COMPREHENSIVE DRAINAGE ANALYSIS

Lot 2 Latigo Business Center Filing No. 1 (School District 49 Central Office Grounds)

10850 East Woodmen Road, Falcon, Colorado 80831

Prepared for:
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Kiowa Project No. 23051

January 15, 2025

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STATEMENTS AND APPROVALS

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

| drainage basin. I accept responsibility for any liability caused by any neglig part in preparing this report. | ent acts, errors or omissions on my |
|---|-------------------------------------|
| Kiowa Engineering Corporation, 1604 South Street, Colorado Springs, RADO L/C 25057 Andrew W. McCord (PE#25057) For and on Behalf of Kiowa Engineering Corporation | Colorado 80904 |
| DEVELOPER'S STATEMENT: | |
| I, the Owner/Developer, have read and will comply with all of the requireport and plan. | rements specified in this drainage |
| School District 49 Name of Developer | |
| Brune Brown | 1-21-25 |
| Authorized Signature | Date |
| Printed Name: _ Bruce Brown Title: Facility Project Manager | |
| Address: 10850 E. Woodmen Rd. Peyton, | Co. 80831 |
| School District 49: | |

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 & 2, El Paso County Engineering Criteria Manual, and Land Development Code, as amended.

Bruce Brown
School District 49

1-21-25
Date

I. PURPOSE

This report is a Comprehensive Drainage Analysis for Lot 2 Latigo Business Center Filing No. 1, located at 10850 East Woodmen Road, Falcon, Colorado 80831, for the redevelopment of the existing School District 49 Central Office Grounds.

The purpose of this report is to identify site drainage patterns and evaluate the site to meet water quality requirements per the runoff reduction standard.

II. GENERAL LOCATION AND DESCRIPTION

A. LOCATION

This proposed redevelopment is located on the School District 49 Central Office Grounds in El Paso County (Falcon, Colorado) within Lot 2 Latigo Business Center Filing No. 1 Subdivision. The parcel schedule number is 53010-02-006 and the legal description is Lot 2 Latigo Business Center Filing No 1. The parcel is located to the north of East Woodmen Road, west of Bent Grass Meadows Drive, east of Falcon Meadows Boulevard, and south of Kittrick Place. The parcel is addressed as 10850 East Woodmen Road.

The surrounding parcels are as follows:

7630 Bent Grass Meadow Drive, Schedule No. 53010-02-005, Zoning I-2, Plat No. R12007, Lot 1 Latigo Business Park Filing No. 1

7675 Falcon Meadow Boulevard, Schedule No. 53020-01-023, Zoning RR-5, Plat No. R05746, Lot 4 The Meadows Filing No. 1

7625 Falcon Meadow Boulevard, Schedule No. 53020-01-015, Zoning RR-5, Plat No. R05746, Lot 3 The Meadows Filing No. 1

 $7565\,Falcon$ Meadow Boulevard, Schedule No. 53020-01-016, Zoning RR-5, Plat No. R05746, Lot 2 The Meadows Filing No. 1

 $7525\,Falcon$ Meadow Boulevard, Schedule No. 53020-01-017, Zoning RR-5, Plat No. R05746, Lot 1 The Meadows Filing No. 1

B. DESCRIPTION OF PROPERTY – EXISTING CONDITIONS

Lot 2 Latigo Business Park Filing No. 1 Subdivision is approximately 443,715 square feet (10.19 acres) and is located on the north side of East Woodmen Road, west side of Bent Grass Meadow Drive, east of Falcon Meadows Boulevard, and south of Kittrick Place. The parcels fall within the Southwest Quarter of Section 1, Township 13 South, Range 65 West of the 6th Principal Meridian of El Paso County, Colorado.

The property currently consists of two buildings and unpaved parking lots in the northern portion of the site. There is existing curb and gutter along Bent Grass Meadow Drive and newly installed curb and gutter along East Woodmen Road.

The existing topography consists of grades between 2 and 25 percent and generally slopes from the north to south across the parcel.

C. EXISTING SOILS

The soils indicative to the site are classified as Columbine gravelly sandy loam and Blakeland loamy sand by the USDA Soil Conservation Service and are listed as National Resources Conservation Service (NRCS) Hydrologic Soil Group A. Group A soils have high

infiltration rates and low runoff potential when thoroughly wetted. A United States Department of Agriculture (USDA) Soil Map is provided in the Appendix.

D. EXISTING DRAINAGE

The existing topography consists of grades between 2 and 25 percent and generally slopes from north to south across the parcel. Existing vegetation is limited to landscaped areas around the perimeter of the existing buildings.

Existing drainage patterns are generally characterized as overland flow. Site flows are directed southeast or southwest to minor swales along the western and eastern boundaries that release to landscape islands along the northside of East Woodmen Road. Flows are then directed east to existing curb and gutter along the west side of Bent Grass Meadow Drive. Runoff then flows north to an existing public 15' CDoT Type R Curb Inlet located at the northwest corner of the intersection of East Woodmen Road and Bent Grass Meadow Drive. This public storm inlet releases east to an open channel on the opposite (east) side of Bent Grass Meadow Drive. Flows continue east and ultimately outfall into Unnamed Tributary to Black Squirrel Creek approximately one thousand feet east of the site.

Lot 2 Latigo Business Center Filing No. 1 does not lie within a designated floodplain according to information published in the Federal Emergency Management Agency (FEMA) Floodplain Map No. 08041C0535G, dated December 7, 2018. The FEMA Floodplain map is provided in Appendix A showing it lies within Zone X, a minimal flood hazard zone.

There are no known non-stormwater discharges that contribute to the storm water systems on site and downstream, both private and public. There are no known drainage reports that impact the site's drainage design.

E. DESCRIPTION OF PROPERTY – PROPOSED CONDITIONS

The proposed development consists of an addition to the existing warehouse, overlot grading to raise the northern portion of the site for construction of two new buildings, proposed new paved areas and concrete vee-pans, and new landscape areas for water quality treatment.

Planned access points to the site are unchanged at East Woodmen Road. No site access is planned for the east margin of the property from Bent Grass Meadow Drive.

III. DRAINAGE BASINS AND SUBBASINS

A. EXISTING BASINS AND SUB-BASINS

The parcel is delineated into two basins, West and East, according to the existing and proposed grading for the existing and developed conditions. Basins are delineated for the purpose of analyzing water quality treatment per the runoff reduction standard. Basin surface areas are identified as either Receiving Pervious Areas (RPA), Unconnected Impervious Areas (UIA), Directly Connected Impervious Areas (DCIA), or Separated Pervious Areas (SPA).

B. ON-SITE BASINS – DEVELOPED CONDITION

Basin W1 contains 1.05 acres of roof, paved, and lawn area. Runoff surface flows to a pervious area with a swale for water quality treatment. The swale releases south to Basin W2. Runoff reduction values for this basin are UIA=35,059 sf with the corresponding RPA=4,775 sf. The UIA/RPA interface width is 204 lf. There is also 3,253 sf of SPA.

Basin W2 contains 0.53 acres of roof, paved, and lawn area. Runoff surface flows to a pervious landscape island with a meandering swale for water quality treatment. Flows release south to Basin W4a. Runoff reduction values for this basin are UIA=18,415 sf with the corresponding RPA=4,564 sf. The UIA/RPA interface width is 176 lf.

Basin W3 contains 0.29 acres of paved and lawn area. Runoff surface flows to a pervious landscape island for water quality treatment. Flows are then conveyed west by a concrete vee-pan to Basin W2. Runoff reduction values for this basin are UIA=9,371 sf with the corresponding RPA=2,216 sf. The UIA/RPA interface width is 174 lf.

Basins W4a, W4f, and W6 contain 0.99 acres of roof, paved and lawn area. Runoff from Basin W4a surface flows to a concrete vee-pan that conveys flows south to Basin W4f. Runoff from Basin W4f surface flows to concrete vee-pan and is conveyed south to Basin W6. Basin W6 is a pervious landscape island that treats flows from Basins W4a and W4f. Basin W6 runoff is self-tributary to a meandering swale that releases south to the East Woodmen Road Right of Way. Runoff reduction values for these basins are UIA=24,086 sf with the corresponding RPA=5,918 sf. The UIA/RPA interface width is 85 lf. There is also 11,507 sf of SPA.

Basin W4b contains 0.13 acres of paved and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release to the south to Basin W4a. Runoff reduction values for this basin are UIA=4,201 sf with the corresponding RPA=1,040 sf. The UIA/RPA interface width is 72 lf. There is also 400 sf of SPA.

Basin W4c contains 0.22 acres of paved and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release south to Basin W4d. Runoff reduction values for this basin are UIA=7,741 sf with the corresponding RPA=1,017 sf. The UIA/RPA interface width is 72 lf. There is also 1,024 sf of SPA.

Basin W4d contains 0.19 acres of paved and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release south to Basin W4f. Runoff reduction values for this basin are UIA=6,027 sf with the corresponding RPA=956 sf. The UIA/RPA interface width is 112 lf.

Basin W4e contains 0.58 acres of roof, paved, and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release south to Basin W4f. Runoff reduction values for this basin are UIA=8,326 sf with the corresponding RPA=1,110 sf. The UIA/RPA interface width is 112 lf.

Basin W5 contains 0.19 acres of paved and lawn area. Runoff surface flows to a pervious landscape island for water quality treatment. Flows then release south to Basin W8. Runoff reduction values for this basin are UIA=7,428 sf with the corresponding RPA=1,063 sf. The UIA/RPA interface width is 130 lf.

Basin W7 contains 0.23 acres of paved and lawn area. Runoff surface flows to a pervious landscape island for water quality treatment. Flows then release south to Basin W8. Runoff reduction values for this basin are UIA=8,227 sf with the corresponding RPA=1,944 sf. The UIA/RPA interface width is 130 lf.

Basin W8 contains 1.95 acres of roof, paved, and lawn area. Runoff flows to pervious areas around the existing buildings for water quality treatment and parking runoff surface flows to existing pervious landscape islands along the north side of East Woodmen Road for water quality treatment. Flows are then conveyed east to the existing inlet along the west side of Bent Grass Meadow Drive. Runoff reduction values for this basin are UIA=59,245 sf with the corresponding RPA=15,337 sf. The UIA/RPA interface width is 300 lf.

Basin E1 contains 0.11 acres of roof, paved, and lawn area. Runoff surface flows to a pervious landscape island and area for water quality treatment. Flows then release south to Basins E2 and E3.

Runoff reduction values for this basin are UIA=3,375 sf with the corresponding RPA=459 sf. The UIA/RPA interface width is 40 lf. There is also 851 sf of SPA.

Basin E2 contains 0.13 acres of paved and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release south to Basins E3 and E4. Runoff reduction values for this basin are UIA=3,915 sf with the corresponding RPA=736 sf. The UIA/RPA interface width is 30 lf. There is also 891 sf of SPA.

Basin E3 contains 0.79 acres of roof, paved, and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release east to Basin E4. Runoff reduction values for this basin are UIA=27,948 sf with the corresponding RPA=3,684 sf. The UIA/RPA interface width is 256 lf.

Basin E4 contains 0.38 acres of roof, paved, and lawn area. Runoff surface flows to a pervious landscape island and area for water quality treatment. Flows then release south to Basin E7. Runoff reduction values for this basin are UIA=11,023 sf with the corresponding RPA=4,720 sf. The UIA/RPA interface width is 74 lf. There is also 583 sf of SPA.

Basin E5 contains 0.18 acres of paved and lawn area. Runoff surface flows to a pervious landscape island and area for water quality treatment. Flows then release south to Basin E6. Runoff reduction values for this basin are UIA=5,888 sf with the corresponding RPA=635 sf. The UIA/RPA interface width is 45 lf. There is also 1,098 sf of SPA.

Basin E6 contains 0.28 acres of roof, paved, and lawn area. Runoff surface flows to a pervious landscape island for water quality treatment. Flows then release south to Basin E7. Runoff reduction values for this basin are UIA=9,895 sf with the corresponding RPA=2,024 sf. The UIA/RPA interface width is 210 lf.

Basin E7 contains 0.24 acres of paved and lawn area. Runoff surface flows to a pervious landscape island for water quality treatment. Flows then release east to Basin E8. Runoff reduction values for this basin are UIA=8,323 sf with the corresponding RPA=1,188 sf. The UIA/RPA interface width is 101 lf.

Basin E8 contains 0.32 acres of paved and lawn area. Runoff surface flows to a pervious landscape island water quality treatment. Flows are conveyed south and release east to Bent Grass Meadow Drive via curb chases or level spreader. Flows continue south in existing curb and gutter to the existing inlet at the intersection with East Woodmen Road. Runoff reduction values for this basin are UIA=8,743 sf with the corresponding RPA=5,010 sf. The UIA/RPA interface width is 415 lf.

Basin E9 contains 0.24 acres of roof, paved, and lawn area. Runoff surface flows to a pervious area for water quality treatment. Flows then release east to Basin E10. Runoff reduction values for this basin are UIA=7,274 sf with the corresponding RPA=1,203 sf. The UIA/RPA interface width is 130 lf. There is also 1,957 sf of SPA.

Basin E10 contains 0.70 acres of roof, paved, and lawn area. Roof runoff discharges via downspouts to pervious areas for water quality treatment. Flows then converge with the remaining basin flows and are conveyed south by a concrete vee-pan to Basins E11 and E12. Runoff reduction values for this basin are UIA=28,551 sf with the corresponding RPA=1,147 sf. The UIA/RPA interface width is 50 lf. There is also 107 sf of SPA. To lessen potential icing along the eastern edge of the existing building, an alternative water quality treatment method would be to extend the existing downspouts to a manifold system that connects to a Stormcepter system that then discharges to an area inlet. Flows would ultimately release by ponding up through the inlet to the concrete vee-pan.

Basin E11 contains 0.18 acres of paved and lawn area. Runoff surface flows to a concrete vee-pan and is conveyed south to Basin E12. The runoff reduction value for this basin is 7,021 sf of SPA.

Please address need for detention and current and proposed flows from the property.

Basin E12 contains 0.22 acres of roof, paved, and lawn area. Runoff flows to pervious areas around the existing buildings for water quality treatment. Flows then continue to a concrete vee-pan that conveys flows south to a proposed area inlet that connects via an 18" Reinforced Concrete Pipe (RCP) to the existing inlet at the intersection of Bent Grass Meadow Drive and East Woodmen Road. Runoff reduction values for this basin are UIA=4,986 sf with the corresponding RPA=4,130 sf. The UIA/RPA interface width is 120 lf.

IV. DRAINAGE DESIGN CRITERIA

A. REGULATIONS

Permanent Control Measures (PCMs) must be provided to achieve Water Quality Treatment. Refer to Green Infrastructure Exhibits in the Appendix for locations of treatment areas.

B. DEVELOPMENT CRITERIA REFERENCE AND CONSTRAINTS

The parcel falls within the Falcon Area Drainage Basin designated by El Paso County Engineering Division with the ultimate receiving waters being the Arkansas River by way of Black Squirrel and Fountain Creeks.

Water Quality Capture Volume (WQCV) runoff reduction was evaluated utilizing the Mile High Flood District (MHFD) spreadsheet UD-BMP v3.07.

Under developed conditions, the drainage on this parcel will have no effect on downstream infrastructure or facilities, streets, utilities, transit, or further development of adjacent lots.

C. HYDROLOGICAL CRITERIA applaned to achieve WQ treatment, correct?

Rational Method runoff calculations have not been performed for existing or developed conditions. The site has solely been analyzed to meet the water quality runoff reduction standard.

Potential Best Management Practices (PBMPs) in the form of Planned Infiltration Areas (PIAs) are planned to achieve Water Quality Treatment.

D. FOUR-STEP PROCESS

The selection of appropriate control measures is based on the characteristics of the site and potential pollutants. The Four-Step Process provides a method of going through the selection process. The following applies the Four-Step Process to the development of Lot 2 Latigo Business Center.

Step 1: Employ Runoff Reduction Practices

The site strategically locates pervious landscape islands and areas to treat unconnected impervious areas. Given the proposed land use, the majority of the site consists of roof or paved surface. The UD-BMP spreadsheet was utilized to quantify the WQCV reduction percentage. The site globally achieves a WQCV reduction percentage of 60%. Runoff Reduction for the whole site is summarized in the table on the following page below:

Please provide maintenance agreement for the maintenance of the RPAs/other treatment facilities. Maintenance agreement templates can be found on the County website here: https://publicworks.elpasoco.com/department-publicworks/county-engineer/

| Water Qu | Water Quality Treatment Credit Value Summary | | | | | | | | | | | |
|-----------|--|--------|----------|--------|--|--|--|--|--|--|--|--|
| Desig. | S.F. | Credit | Acres | % Site | | | | | | | | |
| E1-E8 | 100988 | 77 | 2.318365 | 25.15 | | | | | | | | |
| E9-12 | 56376 | 49 | 1.294215 | 14.04 | | | | | | | | |
| W1-W4 | 151007 | 83 | 3.466644 | 37.60 | | | | | | | | |
| W5-W8 | 93244 | 96 | 2.140588 | 23.22 | | | | | | | | |
| Total sf: | 401615 | 60 | 9.219812 | 100.00 | | | | | | | | |

Step 2: Provide Water Quality Capture Volume

Volume water quality treatment is not required for the site, based on the site globally achieving the minimum WQCV reduction of 60% by the runoff reduction standard.

Step 3: Stabilize Drainageways

The drainage within the site is stabilized with features such as grassed swales, valley pans, an area inlet, and sloped pavement to direct storm water through the site to East Woodmen Road and Bent Grass Meadow Drive Rights of Ways and ultimately to the public curb inlet at the intersection. There are no unstabilized drainageways on this site.

Step 4: Implement Site Specific and Other Source Control BMPs

Site specific Potential Best Management Practices, pervious landscape islands and areas, are utilized to maximize opportunities for infiltration on-site.

V. CONCLUSIONS

The comprehensive drainage analysis for the redevelopment of Lot 2 Latigo Business Park has been evaluated to meet the water quality runoff reduction standard (minimum 60% reduction of the required WQCV) per MHFD Manual Volume 3 and corresponding spreadsheet. The design presented in this report reduces the required WQCV by 60%.

There is no impact on major drainageway planning studies within the larger drainage basin. This development will not adversely affect downstream development.

VI. REFERENCES

El Paso County & Colorado Springs Drainage Manual Volumes I & II (May 2014)

El Paso County Engineering Criteria Manual, El Paso County, Colorado, (Rev. 12/16/2013)

<u>Urban Storm Drainage Criteria Manual, Vol. 1, 2 and 3, and Spreadsheets</u>, Mile High Flood District, latest revisions.

<u>Soil Survey of El Paso County Area, Colorado</u>, prepared by United States Department of Agriculture Soil Conservation Service, dated June 1981.

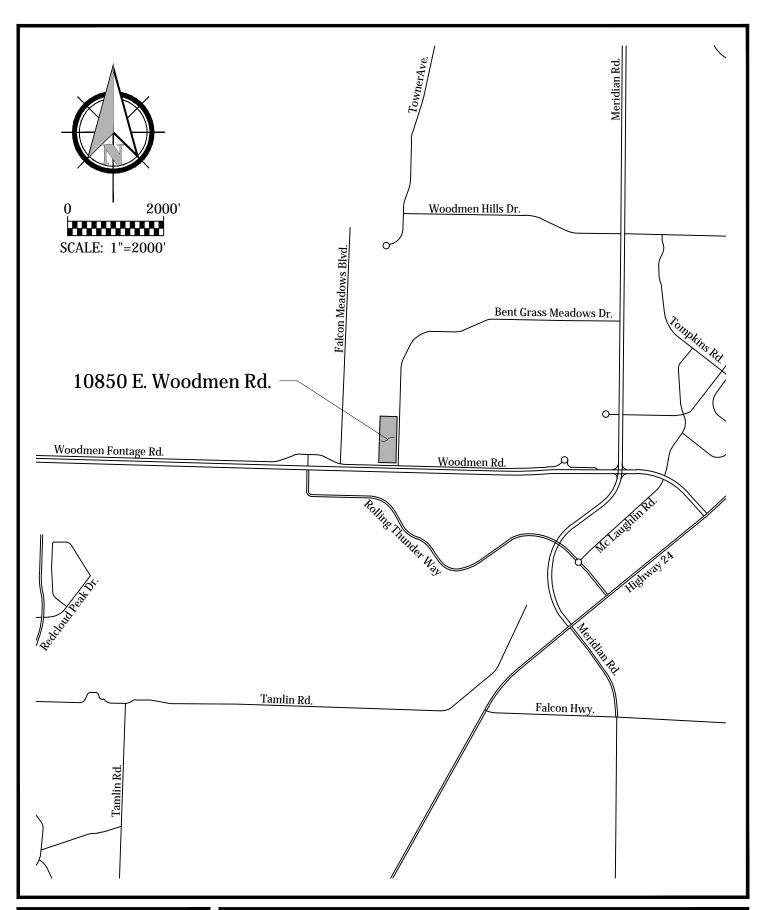
FEMA Flood Online Map Service Center

United States Department of Agriculture National Resources Conservation Service

APPENDIX TABLE OF CONTENTS

APPENDIX A

Figure 1: Vicinity Map Figure 2: Soils Map FEMA Flood Insurance Rate Map





Vicinity Map School District 49 - Grounds Expansion 10850 Woodmen Road - Colorado Springs, CO 80908



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:24.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 22, Sep 3, 2024 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Sep 11, 2018—Oct 20. 2018 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

| | , | | | |
|---------------------------|--|--------|--------------|----------------|
| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
| 8 | Blakeland loamy sand, 1 to 9 percent slopes | А | 1.2 | 10.0% |
| 19 | Columbine gravelly sandy loam, 0 to 3 percent slopes | A | 10.6 | 90.0% |
| Totals for Area of Intere | est | 1 | 11.8 | 100.0% |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

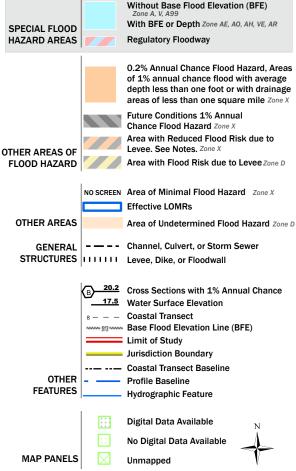
National Flood Hazard Layer FIRMette





Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap

accuracy standards

The pin displayed on the map is an approximate point selected by the user and does not represent

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/2/2025 at 8:21 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

APPENDIX B
Water Quality Calculations and Exhibit

Use up to date MHFD-BMP spreadsheet for runoff reduction. Typical comment.

| | | | Desig | n Procedu | re Form: F | Runoff Red | luction | | | | | | |
|---|---|---|---------------|----------------|------------------|--|--------------|---------------|------------|------------|------------|--------------|--|
| | | | | UD-BMP (Ve | ersion 3.07, Mai | rch 2018) | | | | | | Sheet 1 of 1 | |
| Designer: | AWMc | AWMC Kiowa Engineering Corporation | | | | | | | | | | | |
| Company: | | | | | | | | | | | | | |
| Date: | | January 8, 2025 School District 49 Admin Campus | | | | | | | | | | | |
| Project: | | | | | | | | | | | | | |
| Location: | 18650 E WOO | amen Ka, El Pi | aso County, C | o (Faicon) Are | eas W1 thru W | 4 | | | | | - | | |
| | SITE INFORMATION (User Input in Blue Cells) | | | | | | | | | | | | |
| SITE INFORMATION (Us | WQCV Rainfall Depth 0.60 inches | | | | | | | | | | | | |
| Depth of Average Rui | | | | 1 | /atersheds O | utside of the [| Denver Regio | n, Figure 3-1 | in USDCM V | ol. 3) | | | |
| Area Type | UIA:RPA | SPA | UIA:RPA | UIA:RPA | UIA:RPA | SPA | UIA:RPA | UIA:RPA | UIA:RPA | UIA:RPA | SPA | SPA | |
| Area ID | W1-RPA | W1-SPA | W2 | W3 | W4a,f+W6 | W4a+W4f | W4b-RPA | W4c-RPA | W4d-RPA | W4e-RPA | W4b-SPA | W4c-SPA | |
| Downstream Design Point ID | na | na | na | na | na | na | na | na | na | na | na | na | |
| Downstream BMP Type | None | None | None | None | None | None | None | None | None | None | None | None | |
| DCIA (ft ²) | | | - | | | | | | | - | | | |
| UIA (ft²) | 35,059 | | 18,415 | 9,371 | 24,086 | | 4,201 | 7,741 | 6,027 | 8,326 | | | |
| RPA (ft²) | 4,775 | | 4,564 | 2,216 | 5,918 | 44.507 | 1,040 | 1,017 | 956 | 1,110 | | | |
| SPA (ft²) | | 3,253 | | | | 11,507 | | | | | 400 | 1,024 | |
| HSG A (%) HSG B (%) | 100% 0% | | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | 100% 0% | |
| HSG C/D (%) | 0% | | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | |
| Average Slope of RPA (ft/ft) | 0.040 | | 0.045 | 0.070 | 0.020 | | 0.030 | 0.025 | 0.015 | 0.020 | | | |
| UIA:RPA Interface Width (ft) | 204.00 | | 176.00 | 174.00 | 85.00 | | 72.00 | 72.00 | 112.00 | 112.00 | | | |
| () | | | ., ., ., | | 55155 | | | 7 = 100 | 112100 | | | | |
| CALCULATED RUNOFF | DECILI TO | | | | | | | | | | | | |
| Area ID | W1-RPA | W1-SPA | W2 | W3 | W4a,f+W6 | W4a+W4f | W4b-RPA | W4c-RPA | W4d-RPA | W4e-RPA | W4b-SPA | W4c-SPA | |
| UIA:RPA Area (ft²) | 39,834 | | 22,979 | 11,587 | 30,004 | ************************************** | 5,241 | 8,758 | 6,983 | 9,437 | W4D-3FA | | |
| L/W Ratio | 0.96 | | 0.74 | 0.38 | 4.15 | - | 1.01 | 1.69 | 0.56 | 0.75 | | | |
| UIA / Area | 0.8801 | | 0.8014 | 0.8088 | 0.8028 | | 0.8016 | 0.8839 | 0.8631 | 0.8823 | | | |
| Runoff (in) | 0.14 | 0.00 | 0.00 | 0.02 | 0.00 | 0.00 | 0.00 | 0.16 | 0.12 | 0.16 | 0.00 | 0.00 | |
| Runoff (ft ³) | 480 | 0 | 0 | 17 | 0 | 0 | 1 | 117 | 69 | 125 | 0 | 0 | |
| Runoff Reduction (ft ³) | 981 | 163 | 767 | 374 | 1004 | 575 | 174 | 205 | 182 | 222 | 20 | 51 | |
| | | | | | | | | | | | | | |
| CALCULATED WQCV RI | | | 1110 | 1 | | 1014 10145 | | l | | | | | |
| Area ID | W1-RPA | W1-SPA | W2 | W3 | W4a,f+W6 | W4a+W4f | W4b-RPA | W4c-RPA | W4d-RPA | W4e-RPA | W4b-SPA | W4c-SPA | |
| WQCV (ft ³) | 1461 | 0 | 767 767 | 390 374 | 1004 1004 | 0 | 175 174 | 323 205 | 251 182 | 347 222 | 0 | 0 | |
| WQCV Reduction (ft ³) WQCV Reduction (%) | 981 67% | 0% | 100% | 96% | 1004 | 0% | 99% | 64% | 73% | 64% | 0% | 0% | |
| Untreated WQCV (ft ³) | 480 | 0 / 0 | 0 | 17 | 0 | 0 70 | 1 | 117 | 69 | 125 | 0 70 | 0 | |
| Onlineated WQCV (It.) | 400 | Ů | Ü | | Ů | | | 111 | 00 | 120 | Ŭ | Ů | |
| CALCULATED DESIGN I | | LTS (sums re | esults from a | all columns v | vith the same | Downstrea | m Design Po | oint ID) | | | | | |
| Downstream Design Point ID | na | | | | | | | | | | | | |
| DCIA (ft²) | 0 | | | | | | | | | | | | |
| UIA (ft²) | 113,226 21,596 | | | | | | | | | | | | |
| RPA (ft²) | 16,185 | | | | | | | | | | | | |
| SPA (ft²) Total Area (ft²) | 151,007 | | | | | | | | | | | | |
| Total Impervious Area (ft²) | 113,226 | | | | | | | | | | | | |
| WQCV (ft ³) | 4,718 | | | | | | | | | | | | |
| WQCV Reduction (ft ³) | 3,909 | | | | | | | | | | | | |
| WQCV Reduction (%) | 83% | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 808 | | | | | | | | | | | | |
| - () | | | | • | • | | | • | | | | | |
| CALCULATED SITE RES | ULTS (sums | results from | all columns | s in workshe | et) | | | | | | | | |
| Total Area (ft ²) | 151,007 | | | | | | | | | | | | |
| Total Impervious Area (ft²) | 113,226 | | | | | | | | | | | | |
| WQCV (ft ³) | 4,718 | | | | | | | | | | | | |
| WQCV Reduction (ft ³) | 3,909 | | | | | | | | | | | | |
| WQCV Reduction (%) | 83% | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 808 | l | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

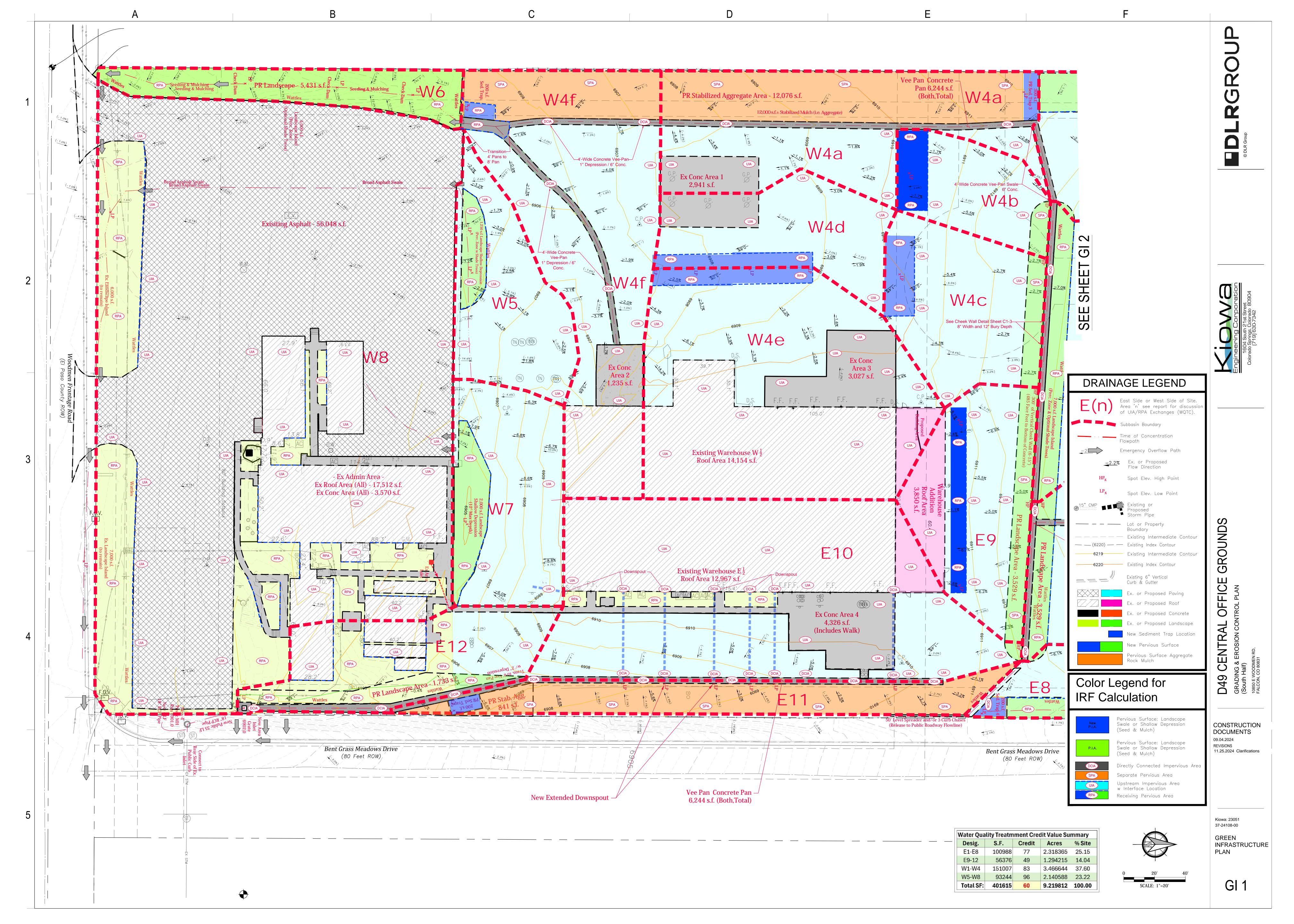
| Design Procedure Form: Runoff Reduction | | | | | | | | | | | | | |
|--|--------------|---|--------------|-------------------------|-----------------|-----------------|------------------|----------------|----------------|--------|---|--|--|
| | | | | UD-BMP (Ve | ersion 3.07, Ma | rch 2018) | | | | | | Sheet 1 of 1 | |
| Designer: | AWMc | i C | -41 | | | | | | | | | | |
| Company: Date: | Kiowa Engine | • | | | | | | | | | | | |
| Project: | | January 8, 2025 School District 49 Admin Campus | | | | | | | | | | | |
| Location: | | | | Co (Falcon) Ar | eas W5, W7 ar | nd W8 (W6 Om | itted as it is u | sed elsewhere | in calculation | 1) | | | |
| | | | | | | | | | | | | | |
| SITE INFORMATION (Us | | | | , | | | | | | | | | |
| Depth of Average Rui | | tainfall Depth g Storm, d ₆ = | 0.60 | inches inches (for V | Vatersheds O | utside of the I | Denver Regio | on, Figure 3-1 | in USDCM V | ol. 3) | | | |
| Area Type | UIA:RPA | UIA:RPA | UIA:RPA | | | | | | | | | | |
| Area ID | W5-RPA | W7-RPA | W8-RPA | | | | | | | | | | |
| Downstream Design Point ID | na | na | na | | | | | | | | | | |
| Downstream BMP Type | None | None | None | | | | | | | | | | |
| DCIA (ft²) | | | | | | | | | | | | - | |
| UIA (ft²) | 7,428 | 8,227 | 59,245 | | | | | | | | | - | |
| RPA (ft²) SPA (ft²) | 1,063 | 1,944 | 15,337 | | | | | | | | | | |
| HSG A (%) | 100% | 100% | 100% | | | | | | | | | | |
| HSG B (%) | 0% | 0% | 0% | | | | | | | | | | |
| HSG C/D (%) | | 0% | 0% | | | | | | | | | | |
| Average Slope of RPA (ft/ft) | 0.055 | 0.070 | 0.040 | | | | | | | | | | |
| UIA:RPA Interface Width (ft) | 130.00 | 130.00 | 300.00 | | | | | | | | | | |
| | | | | | | | | | | | | | |
| CALCULATED RUNOFF | RESULTS | | | | | | | | | | | | |
| Area ID | W5-RPA | W7-RPA | W8-RPA | | | | | | | | | | |
| UIA:RPA Area (ft ²) | 8,491 | 10,171 | 74,582 | | | | | | | | | | |
| L / W Ratio | 0.50 | 0.60 | 0.83 | | | | | | | | | | |
| UIA / Area | 0.8748 | 0.8089 | 0.7944 | | | | | | | | | - | |
| Runoff (in) Runoff (ft ³) | 0.15 103 | 0.02 15 | 0.00 | | | | | | | | | | |
| Runoff Reduction (ft ³) | | 328 | 2469 | | | | | | | | | | |
| | | • | • | • | | | | • | | | • | | |
| CALCULATED WQCV RI | | · | · | 1 | | 1 | 1 | | | | | | |
| Area ID | | W7-RPA | W8-RPA | | | | | | | | | | |
| WQCV (ft ³) WQCV Reduction (ft ³) | 310 207 | 343 328 | 2469 2469 | | | | | | | | | | |
| WQCV Reduction (it) WQCV Reduction (%) | 67% | 96% | 100% | | | | | | | | | + | |
| Untreated WQCV (ft ³) | | 15 | 0 | | | | | | | | | | |
| , | | Į. | Į. | | | Į. | | | | | | | |
| CALCULATED DESIGN I | | LTS (sums r | esults from | all columns v | with the sam | e Downstrea | m Design Po | oint ID) | | | | | |
| Downstream Design Point ID DCIA (ft²) | na 0 | | | | | | | | | | | + | |
| DCIA (ft²) | 74,900 | | | | | | | | | | | | |
| RPA (ft²) | | | | | | | | | | | | | |
| SPA (ft²) | | | | | | | | | | | | | |
| Total Area (ft ²) | | | | | | | | | | | | | |
| Total Impervious Area (ft²) | | | | | | | | | | | | | |
| WQCV (ft ³) | 3,121 | | | | | | | | | | | | |
| WQCV Reduction (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (%) | | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 118 | | | | | | | | | | | | |
| CALCULATED SITE RES | SULTS (sums | results fron | n all column | s in workshe | et) | | | | | | | | |
| Total Area (ft ²) | |] | | | • | | | | | | | | |
| Total Impervious Area (ft ²) | 74,900 | | | | | | | | | | | | |
| WQCV (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (%) | | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 118 | l | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

| | | | Dosio | n Procedu | re Form: I | Runoff Rec | luction | | | | | |
|--|----------------|---------------------------|---------------|---------------|--------------------|-----------------|--------------|---------------|-------------|-----------|-----------|---------------|
| | | | Desig | : | ersion 3.07, Ma | | duction | | | | | Sheet 1 of 1 |
| Designer: | AWMc | | | OB-BINII (VC | 731011 0.07 , IVIG | 1011 20 10) | | | | | | Sileet i oi i |
| Company: | Kiowa Engine | ering Corpora | ation | | | | | | | | - | |
| Date: | January 8, 20 | 25 | | | | | | | | | • | |
| Project: | School Distri | ct 49 Admin C | ampus | | | | | | | | _ | |
| Location: 18650 E Woodmen Rd, El Paso County, Co (Falcon) Areas E1 thru E8 | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| SITE INFORMATION (Us | | | | | | | | | | | | |
| | | ainfall Depth | 0.60 | inches | | | | | | | | |
| Depth of Average Rui | noff Producing | g Storm, d ₆ = | 0.43 | inches (for W | /atersheds O | utside of the I | Denver Regio | n, Figure 3-1 | in USDCM V | ol. 3) | | |
| Area Type | UIA:RPA | SPA | UIA:RPA | SPA | UIA:RPA | UIA:RPA | SPA | UIA:RPA | SPA | UIA:RPA | UIA:RPA | UIA:RPA |
| Area ID | E1-RPA | E1-SPA | E2-RPA | E2-SPA | E3-RPA | E4-RPA | E4-SPA | E5-RPA | E5-SPA | E6 | E7 | E8 |
| Downstream Design Point ID | na | na | na | na | na | na | na | na | na | na | na | na |
| Downstream BMP Type | None | None | None | None | None | None | None | None | None | None | None | None |
| DCIA (ft ²) | | | - | | | | | | | | | |
| UIA (ft²) | 3,375 | | 3,915 | | 27,948 | 11,023 | | 5,888 | | 9,895 | 8,323 | 8,743 |
| RPA (ft²) | 459 | | 736 | | 3,684 | 4,720 | | 635 | 4.000 | 2,024 | 1,188 | 5,010 |
| SPA (ft²) HSG A (%) | 100% | 851 | 100% | 891 100% | 100% | 100% | 583 | 100% | 1,098 | 100% | 100% | 100% |
| HSG B (%) | 0% | | 0% | 0% | 0% | 0% | | 0% | | 0% | 0% | 0% |
| HSG C/D (%) | 0% | | 0% | 0% | 0% | 0% | | 0% | | 0% | 0% | 0% |
| Average Slope of RPA (ft/ft) | 0.050 | | 0.045 | | 0.042 | 0.044 | | 0.048 | | 0.016 | 0.050 | 0.050 |
| UIA:RPA Interface Width (ft) | 40.00 | | 30.00 | | 256.00 | 74.00 | | 45.00 | | 210.00 | 101.00 | 415.00 |
| | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| CALCULATED RUNOFF Area ID | E1-RPA | E1-SPA | E2-RPA | E2-SPA | E3-RPA | E4-RPA | E4-SPA | E5-RPA | E5-SPA | E6 | E7 | E8 |
| UIA:RPA Area (ft²) | 3,834 | | 4,651 | | 31,632 | 15,743 | | 6,523 | | 11,919 | 9,511 | 13,753 |
| L/W Ratio | 2.40 | | 5.17 | | 0.48 | 2.87 | | 3.22 | | 0.27 | 0.93 | 0.08 |
| UIA / Area | 0.8802 | | 0.8418 | | 0.8835 | 0.7002 | | 0.9026 | | 0.8302 | 0.8751 | 0.6357 |
| Runoff (in) | 0.15 | 0.00 | 0.07 | 0.00 | 0.16 | 0.00 | 0.00 | 0.20 | 0.00 | 0.05 | 0.14 | 0.00 |
| Runoff (ft ³) | 49 | 0 | 27 | 0 | 410 | 0 | 0 | 110 | 0 | 52 | 115 | 0 |
| Runoff Reduction (ft ³) | 91 | 43 | 136 | 45 | 754 | 459 | 29 | 136 | 55 | 360 | 232 | 364 |
| | | | | | | | | | | | | |
| CALCULATED WQCV RI | | E4 CDA | E2 DD4 | F0.0D4 | F2 DD4 | E4 DDA | E4 CDA | LE DDA | LE CDA | | | |
| Area ID WQCV (ft ³) | E1-RPA 141 | E1-SPA 0 | E2-RPA 163 | E2-SPA 0 | E3-RPA 1164 | E4-RPA 459 | E4-SPA 0 | E5-RPA 245 | E5-SPA 0 | E6 412 | E7 347 | E8 364 |
| WQCV (It) WQCV Reduction (ft ³) | 91 | 0 | 136 | 0 | 754 | 459 | 0 | 136 | 0 | 360 | 232 | 364 |
| WQCV Reduction (%) | 65% | 0% | 83% | 0% | 65% | 100% | 0% | 55% | 0% | 87% | 67% | 100% |
| Untreated WQCV (ft ³) | 49 | 0 | 27 | 0 | 410 | 0 | 0 | 110 | 0 | 52 | 115 | 0 |
| | | | | - | - | | - | - | | - | | |
| CALCULATED DESIGN I | | LTS (sums r | esults from | all columns v | vith the same | e Downstrea | m Design Po | oint ID) | | | | |
| Downstream Design Point ID | na | | | | | | | | | | | |
| DCIA (ft²) | 79,110 | | | | | | | | | | | |
| UIA (ft²) RPA (ft²) | 18,456 | | | | | | | | | | | |
| SPA (ft²) | | | | | | | | | | | | |
| Total Area (ft ²) | 100,988 | | | | | | | | | | | |
| Total Impervious Area (ft²) | 79,110 | | | | | | | | | | | |
| WQCV (ft ³) | 3,296 | | | | | | | | | | | |
| WQCV Reduction (ft ³) | 2,533 | | | | | | | | | | | |
| WQCV Reduction (%) | 77% | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 763 | | | | | | | | | | | |
| CALCULATED SITE RES | SHI TS /euma | roculte from | all column | in workeho | ot) | | | | | | | |
| Total Area (ft²) | | results IFON | i ali columni | o iii worksne | et) | | | | | | | |
| Total Alea (IL) | 70.440 | | | | | | | | | | | |

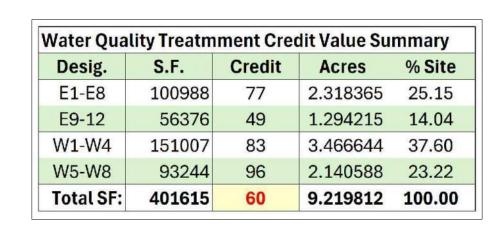
| Total Area (ft²) | 100,988 |
|-----------------------------------|---------|
| Total Impervious Area (ft2) | 79,110 |
| WQCV (ft ³) | 3,296 |
| WQCV Reduction (ft3) | |
| WQCV Reduction (%) | 77% |
| Untreated WQCV (ft ³) | 763 |

| Design Procedure Form: Runoff Reduction | | | | | | | | | | | | | |
|--|-----------------|---------------------------|-------------------|---------------|----------------|------------------|--------------|---------------|------------|--------|---|--|--|
| | | | | UD-BMP (Ve | rsion 3.07, Ma | rch 2018) | | | | | | Sheet 1 of 1 | |
| Designer: | AWMc | | | | | | | | | | _ | | |
| Company: | Kiowa Engine | | ation | | | | | | | | _ | | |
| Date: | January 8, 20 | - | | | | | | | | | | | |
| Project: | School Distri | | | \- (F-1\ A | F0 4h F4 | | | | | | - | | |
| Location: | 18650 E Woo | - | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| SITE INFORMATION (Us | er Input in B | lue Cells) | | | | | | | | | | | |
| | | tainfall Depth | | inches | | | | | | | | | |
| Depth of Average Rui | noff Producing | g Storm, d ₆ = | 0.43 | inches (for W | /atersheds O | utside of the [| Denver Regio | n, Figure 3-1 | in USDCM V | ol. 3) | | | |
| Area Type | UIA:RPA | SPA | UIA:RPA | SPA | SPA | UIA:RPA | | | | | | | |
| Area ID | E9-RPA | E9-SPA | E10-RPA | E10-SPA | E11-SPA | E12-RPA | | | | | | | |
| Downstream Design Point ID | na | na | na | na | na | na | | | | | | | |
| Downstream BMP Type | None | None | None | None | None | None | | | | | | | |
| DCIA (ft ²) | | | | | | | | | | | | | |
| UIA (ft²) | | | 28,551 | | | 4,986 | | | | | | | |
| RPA (ft²) | 1,203 | | 1,147 | | | 4,130 | | | | | | | |
| SPA (ft²) | 100% | 1,957 100% | 1009/ | 107 | 7,021 | 1000/ | | | | | | | |
| HSG A (%) HSG B (%) | 100% | 100% 0% | 100% 0% | | 100% 0% | 100% 0% | | | | | | | |
| HSG C/D (%) | | 0% | 0% | | 0% | 0% | | | | | | | |
| Average Slope of RPA (ft/ft) | 0.055 | | 0.010 | | | 0.020 | | | | | | | |
| UIA:RPA Interface Width (ft) | 130.00 | | 50.00 | - | - | 120.00 | | | | | | | |
| | | • | • | | | | | | | | • | | |
| | | | | | | | | | | | | | |
| CALCULATED RUNOFF | | F0.0D4 | E40 DD4 | E40.0D4 | E44.0D4 | E40 DD4 | | | | | 1 | | |
| Area ID UIA:RPA Area (ft²) | E9-RPA 8,477 | E9-SPA | E10-RPA 29,698 | E10-SPA | E11-SPA | E12-RPA 9,116 | | | | | | | |
| L / W Ratio | 0.50 | | 11.88 | - | | 0.63 | | | | | | | |
| UIA / Area | 0.8581 | | 0.9614 | | | 0.5470 | | | | | | | |
| Runoff (in) | 0.11 | 0.00 | 0.32 | 0.00 | 0.00 | 0.00 | | | | | | | |
| Runoff (ft ³) | 78 | 0 | 783 | 0 | 0 | 0 | | | | | | | |
| Runoff Reduction (ft ³) | 225 | 98 | 406 | 5 | 351 | 208 | | | | | | | |
| 041 0111 4750 140014 01 | - O. II - TO | | | | | | | | | | | | |
| CALCULATED WQCV RI Area ID | | E9-SPA | E10-RPA | E10-SPA | E11-SPA | E12-RPA | | | | | 1 | 1 | |
| WQCV (ft ³) | 303 | 0 0 | 1190 | 0 | 0 | 208 | | | | | | | |
| WQCV (II) WQCV Reduction (ft ³) | | 0 | 406 | 0 | 0 | 208 | | | | | | | |
| WQCV Reduction (%) | 74% | 0% | 34% | 0% | 0% | 100% | | | | | | | |
| Untreated WQCV (ft ³) | 78 | 0 | 783 | 0 | 0 | 0 | | | | | | | |
| | | | - | | | | | | | | | | |
| CALCULATED DESIGN I | | LTS (sums r | esults from a | all columns v | vith the sam | e Downstrea | m Design Po | int ID) | | | | | |
| Downstream Design Point ID | na 0 | | | | | | | | | | | | |
| DCIA (ft²) UIA (ft²) | 40,811 | | | | | | | | | | | | |
| RPA (ft²) | 6,480 | | | | | | | | | | | | |
| SPA (ft²) | 9,085 | | | | | | | | | | | | |
| Total Area (ft ²) | | | | | | | | | | | | | |
| Total Impervious Area (ft²) | 40,811 | | | | | | | | | | | | |
| WQCV (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (%) | | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 862 | | | | | | | | | | | | |
| CALCULATED SITE RES | SULTS (sums | results fron | n all columns | in workshe | et) | | | | | | | | |
| Total Area (ft ²) | | | | | J., | | | | | | | | |
| Total Impervious Area (ft²) | | | | | | | | | | | | | |
| WQCV (ft ³) | |] | | | | | | | | | | | |
| WQCV Reduction (ft ³) | | | | | | | | | | | | | |
| WQCV Reduction (%) | | | | | | | | | | | | | |
| Untreated WQCV (ft ³) | 862 | l | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

| Water Quality Treatmment Credit Value Summary | | | | | | | | | | | |
|---|--------|--------|----------|--------|--|--|--|--|--|--|--|
| Desig. | S.F. | Credit | Acres | % Site | | | | | | | |
| E1-E8 | 100988 | 77 | 2.318365 | 25.15 | | | | | | | |
| E9-12 | 56376 | 49 | 1.294215 | 14.04 | | | | | | | |
| W1-W4 | 151007 | 83 | 3.466644 | 37.60 | | | | | | | |
| W5-W8 | 93244 | 96 | 2.140588 | 23.22 | | | | | | | |
| Total SF: | 401615 | 60 | 9.219812 | 100.00 | | | | | | | |







W¹/₂ PR Covered Pkg Rooftop

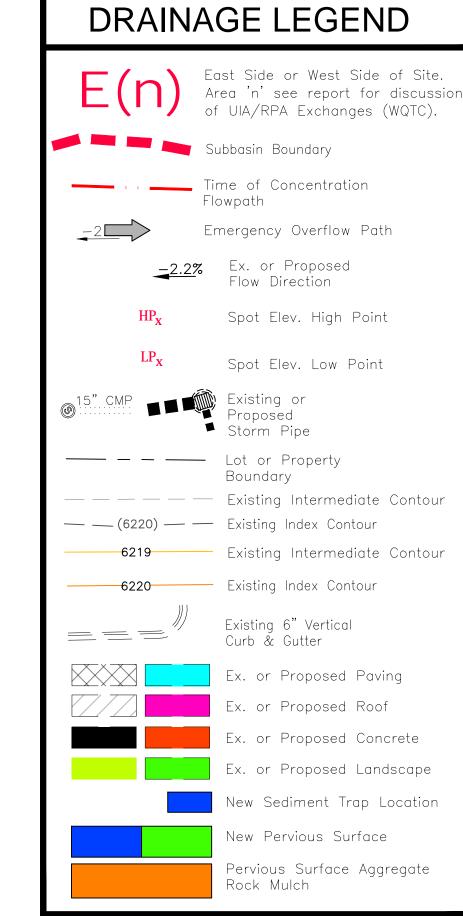
12,929 s.f.

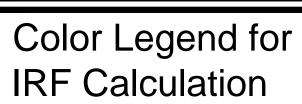
└ Downspout ┘

| W ½ PR East Barn Rooftop

E ½ PR East Barn Rooftop

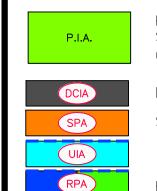
— Downspout —







Pervious Surface: Landscape Swale or Shallow Depression (Seed & Mulch) Pervious Surface: Landscape Swale or Shallow Depression



Directly Connected Impervious Area Separate Pervious Area Upstream Impervious Area w Interface Location Receiving Pervious Area

o Public Roadway Flowline)

Pan 6,244 s.f.

W4c

See Cheek Wall Detail Sheet C1-3-8" Width and 12 Bury Depth

<u>-5.0</u>%

Bent Grașs Meadows Drive

(80 Feet ROW)

4'-Wide Concrete Vee-Pan

Conc Stoops

153 s.f. total

2,500 s.f.± Landscape Island Receiving Pervious Area for Roof Flows

-5.1%

GI 2

INFRASTRUCTURE

FFIC

D49

09.04.2024

Kiowa: 23051

CONSTRUCTION DOCUMENTS

11.25.2024 Clarifications