

October 16, 2024

Brady Shyrock, on Behalf of Galloway  
1155 Kelly Johnson Blvd., Suite 305  
Colorado Springs, CO 80920

RE: Constitution Storage Development; Full Spectrum Detention Pond Certification

Private?

Dear Natasha Grimaldo,

Please accept this letter as formal documentation of conformance of the Full Spectrum Detention Pond (Pr. FSD-1) for stormwater quality and detention at the Constitution Storage Development. The Constitution Storage Development (Site) is located at 2460 Canada Drive, Colorado Springs within El Paso County, Colorado. The project site is located south of Constitution Avenue, west of Canada Drive, & east of Peterson Road. The site is located in a portion of Section 05, Township 14 South, Range 65 West of the 6<sup>th</sup> Principal Meridian, County of El Paso, State of Colorado.

#### Full Spectrum

Survey data detailing the ~~Water Quality~~ Detention Pond at the site was provided to Galloway & Company, Inc. on October 03, 2024 and updated October 14, 2024 by R & R Engineers-Surveyors, Inc., dated 09/18/2024. The pond was constructed based on the pond design prepared by Galloway, Inc. in the approved Constitution Storage Development Final Drainage Report dated July 21, 2023; Revised November 10, 2023.

#### WQCV Design

The WQCV has a volume of 0.074-acre feet and a depth of 2.50 feet. The WQCV has a 99% drain time of 39 hours which is slightly out of conformance with MHFD Criteria and City of Colorado Springs Criteria.

#### EURV, 5-Year, & 100-Year Design

Per the approved FDR, the EURV and 100-year volumes will be conveyed via the Modified CDOT Type C Outlet structure to the proposed chase outfall (S-40) located at the southeast corner of the site at the Canada Drive ROW (curb & gutter). The proposed development does not increase runoff being discharged from the site, therefore the pond release flows can sufficiently be handled by the proposed conveyance system as designed. Treated / detained runoff is then directed to the existing western curb & gutter within the Canada Drive. Storm events larger than the 100-year storm will overtop the emergency overflow weir and free release into the proposed access drive and southeasterly towards Canada Drive.

Development is no longer proposed. Development did increase runoff, which is why the pond was installed. Consider rewording to clarify.



The water quality volume release will be controlled with an orifice plate that will release over a period of 39 hours. The full spectrum detention pond will release treated flows into the proposed chase outfall (S-40) within the Constitution Storage Development to the proposed chase outfall (S-40) located at the southeast corner of the site at the Canada Drive ROW (curb & gutter).

Total area which will not be treated via the on-site facility is less than 1.0 acre and less than 20%, which of the total site, as required.

### **Miscellaneous**

As-builts were also conducted to verify the construction of the overall pond volume / capacity. The required overall pond volume is 0.416 ac-ft. The as-built pond volume is 0.453 ac-ft, therefore it stands in substantial conformance with the approved plans.

One item to note as an exclusion to this overall pond certification, there are two portions of the concrete trickle channel that were constructed below the minimum slope of 0.40% per the County DCM as reflected on the R & R Engineers-Surveyors, Inc. as-built survey (attached) shown in blue arrows. These two areas still have positive drainage and could be attributed to construction tolerances.

### **Conclusion**

In summary I, Brady Shyrock, a registered professional engineer in the State of Colorado, do hereby affirm, to the best of my knowledge, based on the as-built survey provided by R & R Engineers-Surveyors, Inc. and information provided to date by the general contractor, the Full Spectrum Detention Pond for Constitution Storage Development and associated drainage facilities were constructed in accordance with the design intent of the approved drainage report and construction drawings, and in accordance with local standards and specifications, regional jurisdictional design criteria and state statutes.

The site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common improvements, site grading and paving) meet or exceed the minimum design requirements.

The facilities outlined in this certification letter provide the required WQCV, EURV, 5-Year, & 100-Year detention volumes and will meet the required release rates (as documented by the attached MHFD design form), the stage areas, elevations, and outlet dimensions.

Should you have any further questions, or require additional information, please do not hesitate to contact me at (719) 900-7220.

**GALLOWAY**

Please sign

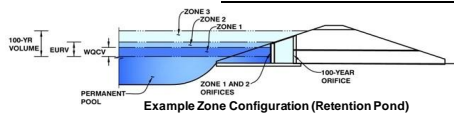
Brady Shyrock, PE  
Project Manager  
[BradyShyrock@GallowayUS.com](mailto:BradyShyrock@GallowayUS.com)

cc: John Radcliffe, PE  
Principal & Regional Office Manager  
[JohnRadcliffe@GallowayUS.com](mailto:JohnRadcliffe@GallowayUS.com)

**Attached Documents:**

- MHFD WQ Detention Pond Calculations
- Approved FDR Drainage Map
- R & R Engineers-Surveyors, Inc. As-Built Survey
- As-Built Drawings

## MHFD-Detention, Version 4.05 (January 2022)

Basin ID: FSD-1

Selected BMP Type =	EDB	
Watershed Area =	3.03	acres
Watershed Length =	520	ft
Watershed Length to Centroid =	225	ft
Watershed Slope =	0.020	ft/ft
Watershed Imperviousness =	73.70%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV)	0.074	acre-feet
Excess Urban Runoff Volume (EURV)	0.287	acre-feet
2-yr Runoff Volume ( $P1 = 1.19$ in.)	0.197	acre-feet
5-yr Runoff Volume ( $P1 = 1.5$ in.)	0.256	acre-feet
10-yr Runoff Volume ( $P1 = 1.75$ in.)	0.304	acre-feet
25-yr Runoff Volume ( $P1 = 2$ in.)	0.362	acre-feet
50-yr Runoff Volume ( $P1 = 2.25$ in.)	0.419	acre-feet
100-yr Runoff Volume ( $P1 = 2.52$ in.)	0.487	acre-feet
500-yr Runoff Volume ( $P1 = 3$ in.)	0.601	acre-feet
Approximate 2-yr Detention Volume	0.188	acre-feet
Approximate 5-yr Detention Volume	0.245	acre-feet
Approximate 10-yr Detention Volume	0.293	acre-feet
Approximate 25-yr Detention Volume	0.350	acre-feet
Approximate 50-yr Detention Volume	0.384	acre-feet
Approximate 100-yr Detention Volume	0.416	acre-feet

Zone 1 Volume (WOCV) =	0.074	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.213	acre-feet
Zone 3 Volume (100-year - Zone 1 & 2) =	0.129	acre-feet
Total Detention Basin Volume =	0.416	acre-feet
Initial Surcharge Volume (ISV) =	user	ft <sup>3</sup>
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth ( $H_{total}$ ) =	user	ft
Depth of Trickle Channel ( $H_{TC}$ ) =	user	ft
Slope of Trickle Channel ( $S_{TC}$ ) =	user	ft/ft
Slopes of Main Basin Sides ( $S_{main}$ ) =	user	H:V
Basin Length-to-Width Ratio ( $R_{L/W}$ ) =	user	

Initial Surcharge Area ( $A_{S1}$ )	=	user	ft <sup>2</sup>
Surcharge Volume Length ( $L_{S1}$ )	=	user	ft
Surcharge Volume Width ( $W_{S1}$ )	=	user	ft
Depth of Basin Floor ( $H_{100R}$ )	=	user	ft
Length of Basin Floor ( $L_{100R}$ )	=	user	ft
Width of Basin Floor ( $W_{100R}$ )	=	user	ft
Area of Basin Floor ( $A_{100R}$ )	=	user	ft <sup>2</sup>
Volume of Basin Floor ( $V_{100R}$ )	=	user	ft <sup>3</sup>
Depth of Main Basin ( $H_{MAIN}$ )	=	user	ft
Length of Main Basin ( $L_{MAIN}$ )	=	user	ft
Width of Main Basin ( $W_{MAIN}$ )	=	user	ft
Area of Main Basin ( $A_{MAIN}$ )	=	user	ft <sup>2</sup>
Volume of Main Basin ( $V_{MAIN}$ )	=	user	ft <sup>3</sup>
Calculated Total Basin Volume ( $V_{S1+MAIN}$ )	=	user	acre-feet

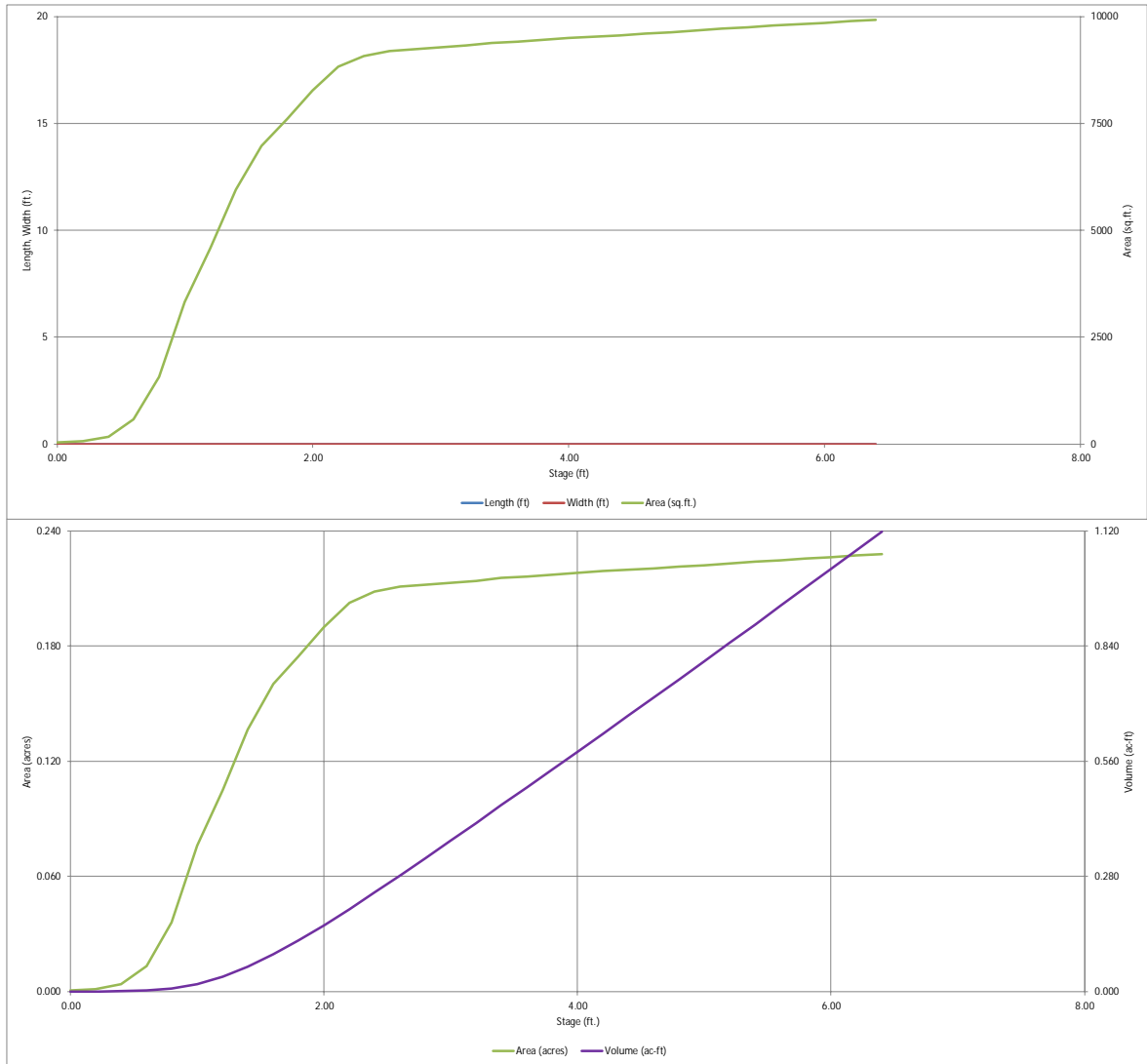
	acre-feet
	acre-feet
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.00	inches

SFB-1 MHFD-Detention v4-05 As-Built 10142024.xlsm, Basin



# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.05 (January 2022)



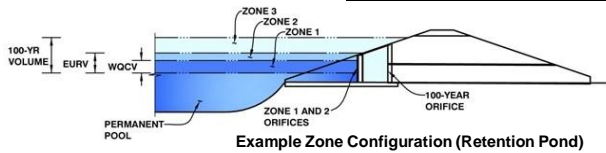
# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)

Project: 6855 Constitution Ave Self Storage

Basin ID: FSD-1

these numbers changed



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WOCV)	1.50	0.074	Orifice Plate
Zone 2 (EURV)	2.63	0.213	Orifice Plate
Zone 3 (100-year)	3.23	0.129	Weir&Pipe (Restrict)
Total (all zones)		0.416	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth = N/A ft (distance below the filtration media surface)  
Underdrain Orifice Diameter = N/A inches

Calculated Parameters for Underdrain  
Underdrain Orifice Area = N/A ft<sup>2</sup>  
Underdrain Orifice Centroid = N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)  
Depth at top of Zone using Orifice Plate = 2.72 ft (relative to basin bottom at Stage = 0 ft)  
Orifice Plate: Orifice Vertical Spacing = N/A inches  
Orifice Plate: Orifice Area per Row = N/A sq. inches

Calculated Parameters for Plate  
WQ Orifice Area per Row = N/A ft<sup>2</sup>  
Elliptical Half-Width = N/A feet  
Elliptical Slot Centroid = N/A feet  
Elliptical Slot Area = N/A ft<sup>2</sup>

these numbers didnt change

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.91	1.58	2.45				
Orifice Area (sq. inches)	0.44	0.60	0.79	0.44				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)  
Depth at top of Zone using Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)  
Vertical Orifice Diameter = Not Selected inches

Calculated Parameters for Vertical Orif  
Vertical Orifice Area = Not Selected ft<sup>2</sup>  
Vertical Orifice Centroid = Not Selected feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Overflow Weir Front Edge Height, H<sub>o</sub> = 3.44 ft (relative to basin bottom at Stage = 0 ft)  
Overflow Weir Front Edge Length = 2.92 feet  
Overflow Weir Gate Slope = 0.00 H:V  
Horiz. Length of Weir Sides = 2.92 feet  
Overflow Gate Type = Type C Gate  
Debris Clogging % = 50%

Calculated Parameters for Overflow W  
Height of Gate Upper Edge, H<sub>u</sub> = 3.44 ft  
Overflow Weir Slope Length = 2.92 feet  
Gate Open Area / 100-yr Orifice Area = 36.57  
Overflow Gate Open Area w/o Debris = 5.93  
Overflow Gate Open Area w/ Debris = 2.97

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 0.00 ft (distance below basin bottom at Stage = 0 ft)  
Outlet Pipe Diameter = 12.00 inches  
Restrictor Plate Height Above Pipe Invert = 3.12 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pl

Outlet Orifice Area = 0.16 ft<sup>2</sup>  
Outlet Orifice Centroid = 0.15 feet  
Half-Central Angle of Restrictor Plate on Pipe = 1.07

PPR2343 called for an 8" pipe. Please confirm

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 3.68 ft (relative to basin bottom at Stage = 0 ft)  
Spillway Crest Length = 20.00 feet  
Spillway End Slopes = 0.00 H:V  
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway  
Spillway Design Flow Depth = 0.29 feet  
Stage at Top of Freeboard = 4.97 feet  
Basin Area at Top of Freeboard = 0.22 acres  
Basin Volume at Top of Freeboard = 0.80 acre-ft

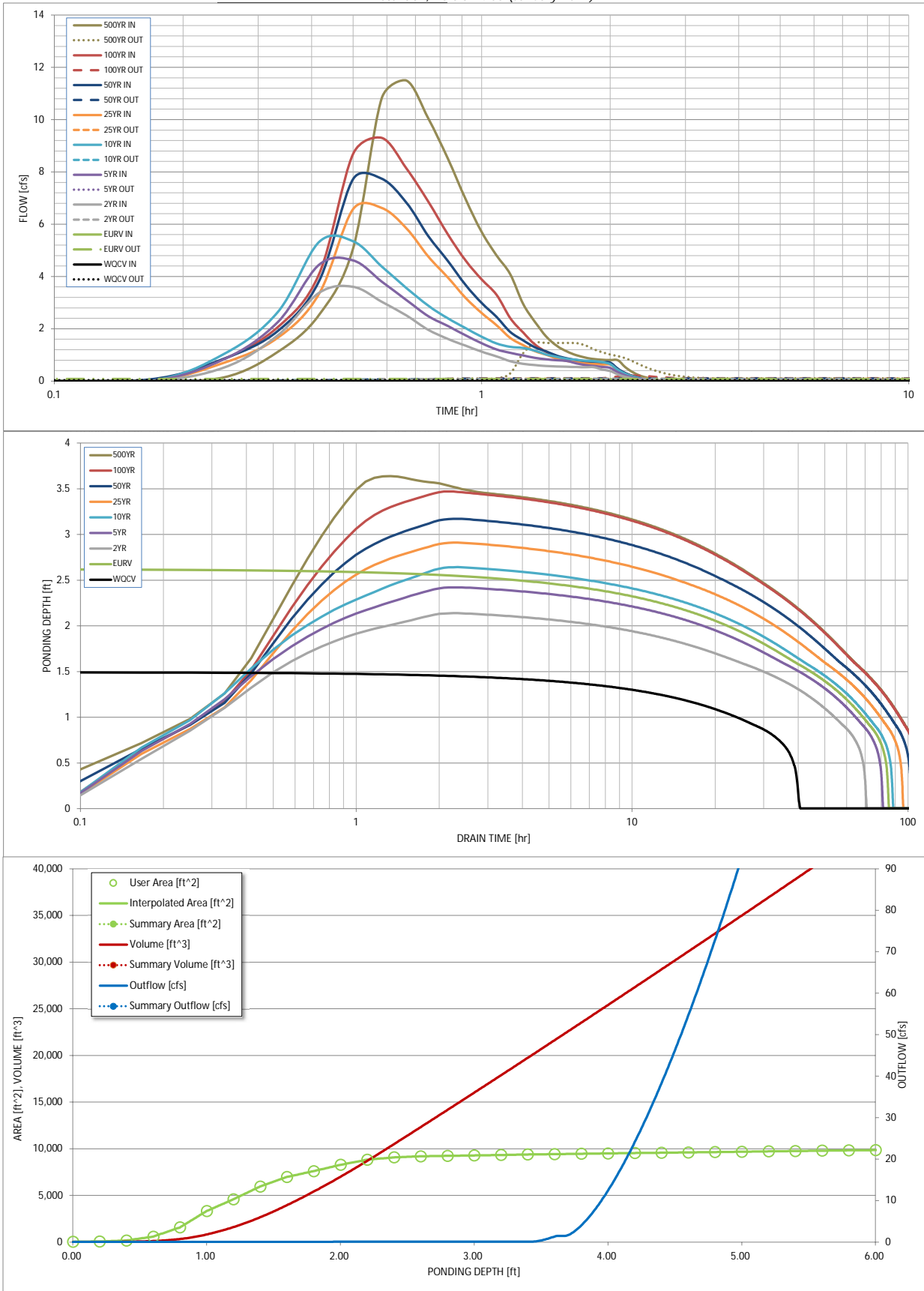
## Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AI)

	WOCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in)	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft)	0.074	0.287	0.197	0.256	0.304	0.362	0.419	0.487
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.197	0.256	0.304	0.362	0.419	0.487
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.0	0.0	0.1	0.6	1.2	2.0
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.01	0.016	0.022	0.20	0.40	0.65
Peak Inflow Q (cfs)	N/A	N/A	3.6	4.6	5.3	6.6	7.7	9.3
Peak Outflow Q (cfs)	0.033	0.083	0.063	0.072	0.084	0.094	0.102	0.209
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	1.5	1.3	0.2	0.1	0.1
Structure Controlling Flow	Plate	Plate	Plate	Plate	Plate	Plate	Plate	Overflow Weir 1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.0
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	38	76	64	73	79	85	91	96
Time to Drain 99% of Inflow Volume (hours)	39	82	68	78	85	92	98	105
Maximum Ponding Depth (ft)	1.50	2.63	2.14	2.42	2.64	2.91	3.17	3.47
Area at Maximum Ponding Depth (acres)	0.15	0.21	0.20	0.21	0.21	0.21	0.21	0.22
Maximum Volume Stored (acre-ft)	0.075	0.289	0.186	0.245	0.291	0.346	0.404	0.468

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

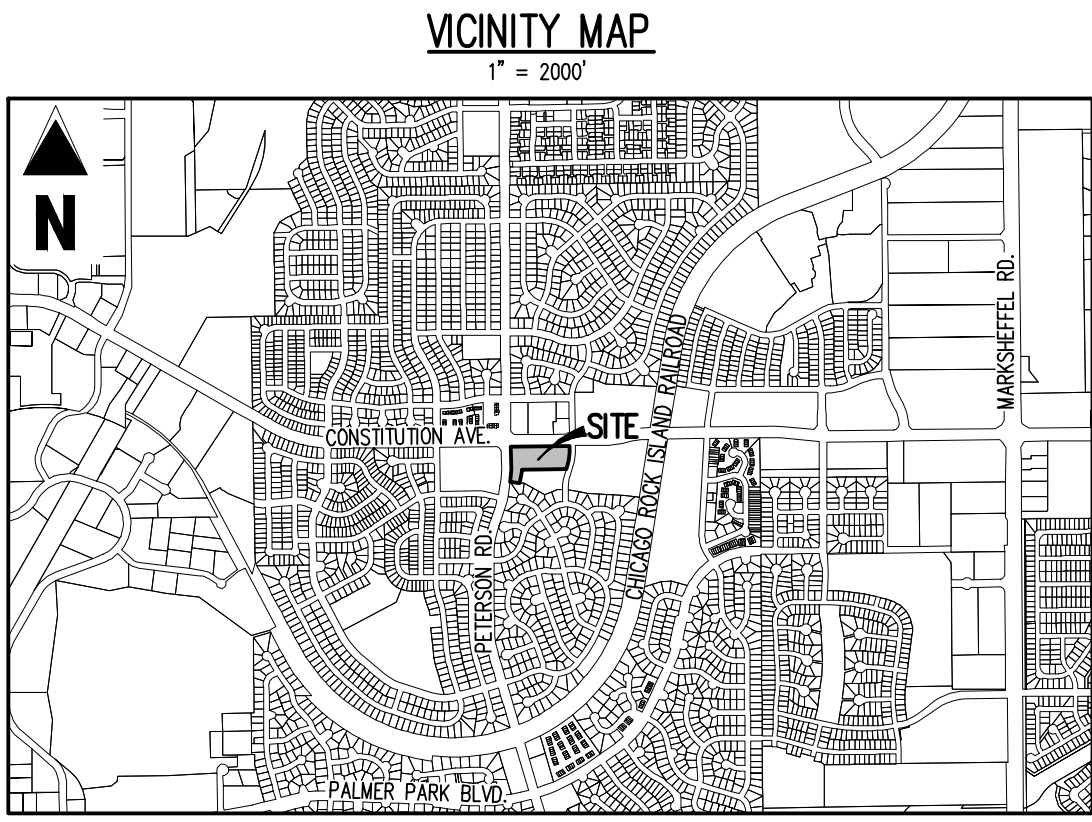
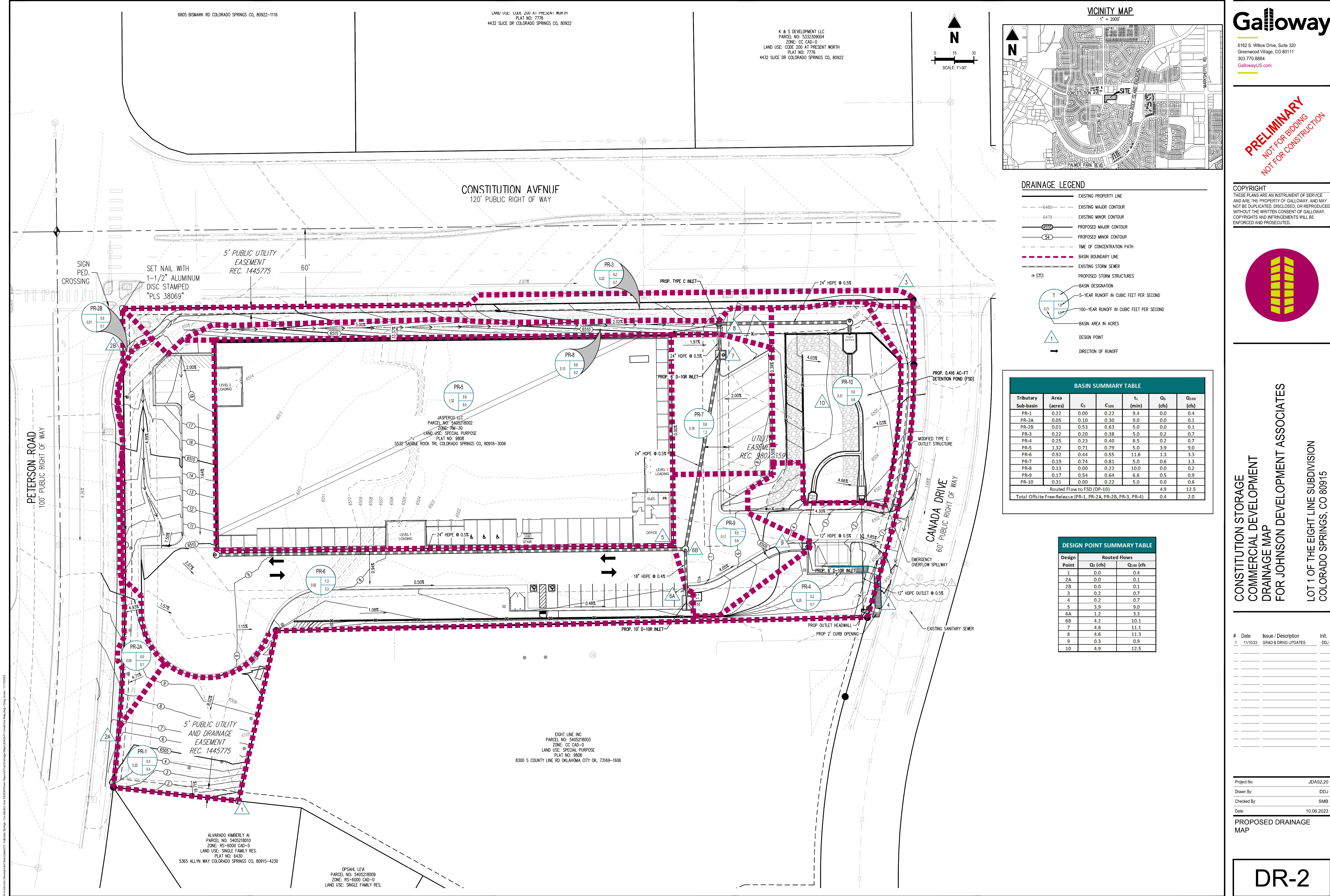
Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.06	0.01	0.15
	0:15:00	0.00	0.00	0.52	0.85	1.05	0.70	0.86	0.85	1.11
	0:20:00	0.00	0.00	1.75	2.26	2.64	1.65	1.91	2.07	2.53
	0:25:00	0.00	0.00	3.37	4.45	5.32	3.34	3.82	4.09	5.07
	0:30:00	0.00	0.00	3.61	4.62	5.35	6.57	7.72	8.69	10.80
	0:35:00	0.00	0.00	3.03	3.81	4.39	6.62	7.74	9.30	11.49
	0:40:00	0.00	0.00	2.51	3.10	3.56	5.84	6.83	8.13	10.06
	0:45:00	0.00	0.00	1.96	2.48	2.88	4.76	5.55	6.87	8.51
	0:50:00	0.00	0.00	1.61	2.11	2.40	3.96	4.60	5.61	6.96
	0:55:00	0.00	0.00	1.35	1.75	2.02	3.18	3.68	4.61	5.70
	1:00:00	0.00	0.00	1.12	1.45	1.70	2.60	2.99	3.89	4.81
	1:05:00	0.00	0.00	0.95	1.21	1.44	2.14	2.45	3.30	4.09
	1:10:00	0.00	0.00	0.76	1.08	1.31	1.65	1.88	2.41	2.96
	1:15:00	0.00	0.00	0.66	0.98	1.27	1.37	1.56	1.86	2.27
	1:20:00	0.00	0.00	0.61	0.89	1.16	1.14	1.29	1.40	1.70
	1:25:00	0.00	0.00	0.58	0.83	1.02	1.00	1.13	1.11	1.34
	1:30:00	0.00	0.00	0.56	0.79	0.92	0.86	0.97	0.94	1.13
	1:35:00	0.00	0.00	0.55	0.77	0.86	0.77	0.87	0.83	0.99
	1:40:00	0.00	0.00	0.54	0.68	0.81	0.71	0.80	0.75	0.90
	1:45:00	0.00	0.00	0.53	0.61	0.78	0.67	0.76	0.70	0.84
	1:50:00	0.00	0.00	0.53	0.57	0.76	0.65	0.73	0.68	0.81
	1:55:00	0.00	0.00	0.44	0.54	0.72	0.63	0.71	0.67	0.80
	2:00:00	0.00	0.00	0.38	0.50	0.65	0.63	0.71	0.67	0.80
	2:05:00	0.00	0.00	0.25	0.34	0.43	0.42	0.47	0.45	0.54
	2:10:00	0.00	0.00	0.17	0.22	0.28	0.28	0.31	0.30	0.35
	2:15:00	0.00	0.00	0.11	0.14	0.18	0.18	0.20	0.19	0.23
	2:20:00	0.00	0.00	0.06	0.09	0.11	0.11	0.12	0.12	0.14
	2:25:00	0.00	0.00	0.04	0.05	0.07	0.07	0.08	0.07	0.09
	2:30:00	0.00	0.00	0.02	0.03	0.04	0.04	0.04	0.04	0.05
	2:35:00	0.00	0.00	0.01	0.01	0.01	0.02	0.02	0.02	0.02
	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00





DRAINAGE LEGEND

- EXISTING PROPERTY LINE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- TIME OF CONCENTRATION PATH
- BASIN BOUNDARY LINE
- EXISTING STORM SEWER
- PROPOSED STORM STRUCTURES
- BASIN DESIGNATION
- 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
- 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
- BASIN AREA IN ACRES
- DESIGN POINT
- DIRECTION OF RUNOFF

BASIN SUMMARY TABLE						
Tributary Sub-basin	Area (acres)	Cs	C100	tc (min)	Qs (cfs)	Q100 (cfs)
PR-1	0.22	0.00	0.22	9.4	0.0	0.4
PR-2A	0.05	0.10	0.30	9.0	0.0	0.1
PR-2B	0.01	0.53	0.63	5.0	0.0	0.1
PR-3	0.22	0.20	0.38	5.0	0.2	0.7
PR-4	0.25	0.23	0.40	8.5	0.2	0.7
PR-5	1.32	0.71	0.79	5.0	3.9	9.0
PR-6	0.92	0.44	0.55	11.6	1.3	3.3
PR-7	0.19	0.74	0.81	5.0	0.6	1.3
PR-8	0.13	0.00	0.22	10.0	0.0	0.2
PR-9	0.17	0.54	0.64	6.6	0.5	0.9
PR-10	0.31	0.00	0.22	5.0	0.0	0.6
Routed Flow to FSD (DP-10)					4.9	12.5
Total Offsite Free-Release (PR-1, PR-2A, PR-2B, PR-3, PR-4)					0.4	2.0

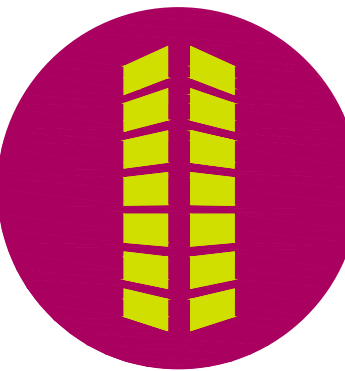
DESIGN POINT SUMMARY TABLE		
Design Point	Routed Flows	
	Qs (cfs)	Q100 (cfs)
1	0.0	0.4
2A	0.0	0.1
2B	0.0	0.1
3	0.2	0.7
4	0.2	0.7
5	3.9	9.0
6A	1.2	3.3
6B	4.2	10.1
7	4.6	11.1
8	4.6	11.3
9	0.3	0.9
10	4.9	12.5

Galloway

6162 S. Willow Drive, Suite 320  
Greenwood Village, CO 80111  
303.770.8884  
gallowayus.com

PRELIMINARY  
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CONSTITUTION STORAGE  
COMMERCIAL DEVELOPMENT  
DRAINAGE MAP  
FOR JOHNSON DEVELOPMENT ASSOCIATES  
LOT 1 OF THE EIGHT LINE SUBDIVISION  
COLORADO SPRINGS, CO 80915

#	Date	Issue / Description	Init.
1	11/10/23	GRAD & DRNG UPDATES	DDJ

Project No: JDA02.20  
Drawn By: DDJ  
Checked By: SMB  
Date: 10.06.2023

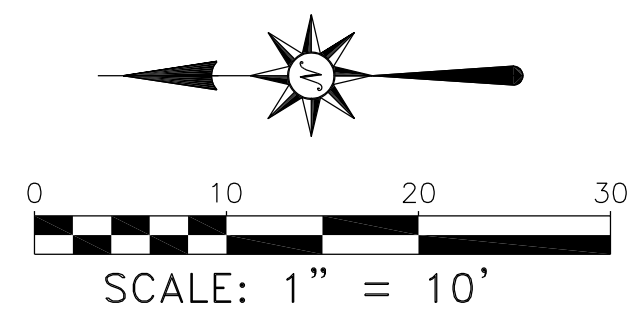
PROPOSED DRAINAGE  
MAP

DR-2



**LEGEND**

- DESIGN ELEVATION
- ELEVATION DIFFERENCE < 0.50'
- ELEVATION DIFFERENCE >= 0.50'
- ADEQUATE SLOPE
- INADEQUATE SLOPE



— 100 Year Volume = 21213.69 Cu Ft = .487 Ac Ft

**ENGINEERS  
SURVEYORS**

# R&R ENGINEERS-SURVEYORS, INC.

1635 W. 13TH AVENUE, SUITE 310

DENVER, COLORADO 80204

PH: 303-753-6730

WWW.RRENGINEERS.COM

## REVISIONS

File No. DC23140 POND

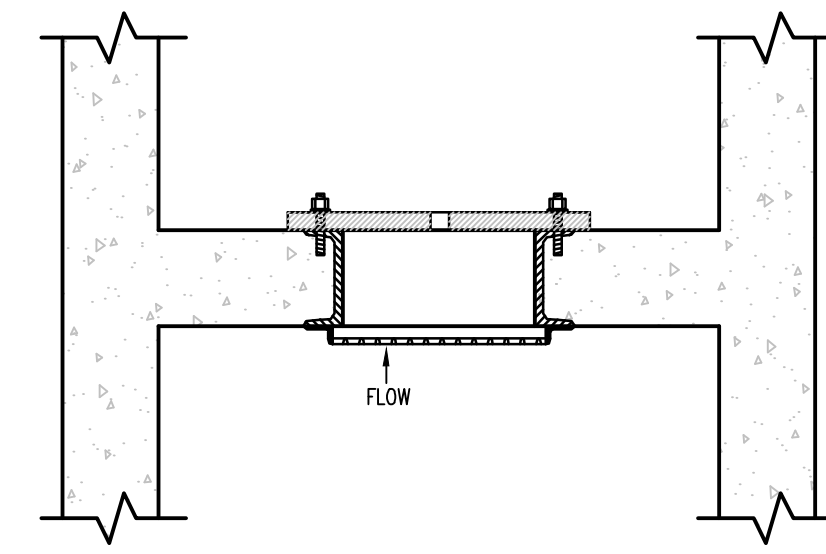
Date Drawn 9/18/24

Drawn By TPF

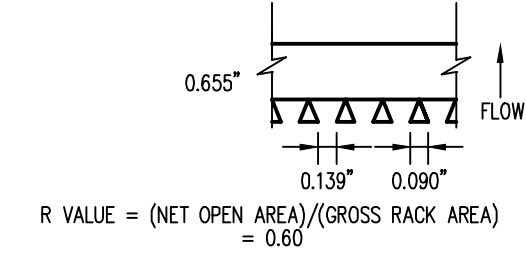
Checked By JDW

Job No.

DC23140



SECTION D-D  
NOT TO SCALE



TRASH RACK SECTION  
SCALE: 1" = 1'

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# PRIVATE PERMANENT CONTROL MEASURE PLAN CONSTITUTION STORAGE

JOHNSON DEVELOPMENT ASSOCIATES

2460 CANADA DRIVE  
COLORADO SPRINGS, CO 80915

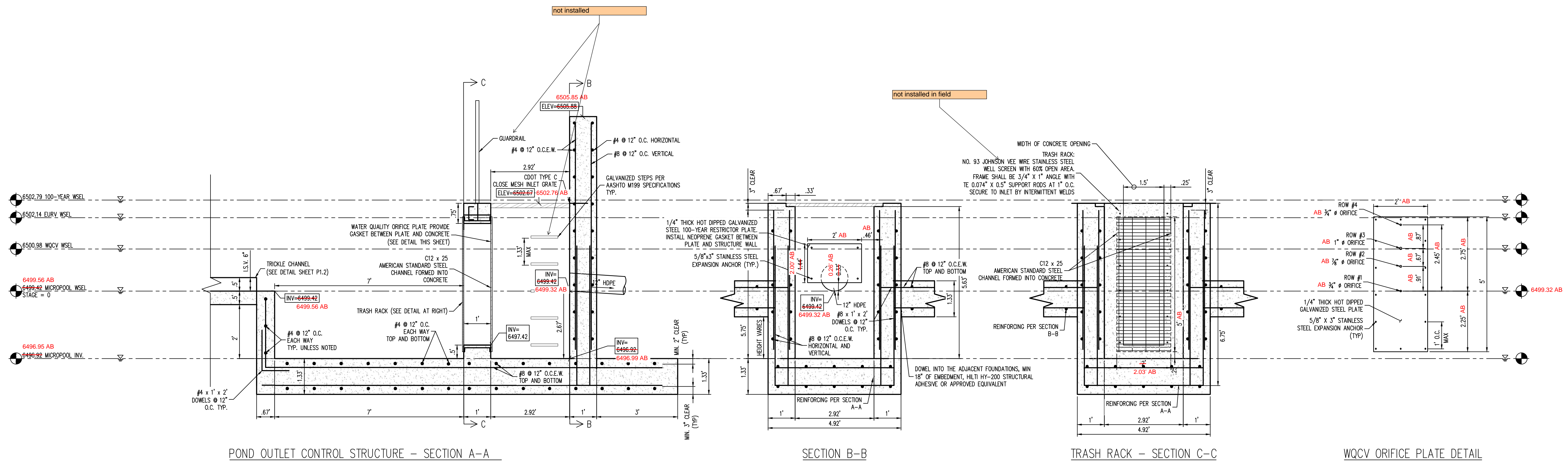
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Project No:	JDA02
Drawn By:	ASA/MRK
Checked By:	BAS
Date:	JULY 2023

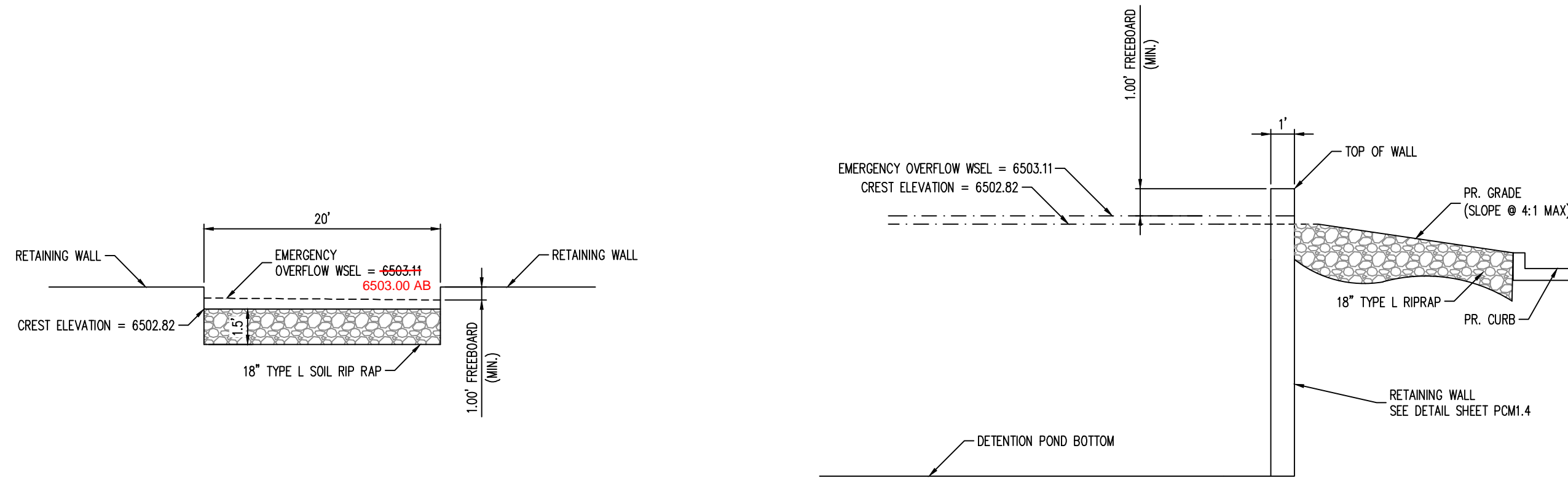
## MICROPOOL & OUTLET STRUCTURE DETAILS

# PCM1.3

Sheet 5 of 6



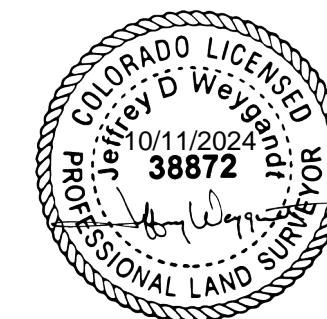
① POND OUTLET STRUCTURE  
SCALE: 1" = 2'



② POND 100-YEAR OVERFLOW - CROSS SECTION  
NOT TO SCALE

③ POND 100-YEAR OVERFLOW - PROFILE  
NOT TO SCALE

**CERTIFICATION**  
I, JEFFREY D. WEYGANDT, A REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF COLORADO, DO HEREBY CERTIFY THAT THE AS-BUILT DIMENSIONS AS SHOWN, MARKED AB, ARE BASED UPON AN ACTUAL SURVEY MADE ON THE GROUND AND THAT IT ACCURATELY REPRESENTS SAID SURVEY TO THE BEST OF MY KNOWLEDGE AND BELIEF AND IS IN ACCORDANCE WITH THE APPLICABLE STANDARDS OF PRACTICE. THE FIELD WORK WAS PERFORMED UNDER MY DIRECT SUPERVISION BETWEEN SEPTEMBER 13, 2024 AND OCTOBER 11, 2024.  
THIS CERTIFICATION IS NOT A GUARANTY OR WARRANTY, EITHER EXPRESSED OR IMPLIED.

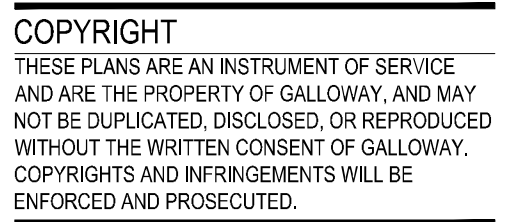


JEFFREY D. WEYGANDT, PLS  
COLORADO REG. NO. 38872  
FOR, AND ON BEHALF OF:  
R&R ENGINEERS-SURVEYORS, INC.  
1635 W. 13TH AVENUE, #310  
DENVER, CO 80204  
(303) 753-6730









## JOHNSON DEVELOPMENT ASSOCIATES

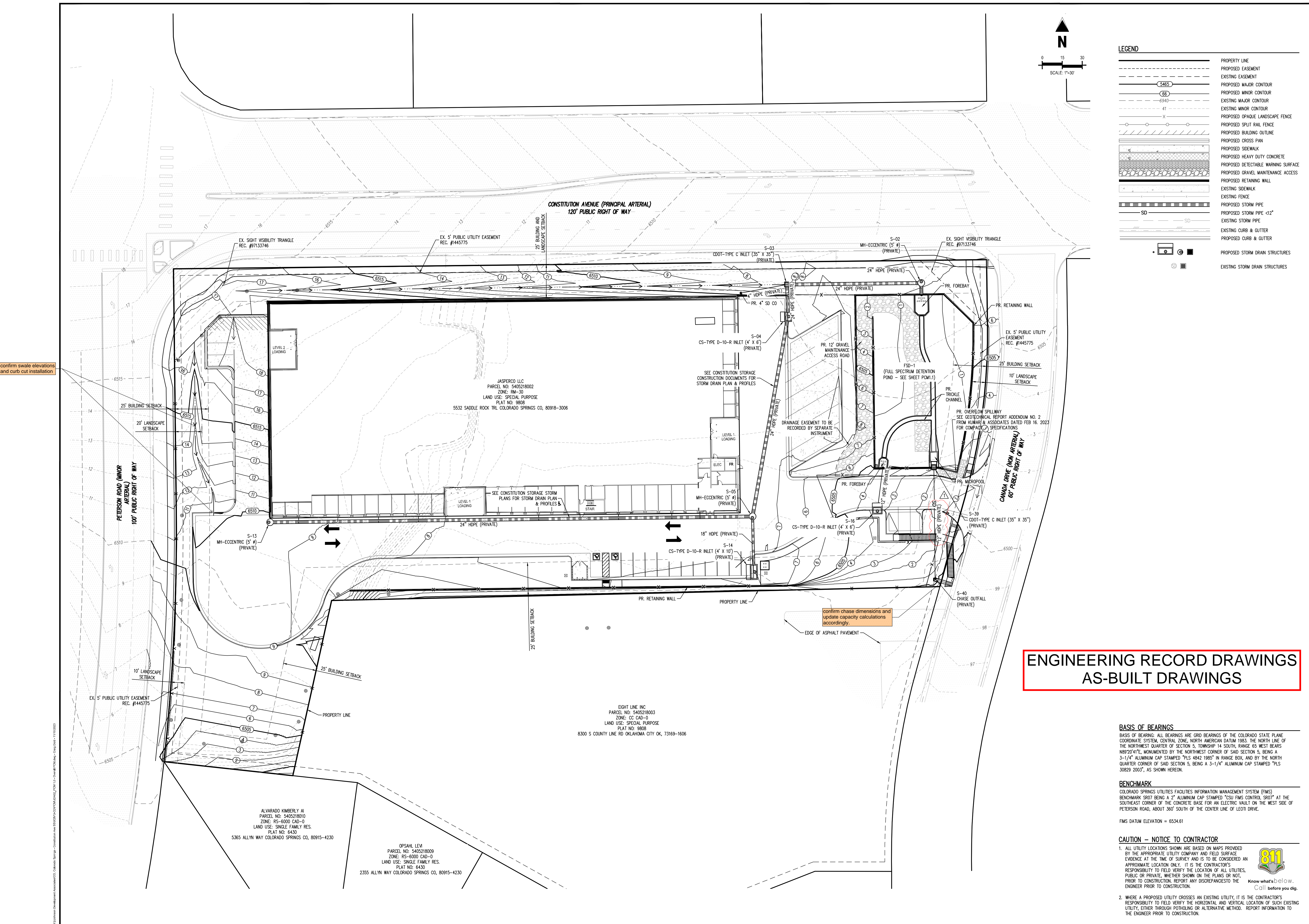
2460 CANADA DRIVE  
COLORADO SPRINGS, CO 80915

#	Date	Issue / Description	Init.
1	11/10/2023	GRADING & DRAINAGE UPDATES	JDM
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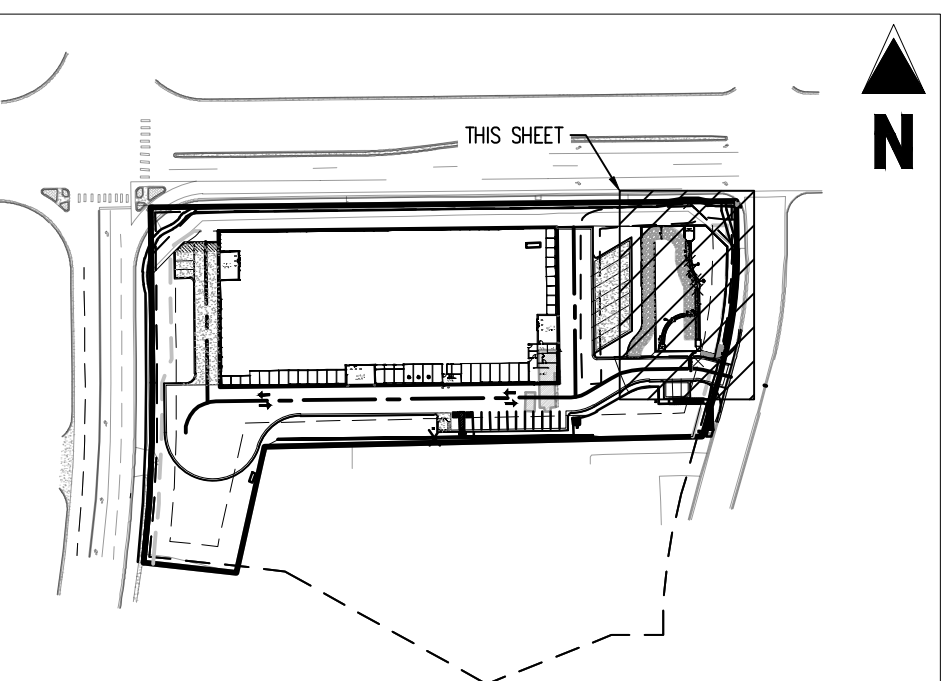
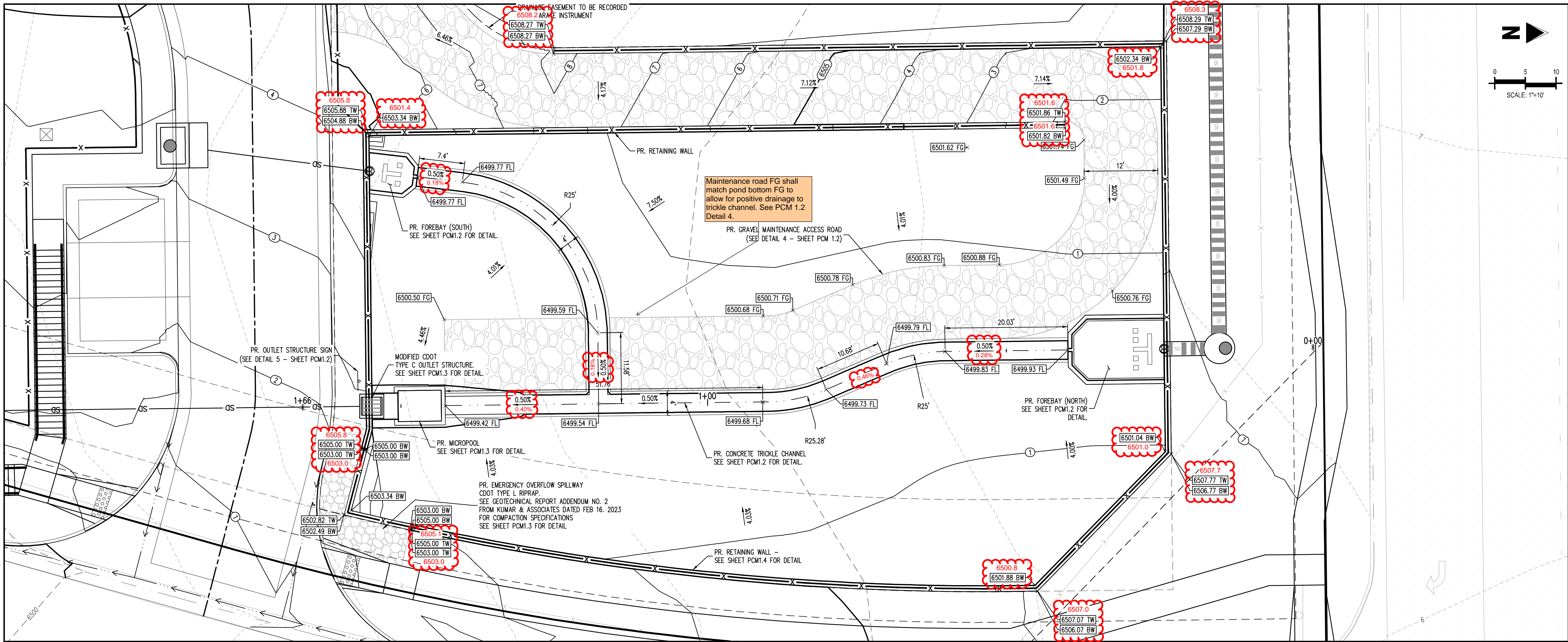
Project No:	JDA02
Drawn By:	ASA/MRK
Checked By:	BAS
Date:	10/15/2024

## OVERALL PCM PLAN

PCM1.0  
Sheet 2 of 6

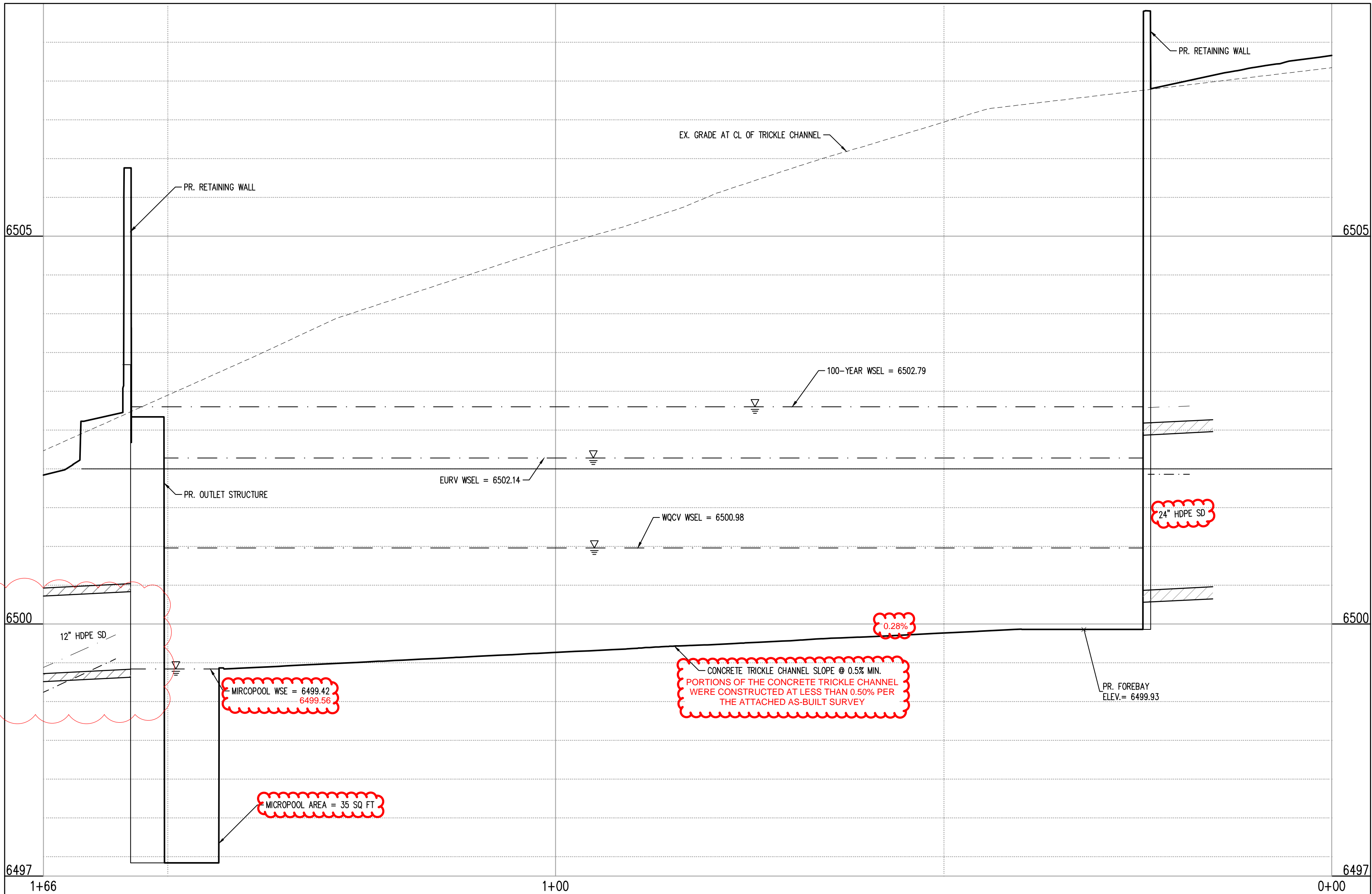






LEGEND	
	PROPERTY LINE
	PROPOSED EASEMENT
	EXISTING EASEMENT
	PROPOSED MAJOR CONTOUR
	PROPOSED MINOR CONTOUR
	EXISTING MAJOR CONTOUR
	EXISTING MINOR CONTOUR
	PROPOSED OPAQUE LANDSCAPE FENCE
	PROPOSED SPLIT RAIL FENCE
	PROPOSED BUILDING OUTLINE
	PROPOSED CROSS PAN
	PROPOSED SIDEWALK
	PROPOSED HEAVY DUTY CONCRETE
	PROPOSED DETECTABLE WARNING SURFACE
	PROPOSED GRAVEL MAINTENANCE ACCESS
	PROPOSED RETAINING WALL
	EXISTING SIDEWALK
	EXISTING FENCE
	PROPOSED STORM PIPE
	PROPOSED STORM PIPE <12"
	EXISTING STORM PIPE
	EXISTING CURB & GUTTER
	PROPOSED CURB & GUTTER
	PROPOSED STORM DRAIN STRUCTURES
	EXISTING STORM DRAIN STRUCTURES

## ENGINEERING RECORD DRAWINGS AS-BUILT DRAWINGS



NOTE: SEE APPROVED DEVELOPMENT PLAN FOR THE PROPOSED LANDSCAPING PLANS FOR LANDSCAPING WITHIN THE PERMANENT BMP AREA.

NOTE: CONTRACTOR SHALL PROTECT ALL EXISTING SURVEY MONUMENTATION. CONTRACTOR SHALL HAVE LICENSED SURVEYOR REPLACE ANY DAMAGED OR DISTURBED MONUMENTATION AT THEIR COST.

NOTE: CONTRACTOR RESPONSIBLE FOR AS-BUILT DRAWINGS, TESTS, REPORTS, AND/OR ANY OTHER CERTIFICATES OF INFORMATION AS REQUIRED FOR ACCEPTANCE OF WORK FROM CITY, UTILITY DISTRICTS OR ANY OTHER GOVERNING AGENCY.

- NOTES:**
- REFER TO CONSTITUTION STORAGE PRIVATE IMPROVEMENT PLANS FOR ALL STORM PIPE, INLET, MANHOLE AND APPURTENANCE INFORMATION.
  - ALL STORM PIPE SHALL BE RCP CLASS 3 MINIMUM OR HDPE IN ACCORDANCE WITH CITY STANDARDS AND SPECIFICATIONS, UNLESS OTHERWISE NOTED. TRENCH BEDDING SHALL BE IN ACCORDANCE WITH CITY SPECIFICATIONS.
  - ALL STORMWATER FACILITIES SHOWN ON THESE PLANS ARE ASSUMED PRIVATE UNLESS OTHERWISE NOTED.
  - WALLS EXCEEDING 4' (MEASURED FROM BOTTOM OF FOOTING TO TOP OF WALL) MAY REQUIRE A BUILDING PERMIT.
  - EXISTING VEGETATION CONSISTS OF NATURAL GRASSES AND SHRUBS.
  - SOILS WITHIN THE SITE ARE TRUCKTON SANDY LOAM.
  - BOTTOM OF POND TO HAVE A MINIMUM SLOPE OF 3% TO THE TRICKLE CHANNEL AND MICROPOOL.

**BASIS OF BEARINGS**

BASIS OF BEARING: ALL BEARINGS ARE GRID BEARINGS OF THE COLORADO STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NORTH AMERICAN DATUM 1983. THE NORTH LINE OF THE NORTHWEST QUARTER OF SECTION 5, TOWNSHIP 14 SOUTH, RANGE 65 WEST BEARS N89°20'41"E, MONUMENTED BY THE NORTHWEST CORNER OF SAID SECTION 5, BEING A 3-1/4" ALUMINUM CAP STAMPED "PLS 4842 1985" IN RANGE BOX, AND BY THE NORTH QUARTER CORNER OF SAID SECTION 5, BEING A 3-1/4" ALUMINUM CAP STAMPED "PLS 30829 2003", AS SHOWN HEREON.

**BENCHMARK**

COLORADO SPRINGS UTILITIES FACILITIES INFORMATION MANAGEMENT SYSTEM (FIMS) BENCHMARK SR07 BEING A 2" ALUMINUM CAP STAMPED "CSU FIMS CONTROL SR07" AT THE SOUTHEAST CORNER OF THE CONCRETE BASE FOR AN ELECTRIC VAULT ON THE WEST SIDE OF PETERSON ROAD, ABOUT 360' SOUTH OF THE CENTER LINE OF LEOTI DRIVE.

FIMS DATUM ELEVATION = 6534.61

- CAUTION - NOTICE TO CONTRACTOR**
- ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY AND IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE LOCATION OF ALL UTILITIES, PUBLIC OR PRIVATE, WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
  - WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF SUCH EXISTING UTILITY, EITHER THROUGH POT-HOLING OR ALTERNATIVE METHOD. REPORT INFORMATION TO THE ENGINEER PRIOR TO CONSTRUCTION.

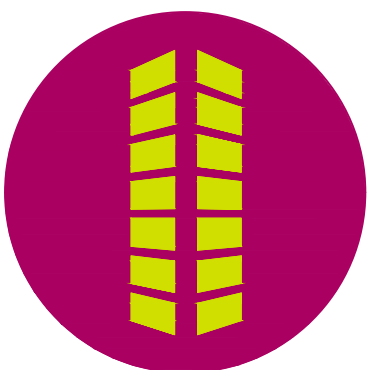


**Galloway**

1155 Kelly Johnson Blvd., Suite 305  
Colorado Springs, CO 80920  
719.900.7220  
gallowayus.com



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PRIVATE PERMANENT CONTROL MEASURE PLAN  
CONSTITUTION STORAGE  
JOHNSON DEVELOPMENT ASSOCIATES  
2460 CANADA DRIVE  
COLORADO SPRINGS, CO 80915

#	Date	Issue / Description	Init.
1	11/10/2023	GRADING & DRAINAGE UPDATES	JDM
2			
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Project No: JDA02  
Drawn By: ASA/MRK  
Checked By: BAS  
Date: 10/15/2024

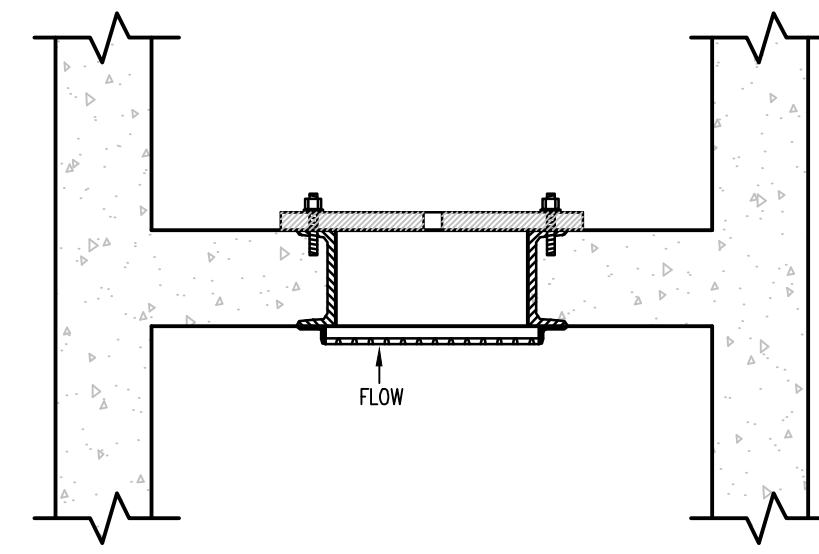
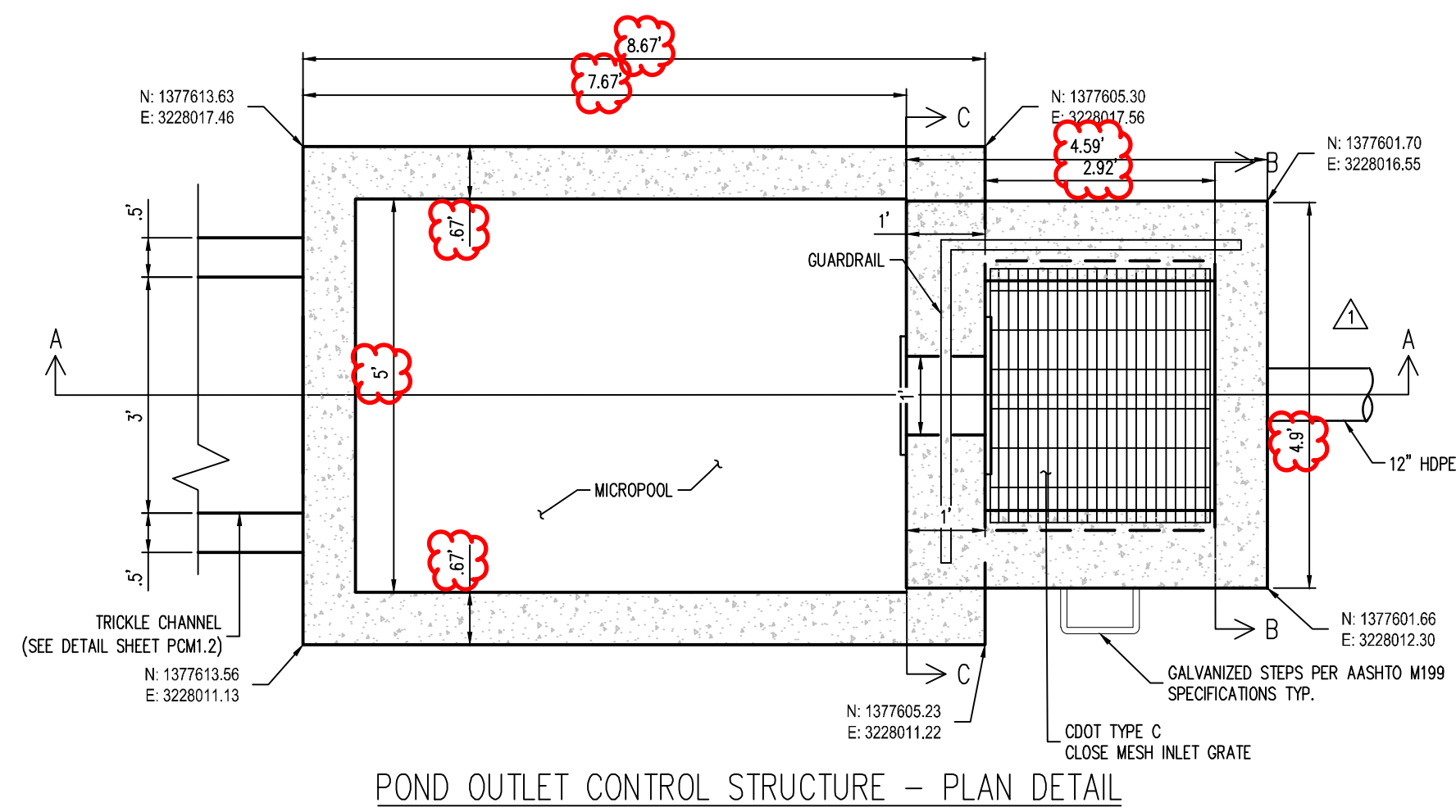
POND PLAN

**PCM1.1**  
Sheet 3 of 6

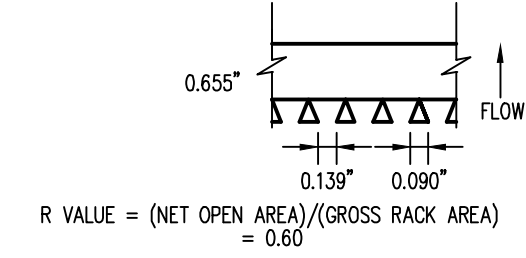




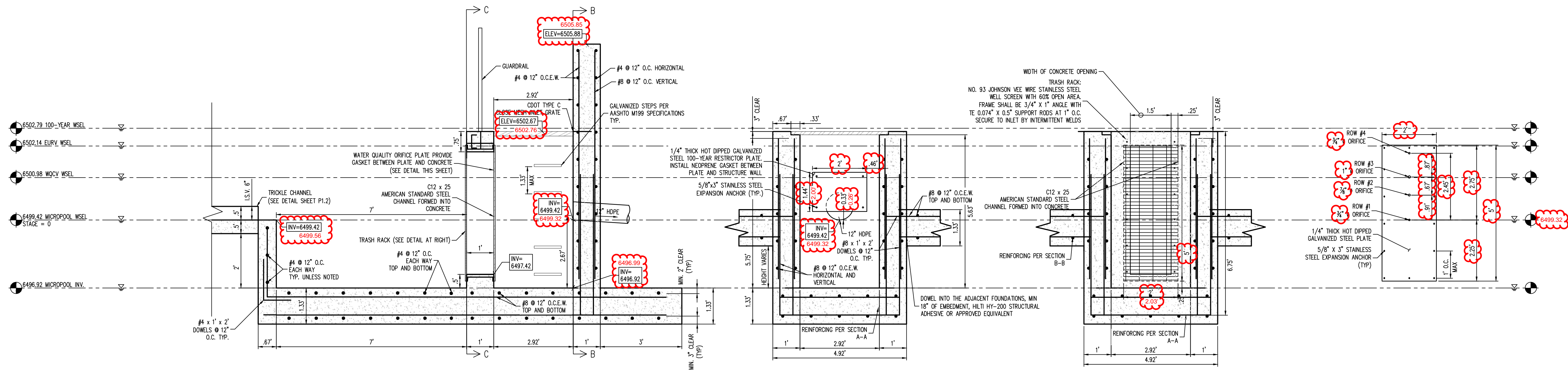




SECTION D-D  
NOT TO SCALE



TRASH RACK SECTION  
SCALE: 1" = 1'



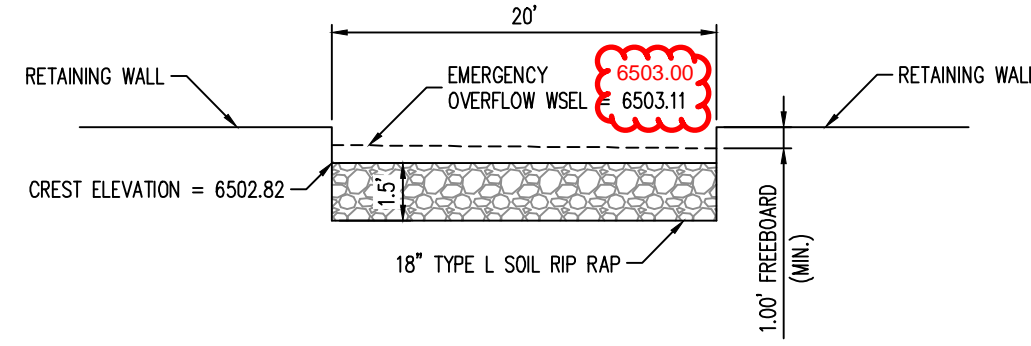
POND OUTLET CONTROL STRUCTURE - SECTION A-A

SECTION B-B

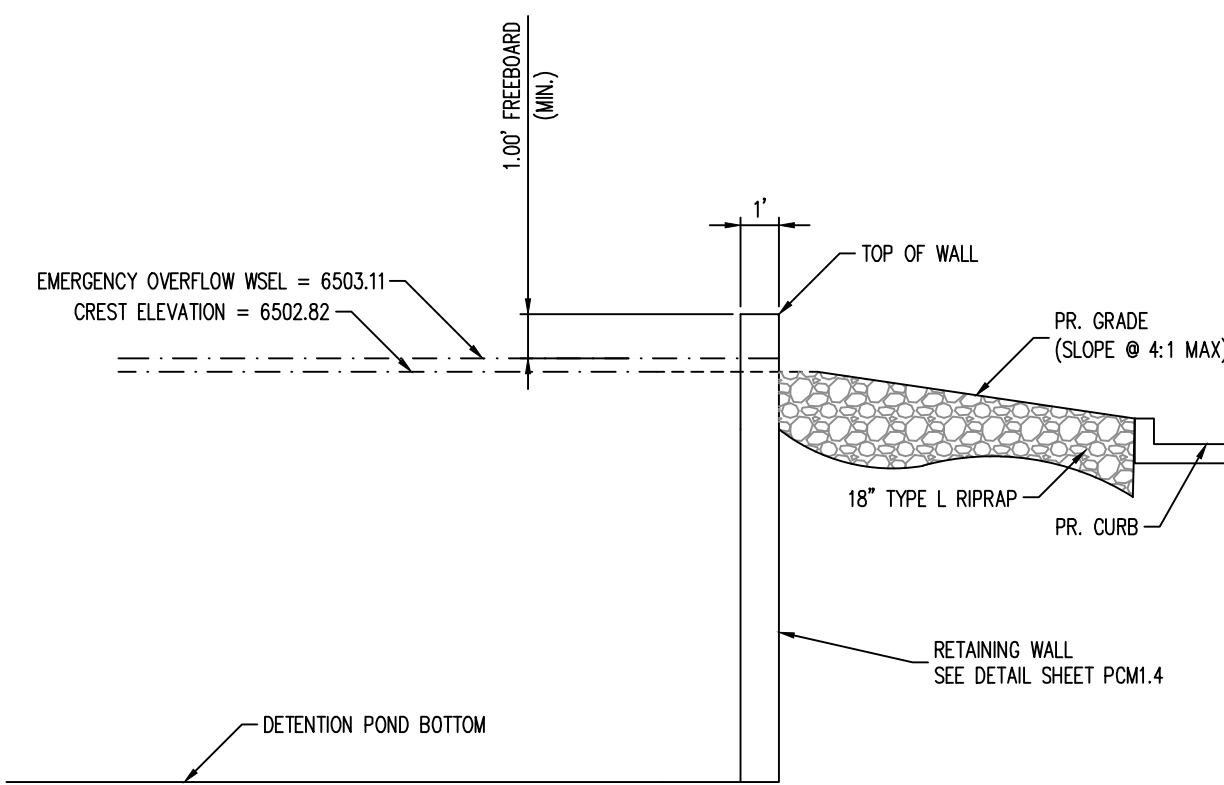
TRASH RACK - SECTION C-C

WQCV ORIFICE PLATE DETAIL

1 POND OUTLET STRUCTURE  
SCALE: 1" = 2'



2 POND 100-YEAR OVERFLOW - CROSS SECTION  
NOT TO SCALE



3 POND 100-YEAR OVERFLOW - PROFILE  
NOT TO SCALE



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PRIVATE PERMANENT CONTROL MEASURE PLAN  
CONSTITUTION STORAGE

JOHNSON DEVELOPMENT ASSOCIATES

2460 CANADA DRIVE  
COLORADO SPRINGS, CO 80915

#	Date	Issue / Description	Init.
1	11/10/2023	GRADING & DRAINAGE UPDATES	JDM
2	01/31/2024	GRADING LABEL UPDATES	CRD
3			
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Project No: JDA02  
Drawn By: ASA/MRK  
Checked By: RAS  
Date: 10/15/2024

MICROPOOL & OUTLET  
STRUCTURE DETAILS

ENGINEERING RECORD DRAWINGS  
AS-BUILT DRAWINGS



