.06	0.08	0.35	0.08	0.37	300	1.70		350	1.70	0.90	6.48	34.21	2.14	3.74	0.2	1.4	EX
EX2	14.59	0.08	0.35	1.17	5.11	300	2.00	26.28	1400	1.40	0.80	29.17	55.45	1.57	2.75	1.8	14.0	EX
EX3	22.86	0.10	0.37	2.29	8.46	300	2.00	25.77	1500	1.40	0.80	31.25	57.02	1.54	2.69	3.5	22.8	EX
DS1	8.67	0.08	0.35	0.69	3.03	300	1.00	33.04	1000	1.10	0.70	23.81	56.85	1.55	2.70	1.1	8.2	OS
DS2	0.49	0.08	0.35	0.04	0.17	115	2.40	15.32	115	2.40	1.20	1.60	16.92	3.18	5.55	0.1	1.0	OS
DS3	1.02	0.29	0.49	0.30	0.50	30	0.50	5.00	0	0.00	0.00	0.00	5.00	5.19	9.06	1.5	4.5	OS
DP1 (EX2 + EX3 +																		DP1 (EX2 + EX3
OS1 + OS2 + OS3)	47.63	0.09	0.36	4.48	17.27	300	1.00	33.04	2600	1.10	5.00	8.67	54.47	1.59	2.78	7.1	48.0	OS1 + OS2 + OS3
DEVELOPED CONDITIONS	1.06	0.08	0.35	0.08	0.37	300	1 70	27.73	350	1.70	0.90	6.48	34.21	2.14	3.74	0.2	1.4	PF
2P1 2P2	14.59	0.08	0.35	1.17	5.11	300	1.70 2.00	26.28	1400	1.40	0.90	0.48 29.17	<u>34.21</u> 55.45	2.14	2.75	1.8	1.4 14.0	PF
P2 PP3	22.86	0.08	0.35	2.51	5.11	300	2.00	25.51	1400	1.40	0.80	29.17 31.25	56.76	1.57	2.75	3.9	22.9	PF
75	22.00	0.11	0.37	2.31	0.40	300	2.00	25.51	1500	1.40	0.00	31.20	30.70	1.55	2.70	3.9	22.9	PF
DS1	8.67	0.08	0.35	0.69	3.03	300	1.00	33.04	1000	1.10	0.70	23.81	56.85	1.55	2.70	1.1	8.2	OS
DS2	0.07	0.08	0.35	0.03	0.17	115	2.40	15.32	115	2.40	1.20	1.60	16.92	3.18	5.55	0.1	1.0	00
DS3	1.02	0.00	0.33	0.30	0.17	30	0.50	5.00	0	0.00	0.00	0.00	5.00	5.10	9.06	1.5	4.5	00
DP1 (PP2 + PP3 + DS1 + OS2 + OS3)	47.63	0.10	0.36	4.71	17.27	300	1.00	33.04	2600	1.10	5.00	8.67	54.47	1.59	2.78	7.5	48.0	DP1 (PP2 + PP3 OS1 + OS2 + OS3
	1																	



APPENDIX C Design charts





and Use or Surface	Percent	Runoff Coefficients											
Characteristics	impervious	2-y	ear	5-y	ear	10-1	year	Z5-1	ear	ar 50-)		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSGALB	HSG C&D	HSG ALB	HSGCLD	HSG ALB	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0,58	0.62	0.60	0.65	0.62	0.68
Residential								10.00				1	
1/8Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0,34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cerneteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.54
Undeveloped Areas				-		-	1		-				
Historic Flow Analysis Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0,44	0.44	0.51	0.48	0.55	0.51	0.59
Streets								-	1		-	-	-
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	_	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30			0.50

Table 6-6. Runoff Coefficients for Rational Method (Source: UDFCD 2001)

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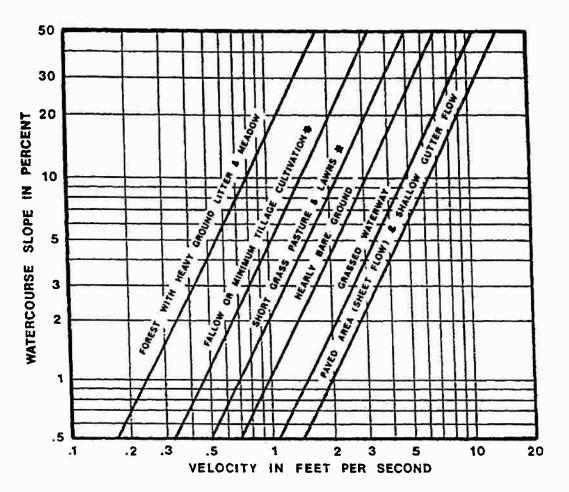


Figure 6-25. Estimate of Average Concentrated Shallow Flow

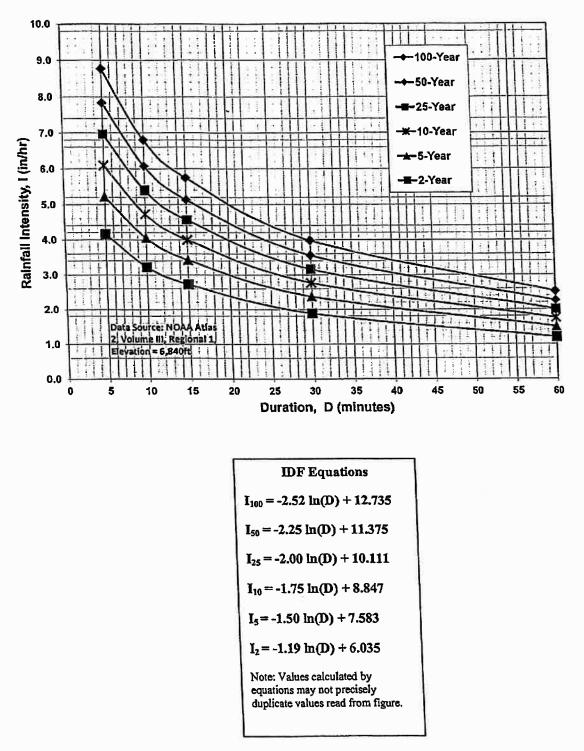


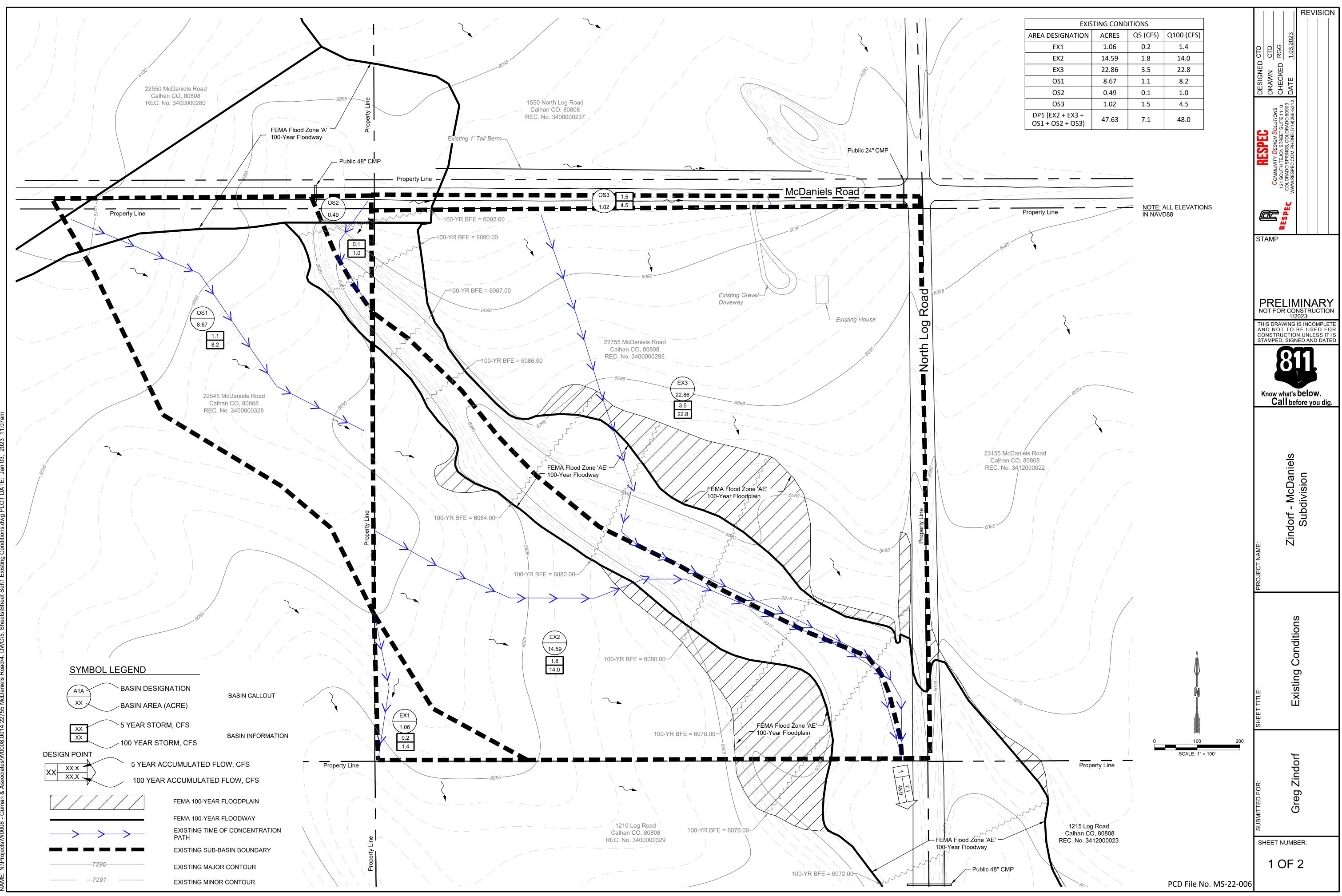
Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



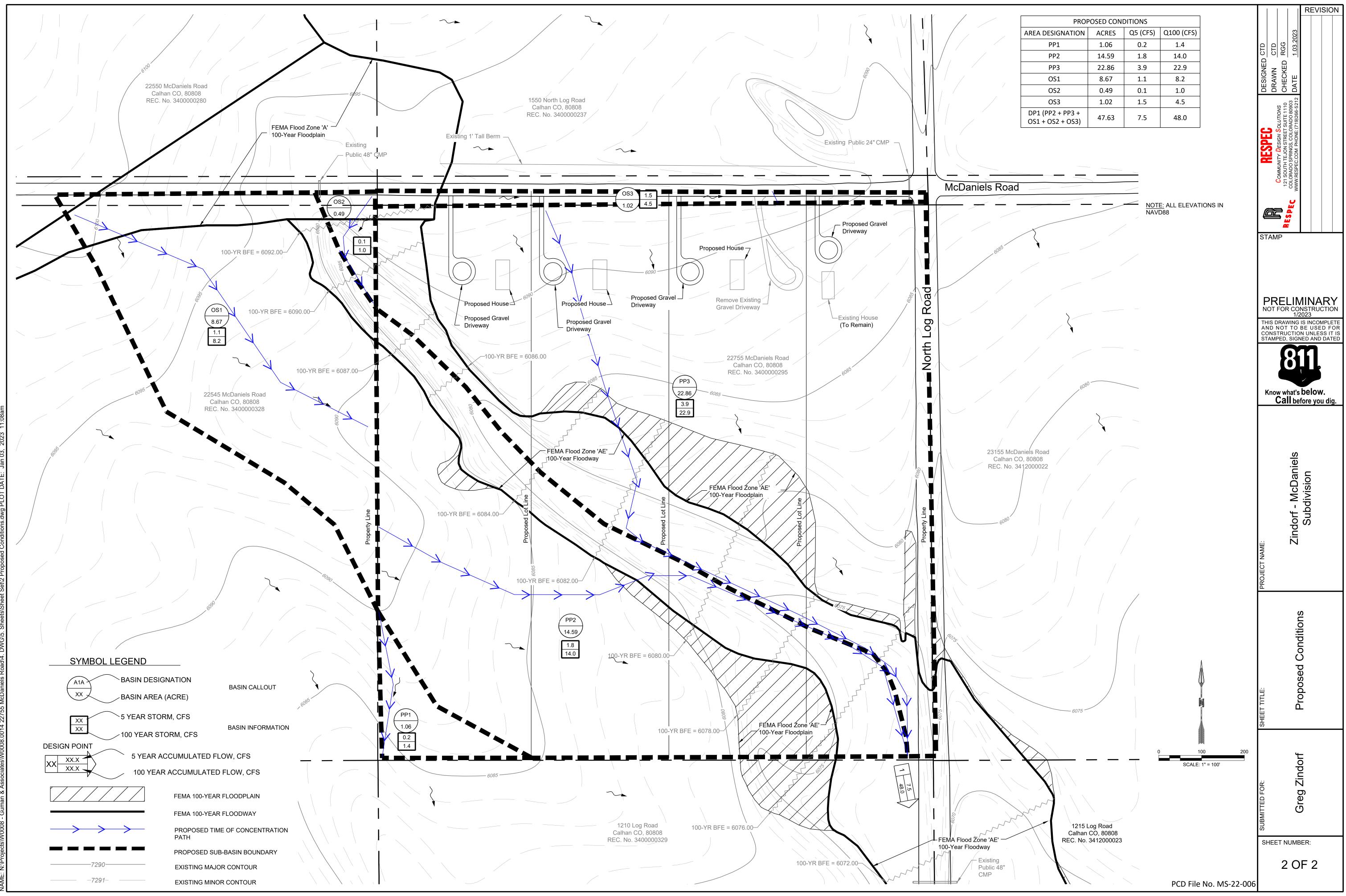
BACK POCKET







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